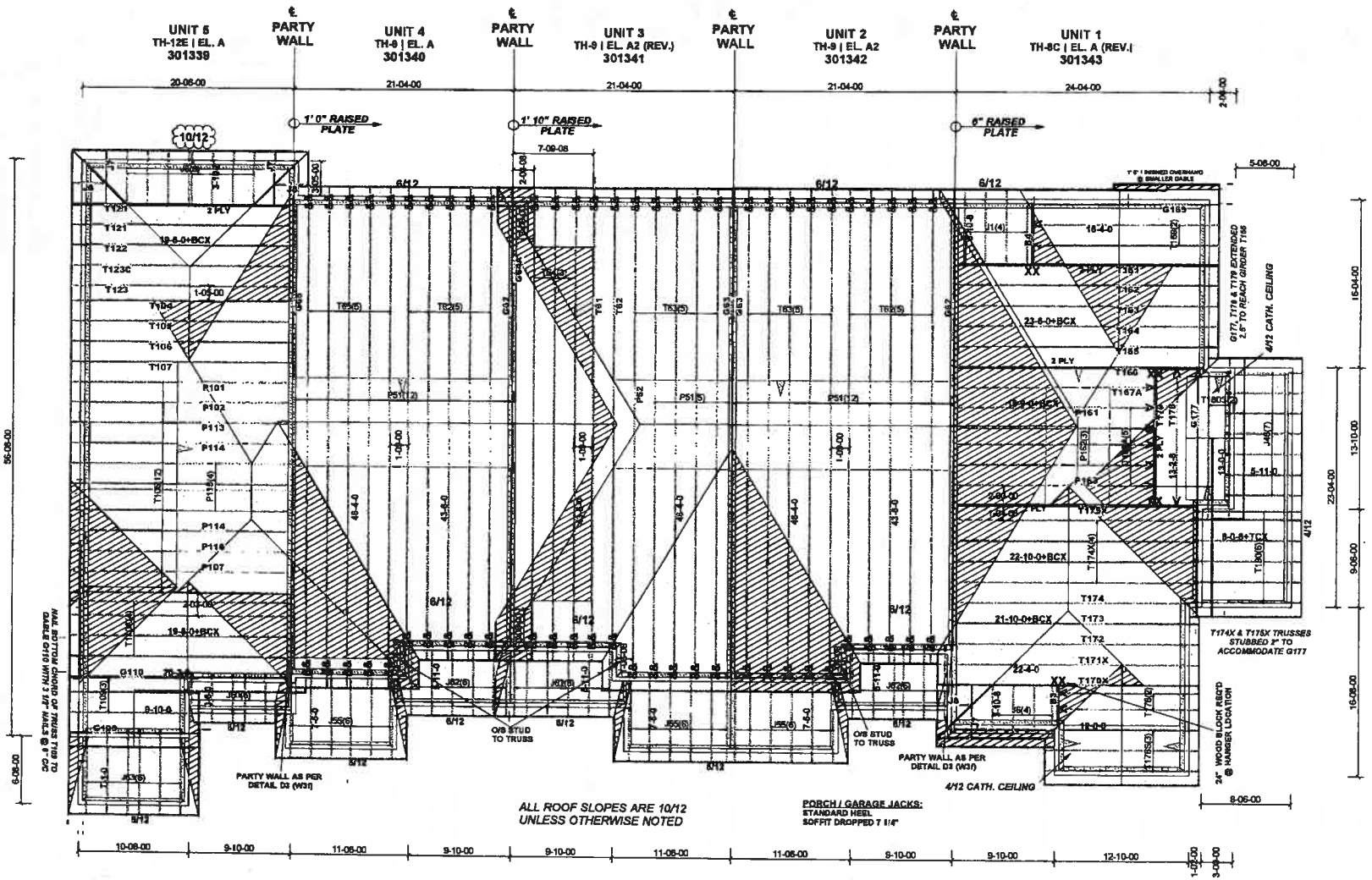


ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE (BRICK)
2" CLADDING ALLOWANCE (STUCCO)
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

T - 180523

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 / 2016
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=54.3psf Sr=8.4psf
DESIGN LOADS
TCDL (8psf)
BCLL (10.5psf)
BCDL (7psf)

DENOTES:
CONVENTIONAL FRAMING

HARDWARE:
LIS26DS - (V)
HGUS26 - (X)
HGUS26-2 - (XX)
TS22 x2 - (##)
TBE6 x2 - (&&)

BEAMS:
B3, B4: 2 - 2x10 SPF #2

TAMARACK	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **BLOCK 145 / UNITS 1-5**
	Layout ID: **297084**	Project: **ALCONA SHORES**	
	Plan Log: **97625**	Date: 10/17/2018 Designer: colinh	

Mitek ver 7.5.0

TAMARACK LUMBER INC
ALPA LUMBER GROUP

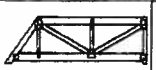
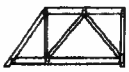
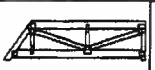
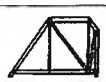
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301339 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-12E ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T104 HALF HIP	10.00 0.00	19-08-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	113.32 71.83		
	1	T105 HALF HIP	10.00 0.00	19-08-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	109.23 69.00		
	13	T106 PIGGYBACK	10.00 0.00	19-08-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	1600.17 994.50		
	1	T107 HALF HIP	10.00 0.00	19-08-00	10-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	135.18 84.50		
	3	T109 COMMON	10.00 0.00	09-10-00	05-08-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.13 87.99		
	1	G109 COMMON	10.00 0.00	09-10-00	05-08-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	47.92 32.33		
	1	G110 COMMON	10.00 0.00	20-02-00	10-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.59 69.00		
	4	T110A COMMON	10.00 0.00	19-08-00	10-00-08	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	452.12 280.00		
	1 2 Ply	T120 HIP GIRDER	10.00 0.00	19-08-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	203.18 132.66		
	1	T121 HIP	10.00 0.00	19-08-00	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	107.24 67.83		
	1	T122 HIP	10.00 0.00	19-08-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	109.81 70.50		
	1	T123 COMMON	10.00 0.00	19-08-00	10-00-08	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	113.03 70.00		
	1	T123C COMMON	10.00 0.00	19-08-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	112.77 69.33		
	1	P101 PIGGYBACK	10.00 0.00	10-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	34.80 23.67		
	1	P102 PIGGYBACK	10.00 0.00	10-00-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	36.12 23.83		
	1	P107 PIGGYBACK	10.00 0.00	10-00-15	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-06-08	33.88 22.00		
	1	P113 PIGGYBACK	10.00 0.00	10-00-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-11-15	41.15 26.50		
	2	P114 PIGGYBACK	10.00 0.00	10-00-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-11-15	86.78 56.34		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

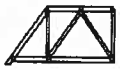
Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301339 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-12E ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	P115 PIGGYBACK	10.00 0.00	10-00-15	05-10-12	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-11-15	166.20 106.68		
	1	P116 PIGGYBACK	10.00 0.00	10-00-15	03-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-11-00	36.84 24.50		
	8	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	121.84 77.36		
	2	J7 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	21.30 14.66		
	2	J8 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	16.74 12.00		
	6	J60 JACK-OPEN	6.00 0.00	03-06-00	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-03-00	65.58 43.98		
	6	J63 JACK-OPEN	6.00 0.00	07-01-00	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 04-00-08	141.36 94.02		

TOTAL # TRUSS= **66.00** TOTAL BFT OF ALL TRUSSES= **2625.01** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4146.28** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301340 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-9 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T62 PIGGYBACK	6.00 0.00	43-06-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1395.65 819.15		
	1	G62 PIGGYBACK	6.00 0.00	43-06-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	273.26 170.00		
	5	T65 PIGGYBACK	6.00 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-07-11	1489.05 894.15		
	1	G65 PIGGYBACK	6.00 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-07-11	290.87 182.50		
	12	P51 PIGGYBACK	6.00 0.00	06-09-10	02-00-09	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	225.96 135.96		
	6	J55 JACK-OPEN	5.00 0.00	07-05-00	03-06-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 00-04-07	138.12 90.00		
	6	J62 JACK-OPEN	6.00 0.00	05-11-00	03-06-04	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 03-05-08	113.22 73.02		

TOTAL # TRUSS= **36.00** TOTAL BFT OF ALL TRUSSES= **2364.78** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3926.13** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= **40.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301341 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-9 ELEVATION: A2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T61 HIP	6.00 / 0.00	43-06-00	06-02-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	256.36 / 152.50		
	1	T62 PIGGYBACK	6.00 / 0.00	43-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	279.13 / 163.83		
	5	T63 PIGGYBACK	6.00 / 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-11-02	1460.85 / 883.35		
	1	G63 PIGGYBACK	6.00 / 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 03-01-10	290.23 / 182.00		
	3	T64 ROOF	6.00 / 0.00	43-05-00	10-02-08	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	819.27 / 498.99		
	1	G64A COMMON	6.00 / 0.00	36-02-00	10-02-08	2 X 6	2 X 6	00-00-00 / 00-00-00	01-02-00 / 01-02-00	209.59 / 133.00		
	5	P51 PIGGYBACK	6.00 / 0.00	06-09-10	02-00-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	94.15 / 56.65		
	1	P52 PIGGYBACK	6.00 / 0.00	06-09-10	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	20.13 / 13.00		
	6	J55 JACK-OPEN	5.00 / 0.00	07-05-00	03-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 00-04-07	138.12 / 90.00		
	6	J62 JACK-OPEN	6.00 / 0.00	05-11-00	03-06-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-06-12 / 03-05-08	113.22 / 73.02		

TOTAL # TRUSS= **30.00** TOTAL BFT OF ALL TRUSSES= **2246.34** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3681.05** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= **40.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301342 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-9 ELEVATION: A2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T62 PIGGYBACK	6.00 / 0.00	43-06-00	09-11-09	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1395.65 / 819.15		
	1	G62 PIGGYBACK	6.00 / 0.00	43-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	273.26 / 170.00		
	5	T63 PIGGYBACK	6.00 / 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-11-02	1460.85 / 883.35		
	1	G63 PIGGYBACK	6.00 / 0.00	46-04-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 03-01-10	290.23 / 182.00		
	12	P51 PIGGYBACK	6.00 / 0.00	06-09-10	02-00-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	225.96 / 135.96		
	6	J55 JACK-OPEN	5.00 / 0.00	07-05-00	03-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 00-04-07	138.12 / 90.00		
	6	J62 JACK-OPEN	6.00 / 0.00	05-11-00	03-06-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-06-12 / 03-05-08	113.22 / 73.02		

TOTAL # TRUSS= **36.00** TOTAL BFT OF ALL TRUSSES= **2353.48** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3897.29** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= **40.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301343 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-8C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T161 HIP GIRDER	10.00 0.00	23-06-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	230.00 146.00		
	1	T162 HIP	10.00 0.00	23-06-00	05-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	114.57 71.67		
	1	T163 HIP	10.00 0.00	23-06-00	06-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	125.55 80.83		
	1	T164 HIP	10.00 0.00	23-06-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	126.63 79.17		
	1	T165 HIP	10.00 0.00	23-06-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	129.75 81.67		
	1 2 Ply	T166 HIP GIRDER	10.00 0.00	23-06-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	275.36 172.34		
	1	T167A HIP	10.00 0.00	18-09-00	10-01-04	2 X 4	2 X 6	00-00-00 00-00-00	05-07-03 00-05-08	122.13 76.83		
	5	T168A PIGGYBACK	10.00 0.00	18-09-00	09-01-04	2 X 4	2 X 6	00-00-00 00-00-00	05-07-03 00-05-08	570.60 363.35		
	2	T169 COMMON	10.00 0.00	16-04-00	08-05-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	168.64 106.66		
	1	G169 COMMON	10.00 0.00	16-04-00	08-05-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	81.98 53.33		
	1	T170X HIP GIRDER	10.00 0.00	22-04-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	119.47 76.33		
	1	T171X HIP	10.00 0.00	22-04-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	103.75 65.67		
	1	T172 HIP	10.00 0.00	21-10-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	118.93 76.33		
	1	T173 HIP	10.00 0.00	21-10-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	129.69 81.67		
	1	T174 COMMON	10.00 0.00	21-10-00	10-11-06	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	124.27 77.33		
	4	T174X COMMON	10.00 0.00	22-10-00	10-11-06	2 X 4	2 X 6	00-00-00 00-00-00	00-09-11 00-05-08	534.12 324.00		
	1 2 Ply	T175X COMMON	10.00 0.00	22-10-00	10-11-06	2 X 6	2 X 6	00-00-00 00-00-00	00-09-11 00-05-08	288.72 178.66		
	2	T176 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.22 74.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/24/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301343 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-8C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T176S SCISSOR	10.00 4.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	176.22 113.01		
	1	G177 ROOF	10.00 4.00	22-08-08	11-00-03	2 X 4	2 X 4	00-00-00 01-03-08	01-05-10 01-07-11	155.66 100.83		
	1	T178 COMMON	10.00 0.00	13-02-08	11-00-03	2 X 4	2 X 4	00-00-00 00-00-00	01-05-10 09-06-11	81.82 52.33		
	1 2 Ply	T179 COMMON	10.00 0.00	13-02-08	11-00-03	2 X 6	2 X 6	00-00-00 00-00-00	01-05-10 09-06-11	233.26 141.00		
	2	T180S SCISSOR	10.00 4.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	125.24 80.66		
	6	T190 MONOPITCH	4.00 0.00	08-11-00	03-03-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 03-00-02	181.50 118.02		
	1	P161 PIGGYBACK	10.00 0.00	04-09-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	16.20 12.00		
	3	P162 PIGGYBACK	10.00 0.00	04-09-15	02-04-15	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	47.88 29.01		
	1	P163 PIGGYBACK	10.00 0.00	04-09-15	01-09-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	15.98 10.67		
	4	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	67.16 42.68		
	4	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	60.92 38.68		
	1	J7 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	10.65 7.33		
	1	J8 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	8.37 6.00		
	7	J46 JACK-OPEN	4.00 0.00	05-11-00	02-03-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-03-10	122.01 75.81		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **3013.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4783.25** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
13	Hangers	LJS26DS	
4	Hangers	HGUS26-2	

TOTAL # ITEMS= **17.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T61	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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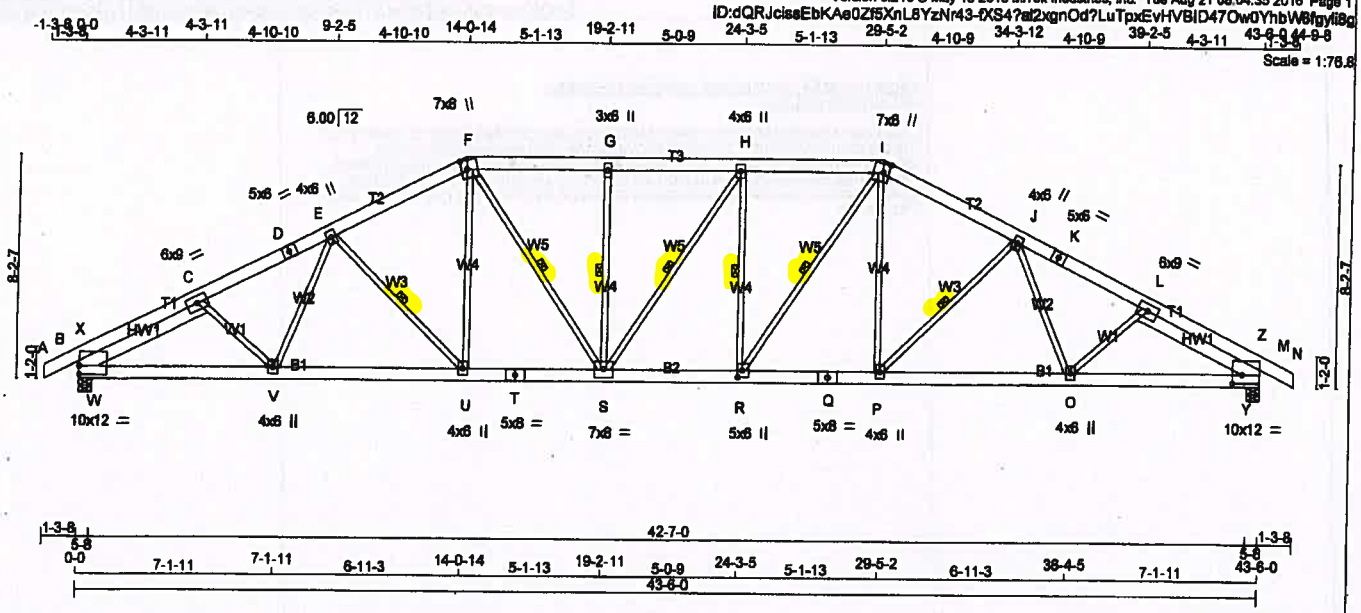

TOTAL WEIGHT = 256 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - N	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TMWW-t	MT20	6.0	9.0		
D	TS-t	MT20	5.0	6.0		
E, H, J						
E	TMWW+t	MT20	4.0	6.0		
F	TTWW+m	MT20	7.0	6.0	4.00	2.50
G	TMW+w	MT20	3.0	6.0		
I	TTWW+m	MT20	7.0	6.0	4.00	2.50
K	TS-t	MT20	5.0	6.0		
L	TMWW-t	MT20	6.0	9.0		
M	TMBMW1-I	MT20	10.0	12.0	3.75	4.50
O, P, U, V						
O	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	6.0	3.00	2.00
S	BMWWW-t	MT20	7.0	6.0		
T	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4781	0	5107	217	-1054	5-8	5-8 2 TBEG
M	4781	0	5107	0	-1054	5-8	5-8 2 TBEG

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1054 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1054 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 217 LBS FACTORED HORIZONTAL REACTION AT JOINT B ok

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3437	2615 / 0	457 / 0	0 / 0	181 / -1127	582 / 0	0 / 0
M	3437	2615 / 0	457 / 0	0 / 0	67 / -1127	582 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	155 / -155	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-U, F-S, G-S, H-S, H-R, I-R, J-P.

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B- X	-5576 / 1015	-170.4	-170.4	0.38 (2)	3.40
X- C	-3235 / 739	-170.4	-170.4	0.30 (2)	4.44
C- D	-7357 / 1633	-170.4	-170.4	0.64 (2)	2.73
D- E	-7357 / 1633	-170.4	-170.4	0.64 (2)	2.73
E- F	-6415 / 1531	-170.4	-170.4	0.57 (2)	3.02
F- G	-6336 / 1573	-170.4	-170.4	0.55 (2)	3.08
G- H	-6336 / 1574	-170.4	-170.4	0.54 (2)	3.08
H- I	-6350 / 1579	-170.4	-170.4	0.56 (3)	3.05
I- J	-6413 / 1530	-170.4	-170.4	0.57 (3)	3.02
J- K	-7358 / 1634	-170.4	-170.4	0.64 (3)	2.73
K- L	-7358 / 1634	-170.4	-170.4	0.64 (3)	2.73
L- Z	-3233 / 740	-170.4	-170.4	0.30 (3)	4.44
Z- M	-5573 / 1016	-170.4	-170.4	0.39 (3)	3.40
M- N	0 / 2	-170.4	-170.4	0.13 (3)	10.00
B- W	-736 / 2867	-38.5	-38.5	0.14 (2)	6.25
W- V	-1480 / 6513	-38.5	-38.5	0.38 (2)	6.25
V- U	-1398 / 6556	-38.5	-38.5	0.37 (2)	6.25
U- T	-1101 / 5709	-38.5	-38.5	0.33 (2)	6.25
T- S	-1101 / 5709	-38.5	-38.5	0.33 (2)	6.25
S- R	-1177 / 6350	-38.5	-38.5	0.35 (3)	6.25
R- Q	-936 / 5707	-38.5	-38.5	0.32 (3)	6.25
Q- P	-936 / 5707	-38.5	-38.5	0.32 (3)	6.25
P- O	-1182 / 6555	-38.5	-38.5	0.37 (3)	6.25
O- Y	-1265 / 6514	-38.5	-38.5	0.36 (3)	6.25
Y- M	-522 / 2868	-38.5	-38.5	0.14 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 598	0.13 (3)
V- E	-290 / 231	0.14 (10)
E- U	-1222 / 426	0.43 (2)
U- F	-236 / 1156	0.27 (13)
F- S	-423 / 2016	0.45 (3)
S- G	-1117 / 335	0.43 (2)
S- H	-545 / 484	0.31 (3)
R- H	-1526 / 420	0.59 (2)
R- I	-434 / 2051	0.48 (2)
P- I	-230 / 1136	0.26 (14)
P- J	-1225 / 427	0.43 (3)
J- O	-277 / 241	0.14 (9)
O- L	0 / 597	0.13 (2)
W- X	-386 / 2860	0.00 (1)
W- C	-4535 / 923	0.88 (2)
L- Y	-4536 / 924	0.88 (3)
Y- Z	-388 / 2857	0.00 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	51.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	75.3	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/180 (2.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.64/1.00 (J-L:3) , BC=0.38/1.00 (V-W:2) , WB=0.88/1.00 (L-Y:3) , SSI=0.37/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.99 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 18005384
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T61	1	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 18052384
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T62	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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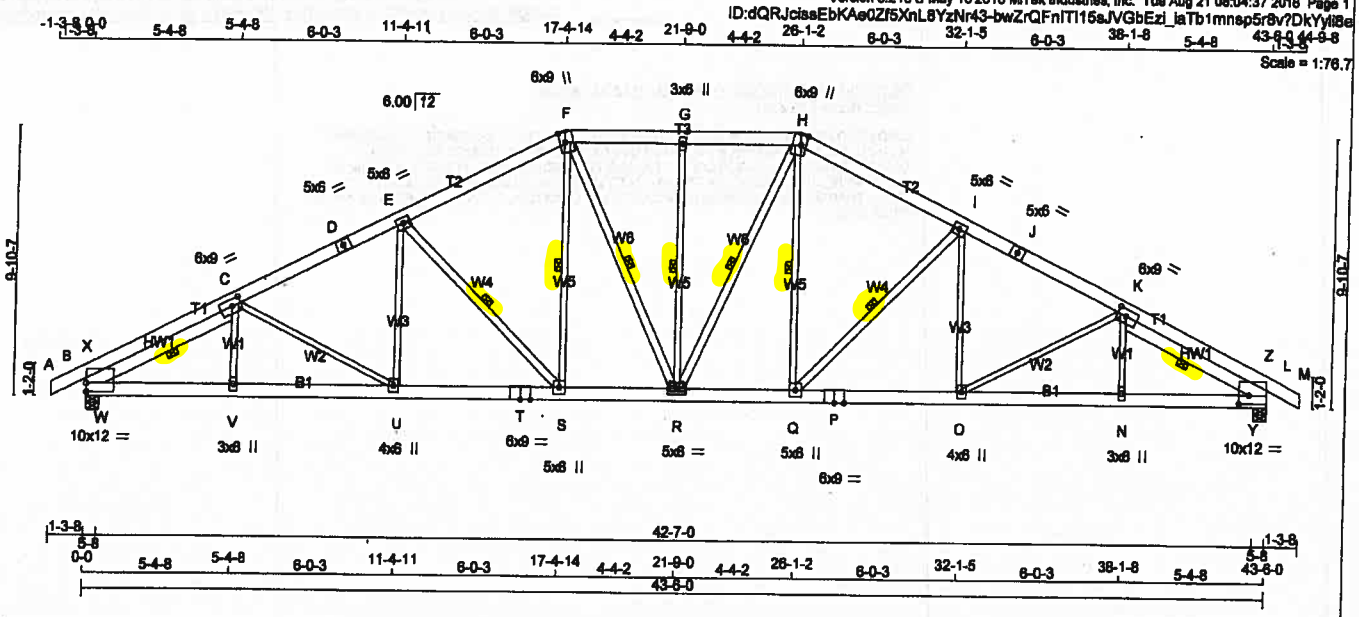

TOTAL WEIGHT = 11 X 279 = 3071 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-t	MT20	10.0	12.0	3.75	
C	TMWWW-t	MT20	6.0	8.0	3.00	4.00
D	TS-t	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	8.0		
F	TTWW+m	MT20	8.0	8.0		Edge 2.25
G	TMW+w	MT20	3.0	8.0		
H	TTWW+m	MT20	8.0	9.0		Edge 2.25
I	TMWW-t	MT20	5.0	8.0		
J	TS-t	MT20	5.0	8.0		
K	TMWWW-t	MT20	8.0	8.0	3.00	4.00
L	TMBMW1-t	MT20	10.0	12.0	3.75	4.50
N	BMW+w	MT20	3.0	8.0		
O	BMWW+t	MT20	4.0	8.0		
P	BS-t	MT20	8.0	8.0		
Q	BMWW+t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	5.0	8.0		
T	BS-t	MT20	8.0	9.0		
U	BMWW+t	MT20	4.0	8.0		
V	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4781	0	4880	263	-1011	5-8	5-8 2-TBEC
L	4781	0	4880	0	-1011	5-8	5-8 2-TBEC

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1011 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1011 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 263 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS
1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3437	2464 / 0	457 / 0	0 / 0	158 / -1098	582 / 0	0 / 0
L	3437	2464 / 0	457 / 0	0 / 0	93 / -1098	582 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	188 / -188	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W, K-Y.

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)		10.00
B- X	-5414 / 1026	-170.4	-170.4	0.45 (2)		3.36
X- C	-3232 / 788	-170.4	-170.4	0.38 (2)		4.34
C- D	-6793 / 1502	-170.4	-170.4	0.84 (2)		2.64
D- E	-6793 / 1502	-170.4	-170.4	0.84 (2)		2.64
E- F	-5686 / 1347	-170.4	-170.4	0.73 (2)		3.09
F- G	-5290 / 1302	-170.4	-170.4	0.34 (2)		3.55
G- H	-5290 / 1302	-170.4	-170.4	0.34 (2)		3.55
H- I	-5686 / 1347	-170.4	-170.4	0.73 (3)		3.09
I- J	-6793 / 1503	-170.4	-170.4	0.84 (3)		2.64
J- K	-6793 / 1503	-170.4	-170.4	0.84 (3)		2.64
K- Z	-3232 / 797	-170.4	-170.4	0.36 (3)		4.34
Z- L	-5414 / 1026	-170.4	-170.4	0.45 (3)		3.36
L- M	0 / 2	-170.4	-170.4	0.13 (3)		10.00
B- W	-821 / 2681	-38.5	-38.5	0.15 (1)		8.25
W- V	-1468 / 6417	-38.5	-38.5	0.35 (2)		8.25
V- U	-1470 / 6414	-38.5	-38.5	0.37 (1)		8.25
U- T	-1280 / 6101	-38.5	-38.5	0.33 (1)		8.25
T- S	-1280 / 6101	-38.5	-38.5	0.33 (1)		8.25
S- R	-876 / 5054	-38.5	-38.5	0.27 (1)		8.25
R- Q	-780 / 5054	-38.5	-38.5	0.27 (1)		8.25
Q- P	-997 / 8101	-38.5	-38.5	0.33 (1)		8.25
P- O	-997 / 8101	-38.5	-38.5	0.33 (1)		8.25
O- N	-1208 / 6414	-38.5	-38.5	0.37 (1)		8.25
N- Y	-1207 / 6417	-38.5	-38.5	0.35 (3)		8.25
Y- L	-557 / 2681	-38.5	-38.5	0.15 (1)		8.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 257	0.06 (17)
C- U	-570 / 267	0.50 (2)
U- E	-42 / 545	0.12 (2)
E- S	-1840 / 560	0.68 (2)
S- F	-335 / 1520	0.34 (2)
F- R	-288 / 1359	0.22 (3)
R- G	-1047 / 313	0.62 (2)
R- H	-286 / 1359	0.22 (2)
Q- H	-335 / 1520	0.34 (3)
Q- I	-1840 / 560	0.68 (3)
O- I	-43 / 545	0.12 (3)
O- K	-570 / 261	0.50 (3)
N- K	0 / 257	0.06 (17)
W- X	-327 / 2678	0.00 (1)
W- C	-4351 / 785	0.55 (1)
K- Y	-4351 / 787	0.55 (1)
Y- Z	-328 / 2678	0.00 (1)

DWG NO. TAM 118005395
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/180 (2.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.84/1.00 (C-E:2) , BC=0.37/1.00 (U-V:1) , WB=0.68/0.90 (E-S:2) , SSI=0.40/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.91 (T) (INPUT = 1.00)

½

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T62	11	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. [illegible]
PROVINCE OF ONTARIO

DESIGN BY TAM 1805345
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G62	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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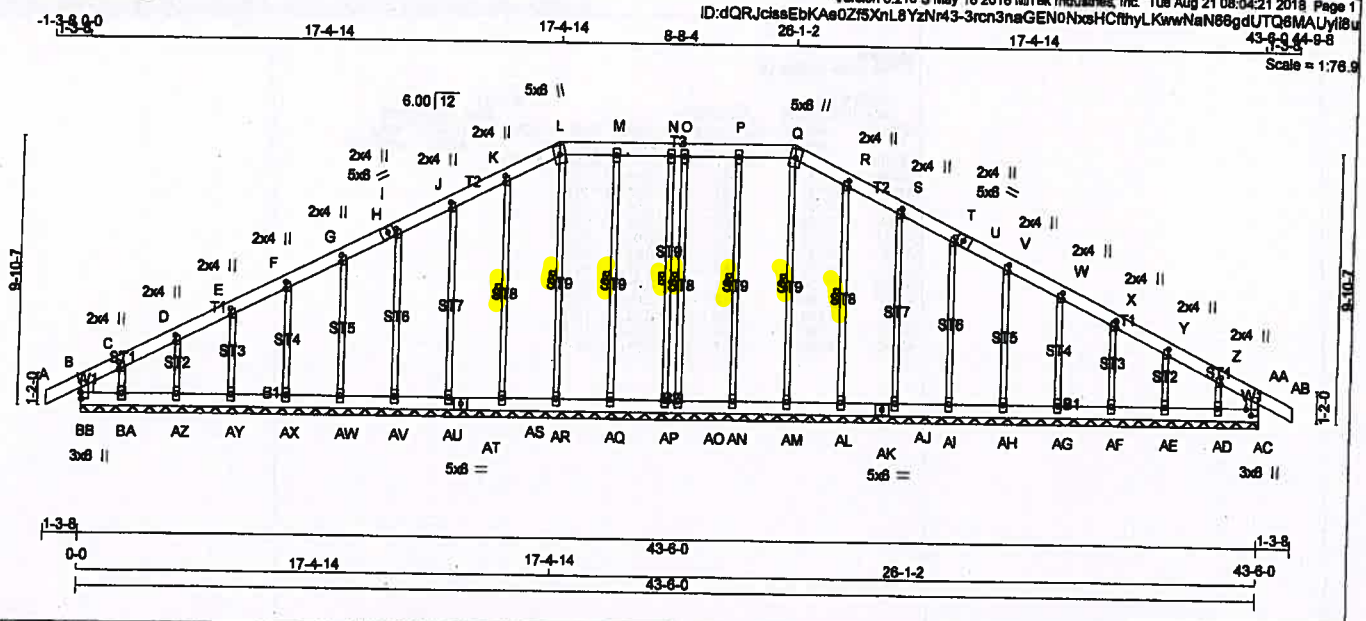

TOTAL WEIGHT = 2 X 273 = 548 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BB- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - Q	2x6	DRY	No.2	SPF
Q - U	2x6	DRY	No.2	SPF
U - AB	2x6	DRY	No.2	SPF
AC- AA	2x6	DRY.	No.2	SPF
BB- AT	2x6	DRY	No.2	SPF
AT- AK	2x6	DRY	No.2	SPF
AK- AC	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, AA, AC, BB						
B						
C, D, E, R, S, T, V, W, X, Y, Z						
C	TMW+w	MT20	2.0	4.0	2.75	
F, G, I, J, K						
F	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	8.0	2.50	2.75
L	TTW+m	MT20	5.0	8.0		
M, N, O, P						
M	TMW+w	MT20	3.0	6.0		
Q	TTW+m	MT20	5.0	8.0		
U	TS-t	MT20	5.0	8.0	2.50	2.75
AC	TMBMV1+p	MT20	3.0	8.0	2.75	2.25
AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AR, AS, AU, AV, AW, AX, AY, AZ, BA						
AD	BMW1+w	MT20	3.0	8.0		
AK	BS-t	MT20	5.0	8.0		
AT	BS-t	MT20	5.0	8.0		
BB	TMBMV1+p	MT20	3.0	8.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF Q-AM, P-AN, O-AO, N-AP, M-AQ, K-AS, R-AL, L-AR.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
BB- B	-369 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
B- C	-96 / 0	-129.7	-129.7	0.08 (1)	8.25	
C- D	-49 / 0	-129.7	-129.7	0.03 (1)	8.25	
D- E	-43 / 0	-129.7	-129.7	0.03 (1)	8.25	
E- F	-34 / 0	-129.7	-129.7	0.03 (1)	8.25	
F- G	-27 / 0	-129.7	-129.7	0.03 (1)	8.25	
G- H	-22 / 0	-129.7	-129.7	0.03 (1)	8.25	
H- I	-22 / 0	-129.7	-129.7	0.03 (1)	8.25	
I- J	-17 / 0	-129.7	-129.7	0.03 (1)	8.25	
J- K	-13 / 0	-129.7	-129.7	0.03 (1)	8.25	
K- L	-10 / 0	-129.7	-129.7	0.03 (1)	10.00	
L- M	-8 / 0	-129.7	-129.7	0.03 (1)	10.00	
M- N	-8 / 0	-129.7	-129.7	0.04 (1)	10.00	
N- O	-8 / 0	-129.7	-129.7	0.04 (1)	10.00	
O- P	-8 / 0	-129.7	-129.7	0.04 (1)	10.00	
P- Q	-8 / 0	-129.7	-129.7	0.03 (1)	10.00	
Q- R	-10 / 0	-129.7	-129.7	0.03 (1)	10.00	
R- S	-13 / 0	-129.7	-129.7	0.03 (1)	8.25	
S- T	-17 / 0	-129.7	-129.7	0.03 (1)	8.25	
T- U	-22 / 0	-129.7	-129.7	0.03 (1)	8.25	
U- V	-22 / 0	-129.7	-129.7	0.03 (1)	8.25	
V- W	-27 / 0	-129.7	-129.7	0.03 (1)	8.25	
W- X	-34 / 0	-129.7	-129.7	0.03 (1)	8.25	
X- Y	-43 / 0	-129.7	-129.7	0.03 (1)	8.25	
Y- Z	-48 / 0	-129.7	-129.7	0.03 (1)	8.25	
Z-AA	-96 / 0	-129.7	-129.7	0.08 (1)	8.25	
AA-AB	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
AC-AA	-369 / 0	0.0	0.0	0.04 (1)	7.81	
BB-BA	0 / 60	-38.5	-38.5	0.04 (1)	10.00	
BA-AZ	0 / 50	-38.5	-38.5	0.02 (2)	10.00	
AZ-AY	0 / 38	-38.5	-38.5	0.02 (2)	10.00	
AY-AX	0 / 30	-38.5	-38.5	0.01 (2)	10.00	
AX-AW	0 / 24	-38.5	-38.5	0.01 (2)	10.00	
AW-AV	0 / 18	-38.5	-38.5	0.01 (2)	10.00	
AV-AU	0 / 15	-38.5	-38.5	0.01 (2)	10.00	
AU-AT	0 / 12	-38.5	-38.5	0.01 (2)	10.00	
AT-AS	0 / 12	-38.5	-38.5	0.01 (2)	10.00	
AS-AR	0 / 8	-38.5	-38.5	0.01 (3)	10.00	
AR-AQ	0 / 8	-38.5	-38.5	0.01 (3)	10.00	
AQ-AP	0 / 8	-38.5	-38.5	0.01 (3)	10.00	
AP-AO	0 / 8	-38.5	-38.5	0.01 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI-(LC)
FR-TO		
AM- Q	-265 / 0	0.18 (1)
AN- P	-254 / 0	0.15 (1)
AO- O	-169 / 0	0.10 (1)
AP- N	-169 / 0	0.10 (1)
AQ- M	-254 / 0	0.15 (1)
AS- K	-259 / 0	0.12 (1)
AU- J	-257 / 0	0.27 (1)
AV- I	-257 / 0	0.18 (1)
AW- G	-257 / 0	0.12 (1)
AX- F	-256 / 0	0.08 (1)
AY- E	-254 / 0	0.05 (1)
AZ- D	-270 / 0	0.04 (1)
BA- C	-135 / 0	0.02 (1)
AL- R	-259 / 0	0.12 (1)
AJ- S	-257 / 0	0.27 (1)
AI- T	-257 / 0	0.18 (1)
AH- V	-257 / 0	0.12 (1)
AG- W	-256 / 0	0.08 (1)
AF- X	-254 / 0	0.05 (1)
AE- Y	-270 / 0	0.04 (1)
AD- Z	-135 / 0	0.02 (1)
AR- L	-265 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	36.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 36.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (AA-AB:1) , BC=0.04/1.00 (BA-BB:1) , WB=0.27/1.00 (S-AJ:1) , SSI=0.10/1.00 (AA-AB:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.14 (AC) (INPUT = 1.00)

DWG NO. TAM T1006373
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G62	2	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AO-AN	0 / 8	-38.5	-38.5	0.01 (3)	10.00
AN-AM	0 / 8	-38.5	-38.5	0.01 (3)	10.00
AM-AL	0 / 9	-38.5	-38.5	0.01 (3)	10.00
AL-AK	0 / 12	-38.5	-38.5	0.01 (2)	10.00
AK-AJ	0 / 12	-38.5	-38.5	0.01 (2)	10.00
AJ-AI	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AI-AH	0 / 19	-38.5	-38.5	0.01 (2)	10.00
AH-AG	0 / 24	-38.5	-38.5	0.01 (2)	10.00
AG-AF	0 / 30	-38.5	-38.5	0.01 (2)	10.00
AF-AE	0 / 38	-38.5	-38.5	0.02 (2)	10.00
AE-AD	0 / 50	-38.5	-38.5	0.02 (2)	10.00
AD-AC	0 / 60	-38.5	-38.5	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.C. AWES
PROVINCE OF ONTARIO

DWG NO. TAM T180 5373
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T63	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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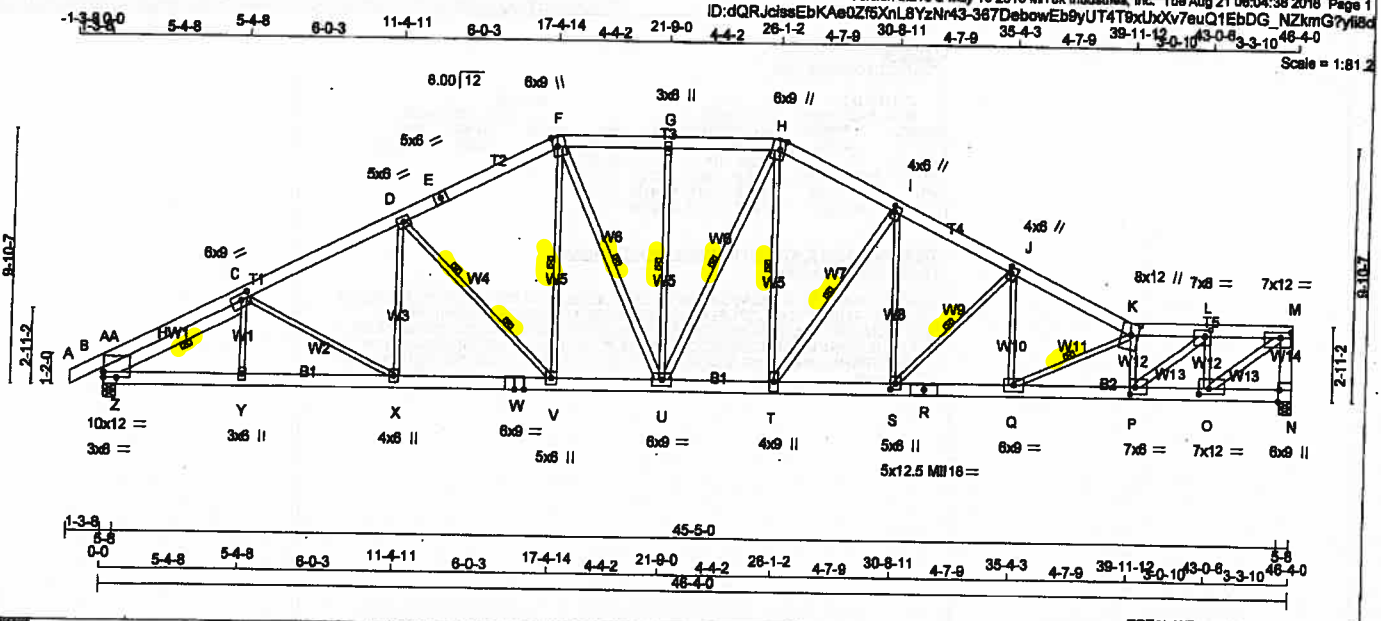

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
B - W	2x6	DRY	2100F 1.8E	SPF
W - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - U	2x4	DRY	No.2	SPF
U - H	2x4	DRY	No.2	SPF
T - I	2x4	DRY	No.2	SPF
P - L	2x4	DRY	2100F 1.8E	SPF
O - M	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	2.50	
B	TP-t	MT20	3.0	6.0	2.50	6.00
C	TMWWW-t	MT20	6.0	8.0	2.75	4.00
D	TMWW-t	MT20	5.0	8.0		
E	TS-t	MT20	5.0	8.0		
F	TTWW+m	MT20	8.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	2.25
I	TMWW+t	MT20	4.0	8.0	2.75	1.00
J	TMWW+t	MT20	4.0	6.0	3.00	1.25
K	TTWW+m	MT20	8.0	12.0	5.25	3.50
L	TMWW-t	MT20	7.0	8.0	3.00	2.75
M	TMW-t	MT20	7.0	12.0	2.50	4.75
N	BMV1+t	MT20	8.0	9.0	Edge	0.50
O	BMWW-t	MT20	7.0	12.0	2.50	4.75
P	BMWW-t	MT20	7.0	8.0	3.25	2.25
Q	BMWW-t	MT20	8.0	9.0		
R	BS-t	MI18	5.0	12.5		
S	BMWW+t	MT20	5.0	8.0	2.75	2.25
T	BMWW+t	MT20	4.0	8.0		
U	BMWWW-t	MT20	8.0	8.0		
V	BMWW+t	MT20	5.0	8.0		
W	BS-t	MI18	8.0	8.0		
X	BMWW+t	MT20	5.0	8.0		
Y	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4839	0	4878	0	-1026	5-8	5-8 - 2 T BEG
B	5077	0	5077	300	-1056	5-8	5-8 2 T BSG

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1026 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1056 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 300 LBS FACTORED HORIZONTAL REACTION AT JOINT B ok

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3489	2493 / 0	487 / 0	0 / 0	125 / -1121	602 / 0	0 / 0
B	3650	2544 / 0	487 / 0	0 / 0	181 / -1152	619 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	214 / -133	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, B

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF D-V.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-V, F-U, G-U, H-U, H-T, I-T, J-S, K-Q, C-Z.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B-AA	-5724 / 1079	-170.4	-170.4	0.47 (2)	3.29
AA- C	-3373 / 826	-170.4	-170.4	0.39 (2)	4.27
C- D	-7380 / 1591	-170.4	-170.4	0.67 (2)	2.54
D- E	-6301 / 1441	-170.4	-170.4	0.78 (2)	2.89
E- F	-6301 / 1441	-170.4	-170.4	0.78 (2)	2.89
F- G	-5978 / 1442	-170.4	-170.4	0.37 (3)	3.32
G- H	-5978 / 1442	-170.4	-170.4	0.37 (3)	3.32
H- I	-6578 / 1554	-170.4	-170.4	0.47 (3)	3.12
I- J	-7962 / 1781	-170.4	-170.4	0.58 (3)	2.73
J- K	-9556 / 2039	-170.4	-170.4	0.76 (3)	2.26
K- L	-10560 / 2221	-170.4	-170.4	0.60 (3)	2.20
L- M	-6159 / 1326	-170.4	-170.4	0.32 (3)	3.31
N- M	-4613 / 1042	0.0	0.0	0.41 (3)	4.84
B- Z	-855 / 3001	-38.5	-38.5	0.16 (1)	6.25
Z- Y	-1548 / 6885	-38.5	-38.5	0.38 (1)	6.25
Y- X	-1549 / 6882	-38.5	-38.5	0.39 (1)	6.25
X- W	-1347 / 6826	-38.5	-38.5	0.38 (1)	6.25
W- V	-1347 / 6826	-38.5	-38.5	0.38 (1)	6.25
V- U	-967 / 5803	-38.5	-38.5	0.31 (1)	6.25
U- T	-895 / 5873	-38.5	-38.5	0.32 (1)	6.25
T- S	-1238 / 7126	-38.5	-38.5	0.38 (1)	6.25
S- R	-1652 / 8580	-38.5	-38.5	0.45 (1)	6.25
R- Q	-1652 / 8580	-38.5	-38.5	0.45 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	0 / 257	0.06 (17)
C- X	-536 / 333	0.47 (2)
X- D	-54 / 520	0.12 (2)
D- V	-1617 / 554	0.67 (2)
V- F	-333 / 1509	0.34 (2)
F- U	-352 / 1696	0.27 (3)
U- G	-1042 / 312	0.61 (2)
U- H	-390 / 1184	0.21 (3)
T- H	-521 / 2326	0.52 (3)
T- I	-2598 / 697	0.88 (3)
S- I	-362 / 1834	0.41 (3)
S- J	-2324 / 592	0.68 (3)
Q- J	-225 / 1452	0.33 (3)
Q- K	-2509 / 568	0.54 (3)
P- K	-3808 / 850	0.81 (1)
P- L	-1188 / 5786	0.43 (1)
O- L	-4340 / 984	0.70 (3)
O- M	-1562 / 7690	0.58 (3)
Z-AA	-360 / 2880	0.00 (1)
Z- C	-4708 / 840	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.44")
ALLOWABLE DEFL.(TL)= L/180 (3.09")
CALCULATED VERT. DEFL.(TL) = L/ 996 (0.55")

CSI: TC=0.67/1.00 (C-D:2) , BC=0.67/1.00 (P-Q:3) , WB=0.88/0.90 (I-T:3) , SSI=0.40/1.00 (D-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1887	766	1987	1656
MI18	438	270	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.98 (W) (INPUT = 1.00)

DWG NO. TAM 17005386 1/2
STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T63	10	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (16)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- P	-2154 / 10720	-38.5	-38.5	0.67 (3)	6.25			
P- O	-1223 / 6159	-38.5	-38.5	0.46 (3)	6.25			
O- N	-20 / 52	-38.5	-38.5	0.02 (8)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(6)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (0.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALLEN
PROVINCE OF ONTARIO

DESIGNED FOR TAM 518053B6
STRUCTURAL
ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G63	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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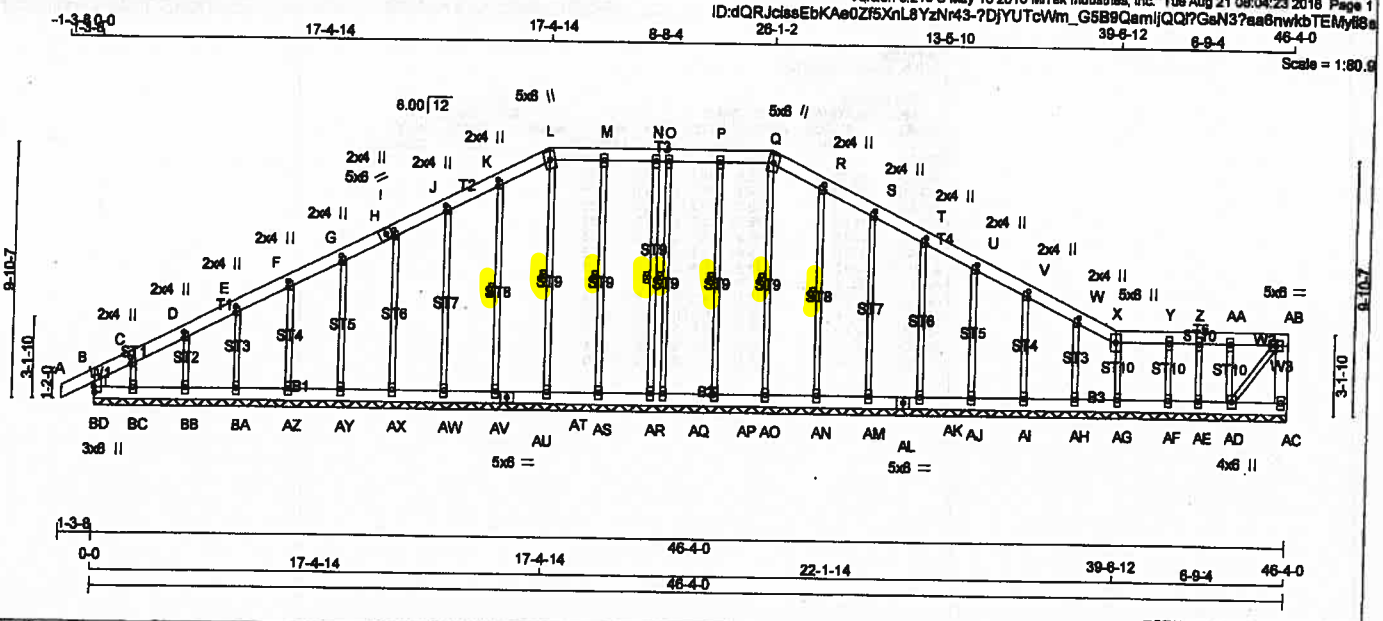

TOTAL WEIGHT = 2 X 290 = 580 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BD- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - Q	2x6	DRY	No.2	SPF
Q - X	2x6	DRY	No.2	SPF
X - AB	2x6	DRY	No.2	SPF
AC- AB	2x6	DRY	No.2	SPF
BD- AU	2x6	DRY	No.2	SPF
AU- AL	2x6	DRY	No.2	SPF
AL- AC	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C, D, E, W						
C	TMW+w	MT20	2.0	4.0	2.75	
F, G, I, J, K, R, S, T, U, V						
F	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	8.0	2.50	2.75
L	TTW+m	MT20	5.0	8.0		
M, N, O, P, Y, Z, AA						
M	TMW+w	MT20	3.0	8.0		
Q	TTW+m	MT20	5.0	8.0		
X	TTW+p	MT20	5.0	8.0		
AB	TMVW-t	MT20	5.0	8.0		
AC	BMV1+p	MT20	3.0	8.0		
AD	BMWW1+t	MT20	4.0	6.0		
AE, AF, AG, AH, AI, AJ, AK, AM, AN, AO, AP, AQ, AR, AS, AT, AV, AW, AX, AY, AZ, BA, BB, BC						
AE	BMW1+w	MT20	3.0	8.0		
AL	BS-t	MT20	5.0	8.0		
AU	BS-t	MT20	5.0	8.0		
BD						
BD	TMBMV1+p	MT20	3.0	8.0	2.75	0.25

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-AO, P-AP, O-AQ, N-AR, M-AS, K-AV, R-AN, L-AT.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
BD- B	-354 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00
B- C	-37 / 0	-129.7	-129.7	0.08 (1)	8.25
C- D	0 / 18	-129.7	-129.7	0.03 (1)	10.00
D- E	0 / 22	-129.7	-129.7	0.04 (1)	10.00
E- F	0 / 31	-129.7	-129.7	0.03 (1)	10.00
F- G	0 / 38	-129.7	-129.7	0.03 (1)	10.00
G- H	0 / 43	-129.7	-129.7	0.03 (1)	10.00
H- I	0 / 43	-129.7	-129.7	0.03 (1)	10.00
I- J	0 / 48	-129.7	-128.7	0.04 (1)	10.00
J- K	0 / 53	-129.7	-129.7	0.04 (1)	10.00
K- L	0 / 45	-129.7	-128.7	0.04 (1)	10.00
L- M	0 / 53	-129.7	-129.7	0.04 (1)	10.00
M- N	0 / 53	-129.7	-128.7	0.04 (1)	10.00
N- O	0 / 53	-129.7	-128.7	0.04 (1)	10.00
O- P	0 / 53	-129.7	-129.7	0.04 (1)	10.00
P- Q	0 / 53	-128.7	-129.7	0.04 (1)	10.00
Q- R	0 / 45	-128.7	-129.7	0.04 (1)	10.00
R- S	0 / 53	-129.7	-129.7	0.04 (1)	10.00
S- T	0 / 48	-128.7	-129.7	0.04 (1)	10.00
T- U	0 / 43	-129.7	-129.7	0.03 (1)	10.00
U- V	0 / 38	-129.7	-129.7	0.03 (1)	10.00
V- W	0 / 33	-129.7	-129.7	0.03 (1)	10.00
W- X	0 / 34	-129.7	-129.7	0.03 (1)	10.00
X- Y	0 / 28	-129.7	-129.7	0.04 (1)	10.00
Y- Z	0 / 27	-129.7	-129.7	0.03 (1)	10.00
Z-AA	0 / 27	-129.7	-129.7	0.03 (1)	10.00
AA-AB	0 / 27	-129.7	-129.7	0.04 (1)	10.00
AC-AB	-94 / 0	0.0	0.0	0.01 (1)	7.81
BD-BC	0 / 0	-38.5	-38.5	0.01 (3)	10.00
BC-BB	-9 / 0	-38.5	-38.5	0.01 (3)	10.00
BB-BA	-20 / 0	-38.5	-38.5	0.01 (3)	8.25
BA-AZ	-28 / 0	-38.5	-38.5	0.01 (3)	8.25
AZ-AY	-34 / 0	-38.5	-38.5	0.01 (3)	8.25
AY-AX	-39 / 0	-38.5	-38.5	0.01 (3)	8.25
AX-AW	-43 / 0	-38.5	-38.5	0.01 (3)	8.25
AW-AV	-47 / 0	-38.5	-38.5	0.01 (3)	8.25
AV-AU	-50 / 0	-38.5	-38.5	0.01 (3)	8.25
AU-AT	-50 / 0	-38.5	-38.5	0.01 (3)	8.25
AT-AS	-53 / 0	-38.5	-38.5	0.01 (3)	8.25
AS-AR	-53 / 0	-38.5	-38.5	0.01 (3)	8.25
AR-AQ	-53 / 0	-38.5	-38.5	0.01 (3)	8.25
AQ-AP	-53 / 0	-38.5	-38.5	0.01 (3)	8.25

WEBS

MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		
AO- Q	-250 / 0	0.15 (1)
AP- P	-275 / 0	0.17 (1)
AQ- O	-170 / 0	0.10 (1)
AR- N	-170 / 0	0.10 (1)
AS- M	-275 / 0	0.17 (1)
AV- K	-282 / 0	0.13 (1)
AW- J	-257 / 0	0.27 (1)
AX- I	-256 / 0	0.18 (1)
AY- G	-257 / 0	0.12 (1)
AZ- F	-257 / 0	0.08 (1)
BA- E	-253 / 0	0.05 (1)
BB- D	-278 / 0	0.04 (1)
BC- C	-115 / 0	0.02 (1)
AN- R	-282 / 0	0.13 (1)
AM- S	-257 / 0	0.27 (1)
AK- T	-256 / 0	0.18 (1)
AJ- U	-257 / 0	0.12 (1)
AI- V	-280 / 0	0.08 (1)
AH- W	-244 / 0	0.05 (1)
AF- Y	-243 / 0	0.04 (1)
AE- Z	-139 / 0	0.02 (1)
AD-AA	-251 / 0	0.04 (1)
AT- L	-249 / 0	0.15 (1)
AG- X	-178 / 0	0.03 (1)
AD-AB	-43 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (AD-AE:3) , WB=0.27/1.00 (S-AM:1) , SSI=0.10/1.00 (AA-AB:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1657	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.12 (R) (INPUT = 1.00)

DWG NO. TAM 71005374
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G63	2	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AP-AO	-53 / 0	-38.5	-38.5	0.01 (3)	6.25
AO-AN	-50 / 0	-38.5	-38.5	0.01 (3)	6.25
AN-AM	-47 / 0	-38.5	-38.5	0.01 (3)	6.25
AM-AL	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AL-AK	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AK-AJ	-39 / 0	-38.5	-38.5	0.01 (3)	6.25
AJ-AI	-34 / 0	-38.5	-38.5	0.01 (3)	6.25
AI-AH	-26 / 0	-38.5	-38.5	0.01 (3)	6.25
AH-AG	-21 / 0	-38.5	-38.5	0.01 (3)	6.25
AG-AF	-27 / 0	-38.5	-38.5	0.01 (3)	6.25
AF-AE	-27 / 0	-38.5	-38.5	0.01 (3)	6.25
AE-AD	-27 / 0	-35.5	-38.5	0.02 (3)	6.25
AD-AC	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805374
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T64	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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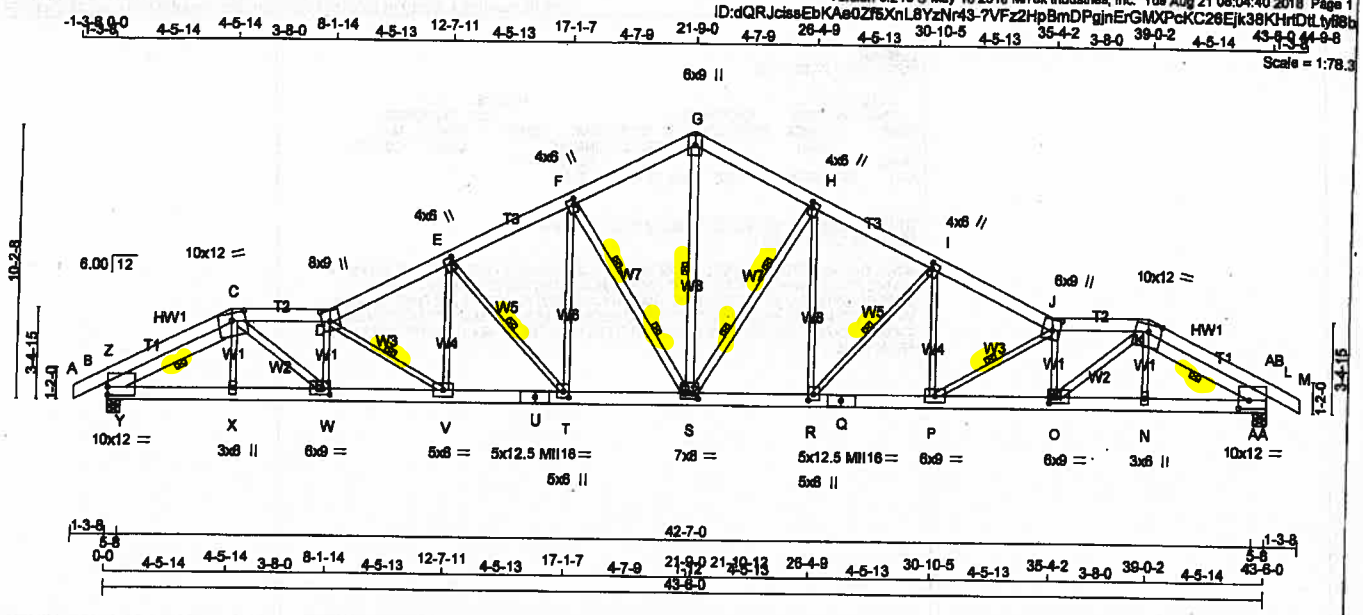

TOTAL WEIGHT = 3 X 273 = 819 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
B - U	2x6	DRY	2100F 1.8E	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - L	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TTWWW-m	MT20	10.0	12.0	3.25	6.00
D	TTWW+m	MT20	6.0	9.0	5.00	3.50
E	TMWW+t	MT20	4.0	6.0	2.75	1.00
F	TMWW+t	MT20	4.0	6.0	2.50	1.00
G	TTW+p	MT20	6.0	6.0		
H	TMWW+t	MT20	4.0	6.0	2.50	1.00
I	TMWW+t	MT20	4.0	6.0	2.75	1.00
J	TTWW+m	MT20	6.0	9.0	5.00	3.50
K	TTWWW-m	MT20	10.0	12.0	3.25	6.00
L	TMBMW1-I	MT20	10.0	12.0	3.75	4.50
N	BMW+w	MT20	3.0	6.0		
O	BMWW-t	MT20	6.0	9.0	2.75	3.25
P	BMWW-t	MT20	6.0	6.0		
Q	BS-t	MII18	5.0	12.5		
R	BMWW+t	MT20	5.0	6.0	2.75	2.25
S	BMWWW-t	MT20	7.0	6.0	2.50	4.00
T	BMWW+t	MT20	5.0	6.0	2.75	2.25
U	BS-t	MII18	5.0	12.5		
V	BMWW-t	MT20	6.0	6.0		
W	BMWW-t	MT20	6.0	6.0	2.75	3.25
X	BMW+w	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG
B	4781	0	4781	273	-954	5-8	5-8 - 2TBE6
L	4781	0	4781	0	-954	5-8	5-8 - 2TBE6

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 954 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 954 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT B ok

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3437	2398 / 0	457 / 0	0 / 0	128 / -1056	582 / 0	0 / 0
L	3437	2398 / 0	457 / 0	0 / 0	128 / -1056	582 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	195 / -195	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-S, D-V, E-T, I-R, J-P, C-Y, K-AA.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF F-S, H-S.

LOADING

TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B- Z	-5100 / 813	-170.4	-170.4	0.37 (2)	3.55
Z- C	-3033 / 732	-170.4	-170.4	0.32 (2)	4.53
C- D	-10015 / 1986	-170.4	-170.4	0.62 (1)	2.23
D- E	-8627 / 1708	-170.4	-170.4	0.63 (1)	2.51
E- F	-7149 / 1435	-170.4	-170.4	0.47 (1)	2.98
F- G	-5735 / 1232	-170.4	-170.4	0.40 (1)	3.38
G- H	-5735 / 1232	-170.4	-170.4	0.40 (1)	3.38
H- I	-7149 / 1435	-170.4	-170.4	0.47 (1)	2.98
I- J	-8627 / 1709	-170.4	-170.4	0.63 (1)	2.51
J- K	-10015 / 1987	-170.4	-170.4	0.62 (1)	2.23
K-AB	-3033 / 733	-170.4	-170.4	0.32 (3)	4.53
AB- L	-5100 / 913	-170.4	-170.4	0.37 (3)	3.55
L- M	0 / 2	-170.4	-170.4	0.13 (3)	10.00
B- Y	-772 / 2671	-38.5	-38.5	0.15 (1)	6.25
Y- X	-1358 / 8255	-38.5	-38.5	0.34 (1)	6.25
X- W	-1358 / 6266	-38.5	-38.5	0.42 (1)	6.25
W- V	-2047 / 10126	-38.5	-38.5	0.61 (1)	6.25
V- U	-1519 / 7927	-38.5	-38.5	0.43 (1)	6.25
U- T	-1518 / 7927	-38.5	-38.5	0.43 (1)	6.25
T- S	-1099 / 6396	-38.5	-38.5	0.39 (1)	6.25
S- R	-912 / 6396	-38.5	-38.5	0.39 (1)	6.25
R- Q	-1246 / 7927	-38.5	-38.5	0.43 (1)	6.25
Q- P	-1246 / 7927	-38.5	-38.5	0.43 (1)	6.25
P- O	-1774 / 10126	-38.5	-38.5	0.61 (1)	6.25
O- N	-1087 / 6266	-38.5	-38.5	0.42 (1)	6.25
N-AA	-1087 / 8255	-38.5	-38.5	0.34 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	0 / 316	0.07 (4)
C- W	-883 / 4881	0.76 (1)
W- D	-3139 / 643	0.57 (1)
S- G	-907 / 4439	0.71 (1)
O- J	-3139 / 642	0.57 (1)
O- K	-882 / 4881	0.76 (1)
N- K	0 / 319	0.07 (4)
D- V	-2688 / 631	0.59 (2)
V- E	-303 / 1709	0.38 (2)
E- T	-2461 / 640	0.76 (2)
T- F	-413 / 1996	0.45 (2)
F- S	-2713 / 732	0.50 (2)
S- H	-2713 / 733	0.50 (3)
R- H	-413 / 1996	0.45 (3)
R- I	-2481 / 641	0.76 (3)
P- I	-303 / 1706	0.38 (3)
P- J	-2668 / 631	0.59 (3)
C- Y	-4442 / 728	0.44 (1)
K-AA	-4442 / 727	0.44 (1)
Y- Z	-257 / 2538	0.00 (1)
AA-AB	-258 / 2538	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.48")
ALLOWABLE DEFL.(TL)= L/180 (2.90")
CALCULATED VERT. DEFL.(TL) = L/ 885 (0.60")

CSI: TC=0.63/1.00 (I-J:1) , BC=0.81/1.00 (O-P:1) , WB=0.78/0.90 (K-O:1) , SSI=0.30/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MII18	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

PROFESSIONAL ENGINEER
STRUCTURAL
11605387

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T64	3	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (16)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI (LC)	MAX UNBRAC FROM TO	MAX. LENGTH FR-TO	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- L	-500 / 2671	-38.5	-38.5 0.15 (1)	6.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(6)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. AIMES
PROVINCE OF ONTARIO

DWG NO. TAM T1805387
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G64A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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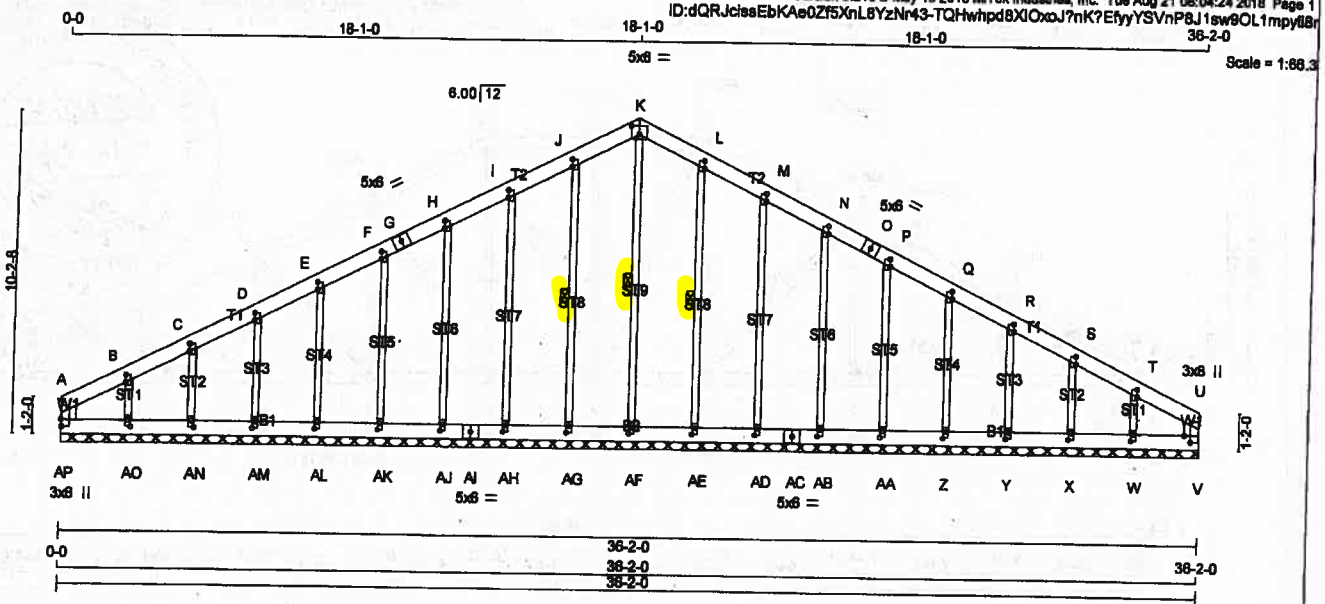

TOTAL WEIGHT = 210 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AP- A	2x6	DRY	No.2	SPF
A - G	2x6	DRY	No.2	SPF
G - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
O - U	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
AP- AI	2x6	DRY	No.2	SPF
AI - AC	2x6	DRY	No.2	SPF
AC- V	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, U, V, AP						
A						
B, C, E, L, M, N, P, Q, R, S, T						
B	TMW+w	MT20	2.0	4.0	2.75	
D, F, H, I, J						
D	TMW+w	MT20	2.0	4.0	3.00	
G	TS-t	MT20	5.0	6.0		
K	TTW-p	MT20	5.0	6.0	3.25	3.00
O	TS-t	MT20	5.0	6.0		
U	TMBMV1+p	MT20	3.0	8.0	2.75	2.25
W, X, Y, Z, AA, AB, AD, AE, AF, AG, AH, AJ, AK, AL, AM, AN, AO						
W	BMW1+w	MT20	2.0	4.0	2.50	1.00
AC	BS-t	MT20	5.0	6.0		
AI	BS-t	MT20	5.0	6.0		
AP	TMBMV1+p	MT20	3.0	6.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF K-AF, J-AG, L-AE.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
AP- A	-175 / 0	0.0	0.0	0.03 (1)		7.81
A- B	-101 / 0	-129.7	-129.7	0.03 (1)		6.25
B- C	-83 / 0	-129.7	-129.7	0.03 (1)		6.25
C- D	-73 / 0	-129.7	-129.7	0.03 (1)		6.25
D- E	-65 / 0	-129.7	-129.7	0.03 (1)		6.25
E- F	-58 / 0	-129.7	-129.7	0.03 (1)		6.25
F- G	-53 / 0	-129.7	-129.7	0.03 (1)		6.25
G- H	-53 / 0	-129.7	-129.7	0.03 (1)		6.25
H- I	-48 / 0	-129.7	-129.7	0.03 (1)		6.25
I- J	-45 / 0	-129.7	-129.7	0.03 (1)		6.25
J- K	-40 / 0	-129.7	-129.7	0.03 (1)		6.25
K- L	-40 / 0	-129.7	-129.7	0.03 (1)		6.25
L- M	-45 / 0	-129.7	-129.7	0.03 (1)		6.25
M- N	-49 / 0	-129.7	-129.7	0.03 (1)		6.25
N- O	-53 / 0	-129.7	-129.7	0.03 (1)		6.25
O- P	-53 / 0	-129.7	-129.7	0.03 (1)		6.25
P- Q	-59 / 0	-129.7	-129.7	0.03 (1)		6.25
Q- R	-65 / 0	-129.7	-129.7	0.03 (1)		6.25
R- S	-73 / 0	-129.7	-129.7	0.03 (1)		6.25
S- T	-83 / 0	-129.7	-129.7	0.03 (1)		6.25
T- U	-101 / 0	-129.7	-129.7	0.03 (1)		6.25
V- U	-175 / 0	0.0	0.0	0.03 (1)		7.81
AP-AO	0 / 90	-38.5	-38.5	0.02 (1)		10.00
AO-AN	0 / 75	-38.5	-38.5	0.02 (2)		10.00
AN-AM	0 / 65	-38.5	-38.5	0.02 (2)		10.00
AM-AL	0 / 58	-38.5	-38.5	0.02 (2)		10.00
AL-AK	0 / 52	-38.5	-38.5	0.02 (2)		10.00
AK-AJ	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AJ-AI	0 / 44	-38.5	-38.5	0.01 (2)		10.00
AI-AH	0 / 44	-38.5	-38.5	0.01 (2)		10.00
AH-AG	0 / 40	-38.5	-38.5	0.01 (2)		10.00
AG-AF	0 / 37	-38.5	-38.5	0.01 (2)		10.00
AF-AE	0 / 37	-38.5	-38.5	0.01 (2)		10.00
AE-AD	0 / 40	-38.5	-38.5	0.01 (2)		10.00
AD-AC	0 / 44	-38.5	-38.5	0.01 (2)		10.00
AC-AB	0 / 44	-38.5	-38.5	0.01 (2)		10.00
AB-AA	0 / 48	-38.5	-38.5	0.02 (2)		10.00
AA- Z	0 / 52	-38.5	-38.5	0.02 (2)		10.00
Z- Y	0 / 58	-38.5	-38.5	0.02 (2)		10.00
Y- X	0 / 65	-38.5	-38.5	0.02 (2)		10.00
X- W	0 / 75	-38.5	-38.5	0.02 (2)		10.00
W- V	0 / 90	-38.5	-38.5	0.02 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AF- K	-241 / 0	0.16 (1)
AG- J	-261 / 0	0.13 (1)
AH- I	-258 / 0	0.30 (1)
AJ- H	-257 / 0	0.21 (1)
AK- F	-257 / 0	0.14 (1)
AL- E	-257 / 0	0.09 (1)
AM- D	-258 / 0	0.06 (1)
AN- C	-256 / 0	0.04 (1)
AO- B	-249 / 0	0.04 (1)
AE- L	-261 / 0	0.13 (1)
AD- M	-258 / 0	0.30 (1)
AB- N	-257 / 0	0.21 (1)
AA- P	-257 / 0	0.14 (1)
Z- Q	-257 / 0	0.09 (1)
Y- R	-256 / 0	0.06 (1)
X- S	-258 / 0	0.04 (1)
W- T	-249 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (K-L:1) , BC=0.02/1.00 (AO-AP:1) , WB=0.30/1.00 (M-AD:1) , SSI=0.08/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (AF) (INPUT = 0.90)
JSI METAL= 0.11 (J) (INPUT = 1.00)

DWG NO. TAM T1805375
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T65	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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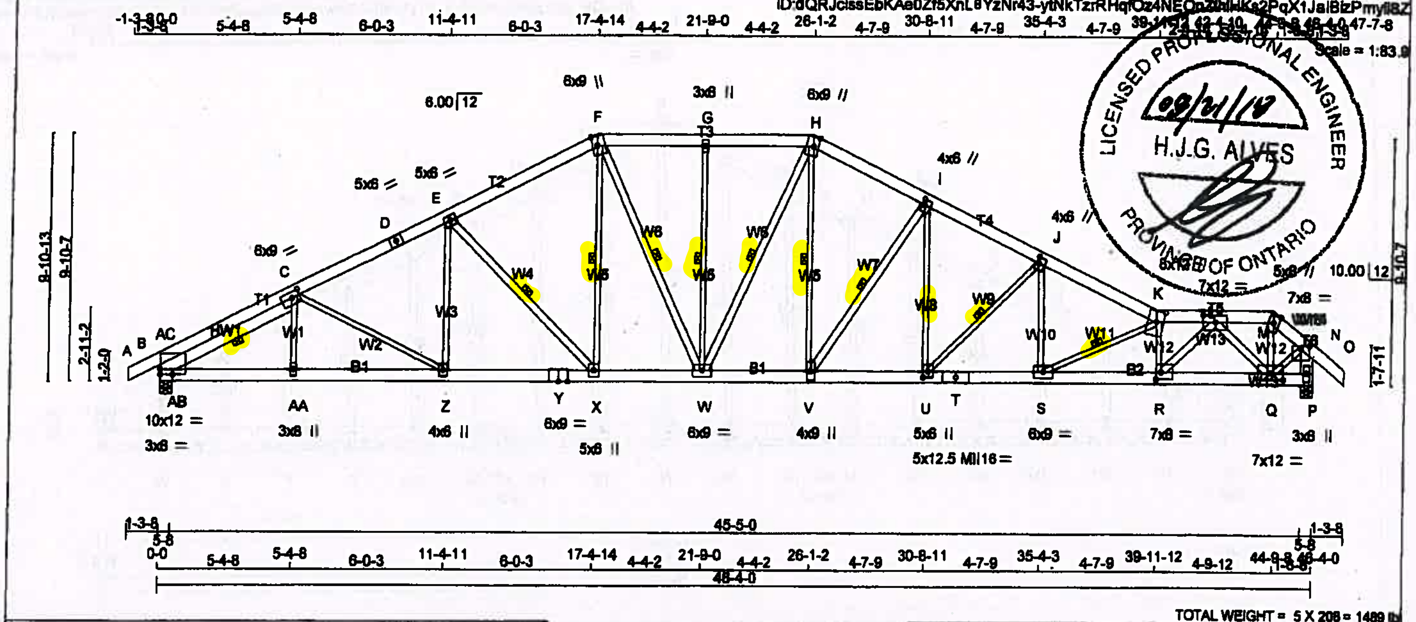

LICENSED PROFESSIONAL ENGINEER
09/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
B - Y	2x6	DRY	2100F 1.8E	SPF
Y - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - X	2x4	DRY	No.2	SPF
F - W	2x4	DRY	No.2	SPF
W - H	2x4	DRY	No.2	SPF
V - I	2x4	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-l	MT20	10.0	12.0	2.50	
B	TP-1	MT20	3.0	6.0	2.50	6.00
C	TMWWW-t	MT20	6.0	8.0	2.75	4.00
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	8.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	8.0	Edge	2.25
I	TMWW+1	MT20	4.0	6.0	2.75	1.00
J	TMWW+t	MT20	4.0	6.0	3.00	1.25
K	TTWW+m	MT20	8.0	12.0	5.25	3.50
L	TMWW-t	MT20	7.0	12.0	3.25	6.00
M	TTW+m	MT20	5.0	6.0	Edge	
N	TMVW-p	MT20	7.0	6.0	1.75	4.00
P	BMV1+p	MT20	3.0	8.0		
Q	BMWWW-t	MT20	7.0	12.0	3.00	6.00
R	BMWW-t	MT20	7.0	8.0	3.25	2.50
S	BMWW-t	MT20	6.0	6.0		
T	BS-t	MII16	5.0	12.5		
U	BMWW+t	MT20	5.0	6.0	2.75	2.25
V	BMWW+t	MT20	4.0	8.0		
W	BMWWW-t	MT20	6.0	8.0		
X	BMWW+t	MT20	5.0	6.0		
Y	BS-t	MT20	6.0	8.0		
Z	BMWW+t	MT20	4.0	8.0		
AA	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	5073	0	5073	298	-1056	5-8	5-8 - 2TBEG
P	5088	0	5276	0	-1091	5-8	5-8 2-TBEG

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1056 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1091 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER .

PROVIDE FOR 298 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3847	2542 / 0	487 / 0	0 / 0	161 / -1152	619 / 0	0 / 0
P	3858	2676 / 0	487 / 0	0 / 0	168 / -1176	620 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	212 / -211	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-X, F-X, F-W, G-W, H-W, H-V, I-V, J-U, K-S, C-AB.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B-AC	-5722 / 1080	-170.4	-170.4	0.47 (2)	3.28
AC- C	-3371 / 826	-170.4	-170.4	0.39 (2)	4.27
C- D	-7375 / 1593	-170.4	-170.4	0.67 (2)	2.53
D- E	-7375 / 1593	-170.4	-170.4	0.67 (2)	2.53
E- F	-6292 / 1441	-170.4	-170.4	0.76 (2)	2.89
F- G	-5988 / 1445	-170.4	-170.4	0.37 (3)	3.33
G- H	-5988 / 1445	-170.4	-170.4	0.37 (3)	3.33
H- I	-6566 / 1558	-170.4	-170.4	0.46 (3)	3.12
I- J	-7945 / 1788	-170.4	-170.4	0.58 (3)	2.74
J- K	-9518 / 2050	-170.4	-170.4	0.76 (3)	2.27
K- L	-10537 / 2250	-170.4	-170.4	0.54 (3)	2.27
L- M	-3174 / 737	-170.4	-170.4	0.12 (3)	4.68
M- N	-4135 / 914	-170.4	-170.4	0.21 (3)	4.09
N- O	0 / 83	-170.4	-170.4	0.15 (3)	10.00
P- N	-5292 / 1096	0.0	0.0	0.35 (3)	4.46
B-AB	-816 / 3000	-38.5	-38.5	0.16 (1)	6.25
AB-AA	-1510 / 6876	-38.5	-38.5	0.38 (1)	6.25
AA- Z	-1511 / 6875	-38.5	-38.5	0.39 (1)	6.25
Z- Y	-1311 / 6822	-38.5	-38.5	0.36 (1)	6.25
Y- X	-1311 / 6822	-38.5	-38.5	0.36 (1)	6.25
X- W	-930 / 5595	-38.5	-38.5	0.31 (1)	6.25
W- V	-858 / 5862	-38.5	-38.5	0.32 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	0 / 256	0.06 (17)
C- Z	-531 / 331	0.46 (2)
Z- E	-52 / 526	0.12 (2)
E- X	-1824 / 558	0.68 (2)
X- F	-331 / 1504	0.34 (2)
F- W	-354 / 1692	0.27 (3)
W- G	-1042 / 312	0.61 (2)
W- H	-384 / 1168	0.21 (3)
V- H	-524 / 2314	0.52 (3)
V- I	-2587 / 701	0.88 (3)
U- I	-366 / 1623	0.41 (3)
U- J	-2294 / 597	0.67 (3)
S- J	-230 / 1420	0.32 (3)
S- K	-2525 / 586	0.55 (3)
R- K	-3743 / 832	0.60 (1)
R- L	-1009 / 5158	0.83 (1)
L- Q	-5688 / 1208	0.85 (3)
Q- M	-504 / 2342	0.53 (3)
Q- N	-637 / 3710	0.83 (3)
AB-AC	-380 / 2690	0.00 (1)
AB- C	-4701 / 839	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.42")
ALLOWABLE DEFL.(TL)= L/180 (3.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.53")

CSI: TC=0.67/1.00 (C-E:2) , BC=0.63/1.00 (R-S:3) , WB=0.88/0.90 (I-V:3) , SSI=0.40/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	786	1667	1656
MII16	436	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17605389
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T65	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (16)

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.99 (L) (INPUT = 1.00)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- U	-1137 / 7110	-38.5	-38.5	0.38 (1)	6.25			
U- T	-1553 / 8546	-38.5	-38.5	0.45 (1)	6.25			
T- S	-1553 / 8546	-38.5	-38.5	0.45 (1)	6.25			
S- R	-2073 / 10695	-38.5	-38.5	0.63 (3)	6.25			
R- Q	-1350 / 7053	-38.5	-38.5	0.45 (3)	6.25			
Q- P	-11 / 23	-38.5	-38.5	0.03 (4)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

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DWG PROGRAM 11805306
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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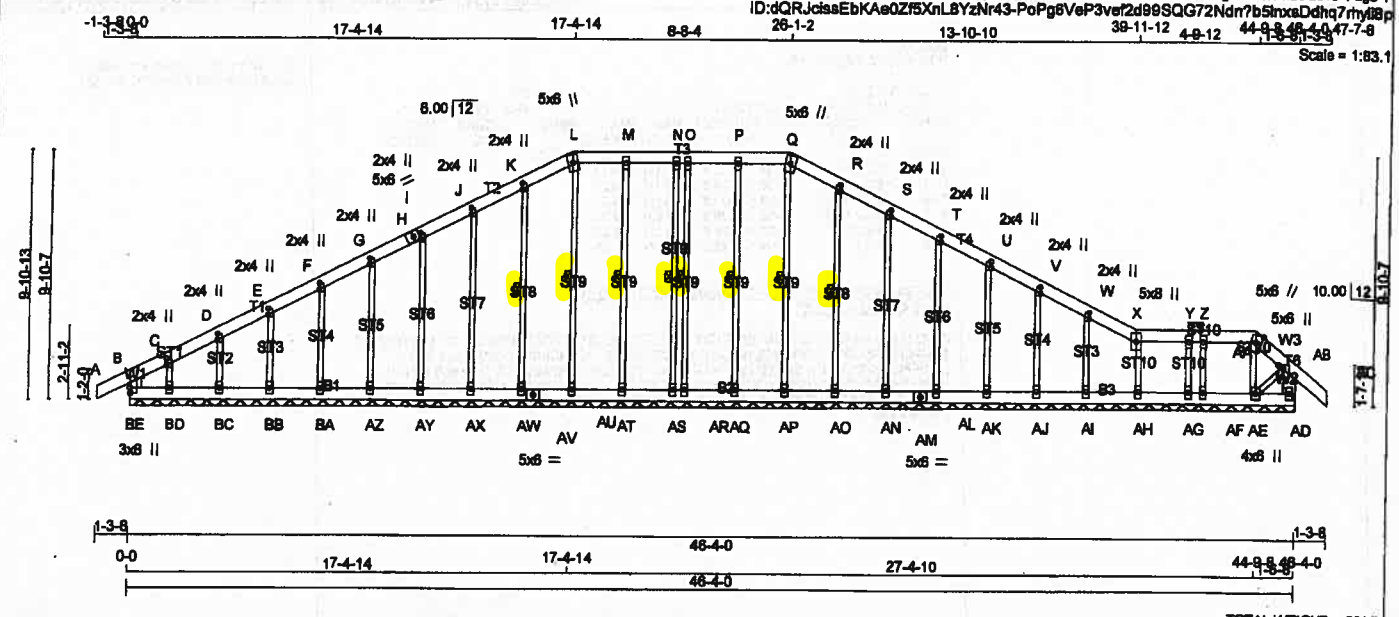

TOTAL WEIGHT = 291 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BE- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - Q	2x6	DRY	No.2	SPF
Q - X	2x6	DRY	No.2	SPF
X - AA	2x6	DRY	No.2	SPF
AA- AC	2x6	DRY	No.2	SPF
AD- AB	2x6	DRY	No.2	SPF
BE- AV	2x6	DRY	No.2	SPF
AV- AM	2x6	DRY	No.2	SPF
AM- AD	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C, D, E						
C	TMW+w	MT20	2.0	4.0	2.75	
F, G, I, J, K, R, S, T, U, V, W						
F	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	6.0	2.50	2.75
L	TTW+m	MT20	5.0	6.0		
M, N, O, P, Y, Z						
M	TMW+w	MT20	3.0	6.0		
Q	TTW+m	MT20	5.0	6.0		
X	TTW+p	MT20	5.0	6.0		
AA	TTW+m	MT20	5.0	6.0	Edge	
AB	TMWW+p	MT20	5.0	6.0	2.00	2.25
AD	BMV1+p	MT20	3.0	6.0		
AE	BMWW1+t	MT20	4.0	6.0		
AF, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR, AS, AT, AU, AW, AX, AY, AZ, BA, BB, BC, BD						
AF	BMW1+w	MT20	3.0	6.0		
AM	BS-t	MT20	5.0	6.0		
AV	BS-t	MT20	5.0	6.0		
BE						
BE	TMBMV1+p	MT20	3.0	6.0	2.75	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

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PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF Q-AP, P-AQ, O-AR, N-AS, M-AT, K-AW, R-AO, L-AU.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
BE- B	-354 / 0	0.0	0.0	0.02 (1)	7.61	
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
B- C	-37 / 0	-129.7	-129.7	0.08 (1)	6.25	
C- D	0 / 16	-129.7	-129.7	0.03 (1)	10.00	
D- E	0 / 22	-129.7	-128.7	0.04 (1)	10.00	
E- F	0 / 31	-129.7	-129.7	0.03 (1)	10.00	
F- G	0 / 38	-129.7	-128.7	0.03 (1)	10.00	
G- H	0 / 43	-129.7	-128.7	0.03 (1)	10.00	
H- I	0 / 43	-129.7	-128.7	0.03 (1)	10.00	
I- J	0 / 48	-129.7	-128.7	0.04 (1)	10.00	
J- K	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
K- L	0 / 45	-129.7	-129.7	0.04 (1)	10.00	
L- M	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
M- N	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
N- O	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
O- P	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
P- Q	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
Q- R	0 / 45	-129.7	-129.7	0.04 (1)	10.00	
R- S	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
S- T	0 / 48	-129.7	-129.7	0.04 (1)	10.00	
T- U	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
U- V	0 / 38	-129.7	-129.7	0.03 (1)	10.00	
V- W	0 / 31	-129.7	-129.7	0.03 (1)	10.00	
W- X	0 / 33	-129.7	-129.7	0.04 (1)	10.00	
X- Y	0 / 26	-129.7	-129.7	0.04 (1)	10.00	
Y- Z	0 / 26	-129.7	-129.7	0.04 (1)	10.00	
Z-AA	0 / 29	-129.7	-129.7	0.04 (1)	10.00	
AA-AB	-27 / 0	-129.7	-129.7	0.09 (1)	6.25	
AB-AC	0 / 60	-129.7	-129.7	0.10 (1)	10.00	
AD-AB	-340 / 0	0.0	0.0	0.02 (1)	7.61	
BE-BD	0 / 0	-38.5	-38.5	0.01 (3)	10.00	
BD-BC	-9 / 0	-38.5	-38.5	0.01 (3)	10.00	
BC-BB	-20 / 0	-38.5	-38.5	0.01 (3)	6.25	
BB-BA	-26 / 0	-38.5	-38.5	0.01 (3)	6.25	
BA-AZ	-34 / 0	-38.5	-38.5	0.01 (3)	6.25	
AZ-AY	-39 / 0	-38.5	-38.5	0.01 (3)	6.25	
AY-AX	-43 / 0	-38.5	-38.5	0.01 (3)	6.25	
AX-AW	-47 / 0	-38.5	-38.5	0.01 (3)	6.25	
AW-AV	-50 / 0	-38.5	-38.5	0.01 (3)	6.25	
AV-AU	-50 / 0	-38.5	-38.5	0.01 (3)	6.25	
AU-AT	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AT-AS	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AS-AR	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AP- Q	-250 / 0	0.15 (1)
AQ- P	-275 / 0	0.17 (1)
AR- O	-170 / 0	0.10 (1)
AS- N	-170 / 0	0.10 (1)
AT- M	-275 / 0	0.17 (1)
AW- K	-282 / 0	0.13 (1)
AX- J	-257 / 0	0.27 (1)
AY- I	-256 / 0	0.16 (1)
AZ- G	-257 / 0	0.12 (1)
BA- F	-257 / 0	0.08 (1)
BB- E	-253 / 0	0.05 (1)
BC- D	-276 / 0	0.04 (1)
BD- C	-116 / 0	0.02 (1)
AO- R	-282 / 0	0.13 (1)
AN- S	-257 / 0	0.27 (1)
AL- T	-256 / 0	0.16 (1)
AK- U	-257 / 0	0.12 (1)
AJ- V	-255 / 0	0.08 (1)
AI- W	-274 / 0	0.08 (1)
AH- X	-200 / 0	0.03 (1)
AG- Y	-202 / 0	0.03 (1)
AE-AA	-145 / 0	0.02 (1)
AF- Z	-187 / 0	0.03 (1)
AE-AB	-27 / 0	0.00 (1)
AU- L	-248 / 0	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (AB-AC:1) , BC=0.01/1.00 (AG-AH:3) , WB=0.27/1.00 (J-AX:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.12 (R) (INPUT = 1.00)

DWG NO. TAM T18005376
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AR-AQ	-53 / 0	-38.5	-38.5	0.01 (3)	6.25			
AQ-AP	-53 / 0	-38.5	-38.5	0.01 (3)	6.25			
AP-AO	-50 / 0	-38.5	-38.5	0.01 (3)	6.25			
AO-AN	-47 / 0	-38.5	-38.5	0.01 (3)	6.25			
AN-AM	-43 / 0	-38.5	-38.5	0.01 (3)	6.25			
AM-AL	-43 / 0	-38.5	-38.5	0.01 (3)	6.25			
AL-AK	-39 / 0	-38.5	-38.5	0.01 (3)	6.25			
AK-AJ	-34 / 0	-38.5	-38.5	0.01 (3)	6.25			
AJ-AI	-28 / 0	-38.5	-38.5	0.01 (3)	6.25			
AI-AH	-20 / 0	-38.5	-38.5	0.01 (3)	6.25			
AH-AG	-28 / 0	-38.5	-38.5	0.01 (3)	6.25			
AG-AF	-26 / 0	-38.5	-38.5	0.01 (3)	6.25			
AF-AE	-28 / 0	-38.5	-38.5	0.01 (3)	6.25			
AE-AD	0 / 0	-38.5	-38.5	0.01 (3)	10.00			

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DWG NO. TAM. T1805376
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	LOB DESC.	DRWG NO.
297081	T104	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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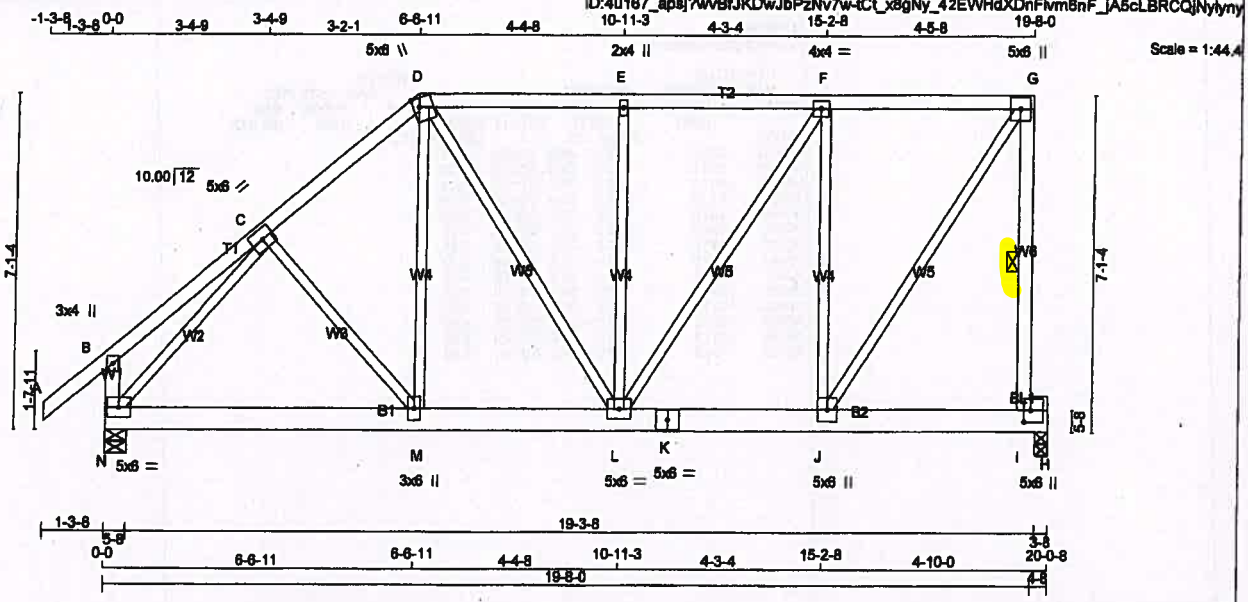


TOTAL WEIGHT = 2 X 113 = 227 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATE	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0		
I	BMVK+p	MT20	5.0	6.0	3.00	1.75
J	BMWW+t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1864	0	1864	0	0	5-8	5-8
H	1837	0	1837	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1389	672 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

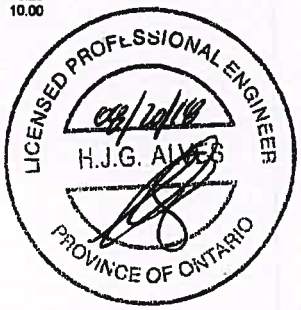
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.16 (1)		10.00
B-C	0 / 30	-129.7	-129.7	0.21 (1)		10.00
C-D	-1499 / 0	-129.7	-129.7	0.25 (1)		5.09
D-E	-1267 / 0	-129.7	-129.7	0.35 (1)		5.27
E-F	-1268 / 0	-129.7	-129.7	0.36 (1)		5.25
F-G	-992 / 0	-129.7	-129.7	0.35 (1)		5.77
I-G	-1791 / 0	0.0	0.0	0.38 (1)		4.99
N-B	-343 / 0	0.0	0.0	0.04 (1)		7.61
N-M	0 / 1169	-38.5	-38.5	0.24 (2)		10.00
M-L	0 / 1131	-39.5	-38.5	0.23 (2)		10.00
L-K	0 / 992	-38.5	-38.5	0.28 (1)		10.00
K-J	0 / 992	-38.5	-38.5	0.28 (1)		10.00
J-I	-39 / 0	-38.5	-38.5	0.57 (1)		6.25
I-H	0 / 0	-38.5	-38.5	0.35 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	-66 / 70	0.03 (1)
M-D	0 / 353	0.08 (2)
N-C	-1634 / 0	0.75 (1)
J-G	0 / 1668	0.42 (1)
D-L	0 / 251	0.06 (1)
J-F	-1056 / 0	0.87 (1)
L-E	-604 / 0	0.49 (1)
L-F	0 / 514	0.12 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.39/1.00 (G-I:1) , BC=0.57/1.00 (I-J:1) , WB=0.67/1.00 (F-J:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	768	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 1180527T
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T105	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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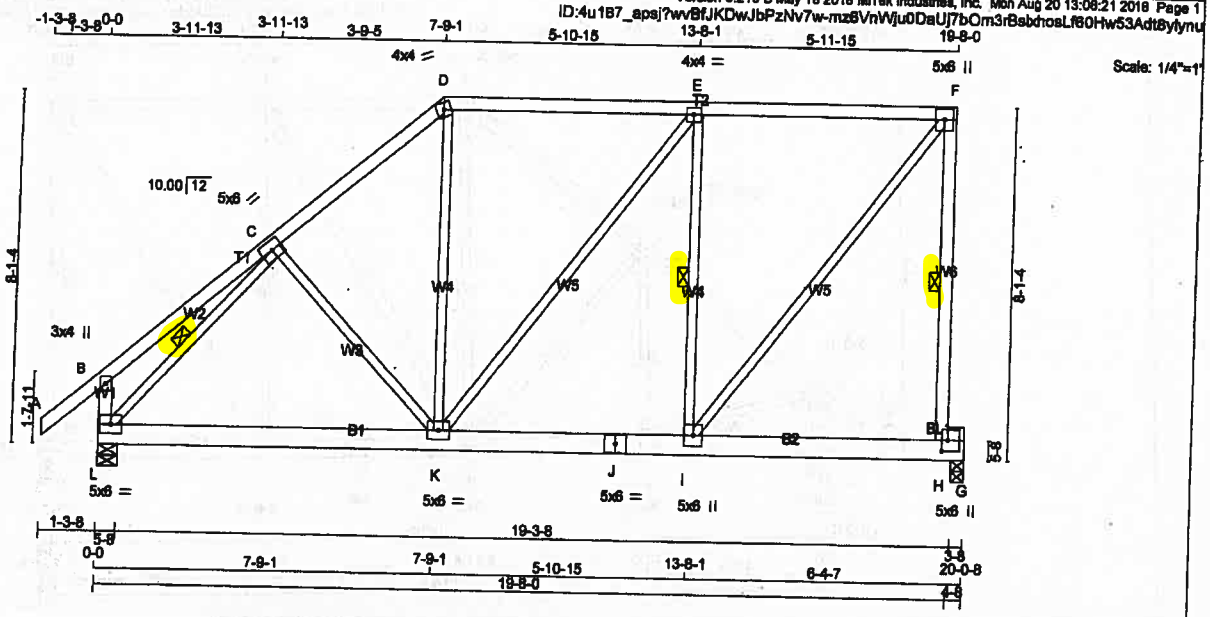

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMVW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMVWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, E-I, C-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.16 (1)	10.00	
B- C	0 / 37	-129.7	-129.7	0.31 (1)	10.00	
C- D	-1410 / 0	-129.7	-129.7	0.36 (1)	5.06	
D- E	-1057 / 0	-129.7	-129.7	0.84 (1)	4.43	
E- F	-1057 / 0	-129.7	-129.7	0.84 (1)	4.43	
H- F	-1699 / 0	0.0	0.0	0.48 (1)	5.10	
L- B	-371 / 0	0.0	0.0	0.04 (1)	7.81	
L- K	0 / 1161	-38.5	-38.5	0.30 (2)	10.00	
K- J	0 / 1057	-38.5	-38.5	0.32 (1)	10.00	
J- I	0 / 1057	-38.5	-38.5	0.32 (1)	10.00	
I- H	-32 / 0	-38.5	-38.5	0.57 (1)	8.25	
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-192 / 42	0.12 (1)
K- D	0 / 401	0.09 (2)
K- E	0 / 55	0.01 (3)
I- E	-957 / 0	0.38 (1)
I- F	0 / 1777	0.40 (1)
L- C	-1801 / 0	0.44 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AYRES
PROVINCE OF ONTARIO

TOTAL WEIGHT = 4 X 109 = 437 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 36.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.84/1.00 (E-F:1) , BC=0.57/1.00 (I-H:1) , WB=0.44/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	799	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)

DWG NO. TAM T1805278
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T106	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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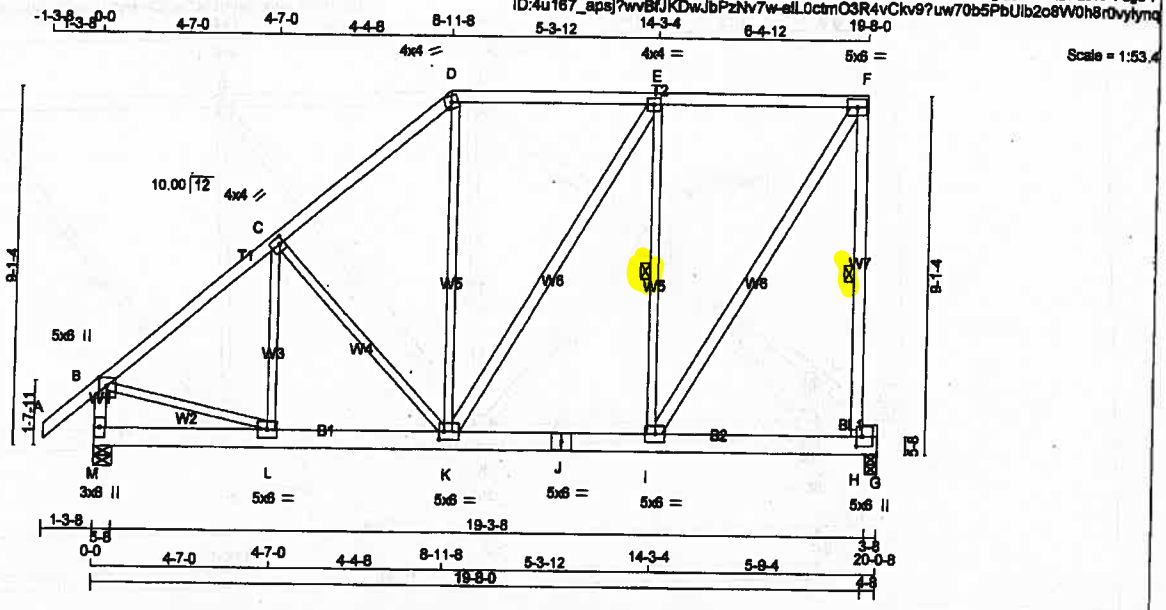

TOTAL WEIGHT = 10 X 123 = 1231 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVK+p	MT20	5.0	6.0	3.00	1.75
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1884	0	1884	0	0	5-8	5-8
G	1837	0	1837	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	736 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1588 / 0	-129.7	-129.7	0.52 (1)	4.61
C- D	-1268 / 0	-129.7	-129.7	0.49 (1)	5.03
D- E	-955 / 0	-129.7	-129.7	0.66 (1)	5.18
E- F	-879 / 0	-129.7	-129.7	0.65 (1)	5.38
H- F	-1736 / 0	0.0	0.0	0.64 (1)	5.05
M- B	-1766 / 0	0.0	0.0	0.18 (1)	6.25
M- L	0 / 0	-38.5	-38.5	0.06 (2)	10.00
L- K	0 / 1252	-38.5	-38.5	0.20 (1)	10.00
K- J	0 / 879	-38.5	-38.5	0.31 (1)	10.00
J- I	0 / 879	-38.5	-38.5	0.31 (1)	10.00
I- H	-29 / 0	-38.5	-38.5	0.57 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-137 / 120	0.06 (1)
C- K	-453 / 0	0.37 (1)
K- D	0 / 332	0.07 (2)
K- E	0 / 149	0.02 (1)
I- E	-966 / 0	0.51 (1)
I- F	0 / 1727	0.29 (1)
B- L	0 / 1298	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
08/10/18
H.J.G. [illegible]
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.66/1.00 (D-E:1) , BC=0.57/1.00 (H-I:1) , WB=0.51/1.00 (E-I:1) , SSI=0.60/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM 1805279
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T107	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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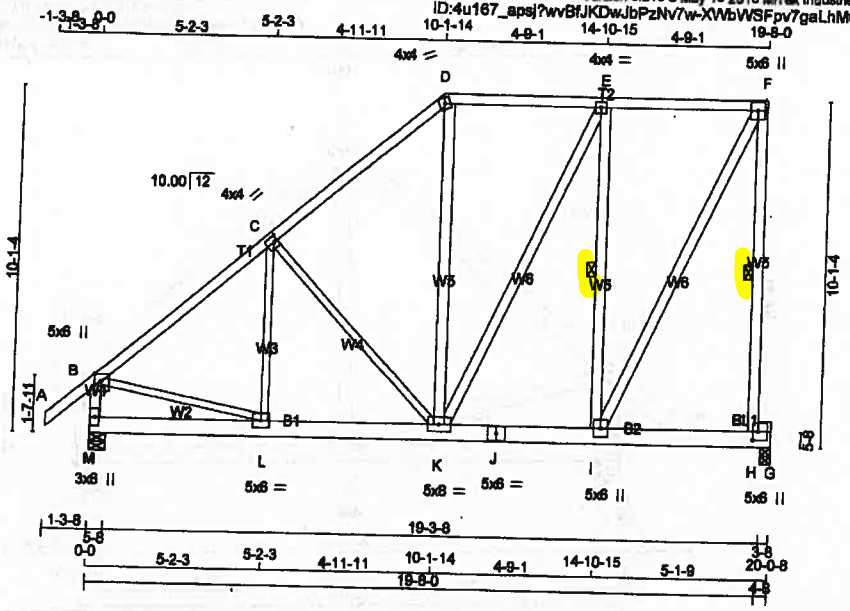

TOTAL WEIGHT = 5 X 135 = 676 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	8.0		
H	BMVK+p	MT20	5.0	8.0	3.00	1.75
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1884	0	1884	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B- C	-1583 / 0	-129.7	-129.7	0.69 (1)	4.33	
C- D	-1177 / 0	-128.7	-129.7	0.64 (1)	4.92	
D- E	-865 / 0	-128.7	-129.7	0.51 (1)	5.76	
E- F	-724 / 0	-128.7	-129.7	0.50 (1)	6.16	
H- F	-1769 / 0	0.0	0.0	0.85 (1)	5.02	
M- B	-1778 / 0	0.0	0.0	0.18 (1)	6.26	
M- L	0 / 0	-38.5	-38.5	0.09 (3)	10.00	
L- K	0 / 1255	-38.5	-38.5	0.21 (1)	10.00	
K- J	0 / 724	-38.5	-38.5	0.26 (1)	10.00	
J- I	0 / 724	-38.5	-38.5	0.26 (1)	10.00	
I- H	-27 / 0	-38.5	-38.5	0.57 (1)	6.25	
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-69 / 175	0.04 (3)
C- K	-560 / 0	0.64 (1)
K- D	0 / 268	0.04 (2)
K- E	0 / 331	0.05 (1)
I- E	-1059 / 0	0.51 (1)
I- F	0 / 1708	0.27 (1)
B- L	0 / 1289	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
08/10/14
H.J.G. MYERS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.85/1.00 (F-H:1) , BC=0.57/1.00 (H-I:1) , WB=0.84/1.00 (C-K:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

DWG NO. TAM T1805280
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297082	T109	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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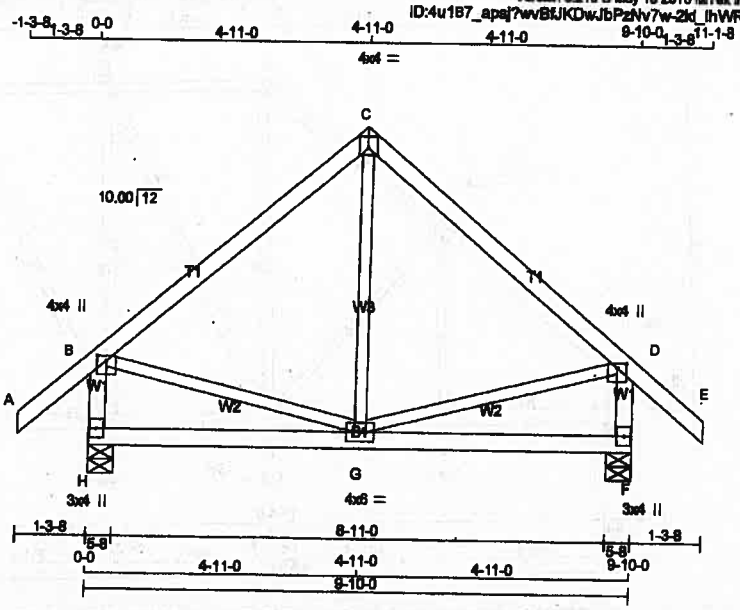

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW/+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW/+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1008	0	1008	0	0	5-8	5-8
F	1008	0	1008	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	729	482 / 0	103 / 0	0 / 0	0 / 0	144 / 0	0 / 0
F	729	482 / 0	103 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-516 / 0	-129.7	-129.7	0.40 (1)	6.25
C- D	-516 / 0	-129.7	-129.7	0.40 (1)	6.25
D- E	0 / 57	-129.7	-129.7	0.18 (1)	10.00
H- B	-934 / 0	0.0	0.0	0.10 (1)	7.81
F- D	-934 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 0	-38.5	-38.5	0.21 (3)	10.00
G- F	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 209	0.05 (3)
B- G	0 / 410	0.09 (1)
G- D	0 / 410	0.09 (1)

TOTAL WEIGHT = 4 X 45 = 179 lb [MiTek]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(65 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.21/1.00 (G-H:3) , WB=0.09/1.00 (D-G:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (D) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1190[illegible]3922
STRUCTURAL
CONFIGURATION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297082	G109	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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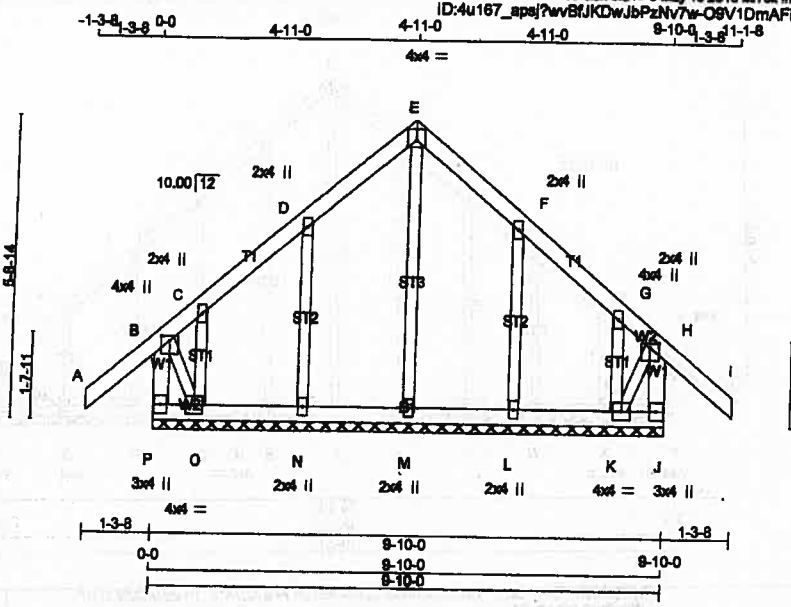

TOTAL WEIGHT = 2 X 48 = 96 lbs

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMWW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-382 / 0	0.0	0.0	0.04 (1)	7.61
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-94 / 0	-129.7	-129.7	0.17 (1)	6.25
C- D	0 / 6	-129.7	-129.7	0.08 (1)	10.00
D- E	-17 / 0	-129.7	-129.7	0.08 (1)	6.25
E- F	-17 / 0	-129.7	-129.7	0.08 (1)	6.25
F- G	0 / 6	-129.7	-129.7	0.08 (1)	10.00
G- H	-94 / 0	-129.7	-129.7	0.17 (1)	6.25
H- I	0 / 57	-129.7	-129.7	0.16 (1)	10.00
J- H	-382 / 0	0.0	0.0	0.04 (1)	7.61
P- O	0 / 0	-38.5	-38.5	0.02 (3)	10.00
O- N	0 / 8	-38.5	-38.5	0.03 (3)	10.00
N- M	-1 / 0	-38.5	-38.5	0.03 (3)	10.00
M- L	-1 / 0	-38.5	-38.5	0.03 (3)	10.00
L- K	0 / 6	-38.5	-38.5	0.03 (3)	10.00
K- J	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-215 / 0	0.10 (1)
N- D	-310 / 0	0.06 (1)
O- C	-29 / 0	0.00 (1)
L- F	-310 / 0	0.06 (1)
K- G	-29 / 0	0.00 (1)
B- O	0 / 14	0.00 (1)
K- H	0 / 14	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (H-I:1) , BC=0.03/1.00 (L-M:3) , WB=0.10/1.00 (E-M:1) , SSI=0.11/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1730 5300
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	G110	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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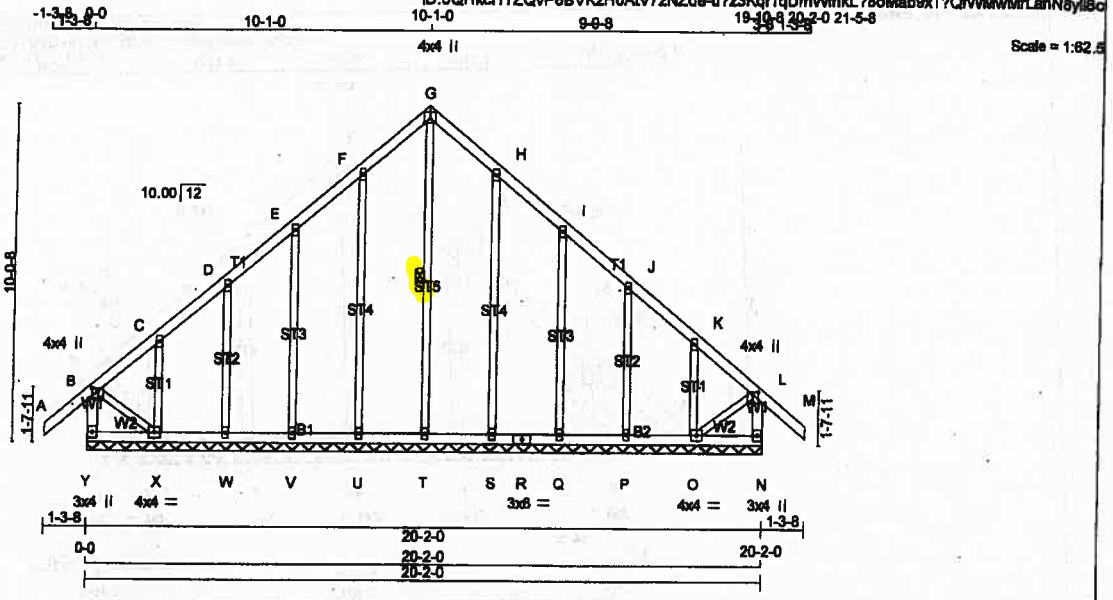

TOTAL WEIGHT = 108 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
R	BB-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-353 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.7	-129.7	0.16 (1)	10.00
B- C	-46 / 0	-129.7	-129.7	0.07 (1)	8.25
C- D	-55 / 0	-129.7	-129.7	0.07 (1)	8.25
D- E	-45 / 0	-129.7	-129.7	0.06 (1)	8.25
E- F	-36 / 0	-129.7	-129.7	0.07 (1)	8.25
F- G	-52 / 0	-129.7	-129.7	0.07 (1)	8.25
G- H	-52 / 0	-129.7	-129.7	0.07 (1)	8.25
H- I	-38 / 0	-129.7	-129.7	0.07 (1)	8.25
I- J	-45 / 0	-129.7	-129.7	0.06 (1)	8.25
J- K	-55 / 0	-129.7	-129.7	0.07 (1)	8.25
K- L	-46 / 0	-128.7	-129.7	0.07 (1)	8.25
L- M	0 / 57	-129.7	-129.7	0.18 (1)	10.00
N- L	-353 / 0	0.0	0.0	0.04 (1)	7.81
Y- X	0 / 0	-38.5	-38.5	0.03 (3)	10.00
X- W	0 / 40	-38.5	-38.5	0.03 (2)	10.00
W- V	0 / 35	-38.5	-38.5	0.03 (2)	10.00
V- U	0 / 31	-38.5	-38.5	0.03 (2)	10.00
U- T	0 / 27	-38.5	-38.5	0.03 (2)	10.00
T- S	0 / 27	-38.5	-38.5	0.03 (2)	10.00
S- R	0 / 31	-38.5	-38.5	0.03 (2)	10.00
R- Q	0 / 31	-38.5	-38.5	0.03 (2)	10.00
Q- P	0 / 35	-38.5	-38.5	0.03 (2)	10.00
P- O	0 / 40	-38.5	-38.5	0.03 (2)	10.00
O- N	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-174 / 0	0.12 (1)
U- F	-294 / 0	0.41 (1)
V- E	-251 / 0	0.19 (1)
W- D	-248 / 0	0.09 (1)
X- C	-284 / 0	0.06 (1)
S- H	-294 / 0	0.41 (1)
Q- I	-251 / 0	0.19 (1)
P- J	-249 / 0	0.09 (1)
O- K	-284 / 0	0.06 (1)
B- X	0 / 57	0.01 (1)
O- L	0 / 57	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.8	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (W-X:2) , WB=0.41/1.00 (H-S:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (G) (INPUT = 0.90)
JSI METAL= 0.16 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. AU
PROVINCE OF ONTARIO

DWG NO. TAM T1805377
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T110A	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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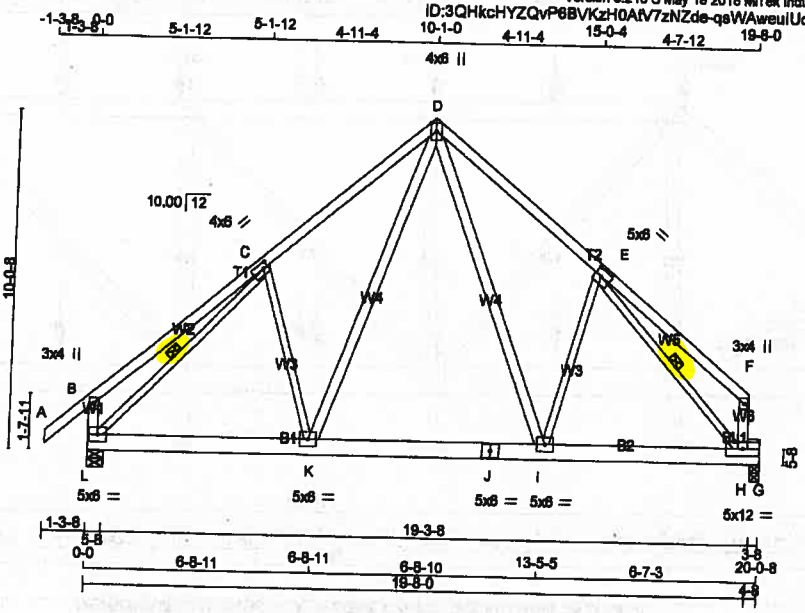

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	5.0	12.0		
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 12 X 113 = 1356 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1864	0	1864	0	0	5-8	IN-SX
G	1637	0	1637	0	0	3-8	5-8 / 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 50	-129.7	-129.7	0.54 (1)	10.00
C- D	-1489 / 0	-129.7	-129.7	0.65 (1)	4.49
D- E	-1510 / 0	-129.7	-129.7	0.60 (1)	4.58
E- F	0 / 74	-129.7	-129.7	0.50 (1)	10.00
L- B	-425 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-198 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 1230	-38.5	-38.5	0.28 (1)	10.00
K- J	0 / 875	-38.5	-38.5	0.32 (1)	10.00
J- I	0 / 875	-38.5	-38.5	0.32 (1)	10.00
I- H	0 / 1184	-38.5	-38.5	0.71 (1)	10.00
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-424 / 14	0.23 (1)
K- D	0 / 668	0.11 (1)
D- I	0 / 745	0.12 (1)
I- E	-234 / 74	0.13 (1)
L- C	-1808 / 0	0.64 (1)
E- H	-1928 / 0	0.83 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. REVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	36.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 36.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.65/1.00 (C-D:1) , BC=0.71/1.00 (H-I:1) , WB=0.64/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)

DWG NO. TAM 1805282
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T120	1	2	TRUSS DESC.	

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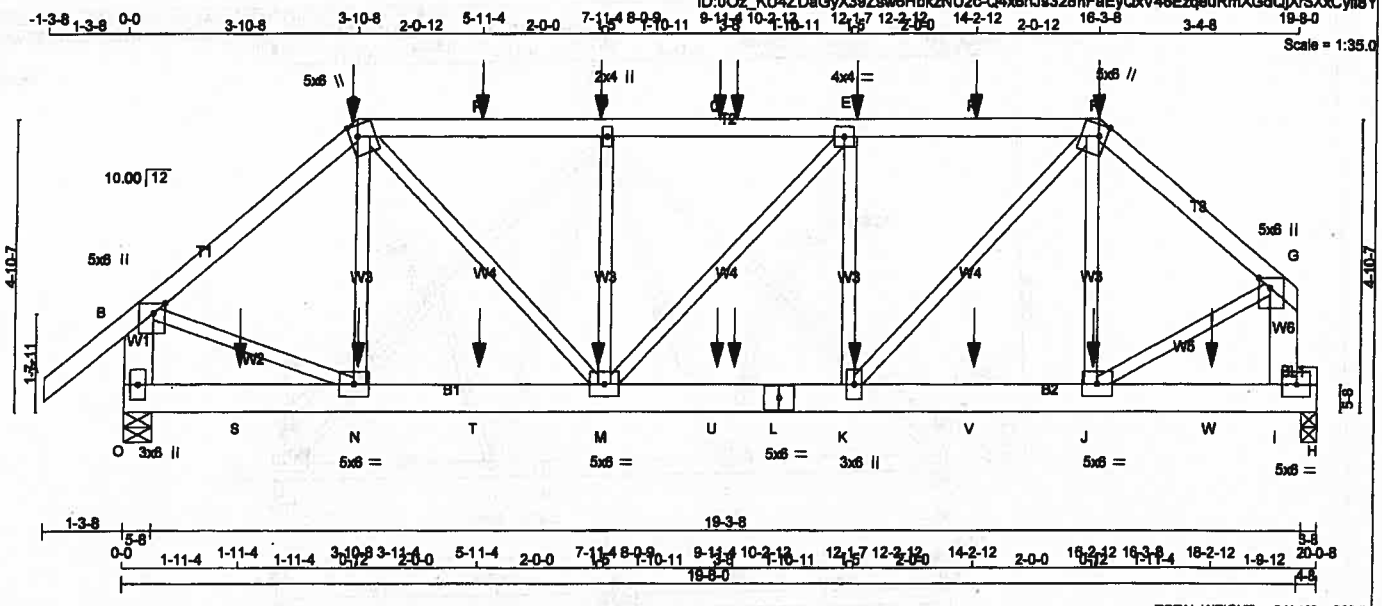

TOTAL WEIGHT = 2 X 102 = 203 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	SIDE(61.0)
O-B	2	12	TOP
I-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-L	2	12	SIDE(183.1)
L-H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	8	
2x4	1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.25	1.50
G	TMVW+p	MT20	5.0	8.0	2.00	2.25
I	BMVK-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	8.0		
K	BMWW+t	MT20	3.0	8.0		
L	BS-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2810	0	2810	0	0	5-8	5-8
H	2464	0	2464	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1902	1241 / 0	309 / 0	0 / 0	0 / 0	351 / 0	0 / 0
H	1802	1154 / 0	310 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.10 (1)		10.00
B-C	-2409 / 0	-129.7	-129.7	0.22 (1)		5.55
C-P	-2838 / 0	-129.7	-129.7	0.27 (1)		5.13
P-D	-2839 / 0	-129.7	-129.7	0.27 (1)		5.13
D-Q	-2839 / 0	-129.7	-129.7	0.26 (1)		5.13
Q-E	-2839 / 0	-129.7	-129.7	0.26 (1)		5.13
E-R	-2811 / 0	-129.7	-129.7	0.26 (1)		5.15
R-F	-2811 / 0	-129.7	-129.7	0.26 (1)		5.15
F-G	-2439 / 0	-129.7	-129.7	0.17 (1)		5.59
O-B	-2546 / 0	0.0	0.0	0.09 (1)		7.81
I-G	-2883 / 0	0.0	0.0	0.11 (1)		7.81
O-S	0 / 0	-38.5	-38.5	0.03 (3)		10.00
S-N	0 / 0	-38.5	-38.5	0.03 (3)		10.00
N-T	0 / 1838	-38.5	-38.5	0.15 (1)		10.00
T-M	0 / 1838	-38.5	-38.5	0.15 (1)		10.00
M-U	0 / 2811	-38.5	-38.5	0.24 (1)		10.00
U-L	0 / 2811	-38.5	-38.5	0.24 (1)		10.00
L-K	0 / 2811	-38.5	-38.5	0.24 (1)		10.00
K-V	0 / 1863	-38.5	-38.5	0.26 (1)		10.00
V-J	0 / 1863	-38.5	-38.5	0.26 (1)		10.00
J-W	-399 / 0	-38.5	-38.5	0.54 (1)		6.25
W-I	-399 / 0	-38.5	-38.5	0.54 (1)		6.25
I-H	0 / 0	-168.2	-168.2	0.33 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
N-C	-331 / 84	0.08 (1)
C-M	0 / 1467	0.18 (1)
M-D	-903 / 0	0.15 (1)
M-E	0 / 41	0.01 (1)
K-E	-922 / 0	0.18 (1)
K-F	0 / 1390	0.17 (1)
J-F	-219 / 112	0.04 (1)
B-N	0 / 1939	0.24 (1)
J-G	0 / 2506	0.31 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-195	-195	—	BACK	VERT	TOTAL
D	7-11-4	-83	-83	—	BACK	VERT	TOTAL
E	12-2-12	-83	-83	—	BACK	VERT	TOTAL
F	16-3-8	-195	-195	—	BACK	VERT	TOTAL
J	16-2-12	-26	-26	—	BACK	VERT	TOTAL
K	12-2-12	-21	-21	—	BACK	VERT	TOTAL
M	7-11-4	-21	-21	—	BACK	VERT	TOTAL
N	3-11-4	-26	-26	—	BACK	VERT	TOTAL
P	5-11-4	-83	-83	—	BACK	VERT	TOTAL
Q	9-11-4	-83	-83	—	BACK	VERT	TOTAL
Q	10-2-12	-83	-83	—	BACK	VERT	TOTAL
R	14-2-12	-83	-83	—	BACK	VERT	TOTAL
S	1-11-4	-11	-16	—	BACK	VERT	TOTAL
T	5-11-4	-21	-21	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	36.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 36.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.54/1.00 (I-J:1)
WB=0.31/1.00 (G-J:1) , SSI=0.50/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.30 (L) (INPUT = 1.00)

BMS NO. TAM 11605389
STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T120	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 281.9 lbs FACTORED DOWN AT 3-10-8, 121.8 lbs FACTORED DOWN AT 5-11-4, 121.8 lbs FACTORED DOWN AT 7-11-4, 121.8 lbs FACTORED DOWN AT 9-11-4, 121.8 lbs FACTORED DOWN AT 10-2-12, 121.8 lbs FACTORED DOWN AT 12-2-12, AND 121.8 lbs FACTORED DOWN AT 14-2-12, AND 281.9 lbs FACTORED DOWN AT 16-3-8 ON TOP CHORD, AND 23.7 lbs FACTORED DOWN AT 1-11-4, 38.0 lbs FACTORED DOWN AT 3-11-4, 30.8 lbs FACTORED DOWN AT 5-11-4, 30.9 lbs FACTORED DOWN AT 7-11-4, 30.9 lbs FACTORED DOWN AT 9-11-4, 30.9 lbs FACTORED DOWN AT 10-2-12, 30.9 lbs FACTORED DOWN AT 12-2-12, 30.8 lbs FACTORED DOWN AT 14-2-12, AND 38.0 lbs FACTORED DOWN AT 16-2-12, AND 23.7 lbs FACTORED DOWN AT 18-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	9-11-4	-21	-21	—	BACK	VERT	TOTAL
U	10-2-12	-21	-21	—	BACK	VERT	TOTAL
V	14-2-12	-21	-21	—	BACK	VERT	TOTAL
W	18-2-12	-11	-16	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/11/16
H.J.G. ALVES
PROVINCE OF ONTARIO

ENGINEERING 7805389
STRUCTURAL
COMPONENT GROUP 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T121	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 13:06:11 2018 Page 1
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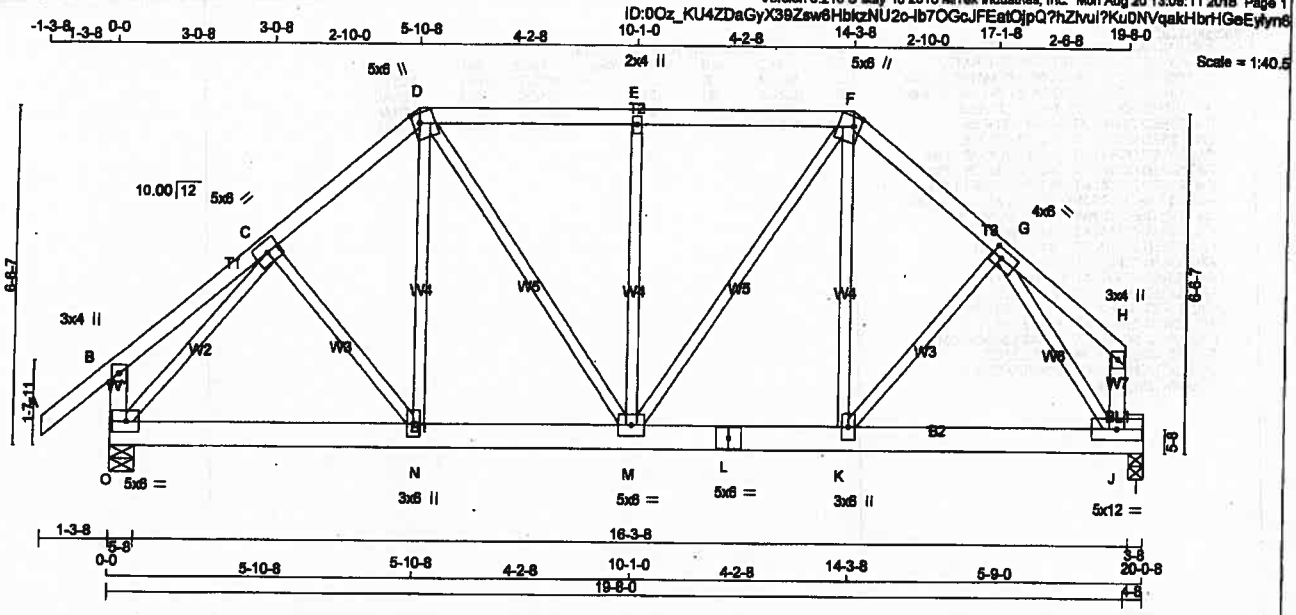

TOTAL WEIGHT = 107 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	5.0	12.0		
K	BMWW+t	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1864	0	1864	0	0	5-8	5-8
I	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
I	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 26	-129.7	-129.7	0.16 (1)	10.00
C- D	-1535 / 0	-129.7	-129.7	0.19 (1)	5.11
D- E	-1403 / 0	-129.7	-129.7	0.41 (1)	4.97
E- F	-1403 / 0	-129.7	-129.7	0.41 (1)	4.97
F- G	-1557 / 0	-129.7	-129.7	0.17 (1)	5.11
G- H	0 / 38	-129.7	-129.7	0.14 (1)	10.00
O- B	-328 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-111 / 0	0.0	0.0	0.01 (1)	7.81
O- N	0 / 1149	-38.5	-38.5	0.22 (1)	10.00
N- M	0 / 1160	-38.5	-38.5	0.21 (2)	10.00
M- L	0 / 1160	-38.5	-38.5	0.38 (1)	10.00
L- K	0 / 1160	-38.5	-38.5	0.36 (1)	10.00
K- J	0 / 1008	-38.5	-38.5	0.69 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 89	0.02 (3)
N- D	0 / 278	0.08 (2)
D- M	0 / 431	0.10 (1)
M- E	-659 / 0	0.43 (1)
M- F	0 / 394	0.09 (1)
K- F	0 / 311	0.07 (2)
K- G	0 / 288	0.08 (1)
O- C	-1842 / 0	0.82 (1)
G- J	-1915 / 0	0.57 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AIKES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	81.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014 .

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.69/1.00 (J-K:1) , WB=0.62/1.00 (C-O:1) , SSI=0.60/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

DWG NO. TAM T1805292
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T122	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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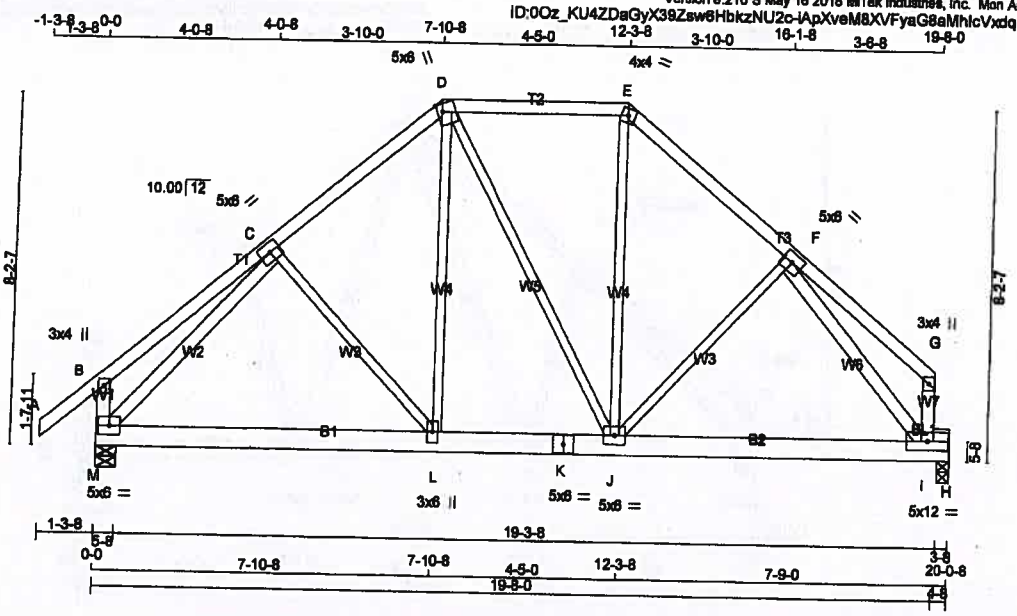

Scale = 1:48.9

TOTAL WEIGHT = 110 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMWWK-t	MT20	5.0	12.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	8.0		
M	BMWW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1864	0	1864	0	0	IN-SX	IN-SX
H	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
M	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B-C	0 / 39	-128.7	-129.7	0.33 (1)	10.00
C-D	-1400 / 0	-129.7	-129.7	0.37 (1)	5.05
D-E	-1060 / 0	-129.7	-129.7	0.46 (1)	5.38
E-F	-1436 / 0	-129.7	-129.7	0.34 (1)	5.06
F-G	0 / 56	-129.7	-128.7	0.29 (1)	10.00
M-B	-372 / 0	0.0	0.0	0.04 (1)	7.81
I-G	-148 / 0	0.0	0.0	0.02 (1)	7.81
M-L	0 / 1181	-38.5	-38.5	0.31 (2)	10.00
L-K	0 / 1048	-38.5	-38.5	0.40 (1)	10.00
K-J	0 / 1048	-38.5	-38.5	0.40 (1)	10.00
J-I	0 / 1079	-38.5	-38.5	0.69 (1)	10.00
I-H	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-205 / 38	0.13 (1)
L-D	0 / 353	0.08 (2)
D-J	0 / 66	0.01 (1)
J-E	0 / 413	0.09 (2)
J-F	-7 / 98	0.02 (3)
M-C	-1797 / 0	0.75 (1)
F-I	-1855 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.69/1.00 (I-J:1) , WB=0.75/1.00 (C-M:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.45 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/10/18
H.J.G.
PROVINCE OF ONTARIO

DWG NO. TAM 11805293
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T123	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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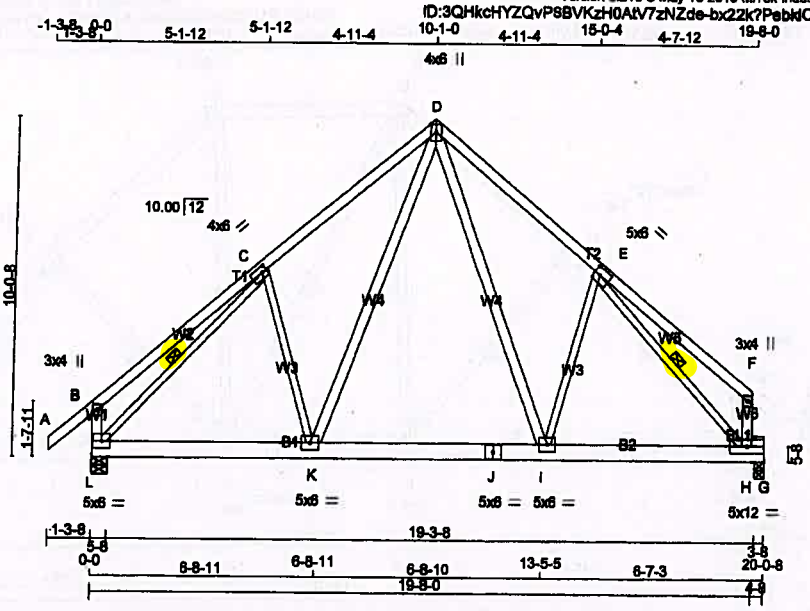

TOTAL WEIGHT = 113 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	8.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	5.0	12.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	8.0		
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1864	0	1864	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.15 (1)		10.00
B-C	0 / 50	-129.7	-129.7	0.54 (1)		10.00
C-D	-1469 / 0	-129.7	-129.7	0.65 (1)		4.49
D-E	-1510 / 0	-129.7	-129.7	0.60 (1)		4.58
E-F	0 / 74	-129.7	-129.7	0.50 (1)		10.00
L-B	-425 / 0	0.0	0.0	0.04 (1)		7.81
H-F	-198 / 0	0.0	0.0	0.02 (1)		7.81
L-K	0 / 1230	-38.5	-38.5	0.26 (1)		10.00
K-J	0 / 875	-38.5	-38.5	0.32 (1)		10.00
J-I	0 / 875	-38.5	-38.5	0.32 (1)		10.00
I-H	0 / 1194	-38.5	-38.5	0.71 (1)		10.00
H-G	0 / 0	-38.5	-38.5	0.35 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-424 / 14	0.23 (1)
K-D	0 / 688	0.11 (1)
D-I	0 / 745	0.12 (1)
I-E	-234 / 74	0.13 (1)
L-C	-1808 / 0	0.84 (1)
E-H	-1928 / 0	0.83 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.65/1.00 (C-D:1) , BC=0.71/1.00 (H-I:1) , WB=0.64/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)

DWG NO. TAM 17605294
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T123C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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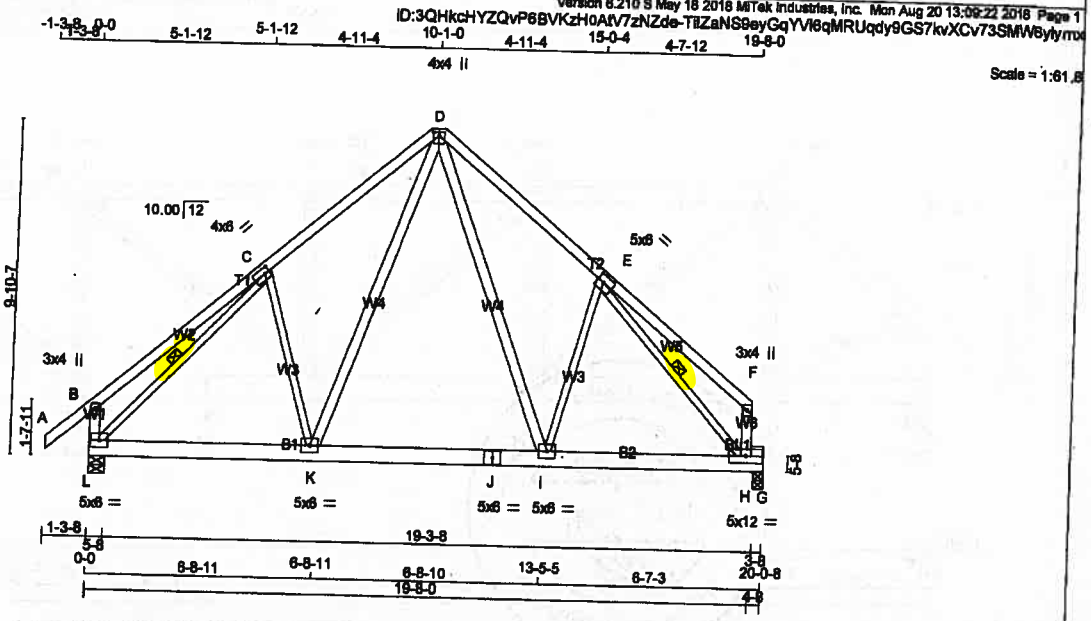

TOTAL WEIGHT = 113 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0	2.00	1.75
D	TTWW+p	MT20	4.0	4.0	1.25	2.00
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	5.0	12.0		
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GRDSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1664	0	1664	0	0	IN-SX	IN-SX
G	1637	0	1637	0	0	5-8	5-8
						3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 50	-129.7	-129.7	0.54 (1)	10.00
C- D	-1489 / 0	-129.7	-129.7	0.65 (1)	4.49
D- E	-1510 / 0	-129.7	-129.7	0.60 (1)	4.58
E- F	0 / 74	-129.7	-129.7	0.50 (1)	10.00
L- B	-425 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-198 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 1230	-38.5	-38.5	0.28 (1)	10.00
K- J	0 / 875	-38.5	-38.5	0.32 (1)	10.00
J- I	0 / 875	-38.5	-38.5	0.32 (1)	10.00
I- H	0 / 1194	-38.5	-38.5	0.71 (1)	10.00
H- G	0 / 0	-38.5	-38.5	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-424 / 14	0.23 (1)
K- D	0 / 686	0.11 (1)
D- I	0 / 745	0.12 (1)
I- E	-234 / 74	0.13 (1)
L- C	-1808 / 0	0.64 (1)
E- H	-1928 / 0	0.63 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MAES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.65/1.00 (C-D:1) , BC=0.71/1.00 (H-I:1) , WB=0.64/1.00 (C-L:1) , SSI=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1637	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)

DWG NO. TAM T1605295
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T161	1	2	TRUSS DESC.	

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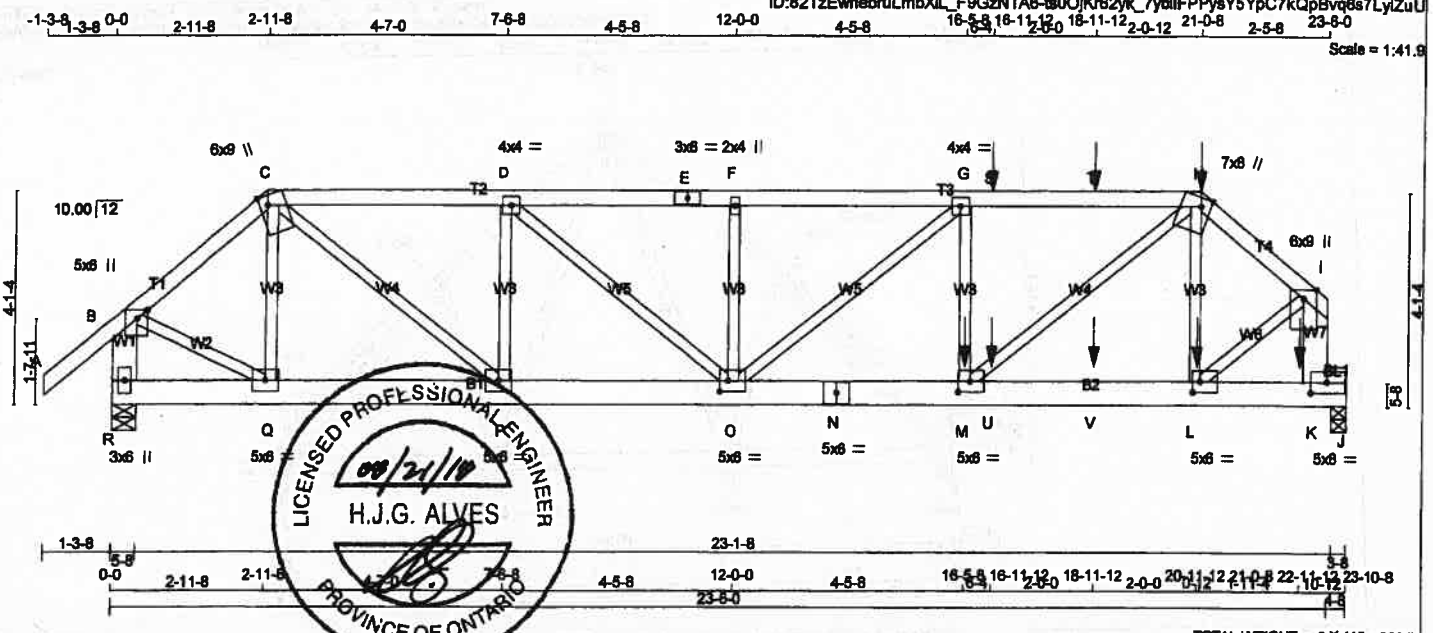

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- I	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- M	1	5	SIDE(343.8)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	8.0	8.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW+p	MT20	6.0	9.0	Edge	
K	BMVK-t	MT20	5.0	8.0	2.50	3.75
L	BMWW-t	MT20	5.0	6.0	2.50	1.75
M	BMWW-t	MT20	5.0	8.0	2.50	2.75
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2973	0	2973	0	0	5-8	5-8
J	4217	0	4217	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2165	1402 / 0	331 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	3074	1984 / 0	469 / 0	0 / 0	0 / 0	802 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 57	-129.7	-129.7	0.10 (1)	10.00	
B- C	-2714 / 0	-129.7	-129.7	0.14 (1)	5.41	
C- D	-4430 / 0	-129.7	-129.7	0.30 (1)	4.28	
D- E	-5774 / 0	-129.7	-129.7	0.36 (1)	3.76	
E- F	-5774 / 0	-129.7	-129.7	0.36 (1)	3.76	
F- G	-5774 / 0	-129.7	-129.7	0.42 (1)	3.68	
G- S	-6244 / 0	-129.7	-129.7	0.50 (1)	3.46	
S- T	-6244 / 0	-129.7	-129.7	0.50 (1)	3.46	
T- H	-6244 / 0	-129.7	-129.7	0.50 (1)	3.46	
H- I	-4157 / 0	-129.7	-129.7	0.13 (1)	4.57	
R- B	-2947 / 0	0.0	0.0	0.11 (1)	7.61	
K- I	-5402 / 0	0.0	0.0	0.21 (1)	8.30	
R- Q	0 / 0	-38.5	-38.5	0.04 (2)	10.00	
Q- P	0 / 2080	-38.5	-38.5	0.17 (1)	10.00	
P- O	0 / 4431	-38.5	-38.5	0.33 (1)	10.00	
O- N	0 / 6245	-38.5	-38.5	0.51 (1)	10.00	
N- M	0 / 6245	-38.5	-38.5	0.51 (1)	10.00	
M- U	0 / 3157	-38.5	-38.5	0.42 (1)	10.00	
U- V	0 / 3157	-38.5	-38.5	0.42 (1)	10.00	
V- L	0 / 3157	-38.5	-38.5	0.42 (1)	10.00	
L- K	-748 / 0	-38.5	-38.5	0.83 (1)	8.25	
K- J	0 / 0	-168.2	-168.2	0.58 (1)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Q- C	-622 / 0	0.06 (1)
P- C	0 / 3048	0.38 (1)
P- D	-1776 / 0	0.23 (1)
O- D	0 / 1751	0.22 (1)
O- F	-499 / 0	0.06 (1)
O- G	-814 / 0	0.18 (1)
M- G	-493 / 8	0.06 (1)
M- H	0 / 3971	0.49 (1)
L- H	-797 / 0	0.10 (1)
B- Q	0 / 2237	0.28 (1)
L- I	0 / 4883	0.58 (1)

DWG NO. TAM T1805540
STRUCTURAL COMPONENT ONLY
1/2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	21-0-8	-28	-28	—	FRONT	VERT	DEAD
H	21-0-8	-112	-112	—	FRONT	VERT	TOTAL
H	21-0-8	-176	-176	—	FRONT	VERT	SNOW
K	22-11-12	-43	-43	—	FRONT	VERT	TOTAL
L	20-11-12	-44	-44	—	FRONT	VERT	TOTAL
M	16-5-8	-1458	-1458	—	FRONT	VERT	TOTAL
S	16-11-12	-106	-106	—	FRONT	VERT	TOTAL
T	16-11-12	-106	-106	—	FRONT	VERT	TOTAL
U	16-11-12	-38	-38	—	FRONT	VERT	TOTAL
V	18-11-12	-38	-38	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.50/1.00 (G-H:1) , BC=0.93/1.00 (K-L:1) , WB=0.58/1.00 (I-L:1) , SSI=0.85/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE RDTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.83 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T161	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 462.1 lbs FACTORED DOWN AT 21-0-8, AND 156.1 lbs FACTORED DOWN AT 18-11-12, AND 156.1 lbs FACTORED DOWN AT 18-11-12 ON TOP CHORD, AND 2019.8 lbs FACTORED DOWN AT 16-5-8, 54.9 lbs FACTORED DOWN AT 16-11-12, 54.9 lbs FACTORED DOWN AT 18-11-12, AND 61.9 lbs FACTORED DOWN AT 20-11-12, AND 61.1 lbs FACTORED DOWN AT 22-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. (MM) T1605549
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T162	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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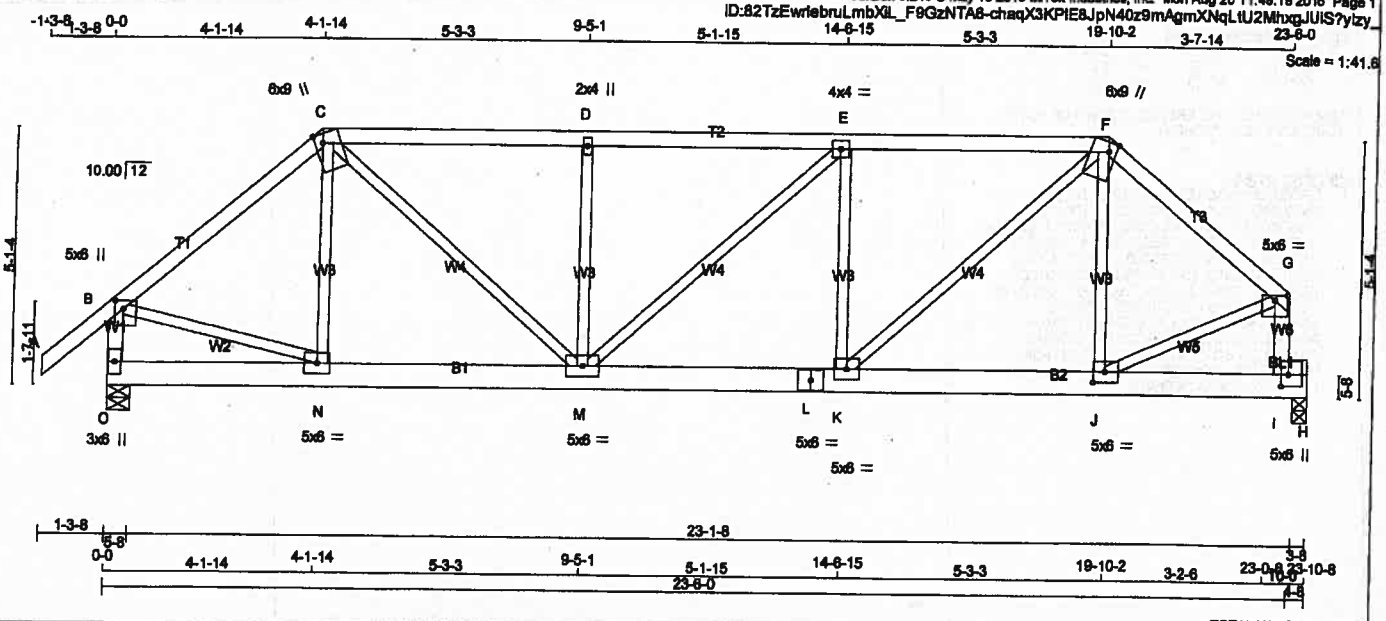

TOTAL WEIGHT = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.00
C	TTWW+m	MT20	6.0	8.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	8.0	Edge	1.75
G	TMVW-p	MT20	5.0	8.0	1.25	3.00
I	BMVX+p	MT20	5.0	8.0	2.75	1.75
J	BMWW-t	MT20	5.0	8.0	2.50	2.75
K	BMWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2167	0	2167	0	0	5-8	5-8
H	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1599	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
H	1441	884 / 0	251 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.16 (1)	10.00
B- C	-1943 / 0	-129.7	-129.7	0.48 (1)	4.30
C- D	-2495 / 0	-129.7	-129.7	0.61 (1)	3.69
D- E	-2496 / 0	-129.7	-129.7	0.62 (1)	3.66
E- F	-2479 / 0	-129.7	-129.7	0.63 (1)	3.67
F- G	-1951 / 0	-129.7	-129.7	0.37 (1)	4.43
O- B	-2124 / 0	0.0	0.0	0.22 (1)	5.81
I- G	-2238 / 0	0.0	0.0	0.25 (1)	5.68
O- N	0 / 0	-38.5	-38.5	0.07 (3)	10.00
N- M	0 / 1484	-38.5	-38.5	0.23 (1)	10.00
M- L	0 / 2478	-38.5	-38.5	0.37 (1)	10.00
L- K	0 / 2479	-38.5	-38.5	0.37 (1)	10.00
K- J	0 / 1481	-38.5	-38.5	0.37 (1)	10.00
J- I	-198 / 0	-38.5	-38.5	0.68 (1)	6.25
I- H	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-214 / 81	0.08 (1)
C- M	0 / 1357	0.31 (1)
M- D	-727 / 0	0.26 (1)
M- E	0 / 23	0.01 (1)
K- E	-747 / 0	0.27 (1)
K- F	0 / 1325	0.30 (1)
J- F	-153 / 104	0.05 (1)
B- N	0 / 1548	0.35 (1)
J- G	0 / 1848	0.42 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AWES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.68/1.00 (I-J:1) , WB=0.42/1.00 (G-J:1) , SSI=0.72/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.47 (L) (INPUT = 1.00)

DWG NO. TAM 71805240
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T163	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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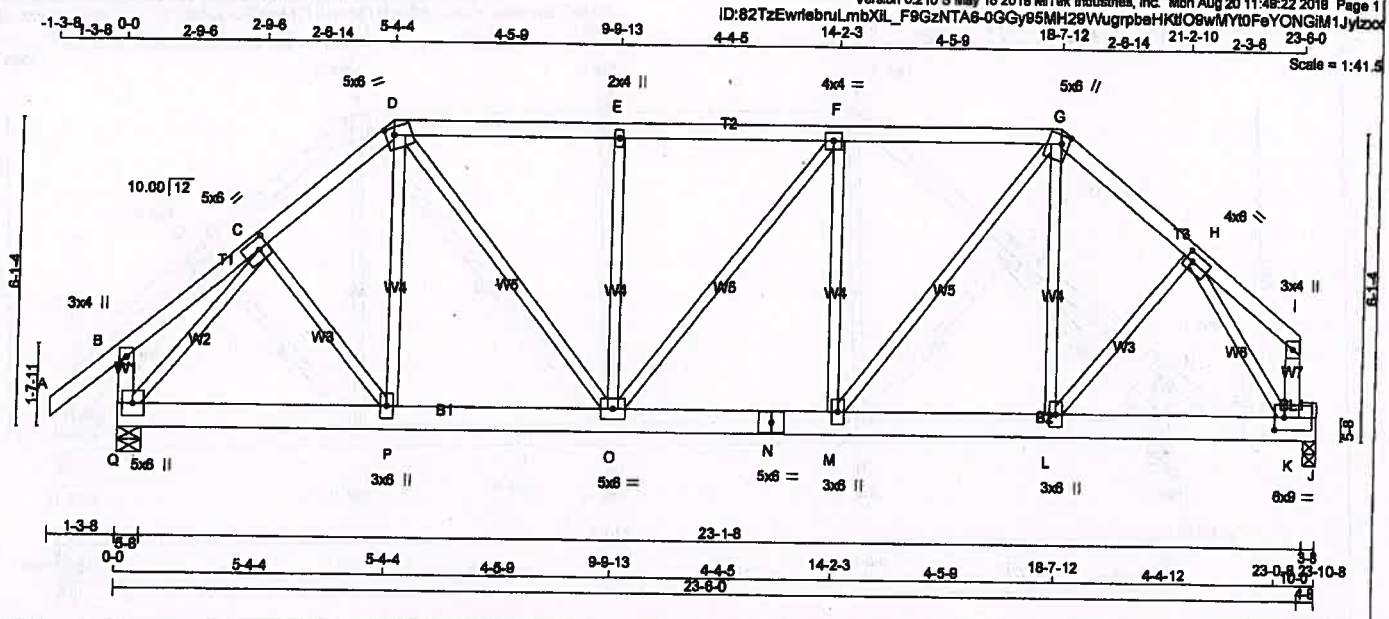

TOTAL WEIGHT = 129 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	8.0	2.00	1.75
H	TMWW-t	MT20	4.0	6.0	2.00	1.75
I	TMV+p	MT20	3.0	4.0		
K	BMVWK-t	MT20	6.0	8.0	3.00	2.25
L, M, P						
L	BMWW+t	MT20	3.0	6.0		
N	BB-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
Q	BMVW1+p	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2167	0	2167	0	0	5-8	5-8
J	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1598	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1441	864 / 0	251 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN BPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	0 / 21	-129.7	-129.7	0.12 (1)		10.00
C- D	-1951 / 0	-129.7	-129.7	0.17 (1)		4.68
D- E	-2068 / 0	-129.7	-129.7	0.40 (1)		4.28
E- F	-2068 / 0	-129.7	-129.7	0.41 (1)		4.25
F- G	-2054 / 0	-129.7	-129.7	0.41 (1)		4.27
G- H	-1872 / 0	-129.7	-129.7	0.18 (1)		4.67
H- I	0 / 33	-129.7	-129.7	0.10 (1)		10.00
Q- B	-318 / 0	0.0	0.0	0.09 (1)		7.81
K- I	-102 / 0	0.0	0.0	0.01 (1)		7.81
Q- P	0 / 1375	-38.5	-38.5	0.23 (1)		10.00
P- O	0 / 1478	-38.5	-38.5	0.23 (1)		10.00
O- N	0 / 2054	-38.5	-38.5	0.31 (1)		10.00
N- M	0 / 2054	-38.5	-38.5	0.31 (1)		10.00
M- L	0 / 1498	-38.5	-38.5	0.40 (1)		10.00
L- K	0 / 1167	-38.5	-38.5	0.62 (1)		10.00
K- J	0 / 0	-38.5	-38.5	0.42 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 171	0.04 (2)
P- D	0 / 173	0.04 (3)
L- G	0 / 198	0.05 (3)
L- H	0 / 484	0.11 (1)
Q- C	-2248 / 0	0.68 (1)
H- K	-2353 / 0	0.62 (1)
M- G	0 / 902	0.20 (1)
D- O	0 / 957	0.22 (1)
M- F	-637 / 0	0.35 (1)
O- E	-612 / 0	0.33 (1)
O- F	0 / 23	0.01 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	36.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.41/1.00 (F-G:1) , BC=0.82/1.00 (K-L:1) , WB=0.68/1.00 (C-Q:1) , SSI=0.72/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

DWG NO. TAM T1805241
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T164	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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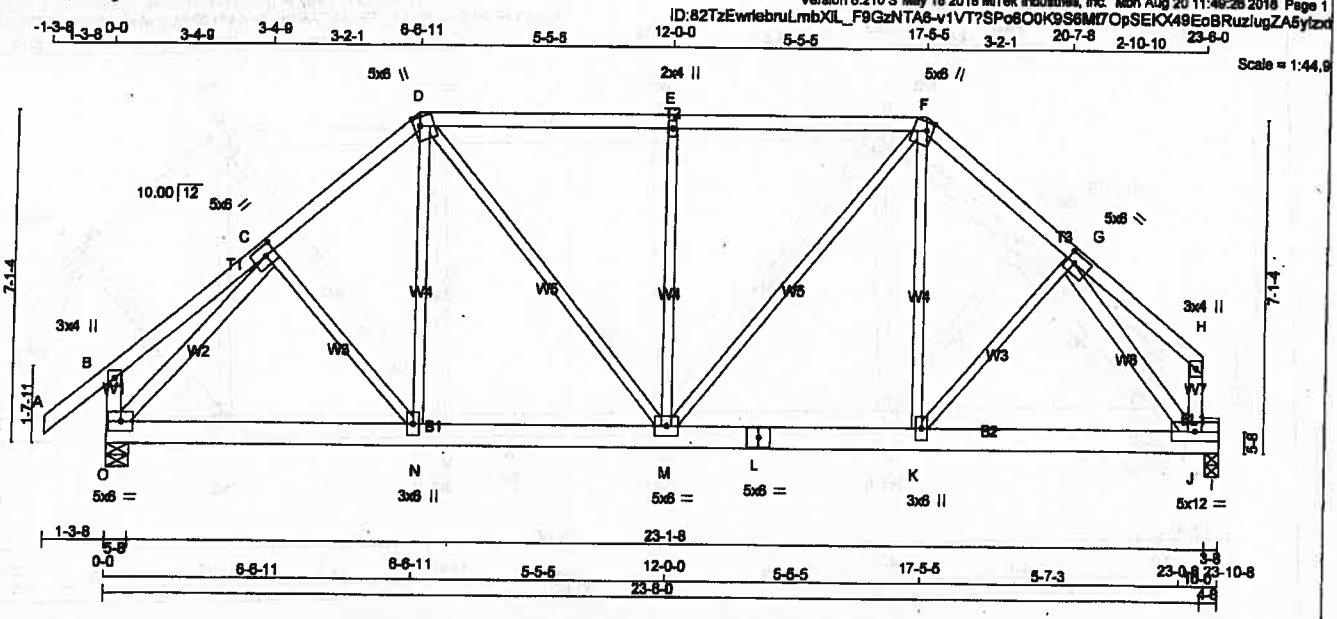

TOTAL WEIGHT = 127 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	8.0	2.50	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	5.0	12.0		
K	BMWW+t	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2167	0	2167	0	0	5-8	5-8
I	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1598	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
I	1441	884 / 0	251 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 57	-128.7	-128.7	1	0.18 (1)	10.00	C- N	0 / 100	0.02 (3)
B- C	0 / 30	-128.7	-128.7	1	0.21 (1)	10.00	N- D	0 / 322	0.07 (2)
C- D	-1818 / 0	-128.7	-128.7	1	0.27 (1)	4.60	D- M	0 / 623	0.14 (1)
D- E	-1841 / 0	-129.7	-129.7	1	0.75 (1)	3.85	M- E	-861 / 0	0.71 (1)
E- F	-1841 / 0	-128.7	-128.7	1	0.75 (1)	3.85	M- F	0 / 592	0.13 (1)
F- G	-1937 / 0	-129.7	-129.7	1	0.24 (1)	4.61	K- F	0 / 348	0.06 (2)
G- H	0 / 45	-129.7	-129.7	1	0.18 (1)	10.00	K- G	0 / 290	0.07 (1)
O- B	-343 / 0	0.0	0.0	1	0.04 (1)	7.61	O- C	-2257 / 0	0.69 (1)
J- H	-121 / 0	0.0	0.0	1	0.01 (1)	7.61	G- J	-2325 / 0	0.59 (1)
O- N	0 / 1438	-38.5	-38.5	1	0.28 (1)	10.00			
N- M	0 / 1449	-38.5	-38.5	2	0.28 (2)	10.00			
M- L	0 / 1469	-38.5	-38.5	1	0.44 (1)	10.00			
L- K	0 / 1469	-38.5	-38.5	1	0.44 (1)	10.00			
K- J	0 / 1273	-38.5	-38.5	1	0.83 (1)	10.00			
J- I	0 / 0	-38.5	-38.5	1	0.42 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AYRES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.83/1.00 (J-K:1) , WB=0.71/1.00 (E-M:1) , SSI=0.72/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.56 (G) (INPUT = 1.00)

DWG NO. TAM T1805242
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T165	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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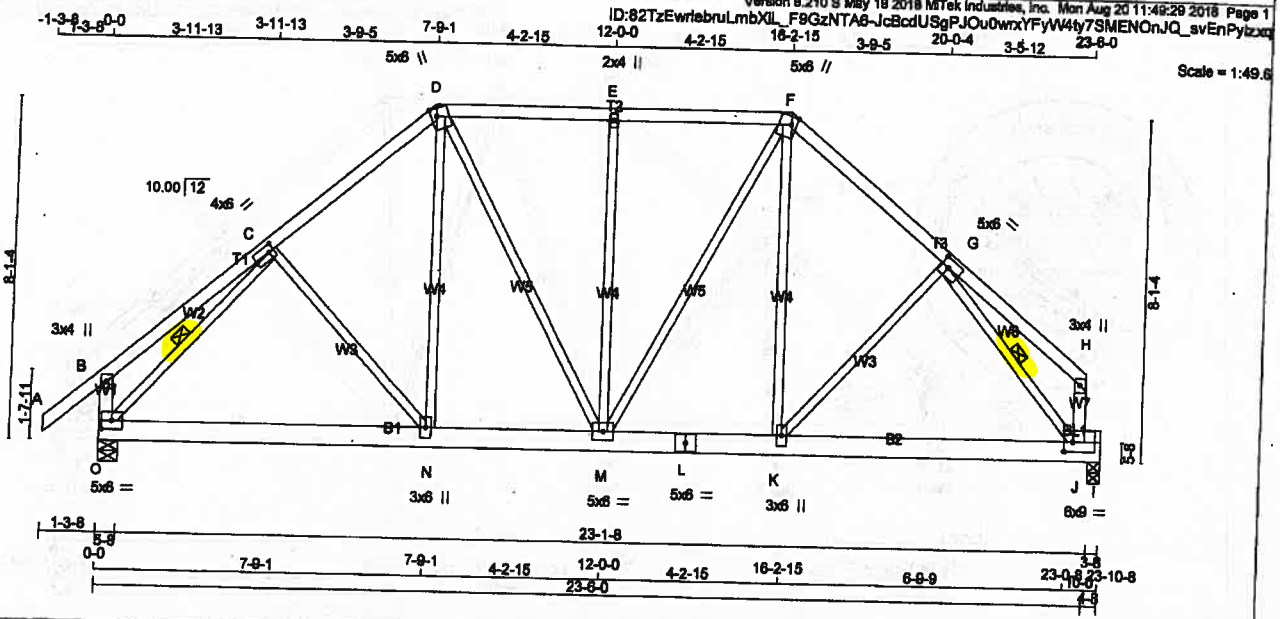

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	6.0	9.0	2.75	2.50
K	BMWW+t	MT20	3.0	8.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2187	0	2187	0	0	5-8	5-8
I	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1598	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
I	1441	864 / 0	251 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 37	-129.7	-129.7	0.31 (1)	10.00
C- D	-1845 / 0	-129.7	-129.7	0.36 (1)	4.53
D- E	-1571 / 0	-129.7	-129.7	0.42 (1)	4.72
E- F	-1571 / 0	-129.7	-129.7	0.42 (1)	4.72
F- G	-1674 / 0	-129.7	-129.7	0.34 (1)	4.55
G- H	0 / 54	-129.7	-129.7	0.27 (1)	10.00
O- B	-372 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-148 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 1469	-38.5	-38.5	0.32 (2)	10.00
N- M	0 / 1381	-38.5	-38.5	0.31 (2)	10.00
M- L	0 / 1417	-38.5	-38.5	0.49 (1)	10.00
L- K	0 / 1417	-38.5	-38.5	0.49 (1)	10.00
K- J	0 / 1332	-38.5	-38.5	0.84 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-122 / 68	0.07 (1)
N- D	0 / 445	0.10 (2)
D- M	0 / 373	0.08 (1)
M- E	-663 / 0	0.79 (1)
M- F	0 / 318	0.07 (1)
K- F	0 / 492	0.11 (2)
K- G	0 / 147	0.03 (3)
O- C	-2237 / 0	0.54 (1)
G- J	-2296 / 0	0.52 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

TOTAL WEIGHT = 130 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 36.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.84/1.00 (J-K:1) , WB=0.79/1.00 (E-M:1) , SSI=0.72/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.56 (G) (INPUT = 1.00)

DWG NO. TAM 1805243
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T166	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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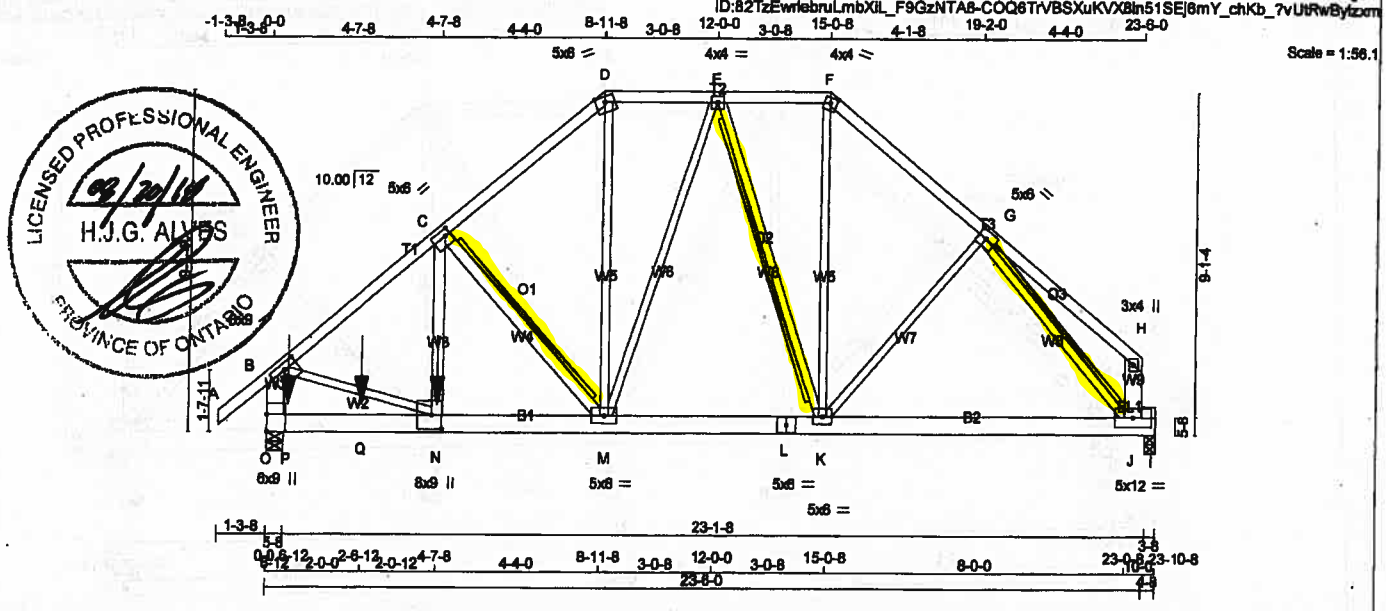

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- H	1	12	TOP
O- B	2	3	SIDE(60.6)
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(183.1)
L- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
C- N	1	2	SIDE(1314.2)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.00	4.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	1.75	2.00
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVWK-t	MT20	6.0	12.0		
K	BMWWW-1	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	8.0	8.0	4.50	3.25
O	BMV1+1	MT20	6.0	8.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	6746	0	6748	0	0	5-8	5-8
I	3286	0	3288	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	6375	4103 / 0	999 / 0	0 / 0	0 / 0	1276 / 0	0 / 0
I	2404	1515 / 0	383 / 0	0 / 0	0 / 0	496 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-M, G-J, E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 57	-129.7	-129.7	0.10 (1)	10.00	N- C	0 / 5150	0.46 (1)	
B- C	-8208 / 0	-129.7	-129.7	0.61 (1)	3.06	C- M	-4508 / 0	0.79 (1)	
C- D	-4434 / 0	-129.7	-129.7	0.32 (1)	4.26	M- D	0 / 2408	0.30 (1)	
D- E	-3397 / 0	-129.7	-129.7	0.12 (1)	4.96	K- F	0 / 1810	0.22 (1)	
E- F	-2665 / 0	-129.7	-129.7	0.11 (1)	5.47	K- G	0 / 343	0.04 (1)	
F- G	-3488 / 0	-129.7	-129.7	0.27 (1)	4.77	B- N	0 / 6550	0.81 (1)	
G- H	0 / 73	-129.7	-129.7	0.20 (1)	10.00	G- J	-4004 / 0	0.80 (1)	
O- B	-7374 / 0	0.0	0.0	0.27 (1)	5.52	M- E	0 / 934	0.12 (1)	
J- H	-192 / 0	0.0	0.0	0.01 (1)	7.61	E- K	-1359 / 0	0.81 (1)	
O- P	0 / 0	-38.5	-38.5	0.50 (1)	10.00				
P- Q	0 / 0	-38.5	-38.5	0.50 (1)	10.00				
Q- N	0 / 0	-38.5	-38.5	0.50 (1)	10.00				
N- M	0 / 6333	-38.5	-38.5	0.54 (1)	10.00				
M- L	0 / 3099	-38.5	-38.5	0.51 (1)	10.00				
L- K	0 / 3099	-38.5	-38.5	0.51 (1)	10.00				
K- J	0 / 2422	-38.5	-38.5	0.68 (1)	10.00				
J- I	0 / 0	-168.2	-168.2	0.45 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	4-7-8	-4185	-4185	—	FRONT	VERT	TOTAL
P	8-12	-736	-736	—	FRONT	VERT	TOTAL
Q	2-8-12	-768	-768	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 984 (0.29")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.68/1.00 (J-K:1) , WB=0.61/1.00 (B-N:1) , SSI=0.67/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.60)
JSI METAL= 0.71 (N) (INPUT = 1.00)

DWG NO. TAM T1605244
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T166	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1010.7 lbs FACTORED DOWN AT 8-12, AND 1057.1 lbs FACTORED DOWN AT 2-8-12, AND 5739.6 lbs FACTORED DOWN AT 4-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AIKEN
PROVINCE OF ONTARIO

DWG NO. TAM T1805244
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T167A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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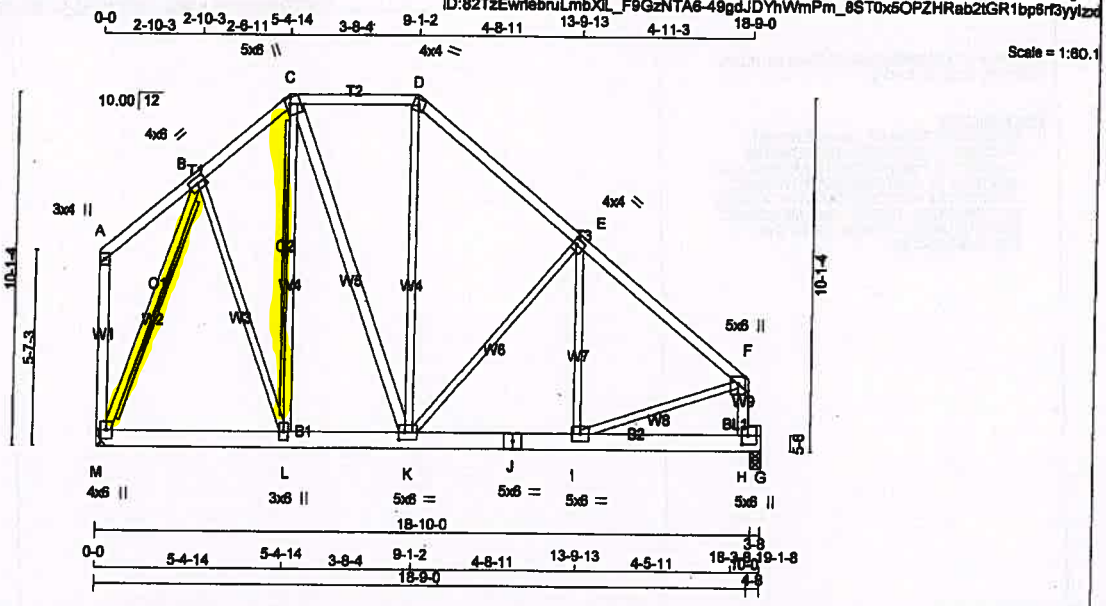

Scale = 1:80.1

TOTAL WEIGHT = 122 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	8.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	8.0	2.50	2.25
H	BMVK+p	MT20	5.0	8.0	3.00	2.00
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	2.00
L	BMWW+t	MT20	3.0	8.0		
M	BMVW1+p	MT20	4.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1580	0	1580	0	0	3-8	3-8
M	1608	0	1608	0	0	MECHANICAL	= LSS26DS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1148	703 / 0	201 / 0	0 / 0	0 / 0	244 / 0	0 / 0
M	1160	731 / 0	201 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-L, B-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.15 (1)	10.00
B- C	-882 / 0	-129.7	-129.7	0.16 (1)	6.25
C- D	-784 / 0	-129.7	-129.7	0.31 (1)	6.25
D- E	-1070 / 0	-129.7	-129.7	0.56 (1)	5.25
E- F	-1468 / 0	-129.7	-129.7	0.61 (1)	4.56
M- A	-143 / 0	0.0	0.0	0.08 (1)	7.81
H- F	-1677 / 0	0.0	0.0	0.18 (1)	6.41
M- L	0 / 512	-38.5	-38.5	0.14 (2)	10.00
L- K	0 / 663	-38.5	-38.5	0.15 (2)	10.00
K- J	0 / 1160	-38.5	-38.5	0.31 (1)	10.00
J- I	0 / 1160	-38.5	-38.5	0.31 (1)	10.00
I- H	-148 / 0	-38.5	-38.5	0.54 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-169 / 51	0.13 (1)
C- K	0 / 376	0.08 (1)
K- D	0 / 216	0.05 (2)
K- E	-620 / 0	0.68 (1)
I- E	0 / 204	0.05 (3)
I- F	0 / 1400	0.32 (1)
B- L	0 / 420	0.09 (1)
M- B	-1468 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.54/1.00 (H-I:1) , WB=0.68/1.00 (E-K:1) , SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (I) (INPUT = 0.80)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. RIVES
PROVINCE OF ONTARIO

DWG NO. TAM 17805245
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T168A	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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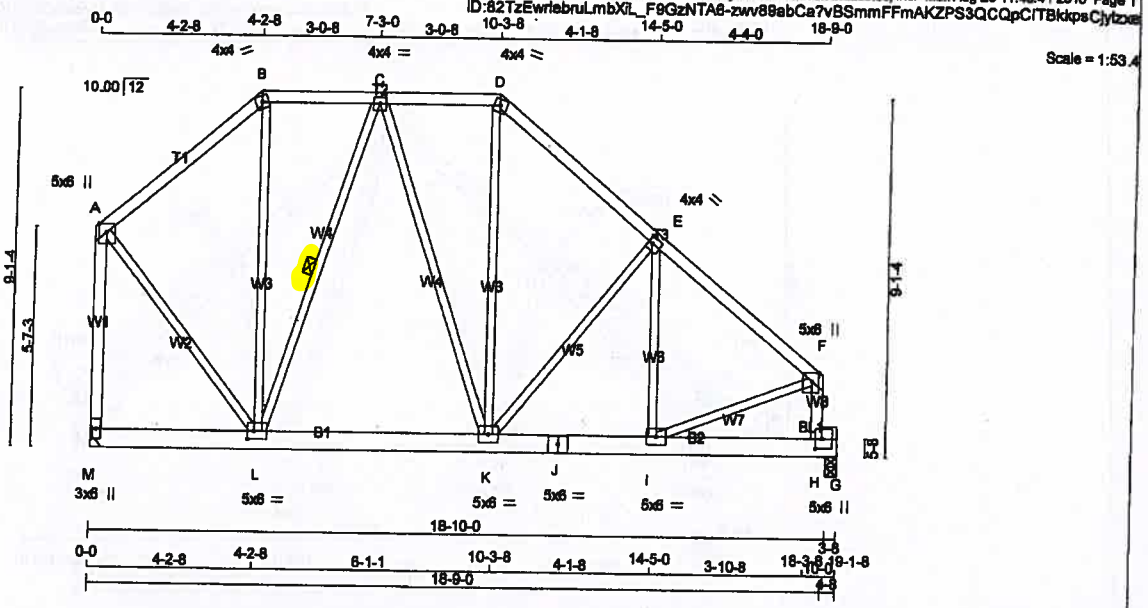

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	2.50	2.25
H	BMVK+p	MT20	5.0	6.0	3.00	2.00
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1608	0	1608	0	0	MECHANICAL - LSS26DS	
G	1580	0	1580	0	0	3-8	3-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1180	731 / 0	201 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	1148	703 / 0	201 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-818 / 0	-129.7	-129.7	0.42 (1)	6.08
B- C	-626 / 0	-129.7	-129.7	0.18 (1)	6.25
C- D	-681 / 0	-129.7	-129.7	0.18 (1)	6.25
D- E	-1167 / 0	-129.7	-129.7	0.42 (1)	5.29
E- F	-1494 / 0	-129.7	-129.7	0.45 (1)	4.62
M- A	-1548 / 0	0.0	0.0	0.67 (1)	6.81
H- F	-1709 / 0	0.0	0.0	0.18 (1)	6.38
M- L	0 / 0	-38.5	-38.5	0.09 (3)	10.00
L- K	0 / 833	-38.5	-38.5	0.17 (2)	10.00
K- J	0 / 1178	-38.5	-38.5	0.26 (1)	10.00
J- I	0 / 1178	-38.5	-38.5	0.26 (1)	10.00
I- H	-152 / 0	-38.5	-38.5	0.54 (1)	6.25
H- G	0 / 0	-38.5	-38.5	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	0 / 155	0.04 (3)
L- C	-648 / 0	0.38 (1)
C- K	0 / 158	0.04 (2)
K- D	0 / 375	0.08 (1)
K- E	-483 / 0	0.39 (1)
I- E	-56 / 133	0.03 (3)
A- L	0 / 977	0.22 (1)
I- F	0 / 1423	0.32 (1)

TOTAL WEIGHT = 5 X 114 = 571 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.67/1.00 (A-M:1) , BC=0.54/1.00 (H-I:1) , WB=0.39/1.00 (E-K:1) , BSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AYRES
PROVINCE OF ONTARIO

DWG NO. TAM T1805246
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T169	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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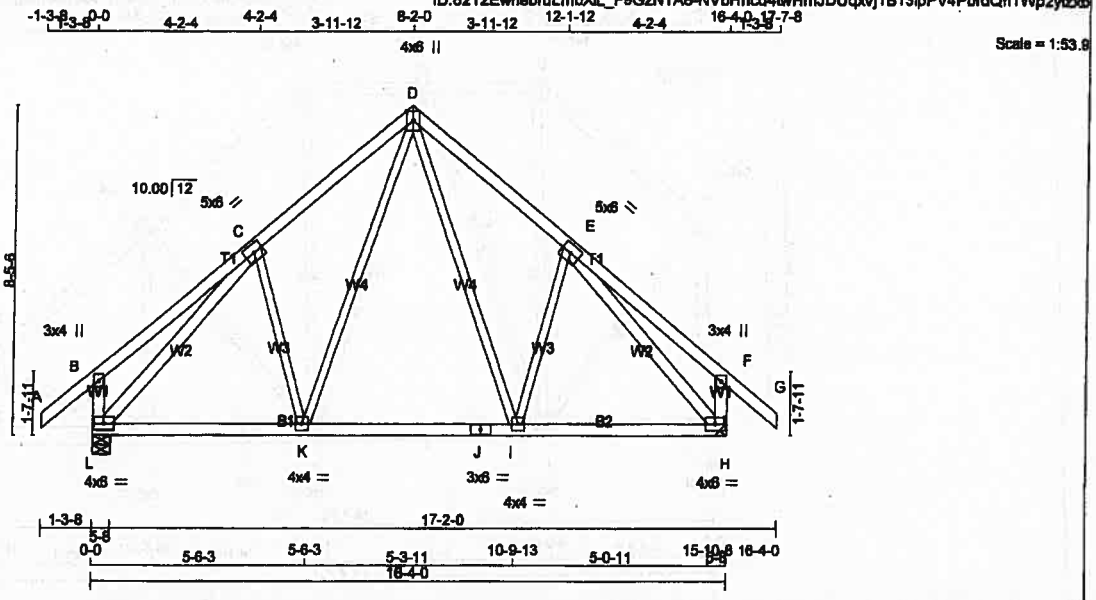

TOTAL WEIGHT = 2 X 84 = 169 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1553	0	1553	0	0	5-8	5-8
H	1553	0	1553	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1131	730 / 0	172 / 0	0 / 0	0 / 0	229 / 0	0 / 0
H	1131	730 / 0	172 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.56 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 39	-129.7	-129.7	0.35 (1)	10.00
C- D	-1160 / 0	-129.7	-129.7	0.28 (1)	5.56
D- E	-1160 / 0	-129.7	-129.7	0.28 (1)	5.56
E- F	0 / 39	-129.7	-129.7	0.35 (1)	10.00
F- G	0 / 57	-129.7	-129.7	0.18 (1)	10.00
L- B	-387 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-387 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 940	-38.5	-38.5	0.31 (2)	10.00
K- J	0 / 887	-38.5	-38.5	0.28 (2)	10.00
J- I	0 / 887	-38.5	-38.5	0.28 (2)	10.00
I- H	0 / 940	-38.5	-38.5	0.31 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 547	0.12 (1)
I- E	-301 / 28	0.12 (1)
K- D	0 / 547	0.12 (1)
C- K	-301 / 28	0.12 (1)
L- C	-1432 / 0	0.87 (1)
E- H	-1432 / 0	0.87 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.31/1.00 (K-L:2) , WB=0.87/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM 1805297
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G169	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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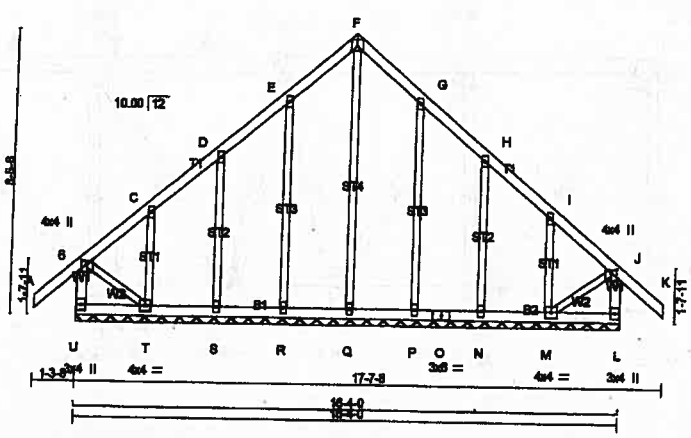

TOTAL WEIGHT = 82 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, P, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U-B	-350 / 0	0.0	0.0	0.04 (1)	7.81
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B-C	-40 / 0	-129.7	-129.7	0.08 (1)	6.25
C-D	-52 / 0	-129.7	-129.7	0.08 (1)	6.25
D-E	-34 / 0	-129.7	-129.7	0.08 (1)	6.25
E-F	-51 / 0	-129.7	-129.7	0.08 (1)	6.25
F-G	-51 / 0	-129.7	-129.7	0.08 (1)	6.25
G-H	-34 / 0	-129.7	-129.7	0.08 (1)	6.25
H-I	-52 / 0	-129.7	-129.7	0.08 (1)	6.25
I-J	-40 / 0	-129.7	-129.7	0.08 (1)	6.25
J-K	0 / 57	-129.7	-129.7	0.16 (1)	10.00
L-J	-350 / 0	0.0	0.0	0.04 (1)	7.81
U-T	0 / 0	-38.5	-38.5	0.03 (3)	10.00
T-S	0 / 35	-38.5	-38.5	0.04 (2)	10.00
S-R	0 / 30	-38.5	-38.5	0.03 (2)	10.00
R-Q	0 / 29	-38.5	-38.5	0.03 (2)	10.00
Q-P	0 / 29	-38.5	-38.5	0.03 (2)	10.00
P-O	0 / 30	-38.5	-38.5	0.03 (2)	10.00
O-N	0 / 30	-38.5	-38.5	0.03 (2)	10.00
N-M	0 / 35	-38.5	-38.5	0.04 (2)	10.00
M-L	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-F	-175 / 0	0.24 (1)
R-E	-297 / 0	0.23 (1)
S-D	-238 / 0	0.09 (1)
T-C	-293 / 0	0.08 (1)
P-G	-297 / 0	0.23 (1)
N-H	-238 / 0	0.09 (1)
M-I	-293 / 0	0.08 (1)
B-T	0 / 51	0.01 (1)
M-J	0 / 51	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (J-K:1) , BC=0.04/1.00 (M-N:2) , WB=0.24/1.00 (F-Q:1) , SSI=0.11/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM F1805546
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T170X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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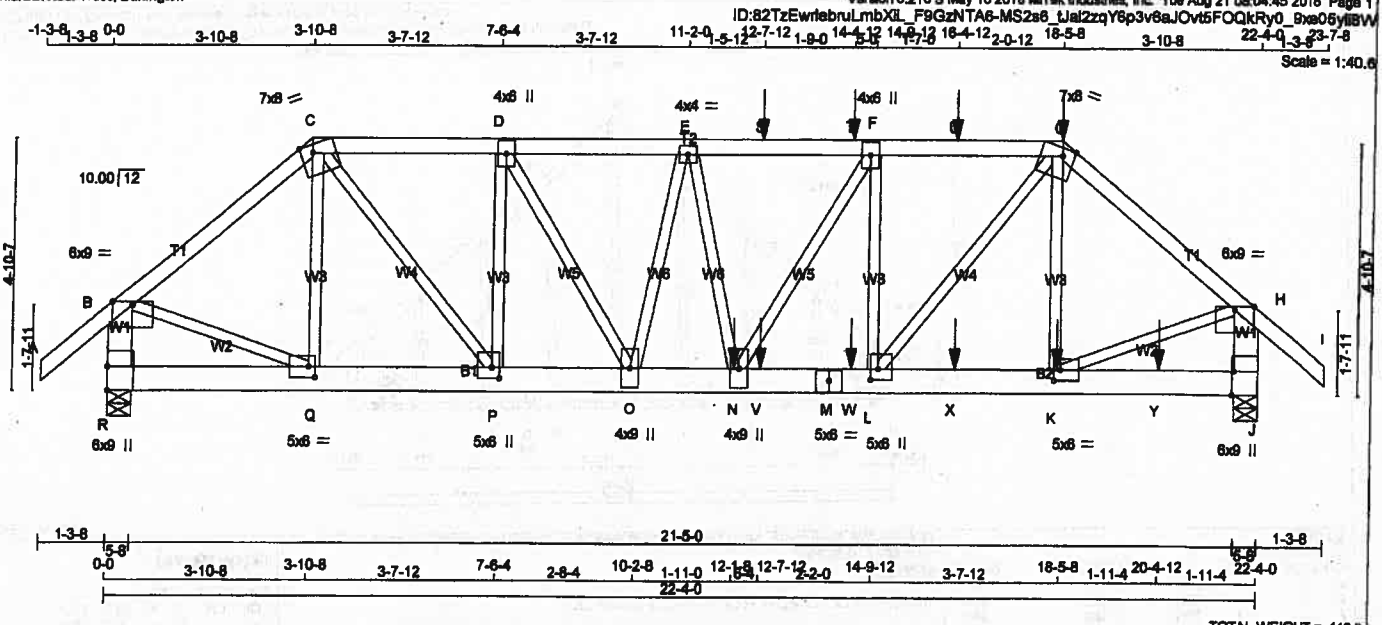

TOTAL WEIGHT = 119 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	6.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW+t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+t	MT20	4.0	8.0		
G	TTWW-m	MT20	7.0	6.0	Edge	2.75
H	TMWW-p	MT20	6.0	9.0	Edge	
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWW+t	MT20	5.0	6.0	2.50	1.75
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWW+t	MT20	4.0	9.0		
P	BMWW+t	MT20	5.0	6.0	2.50	1.75
Q	BMWW-t	MT20	5.0	6.0	2.50	1.50
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.8 lbs FACTORED DOWN AT 12-7-12, 121.8 lbs FACTORED DOWN AT 14-4-12, AND 121.8 lbs FACTORED DOWN AT 16-4-12, AND 281.9 lbs FACTORED DOWN AT 18-5-8 ON TOP CHORD, AND 1888.8 lbs FACTORED DOWN AT 12-1-8, 45.8 lbs FACTORED DOWN AT 12-7-12, 45.8 lbs FACTORED DOWN AT 14-4-12, 45.8 lbs FACTORED DOWN AT 18-4-12, AND 45.8 lbs FACTORED DOWN AT 18-4-12, AND 36.3 lbs FACTORED DOWN AT 20-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3049	0	3049	0	0	5-8	5-8
J	3567	0	3567	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2217	1442 / 0	329 / 0	0 / 0	0 / 0	446 / 0	0 / 0
J	2595	1681 / 0	389 / 0	0 / 0	0 / 0	525 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.20 (1)	10.00
B-C	-2914 / 0	-129.7	-129.7	0.58 (1)	3.49
C-D	-3778 / 0	-129.7	-129.7	0.45 (1)	3.18
D-E	-4538 / 0	-129.7	-129.7	0.54 (1)	2.60
E-S	-4915 / 0	-129.7	-129.7	0.74 (1)	2.38
S-T	-4915 / 0	-129.7	-129.7	0.74 (1)	2.38
T-F	-4915 / 0	-129.7	-129.7	0.74 (1)	2.38
F-U	-4331 / 0	-129.7	-129.7	0.66 (1)	2.68
U-G	-4331 / 0	-129.7	-129.7	0.66 (1)	2.68
G-H	-3491 / 0	-129.7	-129.7	0.64 (1)	3.13
H-I	0 / 57	-129.7	-129.7	0.20 (1)	10.00
R-B	-2989 / 0	0.0	0.0	0.22 (1)	6.04
J-H	-3496 / 0	0.0	0.0	0.26 (1)	5.85
R-Q	0 / 0	-38.5	-38.5	0.06 (2)	10.00
Q-P	0 / 2221	-38.5	-38.5	0.34 (1)	10.00
P-O	0 / 3778	-38.5	-38.5	0.54 (1)	10.00
O-N	0 / 4786	-38.5	-38.5	0.83 (1)	10.00
N-V	0 / 4331	-38.5	-38.5	0.77 (1)	10.00
V-M	0 / 4331	-38.5	-38.5	0.77 (1)	10.00
M-W	0 / 4331	-38.5	-38.5	0.77 (1)	10.00
W-L	0 / 4331	-38.5	-38.5	0.77 (1)	10.00
L-X	0 / 2661	-38.5	-38.5	0.39 (1)	10.00
X-K	0 / 2661	-38.5	-38.5	0.39 (1)	10.00
K-Y	0 / 0	-38.5	-38.5	0.07 (3)	10.00
Y-J	0 / 0	-38.5	-38.5	0.07 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-504 / 0	0.17 (1)
C-P	0 / 2462	0.61 (1)
P-D	-1794 / 0	0.61 (1)
D-O	0 / 1503	0.37 (1)
O-E	-1132 / 0	0.40 (1)
E-N	0 / 598	0.15 (1)
N-F	0 / 1153	0.29 (1)
L-F	-1762 / 0	0.60 (1)
L-G	0 / 2644	0.65 (1)
K-G	-588 / 0	0.20 (1)
B-Q	0 / 2335	0.58 (1)
K-H	0 / 2788	0.69 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	18-5-8	-195	-195	—	BACK	VERT	TOTAL
K	18-4-12	-33	-33	—	BACK	VERT	TOTAL
N	12-1-8	-1214	-1214	—	BACK	VERT	TOTAL
S	12-7-12	-83	-83	—	BACK	VERT	TOTAL
T	14-4-12	-83	-83	—	BACK	VERT	TOTAL
U	16-4-12	-83	-83	—	BACK	VERT	TOTAL
V	12-7-12	-33	-33	—	BACK	VERT	TOTAL
W	14-4-12	-33	-33	—	BACK	VERT	TOTAL
X	16-4-12	-33	-33	—	BACK	VERT	TOTAL
Y	20-4-12	-22	-28	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. AIKES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.74/1.00 (E-F:1) , BC=0.83/1.00 (N-O:1) , WB=0.69/1.00 (H-K:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (P) (INPUT = 0.90)
JSI METAL= 0.84 (M) (INPUT = 1.00)

T1905390

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	T171X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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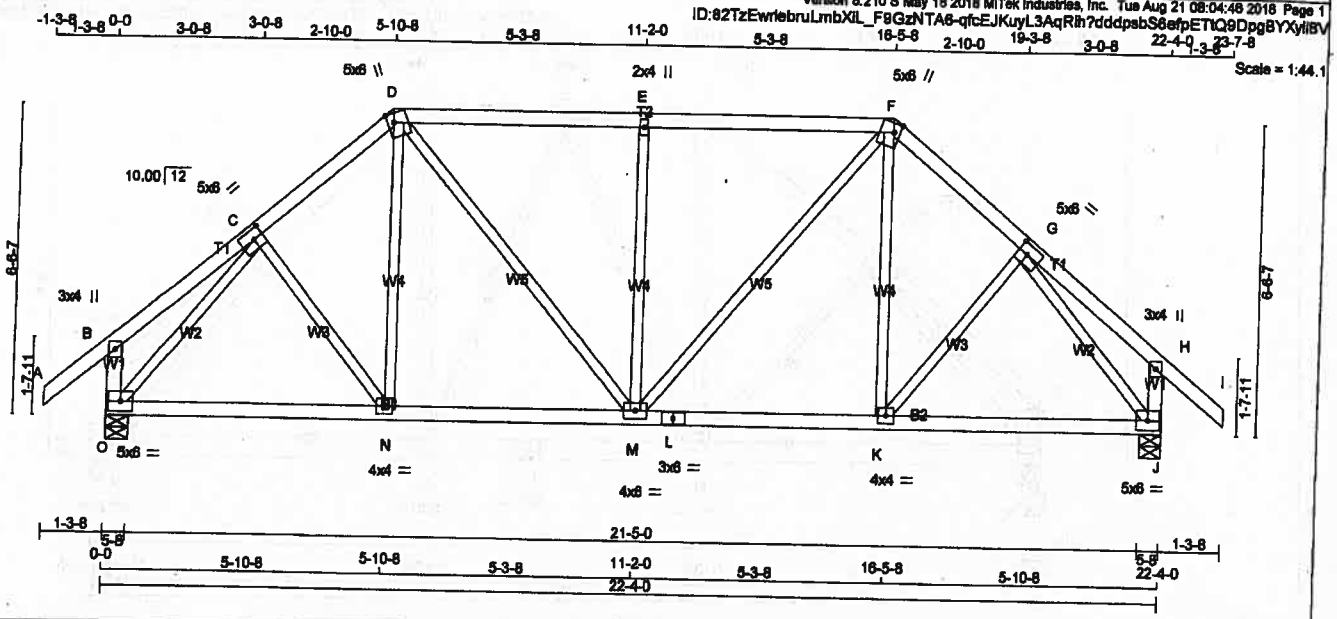

TOTAL WEIGHT = 104 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	8.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	8.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2057	0	2057	0	0	5-8	5-8
J	2057	0	2057	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1501	959 / 0	235 / 0	0 / 0	0 / 0	307 / 0	0 / 0
J	1501	959 / 0	235 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 25	-129.7	-129.7	0.18 (1)	10.00
C- D	-1757 / 0	-129.7	-129.7	0.18 (1)	4.91
D- E	-1756 / 0	-129.7	-129.7	0.50 (1)	4.44
E- F	-1756 / 0	-129.7	-129.7	0.50 (1)	4.44
F- G	-1757 / 0	-129.7	-129.7	0.18 (1)	4.81
G- H	0 / 25	-129.7	-129.7	0.18 (1)	10.00
H- I	0 / 57	-129.7	-129.7	0.18 (1)	10.00
O- B	-329 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-329 / 0	0.0	0.0	0.04 (1)	7.81
O- N	0 / 1275	-38.5	-38.5	0.40 (2)	10.00
N- M	0 / 1329	-38.5	-38.5	0.41 (2)	10.00
M- L	0 / 1329	-38.5	-38.5	0.41 (2)	10.00
L- K	0 / 1329	-38.5	-38.5	0.41 (2)	10.00
K- J	0 / 1275	-38.5	-38.5	0.40 (2)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- N	0 / 115	0.03 (3)
N- D	0 / 237	0.06 (3)
D- M	0 / 660	0.15 (1)
M- E	-636 / 0	0.58 (1)
M- F	0 / 660	0.15 (1)
K- F	0 / 237	0.06 (3)
K- G	0 / 115	0.03 (3)
O- C	-2073 / 0	0.74 (1)
G- J	-2073 / 0	0.74 (1)

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. ALVIS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.41/1.00 (M-N:2) , WB=0.74/1.00 (C-O:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (O) (INPUT = 0.60)
JSI METAL= 0.51 (G) (INPUT = 1.00)

T1905391
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T172	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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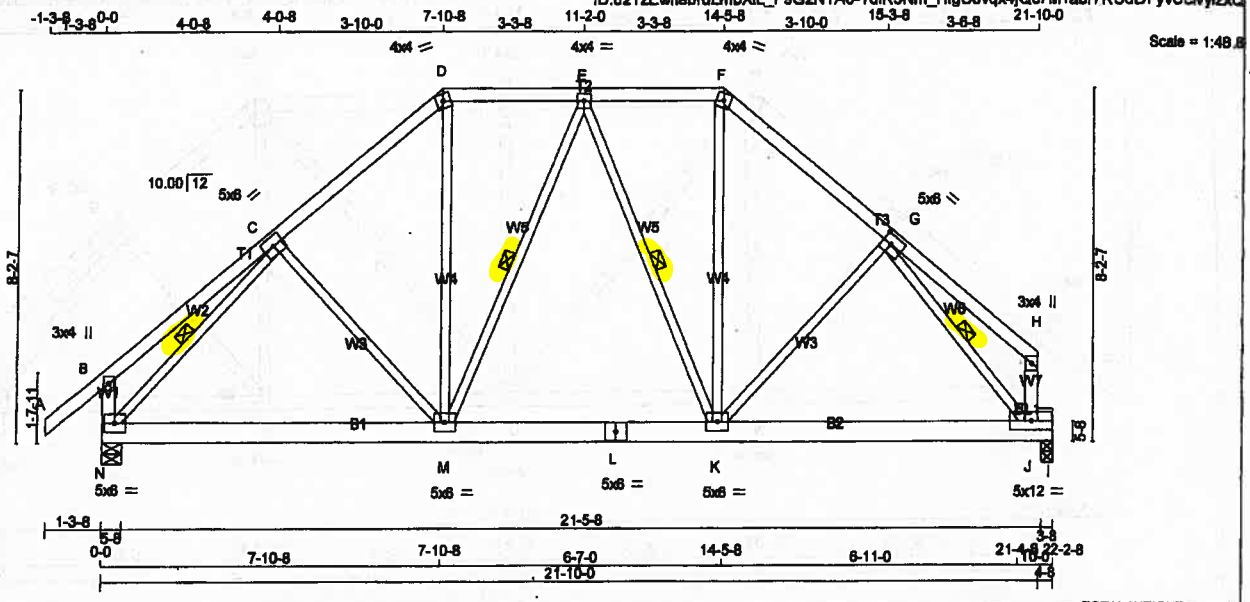

TOTAL WEIGHT = 119 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
B1,1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMWWK-t	MT20	5.0	12.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2048	0	2048	0	0	5-8	5-8
I	1819	0	1819	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1483	954 / 0	235 / 0	0 / 0	0 / 0	305 / 0	0 / 0
I	1338	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	0 / 37	-129.7	-129.7	0.32 (1)		10.00
C- D	-1640 / 0	-129.7	-129.7	0.38 (1)		4.75
D- E	-1238 / 0	-129.7	-129.7	0.23 (1)		5.49
E- F	-1264 / 0	-129.7	-129.7	0.23 (1)		5.44
F- G	-1871 / 0	-129.7	-129.7	0.34 (1)		4.77
G- H	0 / 55	-129.7	-129.7	0.28 (1)		10.00
N- B	-375 / 0	0.0	0.0	0.04 (1)		7.61
J- H	-151 / 0	0.0	0.0	0.02 (1)		7.61
N- M	0 / 1339	-38.5	-38.5	0.33 (2)		10.00
M- L	0 / 1353	-38.5	-38.5	0.44 (1)		10.00
L- K	0 / 1353	-38.5	-38.5	0.44 (1)		10.00
K- J	0 / 1218	-38.5	-38.5	0.77 (1)		10.00
J- I	0 / 0	-38.5	-38.5	0.39 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-188 / 54	0.10 (1)
M- D	0 / 656	0.15 (1)
M- E	-308 / 0	0.15 (1)
E- K	-235 / 0	0.11 (1)
K- F	0 / 671	0.15 (1)
K- G	0 / 121	0.03 (3)
N- C	-2037 / 0	0.50 (1)
G- J	-2094 / 0	0.48 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.77/1.00 (J-K:1) , WB=0.50/1.00 (C-N:1) , SSI=0.67/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

DWG NO. TAM T1805250
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T173	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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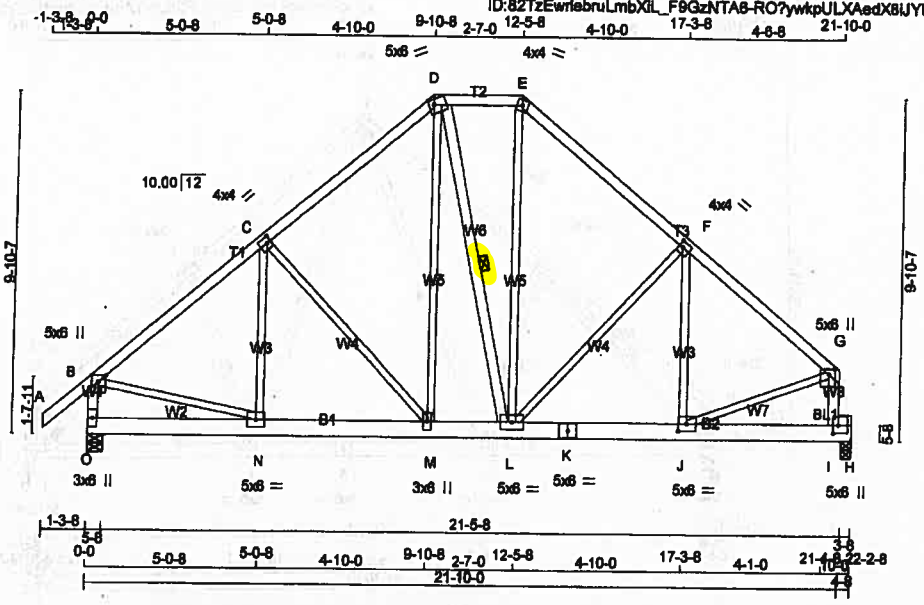

TOTAL WEIGHT = 130 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
I	BMVK+p	MT20	5.0	6.0	2.75	1.75
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2046	0	2046	0	0	5-8	5-8
H	1618	0	1619	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1493	954 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1338	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-1803 / 0	-129.7	-129.7	0.87 (1)	4.16
C- D	-1456 / 0	-129.7	-129.7	0.82 (1)	4.58
D- E	-1080 / 0	-129.7	-129.7	0.16 (1)	5.88
E- F	-1448 / 0	-129.7	-129.7	0.56 (1)	4.71
F- G	-1809 / 0	-129.7	-129.7	0.58 (1)	4.27
O- B	-1980 / 0	0.0	0.0	0.20 (1)	6.01
I- G	-1994 / 0	0.0	0.0	0.22 (1)	5.87
O- N	0 / 0	-38.5	-38.5	0.08 (3)	10.00
N- M	0 / 1422	-38.5	-38.5	0.22 (1)	10.00
M- L	0 / 1083	-38.5	-38.5	0.14 (1)	10.00
L- K	0 / 1427	-38.5	-38.5	0.34 (1)	10.00
K- J	0 / 1427	-38.5	-38.5	0.34 (1)	10.00
J- I	-177 / 0	-38.5	-38.5	0.63 (1)	8.25
I- H	0 / 0	-38.5	-38.5	0.39 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-134 / 152	0.07 (1)
C- M	-512 / 0	0.53 (1)
M- D	0 / 512	0.12 (1)
D- L	-12 / 5	0.01 (1)
L- E	0 / 461	0.11 (1)
L- F	-523 / 0	0.54 (1)
J- F	-79 / 158	0.04 (1)
B- N	0 / 1463	0.33 (1)
J- G	0 / 1705	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.83/1.00 (I-J:1) , WB=0.54/1.00 (F-L:1) , SSI=0.67/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.40 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALDES
PROVINCE OF ONTARIO

DWG NO. TAN T1805251
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T174	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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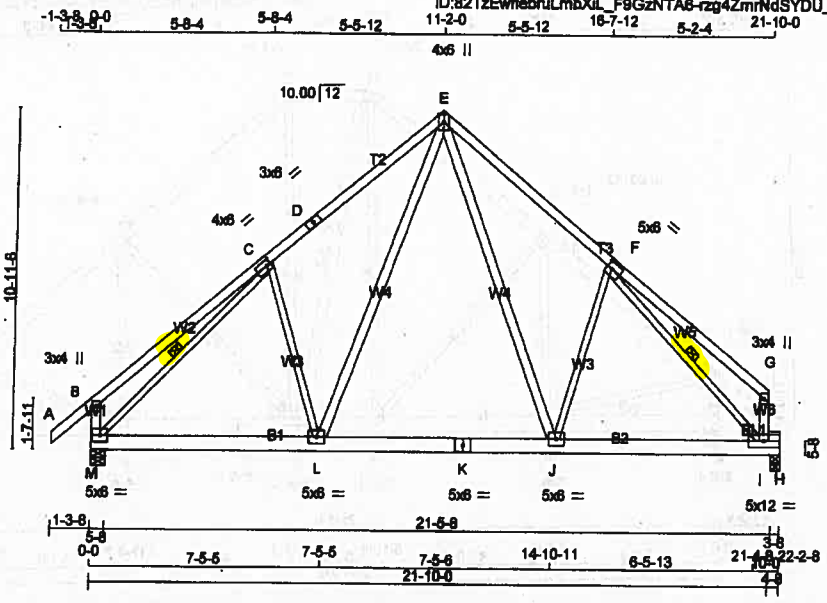

TOTAL WEIGHT = 124 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	5.0	6.0	2.50	2.00
G	TMV+p	MT20	3.0	4.0		
I	BMVWK-t	MT20	5.0	12.0		
J	BMWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG
M	2046	0	2046	0	0	5-8	5-8
H	1819	0	1819	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1493	954 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1338	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B-C	0 / 58	-129.7	-129.7	0.67 (1)	10.00	
C-D	-1682 / 0	-129.7	-128.7	0.67 (1)	3.92	
D-E	-1682 / 0	-129.7	-128.7	0.67 (1)	3.92	
E-F	-1704 / 0	-129.7	-129.7	0.80 (1)	4.05	
F-G	0 / 82	-129.7	-129.7	0.82 (1)	10.00	
M-B	-451 / 0	0.0	0.0	0.05 (1)	7.81	
I-G	-222 / 0	0.0	0.0	0.02 (1)	7.81	
M-L	0 / 1396	-38.5	-38.5	0.30 (1)	10.00	
L-K	0 / 982	-38.5	-38.5	0.37 (1)	10.00	
K-J	0 / 982	-38.5	-38.5	0.37 (1)	10.00	
J-I	0 / 1360	-38.5	-38.5	0.79 (1)	10.00	
I-H	0 / 0	-38.5	-38.5	0.39 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-491 / 8	0.32 (1)
L-E	0 / 788	0.13 (1)
E-J	0 / 847	0.14 (1)
J-F	-302 / 67	0.20 (1)
M-C	-2028 / 0	0.86 (1)
F-I	-2148 / 0	0.84 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.67/1.00 (C-E:1) , BC=0.79/1.00 (I-J:1) , WB=0.86/1.00 (C-M:1) , SSI=0.67/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.58 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AMES
PROVINCE OF ONTARIO

DWG NO. TAM T1805252
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T174X	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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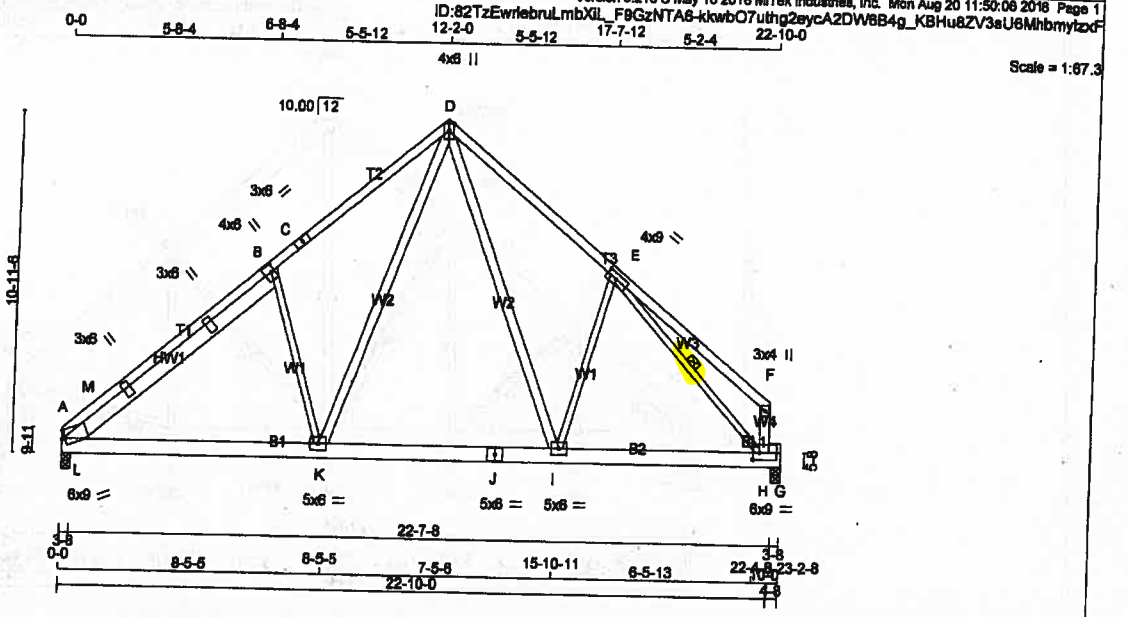

TOTAL WEIGHT = 4 X 134 = 534 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
A - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBW1-m	MT20	8.0	9.0	3.00	0.75
A	TP-m	MT20	3.0	6.0	0.50	
A	TP-m	MT20	3.0	6.0	0.50	
B	TMWW+t	MT20	4.0	6.0	3.00	1.25
C	TS-t	MT20	3.0	6.0		
D	TTWW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	9.0	2.00	4.00
F	TMV+p	MT20	3.0	4.0		
H	BMVWK-t	MT20	6.0	8.0	2.75	2.50
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1951	0	1951	0	0	3-8	3-8
G	1903	0	1903	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1432	867 / 0	244 / 0	0 / 0	0 / 0	302 / 0	0 / 0
G	1400	859 / 0	244 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- M		-2269 / 0	-129.7	-129.7	0.43 (1)		4.01
M- B		-1883 / 0	-129.7	-129.7	0.93 (1)		3.77
B- C		-1918 / 0	-129.7	-129.7	0.86 (1)		3.75
C- D		-1816 / 0	-129.7	-129.7	0.86 (1)		3.75
D- E		-1817 / 0	-129.7	-129.7	0.81 (1)		3.94
E- F		0 / 82	-129.7	-129.7	0.62 (1)		10.00
H- F		-231 / 0	0.0	0.0	0.03 (1)		7.81
A- L		0 / 1489	-38.5	-38.5	0.23 (1)		10.00
L- K		0 / 1624	-38.5	-38.5	0.37 (1)		10.00
K- J		0 / 1073	-38.5	-38.5	0.36 (1)		10.00
J- I		0 / 1073	-38.5	-38.5	0.38 (1)		10.00
I- H		0 / 1437	-38.5	-38.5	0.83 (1)		10.00
H- G		0 / 0	-38.5	-38.5	0.41 (1)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K		-713 / 0	0.47 (1)
K- D		0 / 1055	0.17 (1)
D- I		0 / 833	0.13 (1)
I- E		-289 / 74	0.19 (1)
E- H		-2260 / 0	0.88 (1)
L- M		0 / 358	0.00 (1)
L- B		-218 / 22	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.93/1.00 (B-M:1) , BC=0.83/1.00 (H-I:1) , WB=0.88/1.00 (E-H:1) , SSI=0.70/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AWES
PROVINCE OF ONTARIO

DWG NO. TAM 1805253
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T175X	1	3	TRUSS DESC.	

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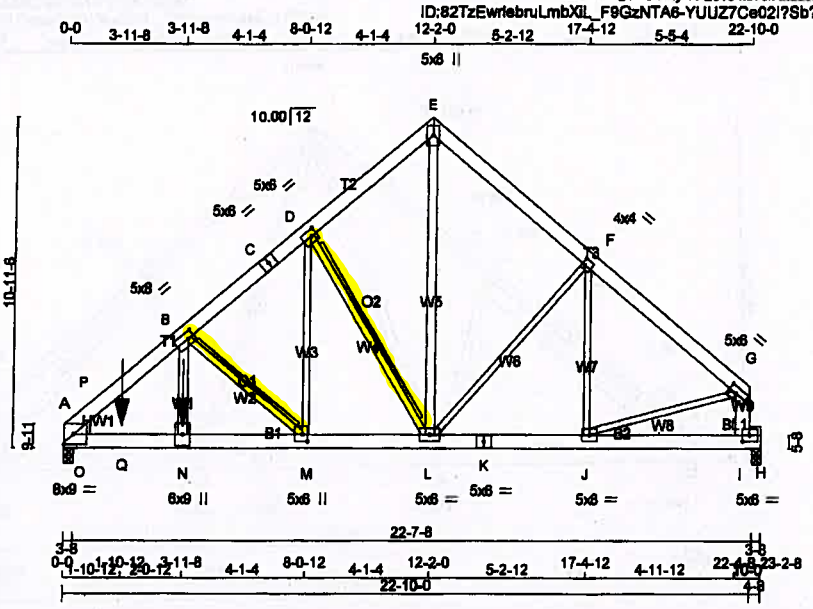

TOTAL WEIGHT = 3 X 145 = 436 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
A - K	2x6	DRY	2100F 1.8E	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- G	2	12	TOP
I- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A- K	2	5	SIDE(890.1)
K- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
B- N	2	4	SIDE(826.7)
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-t	MT20	8.0	9.0		Edge
B	TMWW-t	MT20	5.0	8.0	2.50	2.50
C	TS-t	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	8.0	2.50	2.25
E	TTW+p	MT20	5.0	8.0		
F	TMWW-t	MT20	4.0	4.0	2.50	1.25
G	TMVW-t	MT20	5.0	8.0	2.50	3.25
I	BMVK-t	MT20	5.0	8.0	2.50	2.75
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	8.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW+t	MT20	5.0	6.0	2.75	2.00
N	BMW+w	MT20	8.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
A	7724	0	7724	0	0	3-8	3-8	2x6 L
H	3054	0	3054	0	0	3-8	3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX/MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	5640	3597 / 0	899 / 0	0 / 0	0 / 0	1143 / 0	0 / 0
H	2235	1407 / 0	368 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-M
2x6 DRY SPF No.2 T-BRACE AT D-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" DOMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- P	-11933 / 0	-129.7	-129.7	0.18 (1)		4.21
P- B	-9710 / 0	-129.7	-129.7	0.18 (1)		4.60
B- C	-4898 / 0	-129.7	-129.7	0.09 (1)		6.07
C- D	-4898 / 0	-129.7	-129.7	0.09 (1)		6.07
D- E	-2942 / 0	-129.7	-129.7	0.08 (1)		8.25
E- F	-2946 / 0	-129.7	-129.7	0.09 (1)		6.25
F- G	-3301 / 0	-129.7	-129.7	0.09 (1)		6.25
I- G	-3324 / 0	0.0	0.0	0.08 (1)		7.61
A- O	0 / 7354	-38.5	-38.5	0.22 (1)		10.00
O- Q	0 / 7354	-38.5	-38.5	0.22 (1)		10.00
Q- N	0 / 7354	-38.5	-38.5	0.22 (1)		10.00
N- M	0 / 7354	-38.5	-38.5	0.17 (1)		10.00
M- L	0 / 3826	-38.5	-38.5	0.07 (1)		10.00
L- K	0 / 2569	-38.5	-38.5	0.10 (1)		10.00
K- J	0 / 2569	-38.5	-38.5	0.10 (1)		10.00
J- I	-492 / 0	-38.5	-38.5	0.22 (1)		6.25
I- H	0 / 0	-168.2	-168.2	0.14 (1)		10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	0 / 5869	0.31 (1)
B- M	-4678 / 0	0.49 (1)
M- D	0 / 3331	0.25 (1)
D- L	-3181 / 0	0.41 (1)
L- E	0 / 3243	0.17 (1)
L- F	-520 / 0	0.22 (1)
J- F	-151 / 154	0.03 (1)
J- G	0 / 3160	0.24 (1)
O- P	0 / 2814	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	3-11-8	-4221	-4221	—	BACK	VERT	TOTAL
Q	1-10-12	-768	-768	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 39.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 39.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.18/1.00 (A-P:1) , BC=0.22/1.00 (I-J:1) , WB=0.49/1.00 (B-M:1) , SSI=0.39/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.43 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805254
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T175X	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1057.1 lbs FACTORED DOWN AT 1-10-12, AND 5788.9 lbs FACTORED DOWN AT 3-11-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/16
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T0805254
STRUCTURAL
COMPONENT ONLY
3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T176	2	1	TRUSS DESC.	

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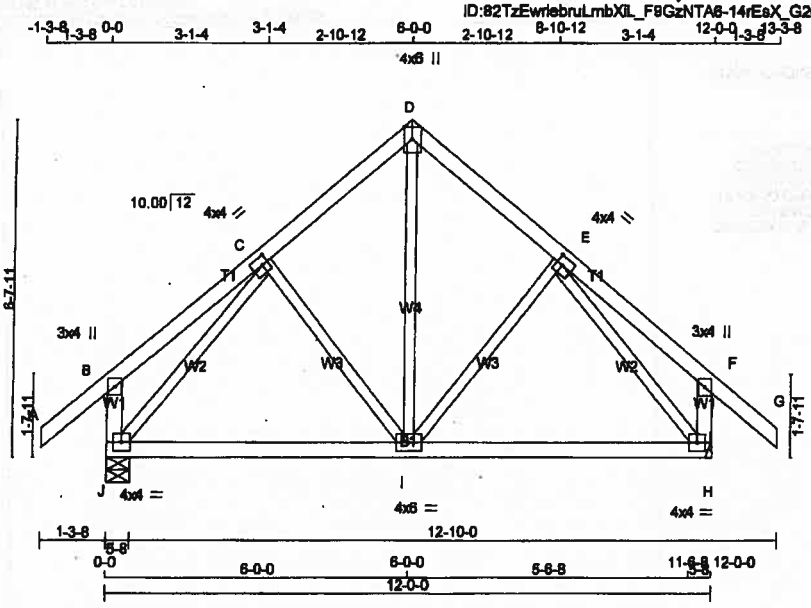

TOTAL WEIGHT = 2 X 58 = 116 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	8.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1169	0	1169	0	0	5-8	5-8
H	1169	0	1169	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	863	585 / 0	129 / 0	0 / 0	0 / 0	173 / 0	0 / 0
H	863	585 / 0	129 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	0 / 28	-129.7	-129.7	0.18 (1)		10.00
C- D	-686 / 0	-129.7	-129.7	0.14 (1)		8.25
D- E	-686 / 0	-129.7	-129.7	0.14 (1)		8.25
E- F	0 / 28	-129.7	-129.7	0.18 (1)		10.00
F- G	0 / 57	-129.7	-129.7	0.16 (1)		10.00
J- B	-329 / 0	0.0	0.0	0.03 (1)		7.81
H- F	-329 / 0	0.0	0.0	0.03 (1)		7.81
J- I	0 / 606	-38.5	-38.5	0.37 (2)		10.00
I- H	0 / 606	-38.5	-38.5	0.37 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 535	0.12 (1)
I- E	-160 / 27	0.06 (1)
C- I	-160 / 27	0.06 (1)
J- C	-881 / 0	0.36 (1)
E- H	-881 / 0	0.36 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 39.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.37/1.00 (H-I:2) , WB=0.36/1.00 (C-J:1) , SSI=0.15/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)

DWG NO. TAM T1805255
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T176S	3	1	TRUSS DESC.	

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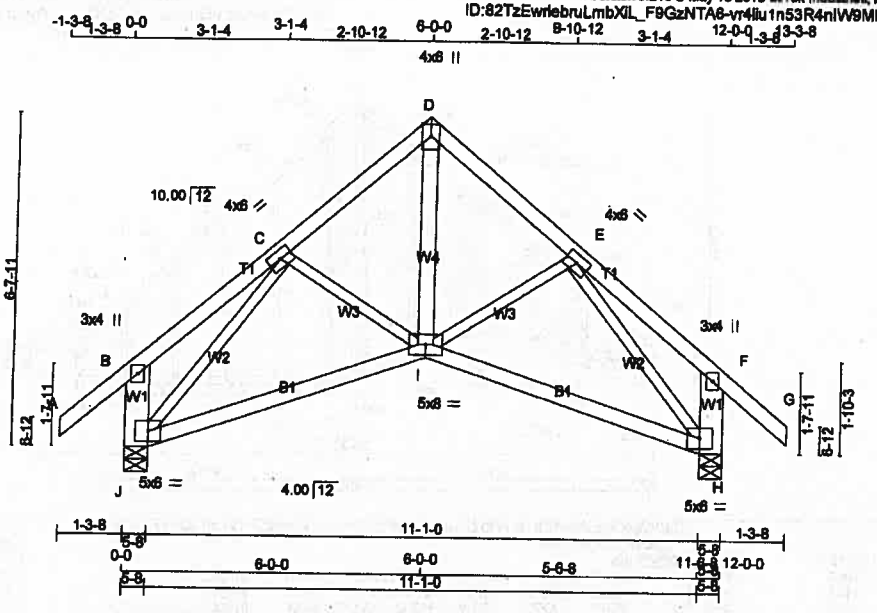


TOTAL WEIGHT = 3 X 59 = 176 lb [M(F)]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	5.0	8.0		
I	BBWWW-p	MT20	5.0	8.0		
J	BVMW1-p	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1189	0	1189	0	0	5-8	5-8
H	1189	0	1189	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	863	565 / 0	128 / 0	0 / 0	0 / 0	173 / 0	0 / 0
H	863	565 / 0	128 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	I- D	0 / 824	0.13 (1)
B- C	0 / 26	-129.7	-129.7	0.17 (1)	10.00	I- E	-85 / 45	0.02 (1)
C- D	-911 / 0	-129.7	-129.7	0.14 (1)	8.25	C- I	-85 / 45	0.02 (1)
D- E	-911 / 0	-129.7	-128.7	0.14 (1)	8.25	J- C	-1244 / 0	0.44 (1)
E- F	0 / 28	-129.7	-129.7	0.17 (1)	10.00	E- H	-1244 / 0	0.44 (1)
F- G	0 / 57	-129.7	-129.7	0.18 (1)	10.00			
J- B	-332 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-332 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 792	-38.5	-38.5	0.40 (2)	10.00			
I- H	0 / 792	-38.5	-38.5	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.40/1.00 (I-J:2) , WB=0.44/1.00 (C-J:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

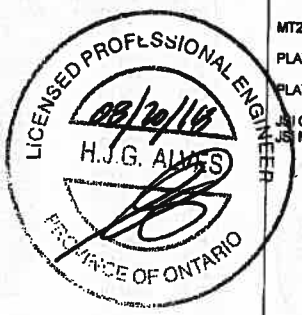
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (H) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)



DWG NO. TAM T180256
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G177	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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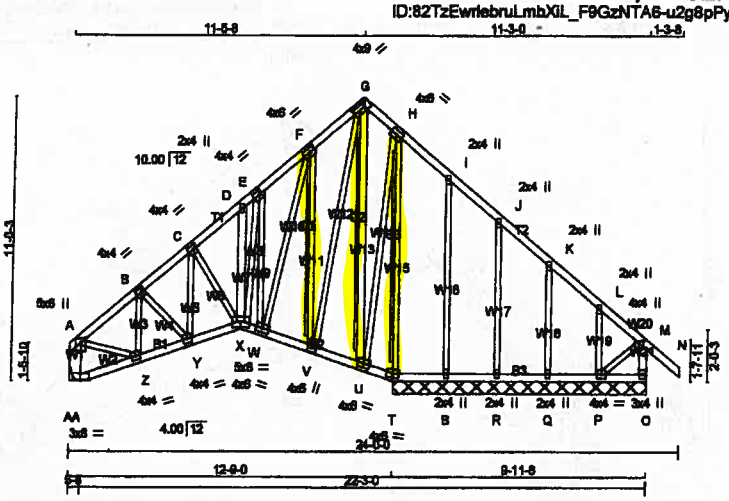

TOTAL WEIGHT = 156 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
AA- A	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
AA- X	2x4	DRY	No.2	SPF
X - T	2x4	DRY	No.2	SPF
T - O	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - G	2x4	DRY	No.2	SPF
X - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D, I, J, K, L						
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMWW-t	MT20	4.0	8.0	2.00	2.25
G	TTWW-h	MT20	4.0	8.0	2.00	5.75
H	TMWW-t	MT20	4.0	8.0		
M	TMVW+p	MT20	4.0	4.0	1.00	2.00
O	BMV1+p	MT20	3.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
Q, R, S						
Q	BMW1+w	MT20	2.0	4.0		
T	BBW1-m	MT20	4.0	8.0		
U	BMWW-t	MT20	4.0	8.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW-t	MT20	4.0	8.0		
X	BBWWW-p	MT20	5.0	8.0		
Y	BMWW-t	MT20	4.0	4.0		
Z	BMWW-t	MT20	4.0	4.0		
AA	BVM1-p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
AA	1056	0	1056	0	0	MECHANICAL	LSS26DS
T	1282	0	1282	0	0	9-11-8	9-11-8
O	390	0	390	0	0	9-11-8	9-11-8
S	363	0	363	0	0	9-11-8	9-11-8
R	313	0	313	0	0	9-11-8	9-11-8
Q	352	0	352	0	0	9-11-8	9-11-8
P	243	0	243	0	0	9-11-8	9-11-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT AA.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	775	479 / 0	132 / 0	0 / 0	0 / 0	163 / 0	0 / 0
T	940	587 / 0	158 / 0	0 / 0	0 / 0	196 / 0	0 / 0
O	271	218 / 0	13 / 0	0 / 0	0 / 0	42 / 0	0 / 0
S	268	181 / 0	48 / 0	0 / 0	0 / 0	58 / 0	0 / 0
R	230	142 / 0	39 / 0	0 / 0	0 / 0	48 / 0	0 / 0
Q	258	162 / 0	42 / 0	0 / 0	0 / 0	53 / 0	0 / 0
P	185	93 / 0	46 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, O, S, R, Q, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT H-T, G-U, F-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1108 / 0	-129.7	-129.7	0.13 (1)	5.87
B-C	-1117 / 0	-129.7	-129.7	0.11 (1)	5.87
C-D	-933 / 0	-129.7	-129.7	0.09 (1)	8.25
D-E	-834 / 0	-129.7	-129.7	0.09 (1)	8.25
E-F	-758 / 0	-129.7	-129.7	0.07 (1)	8.25
F-G	-391 / 0	-129.7	-129.7	0.07 (1)	8.25
G-H	-150 / 0	-129.7	-129.7	0.13 (1)	8.25
H-I	0 / 26	-129.7	-129.7	0.13 (1)	10.00
I-J	0 / 20	-129.7	-129.7	0.09 (1)	10.00
J-K	0 / 5	-129.7	-129.7	0.07 (1)	10.00
K-L	0 / 11	-129.7	-129.7	0.07 (1)	10.00
L-M	-45 / 0	-129.7	-129.7	0.17 (1)	8.25
M-N	0 / 57	-129.7	-129.7	0.18 (1)	10.00
AA-A	-1009 / 0	0.0	0.0	0.07 (1)	7.81
O-M	-362 / 0	0.0	0.0	0.04 (1)	7.81
AA-Z	0 / 3	-38.5	-38.5	0.05 (2)	10.00
Z-Y	0 / 907	-38.5	-38.5	0.17 (1)	10.00
Y-X	0 / 898	-38.5	-38.5	0.17 (1)	10.00
X-W	0 / 635	-38.5	-38.5	0.15 (1)	10.00
W-V	0 / 337	-38.5	-38.5	0.10 (1)	10.00
V-U	0 / 113	-38.5	-38.5	0.11 (1)	10.00
U-T	-24 / 0	-38.5	-38.5	0.09 (1)	8.25
T-S	-13 / 0	-38.5	-38.5	0.03 (3)	8.25
S-R	-10 / 0	-38.5	-38.5	0.03 (3)	10.00
R-Q	-6 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-H	-1168 / 0	0.87 (1)
S-I	-273 / 0	0.39 (1)
R-J	-238 / 0	0.17 (1)
Q-K	-276 / 0	0.10 (1)
P-L	-163 / 0	0.03 (1)
P-M	-3 / 8	0.00 (1)
U-G	-845 / 0	0.62 (1)
U-H	0 / 779	0.16 (1)
W-E	-948 / 0	0.52 (1)
X-D	-11 / 30	0.01 (3)
V-F	-1008 / 0	0.49 (1)
Y-C	0 / 103	0.02 (3)
Z-B	-325 / 0	0.08 (1)
A-Z	0 / 874	0.20 (1)
B-Y	-18 / 0	0.00 (1)
C-X	-283 / 0	0.07 (1)
X-E	0 / 747	0.17 (1)
W-F	0 / 999	0.22 (1)
V-G	0 / 1055	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.18/1.00 (M-N:1) , BC=0.17/1.00 (X-Y:1) , WB=0.87/1.00 (H-T:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (W) (INPUT = 0.90)
JSI METAL= 0.33 (Z) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/24/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805547
STRUCTURAL COMPONENT ONLY
1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G177	1	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	LC1 TO	MAX. CSI (LC)	WEBS MEMB.	MAX. UNBRAC LENGTH	FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO		
Q- P	-3 / 0	-38.5	-38.5	0.03 (3)		10.00		
P- O	0 / 0	-38.5	-38.5	0.03 (3)		10.00		

DWG NO. TAM 1005547
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T178	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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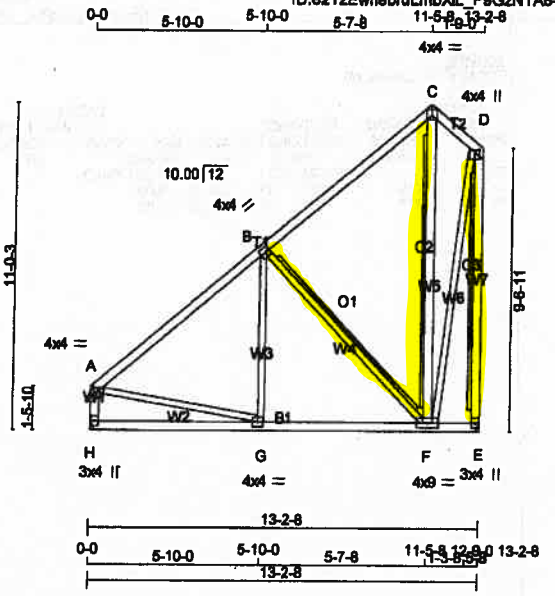

TOTAL WEIGHT = 82 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	4.0	4.0	1.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMWW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER -

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1111	0	1111	0	0	MECHANICAL	
E	1111	0	1111	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	815	505 / 0	139 / 0	0 / 0	0 / 0	172 / 0	0 / 0
E	815	505 / 0	139 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT B-F
2x4 DRY SPF No.2 T-BRACE AT C-F, D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-872 / 0	-129.7	-128.7	0.58 (1)	5.65
B- C	-270 / 0	-129.7	-129.7	0.55 (1)	6.25
C- D	-210 / 0	-129.7	-129.7	0.05 (1)	6.25
H- A	-1022 / 0	0.0	0.0	0.11 (1)	7.75
E- D	-1098 / 0	0.0	0.0	0.84 (1)	7.81
H- G	0 / 0	-38.5	-38.5	0.26 (3)	10.00
G- F	0 / 714	-38.5	-38.5	0.34 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.10 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	0 / 278	0.07 (3)
B- F	-808 / 0	0.90 (1)
F- C	-116 / 87	0.09 (1)
A- G	0 / 726	0.18 (1)
F- D	0 / 866	0.18 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AMOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.34/1.00 (F-G:2) , WB=0.90/1.00 (B-F:1) , SSI=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (A) (INPUT = 0.90)
JSI METAL= 0.28 (A) (INPUT = 1.00)

DWG NO. TAM T1805257
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T179	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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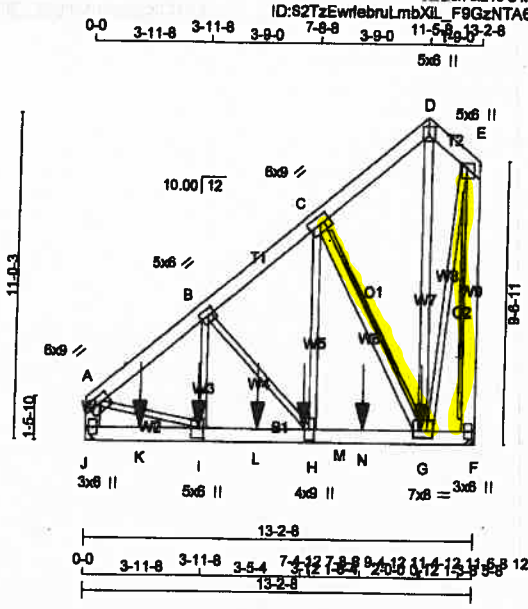

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

TOTAL WEIGHT = 2 X 117 = 233 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 2	12	TOP
D- E 2	12	TOP
J- A 2	12	TOP
F- E 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
J- F 2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS		
B- I 1	6	SIDE(226.2)
G- E 1	6	SIDE(107.4)
C- H 1	6	SIDE(226.2)
2x3 1	6	
G- D 1	6	SIDE(118.6)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	8.0	8.0	2.75	4.25
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	8.0	9.0	3.00	4.25
D	TTW+p	MT20	5.0	8.0	3.75	2.50
E	TMWW+p	MT20	5.0	8.0	2.00	2.00
F	BMV1+p	MT20	3.0	8.0		
G	BMWWW-t	MT20	7.0	8.0	2.75	2.00
H	BMWW+t	MT20	4.0	8.0		
I	BMWW+t	MT20	5.0	8.0	4.25	1.50
J	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	5793	0	5793	0	0	MECHANICAL	
F	5842	0	5842	0	0	MECHANICAL	

HGUS26-2

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4232	2665 / 0	875 / 0	0 / 0	0 / 0	872 / 0	0 / 0
F	4268	2708 / 0	681 / 0	0 / 0	0 / 0	879 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-G, E-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5831 / 0	-129.7	-129.7	0.17 (1)		4.78
B- C	-3871 / 0	-129.7	-129.7	0.10 (1)		5.66
C- D	-1270 / 0	-129.7	-128.7	0.08 (1)		8.25
D- E	-1235 / 0	-129.7	-129.7	0.02 (1)		8.25
J- A	-5083 / 0	0.0	0.0	0.18 (1)		8.47
F- E	-5768 / 0	0.0	0.0	0.62 (1)		7.81
J- K	0 / 0	-38.5	-38.5	0.46 (1)		10.00
K- I	0 / 0	-38.5	-38.5	0.46 (1)		10.00
I- L	0 / 4470	-38.5	-38.5	0.64 (1)		10.00
L- M	0 / 4470	-38.5	-38.5	0.64 (1)		10.00
M- H	0 / 4470	-38.5	-38.5	0.64 (1)		10.00
H- N	0 / 3012	-38.5	-38.5	0.58 (1)		10.00
N- G	0 / 3012	-38.5	-38.5	0.58 (1)		10.00
G- F	0 / 0	-38.5	-38.5	0.01 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 2462	0.30 (1)
B- H	-2200 / 0	0.58 (1)
H- C	0 / 5151	0.64 (1)
C- G	-4546 / 0	0.50 (1)
G- D	0 / 1280	0.11 (1)
A- I	0 / 4594	0.57 (1)
G- E	0 / 4985	0.62 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	11-4-12	-1145	-1145	—	BACK	VERT	TOTAL
I	3-9-12	-1145	-1145	—	BACK	VERT	TOTAL
K	1-8-12	-1145	-1145	—	BACK	VERT	TOTAL
L	5-9-12	-1145	-1145	—	BACK	VERT	TOTAL
M	7-4-12	-1145	-1145	—	BACK	VERT	TOTAL
N	9-4-12	-1145	-1145	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.64/1.00 (H-I:1) , WB=0.64/1.00 (C-H:1) , SSI=0.77/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

DWG NO. TAM T1805258
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T179	1	2	TRUSS DESC.	

HANGER(S) NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1569.1 lbs FACTORED DOWN AT 1-9-12, 1569.1 lbs FACTORED DOWN AT 3-9-12, 1569.1 lbs FACTORED DOWN AT 5-9-12, 1569.1 lbs FACTORED DOWN AT 7-4-12, AND 1568.1 lbs FACTORED DOWN AT 9-4-12, AND 1569.1 lbs FACTORED DOWN AT 11-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G
PROVINCE OF ONTARIO

DWG NO. TAM T1805258
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T180S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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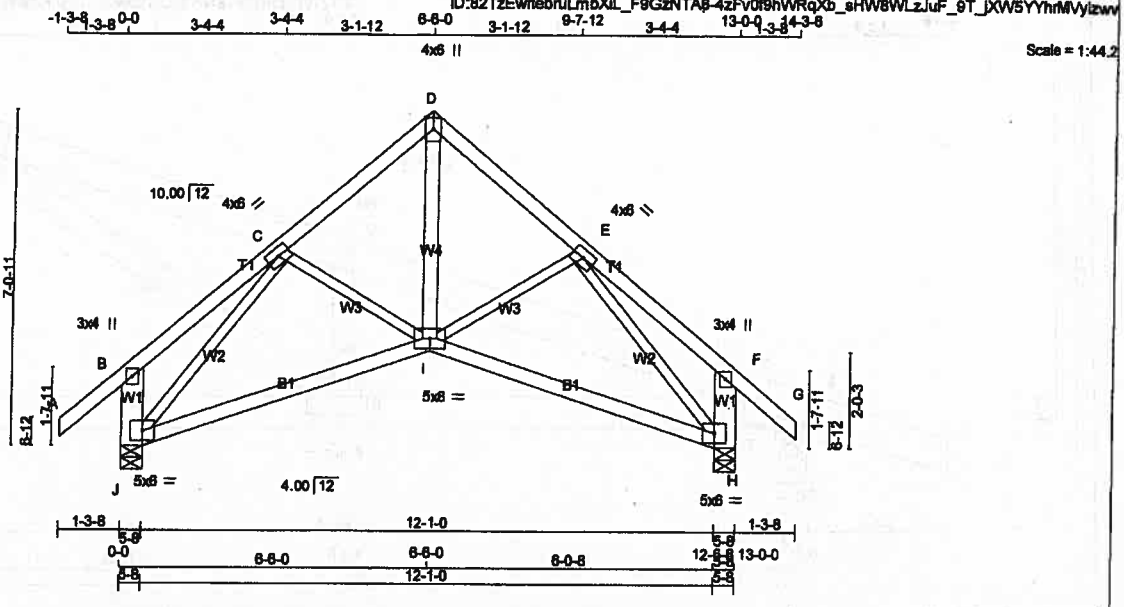


TOTAL WEIGHT = 2 X 63 = 125 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0		
D	TTW+p	MT20	4.0	8.0	Edge	
E	TMWW-t	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	5.0	8.0		
I	BBWWW-p	MT20	5.0	8.0		
J	BVMW1-p	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1273	0	1273	0	0	5-8	5-8
H	1273	0	1273	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	925	603 / 0	138 / 0	0 / 0	0 / 0	186 / 0	0 / 0
H	925	603 / 0	138 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)		10.00
B- C	0 / 29	-129.7	-129.7	0.20 (1)		10.00
C- D	-1013 / 0	-129.7	-129.7	0.17 (1)		8.04
D- E	-1013 / 0	-129.7	-129.7	0.17 (1)		8.04
E- F	0 / 29	-129.7	-129.7	0.20 (1)		10.00
F- G	0 / 57	-129.7	-129.7	0.18 (1)		10.00
J- B	-344 / 0	0.0	0.0	0.03 (1)		7.81
H- F	-344 / 0	0.0	0.0	0.03 (1)		7.81
J- I	0 / 893	-38.5	-38.5	0.47 (2)		10.00
I- H	0 / 893	-38.5	-38.5	0.47 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 927	0.15 (1)
I- E	-107 / 45	0.03 (1)
C- I	-107 / 45	0.03 (1)
J- C	-1377 / 0	0.56 (1)
E- H	-1377 / 0	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 715 (0.22")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.47/1.00 (H-I:2) , WB=0.56/1.00 (C-J:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

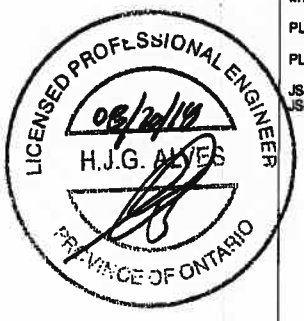
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1667	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (H) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)



DWG NO. TAM T1805259
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T190	6	1	TRUSS DESC.	

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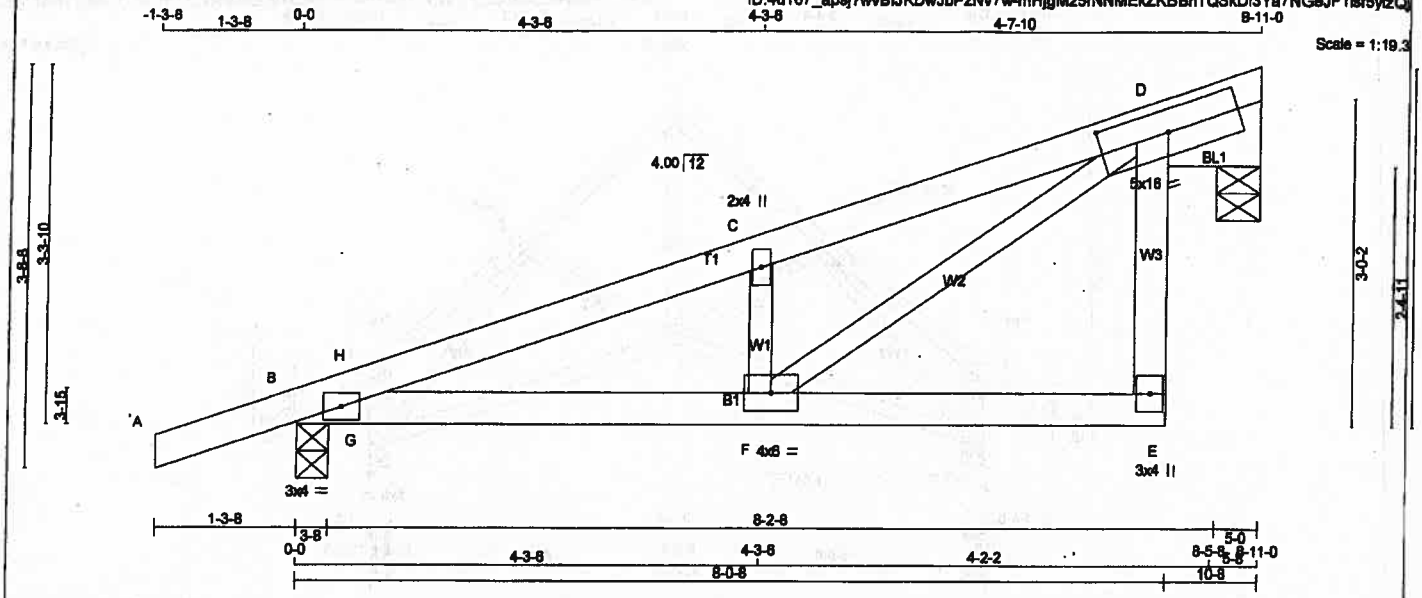

TOTAL WEIGHT = 8 X 31 = 184 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWK1-t	MT20	5.0	16.0	2.50	7.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	670	0	670	0	0	5-0	5-0
B	837	0	837	0	0	3-8	3-8

(** SEE "BEARING NOTE" **)

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	492	302 / 0	66 / 0	0 / 0	0 / 0	105 / 0	0 / 0
B	608	404 / 0	83 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-129.7	-129.7	0.18 (1)	10.00	F- C	-574 / 0	0.08 (1)
B- H	-1158 / 0	-129.7	-129.7	0.04 (2)	5.80	F- D	0 / 1308	0.29 (1)
H- C	-1108 / 0	-129.7	-129.7	0.21 (1)	5.78	G- H	-129 / 82	0.00 (1)
C- D	-1123 / 0	-129.7	-129.7	0.21 (1)	5.76			
E- D	0 / 73	0.0	0.0	0.01 (3)	10.00			
B- G	0 / 1068	-38.5	-38.5	0.31 (1)	10.00			
G- F	0 / 1068	-38.5	-38.5	0.32 (1)	10.00			
F- E	-4 / 0	-38.5	-38.5	0.08 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.32/1.00 (F-G:1) , WB=0.29/1.00 (D-F:1) , SSI=0.22/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.76 (F) (INPUT = 0.90)
SSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ADVBS
PROVINCE OF ONTARIO

DWG NO. (MiTek) T1905260
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P51	29	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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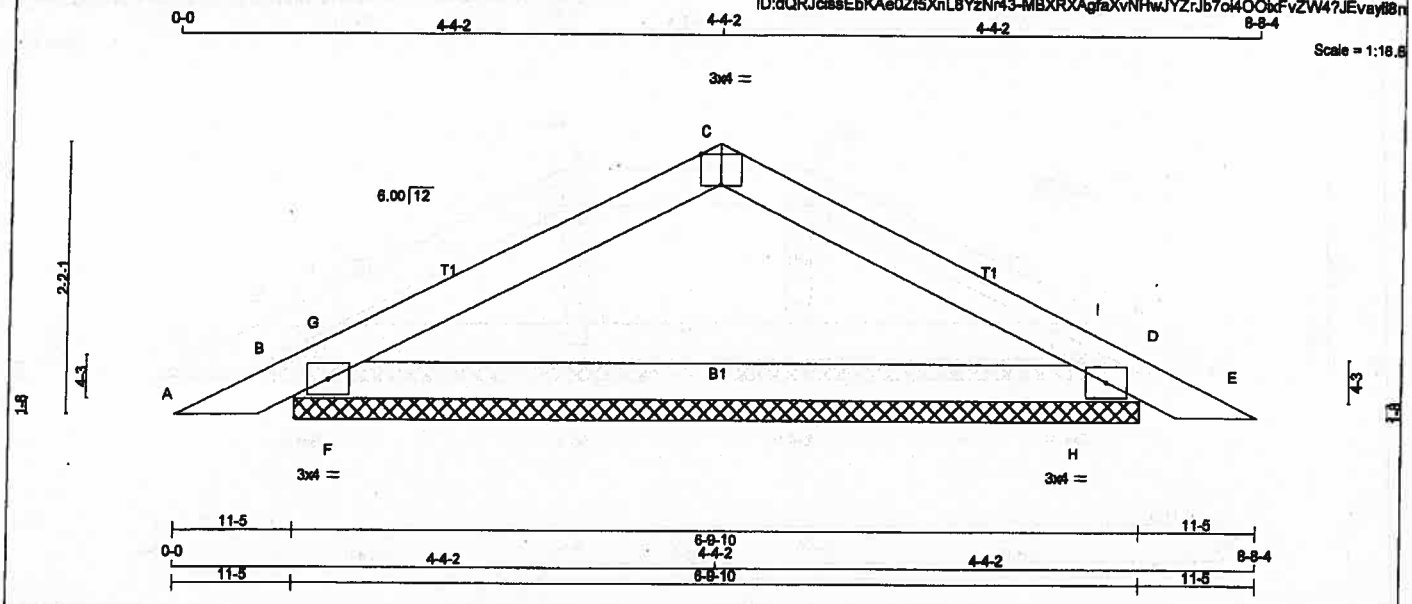

TOTAL WEIGHT = 29 X 19 = 546 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0		Edge 2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	686	0	686	0	0	6-8-10	6-8-10
D	686	0	686	0	0	6-8-10	6-8-10

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	497	327 / 0	71 / 0	0 / 0	0 / 0	99 / 0	0 / 0
D	497	327 / 0	71 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	-700 / 0	-129.7	-129.7	0.20 (2)	8.25
G- C	-507 / 0	-129.7	-129.7	0.30 (1)	8.25
C- I	-507 / 0	-129.7	-129.7	0.30 (1)	8.25
I- D	-700 / 0	-129.7	-129.7	0.20 (2)	8.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- F	0 / 470	-38.5	-38.5	0.15 (3)	10.00
F- H	0 / 470	-38.5	-38.5	0.29 (1)	10.00
H- D	0 / 470	-38.5	-38.5	0.15 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 368	0.00 (1)
H- I	0 / 368	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	81.7	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-I:1) , BC=0.29/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.25/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805378
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P52	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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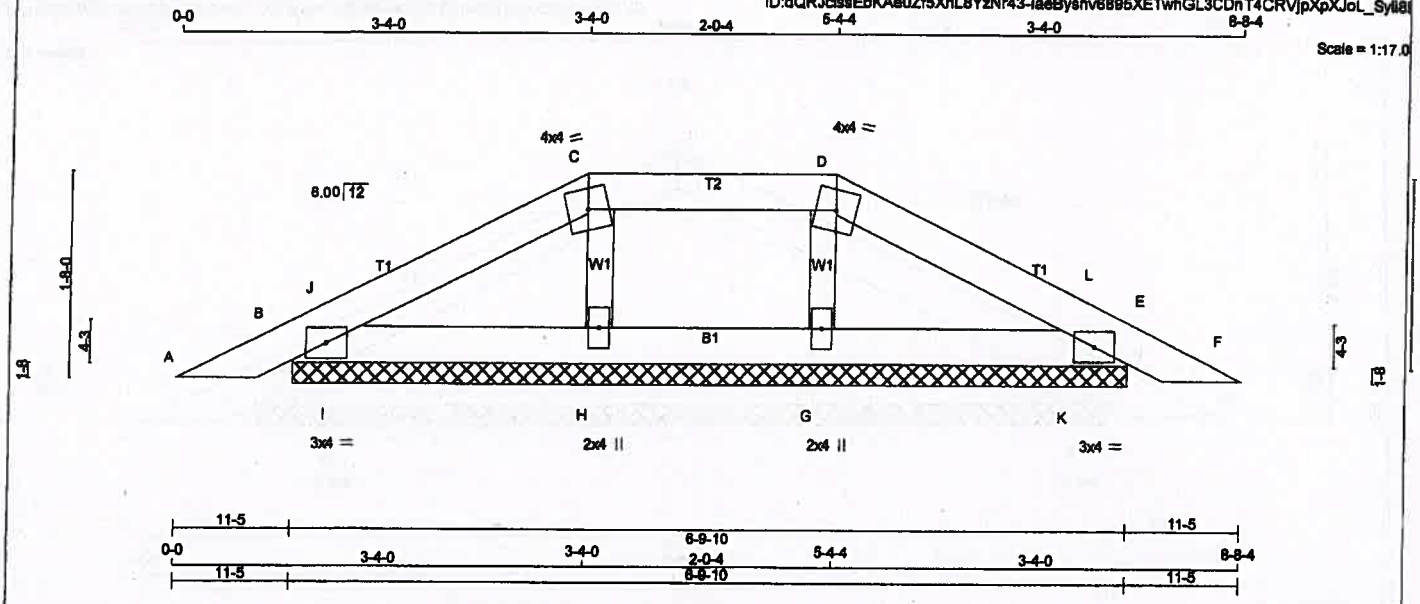

TOTAL WEIGHT = 20 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1+w	MT20	2.0	4.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	335	0	335	0	0	6-9-10	6-9-10
E	335	0	335	0	0	6-9-10	6-9-10
H	350	0	350	0	0	6-9-10	6-9-10
G	350	0	350	0	0	6-9-10	6-9-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	172 / 0	24 / 0	0 / 0	0 / 0	43 / 0	0 / 0
E	239	172 / 0	24 / 0	0 / 0	0 / 0	43 / 0	0 / 0
H	259	155 / 0	47 / 0	0 / 0	0 / 0	58 / 0	0 / 0
G	259	155 / 0	47 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- J	-73 / 0	-129.7	-129.7	0.02 (3)	8.25	
J- C	-65 / 0	-129.7	-129.7	0.09 (1)	8.25	
C- D	-40 / 0	-129.7	-129.7	0.09 (1)	8.25	
D- L	-65 / 0	-129.7	-129.7	0.09 (1)	8.25	
L- E	-73 / 0	-129.7	-129.7	0.02 (3)	8.25	
E- F	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- I	0 / 59	-38.5	-38.5	0.08 (1)	10.00	
I- H	0 / 58	-38.5	-38.5	0.09 (1)	10.00	
H- G	0 / 40	-38.5	-38.5	0.05 (1)	10.00	
G- K	0 / 59	-38.5	-38.5	0.09 (1)	10.00	
K- E	0 / 59	-38.5	-38.5	0.09 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-233 / 0	0.03 (1)
G- D	-233 / 0	0.03 (1)
I- J	-138 / 24	0.00 (1)
K- L	-138 / 24	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-D:1) , BC=0.09/1.00 (B-I:1) , WB=0.03/1.00 (C-H:1) , SSI=0.12/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.25 (B) (INPUT = 0.90)
SI METAL= 0.05 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM H1805329
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	P101	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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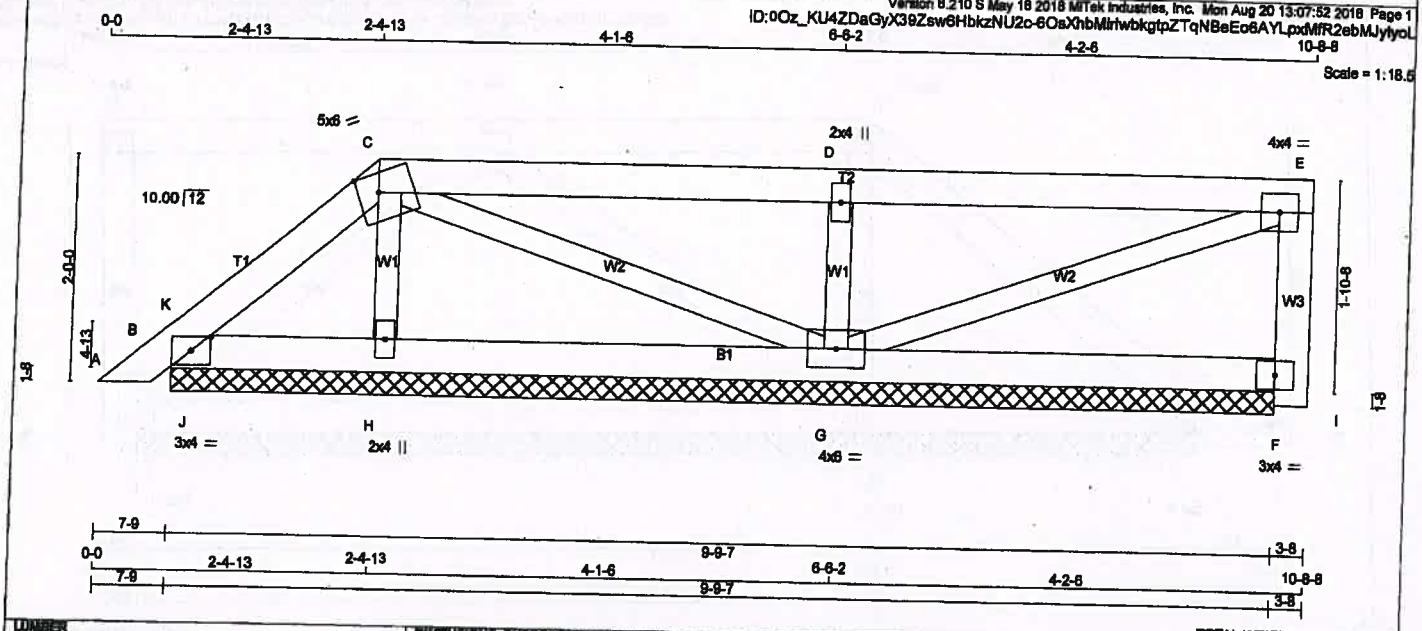

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Units is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMVW-I	MT20	4.0	4.0		
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	11	0	11	0	-42	9-9-7	9-9-7
H	442	0	442	0	0	9-9-7	9-9-7
G	1312	0	1312	0	0	9-9-7	9-9-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	-1	29 / 0	0 / -20	0 / 0	0 / 0	0 / -9	0 / 0
H	332	160 / 0	74 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	980	603 / 0	158 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	0 / 330	-129.7	-129.7	0.08 (1)	10.00
K- C	0 / 288	-129.7	-129.7	0.10 (1)	10.00
C- D	0 / 686	-129.7	-129.7	0.52 (1)	10.00
D- E	0 / 686	-129.7	-129.7	0.52 (1)	10.00
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00
F- E	0 / 79	0.0	0.0	0.01 (3)	10.00
B- J	-219 / 0	-38.5	-38.5	0.08 (1)	8.25
J- H	-219 / 0	-38.5	-38.5	0.08 (3)	8.25
H- G	-239 / 0	-38.5	-38.5	0.15 (2)	8.25
G- F	0 / 0	-38.5	-38.5	0.15 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-304 / 0	0.04 (1)
C- G	-478 / 0	0.15 (1)
G- D	-667 / 0	0.10 (1)
G- E	-733 / 0	0.23 (1)
J- K	-177 / 0	0.00 (1)

TOTAL WEIGHT = 2 X 35 = 70 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	81.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52/1.00 (C-D:1) , BC=0.15/1.00 (G-H:2) , WB=0.23/1.00 (E-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 71605270
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	P102	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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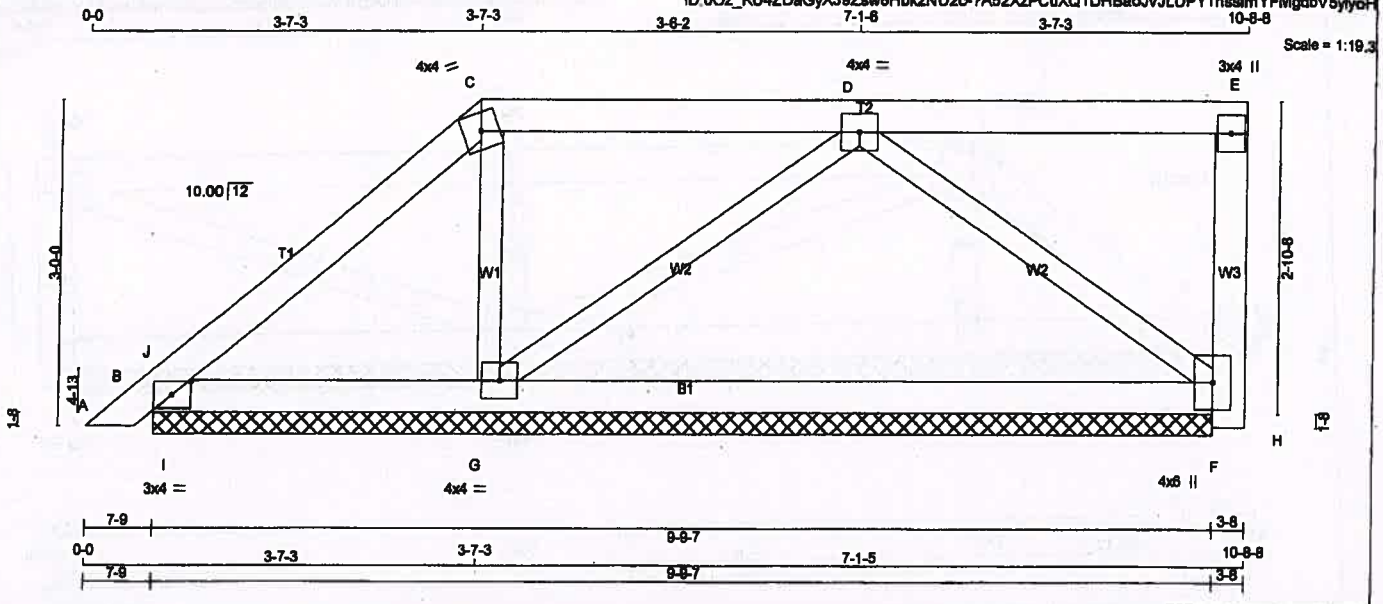

TOTAL WEIGHT = 2 X 36 = 72 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BVMW1+p	MT20	4.0	6.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	264	0	264	0	0	9-9-7	9-9-7
F	581	0	581	0	0	9-9-7	9-9-7
G	940	0	940	0	0	9-9-7	9-9-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	178	181 / 0	0 / -5	0 / 0	0 / 0	22 / 0	0 / 0
F	406	269 / 0	57 / 0	0 / 0	0 / 0	80 / 0	0 / 0
G	707	382 / 0	159 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 20	-128.7	-128.7	0.03 (1)	10.00	G-C	-320 / 0	0.05 (1)
B-J	0 / 193	-128.7	-128.7	0.12 (1)	10.00	G-D	-490 / 0	0.15 (1)
J-C	-23 / 7	-128.7	-128.7	0.12 (1)	6.25	D-F	-471 / 0	0.15 (1)
C-D	0 / 13	-128.7	-128.7	0.27 (1)	10.00	I-J	-487 / 0	0.00 (1)
D-E	0 / 0	-128.7	-128.7	0.27 (1)	10.00			
H-F	0 / 0	0.0	0.0	0.00 (1)	10.00			
F-E	-179 / 0	0.0	0.0	0.03 (1)	7.81			
B-I	-11 / 8	-38.5	-38.5	0.15 (1)	6.25			
I-G	-11 / 6	-38.5	-38.5	0.33 (3)	8.25			
G-F	0 / 383	-38.5	-38.5	0.37 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(85 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (C-D:1) , BC=0.37/1.00 (F-G:2) , WB=0.15/1.00 (D-G:1) , SSI=0.35/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.44 (G) (INPUT = 0.90)
JSI METAL= 0.14 (G) (INPUT = 1.00)

DWG NO. TAM T1805271
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297082	P107	2	1	TRUSS DESC.	

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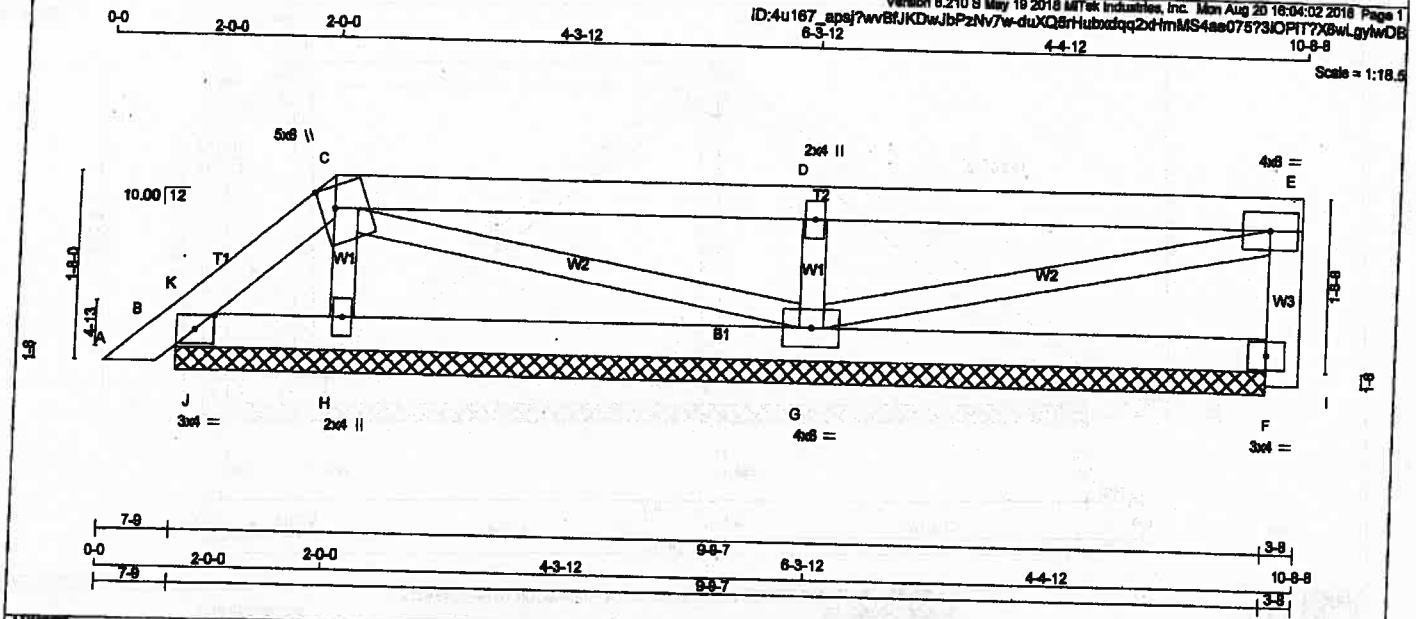

TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	-129	0	0	0	-133	9-9-7	9-9-7
H	528	0	528	0	0	9-9-7	9-9-7
G	1367	0	1367	0	0	9-9-7	9-9-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 180 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX/MIN COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	-105	0 / -31	0 / -41	0 / 0	0 / 0	0 / -32	0 / 0
H	395	215 / 0	88 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	1001	628 / 0	166 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	0 / 483	-129.7	-129.7	0.09 (1)	10.00
K- C	0 / 441	-129.7	-129.7	0.10 (1)	10.00
C- D	0 / 933	-129.7	-129.7	0.62 (1)	10.00
D- E	0 / 932	-129.7	-129.7	0.62 (1)	10.00
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00
F- E	0 / 81	0.0	0.0	0.01 (3)	10.00
B- J	-334 / 0	-38.5	-38.5	0.04 (1)	6.25
J- H	-334 / 0	-38.5	-38.5	0.08 (3)	6.25
H- G	-387 / 0	-38.5	-38.5	0.19 (2)	6.25
G- F	0 / 0	-38.5	-38.5	0.18 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-397 / 0	0.06 (1)
C- G	-587 / 0	0.19 (1)
G- D	-727 / 0	0.10 (1)
G- E	-969 / 0	0.31 (1)
J- K	-150 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF	
	DL =	8.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	61.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.62/1.00 (C-D:1) , BC=0.19/1.00 (G-H:2) , WB=0.31/1.00 (E-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

TRUSS GRIP= 0.74 (G) (INPUT = 0.90)
TRUSS METAL= 0.23 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11803307
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P113	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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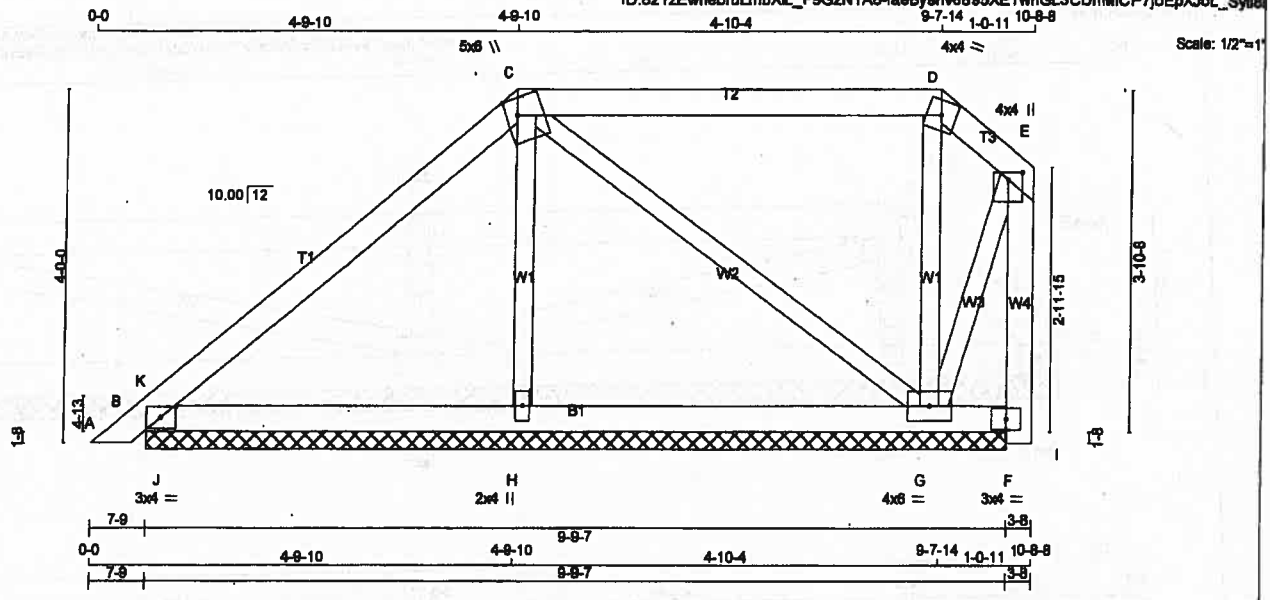

TOTAL WEIGHT = 41 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW+p	MT20	4.0	4.0	1.00	2.00
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	513	0	513	0	0	9-9-7	9-9-7
H	585	0	585	0	0	9-9-7	9-9-7
G	687	0	687	0	0	9-9-7	9-9-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	366	258 / 0	40 / 0	0 / 0	0 / 0	68 / 0	0 / 0
H	427	226 / 0	99 / 0	0 / 0	0 / 0	102 / 0	0 / 0
G	499	327 / 0	72 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00	H- C	-399 / 0	0.07 (1)
B- K	-89 / 80	-129.7	-129.7	0.14 (1)	6.25	C- G	-199 / 0	0.12 (1)
K- C	-196 / 0	-129.7	-129.7	0.28 (1)	6.25	G- D	-399 / 0	0.09 (1)
C- D	-5 / 30	-129.7	-129.7	0.52 (1)	10.00	G- E	-46 / 16	0.01 (1)
D- E	-9 / 24	-129.7	-129.7	0.03 (1)	10.00	J- K	-644 / 41	0.00 (1)
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00			
F- E	-29 / 0	0.0	0.0	0.00 (3)	7.81			
B- J	0 / 139	-38.5	-38.5	0.24 (1)	10.00			
J- H	0 / 139	-38.5	-38.5	0.24 (1)	10.00			
H- G	0 / 129	-38.5	-38.5	0.19 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.09 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52/1.00 (C-D:1) , BC=0.24/1.00 (H-J:1) , WB=0.12/1.00 (C-G:1) , SSI=0.52/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1667	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.18 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G.
PROVINCE OF ONTARIO

DWG NO. TAM T18053B0
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P114	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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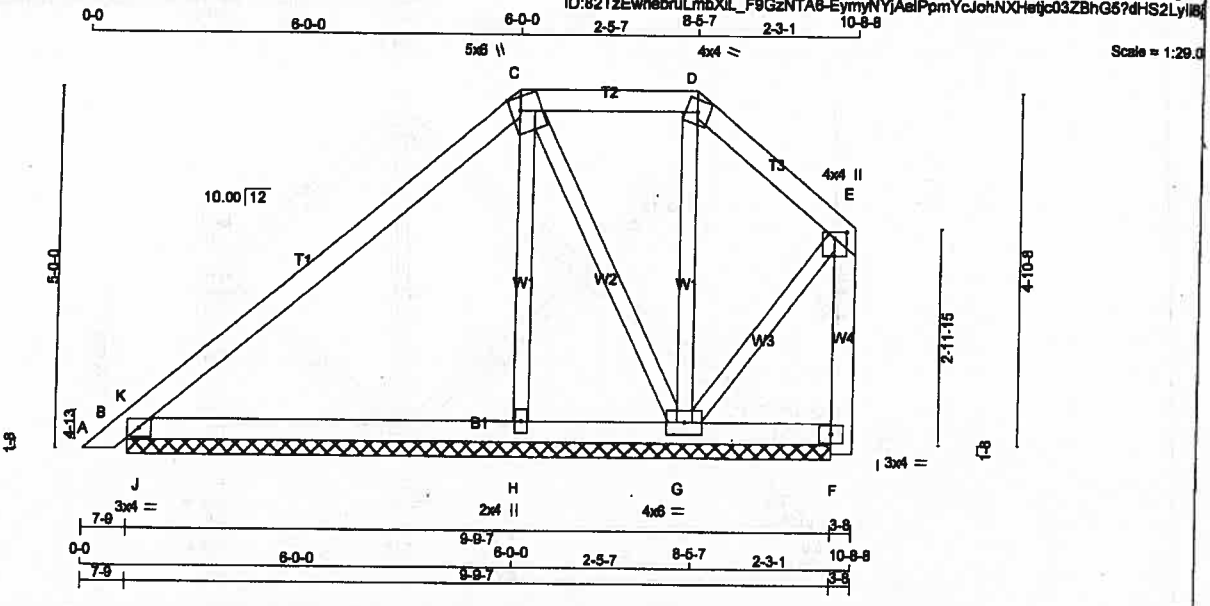

TOTAL WEIGHT = 2 X 43 = 87 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN.	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	530	0	530	0	0	9-9-7	9-9-7
H	440	0	440	0	0	9-9-7	9-9-7
G	795	0	795	0	0	9-9-7	9-9-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	380	283 / 0	45 / 0	0 / 0	0 / 0	72 / 0	0 / 0
H	340	155 / 0	96 / 0	0 / 0	0 / 0	99 / 0	0 / 0
G	571	393 / 0	70 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	-94 / 444	-129.7	-129.7	0.30 (1)	8.25
K- C	-88 / 5	-129.7	-129.7	0.47 (1)	8.25
C- D	0 / 110	-129.7	-129.7	0.15 (1)	10.00
D- E	0 / 132	-129.7	-129.7	0.13 (1)	10.00
I- F	0 / 0	0.0	0.0	0.00 (1)	10.00
F- E	0 / 81	0.0	0.0	0.01 (2)	10.00
B- J	0 / 51	-38.5	-38.5	0.37 (1)	10.00
J- H	0 / 51	-38.5	-38.5	0.37 (1)	10.00
H- G	0 / 48	-38.5	-38.5	0.29 (1)	10.00
G- F	0 / 0	-38.5	-38.5	0.05 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-109 / 10	0.04 (1)
C- G	-345 / 0	0.15 (1)
G- D	-390 / 0	0.14 (1)
G- E	-152 / 0	0.03 (1)
J- K	-1070 / 94	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (C-K:1) , BC=0.37/1.00 (H-J:1) , WB=0.15/1.00 (C-G:1) , SSI=0.65/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JOINT GRIP= 0.44 (B) (INPUT = 0.90)
JOINT METAL= 0.14 (D) (INPUT = 1.00)

DWG NO. TAM 1105381
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P115	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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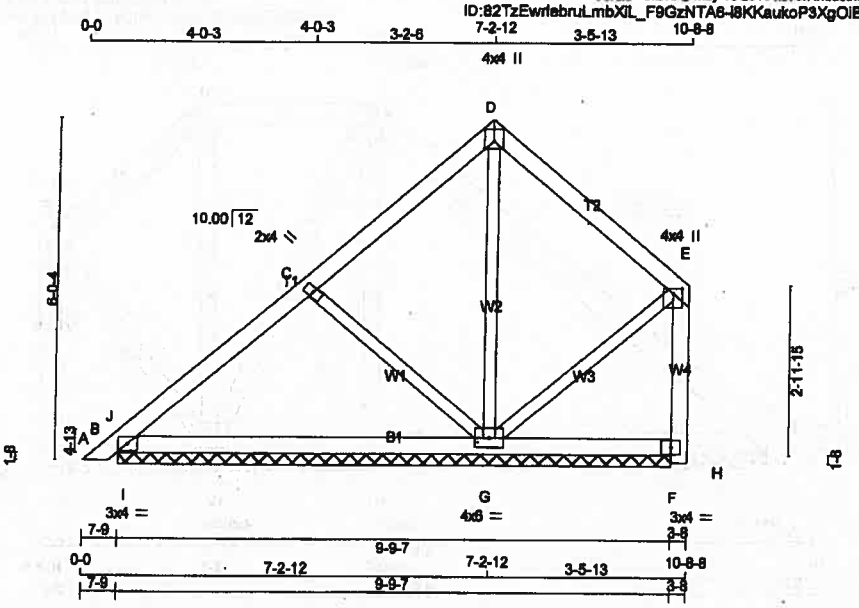

TOTAL WEIGHT = 4 X 42 = 168 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BVM-I	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	470	0	470	0	0	9-9-7	9-9-7
G	1295	0	1295	0	0	9-9-7	9-9-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	341	223 / 0	50 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	951	589 / 0	162 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 20	-129.7	-129.7	0.03 (1)	10.00	C-G	-434 / 0	0.14 (1)
B-J	-515 / 0	-129.7	-129.7	0.17 (3)	6.25	G-D	-646 / 0	0.33 (1)
J-C	-169 / 0	-129.7	-129.7	0.26 (1)	6.25	G-E	-199 / 0	0.08 (1)
C-D	0 / 169	-129.7	-129.7	0.26 (1)	10.00	I-J	0 / 516	0.00 (1)
D-E	0 / 205	-129.7	-129.7	0.30 (1)	10.00			
H-F	0 / 0	0.0	0.0	0.00 (1)	10.00			
F-E	0 / 39	0.0	0.0	0.01 (3)	10.00			
B-I	0 / 173	-38.5	-38.5	0.13 (3)	10.00			
I-G	0 / 173	-38.5	-38.5	0.25 (2)	10.00			
G-F	0 / 0	-38.5	-38.5	0.23 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.25/1.00 (G-I:2) , WB=0.33/1.00 (D-G:1) , SSI=0.31/1.00 (B-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.87 (D) (INPUT = 0.90)
SSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805382
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297084	P116	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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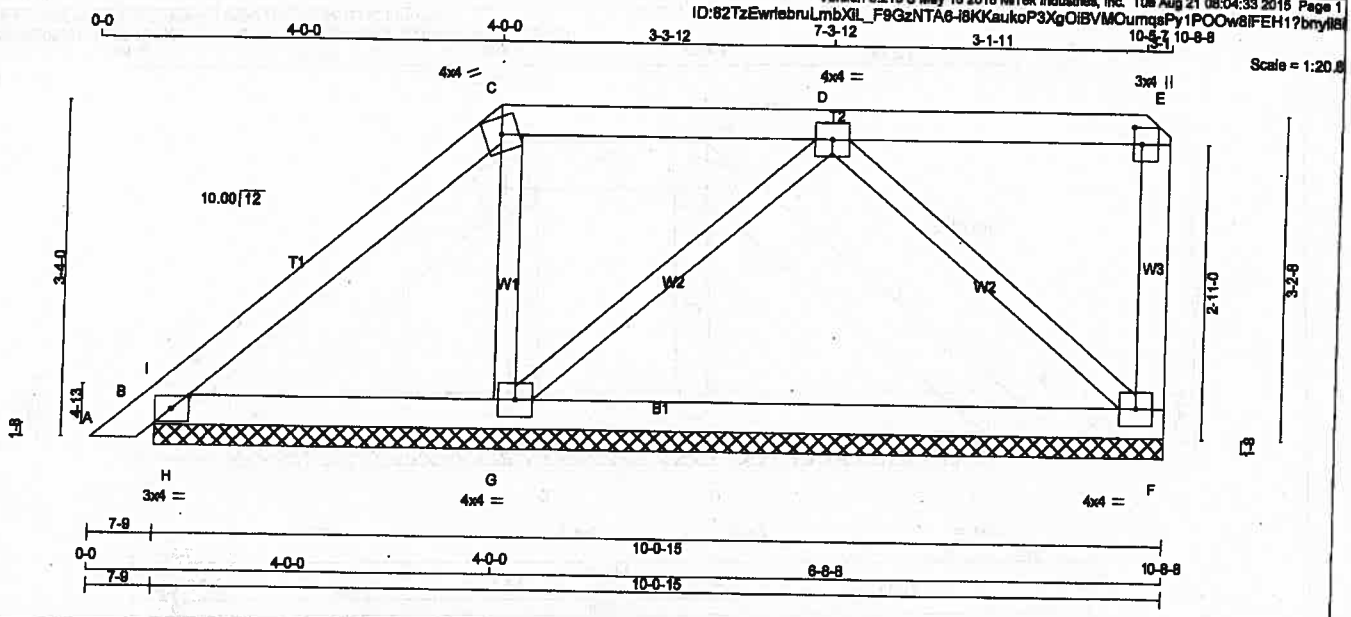

TOTAL WEIGHT = 37 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0	2.00	1.00
F	BMWW1-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	545	0	545	0	0	10-0-15	10-0-15
B	342	0	342	0	0	10-0-15	10-0-15
G	878	0	878	0	0	10-0-15	10-0-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	395	262 / 0	55 / 0	0 / 0	0 / 0	78 / 0	0 / 0
B	237	192 / 0	9 / 0	0 / 0	0 / 0	36 / 0	0 / 0
G	660	358 / 0	148 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)		10.00
B- I	0 / 182	-129.7	-129.7	0.13 (1)		10.00
I- C	-72 / 0	-129.7	-129.7	0.17 (1)		6.25
C- D	-32 / 2	-129.7	-129.7	0.24 (1)		6.25
D- E	0 / 0	-129.7	-129.7	0.24 (1)		10.00
F- E	-169 / 0	0.0	0.0	0.03 (1)		7.81
B- H	0 / 44	-38.5	-38.5	0.18 (1)		10.00
H- G	0 / 44	-38.5	-38.5	0.30 (2)		10.00
G- F	0 / 322	-38.5	-38.5	0.33 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-305 / 0	0.08 (1)
G- D	-391 / 0	0.12 (1)
D- F	-424 / 0	0.13 (1)
H- I	-538 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1) , BC=0.33/1.00 (F-G:2) , WB=0.13/1.00 (D-F:1) , SSI=0.40/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.36 (F) (INPUT = 0.90)
USI METAL= 0.12 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/21/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 18053B3
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	P161	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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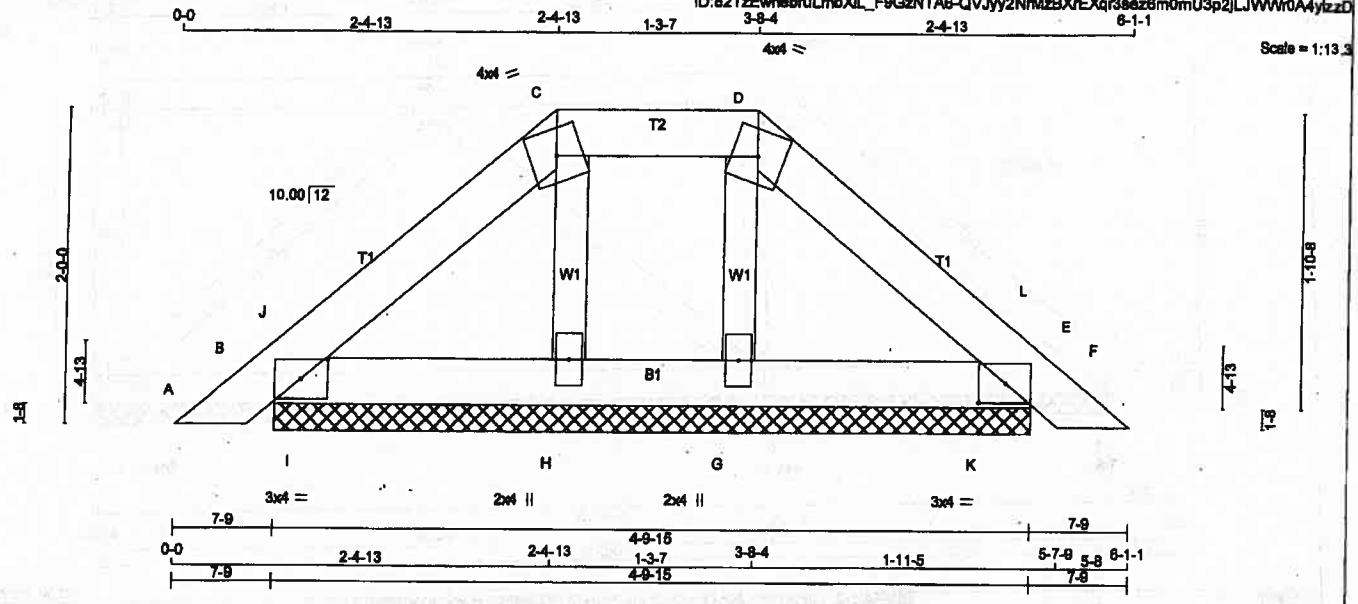

TOTAL WEIGHT = 16 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	282	0	282	0	0	4-9-15	4-9-15
E	282	0	282	0	0	4-9-15	4-9-15
H	213	0	213	0	0	4-9-15	4-9-15
G	213	0	213	0	0	4-9-15	4-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	187	134 / 0	19 / 0	0 / 0	0 / 0	34 / 0	0 / 0
E	187	134 / 0	19 / 0	0 / 0	0 / 0	34 / 0	0 / 0
H	158	91 / 0	32 / 0	0 / 0	0 / 0	35 / 0	0 / 0
G	158	91 / 0	32 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- J	-71 / 0	-129.7	-129.7	0.01 (3)	6.25
J- C	-78 / 0	-129.7	-129.7	0.05 (1)	6.25
C- D	-49 / 0	-129.7	-129.7	0.04 (1)	6.25
D- L	-78 / 0	-129.7	-129.7	0.05 (1)	6.25
L- E	-71 / 0	-129.7	-129.7	0.01 (3)	6.25
E- F	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- I	0 / 58	-38.5	-38.5	0.05 (1)	10.00
I- H	0 / 58	-38.5	-38.5	0.05 (1)	10.00
H- G	0 / 49	-38.5	-38.5	0.03 (1)	10.00
G- K	0 / 58	-38.5	-38.5	0.05 (1)	10.00
K- E	0 / 58	-38.5	-38.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-130 / 0	0.02 (1)
G- D	-130 / 0	0.02 (1)
I- J	-124 / 16	0.00 (1)
K- L	-124 / 16	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-J:1) , BC=0.05/1.00 (H-I:1) , WB=0.02/1.00 (C-H:1) , SSI=0.10/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	768	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

DWG NO. TAM 1180529
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	P162	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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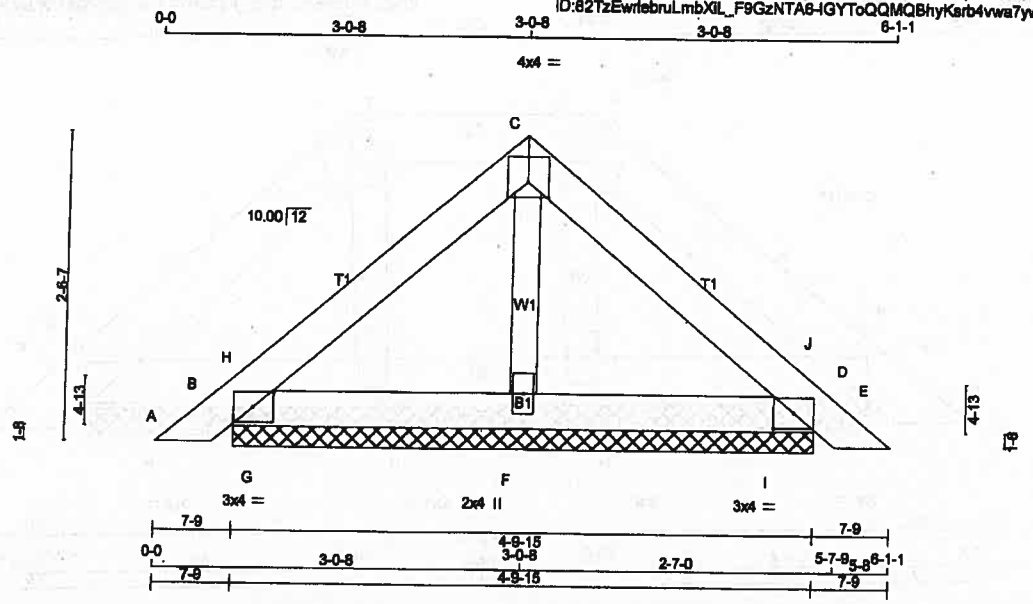

TOTAL WEIGHT = 3 X 16 = 48 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	321	0	321	0	0	4-9-15	4-9-15
D	321	0	321	0	0	4-9-15	4-9-15
F	310	0	310	0	0	4-9-15	4-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	228	184 / 0	23 / 0	0 / 0	0 / 0	41 / 0	0 / 0
D	228	184 / 0	23 / 0	0 / 0	0 / 0	41 / 0	0 / 0
F	234	123 / 0	56 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- H	-50 / 0	-129.7	-129.7	0.04 (1)	6.25
H- C	-117 / 0	-129.7	-129.7	0.09 (1)	6.25
C- J	-117 / 0	-129.7	-129.7	0.09 (1)	6.25
J- D	-50 / 0	-129.7	-129.7	0.04 (1)	6.25
D- E	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- G	0 / 84	-38.5	-38.5	0.09 (1)	10.00
G- F	0 / 84	-38.5	-38.5	0.08 (1)	10.00
F- I	0 / 84	-38.5	-38.5	0.09 (1)	10.00
I- D	0 / 84	-38.5	-38.5	0.09 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-118 / 0	0.02 (1)
G- H	-252 / 10	0.00 (1)
I- J	-252 / 10	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-J:1), BC=0.09/1.00 (F-I:1), WB=0.02/1.00 (C-F:1), SSI=0.19/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805220
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	P163	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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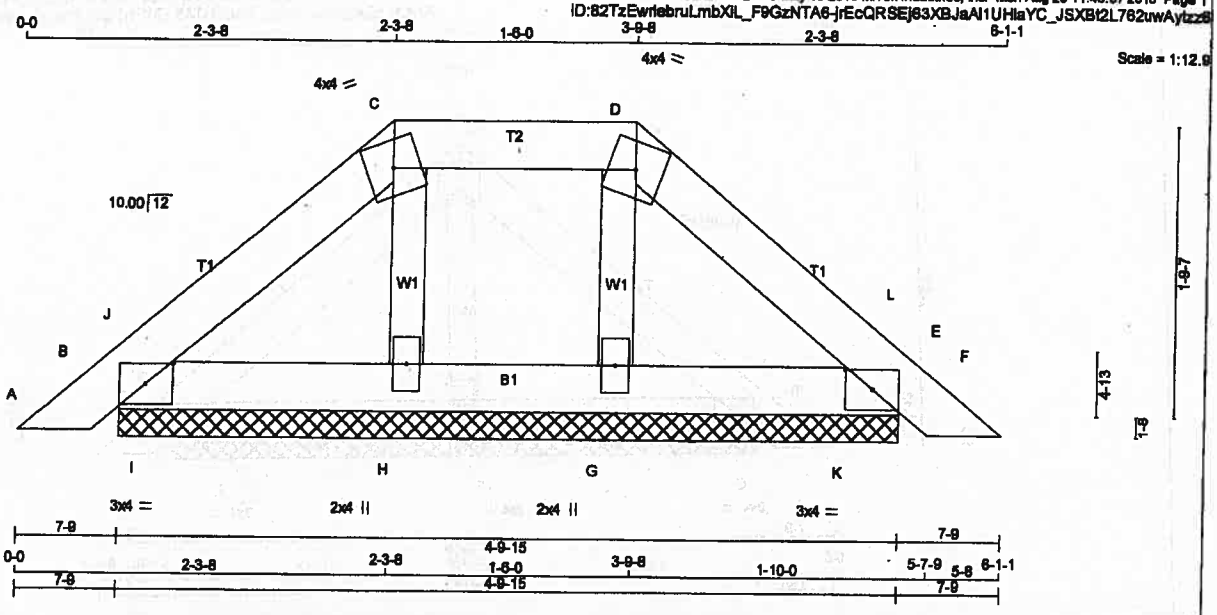


TOTAL WEIGHT = 18 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG BRG	REQRD BRG IN-SX
B	253	0	253	0	0	4-9-15	4-9-15
E	253	0	253	0	0	4-9-15	4-9-15
H	223	0	223	0	0	4-9-15	4-9-15
G	223	0	223	0	0	4-9-15	4-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	180	130 / 0	18 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	180	130 / 0	18 / 0	0 / 0	0 / 0	32 / 0	0 / 0
H	166	98 / 0	33 / 0	0 / 0	0 / 0	37 / 0	0 / 0
G	166	98 / 0	33 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-128.7	-128.7	0.03 (1)	10.00	
B- J	-72 / 0	-128.7	-128.7	0.01 (3)	6.25	
J- C	-75 / 0	-128.7	-128.7	0.04 (1)	6.25	
C- D	-46 / 0	-128.7	-128.7	0.05 (1)	6.25	
D- L	-75 / 0	-128.7	-128.7	0.04 (1)	6.25	
L- E	-72 / 0	-128.7	-128.7	0.01 (3)	6.25	
E- F	0 / 20	-128.7	-128.7	0.03 (1)	10.00	
B- I	0 / 58	-38.5	-38.5	0.05 (1)	10.00	
I- H	0 / 58	-38.5	-38.5	0.05 (1)	10.00	
H- G	0 / 46	-38.5	-38.5	0.03 (1)	10.00	
G- K	0 / 58	-38.5	-38.5	0.05 (1)	10.00	
K- E	0 / 58	-38.5	-38.5	0.05 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-140 / 0	0.02 (1)
G- D	-140 / 0	0.02 (1)
I- J	-111 / 16	0.00 (1)
K- L	-111 / 16	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1) , BC=0.05/1.00 (G-K:1) , WB=0.02/1.00 (D-G:1) , SSI=0.09/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM T1805221
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

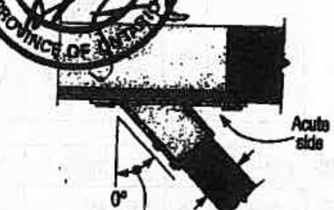
HHUS – Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS – Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance [illegible] lb.	[illegible] lb.	S-P-F Uplift (K_d = 1.15) lb.	S-P-F Normal (K_d = 1.00) lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 9/16	3⅛	1¾	2¼	(4) 10d	(2) 10d	710	1625	645	1155
								[illegible]	[illegible]	2.87	5.14
LU24L	22	1 9/16	3	1½	2 11/16	(4) 10d	(2) 10d x 1½"	380	1020	320	725
								[illegible]	[illegible]	1.42	3.22
LU26L	22	1 9/16	6	1½	4⅝	(6) 10d	(4) 10d x 1½"	720	1605	645	1140
								[illegible]	[illegible]	2.87	5.07
LUS26	18	1 9/16	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
								[illegible]	[illegible]	5.74	7.25
HUS26	16	1⅝	5⅜	3	3 13/16	(14) 16d	(6) 16d	2705	4940	2065	3875
								[illegible]	[illegible]	9.20	17.24
LJS26DS	18	1 9/16	5	3½	4⅝	(18) 16d	(8) 16d	2055	4265	1460	4115
								[illegible]	[illegible]	6.49	18.31
HGUS26	12	2⅛	5⅜	5	4⅛	(20) 16d	(8) 16d	2685	6625	2685	5700
								[illegible]	[illegible]	11.96	25.35
LU28L	20	1 9/16	6¾	1½	5⅞	(8) 10d	(6) 10d x 1½"	1140	2185	1020	1550
								[illegible]	[illegible]	4.54	6.89
LUS28	18	1 9/16	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420	2520	1290	1790
								[illegible]	[illegible]	5.74	7.96
HUS28	16	1⅝	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605	5365	2675	4345
								[illegible]	[illegible]	11.90	19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3310	6900
								[illegible]	[illegible]	14.74	30.73
LU210L	20	1 9/16	8	1½	7⅝	(10) 10d	(6) 10d x 1½"	1140	2495	1020	1770
								[illegible]	[illegible]	4.54	7.87
LUS210	18	1 9/16	7 13/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
								[illegible]	[illegible]	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

SIMPSON Strong-Tie

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_h[1]	Fasteners Header	Joist	Factored Resistance [illegible] lb / kN	[illegible] lb / kN	S-P-F Uplift (K_D = 1.15) lb / kN	S-P-F Normal (K_D = 1.00) lb / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 [illegible]	2020 [illegible]	590 2.62	1435 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 [illegible]	2595 [illegible]	1545 6.87	1920 8.54
HHUS26-2	14	3 5/16	5 7/16	3	3 13/16	(14) 16d	(6) 16d	2850 [illegible]	7335 [illegible]	2065 9.20	5205 23.15
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385 [illegible]	8950 [illegible]	3110 13.83	6355 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 [illegible]	3325 [illegible]	1545 6.87	2575 11.45
HHUS28-2	14	3 5/16	7 1/16	3	6⅛	(22) 16d	(8) 16d	3765 [illegible]	8940 [illegible]	2675 11.90	6345 28.22
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070 [illegible]	12980 [illegible]	4310 19.17	9215 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 [illegible]	4500 [illegible]	2320 10.32	3195 14.21
HHUS210-2	14	3 5/16	9 1/16	3	8	(30) 16d	(10) 16d	4670 [illegible]	9660 [illegible]	4235 18.84	7000 31.14
HGUS210-2	12	3 5/16	9 1/16	4	8⅛	(46) 16d	(16) 16d	6840 [illegible]	14015 [illegible]	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5⅛	4	4⅛	(20) 16d	(8) 16d	4385 [illegible]	8950 [illegible]	3110 13.83	6355 28.27
HGUS28-3	12	4 15/16	7¼	4	6⅛	(36) 16d	(12) 16d	6070 [illegible]	12980 [illegible]	4310 19.17	9215 40.99
HHUS210-3	14	4 11/16	9	3	7 13/16	(30) 16d	(10) 16d	4670 [illegible]	9670 [illegible]	4235 18.84	6865 30.54
HGUS210-3	12	4 15/16	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 [illegible]	14645 [illegible]	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385 [illegible]	8950 [illegible]	3110 13.83	6355 28.27
HGUS28-4	12	6 9/16	7 1/16	4	6⅛	(36) 16d	(12) 16d	6070 [illegible]	12980 [illegible]	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7 13/16	(30) 16d	(10) 16d	4670 [illegible]	10155 [illegible]	4235 18.84	7210 32.07
HGUS210-4	12	6 9/16	9 1/16	4	8⅛	(46) 16d	(16) 16d	6840 [illegible]	14645 [illegible]	4855 21.60	10400 46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 [illegible]	14995 [illegible]	5425 24.13	10645 47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 [illegible]	16400 [illegible]	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3 9/16	4¾	2	3⅝	(4) 16d	(4) 16d	1720 [illegible]	2595 [illegible]	1545 6.87	1920 8.54
HHUS46	14	3⅝	5¼	3	3 13/16	(14) 16d	(6) 16d	2540 [illegible]	7335 [illegible]	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4 1/16	(20) 16d	(8) 16d	4385 [illegible]	8950 [illegible]	3110 13.83	6355 28.27
LUS48	18	3 9/16	6¾	2	3 9/16	(6) 16d	(4) 16d	1720 [illegible]	3325 [illegible]	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 [illegible]	8940 [illegible]	2675 11.90	6345 28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	(36) 16d	(12) 16d	6070 [illegible]	12980 [illegible]	4310 19.17	9215 40.99
LUS410	18	3 9/16	8¾	2	5 9/16	(8) 16d	(6) 16d	2580 [illegible]	4500 [illegible]	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8 1/16	(46) 16d	(16) 16d	6840 [illegible]	14015 [illegible]	4855 21.60	10270 45.69
HGUS412	12	3⅝	10 9/16	4	10 1/16	(56) 16d	(20) 16d	7640 [illegible]	14995 [illegible]	5425 24.13	10645 47.35
HGUS414	12	3⅝	12 9/16	4	11 1/16	(66) 16d	(22) 16d	10130 45.06	16400 72.96	7195 32.00	11645 51.80

See footnotes on p. 258.

Plated Truss Connectors

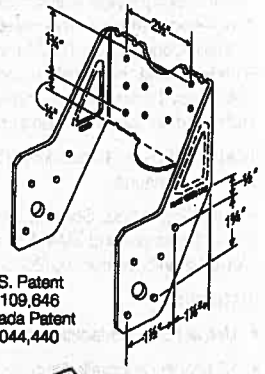
TBE – Truss Bearing Enhancers

SIMPSON Strong-Tie

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

Finish: G90 galvanized



TBE4
(TBE6 similar)

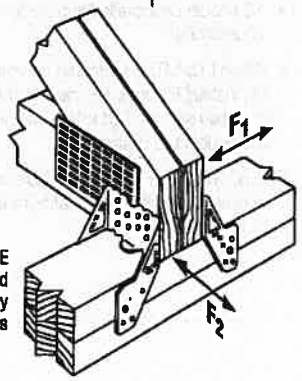
U.S. Patent
5,109,646
Canada Patent
2,044,440

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to with stand the loads imposed by the nails.

Installation:

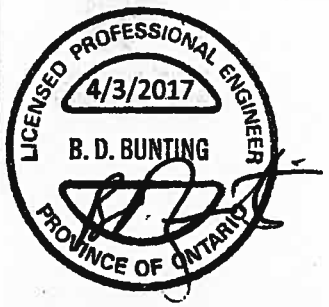
- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.



Two TBE installed with two ply girder truss

Model No.	No. of Plies	Fasteners: Truss	Fasteners: Plate	TBE Only Factored Resistance: Uplift (K_D = 1.15)	TBE Only Factored Resistance: Normal (K_D = 1.00)	TBE Only Factored Resistance: Lateral (K_D = 1.15) F_1	TBE Only Factored Resistance: Lateral (K_D = 1.15) F_2	Combined TBE and Wood Bearing Factored Resistance: Normal[2] (K_D = 1.00)	Combined TBE and Wood Bearing Factored Resistance: TBL[4]
				lb.	lb.	lb.	lb.	lb.	in.
D.Fir-L									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1605	3660	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3660	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3660	655	1415	20705	4.25
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3540	490	1745	10235	8.41
	2	(20) 10d	(20) 10d	1760	3860	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3860	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3860	490	1745	30640	6.29
S-P-F									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3220	615	1415	6445	6.99
	2	(20) 10d	(20) 10d	1605	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	16345	4.43
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q_r/A_b and Q'_r/A'_b = 812 psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.



(800) 999-5099
stronglie.com

TS/LTS/MTS/HTS

SIMPSON Strong-Tie

Twist Straps

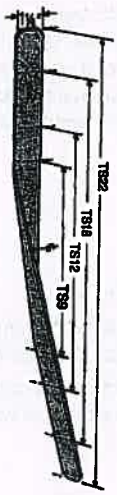
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

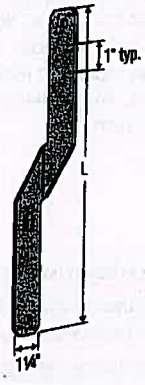
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



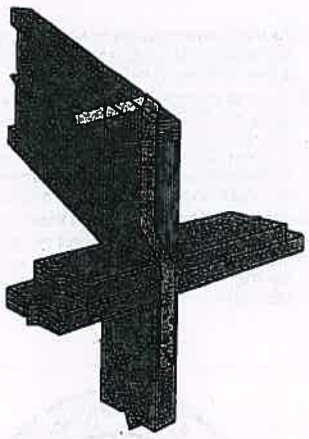
TS

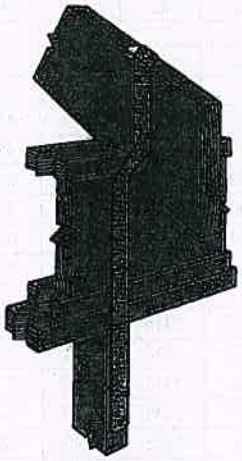


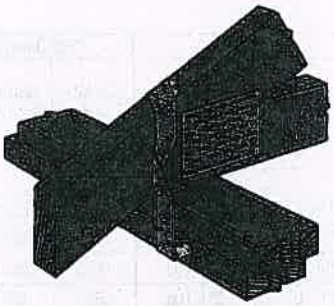
LTS12
(MTS and HTS similar)

MTS30
(HTS30 similar)

HTS30C
(MTS30C similar)


MTS30 Installation with I-Joist Rafter


Typical MTS30 Installation


MTS Installation as a Truss-to-Top Plate Tie


Typical TS Installation

TS/LTS/MTS/HTS

SIMPSON Strong-Tie

Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance (K_D = 1.15) D.Fir-L lb. kN	Factored Resistance (K_D = 1.15) S-P-F lb. kN
TS9	9	(8) 16d	1125 5.00	1040 4.63
TS12	11⅝	(10) 16d	1410 6.27	1300 5.78
TS18	17¾	(14) 16d	1970 8.76	1820 8.10
TS22	21⅝	(18) 16d	2125 9.45	2125 9.45
SS LTS12	12	(12) 10d x 1½"	1015 4.52	720 3.20
LTS16	16	(12) 10d x 1½"	1015 4.52	720 3.20
SS LTS18	18	(12) 10d x 1½"	1015 4.52	720 3.20
LTS20	20	(12) 10d x 1½"	1015 4.52	720 3.20
MTS12	12	(14) 10d x 1½"	1570 6.98	1180 5.25
MTS16	16	(14) 10d x 1½"	1570 6.98	1180 5.25
MTS18	18	(14) 10d x 1½"	1570 6.98	1180 5.25
SS MTS20	20	(14) 10d x 1½"	1570 6.98	1180 5.25
MTS30	30	(14) 10d x 1½"	1570 6.98	1180 5.25
MTS24C	24	(14) 10d x 1½"	1570 6.98	1180 5.25
MTS30C	30	(14) 10d x 1½"	1570 6.98	1180 5.25
HTS16	16	(16) 10d x 1½"	2050 9.12	1455 6.47
HTS20	20	(24) 10d x 1½"	2050 9.12	1455 6.47
HTS24	24	(24) 10d x 1½"	2050 9.12	1455 6.47
HTS28	28	(24) 10d x 1½"	2050 9.12	1455 6.47
HTS30	30	(24) 10d x 1½"	2050 9.12	1455 6.47
HTS30C	30	(24) 10d x 1½"	2050 9.12	1455 6.47

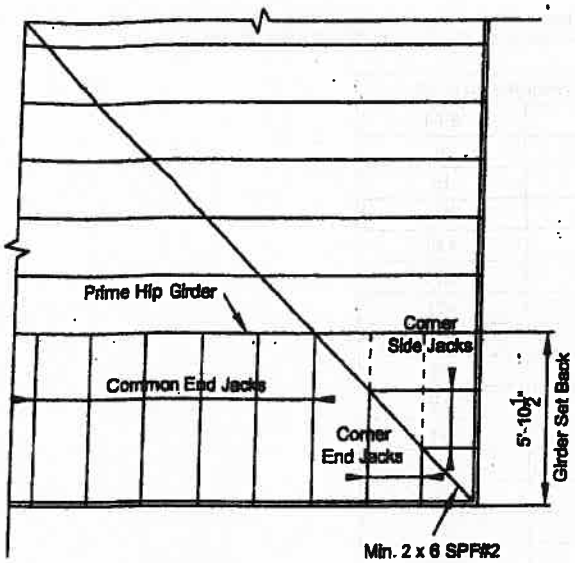
1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

LUMBER SPECIFICATION

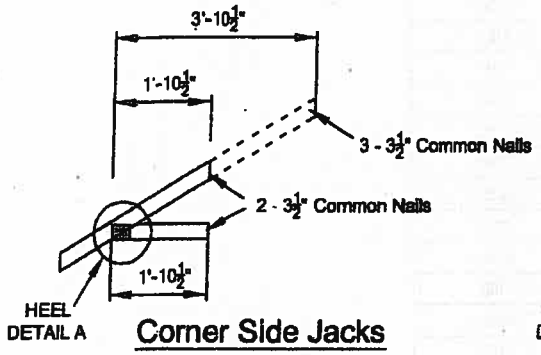
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

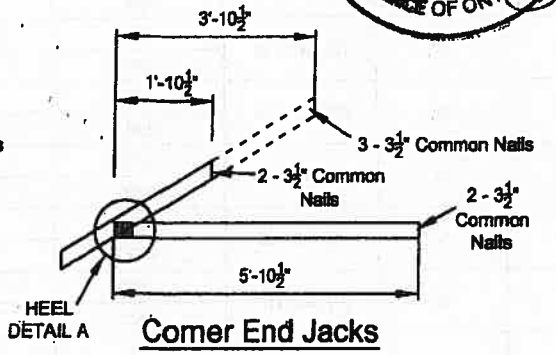
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



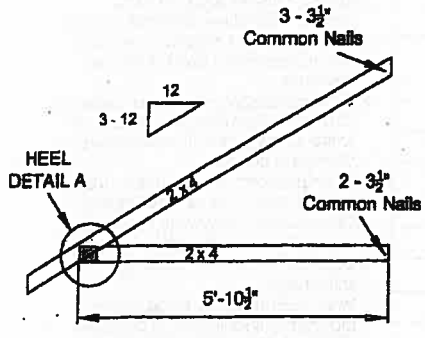
45° Hip End



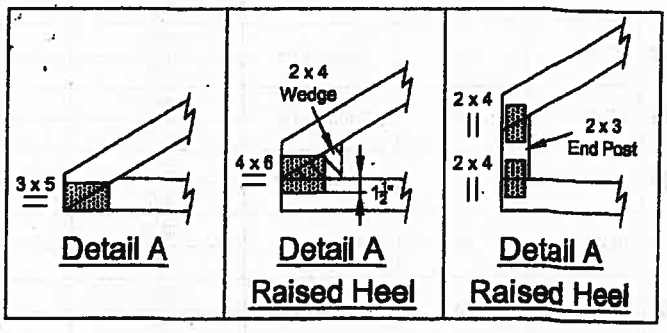
Corner Side Jacks



Corner End Jacks

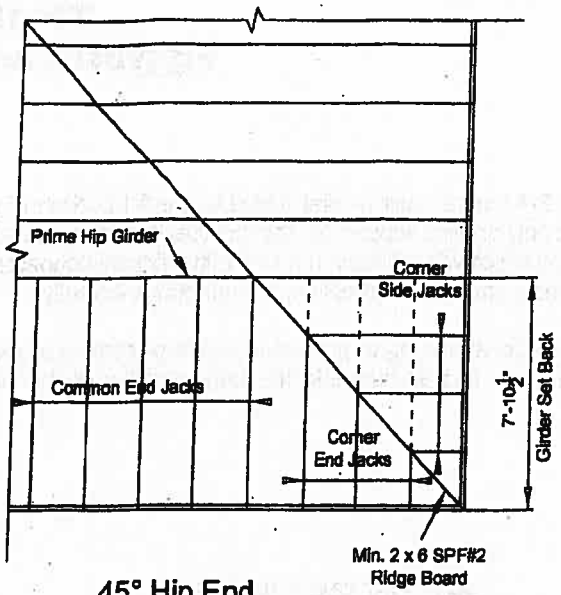


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

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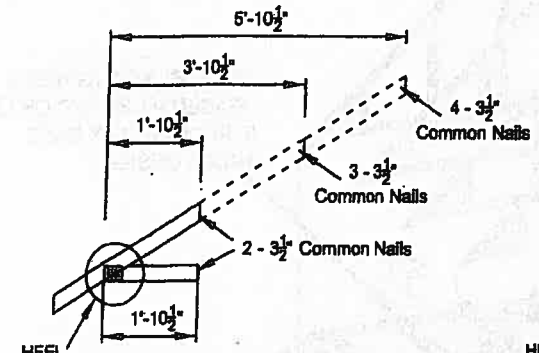
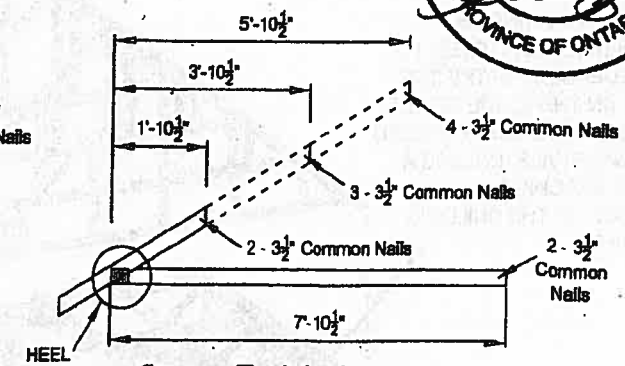
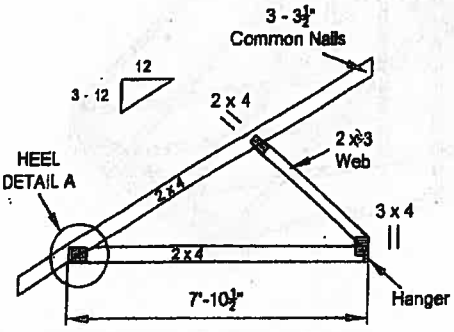


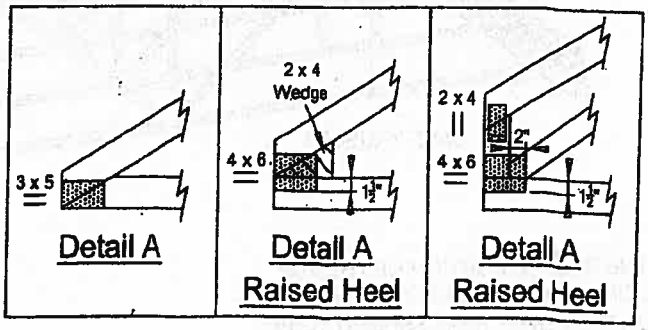
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

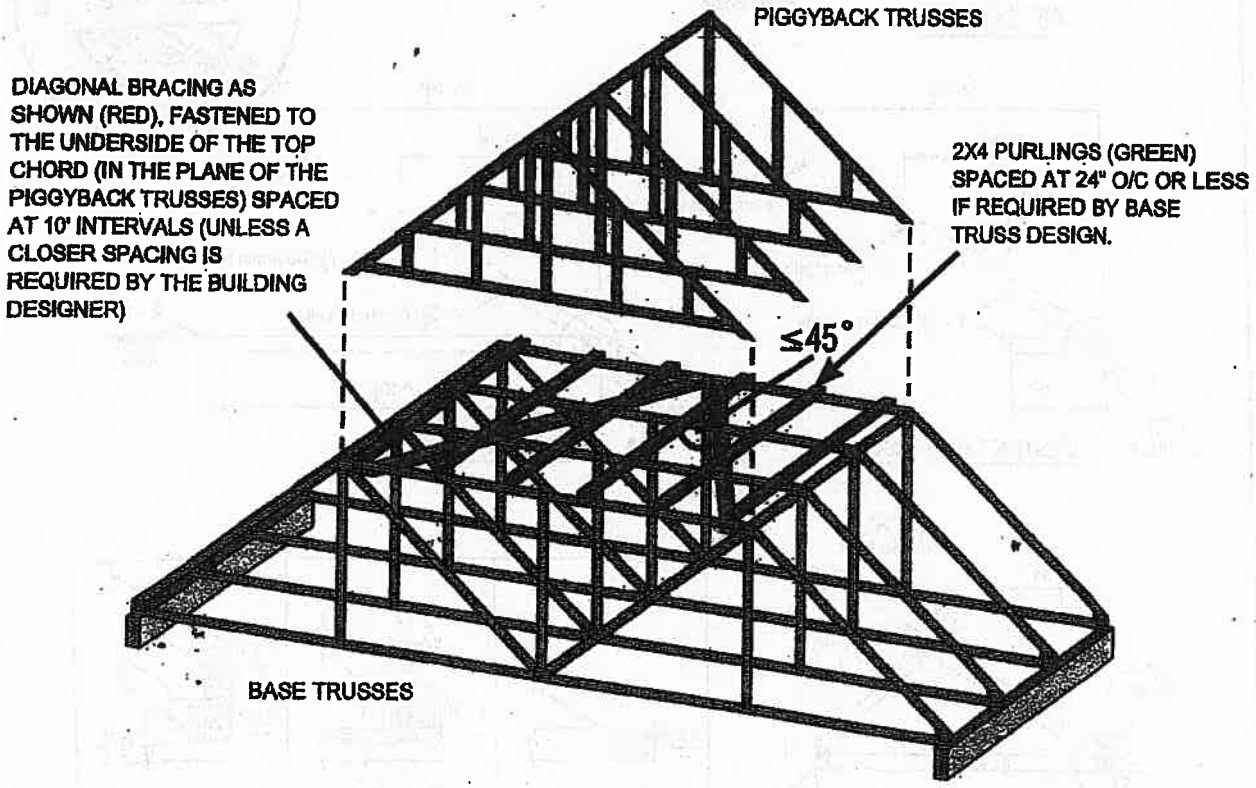
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

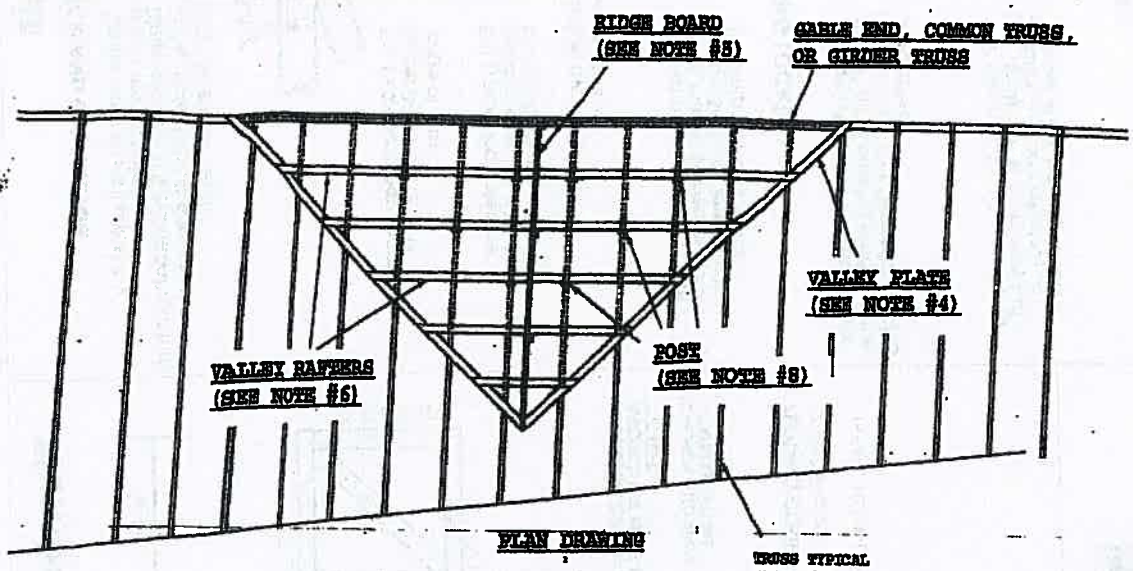


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

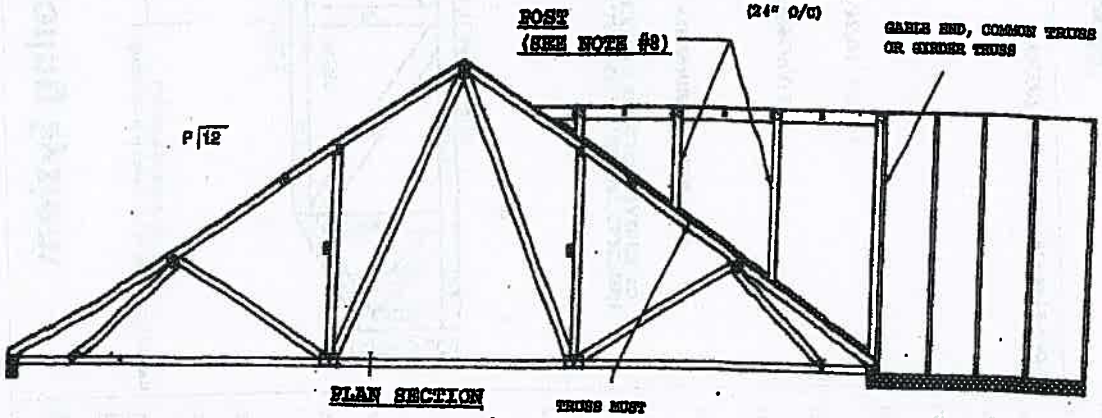
SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

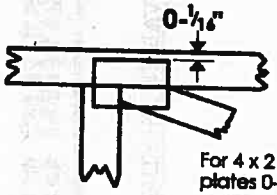
LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

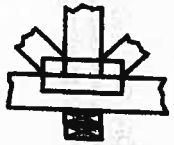
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

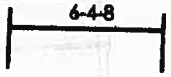
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

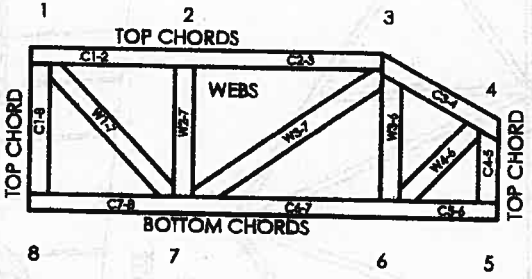
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek

POWER TO PERFORM.™

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T. I. or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018