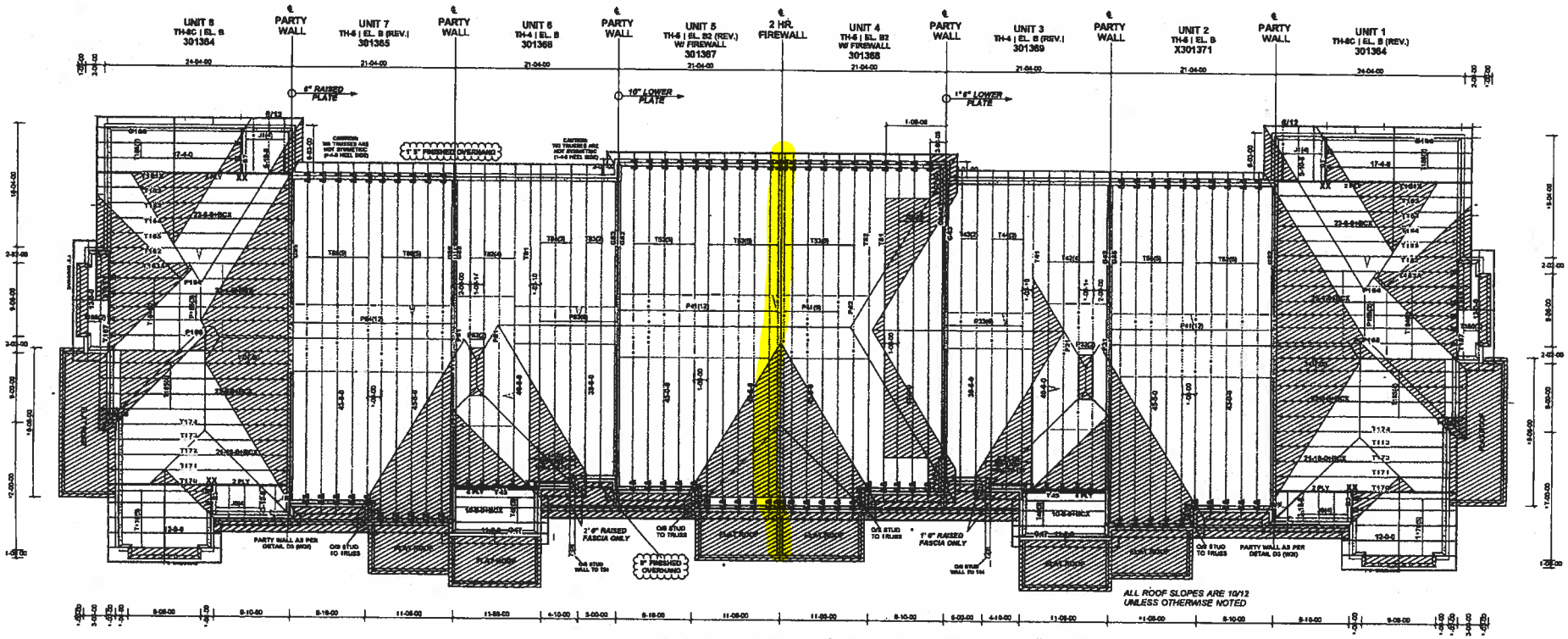


ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE (BRICK)
2" CLADDING ALLOWANCE (STUCCO)
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

T - 180523

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 S/P/F #2 24" O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 / 2018
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=54.3psf Sr=8.4psf
DESIGN LOADS
TCDL (6psf)
BCLL (10.5psf)
BCDL (7psf)

DENOTES: CONVENTIONAL FRAMING

HARDWARE: .
LJS26D8 - (V)
HGUS26 - (X)
HGUS26-2 - (XX)
T322 x2 - (##)
TBE6 x2 - (&&)

BEAMS:
B3, B7: 2 - 2x10 SPF #2

TAMARACK TRUSS INC.	Job Track: **42067**	Builder / Location: **BAYVIEW WELLINGTON / INNISFIL**	Model / Elevation: **BLOCK 144 / UNITS 1-8**
	Layout ID: **297083**	Project: **ALCONA SHORES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
	Plan Log: **97625**	Date: 10/17/2018 Designer: colinh	Mitek ver 7.5.0

REVIEWED

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

TAMARACK LUMBER INC
ALPA LUMBER GROUP

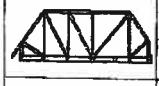
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301364 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-8C ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T162 HIP	10.00 0.00	23-06-00	05-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	114.57 71.67		
	1	T163 HIP	10.00 0.00	23-06-00	06-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	125.55 80.83		
	1	T164 HIP	10.00 0.00	23-06-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	126.63 79.17		
	1	T165 HIP	10.00 0.00	23-06-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	129.75 81.67		
	1 2 Ply	T170 HIP GIRDER	10.00 0.00	21-10-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	218.58 139.34		
	1	T171 HIP	10.00 0.00	21-10-00	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	114.67 73.17		
	1	T172 HIP	10.00 0.00	21-10-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	118.93 76.33		
	1	T173 HIP	10.00 0.00	21-10-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	129.69 81.67		
	1	T174 COMMON	10.00 0.00	21-10-00	10-11-06	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	124.27 77.33		
	5	T176 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	290.55 185.00		
	1 2 Ply	T181X HIP GIRDER	10.00 0.00	24-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	233.96 151.34		
	1	T182 HIP	10.00 0.00	23-06-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	134.08 83.83		
	1	T183A HIP	10.00 0.00	23-01-00	10-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-11-14 00-05-08	133.92 83.00		
	6	T184 PIGGYBACK	10.00 0.00	23-01-00	09-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-06-14 00-05-08	746.70 466.98		
	4	T185 ROOF	10.00 0.00	23-00-00	10-11-06	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	534.80 334.68		
	2	T186 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.22 79.00		
	1 2 Ply	T187 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 01-07-11	142.12 93.00		
	2	T188 COMMON	10.00 0.00	17-04-00	08-10-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	169.56 108.66		

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

TAMARACK LUMBER INC
ALPA LUMBER GROUP

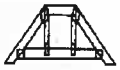
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301364 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-8C ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G188 COMMON	10.00 / 0.00	17-04-00	08-10-06	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	89.86 / 58.83		
	1	P164 PIGGYBACK	10.00 / 0.00	04-03-15	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	15.06 / 11.33		
	3	P165 PIGGYBACK	10.00 / 0.00	04-03-15	02-02-07	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	39.69 / 26.01		
	1	P166 PIGGYBACK	10.00 / 0.00	04-03-15	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	14.25 / 9.67		
	4	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	67.16 / 42.68		
	4	J6 JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	60.40 / 38.68		
	1	J7 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 02-01-01	01-07-11 / 00-03-08	10.65 / 7.33		
	1	J8 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	8.37 / 6.00		

TOTAL # TRUSS= 51.00 TOTAL BFT OF ALL TRUSSES= 2547.20 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4017.99 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
10	Hangers	LJS26DS	
2	Hangers	HGUS26-2	

TOTAL # ITEMS= 12.00

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Right

TAMARACK LUMBER INC
ALPA LUMBER GROUP

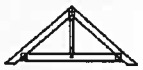
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301365 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-5 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T85 PIGGYBACK	6.00 / 0.00	43-00-00	09-10-07	2 X 6	2 X 6	01-08-08 / 01-03-08	01-04-08 / 01-02-00	1382.55 / 819.15		
	1	G85 PIGGYBACK	6.00 / 0.00	43-00-00	09-10-07	2 X 6	2 X 6	01-08-08 / 01-03-08	01-04-08 / 01-02-00	272.58 / 171.00		
	5	T86 PIGGYBACK	6.00 / 0.00	45-05-00	09-10-07	2 X 6	2 X 6	01-08-08 / 00-00-00	01-04-08 / 02-11-02	1452.65 / 872.50		
	1	G86 PIGGYBACK	6.00 / 0.00	45-05-00	09-10-07	2 X 6	2 X 6	01-08-08 / 00-00-00	01-04-08 / 02-11-02	285.34 / 180.50		
	12	P84 PIGGYBACK	6.00 / 0.00	06-08-10	02-00-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	237.24 / 147.96		

TOTAL # TRUSS= 24.00 TOTAL BFT OF ALL TRUSSES= 2191.11 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3630.36 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= 40.00

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301366 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-4 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 4 Ply	T45 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 6	2 X 8	00-00-00 / 00-00-00	01-07-11 / 00-07-04	395.20 / 249.32		
	3	T46 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-03-08	174.93 / 111.99		
	1	G47 COMMON	10.00 / 0.00	11-02-00	06-03-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	54.26 / 37.00		
	1	T81 ROOF	6.00 / 0.00	40-06-00	07-06-15	2 X 6	2 X 6	01-08-08 / 00-00-00	01-04-08 / 02-07-11	236.52 / 144.83		
	4	T82 PIGGYBACK	6.00 / 0.00	40-06-00	09-02-15	2 X 6	2 X 6	01-08-08 / 00-00-00	01-04-08 / 02-07-11	1024.28 / 614.68		
	1	G82 PIGGYBACK	6.00 / 0.00	40-06-00	09-02-15	2 X 6	2 X 6	01-08-08 / 00-00-00	01-04-08 / 02-07-11	269.04 / 168.50		
	2	T83 PIGGYBACK	6.00 / 0.00	39-06-00	09-10-07	2 X 6	2 X 6	01-08-08 / 01-00-08	01-04-08 / 01-00-08	453.30 / 270.00		
	1	G83 PIGGYBACK	6.00 / 0.00	39-06-00	09-10-07	2 X 6	2 X 6	01-08-08 / 01-00-08	01-04-08 / 01-00-08	245.88 / 157.00		
	3	T84 PIGGYBACK	6.00 / 0.00	40-06-00	09-10-07	2 X 6	2 X 6	01-08-08 / 01-03-08	01-04-08 / 02-07-11	729.09 / 438.51		
	2	P81 PIGGYBACK	6.00 / 0.00	15-03-02	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-13	98.12 / 61.66		
	2	P82 PIGGYBACK	6.00 / 0.00	15-03-02	03-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-13	98.98 / 63.34		
	6	P83 PIGGYBACK	6.00 / 0.00	02-11-10	01-01-01	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	57.42 / 36.00		

TOTAL # TRUSS= 30.00 TOTAL BFT OF ALL TRUSSES= 2352.83 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3837.02 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= 5.00

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

TAMARACK LUMBER INC
ALPA LUMBER GROUP

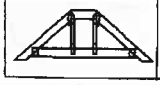
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301368 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-5 ELEVATION: B2 W/ FIREWALL

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T51 HIP	6.00 / 0.00	43-00-00	08-02-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	254.34 / 152.50		
	1	T52 PIGGYBACK	6.00 / 0.00	43-00-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	277.11 / 163.83		
	6	T53 PIGGYBACK	6.00 / 0.00	45-05-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-07-11	1772.94 / 1066.98		
	3	T54 ROOF	6.00 / 0.00	43-00-00	09-06-08	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	810.60 / 489.99		
	1	G54A COMMON	6.00 / 0.00	33-06-00	09-06-08	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 01-02-00	167.21 / 100.83		
	5	P41 PIGGYBACK	6.00 / 0.00	06-03-10	01-11-01	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	88.15 / 56.65		
	1	P42 PIGGYBACK	6.00 / 0.00	06-03-10	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	18.99 / 12.33		

TOTAL # TRUSS= 18.00 TOTAL BFT OF ALL TRUSSES= 2043.11 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3389.34 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
44	Hangers	TBE6	

TOTAL # ITEMS= 44.00

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

TAMARACK LUMBER INC
ALPA LUMBER GROUP

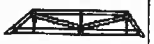
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301369 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-4 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T41 ROOF	6.00 / 0.00	40-06-00	07-06-15	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-07-11	238.18 / 143.50		
	4	T42 PIGGYBACK	6.00 / 0.00	40-06-00	09-02-15	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-07-11	1011.84 / 610.68		
	1	G42 PIGGYBACK	6.00 / 0.00	40-06-00	09-02-15	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-07-11	267.92 / 167.50		
	2	T43 PIGGYBACK	6.00 / 0.00	39-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	446.74 / 267.34		
	1	G43 PIGGYBACK	6.00 / 0.00	39-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	244.34 / 155.00		
	3	T44 PIGGYBACK	6.00 / 0.00	40-06-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 02-07-11	718.20 / 433.50		
	1 / 4 Ply	T45 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 6	2 X 8	00-00-00 / 00-00-00	01-07-11 / 00-07-04	395.20 / 249.32		
	3	T46 MONOPITCH	10.00 / 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-03-08	174.93 / 111.99		
	1	G47 COMMON	10.00 / 0.00	11-02-00	06-03-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	54.26 / 37.00		
	2	P31 PIGGYBACK	6.00 / 0.00	14-10-02	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-13	95.56 / 60.34		
	2	P32 PIGGYBACK	6.00 / 0.00	14-10-02	03-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-13	94.08 / 58.66		
	6	P33 PIGGYBACK	6.00 / 0.00	02-09-10	01-00-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	55.02 / 36.00		

TOTAL # TRUSS= 30.00 TOTAL BFT OF ALL TRUSSES= 2330.83 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3796.27 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= 5.00

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301371 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-5 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T52 PIGGYBACK	6.00 / 0.00	43-00-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	1385.55 / 819.15		
	1	G52 PIGGYBACK	6.00 / 0.00	43-00-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	264.17 / 167.50		
	5	T55 PIGGYBACK	6.00 / 0.00	45-05-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-11-02	1477.45 / 885.85		
	1	G55 PIGGYBACK	6.00 / 0.00	45-05-00	09-10-07	2 X 6	2 X 6	01-03-08 / 00-00-00	01-02-00 / 02-11-02	276.92 / 176.00		
	12	P41 PIGGYBACK	6.00 / 0.00	06-03-10	01-11-01	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-03	211.56 / 135.96		

TOTAL # TRUSS= 24.00 TOTAL BFT OF ALL TRUSSES= 2184.46 BFT. TO AL WEIGHT OF ALL TRUSSES= 3615.65 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= 40.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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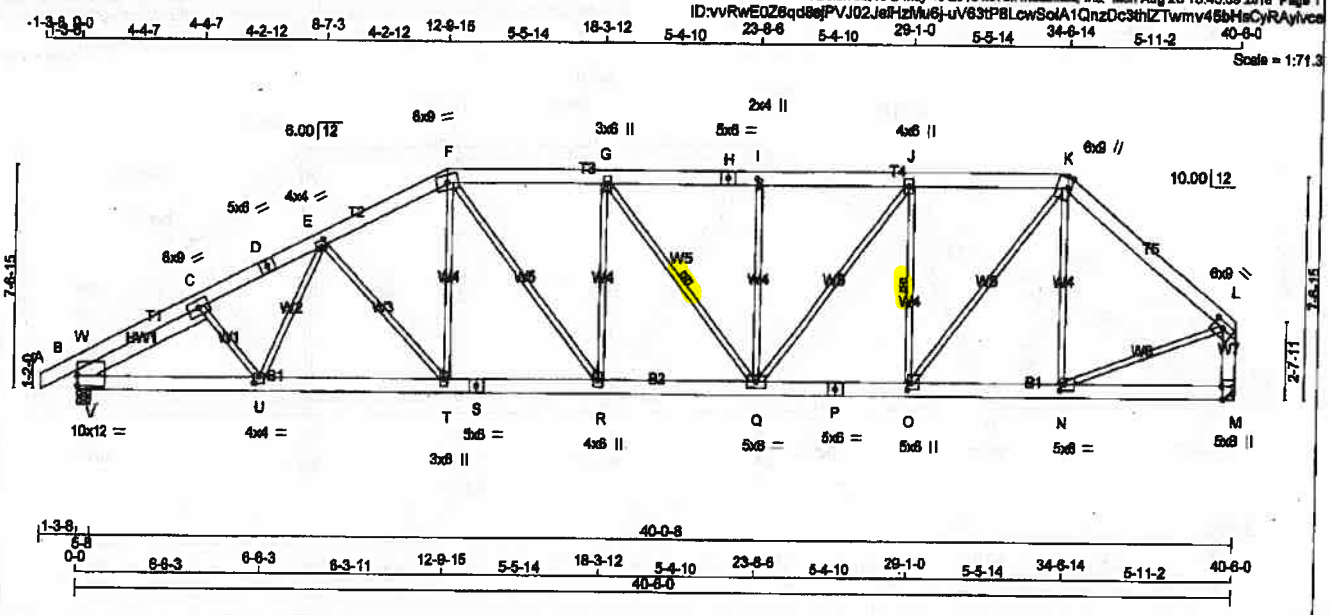

TOTAL WEIGHT = 238 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
B - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	4.25	
C	TMWW-t	MT20	6.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TTWW-m	MT20	6.0	9.0		
G	TMWW+t	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TMWW+t	MT20	4.0	6.0		
K	TTWW+m	MT20	6.0	9.0	4.00	1.25
L	TMVW-t	MT20	6.0	9.0	2.50	4.25
M	BMV1+t	MT20	5.0	6.0	Edge	0.50
N	BMWW-t	MT20	5.0	6.0	2.50	1.75
O	BMWW+t	MT20	5.0	6.0	2.75	1.50
P	SS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW+t	MT20	4.0	6.0		
S	SS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	3.0	8.0		
U	BMWW-t	MT20	4.0	4.0	2.50	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	FACTORED INPUT BRG IN-SX	REQRD BRG IN-BX
B	3586	0	3586	0	0	6-8	5-8
M	3405	0	3405	0	0	MECHANICAL	

MGW526

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2623	1654 / 0	425 / 0	0 / 0	0 / 0	543 / 0	0 / 0
M	2500	1548 / 0	425 / 0	0 / 0	0 / 0	527 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-129.7	-129.7	0.08 (1)	10.00	
B- W	-3997 / 0	-129.7	-129.7	0.25 (1)	4.11	
W- C	-2325 / 0	-129.7	-129.7	0.20 (1)	6.18	
C- D	-5232 / 0	-129.7	-129.7	0.28 (1)	3.64	
D- E	-5232 / 0	-129.7	-129.7	0.28 (1)	3.64	
E- F	-4730 / 0	-129.7	-129.7	0.27 (1)	3.83	
F- G	-4868 / 0	-129.7	-129.7	0.40 (1)	3.68	
G- H	-4733 / 0	-129.7	-129.7	0.39 (1)	3.70	
H- I	-4733 / 0	-129.7	-129.7	0.39 (1)	3.70	
I- J	-4733 / 0	-129.7	-129.7	0.38 (1)	3.71	
J- K	-3977 / 0	-129.7	-129.7	0.34 (1)	4.04	
K- L	-3215 / 0	-129.7	-129.7	0.48 (1)	4.24	
M- L	-3314 / 0	0.0	0.0	0.27 (1)	5.84	
B- V	0 / 2055	-38.5	-38.5	0.27 (1)	10.00	
V- U	0 / 4598	-38.5	-38.5	0.63 (1)	10.00	
U- T	0 / 4862	-38.5	-38.5	0.63 (1)	10.00	
T- S	0 / 4218	-38.5	-38.5	0.58 (1)	10.00	
S- R	0 / 4218	-38.5	-38.5	0.58 (1)	10.00	
R- Q	0 / 4857	-38.5	-38.5	0.64 (1)	10.00	
Q- P	0 / 3978	-38.5	-38.5	0.54 (1)	10.00	
P- O	0 / 3978	-38.5	-38.5	0.54 (1)	10.00	
O- N	0 / 2454	-38.5	-38.5	0.39 (1)	10.00	
N- M	0 / 0	-38.5	-38.5	0.12 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 197	0.04 (2)
U- E	0 / 187	0.04 (3)
E- T	-674 / 0	0.52 (1)
T- F	0 / 749	0.17 (1)
F- R	0 / 1045	0.24 (1)
R- G	-624 / 0	0.59 (1)
G- Q	-204 / 0	0.11 (1)
Q- I	-644 / 0	0.59 (1)
Q- J	0 / 1253	0.28 (1)
O- J	-1770 / 0	0.59 (1)
O- K	0 / 2488	0.58 (1)
N- K	-601 / 0	0.55 (1)
N- L	0 / 2610	0.59 (1)
V- W	0 / 2085	0.05 (3)
V- C	-3157 / 0	0.68 (1)

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.B.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.48/1.00 (K-L:1) , BC=0.64/1.00 (Q-R:1) , WB=0.63/1.00 (C-V:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.89 (L) (INPUT = 0.80)
CSI METAL= 0.77 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805348
STRUCTURAL COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T42	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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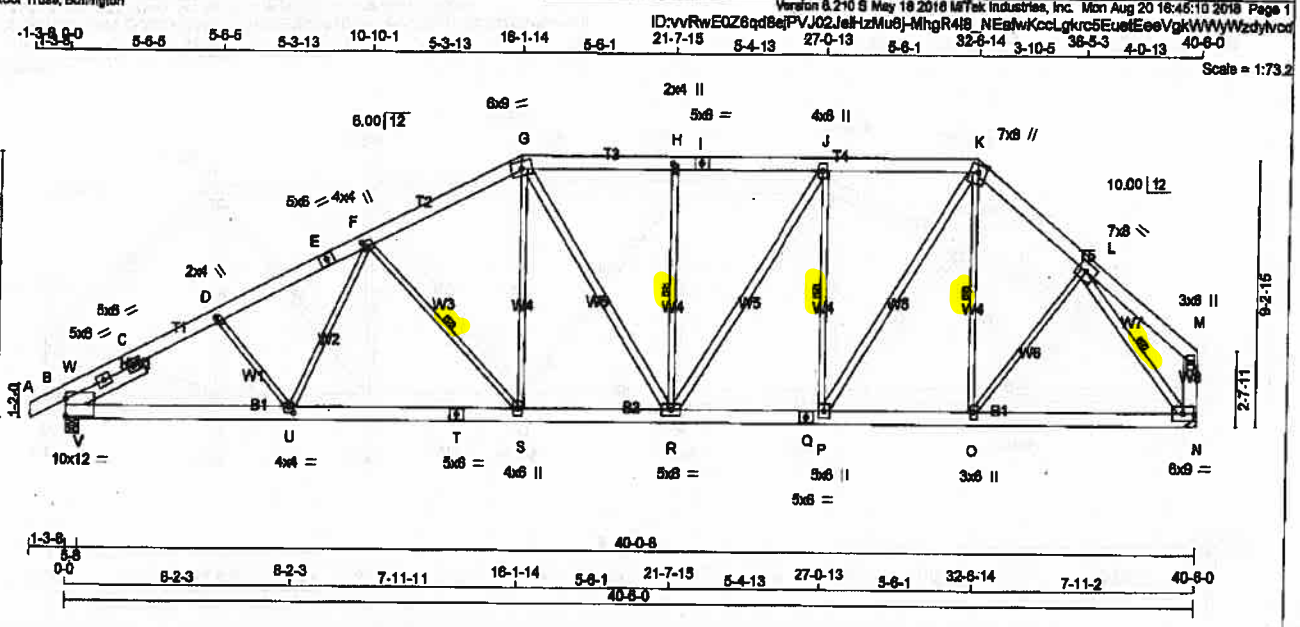

TOTAL WEIGHT = 4 X 253 = 1012 lb (MIF)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - R	2x4	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
L - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	10.0	12.0	7.00	
B	RT-t	MT20	5.0	8.0		
B	RT-I	MT20	5.0	8.0		
D	TMW+w	MT20	2.0	4.0	2.00	0.75
E	TS-t	MT20	5.0	8.0		
F	TMWW+t	MT20	4.0	4.0	2.50	1.50
G	TTWW-m	MT20	8.0	9.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TS-t	MT20	5.0	8.0		
J	TMWW+t	MT20	4.0	8.0		
K	TTWW+m	MT20	7.0	8.0	Edge	
L	TMWW-t	MT20	7.0	8.0		
M	TMV+p	MT20	3.0	8.0		
N	BMWW+t	MT20	8.0	9.0		
O	BMWW+t	MT20	3.0	8.0		
P	BMWW+t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	8.0		
T	BS-t	MT20	5.0	8.0		
U	BMWW-t	MT20	4.0	4.0	2.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3588	0	3588	0	0	5-8	5-8
N	3405	0	3405	0	0	MECHANICAL	- HGUS26

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2623	1654 / 0	425 / 0	0 / 0	0 / 0	543 / 0	0 / 0
N	2500	1548 / 0	425 / 0	0 / 0	0 / 0	527 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.80 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

8 LATERAL BRACE(S) AT 1/2 LENGTH OF F-S, H-R, J-P, K-Q, L-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-129.7	-129.7	0.08 (1)	10.00
B- W	-4633 / 0	-129.7	-129.7	0.15 (1)	3.98
W- C	-3678 / 0	-129.7	-129.7	0.59 (1)	3.78
C- D	-3678 / 0	-129.7	-129.7	0.59 (1)	3.78
D- E	-5070 / 0	-129.7	-129.7	0.43 (1)	3.54
E- F	-5070 / 0	-129.7	-129.7	0.43 (1)	3.54
F- G	-4270 / 0	-129.7	-129.7	0.35 (1)	3.91
G- H	-3945 / 0	-129.7	-129.7	0.29 (1)	4.11
H- I	-3945 / 0	-129.7	-129.7	0.28 (1)	4.14
I- J	-3945 / 0	-129.7	-129.7	0.28 (1)	4.14
J- K	-3528 / 0	-129.7	-129.7	0.27 (1)	4.33
K- L	-3303 / 0	-129.7	-129.7	0.16 (1)	4.68
L- M	0 / 29	-129.7	-129.7	0.12 (1)	10.00
N- M	-208 / 0	0.0	0.0	0.02 (1)	7.81
B- V	0 / 2054	-38.5	-38.5	0.49 (1)	10.00
V- U	0 / 4589	-38.5	-38.5	0.79 (1)	10.00
U- T	0 / 4485	-38.5	-38.5	0.67 (1)	10.00
T- S	0 / 4485	-38.5	-38.5	0.67 (1)	10.00
S- R	0 / 5793	-38.5	-38.5	0.68 (1)	10.00
R- Q	0 / 3527	-38.5	-38.5	0.48 (1)	10.00
Q- P	0 / 3527	-38.5	-38.5	0.48 (1)	10.00
P- O	0 / 2806	-38.5	-38.5	0.44 (1)	10.00
O- N	0 / 2226	-38.5	-38.5	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- U	0 / 135	0.03 (3)
U- F	0 / 311	0.07 (3)
F- S	-1050 / 0	0.48 (1)
S- G	0 / 1081	0.24 (1)
G- R	0 / 266	0.05 (1)
R- H	-753 / 0	0.38 (1)
R- J	0 / 789	0.13 (1)
P- J	-1434 / 0	0.74 (1)
P- K	0 / 1917	0.31 (1)
Q- K	-48 / 178	0.04 (3)
Q- L	0 / 458	0.10 (1)
L- N	-3792 / 0	0.78 (1)
V- W	87 / 345	0.00 (1)
V- C	-3538 / 0	0.31 (1)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. MYERS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	39.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN.O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.69/1.00 (D-W:1) , BC=0.79/1.00 (U-V:1) , WB=0.77/1.00 (L-N:1) , SSI=0.48/1.00 (B-V:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1867	768	1997	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.90 (U) (INPUT = 0.90)
CSI METAL= 0.86 (T) (INPUT = 1.00)

DWG A.O. TAM 11305349
STRUCTURAL
CONSIDERATIONS ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G42	1	1	TRUSS DESC.	

Tamarack Roof Trusses, Burlington

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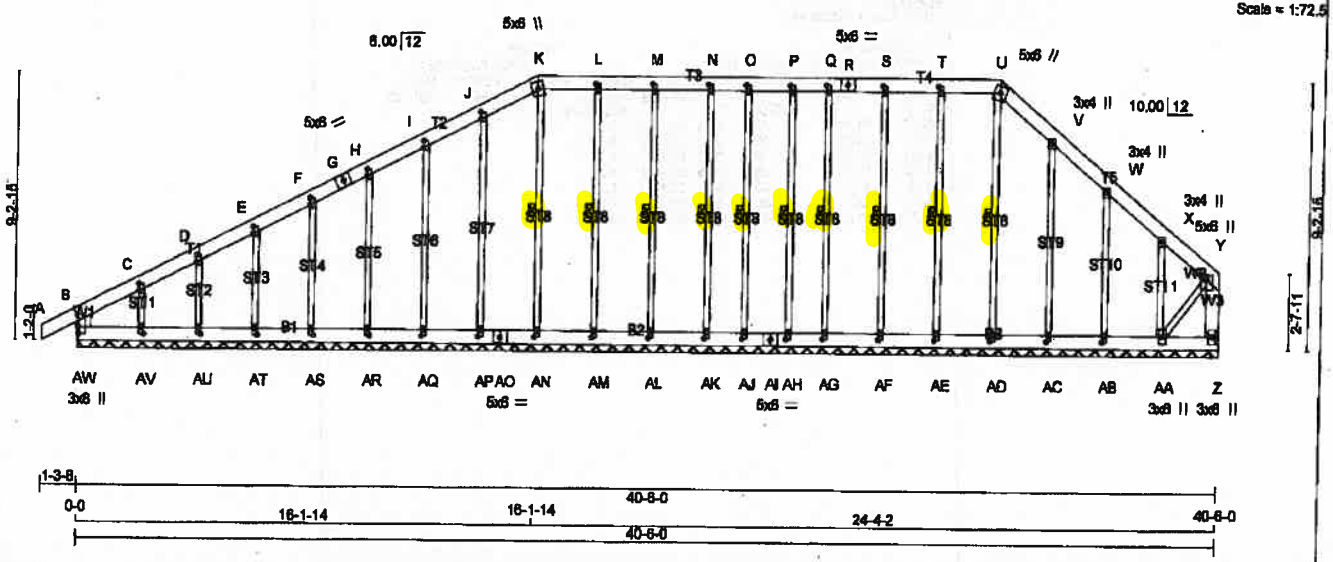

TOTAL WEIGHT = 288 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AW B	2x6	DRY	No.2	SPF
A - G	2x6	DRY	No.2	SPF
G - K	2x6	DRY	No.2	SPF
K - R	2x6	DRY	No.2	SPF
R - U	2x6	DRY	No.2	SPF
U - Y	2x6	DRY	No.2	SPF
Z - Y	2x6	DRY	No.2	SPF
AW AO	2x6	DRY	No.2	SPF
AO AI	2x6	DRY	No.2	SPF
AI - Z	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C, D, E, F, H, I, J						
C	TMW+w	MT20	2.0	4.0	3.00	
G	TS-t	MT20	5.0	6.0		
K	TTW+m	MT20	5.0	6.0		
L, M, N, O, P, Q, S, T						
L	TMW+w	MT20	2.0	4.0	2.50	1.00
R	TS-t	MT20	5.0	6.0		
U	TTW+m	MT20	5.0	6.0		
V, W, X						
V	TMW+w	MT20	3.0	4.0		
Y	TMWW+p	MT20	6.0	8.0	2.00	2.25
Z	BMV1+p	MT20	3.0	6.0		
AA	BMWW1+t	MT20	3.0	6.0		
AB, AC, AD, AE, AF, AG, AH, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU, AV						
AB	BMW1+w	MT20	2.0	4.0	2.50	1.00
AI	BS-t	MT20	5.0	6.0		
AO	BS-t	MT20	5.0	6.0		
AW						
AW	TMBMV1+p	MT20	3.0	8.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF U-AD, T-AE, S-AF, Q-AG, P-AH, O-AJ, N-AK, M-AL, L-AM, K-AN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AW- B	-296 / 0	0.0	0.0	0.02 (1)	7.81	
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
B- C	0 / 11	-129.7	-129.7	0.04 (1)	10.00	
C- D	0 / 17	-129.7	-129.7	0.04 (1)	10.00	
D- E	0 / 29	-129.7	-129.7	0.03 (1)	10.00	
E- F	0 / 37	-129.7	-129.7	0.03 (1)	10.00	
F- G	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
G- H	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
H- I	0 / 49	-129.7	-129.7	0.04 (1)	10.00	
I- J	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
J- K	0 / 46	-129.7	-129.7	0.04 (1)	10.00	
K- L	0 / 54	-129.7	-129.7	0.04 (1)	10.00	
L- M	0 / 54	-129.7	-129.7	0.04 (1)	10.00	
M- N	0 / 54	-129.7	-129.7	0.03 (1)	10.00	
N- O	0 / 54	-129.7	-129.7	0.03 (1)	10.00	
O- P	0 / 54	-129.7	-129.7	0.02 (1)	10.00	
P- Q	0 / 54	-129.7	-129.7	0.03 (1)	10.00	
Q- R	0 / 54	-129.7	-129.7	0.03 (1)	10.00	
R- S	0 / 54	-129.7	-129.7	0.03 (1)	10.00	
S- T	0 / 54	-129.7	-129.7	0.04 (1)	10.00	
T- U	0 / 54	-129.7	-129.7	0.04 (1)	10.00	
U- V	0 / 50	-129.7	-129.7	0.04 (1)	10.00	
V- W	0 / 60	-129.7	-129.7	0.04 (1)	10.00	
W- X	0 / 50	-129.7	-129.7	0.04 (1)	10.00	
X- Y	0 / 50	-129.7	-129.7	0.03 (1)	10.00	
Z- Y	-48 / 0	0.0	0.0	0.00 (1)	7.81	
AW-AV	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
AV-AU	-17 / 0	-38.5	-38.5	0.02 (3)	6.25	
AU-AT	-28 / 0	-38.5	-38.5	0.01 (3)	6.25	
AT-AS	-33 / 0	-38.5	-38.5	0.01 (3)	6.25	
AS-AR	-39 / 0	-38.5	-38.5	0.01 (3)	6.25	
AR-AQ	-43 / 0	-38.5	-38.6	0.01 (3)	6.25	
AQ-AP	-47 / 0	-38.5	-38.5	0.01 (3)	6.25	
AP-AO	-51 / 0	-38.5	-38.5	0.01 (3)	6.25	
AO-AN	-51 / 0	-38.5	-38.5	0.01 (3)	6.25	
AN-AM	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AM-AL	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AL-AK	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AK-AJ	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AJ-AI	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AI-AH	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	
AH-AG	-54 / 0	-38.5	-38.5	0.01 (3)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AD- U	-262 / 0	0.13 (1)
AE- T	-294 / 0	0.15 (1)
AF- S	-261 / 0	0.13 (1)
AG- Q	-214 / 0	0.11 (1)
AH- P	-183 / 0	0.09 (1)
AJ- O	-183 / 0	0.09 (1)
AK- N	-214 / 0	0.11 (1)
AL- M	-261 / 0	0.13 (1)
AM- L	-293 / 0	0.15 (1)
AP- J	-262 / 0	0.34 (1)
AQ- I	-257 / 0	0.21 (1)
AR- H	-256 / 0	0.14 (1)
AS- F	-257 / 0	0.09 (1)
AT- E	-258 / 0	0.08 (1)
AU- D	-249 / 0	0.04 (1)
AV- C	-291 / 0	0.04 (1)
AC- V	-285 / 0	0.27 (1)
AB- W	-251 / 0	0.12 (1)
AA- X	-285 / 0	0.09 (1)
AA- Y	-39 / 0	0.04 (1)
AN- K	-245 / 0	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.B.F. G.B.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (AU-AV:3) , WB=0.34/1.00 (J-AP:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	BHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.51 (AD) (INPUT = 0.90)
SI METAL= 0.12 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805326
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
jpardoto
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G42	1	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AG-AF	-54 / 0	-38.5	-38.5	0.01	(3)	6.25
AF-AE	-54 / 0	-38.5	-38.5	0.01	(3)	6.25
AE-AD	-54 / 0	-38.5	-38.5	0.01	(3)	6.25
AD-AC	-51 / 0	-38.6	-38.5	0.01	(3)	6.25
AC-AB	-45 / 0	-38.5	-38.5	0.01	(3)	6.25
AB-AA	-35 / 0	-38.5	-38.5	0.01	(3)	6.25
AA-Z	0 / 0	-38.5	-38.5	0.01	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805325
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T43	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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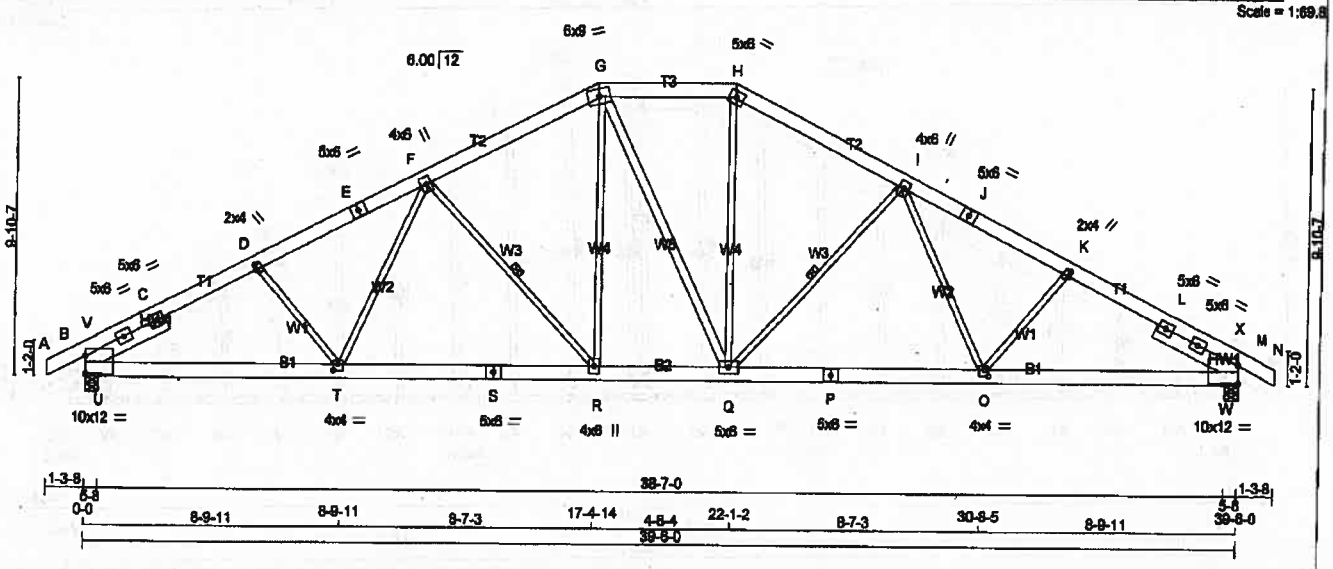

TOTAL WEIGHT = 2 X 223 = 447 lb.

[MUF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - N	2x6	DRY	No.2	SPF
B - B	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	10.0	12.0	7.00	
B, B, M, M						
B	RT-t	MT20	5.0	6.0		
D	TMW+w	MT20	2.0	4.0	2.00	0.75
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TTWW-m	MT20	6.0	9.0		
H	TTW-h	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMW+w	MT20	2.0	4.0	2.00	0.75
M	TMBMR1-I	MT20	10.0	12.0	7.00	
O	BMWW-t	MT20	4.0	4.0	2.50	1.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0	2.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3502	0	3502	0	0	5-8	5-8
M	3502	0	3502	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2591	1616 / 0	415 / 0	0 / 0	0 / 0	530 / 0	0 / 0
M	2591	1616 / 0	415 / 0	0 / 0	0 / 0	530 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R, I-Q.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-129.7	-129.7	0.08 (1)	10.00	
B- V	-4617 / 0	-129.7	-129.7	0.15 (1)	3.99	
V- C	-3594 / 0	-129.7	-129.7	0.60 (1)	3.61	
C- D	-3594 / 0	-129.7	-129.7	0.60 (1)	3.61	
D- E	-4880 / 0	-129.7	-129.7	0.43 (1)	3.61	
E- F	-4880 / 0	-129.7	-129.7	0.43 (1)	3.61	
F- G	-3898 / 0	-129.7	-129.7	0.37 (1)	4.05	
G- H	-3468 / 0	-129.7	-129.7	0.23 (1)	4.39	
H- I	-3905 / 0	-129.7	-129.7	0.37 (1)	4.04	
I- J	-4877 / 0	-129.7	-129.7	0.43 (1)	3.61	
J- K	-4877 / 0	-129.7	-129.7	0.43 (1)	3.61	
K- L	-3597 / 0	-129.7	-129.7	0.60 (1)	3.61	
L- X	-3597 / 0	-129.7	-129.7	0.60 (1)	3.61	
X- M	-4629 / 0	-129.7	-129.7	0.15 (1)	3.98	
M- N	0 / 1	-129.7	-129.7	0.08 (1)	10.00	
B- U	0 / 2041	-38.5	-38.5	0.51 (1)	10.00	
U- T	0 / 4491	-38.5	-38.5	0.00 (1)	10.00	
T- S	0 / 4260	-38.5	-38.5	0.66 (1)	10.00	
S- R	0 / 4260	-38.5	-38.5	0.66 (1)	10.00	
R- Q	0 / 3480	-38.5	-38.5	0.56 (1)	10.00	
Q- P	0 / 4261	-38.5	-38.5	0.65 (1)	10.00	
P- O	0 / 4261	-38.5	-38.5	0.65 (1)	10.00	
O- W	0 / 4490	-38.5	-38.5	0.61 (1)	10.00	
W- M	0 / 2047	-38.5	-38.5	0.62 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- T	-113 / 100	0.04 (1)
T- F	0 / 445	0.10 (2)
F- R	-1192 / 0	0.62 (1)
R- G	0 / 1124	0.25 (1)
G- Q	0 / 14	0.00 (1)
Q- H	0 / 1139	0.28 (1)
Q- I	-1165 / 0	0.61 (1)
I- O	0 / 436	0.10 (2)
O- K	-114 / 99	0.04 (1)
U- V	0 / 3450	0.00 (1)
U- C	-3419 / 0	0.30 (1)
W- X	0 / 3455	0.00 (1)
W- L	-3408 / 0	0.30 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AUGES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.80/1.00 (D-V:1) , BC=0.61/1.00 (O-W:1) , WB=0.62/1.00 (F-R:1) , SSI=0.64/1.00 (M-W:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (T) (INPUT = 0.90)
JSI METAL= 0.93 (S) (INPUT = 1.00)

DWG NO. TAM 1805350
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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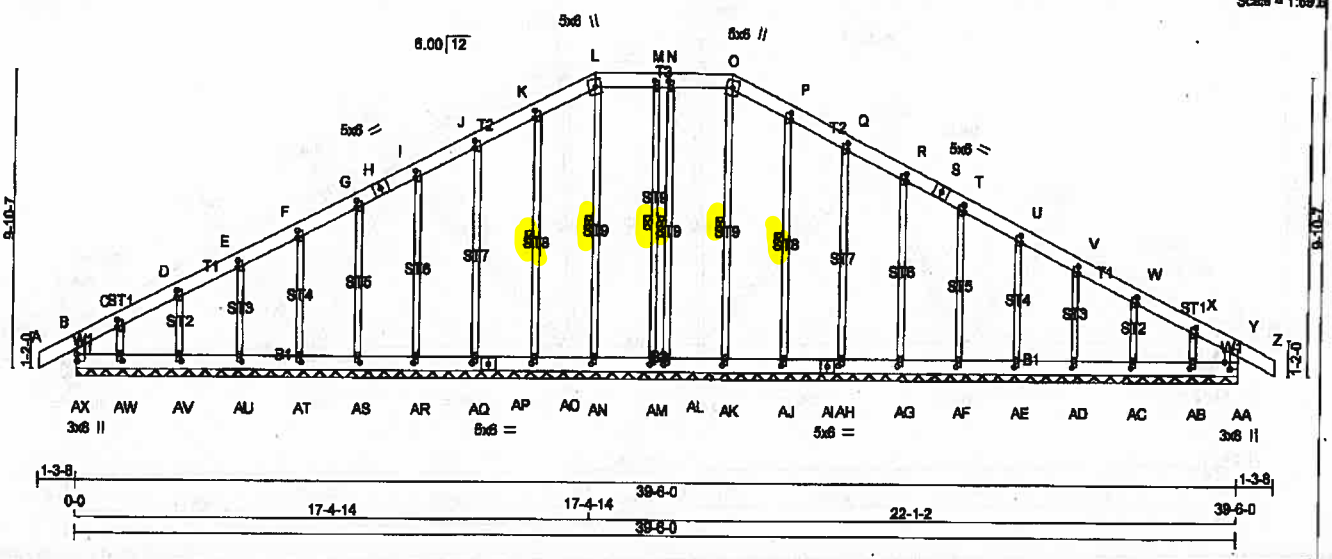

TOTAL WEIGHT = 244 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AX- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - O	2x6	DRY	No.2	SPF
O - S	2x6	DRY	No.2	SPF
B - Z	2x6	DRY	No.2	SPF
AA- Y	2x6	DRY	No.2	SPF
AX- AP	2x6	DRY	No.2	SPF
AP- AI	2x6	DRY	No.2	SPF
AI - AA	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, Y, AA, AX						
B						
C, K, X						
C	TMW+w	MT20	2.0	4.0	2.75	
D, E, F, G, I, J, P, Q, R, T, U, V, W						
D	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	6.0		
M	TMW+w	MT20	2.0	4.0	2.50	1.00
N	TMW+w	MT20	2.0	4.0	2.50	1.00
O	TTW+m	MT20	5.0	6.0		
S	TS-t	MT20	5.0	6.0		
AA	TMBMV1+p	MT20	3.0	6.0	2.75	2.25
AB, AC, AD, AE, AF, AG, AH, AJ, AK, AL, AM, AN, AO, AQ, AR, AS, AT, AU, AV, AW						
AB	BMW1+w	MT20	2.0	4.0	2.50	1.00
AI	BS-t	MT20	5.0	6.0		
AP	BS-t	MT20	5.0	6.0		
AX	TMBMV1+p	MT20	3.0	6.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AK, N-AL, M-AM, K-AO, P-AJ, L-AN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDB

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AX- B	-368 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
B- C	-93 / 0	-129.7	-129.7	0.07 (1)	6.25	
C- D	-46 / 0	-129.7	-129.7	0.03 (1)	6.25	
D- E	-40 / 0	-129.7	-129.7	0.03 (1)	6.25	
E- F	-31 / 0	-129.7	-129.7	0.03 (1)	6.25	
F- G	-24 / 0	-129.7	-129.7	0.03 (1)	6.25	
G- H	-18 / 0	-129.7	-129.7	0.03 (1)	6.25	
H- I	-18 / 0	-129.7	-129.7	0.03 (1)	6.25	
I- J	-14 / 0	-129.7	-129.7	0.03 (1)	6.25	
J- K	-10 / 0	-129.7	-129.7	0.03 (1)	10.00	
K- L	-8 / 0	-129.7	-129.7	0.03 (1)	10.00	
L- M	-3 / 0	-129.7	-129.7	0.04 (1)	10.00	
M- N	-3 / 0	-129.7	-129.7	0.04 (1)	10.00	
N- D	-3 / 0	-129.7	-129.7	0.04 (1)	10.00	
O- P	-8 / 0	-129.7	-129.7	0.03 (1)	10.00	
P- Q	-10 / 0	-129.7	-129.7	0.03 (1)	10.00	
Q- R	-14 / 0	-129.7	-129.7	0.03 (1)	6.25	
R- B	-18 / 0	-129.7	-129.7	0.03 (1)	6.25	
S- T	-18 / 0	-129.7	-129.7	0.03 (1)	6.25	
T- U	-24 / 0	-129.7	-129.7	0.03 (1)	6.25	
U- V	-31 / 0	-129.7	-129.7	0.03 (1)	6.25	
V- W	-40 / 0	-129.7	-129.7	0.03 (1)	6.25	
W- X	-46 / 0	-129.7	-129.7	0.03 (1)	6.25	
X- Y	-93 / 0	-129.7	-129.7	0.07 (1)	6.25	
Y- Z	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
AA- Y	-368 / 0	0.0	0.0	0.04 (1)	7.81	
AX-AW	0 / 57	-38.5	-38.5	0.04 (1)	10.00	
AW-AV	0 / 47	-38.5	-38.5	0.02 (2)	10.00	
AV-AU	0 / 35	-38.5	-38.5	0.02 (2)	10.00	
AU-AT	0 / 27	-38.5	-38.5	0.01 (2)	10.00	
AT-AS	0 / 21	-38.5	-38.5	0.01 (2)	10.00	
AS-AR	0 / 16	-38.5	-38.5	0.01 (2)	10.00	
AR-AQ	0 / 12	-38.5	-38.5	0.01 (2)	10.00	
AQ-AP	0 / 9	-38.5	-38.5	0.01 (3)	10.00	
AP-AO	0 / 9	-38.5	-38.5	0.01 (3)	10.00	
AO-AN	0 / 8	-38.5	-38.5	0.01 (3)	10.00	
AN-AM	0 / 3	-38.5	-38.5	0.01 (3)	10.00	
AM-AL	0 / 3	-38.5	-38.5	0.01 (3)	10.00	
AL-AK	0 / 3	-38.5	-38.5	0.01 (3)	10.00	
AK-AJ	0 / 8	-38.5	-38.5	0.01 (3)	10.00	
AJ-AI	0 / 9	-38.5	-38.5	0.01 (3)	10.00	

WEBB

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
AK- O	-248 / 0	0.15 (1)
AL- N	-177 / 0	0.11 (1)
AM- M	-177 / 0	0.11 (1)
AO- K	-282 / 0	0.12 (1)
AQ- J	-258 / 0	0.27 (1)
AR- I	-257 / 0	0.16 (1)
AS- G	-257 / 0	0.12 (1)
AT- F	-258 / 0	0.08 (1)
AU- E	-254 / 0	0.05 (1)
AV- D	-269 / 0	0.04 (1)
AW- C	-138 / 0	0.02 (1)
AJ- P	-282 / 0	0.12 (1)
AH- Q	-258 / 0	0.27 (1)
AG- R	-257 / 0	0.16 (1)
AF- T	-257 / 0	0.12 (1)
AE- U	-258 / 0	0.08 (1)
AD- V	-254 / 0	0.05 (1)
AC- W	-269 / 0	0.04 (1)
AB- X	-138 / 0	0.02 (1)
AN- L	-249 / 0	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2015
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.04/1.00 (AW-AX:1) , WB=0.27/1.00 (J-AQ:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.51 (AK) (INPUT = 0.90)
SI METAL= 0.13 (AX) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805326
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL 2018-0833-0840 REVIEWED Building Permit Plans

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AI-AH	0 / 8	-38.5	-38.5	0.01 (3)		10.00
AH-AG	0 / 12	-38.5	-38.5	0.01 (2)		10.00
AG-AF	0 / 16	-38.5	-38.5	0.01 (2)		10.00
AF-AE	0 / 21	-38.5	-38.5	0.01 (2)		10.00
AE-AD	0 / 27	-38.5	-38.5	0.01 (2)		10.00
AD-AC	0 / 35	-38.5	-38.5	0.02 (2)		10.00
AC-AB	0 / 47	-38.6	-38.5	0.02 (2)		10.00
AB-AA	0 / 57	-38.6	-38.5	0.04 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T18O5326
STRUCTURAL
COMPONENT ONLY 2/2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T44	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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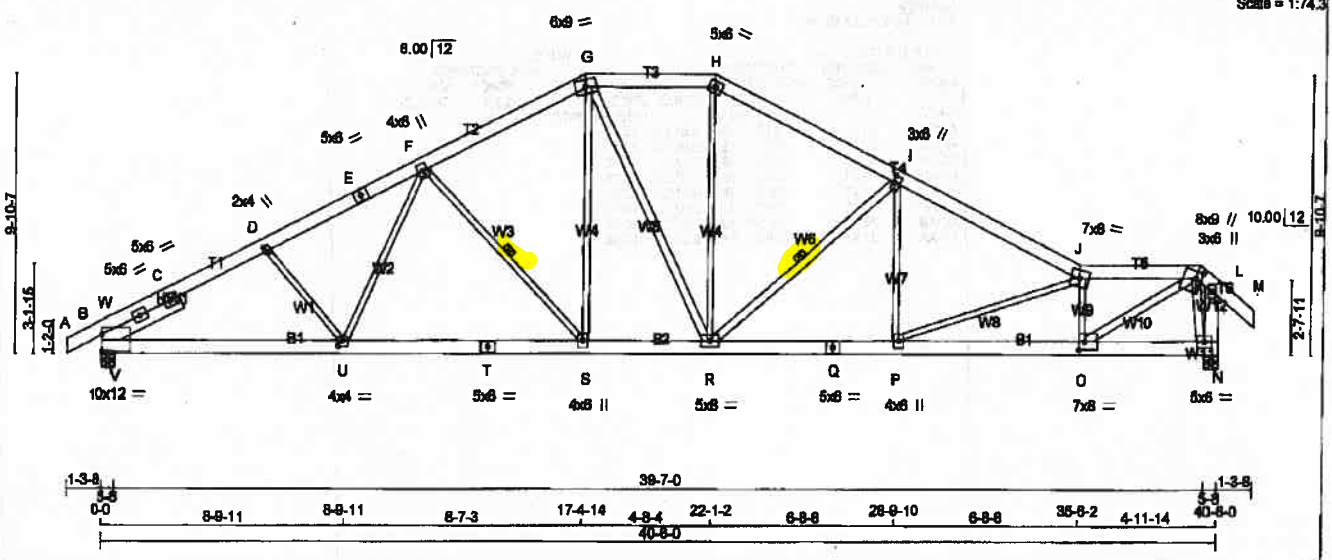

TOTAL WEIGHT = 3 X 239 = 718 lb (MTF)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - R	2x4	DRY	No.2	SPF
R - I	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	10.0	12.0	7.00	
B	RT-I	MT20	5.0	6.0		
B	RT-I	MT20	5.0	6.0		
D	TMW+w	MT20	2.0	4.0	2.00	0.75
E	TS-I	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TTWW-m	MT20	6.0	9.0		
H	TTW-h	MT20	5.0	6.0		
I	TMWW+t	MT20	3.0	6.0	3.00	0.75
J	TTWW-m	MT20	7.0	8.0		
K	TTWW+m	MT20	8.0	9.0	3.00	2.50
L	TMV+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	7.0	6.0	3.50	2.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
B	BMWW+t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	4.0	4.0	2.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3583	0	3583	0	0	5-8	5-8
N	3595	0	3595	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2821	1653 / 0	425 / 0	0 / 0	0 / 0	543 / 0	0 / 0
N	2829	1660 / 0	425 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-S, I-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-129.7	-129.7	0.08 (1)	10.00
B- W	-4738 / 0	-129.7	-129.7	0.16 (1)	3.93
W- C	-3892 / 0	-129.7	-129.7	0.62 (1)	3.72
C- D	-3892 / 0	-129.7	-129.7	0.62 (1)	3.72
D- E	-5030 / 0	-129.7	-129.7	0.44 (1)	3.54
E- F	-5030 / 0	-129.7	-129.7	0.44 (1)	3.54
F- G	-4069 / 0	-129.7	-129.7	0.38 (1)	3.66
G- H	-3842 / 0	-129.7	-129.7	0.24 (1)	4.30
H- J	-4105 / 0	-129.7	-129.7	0.45 (1)	3.88
I- J	-5334 / 0	-129.7	-129.7	0.52 (1)	3.36
J- K	-5363 / 0	-129.7	-129.7	0.27 (1)	3.59
K- L	-121 / 0	-129.7	-129.7	0.07 (1)	6.25
L- M	0 / 60	-129.7	-129.7	0.10 (1)	10.00
N- L	-422 / 0	0.0	0.0	0.03 (1)	7.81
B- V	0 / 2092	-38.5	-38.5	0.53 (1)	10.00
V- U	0 / 4817	-38.5	-38.5	0.83 (1)	10.00
U- T	0 / 4405	-38.5	-38.5	0.68 (1)	10.00
T- S	0 / 4405	-38.5	-38.5	0.68 (1)	10.00
S- R	0 / 3612	-38.5	-38.5	0.56 (1)	10.00
R- Q	0 / 4814	-38.5	-38.5	0.66 (1)	10.00
Q- P	0 / 4814	-38.5	-38.5	0.66 (1)	10.00
P- O	0 / 5471	-38.5	-38.5	0.76 (1)	10.00
O- N	0 / 603	-38.5	-38.5	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- U	-95 / 106	0.03 (1)
U- F	0 / 425	0.10 (2)
F- S	-1182 / 0	0.61 (1)
S- G	0 / 1152	0.26 (1)
G- R	0 / 68	0.01 (1)
R- H	0 / 1182	0.27 (1)
R- I	-1583 / 0	0.63 (1)
P- I	0 / 597	0.13 (2)
P- J	-707 / 0	0.73 (1)
O- J	-2767 / 0	0.47 (1)
K- N	-3171 / 0	0.39 (1)
O- K	0 / 5431	0.87 (1)
V- W	0 / 3555	0.00 (1)
V- C	-3522 / 0	0.31 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. [illegible]
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.B.F. RAIN LOAD) EQUALS 38.2 P.B.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.44")

CSI: TC=0.62/1.00 (D-W:1) , BC=0.83/1.00 (U-V:1) , WB=0.87/1.00 (K-O:1) , SSI=0.55/1.00 (B-V:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	BECTION (PLI) MAX	MIN
MT20	616	354	1697	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

KSI GRIP= 0.90 (K) (INPUT = 0.90)
KSI METAL= 0.94 (T) (INPUT = 1.00)

D.W.G NO. TAM 11805351
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T45	2	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2016 MiTek Industries, Inc. Mon Aug 20 16:45:14 2018 Page 1
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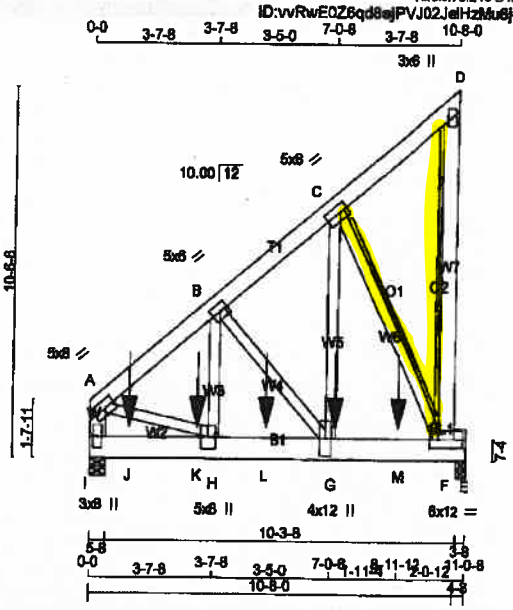

TOTAL WEIGHT = 8 X 99 = 790 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT BEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
I-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	3	4	SIDE(1466.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.60	3.25
B	TMWW-t	MT20	5.0	8.0	2.50	2.75
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TMV+p	MT20	3.0	6.0		
F	BMVWK-t	MT20	6.0	12.0	3.00	3.00
G	BMWW+t	MT20	4.0	12.0		
H	BMWW+t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	9989	0	9989	0	0	5-8	5-8
E	6652	0	6652	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	7315	4588 / 0	1204 / 0	0 / 0	0 / 0	1523 / 0	0 / 0
E	6336	3957 / 0	1049 / 0	0 / 0	0 / 0	1322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING
TOP CHORD TO BE BHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F, C-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	-8481 / 0	-129.7	-129.7	0.10 (1)	5.27
B- C	-5337 / 0	-129.7	-129.7	0.07 (1)	8.25
C- D	-32 / 0	-129.7	-129.7	0.04 (1)	8.25
F- D	-181 / 0	0.0	0.0	0.01 (1)	7.81
I- A	-7565 / 0	0.0	0.0	0.14 (1)	7.00
I- J	0 / 0	-38.5	-38.5	0.37 (1)	10.00
J- K	0 / 0	-38.5	-38.5	0.37 (1)	10.00
K- H	0 / 0	-38.5	-38.5	0.37 (1)	10.00
H- L	0 / 6481	-38.5	-38.5	0.39 (1)	10.00
L- G	0 / 6481	-38.5	-38.5	0.39 (1)	10.00
G- M	0 / 4166	-38.5	-38.5	0.60 (1)	10.00
M- F	0 / 4166	-38.5	-38.5	0.60 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.49 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	0 / 4428	0.18 (1)
B- G	-3598 / 0	0.27 (1)
G- C	0 / 11310	0.45 (1)
C- F	-9454 / 0	0.37 (1)
A- H	0 / 5749	0.27 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	7-1-14	-2465	-2465	—	BACK	VERT	TOTAL
J	1-1-14	-2465	-2465	—	BACK	VERT	TOTAL
K	3-1-14	-2465	-2465	—	BACK	VERT	TOTAL
L	5-1-14	-2465	-2465	—	BACK	VERT	TOTAL
M	8-11-12	-2465	-2465	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.14/1.00 (A-I:1) , BC=0.60/1.00 (F-G:1) , WB=0.45/1.00 (C-G:1) , SSI=0.82/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT REBPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SIGRIP= 0.90 (F) (INPUT = 0.90)
SUMETAL= 0.58 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

BCIN: 21402 TAM 118005352
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T45	2	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:vvRwE0Z8qd8ejPVJ02JelHzMu8I-FTvyw8CURS45PxvOaWpnmxOhIUbUaMWKR7wI8OvfvcZ

HANGER(S) NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3368.7 lbs FACTORED DOWN AT 1-1-14, 3368.7 lbs FACTORED DOWN AT 3-1-14, 3368.7 lbs FACTORED DOWN AT 5-1-14, AND 3368.7 lbs FACTORED DOWN AT 7-1-14, AND 3368.7 lbs FACTORED DOWN AT 8-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

P.S.D. NO. TAM T1805352
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T46	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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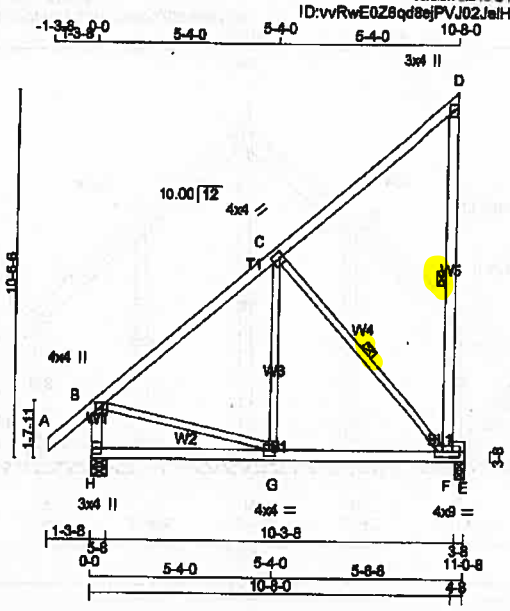

TOTAL WEIGHT = 6 X 58 = 350 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.28
D	TMV+p	MT20	3.0	4.0		
F	BMVWK-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1107	0	1107	0	0	5-8	5-8
E	881	0	881	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
H	803	527 / 0	116 / 0	0 / 0	0 / 0	160 / 0	0 / 0
E	649	394 / 0	116 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-680 / 0	-129.7	-129.7	0.67 (1)	5.91
C- D	-50 / 0	-129.7	-129.7	0.02 (1)	6.25
F- D	-284 / 0	0.0	0.0	0.14 (1)	6.25
H- B	-1048 / 0	0.0	0.0	0.11 (1)	7.88
H- G	0 / 0	-38.5	-38.5	0.34 (2)	10.00
G- F	0 / 564	-38.5	-38.5	0.71 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.28 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 316	0.08 (3)
C- F	-848 / 0	0.34 (1)
B- G	0 / 561	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(66 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 988 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 621 (0.21")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.71/1.00 (F-G:1) , WB=0.34/1.00 (C-F:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

J1 GRIP= 0.82 (F) (INPUT = 0.90)
J2 METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MOYES
PROVINCE OF ONTARIO

DWG NO. TAM 11805353
STRUCTURAL
CONSIGNMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G47	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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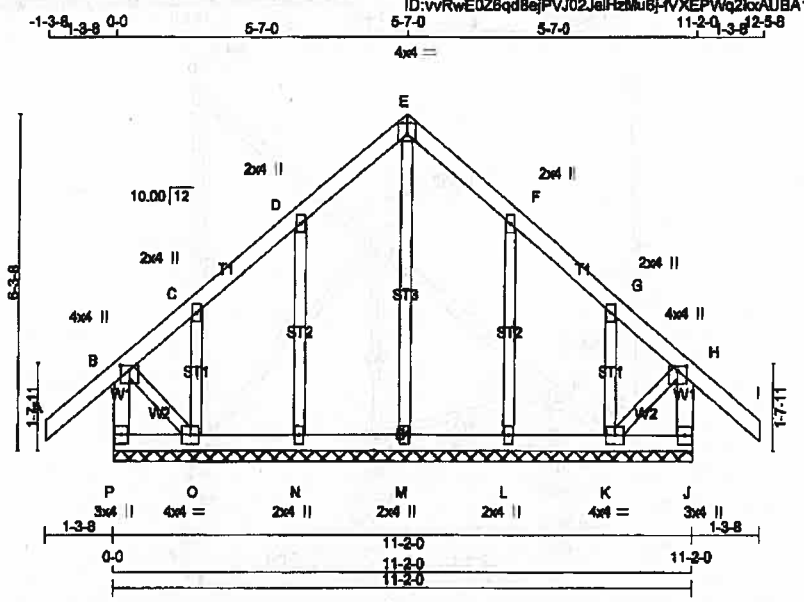

TOTAL WEIGHT = 2 X 54 = 109 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMWW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-389 / 0	0.0	0.0	0.04 (1)	7.61	
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B-C	-78 / 0	-129.7	-129.7	0.17 (1)	6.25	
C-D	-8 / 0	-129.7	-129.7	0.08 (1)	10.00	
D-E	-33 / 0	-129.7	-129.7	0.08 (1)	6.25	
E-F	-33 / 0	-129.7	-129.7	0.08 (1)	6.25	
F-G	-8 / 0	-129.7	-129.7	0.08 (1)	10.00	
G-H	-78 / 0	-129.7	-129.7	0.17 (1)	6.25	
H-I	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
J-H	-389 / 0	0.0	0.0	0.04 (1)	7.61	
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
O-N	0 / 18	-38.5	-38.5	0.02 (3)	10.00	
N-M	0 / 11	-38.5	-38.5	0.02 (3)	10.00	
M-L	0 / 11	-38.5	-38.5	0.02 (3)	10.00	
L-K	0 / 18	-38.5	-38.5	0.02 (3)	10.00	
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
M-E	-194 / 0	0.12 (1)
N-D	-314 / 0	0.10 (1)
O-C	-122 / 0	0.02 (1)
L-F	-314 / 0	0.10 (1)
K-G	-122 / 0	0.02 (1)
B-O	0 / 29	0.01 (1)
K-H	0 / 29	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 6 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (N-O:3) , WB=0.12/1.00 (E-M:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17865327
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T51	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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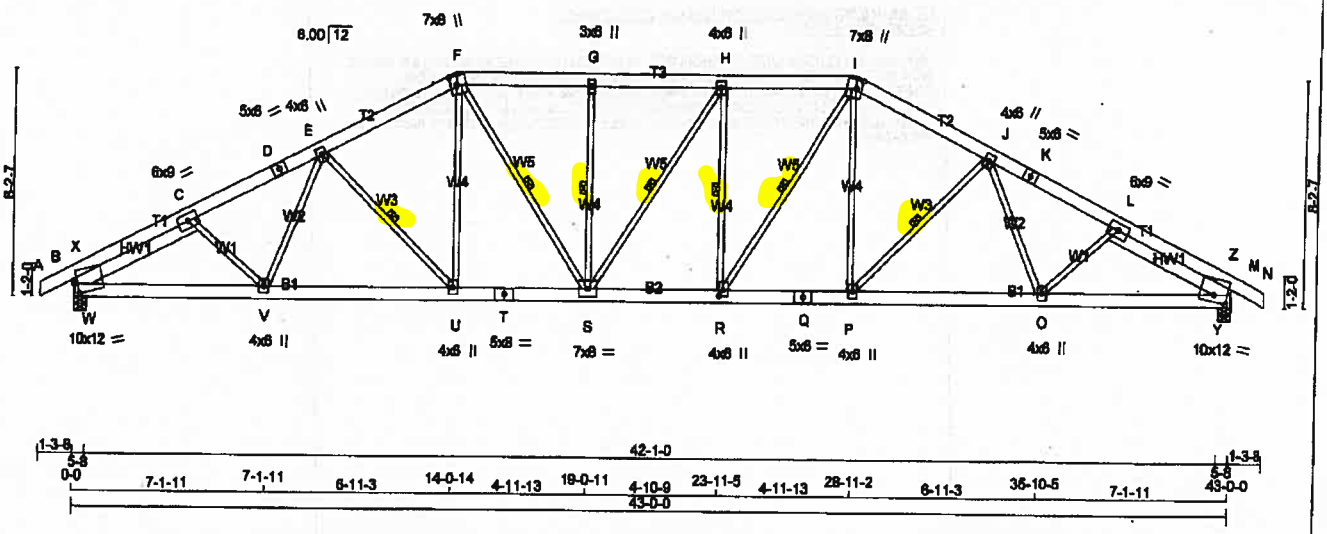

TOTAL WEIGHT = 254 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - N	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m	MT20	10.0	12.0	4.50	1.50
C	TMWW-t	MT20	6.0	9.0		
D	TS-t	MT20	5.0	6.0		
E, H, J						
E	TMWW+t	MT20	4.0	6.0		
F	TTWW+m	MT20	7.0	6.0	4.00	2.75
G	TMW+w	MT20	3.0	6.0		
I	TTWW+m	MT20	7.0	6.0	4.00	2.50
K	TS-t	MT20	5.0	6.0		
L	TMWW-t	MT20	6.0	9.0		
M	TMBMW1-m	MT20	10.0	12.0	3.00	6.00
O, P, U, V						
O	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	6.0	2.50	1.75
S	BMWWW-t	MT20	7.0	6.0		
T	BS-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
Aug/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4729	0	5042	217	-1040	5-8	~~5-8~~ 2 TBE6
M	4729	0	5042	0	-1040	5-8	~~5-8~~ 2 TBE6

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1040 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1040 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 217 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3399	2580 / 0	452 / 0	0 / 0	179 / -1113	576 / 0	0 / 0
M	3399	2580 / 0	452 / 0	0 / 0	97 / -1113	576 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	155 / -155	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-S, G-S, H-S, H-R, I-R, J-P.

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B-X	-5533 / 1006	-170.4	-170.4	0.38 (2)	3.42
X-C	-3212 / 734	-170.4	-170.4	0.30 (2)	4.45
C-D	-7237 / 1609	-170.4	-170.4	0.62 (2)	2.78
D-E	-7237 / 1606	-170.4	-170.4	0.62 (2)	2.78
E-F	-6280 / 1503	-170.4	-170.4	0.56 (2)	3.08
F-G	-6194 / 1539	-170.4	-170.4	0.51 (2)	3.14
G-H	-6196 / 1540	-170.4	-170.4	0.50 (2)	3.16
H-I	-6205 / 1544	-170.4	-170.4	0.51 (3)	3.14
I-J	-6278 / 1503	-170.4	-170.4	0.56 (3)	3.08
J-K	-7238 / 1610	-170.4	-170.4	0.62 (3)	2.77
K-L	-7238 / 1610	-170.4	-170.4	0.62 (3)	2.77
L-Z	-3211 / 735	-170.4	-170.4	0.30 (3)	4.45
Z-M	-5530 / 1006	-170.4	-170.4	0.38 (3)	3.42
M-N	0 / 2	-170.4	-170.4	0.13 (3)	10.00
B-W	-733 / 2845	-38.5	-38.5	0.35 (2)	6.25
W-V	-1480 / 6416	-38.5	-38.5	0.67 (2)	6.25
V-U	-1375 / 6444	-38.5	-38.5	0.66 (2)	6.25
U-T	-1076 / 5589	-38.5	-38.5	0.75 (2)	6.25
T-S	-1076 / 5589	-38.5	-38.5	0.75 (2)	6.25
S-R	-1144 / 6206	-38.5	-38.5	0.80 (1)	6.25
R-Q	-915 / 5587	-38.5	-38.5	0.75 (3)	6.25
Q-P	-915 / 5587	-38.5	-38.5	0.75 (3)	6.25
P-O	-1159 / 6444	-38.5	-38.5	0.87 (3)	6.25
O-Y	-1245 / 6416	-38.5	-38.5	0.87 (3)	6.25
Y-M	-517 / 2844	-38.5	-38.5	0.35 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-V	0 / 588	0.13 (3)
V-E	-276 / 236	0.13 (10)
E-U	-1235 / 429	0.43 (2)
U-F	-238 / 1151	0.27 (13)
F-S	-412 / 1980	0.44 (3)
S-G	-1076 / 323	0.41 (2)
S-H	-540 / 490	0.30 (3)
R-H	-1498 / 410	0.58 (2)
R-I	-421 / 1986	0.45 (2)
P-I	-231 / 1137	0.26 (14)
P-J	-1237 / 430	0.43 (3)
J-O	-265 / 245	0.13 (9)
O-L	0 / 587	0.13 (2)
W-X	-379 / 2830	0.00 (1)
W-C	-4441 / 905	0.86 (2)
L-Y	-4443 / 905	0.86 (3)
Y-Z	-382 / 2828	0.00 (1)

DWG NO. TAM 1100354
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL	=	51.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.62/1.00 (J-L:3) , BC=0.87/1.00 (V-W:2) , WB=0.66/1.00 (L-Y:3) , SSI=0.38/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.97 (T) (INPUT = 1.00)

1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T51	1	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
Aug/20/18
H.J.G. ACWLS
PROVINCE OF ONTARIO

DWG NO. TAM18053S4
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T52	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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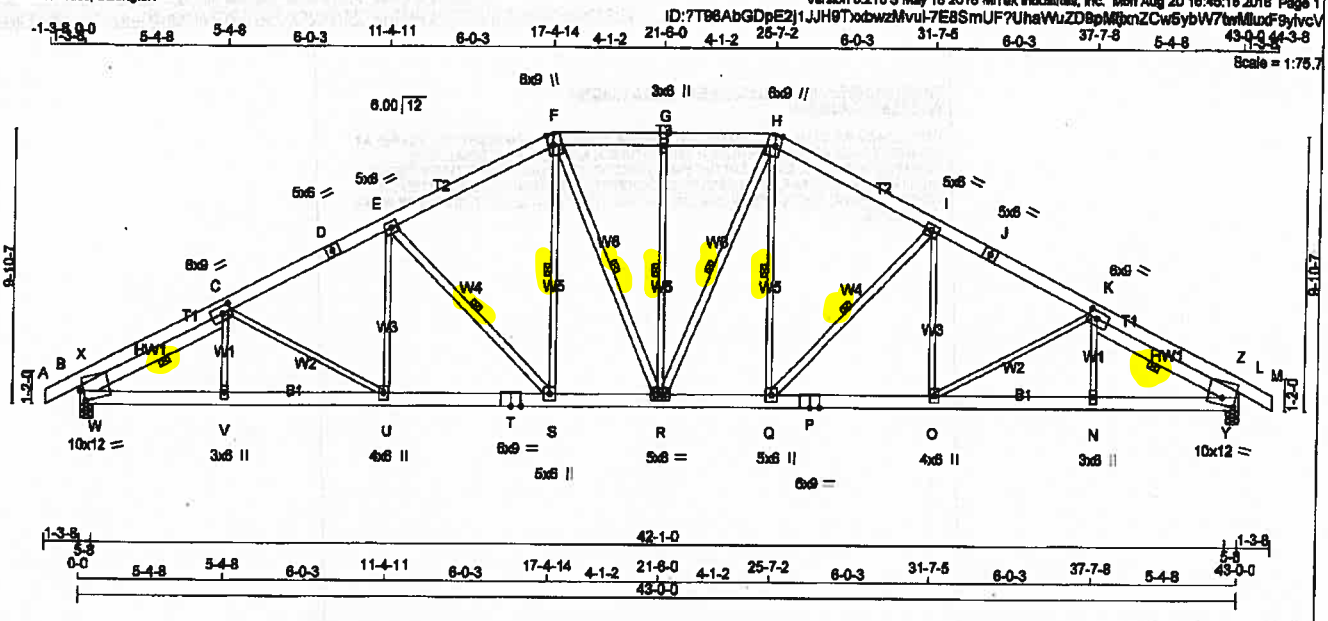

TOTAL WEIGHT = 11 X 277 = 3048 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x6	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Units is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m	MT20	10.0	12.0	4.50	1.50
C	TMWWW-t	MT20	6.0	9.0	3.00	4.00
O	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	6.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	6.0	9.0	3.00	4.00
L	TMBMW1-m	MT20	10.0	12.0	3.00	6.00
N	BMW+w	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	5.0	6.0		
T	BS-t	MT20	6.0	9.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4729	0	4812	283	-997	5-8	5-8 2TBE6
L	4729	0	4812	0	-997	5-8	5-8 2TBE6

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 997 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 997 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 283 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3399	2427 / 0	452 / 0	0 / 0	154 / -1082	576 / 0	0 / 0
L	3399	2427 / 0	452 / 0	0 / 0	94 / -1082	576 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	188 / -188	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W, K-Y.

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00	
B- X	-5381 / 1022	-170.4	-170.4	0.45 (2)	3.38	
X- C	-3207 / 798	-170.4	-170.4	0.38 (2)	4.38	
C- D	-6890 / 1475	-170.4	-170.4	0.83 (2)	2.89	
D- E	-6890 / 1475	-170.4	-170.4	0.83 (2)	2.89	
E- F	-5578 / 1316	-170.4	-170.4	0.72 (2)	3.13	
F- G	-5165 / 1273	-170.4	-170.4	0.30 (3)	3.64	
G- H	-5165 / 1273	-170.4	-170.4	0.30 (3)	3.64	
H- I	-5578 / 1316	-170.4	-170.4	0.72 (3)	3.13	
I- J	-6890 / 1475	-170.4	-170.4	0.83 (3)	2.89	
J- K	-6890 / 1475	-170.4	-170.4	0.83 (3)	2.89	
K- Z	-3207 / 795	-170.4	-170.4	0.38 (3)	4.38	
Z- L	-5381 / 1021	-170.4	-170.4	0.45 (3)	3.38	
L- M	0 / 2	-170.4	-170.4	0.13 (3)	10.00	
B- W	-819 / 2856	-38.5	-38.5	0.37 (1)	6.25	
W- V	-1446 / 6331	-38.5	-38.5	0.83 (1)	6.25	
V- U	-1447 / 6329	-38.5	-38.5	0.85 (1)	6.25	
U- T	-1238 / 6010	-38.5	-38.5	0.78 (1)	6.25	
T- S	-1238 / 6010	-38.5	-38.5	0.78 (1)	6.25	
S- R	-850 / 4957	-38.5	-38.5	0.64 (1)	6.25	
R- Q	-740 / 4957	-38.5	-38.5	0.64 (1)	6.25	
Q- P	-972 / 6010	-38.5	-38.5	0.78 (1)	6.25	
P- O	-972 / 6010	-38.5	-38.5	0.78 (1)	6.25	
O- N	-1166 / 6329	-38.5	-38.5	0.85 (1)	6.25	
N- Y	-1184 / 6331	-38.5	-38.5	0.83 (1)	6.25	
Y- L	-855 / 2856	-38.5	-38.5	0.37 (1)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 257	0.06 (17)
C- U	-561 / 269	0.50 (2)
U- E	-44 / 552	0.12 (2)
E- S	-1850 / 562	0.69 (2)
S- F	-338 / 1521	0.34 (2)
F- R	-277 / 1312	0.21 (3)
R- G	-977 / 282	0.57 (2)
R- H	-277 / 1312	0.21 (2)
Q- H	-338 / 1521	0.34 (3)
Q- I	-1850 / 562	0.69 (3)
O- I	-44 / 552	0.12 (3)
O- K	-561 / 253	0.50 (3)
N- K	0 / 257	0.06 (17)
W- X	-324 / 2646	0.00 (1)
W- C	-4269 / 760	0.54 (1)
K- Y	-4269 / 764	0.54 (1)
Y- Z	-325 / 2646	0.00 (1)

DWG NO. TAM 170555
STRUCTURAL COMPONENTS ONLY
1/2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	51.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	75.3	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.83/1.00 (I-K:3) , BC=0.85/1.00 (N-O:1) , WB=0.69/1.00 (I-Q:3) , SSI=0.40/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

...: GRIP= 0.85 (C) (INPUT = 0.90) - JSI METAL= 0.89 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T52	11	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/16
H.J.G. ALVES
PROVINCE OF ONTARIO

ENG NO. TAM 100 5355
STRUCTURAL
COMPONENTS ONLY
2/2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G52	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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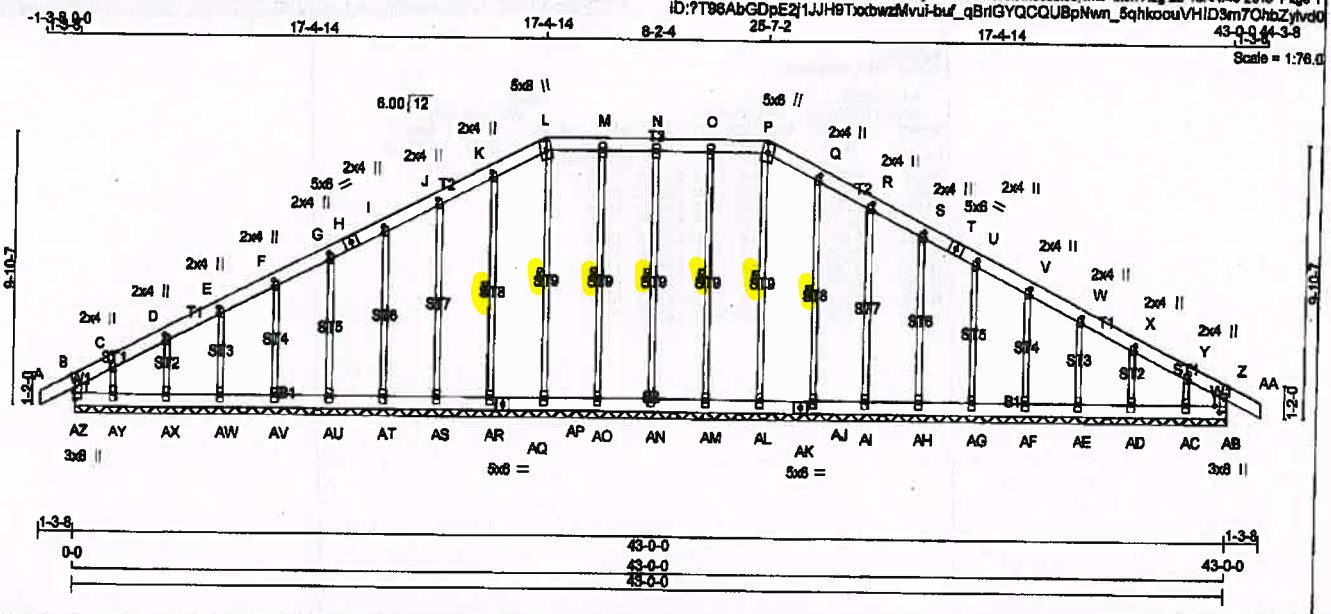

TOTAL WEIGHT = 2 X 284 = 568 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AZ- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
P - T	2x6	DRY	No.2	SPF
T - AA	2x6	DRY	No.2	SPF
AB- Z	2x6	DRY	No.2	SPF
AZ- AQ	2x6	DRY	No.2	SPF
AQ- AK	2x6	DRY	No.2	SPF
AK- AB	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, Z, AB, AZ						
B						
C, D, E, Q, R, S, U, V, W, X, Y						
C	TMW+w	MT20	2.0	4.0	2.75	
F, G, I, J, K						
F	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	6.0		
M, N, O						
M	TMW+w	MT20	3.0	6.0		
P	TTW+m	MT20	5.0	8.0		
T	TS-t	MT20	5.0	6.0		
AB	TMBMV1+p	MT20	3.0	8.0	2.75	2.25
AC, AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AR, AS, AT, AU, AV, AW, AX, AY						
AC	BMW1+w	MT20	3.0	6.0		
AK	BS-t	MT20	5.0	6.0		
AQ	BS-t	MT20	5.0	6.0		
AZ	TMBMV1+p	MT20	3.0	8.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-AL, O-AM, N-AN, M-AO, L-AP, K-AR, Q-AJ.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AZ- B	-368 / 0	0.0	0.0	0.04 (1)	7.61
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00
B- C	-96 / 0	-129.7	-129.7	0.08 (1)	6.25
C- D	-48 / 0	-129.7	-129.7	0.03 (1)	6.25
D- E	-43 / 0	-129.7	-129.7	0.03 (1)	6.25
E- F	-33 / 0	-129.7	-129.7	0.03 (1)	6.25
F- G	-27 / 0	-129.7	-129.7	0.03 (1)	6.25
G- H	-21 / 0	-129.7	-129.7	0.03 (1)	6.25
H- I	-21 / 0	-129.7	-129.7	0.03 (1)	6.25
I- J	-16 / 0	-129.7	-129.7	0.03 (1)	6.25
J- K	-12 / 0	-129.7	-129.7	0.03 (1)	6.25
K- L	-9 / 0	-129.7	-129.7	0.03 (1)	10.00
L- M	-5 / 0	-129.7	-129.7	0.03 (1)	10.00
M- N	-5 / 0	-129.7	-129.7	0.03 (1)	10.00
N- O	-5 / 0	-129.7	-129.7	0.03 (1)	10.00
O- P	-5 / 0	-129.7	-129.7	0.03 (1)	10.00
P- Q	-9 / 0	-129.7	-129.7	0.03 (1)	10.00
Q- R	-12 / 0	-129.7	-129.7	0.03 (1)	6.25
R- S	-16 / 0	-129.7	-129.7	0.03 (1)	6.25
S- T	-21 / 0	-129.7	-129.7	0.03 (1)	6.25
T- U	-21 / 0	-129.7	-129.7	0.03 (1)	6.25
U- V	-27 / 0	-129.7	-129.7	0.03 (1)	6.25
V- W	-33 / 0	-129.7	-129.7	0.03 (1)	6.25
W- X	-43 / 0	-129.7	-129.7	0.03 (1)	6.25
X- Y	-48 / 0	-129.7	-129.7	0.03 (1)	6.25
Y- Z	-96 / 0	-129.7	-129.7	0.08 (1)	6.25
Z-AA	0 / 40	-129.7	-129.7	0.09 (1)	10.00
AB- Z	-368 / 0	0.0	0.0	0.04 (1)	7.61
AZ-AY	0 / 59	-38.5	-38.5	0.04 (1)	10.00
AY-AX	0 / 49	-38.5	-38.5	0.02 (2)	10.00
AX-AW	0 / 37	-38.5	-38.5	0.02 (2)	10.00
AW-AV	0 / 30	-38.5	-38.5	0.01 (2)	10.00
AV-AU	0 / 24	-38.5	-38.5	0.01 (2)	10.00
AU-AT	0 / 19	-38.5	-38.5	0.01 (2)	10.00
AT-AS	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AS-AR	0 / 11	-38.5	-38.5	0.01 (2)	10.00
AR-AQ	0 / 8	-38.5	-38.5	0.01 (3)	10.00
AQ-AP	0 / 8	-38.5	-38.5	0.01 (3)	10.00
AP-AO	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AO-AN	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AN-AM	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AM-AL	0 / 5	-38.5	-38.5	0.01 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AL- P	-261 / 0	0.16 (1)
AM- O	-266 / 0	0.16 (1)
AN- N	-258 / 0	0.16 (1)
AO- M	-268 / 0	0.16 (1)
AP- L	-261 / 0	0.16 (1)
AR- K	-258 / 0	0.12 (1)
AS- J	-257 / 0	0.27 (1)
AT- I	-257 / 0	0.16 (1)
AU- G	-257 / 0	0.12 (1)
AV- F	-258 / 0	0.08 (1)
AW- E	-254 / 0	0.05 (1)
AX- D	-270 / 0	0.04 (1)
AY- C	-136 / 0	0.02 (1)
AJ- Q	-258 / 0	0.12 (1)
AI- R	-257 / 0	0.27 (1)
AH- S	-257 / 0	0.16 (1)
AG- U	-257 / 0	0.12 (1)
AF- V	-258 / 0	0.08 (1)
AE- W	-254 / 0	0.05 (1)
AD- X	-270 / 0	0.04 (1)
AC- Y	-136 / 0	0.02 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (Z-AA:1) , BC=0.04/1.00 (AB-AC:1) , WB=0.27/1.00 (R-AI:1) , SSI=0.10/1.00 (Z-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.13 (AB) (INPUT = 1.00)

DWG NO. TAM 11805328
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G52	2	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AL-AK	0 / 8	-38.5	-38.5	0.01	(3)	10.00
AK-AJ	0 / 8	-38.5	-38.5	0.01	(3)	10.00
AJ-AI	0 / 11	-38.5	-38.5	0.01	(2)	10.00
AI-AH	0 / 15	-38.5	-38.5	0.01	(2)	10.00
AH-AG	0 / 19	-38.5	-38.5	0.01	(2)	10.00
AG-AF	0 / 24	-38.5	-38.5	0.01	(2)	10.00
AF-AE	0 / 30	-38.5	-38.5	0.01	(2)	10.00
AE-AD	0 / 37	-38.5	-38.5	0.02	(2)	10.00
AD-AC	0 / 49	-38.5	-38.5	0.02	(2)	10.00
AC-AB	0 / 59	-38.5	-38.5	0.04	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG.NO. TAM #1805328
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T53	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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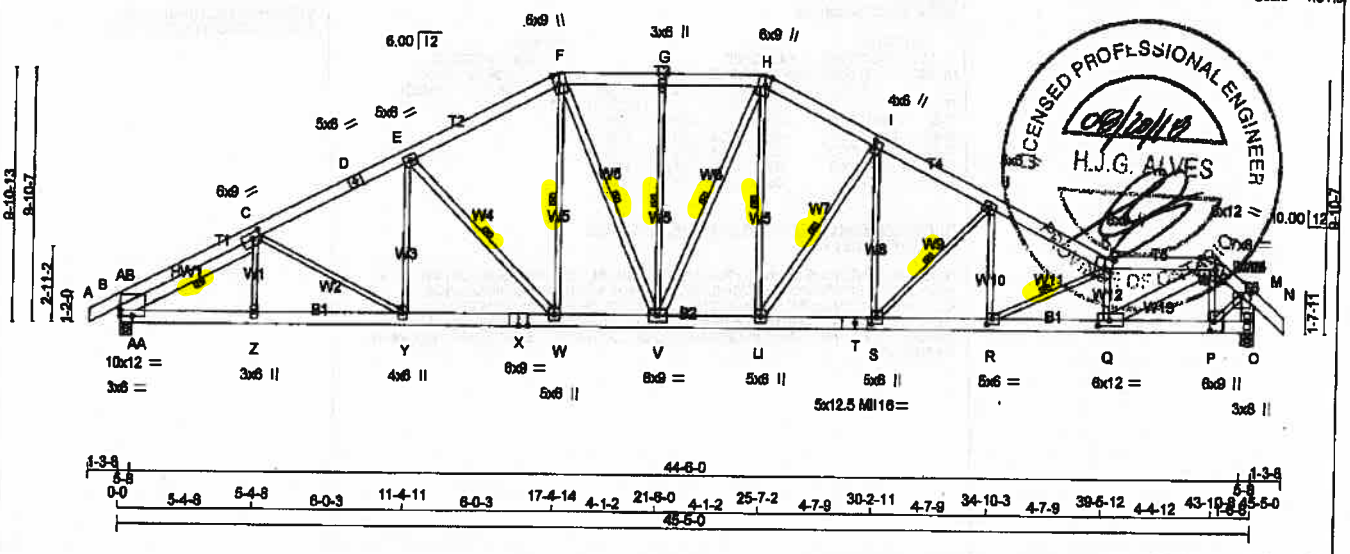

TOTAL WEIGHT = 12 X 295 = 3546 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
B - X	2x6	DRY	2100F 1.8E	SPF
X - T	2x6	DRY	2100F 1.8E	SPF
T - O	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - W	2x4	DRY	No.2	SPF
F - V	2x4	DRY	No.2	SPF
V - H	2x4	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
U - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	2.50	
B	TP-I	MT20	3.0	6.0	2.50	6.00
C	TMWWW-t	MT20	6.0	9.0	2.75	4.00
D	TS-I	MT20	6.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TMWW+t	MT20	4.0	6.0	3.00	1.00
J	TMWW-I	MT20	5.0	6.0		
K	TTWW+m	MT20	6.0	9.0	4.75	3.25
L	TTWW-m	MT20	6.0	12.0	Edge	8.50
M	TMWW-p	MT20	7.0	8.0	1.75	4.00
O	BMV1+p	MT20	3.0	8.0		
P	BMWW+t	MT20	6.0	9.0	4.50	1.75
Q	BMWW-t	MT20	9.0	12.0	2.75	5.25
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMWW+t	MT20	5.0	6.0	3.00	2.25
T	BS-t	MII16	5.0	12.5		
U	BMWW+t	MT20	5.0	6.0		
V	BMWWW-t	MT20	6.0	9.0		
W	BMWW+t	MT20	5.0	6.0		
X	BS-t	MT20	6.0	9.0		
Y	BMWW+t	MT20	4.0	6.0		
Z	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM 17805356 1/2
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4977	0	4977	298	-1036	5-8	5-8
O	4992	0	5153	0	-1086	5-8	5-8

2-TBE6
2-TBE6

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1036 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1086 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 298 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3578	2494 / 0	477 / 0	0 / 0	157 / -1130	607 / 0	0 / 0
O	3589	2611 / 0	477 / 0	0 / 0	183 / -1152	608 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	212 / -211	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, F-W, F-V, G-V, H-V, H-U, H-J, J-S, K-R, C-AA.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B-AB	-5808 / 1056	-170.4	-170.4	0.48 (2)	3.32
AB- C	-3807 / 815	-170.4	-170.4	0.39 (2)	4.31
C- D	-7184 / 1552	-170.4	-170.4	0.65 (2)	2.59
D- E	-7184 / 1552	-170.4	-170.4	0.65 (2)	2.59
E- F	-6092 / 1398	-170.4	-170.4	0.74 (2)	2.95
F- G	-5736 / 1392	-170.4	-170.4	0.32 (3)	3.44
G- H	-5736 / 1392	-170.4	-170.4	0.32 (3)	3.44
H- I	-6308 / 1498	-170.4	-170.4	0.45 (3)	3.20
I- J	-7608 / 1710	-170.4	-170.4	0.55 (3)	2.83
J- K	-9034 / 1940	-170.4	-170.4	0.70 (3)	2.41
K- L	-9810 / 2058	-170.4	-170.4	0.76 (3)	2.09
L- M	-4119 / 915	-170.4	-170.4	0.21 (3)	4.10
M- N	0 / 93	-170.4	-170.4	0.15 (3)	10.00
O- M	-5273 / 1086	0.0	0.0	0.35 (3)	4.47
B-AA	-808 / 2843	-38.5	-38.5	0.16 (1)	6.25
AA- Z	-1478 / 6726	-38.5	-38.5	0.37 (1)	6.25
Z- Y	-1479 / 6723	-38.5	-38.5	0.36 (1)	6.25
Y- X	-1274 / 6451	-38.5	-38.5	0.35 (1)	6.25
X- W	-1274 / 6451	-38.5	-38.5	0.35 (1)	6.25
W- V	-892 / 5417	-38.5	-38.5	0.30 (1)	6.25
V- U	-817 / 5630	-38.5	-38.5	0.31 (1)	6.25
U- T	-1067 / 6609	-38.5	-38.5	0.36 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	0 / 298	0.06 (17)
C- Y	-552 / 310	0.48 (2)
Y- E	-42 / 539	0.12 (2)
E- W	-1835 / 558	0.68 (2)
W- F	-332 / 1502	0.34 (2)
F- V	-334 / 1592	0.28 (3)
V- G	-972 / 291	0.57 (2)
V- H	-376 / 1168	0.20 (3)
U- H	-501 / 2195	0.49 (3)
Q- K	-3835 / 881	0.82 (1)
Q- L	-1556 / 7628	0.90 (1)
P- L	-1650 / 336	0.27 (3)
P- M	-638 / 3695	0.83 (3)
U- I	-2456 / 674	0.83 (3)
S- I	-336 / 1578	0.36 (3)
S- J	-2098 / 568	0.61 (3)
R- J	-187 / 1212	0.27 (3)
R- K	-2019 / 486	0.44 (3)
AA-AB	-345 / 2821	0.00 (1)
AA- C	-4586 / 815	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	51.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	75.3	PSF

SPACING = 24.0 IN. O/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.49")

CSI: TC=0.85/1.00 (C-E:2) , BC=0.81/1.00 (Q-R:3) , WB=0.90/0.90 (L-Q:1) , SSI=0.40/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	768	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T53	12	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (18)

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.96 (X) (INPUT = 1.00)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- S	-1087 / 6809	-38.5	-38.5	0.38 (1)	6.25
S- R	-1455 / 8112	-38.5	-38.5	0.49 (1)	6.25
R- Q	-1884 / 9772	-38.5	-38.5	0.61 (3)	6.25
Q- P	-491 / 2992	-38.5	-38.5	0.29 (3)	6.25
P- O	-11 / 23	-38.5	-38.5	0.09 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}, INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805356
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T54	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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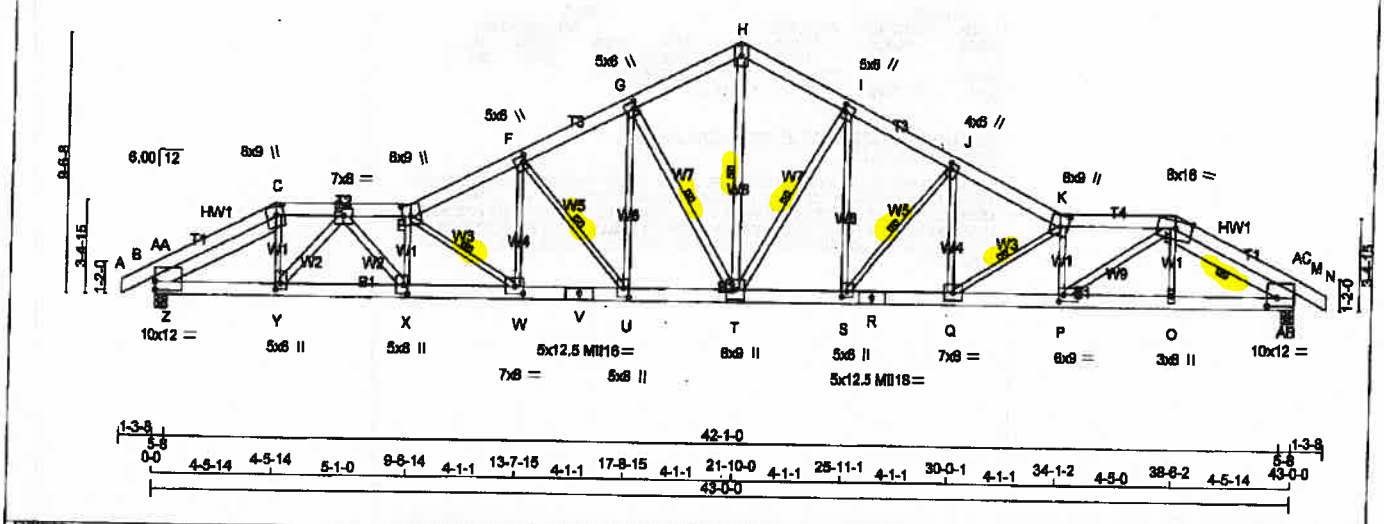

TOTAL WEIGHT = 3 X 270 = 811 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
B - V	2x6	DRY	2100F 1.8E	SPF
V - R	2x6	DRY	2100F 1.8E	SPF
R - M	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
T - I	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
D - X	2x4	DRY	No.2	SPF
Y - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.75	
C	TTWW+m	MT20	6.0	9.0	3.75	3.50
D	TMWW-t	MT20	7.0	8.0		
E	TTWW+m	MT20	8.0	9.0	5.25	3.50
F	TMWW+t	MT20	5.0	6.0	3.00	1.25
G	TMWW+t	MT20	5.0	6.0	3.00	1.25
H	TTW+p	MT20	6.0	9.0		
I	TMWW+t	MT20	5.0	6.0	3.00	1.50
J	TMWW+t	MT20	4.0	6.0	2.50	1.00
K	TTWW+m	MT20	8.0	8.0	5.00	3.50
L	TTWWW-m	MT20	6.0	16.0	Edge	7.50
M	TMBMW1-I	MT20	10.0	12.0	3.75	4.50
O	BMW+w	MT20	3.0	6.0		
P	BMWW-t	MT20	6.0	9.0	3.00	2.25
Q	BMWW-t	MT20	7.0	8.0		
R	BS-t	MII16	5.0	12.5		
S	BMWW+t	MT20	5.0	8.0	2.50	2.25
T	BMWWW+t	MT20	6.0	9.0		
U	BMWW+t	MT20	5.0	8.0	4.00	2.25
V	BS-t	MII16	5.0	12.5		
W	BMWW-t	MT20	7.0	8.0	3.25	3.75
X	BMWW+t	MT20	5.0	6.0	4.00	1.75
Y	BMWW+t	MT20	5.0	6.0	2.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4729	0	4729	254	-952	5-8	5-8
M	4729	0	4729	0	-943	5-8	5-8

2-TBE6
2-TBE6

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 952 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 943 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 254 LBS FACTORED HORIZONTAL REACTION AT JOINT B OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3399	2372 / 0	452 / 0	0 / 0	130 / -1050	576 / 0	0 / 0
M	3399	2372 / 0	452 / 0	0 / 0	126 / -1043	576 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	161 / -161	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 1.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, F-U, G-T, H-T, I-T, J-S, K-Q, L-AB.

LOADING

TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00
B-AA	-5220 / 955	-170.4	-170.4	0.38 (2)	3.52
AA- C	-3080 / 750	-170.4	-170.4	0.31 (2)	4.52
C- D	-6268 / 1306	-170.4	-170.4	0.26 (1)	3.34
D- E	-11178 / 2183	-170.4	-170.4	0.61 (1)	2.09
E- F	-9407 / 1815	-170.4	-170.4	0.63 (1)	2.40
F- G	-7479 / 1488	-170.4	-170.4	0.42 (1)	2.98
G- H	-6012 / 1258	-170.4	-170.4	0.36 (1)	3.33
H- I	-6012 / 1268	-170.4	-170.4	0.36 (1)	3.33
I- J	-7388 / 1460	-170.4	-170.4	0.41 (1)	2.98
J- K	-9159 / 1755	-170.4	-170.4	0.60 (1)	2.47
K- L	-10553 / 2050	-170.4	-170.4	0.79 (1)	1.93
L-AC	-2997 / 728	-170.4	-170.4	0.32 (3)	4.57
AC- M	-5008 / 996	-170.4	-170.4	0.36 (3)	3.58
M- N	0 / 2	-170.4	-170.4	0.13 (3)	10.00
B- Z	-770 / 2714	-38.5	-38.5	0.13 (1)	6.25
Z- Y	-1326 / 6149	-38.5	-38.5	0.32 (1)	6.25
Y- X	-1847 / 6859	-38.5	-38.5	0.53 (1)	6.25
X- W	-2241 / 11275	-38.5	-38.5	0.84 (1)	6.25
W- V	-1602 / 8442	-38.5	-38.5	0.48 (1)	6.25
V- U	-1602 / 8442	-38.5	-38.5	0.48 (1)	6.25
U- T	-1149 / 6892	-38.5	-38.5	0.41 (1)	6.25
T- S	-982 / 6809	-38.5	-38.5	0.41 (1)	6.25
S- R	-1294 / 8220	-38.5	-38.5	0.45 (1)	6.25
R- Q	-1294 / 8220	-38.5	-38.5	0.45 (1)	6.25
Q- P	-1857 / 10660	-38.5	-38.5	0.65 (1)	6.25
P- O	-1070 / 6203	-38.5	-38.5	0.43 (1)	6.25
O-AB	-1070 / 6191	-38.5	-38.5	0.35 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-588 / 3378	0.76 (1)
X- E	-2743 / 837	0.50 (1)
E- W	-3489 / 787	0.72 (1)
W- F	-420 / 2240	0.50 (1)
F- U	-2846 / 713	0.79 (2)
U- G	-482 / 2294	0.52 (2)
G- T	-2875 / 753	0.87 (2)
T- H	-961 / 4759	0.76 (1)
T- I	-2717 / 721	0.83 (3)
S- I	-446 / 2120	0.48 (3)
S- J	-2931 / 658	0.73 (3)
Q- J	-370 / 2000	0.45 (1)
Q- K	-3005 / 683	0.62 (1)
P- K	-3010 / 816	0.54 (1)
P- L	-626 / 5285	0.85 (1)
O- L	0 / 351	0.09 (4)
D- X	-588 / 3843	0.59 (1)
Y- D	-4070 / 787	0.71 (1)
C- Z	-4257 / 690	0.90 (1)
L-AB	-4414 / 714	0.44 (1)
Z-AA	-288 / 2632	0.00 (1)
AB-AC	-246 / 2476	0.00 (1)

LICENCE NO. TAM 1605357
STRUCTURAL
COMPONENTS ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	51.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	75.3	PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.56")
ALLOWABLE DEFL.(TL)= L/160 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 737 (0.70")

CSI: TC=0.79/1.00 (K-L:1) , BC=0.65/1.00 (P-Q:1) , WB=0.90/0.90 (C-Z:1) , SSI=0.27/1.00 (L-AC:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	758	1957	1656
MII16	436	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

.ISI GRIP= 0.90 (Y) (INPUT = 0.90)
METAL= 0.95 (Y) (INPUT = 1.00)

1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T54	3	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- M	-493 / 2629	-38.5	-38.5	0.15 (1)	8.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(9)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. REVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805357
STRUCTURAL
COMPONENTS ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G54A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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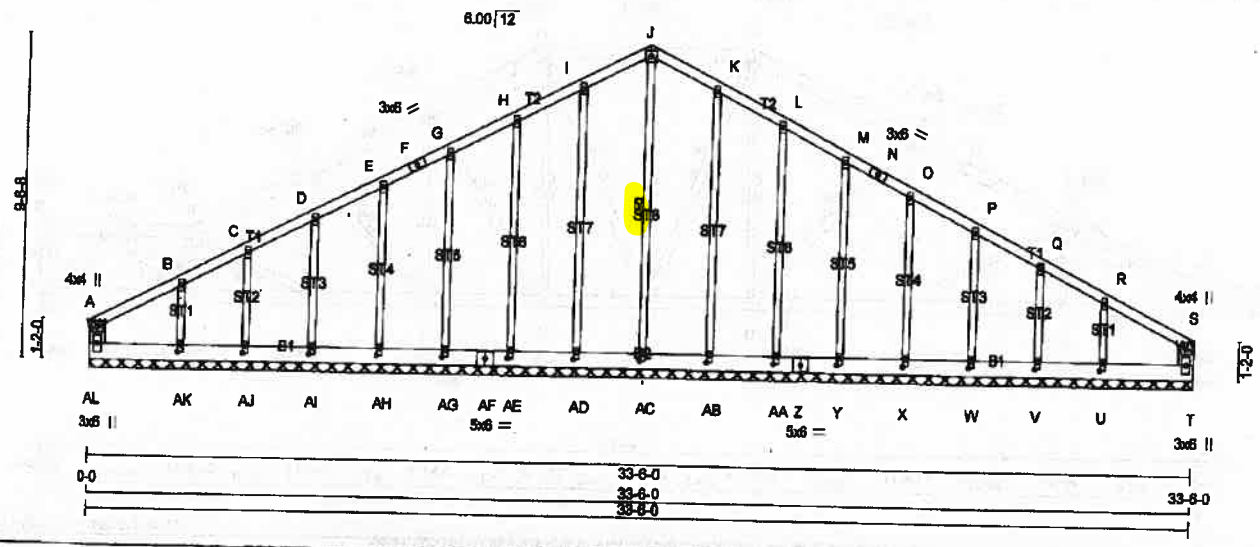

TOTAL WEIGHT = 167 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AL- A	2x6	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
AL- AF	2x6	DRY	No.2	SPF
AF- Z	2x6	DRY	No.2	SPF
Z - T	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	4.0		
B, C, O, E, G, H, I, K, L, M, O, P, Q, R						
B	TMW+w	MT20	2.0	4.0		
F	T8-t	MT20	3.0	6.0		
J	TTW-p	MT20	4.0	4.0		
N	TS-t	MT20	3.0	6.0		
S	TMV+p	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	6.0		
U, V, W, X, Y, AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK						
U	BMW1+w	MT20	2.0	4.0	2.50	1.00
Z	BS-t	MT20	5.0	6.0		
AF	BS-t	MT20	5.0	6.0		
AL	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-AC.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AL- A	-220 / 0	0.0	0.0	0.05 (1)	7.81	
A- B	-97 / 0	-129.7	-129.7	0.10 (1)	6.25	
B- C	-91 / 0	-129.7	-129.7	0.09 (1)	6.25	
C- D	-78 / 0	-129.7	-129.7	0.08 (1)	6.25	
D- E	-75 / 0	-129.7	-129.7	0.08 (1)	6.25	
E- F	-71 / 0	-129.7	-129.7	0.08 (1)	6.25	
F- G	-71 / 0	-129.7	-129.7	0.08 (1)	6.25	
G- H	-68 / 0	-129.7	-129.7	0.08 (1)	6.25	
H- I	-68 / 0	-129.7	-129.7	0.08 (1)	6.25	
I- J	-82 / 0	-129.7	-129.7	0.07 (1)	6.25	
J- K	-82 / 0	-129.7	-129.7	0.07 (1)	6.25	
K- L	-68 / 0	-129.7	-129.7	0.08 (1)	6.25	
L- M	-68 / 0	-129.7	-129.7	0.08 (1)	6.25	
M- N	-71 / 0	-129.7	-129.7	0.08 (1)	6.25	
N- O	-71 / 0	-129.7	-129.7	0.08 (1)	6.25	
O- P	-78 / 0	-129.7	-129.7	0.08 (1)	6.25	
P- Q	-78 / 0	-129.7	-129.7	0.08 (1)	6.25	
Q- R	-91 / 0	-129.7	-129.7	0.09 (1)	6.25	
R- S	-97 / 0	-129.7	-129.7	0.10 (1)	6.25	
T- S	-220 / 0	0.0	0.0	0.05 (1)	7.81	
AL-AK	0 / 88	-38.5	-38.5	0.03 (2)	10.00	
AK-AJ	0 / 76	-38.5	-38.5	0.03 (2)	10.00	
AJ-AI	0 / 71	-38.5	-38.5	0.02 (2)	10.00	
AI-AH	0 / 67	-38.5	-38.5	0.02 (2)	10.00	
AH-AG	0 / 64	-38.5	-38.5	0.02 (2)	10.00	
AG-AF	0 / 61	-38.5	-38.5	0.02 (2)	10.00	
AF-AE	0 / 61	-38.5	-38.5	0.02 (2)	10.00	
AE-AD	0 / 59	-38.5	-38.5	0.02 (2)	10.00	
AD-AC	0 / 57	-38.5	-38.5	0.02 (2)	10.00	
AC-AB	0 / 57	-38.5	-38.5	0.02 (2)	10.00	
AB-AA	0 / 59	-38.5	-38.5	0.02 (2)	10.00	
AA- Z	0 / 61	-38.5	-38.5	0.02 (2)	10.00	
Z- Y	0 / 61	-38.5	-38.5	0.02 (2)	10.00	
Y- X	0 / 64	-38.5	-38.5	0.02 (2)	10.00	
X- W	0 / 67	-38.5	-38.5	0.02 (2)	10.00	
W- V	0 / 71	-38.5	-38.5	0.02 (2)	10.00	
V- U	0 / 76	-38.5	-38.5	0.03 (2)	10.00	
U- T	0 / 88	-38.5	-38.5	0.03 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AC- J	-217 / 0	0.12 (1)
AD- I	-259 / 0	0.36 (1)
AE- H	-259 / 0	0.25 (1)
AG- G	-256 / 0	0.17 (1)
AH- E	-257 / 0	0.11 (1)
AI- D	-261 / 0	0.07 (1)
AJ- C	-241 / 0	0.05 (1)
AK- B	-310 / 0	0.05 (1)
AB- K	-259 / 0	0.36 (1)
AA- L	-259 / 0	0.25 (1)
Y- M	-256 / 0	0.17 (1)
X- O	-257 / 0	0.11 (1)
W- P	-261 / 0	0.07 (1)
V- Q	-241 / 0	0.05 (1)
U- R	-310 / 0	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (R-S:1) , BC=0.03/1.00 (T-U:2) , WB=0.36/1.00 (K-AB:1) , SSI=0.12/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (AC) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM #1605329
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T55	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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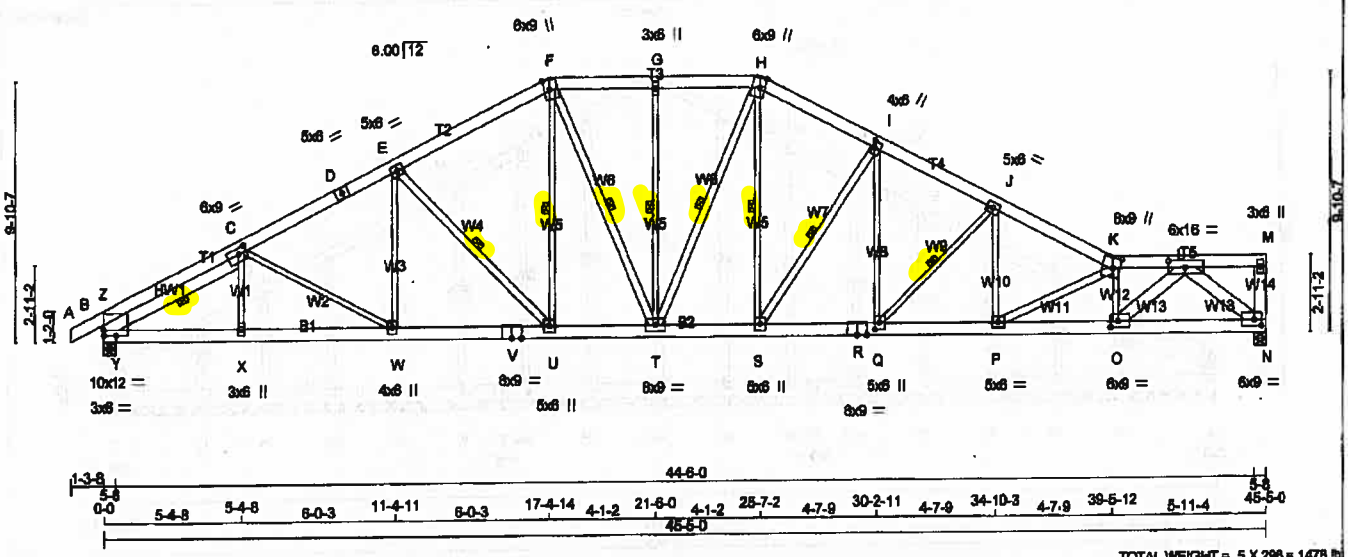

TOTAL WEIGHT = 5 X 296 = 1478 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
B - V	2x6	DRY	2100F 1.8E	SPF
V - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - U	2x4	DRY	No.2	SPF
F - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	2.50	
B	TP-I	MT20	3.0	6.0	2.50	6.00
C	TMWWW-1	MT20	6.0	9.0	2.75	4.00
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TMWW+t	MT20	4.0	6.0	3.00	1.00
J	TMWW-t	MT20	5.0	6.0		
K	TTWW+m	MT20	6.0	9.0	4.75	3.25
L	TMWW-t	MT20	6.0	16.0	3.00	7.75
M	TMV+p	MT20	3.0	6.0		
N	BMVW1-t	MT20	6.0	9.0	2.75	3.25
O	BMWW-t	MT20	6.0	9.0	2.75	3.50
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	3.00	2.25
R	BS-t	MT20	6.0	9.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMWWW-t	MT20	6.0	9.0		
U	BMWW+t	MT20	5.0	6.0		
V	BS-t	MT20	6.0	9.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4743	0	4856	0	-1003	5-8	5-8 2-TBE6
B	4861	0	4861	300	-1036	5-8	5-8 2-TBE6

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1003 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1036 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 300 LBS FACTORED HORIZONTAL REACTION AT JOINT B ok

UNFACTORED REACTIONS

JT	1ST LOADE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3420	2428 / 0	477 / 0	0 / 0	122 / -1096	590 / 0	0 / 0
B	3581	2497 / 0	477 / 0	0 / 0	157 / -1130	607 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	214 / -133	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, B

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-U, F-T, G-T, H-T, H-S, I-S, J-Q, C-Y.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 2	-170.4	-170.4	0.13 (2)	10.00	X- C	0 / 256	0.06 (17)
B- Z	-5613 / 1056	-170.4	-170.4	0.46 (2)	3.32	C- W	-552 / 311	0.48 (2)
Z- C	-3310 / 615	-170.4	-170.4	0.39 (2)	4.31	W- E	-42 / 539	0.12 (2)
C- D	-7192 / 1552	-170.4	-170.4	0.85 (2)	2.59	E- U	-1835 / 558	0.68 (2)
D- E	-7192 / 1552	-170.4	-170.4	0.85 (2)	2.59	U- F	-332 / 1502	0.34 (2)
E- F	-6100 / 1398	-170.4	-170.4	0.74 (2)	2.85	F- T	-332 / 1596	0.26 (3)
F- G	-5745 / 1389	-170.4	-170.4	0.32 (3)	3.44	T- G	-972 / 291	0.57 (2)
G- H	-5745 / 1389	-170.4	-170.4	0.32 (3)	3.44	T- H	-381 / 1189	0.20 (3)
H- I	-6320 / 1493	-170.4	-170.4	0.45 (3)	3.20	S- H	-497 / 2204	0.50 (3)
I- J	-7629 / 1704	-170.4	-170.4	0.55 (3)	2.83	O- K	-3676 / 828	0.59 (1)
J- K	-8048 / 1927	-170.4	-170.4	0.71 (3)	2.40	S- I	-2475 / 671	0.84 (3)
K- L	-8789 / 2048	-170.4	-170.4	0.81 (3)	2.41	Q- I	-332 / 1686	0.38 (3)
L- M	-20 / 52	-170.4	-170.4	0.11 (3)	6.25	Q- J	-2090 / 547	0.81 (3)
N- M	-261 / 78	0.0	0.0	0.03 (14)	7.61	P- J	-178 / 1207	0.27 (3)
						P- K	-2193 / 485	0.72 (3)
B- Y	-643 / 2948	-38.5	-38.5	0.16 (1)	6.25	O- L	-1095 / 5730	0.68 (1)
Y- X	-1515 / 6732	-38.5	-38.5	0.37 (1)	6.25	L- N	-7144 / 1514	0.77 (3)
X- W	-1516 / 6730	-38.5	-38.5	0.38 (1)	6.25	Y- Z	-345 / 2623	0.00 (1)
W- V	-1311 / 6458	-38.5	-38.5	0.35 (1)	6.25	Y- C	-4590 / 614	0.58 (1)
V- U	-1311 / 6458	-38.5	-38.5	0.35 (1)	6.25			
U- T	-829 / 5424	-38.5	-38.5	0.30 (1)	6.25			
T- B	-854 / 5840	-38.5	-38.5	0.31 (1)	6.25			
S- R	-1170 / 6826	-38.5	-38.5	0.38 (1)	6.25			
R- Q	-1170 / 6826	-38.5	-38.5	0.38 (1)	6.25			
Q- P	-1582 / 6124	-38.5	-38.5	0.44 (1)	6.25			
P- O	-1960 / 9944	-38.5	-38.5	0.58 (3)	6.25			

DWG NO. TAM 110053 58
STRUCTURAL
CONFORMS ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	51.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. O/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.38")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.47")

CSI: TC=0.85/1.00 (C-E:2) , BC=0.56/1.00 (O-P:3) , WB=0.84/0.90 (I-S:3) , SSI=0.40/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (N) (INPUT = 0.90)
JSI METAL= 0.89 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T55	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:?T96AbGDpE2j1JJH8TjqdwzMvui-UBylpBJ8JDCp_K56cvTudqG2F6kLBLkfV1bwMylvcO

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO
O- N	-1132 / 5584	-38.5	-38.5	0.36 (3)	6.25

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS. CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805358
STRUCTURAL
GOVERNMENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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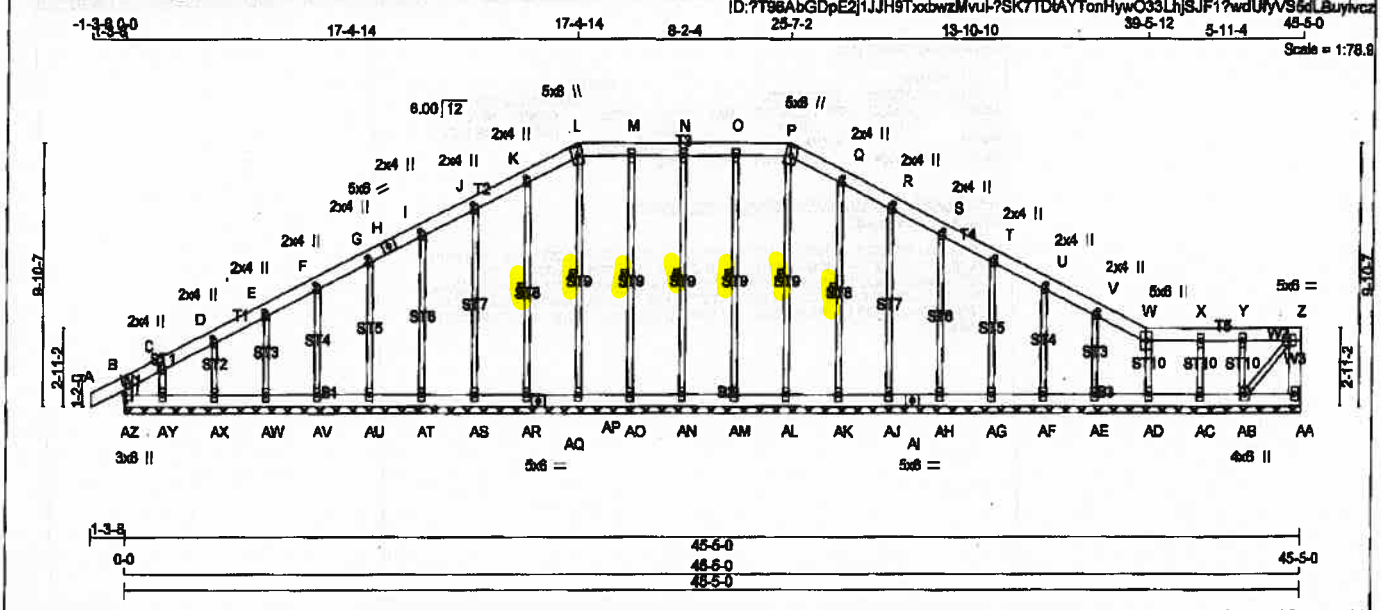

TOTAL WEIGHT = 277 lb (N)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AZ- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
P - W	2x6	DRY	No.2	SPF
W - Z	2x6	DRY	No.2	SPF
AA- Z	2x6	DRY	No.2	SPF
AZ- AQ	2x6	DRY	No.2	SPF
AQ- AI	2x6	DRY	No.2	SPF
AI - AA	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C, D, E, R, V						
C	TMW+w	MT20	2.0	4.0	2.75	
F, G, I, J, K, Q, S, T, U						
F	TMW+w	MT20	2.0	4.0	3.00	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	8.0		
M, N, O, X, Y						
M	TMW+w	MT20	3.0	6.0		
P	TTW+m	MT20	5.0	8.0		
W	TTW+p	MT20	5.0	6.0		
Z	TMWW-t	MT20	5.0	6.0		
AA	BMV1+p	MT20	3.0	6.0		
AB	BMWW1+t	MT20	4.0	6.0		
AC, AD, AE, AF, AG, AH, AJ, AK, AL, AM, AN, AO, AP, AR, AS, AT, AU, AV, AW, AX, AY						
AC	BMW1+w	MT20	3.0	6.0		
AI	BS-t	MT20	5.0	6.0		
AQ	BS-t	MT20	5.0	6.0		
AZ						
AZ	TMBMV1+p	MT20	3.0	6.0	2.75	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-AL, O-AM, N-AN, M-AO, L-AP, K-AR, Q-AK.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AZ- B	-354 / 0	0.0	0.0	0.02 (1)	7.61	
A- B	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
B- C	-37 / 0	-129.7	-129.7	0.08 (1)	6.25	
C- D	0 / 18	-129.7	-129.7	0.03 (1)	10.00	
D- E	0 / 22	-129.7	-129.7	0.04 (1)	10.00	
E- F	0 / 31	-129.7	-129.7	0.05 (1)	10.00	
F- G	0 / 38	-129.7	-129.7	0.03 (1)	10.00	
G- H	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
H- I	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
I- J	0 / 48	-129.7	-129.7	0.04 (1)	10.00	
J- K	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
K- L	0 / 45	-129.7	-129.7	0.04 (1)	10.00	
L- M	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
M- N	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
N- O	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
O- P	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
P- Q	0 / 45	-129.7	-129.7	0.04 (1)	10.00	
Q- R	0 / 53	-129.7	-129.7	0.04 (1)	10.00	
R- S	0 / 48	-129.7	-129.7	0.04 (1)	10.00	
S- T	0 / 43	-129.7	-129.7	0.03 (1)	10.00	
T- U	0 / 38	-129.7	-129.7	0.03 (1)	10.00	
U- V	0 / 31	-129.7	-129.7	0.03 (1)	10.00	
V- W	0 / 33	-129.7	-129.7	0.04 (1)	10.00	
W- X	0 / 28	-129.7	-129.7	0.04 (1)	10.00	
X- Y	0 / 28	-129.7	-129.7	0.03 (1)	10.00	
Y- Z	0 / 28	-129.7	-129.7	0.04 (1)	10.00	
AA- Z	-92 / 0	0.0	0.0	0.01 (1)	7.61	
AZ-AY	0 / 0	-38.5	-38.5	0.01 (3)	10.00	
AY-AX	-9 / 0	-38.5	-38.5	0.01 (3)	10.00	
AX-AW	-20 / 0	-38.5	-38.5	0.01 (3)	6.25	
AW-AV	-28 / 0	-38.5	-38.5	0.01 (3)	9.25	
AV-AU	-34 / 0	-38.5	-38.5	0.01 (3)	6.25	
AU-AT	-39 / 0	-38.5	-38.5	0.01 (3)	6.25	
AT-AS	-43 / 0	-38.5	-38.5	0.01 (3)	6.25	
AS-AR	-47 / 0	-38.5	-38.5	0.01 (3)	6.25	
AR-AQ	-50 / 0	-38.5	-38.5	0.01 (3)	6.25	
AQ-AP	-50 / 0	-38.5	-38.5	0.01 (3)	6.25	
AP-AO	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AO-AN	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AN-AM	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AM-AL	-53 / 0	-38.5	-38.5	0.01 (3)	6.25	
AL-AK	-50 / 0	-38.5	-38.5	0.01 (3)	6.25	
AK-AJ	-47 / 0	-38.5	-38.5	0.01 (3)	6.25	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
AL- P	-245 / 0	0.15 (1)
AM- O	-257 / 0	0.17 (1)
AN- N	-259 / 0	0.16 (1)
AO- M	-269 / 0	0.17 (1)
AP- L	-245 / 0	0.15 (1)
AR- K	-282 / 0	0.13 (1)
AS- J	-257 / 0	0.27 (1)
AT- I	-258 / 0	0.16 (1)
AU- G	-257 / 0	0.12 (1)
AV- F	-257 / 0	0.08 (1)
AW- E	-253 / 0	0.05 (1)
AX- D	-276 / 0	0.04 (1)
AY- C	-116 / 0	0.02 (1)
AK- Q	-262 / 0	0.13 (1)
AJ- R	-257 / 0	0.27 (1)
AH- S	-258 / 0	0.16 (1)
AG- T	-257 / 0	0.12 (1)
AF- U	-255 / 0	0.08 (1)
AE- V	-274 / 0	0.06 (1)
AD- W	-202 / 0	0.03 (1)
AC- X	-263 / 0	0.04 (1)
AB- Y	-269 / 0	0.04 (1)
AB- Z	-42 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (AB-AC:3) , WB=0.27/1.00 (J-AS:1) , SSI=0.10/1.00 (Y-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	786	1867	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.40 (C) (INPUT = 0.90)
DSI METAL= 0.12 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805330
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 18:44:48 2018 Page 2
ID:?T98AbGDpE2i1JJH9TxobwzMvuI-?SK7TDtAYTonHywO33LhISJF1?wdUfyVS5dLBuyfvcz

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AJ-AI	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AI-AH	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AH-AG	-39 / 0	-38.5	-38.5	0.01 (3)	6.25
AG-AF	-34 / 0	-38.5	-38.5	0.01 (3)	6.25
AF-AE	-28 / 0	-38.5	-38.5	0.01 (3)	6.25
AE-AD	-20 / 0	-38.5	-38.5	0.01 (3)	6.25
AD-AC	-28 / 0	-38.5	-38.5	0.01 (3)	6.25
AC-AB	-28 / 0	-38.5	-38.5	0.02 (3)	6.25
AB-AA	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MYERS
PROVINCE OF ONTARIO

DWG NO. TAM 17805330
STRUCTURAL
COMPONENT ONLY 8/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T81	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2016 MiTek Industries, Inc. Mon Aug 20 16:45:24 2018 Page 1

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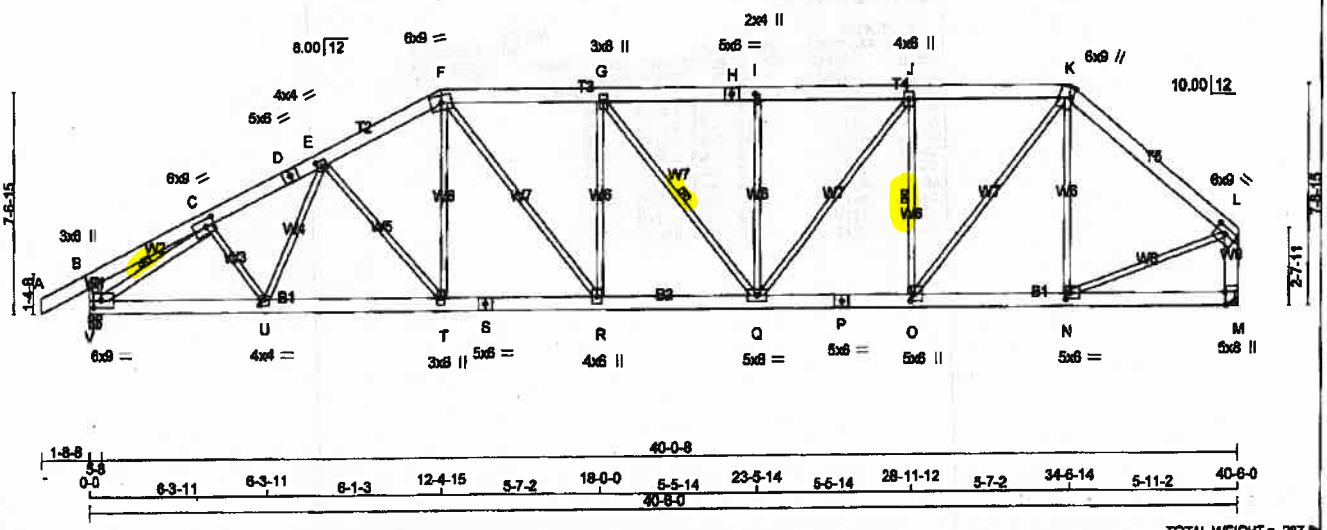

TOTAL WEIGHT = 237 lb.

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	6.0		
C	TMWW-t	MT20	6.0	9.0	3.00	3.75
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TTWW-m	MT20	6.0	9.0		
G	TMWW+t	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TMWW+t	MT20	4.0	6.0		
K	TTWW+m	MT20	6.0	9.0	4.00	1.25
L	TMWW-t	MT20	6.0	9.0	2.50	4.25
M	BMV1+t	MT20	5.0	6.0	Edge	0.50
N	BMWW-t	MT20	5.0	6.0	2.50	1.75
O	BMWW+t	MT20	5.0	6.0	2.75	1.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	4.0	2.50	1.75
V	BMW1-t	MT20	6.0	9.0	3.00	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3640	0	3640	0	0	5-8	5-8
M	3405	0	3405	0	0	MECHANICAL	

-HGUS26

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2680	1686 / 0	425 / 0	0 / 0	0 / 0	548 / 0	0 / 0
M	2600	1548 / 0	425 / 0	0 / 0	0 / 0	526 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q, J-O, C-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00
B- C	-4 / 6	-129.7	-129.7	0.17 (1)	10.00
C- D	-4966 / 0	-129.7	-129.7	0.25 (1)	3.76
D- E	-4965 / 0	-129.7	-129.7	0.25 (1)	3.76
E- F	-4846 / 0	-129.7	-129.7	0.25 (1)	3.87
F- G	-4844 / 0	-129.7	-129.7	0.41 (1)	3.86
G- H	-4752 / 0	-129.7	-129.7	0.40 (1)	3.69
H- I	-4752 / 0	-129.7	-129.7	0.40 (1)	3.69
I- J	-4752 / 0	-129.7	-129.7	0.40 (1)	3.69
J- K	-4002 / 0	-129.7	-129.7	0.36 (1)	4.02
K- L	-3216 / 0	-129.7	-129.7	0.48 (1)	4.24
V- B	-490 / 0	0.0	0.0	0.03 (1)	7.81
M- L	-3314 / 0	0.0	0.0	0.27 (1)	5.84
V- U	0 / 4243	-38.5	-38.5	0.61 (1)	10.00
U- T	0 / 4480	-38.5	-38.5	0.61 (1)	10.00
T- S	0 / 4143	-38.5	-38.5	0.57 (1)	10.00
S- R	0 / 4143	-38.5	-38.5	0.57 (1)	10.00
R- Q	0 / 4844	-38.5	-38.5	0.64 (1)	10.00
Q- P	0 / 4002	-38.5	-38.5	0.54 (1)	10.00
P- O	0 / 4002	-38.5	-38.5	0.54 (1)	10.00
O- N	0 / 2454	-38.5	-38.5	0.39 (1)	10.00
N- M	0 / 0	-38.5	-38.5	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 370	0.08 (2)
U- E	-71 / 122	0.03 (1)
E- T	-530 / 0	0.40 (1)
T- F	0 / 652	0.15 (1)
F- R	0 / 1134	0.28 (1)
R- G	-684 / 0	0.63 (1)
G- Q	-150 / 0	0.08 (1)
Q- I	-657 / 0	0.61 (1)
Q- J	0 / 1228	0.28 (1)
O- J	-1760 / 0	0.58 (1)
O- K	0 / 2488	0.68 (1)
N- K	-599 / 0	0.65 (1)
V- C	-3216 / 0	0.74 (1)
N- L	-97 / 3216	0.69 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.48/1.00 (K-L:1) , BC=0.64/1.00 (Q-R:1) , WB=0.74/1.00 (C-V:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	516	354	1667	788	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.67 (C) (INPUT = 1.00)

DWG NO. TAM T1805359
STRUCTURAL
COMPONENTS INC.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T82	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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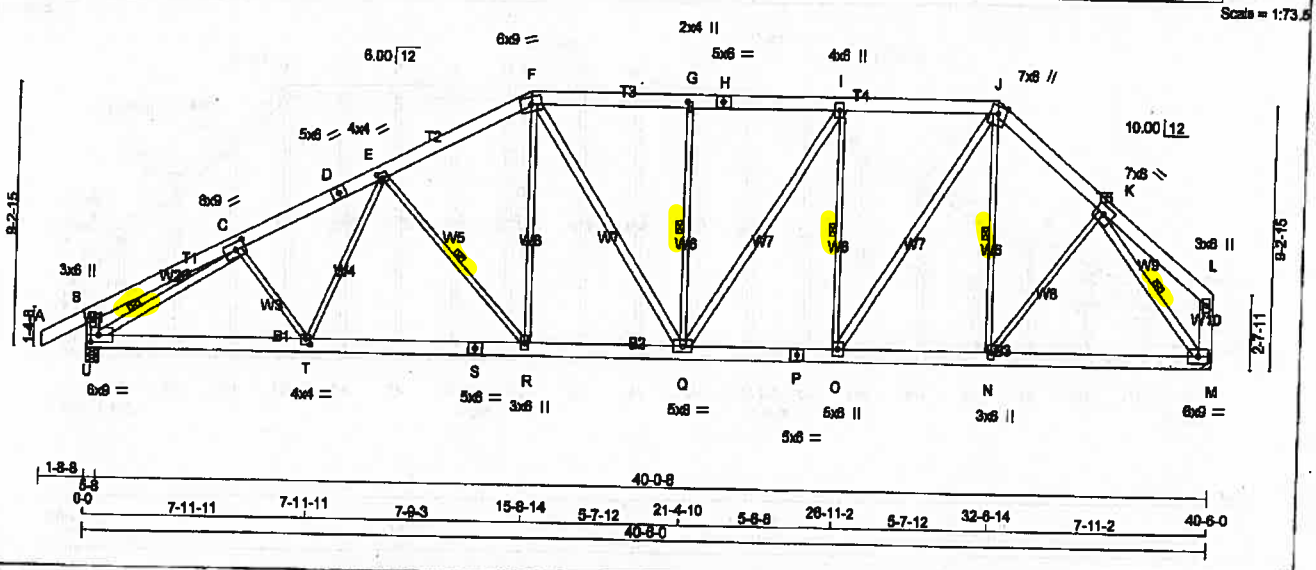

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
B - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Units is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	6.0		
C	TMWW-t	MT20	6.0	9.0	3.00	4.00
D	TS-t	MT20	5.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.50	1.50
F	TTWW-m	MT20	6.0	9.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	8.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	
K	TMWW-t	MT20	7.0	6.0		
L	TMV+p	MT20	3.0	6.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMWW+t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	3.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0	2.50	2.00
U	BMVW1-t	MT20	6.0	9.0	3.00	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3640	0	3640	0	0	5-8	5-8
M	3405	0	3405	0	0	MECHANICAL	— HGU526

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2660	1686 / 0	425 / 0	0 / 0	0 / 0	548 / 0	0 / 0
M	2500	1546 / 0	425 / 0	0 / 0	0 / 0	528 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, G-Q, I-O, J-N, K-M.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00
B- C	-1 / 17	-129.7	-129.7	0.24 (1)	10.00
C- D	-4969 / 0	-129.7	-129.7	0.31 (1)	3.71
D- E	-4969 / 0	-129.7	-129.7	0.31 (1)	3.71
E- F	-4216 / 0	-129.7	-129.7	0.29 (1)	4.00
F- G	-3956 / 0	-129.7	-129.7	0.30 (1)	4.09
G- H	-3956 / 0	-129.7	-129.7	0.29 (1)	4.12
H- I	-3956 / 0	-129.7	-129.7	0.28 (1)	4.12
I- J	-3547 / 0	-129.7	-129.7	0.28 (1)	4.31
J- K	-3304 / 0	-129.7	-129.7	0.16 (1)	4.58
K- L	0 / 29	-129.7	-129.7	0.12 (1)	10.00
U- B	-536 / 0	0.0	0.0	0.03 (1)	7.81
M- L	-206 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 4438	-38.5	-38.5	0.68 (1)	10.00
T- S	0 / 4358	-38.5	-38.5	0.62 (1)	10.00
S- R	0 / 4358	-38.5	-38.5	0.62 (1)	10.00
R- Q	0 / 3754	-38.5	-38.5	0.55 (1)	10.00
Q- P	0 / 3547	-38.5	-38.5	0.48 (1)	10.00
P- O	0 / 3547	-38.5	-38.5	0.48 (1)	10.00
O- N	0 / 2506	-38.5	-38.5	0.44 (1)	10.00
N- M	0 / 2226	-38.5	-38.5	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 193	0.05 (3)
T- E	0 / 346	0.08 (2)
E- R	-923 / 0	0.40 (1)
R- F	0 / 969	0.22 (1)
F- Q	0 / 373	0.09 (1)
Q- G	-773 / 0	0.40 (1)
Q- I	0 / 767	0.12 (1)
O- I	-1422 / 0	0.73 (1)
O- J	0 / 1931	0.91 (1)
N- J	45 / 178	0.04 (3)
N- K	0 / 65	0.01 (4)
U- C	-3940 / 0	0.68 (1)
M- N	-2793 / 0	0.74 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

TOTAL WEIGHT = 4 X 256 = 1024 lb [MF]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.6	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 9.4 P.S.F RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.31/1.00 (C-E:1) . BC=0.68/1.00 (T-U:1) . WB=0.77/0.90 (K-M:1) . SSI=0.26/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLF) MAX	MIN	BECTION (PLF) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)

DWG NO. TAM T1905360
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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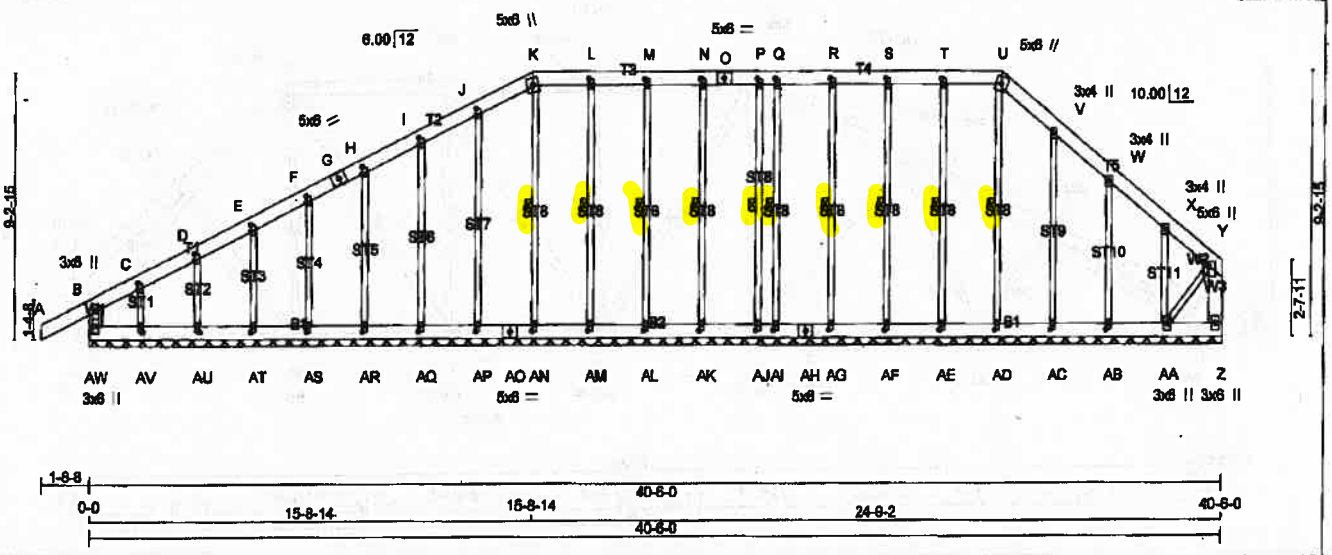

TOTAL WEIGHT = 269 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AW- B	2x6	DRY	No.2	SPF
A - G	2x6	DRY	No.2	SPF
G - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
O - U	2x6	DRY	No.2	SPF
U - Y	2x6	DRY	No.2	SPF
Z - Y	2x6	DRY	No.2	SPF
AW- AO	2x6	DRY	No.2	SPF
AO- AH	2x6	DRY	No.2	SPF
AH- Z	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C, D, E, F, H, I, J						
C	TMW+w	MT20	2.0	4.0	3.00	
G	TS-t	MT20	5.0	8.0		
K	TTW+m	MT20	5.0	8.0		
L, M, N, P, Q, R, S, T						
L	TMW+w	MT20	2.0	4.0	2.50	1.00
O	TS-t	MT20	5.0	8.0		
U	TTW+m	MT20	5.0	8.0		
V, W, X						
V	TMW+w	MT20	3.0	4.0		
Y	TMVW+p	MT20	5.0	6.0	2.00	2.25
Z	BMV1+p	MT20	3.0	6.0		
AA	BMMW1+t	MT20	3.0	6.0		
AB, AC, AD, AE, AF, AG, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU, AV						
AB	BMW1+w	MT20	2.0	4.0	2.50	1.00
AH	BS-t	MT20	5.0	8.0		
AO	BS-t	MT20	5.0	8.0		
AW	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF U-AD, T-AE, S-AF, R-AG, Q-AI, P-AJ, N-AK, M-AL, L-AM, K-AN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AW- B	-470 / 0	0.0	0.0	0.03 (1)	7.61
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00
B- C	-55 / 0	-129.7	-129.7	0.14 (1)	6.25
C- D	0 / 17	-129.7	-129.7	0.03 (1)	10.00
D- E	0 / 16	-129.7	-129.7	0.03 (1)	10.00
E- F	0 / 25	-129.7	-129.7	0.03 (1)	10.00
F- G	0 / 32	-129.7	-129.7	0.03 (1)	10.00
G- H	0 / 32	-129.7	-129.7	0.03 (1)	10.00
H- I	0 / 37	-129.7	-129.7	0.03 (1)	10.00
I- J	0 / 42	-129.7	-129.7	0.04 (1)	10.00
J- K	0 / 35	-129.7	-129.7	0.04 (1)	10.00
K- L	0 / 43	-129.7	-129.7	0.04 (1)	10.00
L- M	0 / 43	-129.7	-129.7	0.04 (1)	10.00
M- N	0 / 43	-129.7	-129.7	0.04 (1)	10.00
N- O	0 / 43	-129.7	-129.7	0.04 (1)	10.00
O- P	0 / 43	-129.7	-129.7	0.04 (1)	10.00
P- Q	0 / 43	-129.7	-129.7	0.04 (1)	10.00
Q- R	0 / 43	-129.7	-129.7	0.04 (1)	10.00
R- S	0 / 43	-129.7	-129.7	0.04 (1)	10.00
S- T	0 / 43	-129.7	-129.7	0.04 (1)	10.00
T- U	0 / 44	-129.7	-129.7	0.04 (1)	10.00
U- V	0 / 37	-129.7	-129.7	0.04 (1)	10.00
V- W	0 / 47	-129.7	-129.7	0.04 (1)	10.00
W- X	0 / 38	-129.7	-129.7	0.03 (1)	10.00
X- Y	0 / 37	-129.7	-129.7	0.03 (1)	10.00
Z- Y	-67 / 0	0.0	0.0	0.01 (1)	7.61
AW-AV	0 / 0	-38.5	-38.5	0.01 (3)	10.00
AV-AU	-5 / 0	-38.5	-38.5	0.01 (3)	10.00
AU-AT	-16 / 0	-38.5	-38.5	0.01 (3)	6.25
AT-AS	-23 / 0	-38.5	-38.5	0.01 (3)	6.25
AS-AR	-29 / 0	-38.5	-38.5	0.01 (3)	6.25
AR-AQ	-33 / 0	-38.5	-38.5	0.01 (3)	6.25
AQ-AP	-37 / 0	-38.5	-38.5	0.01 (3)	6.25
AP-AO	-40 / 0	-38.5	-38.5	0.01 (3)	6.25
AO-AN	-40 / 0	-38.5	-38.5	0.01 (3)	6.25
AN-AM	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AM-AL	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AL-AK	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AK-AJ	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AJ-AI	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AI-AH	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AH-AG	-43 / 0	-38.5	-38.5	0.01 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AD- U	-254 / 0	0.13 (1)
AE- T	-292 / 0	0.15 (1)
AF- S	-263 / 0	0.14 (1)
AG- R	-246 / 0	0.13 (1)
AI- Q	-178 / 0	0.09 (1)
AJ- P	-176 / 0	0.09 (1)
AK- N	-248 / 0	0.13 (1)
AL- M	-263 / 0	0.14 (1)
AM- L	-291 / 0	0.15 (1)
AP- J	-282 / 0	0.34 (1)
AQ- I	-257 / 0	0.21 (1)
AR- H	-256 / 0	0.14 (1)
AS- F	-257 / 0	0.09 (1)
AT- E	-253 / 0	0.08 (1)
AU- D	-282 / 0	0.05 (1)
AV- C	-92 / 0	0.01 (1)
AC- V	-284 / 0	0.27 (1)
AB- W	-251 / 0	0.12 (1)
AA- X	-265 / 0	0.08 (1)
AA- Y	-24 / 0	0.00 (1)
AN- K	-240 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.01/1.00 (AA-AB:3) , WB=0.34/1.00 (J-AP:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

TPI GRIP= 0.51 (AD) (INPUT = 0.90)
PLUS METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 16605331
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AG-AF	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AF-AE	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AE-AD	-43 / 0	-38.5	-38.5	0.01 (3)	6.25
AD-AC	-40 / 0	-38.5	-38.5	0.01 (3)	8.25
AC-AB	-34 / 0	-38.5	-38.5	0.01 (3)	6.25
AB-AA	-27 / 0	-38.5	-38.5	0.01 (3)	6.25
AA-Z	0 / 0	-38.5	-38.5	0.01 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17605331
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	CRWG NO.
297083	T83	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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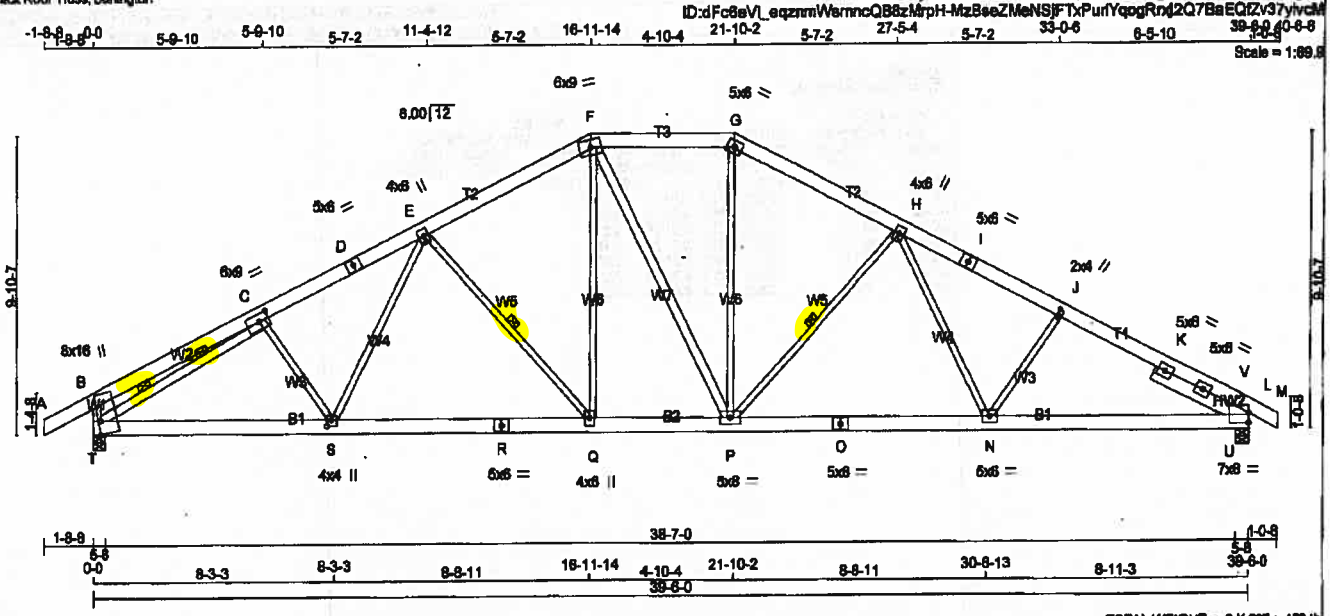

TOTAL WEIGHT = 2 X 227 = 453 lb

[MP]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - P	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
B	TMBMVW1*+m	MT20	6.0	16.0	Edge	1.00
C	TMWW-t	MT20	6.0	9.0	3.00	4.00
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	6.0	9.0		
G	TTW-h	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMW+w	MT20	2.0	4.0	2.50	1.00
L	TMBMR1-t	MT20	7.0	6.0	4.00	
L	RT-t	MT20	5.0	6.0		
L	RT-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	4.0	4.0	2.50	1.75
T						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3556	0	3556	0	0	5-8	5-8
L	3470	0	3470	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX.MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2598	1648 / 0	415 / 0	0 / 0	0 / 0	535 / 0	0 / 0
L	2539	1597 / 0	415 / 0	0 / 0	0 / 0	527 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 53	-129.7	-129.7	0.16 (1)	10.00
B- C	0 / 21	-129.7	-129.7	0.27 (1)	10.00
C- D	-4835 / 0	-129.7	-129.7	0.34 (1)	3.73
D- E	-4835 / 0	-129.7	-129.7	0.34 (1)	3.73
E- F	-3865 / 0	-129.7	-129.7	0.31 (1)	4.13
F- G	-3474 / 0	-129.7	-129.7	0.25 (1)	4.37
G- H	-5911 / 0	-129.7	-129.7	0.34 (1)	4.08
H- I	-5025 / 0	-129.7	-129.7	0.39 (1)	3.60
I- J	-5025 / 0	-129.7	-129.7	0.39 (1)	3.60
J- K	-4038 / 0	-129.7	-129.7	0.43 (1)	3.91
K- V	-4038 / 0	-129.7	-129.7	0.43 (1)	3.91
V- L	-4525 / 0	-129.7	-129.7	0.18 (1)	4.01
L- M	0 / 1	-129.7	-129.7	0.08 (1)	10.00
T- B	-554 / 0	0.0	0.0	0.08 (1)	7.61
T- S	0 / 4367	-38.5	-38.5	0.66 (1)	10.00
S- R	0 / 4139	-38.5	-38.5	0.63 (1)	10.00
R- Q	0 / 4139	-38.5	-38.5	0.63 (1)	10.00
Q- P	0 / 3436	-38.5	-38.5	0.55 (1)	10.00
P- O	0 / 4286	-38.5	-38.5	0.65 (1)	10.00
O- N	0 / 4286	-38.5	-38.5	0.65 (1)	10.00
N- U	0 / 4686	-38.5	-38.5	0.74 (1)	10.00
U- L	0 / 2622	-38.5	-38.5	0.50 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-59 / 162	0.04 (3)
S- E	0 / 483	0.11 (2)
E- Q	-1085 / 0	0.54 (1)
Q- F	0 / 1029	0.23 (1)
F- P	0 / 81	0.01 (1)
P- G	0 / 1134	0.26 (1)
P- H	-1235 / 0	0.63 (1)
H- N	0 / 588	0.13 (2)
N- J	-271 / 47	0.09 (1)
T- C	-5226 / 0	0.72 (1)
U- V	0 / 2458	0.00 (1)
U- K	-2795 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.43/1.00 (J-V:1) , BC=0.74/1.00 (N-U:1) , WB=0.72/0.80 (C-T:1) , SSI=0.29/1.00 (L-U:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

TUS GRIP= 0.89 (S) (INPUT = 0.90)
TUS METAL= 0.88 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MYERS
PROVINCE OF ONTARIO

DWG NO. TAM 71305361
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G83	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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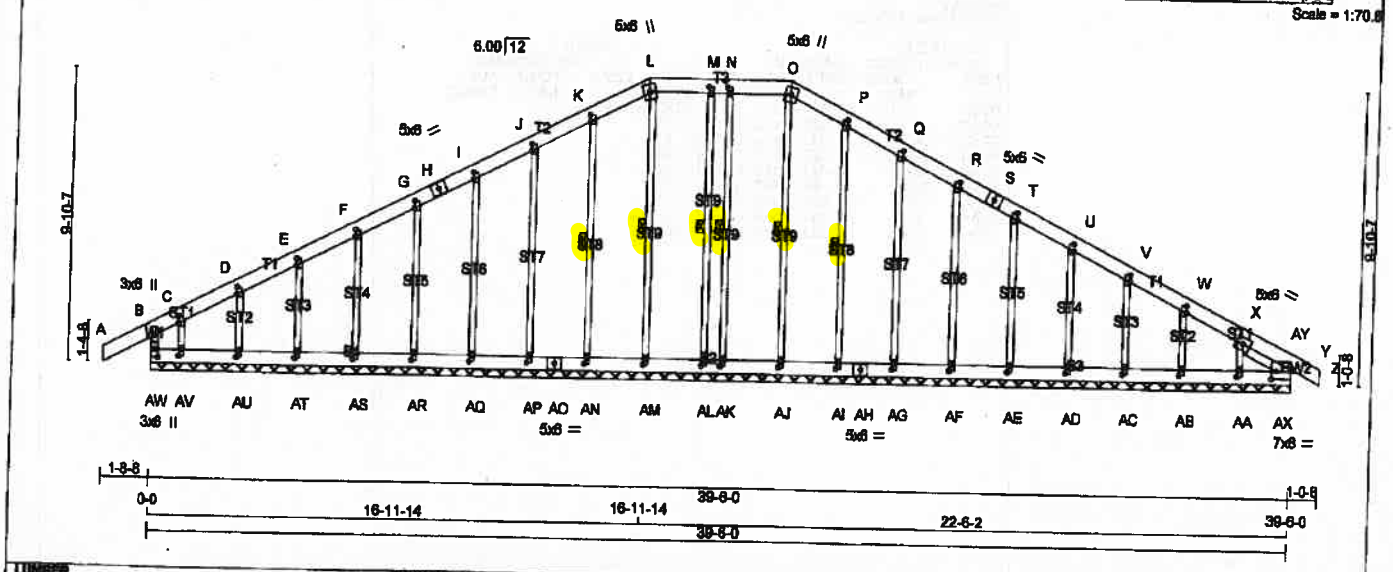

TOTAL WEIGHT = 246 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AW B	2x4	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - O	2x6	DRY	No.2	SPF
O - S	2x6	DRY	No.2	SPF
S - Z	2x6	DRY	No.2	SPF
AW AO	2x6	DRY	No.2	SPF
AO- AH	2x6	DRY	No.2	SPF
AH- Y	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C, D, E, G, I, J, K, P, Q, R, T, U, V, W						
C	TMW+w	MT20	2.0	4.0	3.00	
F	TMW+w	MT20	2.0	4.0	2.75	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	6.0		
M	TMW+w	MT20	2.0	4.0	2.50	1.00
N	TMW+w	MT20	2.0	4.0	2.50	1.00
O	TTW+m	MT20	5.0	6.0		
S	TS-t	MT20	5.0	6.0		
X	TMWW-t	MT20	5.0	6.0		
Y	TMBMW1-t	MT20	7.0	6.0	3.25	2.00
AA, AB, AC, AD, AE, AF, AG, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU, AV						
AA	BMW1+w	MT20	2.0	4.0	2.50	1.00
AH	BS-t	MT20	5.0	6.0		
AO	BS-t	MT20	5.0	6.0		
AW	BMV1+p	MT20	3.0	6.0	3.25	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AJ, N-AK, M-AL, K-AN, P-AI, L-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AW- B	-501 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00
B- C	-80 / 0	-129.7	-129.7	0.14 (1)	6.25
C- D	0 / 5	-129.7	-129.7	0.03 (1)	10.00
D- E	0 / 11	-129.7	-129.7	0.03 (1)	10.00
E- F	0 / 20	-129.7	-129.7	0.03 (1)	10.00
F- G	0 / 26	-129.7	-129.7	0.03 (1)	10.00
G- H	0 / 32	-129.7	-129.7	0.03 (1)	6.25
H- I	0 / 32	-129.7	-129.7	0.03 (1)	6.25
I- J	0 / 37	-129.7	-129.7	0.03 (1)	10.00
J- K	0 / 41	-129.7	-129.7	0.04 (1)	10.00
K- L	0 / 34	-129.7	-129.7	0.03 (1)	10.00
L- M	0 / 42	-129.7	-129.7	0.04 (1)	10.00
M- N	0 / 42	-129.7	-129.7	0.04 (1)	10.00
N- O	0 / 42	-129.7	-129.7	0.04 (1)	10.00
O- P	0 / 34	-129.7	-129.7	0.03 (1)	10.00
P- Q	0 / 41	-129.7	-129.7	0.04 (1)	10.00
Q- R	0 / 37	-129.7	-129.7	0.03 (1)	10.00
R- S	0 / 32	-129.7	-129.7	0.03 (1)	6.25
S- T	0 / 32	-129.7	-129.7	0.03 (1)	6.25
T- U	0 / 26	-129.7	-129.7	0.03 (1)	10.00
U- V	0 / 20	-129.7	-129.7	0.03 (1)	10.00
V- W	0 / 11	-129.7	-129.7	0.03 (1)	10.00
W- X	0 / 1	-129.7	-129.7	0.03 (1)	10.00
X-AY	-13 / 0	-129.7	-129.7	0.02 (1)	6.25
AY- Y	-39 / 0	-129.7	-129.7	0.00 (1)	6.25
Y- Z	0 / 1	-129.7	-129.7	0.06 (1)	10.00
AW-AV	0 / 0	-38.5	-38.5	0.01 (3)	10.00
AV-AU	0 / 2	-38.5	-38.5	0.01 (3)	10.00
AU-AT	-10 / 0	-38.5	-38.5	0.01 (3)	10.00
AT-AS	-16 / 0	-38.5	-38.5	0.01 (3)	6.25
AS-AR	-24 / 0	-38.5	-38.5	0.01 (3)	6.25
AR-AQ	-29 / 0	-38.5	-38.5	0.01 (3)	6.25
AQ-AP	-33 / 0	-38.5	-38.5	0.01 (3)	6.25
AP-AO	-36 / 0	-38.5	-38.5	0.01 (3)	6.25
AO-AN	-36 / 0	-38.5	-38.5	0.01 (3)	6.25
AN-AM	-40 / 0	-38.5	-38.5	0.01 (3)	6.25
AM-AL	-42 / 0	-38.5	-38.5	0.01 (3)	6.25
AL-AK	-42 / 0	-38.5	-38.5	0.01 (3)	6.25
AK-AJ	-42 / 0	-38.5	-38.5	0.01 (3)	6.25
AJ-AI	-40 / 0	-38.5	-38.5	0.01 (3)	6.25
AI-AH	-36 / 0	-38.5	-38.5	0.01 (3)	6.25
AH-AG	-36 / 0	-38.5	-38.5	0.01 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AI- O	-236 / 0	0.14 (1)
AK- N	-205 / 0	0.12 (1)
AL- M	-205 / 0	0.12 (1)
AN- K	-281 / 0	0.13 (1)
AP- J	-258 / 0	0.27 (1)
AQ- I	-256 / 0	0.16 (1)
AR- G	-257 / 0	0.12 (1)
AS- F	-258 / 0	0.08 (1)
AT- E	-255 / 0	0.05 (1)
AU- D	-271 / 0	0.04 (1)
AV- C	0 / 24	0.01 (1)
AI- P	-281 / 0	0.13 (1)
AG- Q	-258 / 0	0.27 (1)
AF- R	-258 / 0	0.16 (1)
AE- T	-257 / 0	0.12 (1)
AD- U	-256 / 0	0.08 (1)
AC- V	-258 / 0	0.05 (1)
AB- W	-252 / 0	0.04 (1)
AA- X	-240 / 0	0.03 (1)
AM- L	-236 / 0	0.14 (1)
X-AX	0 / 28	0.00 (1)
AX-AY	-69 / 7	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH;
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (Y-AX:1) , WB=0.27/1.00 (Q-AG:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.51 (AT) (INPUT = 0.90)
LSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805332
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G83	1	1	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AG-AF	-33 / 0	-38.5	-38.5	0.01 (3)	6.25
AF-AE	-26 / 0	-38.5	-38.5	0.01 (3)	6.25
AE-AD	-24 / 0	-38.5	-38.5	0.01 (3)	6.25
AD-AC	-18 / 0	-38.5	-38.5	0.01 (3)	6.25
AC-AB	-10 / 0	-38.5	-38.5	0.01 (3)	10.00
AB-AA	0 / 0	-38.5	-38.5	0.01 (2)	10.00
AA-AX	0 / 0	-38.5	-38.5	0.01 (1)	10.00
AX- Y	0 / 20	-38.5	-38.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17605332
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T84	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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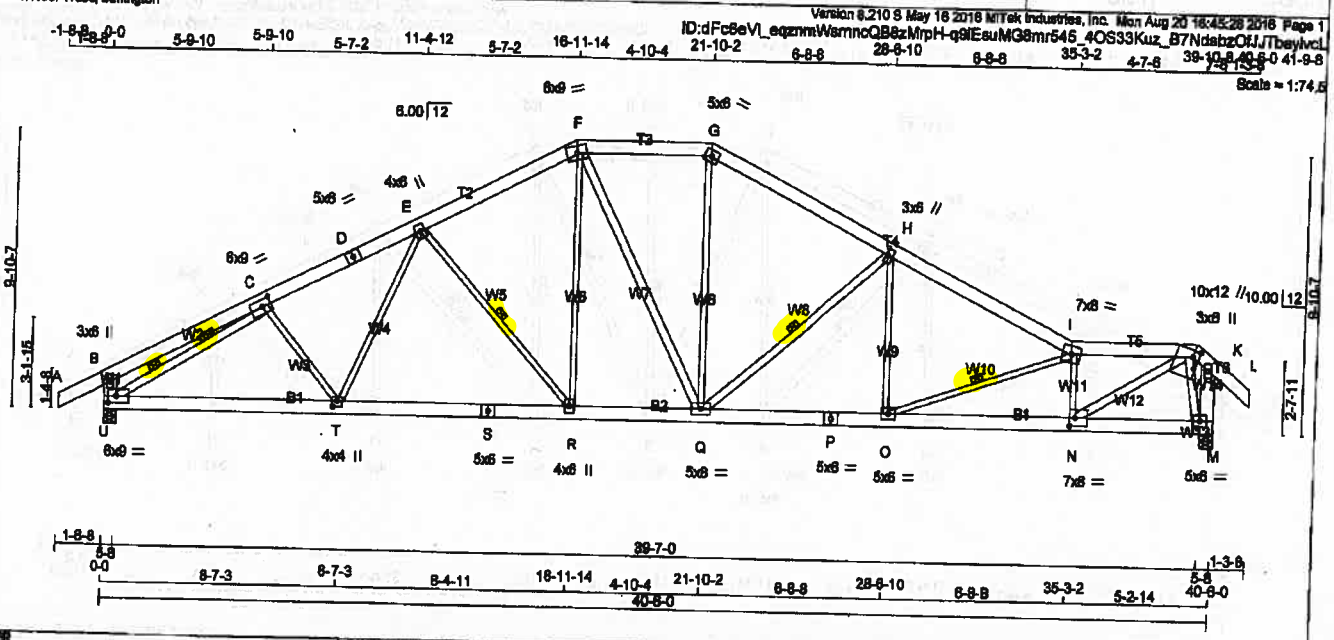

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	6.0	9.0	3.00	4.00
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	6.0	9.0		
G	TTW-h	MT20	5.0	6.0		
H	TMWW+t	MT20	3.0	6.0	3.00	0.75
I	TTWW-m	MT20	7.0	6.0		
J	TTWW+m	MT20	10.0	12.0	3.00	2.75
K	TMV+p	MT20	3.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMWW-t	MT20	7.0	6.0	3.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	4.0	2.50	1.75
U	BMVW1-t	MT20	6.0	9.0	3.00	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3697	0	3697	0	0	5-8	5-8
M	3595	0	3595	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
U	2858	1684 / 0	425 / 0	0 / 0	0 / 0	548 / 0	0 / 0
M	2829	1660 / 0	425 / 0	0 / 0	0 / 0	544 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q, I-O.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00	
B- C	0 / 21	-129.7	-129.7	0.27 (1)	10.00	
C- D	-4966 / 0	-129.7	-129.7	0.34 (1)	3.68	
D- E	-4966 / 0	-129.7	-129.7	0.34 (1)	3.68	
E- F	-4028 / 0	-129.7	-129.7	0.32 (1)	4.05	
F- G	-3650 / 0	-129.7	-129.7	0.25 (1)	4.28	
G- H	-4114 / 0	-129.7	-129.7	0.45 (1)	3.85	
H- I	-5400 / 0	-129.7	-129.7	0.52 (1)	3.34	
I- J	-5598 / 0	-129.7	-129.7	0.30 (1)	3.49	
J- K	-121 / 0	-129.7	-129.7	0.07 (1)	8.25	
K- L	0 / 60	-129.7	-129.7	0.10 (1)	10.00	
U- B	-555 / 0	0.0	0.0	0.04 (1)	7.81	
M- K	-422 / 0	0.0	0.0	0.03 (1)	7.81	
U- T	0 / 4477	-38.5	-38.5	0.70 (1)	10.00	
T- S	0 / 4284	-38.5	-38.5	0.85 (1)	10.00	
S- R	0 / 4284	-38.5	-38.5	0.85 (1)	10.00	
R- Q	0 / 3584	-38.5	-38.5	0.52 (1)	10.00	
Q- P	0 / 4873	-38.5	-38.5	0.68 (1)	10.00	
P- O	0 / 4873	-38.5	-38.5	0.68 (1)	10.00	
O- N	0 / 5705	-38.5	-38.5	0.81 (1)	10.00	
N- M	0 / 802	-38.5	-38.5	0.20 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-44 / 188	0.04 (3)
T- E	0 / 494	0.10 (2)
E- R	-1064 / 0	0.54 (1)
R- F	0 / 1043	0.23 (1)
F- Q	0 / 147	0.02 (1)
Q- G	0 / 1176	0.26 (1)
Q- H	-1651 / 0	0.66 (1)
O- H	0 / 656	0.15 (2)
O- I	-896 / 0	0.32 (1)
N- I	-2742 / 0	0.47 (1)
U- C	-5358 / 0	0.73 (1)
J- M	-3165 / 0	0.39 (1)
N- J	0 / 5620	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.7	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.52/1.00 (H-I:1) , BC=0.61/1.00 (N-O:1) , WB=0.90/0.90 (J-N:1) , SSI=0.29/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.92 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. TOLS
PROVINCE OF ONTARIO

DWG NO. TAM 1805362
STRUCTURAL
COMPONENTS ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stage

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T85	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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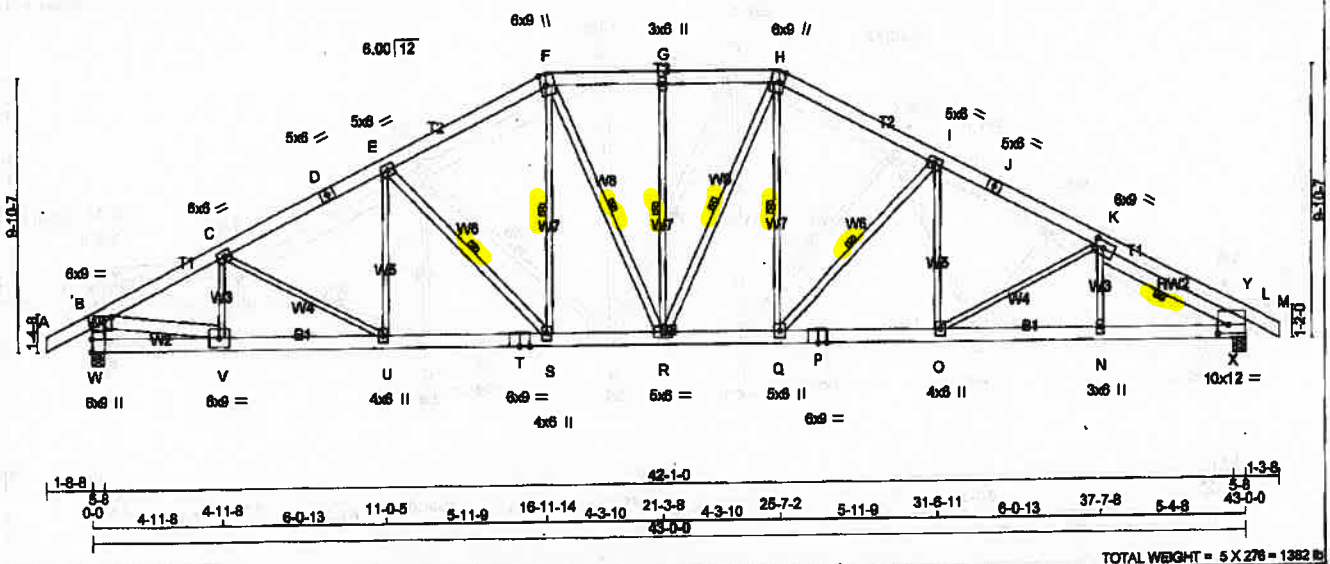

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF
B - V	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	6.0	9.0	2.25	4.25
C, E, I						
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge 2.25	
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge 2.25	
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	6.0	9.0	3.00	4.00
L	TMBMW1-t	MT20	10.0	12.0	3.75	4.50
N	BMW+w	MT20	3.0	6.0		
O, S, U						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
T	BS-t	MT20	6.0	9.0		
V	BMWW-t	MT20	6.0	9.0		
W	BMV1+t	MT20	6.0	9.0	6.50	

Edge -1NDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AYES
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4800	0	4897	288	-1015	5-8	5-8 -2TBE6
L	4729	0	4844	0	-1002	5-8	5-8 -2TBE6

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1015 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1002 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 288 LBS FACTORED HORIZONTAL REACTION AT JOINT W ok

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3447	2480 / 0	452 / 0	0 / 0	151 / -1098	581 / 0	0 / 0
L	3399	2448 / 0	452 / 0	0 / 0	93 / -1088	576 / 0	0 / 0
HORIZONTAL REACTIONS							
W	—	0 / 0	0 / 0	0 / 0	204 / -172	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, K-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 62	-170.4	-170.4	0.23 (2)	10.00	
B-C	-6811 / 1405	-170.4	-170.4	0.58 (2)	2.99	
C-D	-6531 / 1451	-170.4	-170.4	0.80 (2)	2.76	
D-E	-6531 / 1451	-170.4	-170.4	0.80 (2)	2.76	
E-F	-5534 / 1317	-170.4	-170.4	0.70 (2)	3.17	
F-G	-5169 / 1283	-170.4	-170.4	0.33 (3)	3.60	
G-H	-5169 / 1283	-170.4	-170.4	0.33 (3)	3.60	
H-I	-5578 / 1330	-170.4	-170.4	0.72 (3)	3.13	
I-J	-6682 / 1484	-170.4	-170.4	0.83 (3)	2.67	
J-K	-6682 / 1484	-170.4	-170.4	0.83 (3)	2.67	
K-Y	-3203 / 790	-170.4	-170.4	0.36 (3)	4.36	
Y-L	-5370 / 1016	-170.4	-170.4	0.45 (3)	3.38	
L-M	0 / 2	-170.4	-170.4	0.13 (3)	10.00	
W-B	-4790 / 1031	0.0	0.0	0.31 (2)	4.70	
W-V	-267 / 280	-38.5	-38.5	0.05 (2)	6.25	
V-U	-1380 / 5943	-38.5	-38.5	0.35 (1)	6.25	
U-T	-1211 / 5858	-38.5	-38.5	0.32 (1)	6.25	
T-S	-1211 / 5858	-38.5	-38.5	0.32 (1)	6.25	
S-R	-848 / 4916	-38.5	-38.5	0.28 (1)	6.25	
R-Q	-742 / 4957	-38.5	-38.5	0.27 (1)	6.25	
Q-P	-976 / 6000	-38.5	-38.5	0.33 (1)	6.25	
P-O	-976 / 6000	-38.5	-38.5	0.33 (1)	6.25	
O-N	-1195 / 6334	-38.5	-38.5	0.36 (3)	6.25	
N-X	-1194 / 6337	-38.5	-38.5	0.35 (3)	6.25	
X-L	-551 / 2858	-38.5	-38.5	0.15 (1)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-913 / 289	0.16 (1)
C-U	-311 / 412	0.28 (2)
U-E	-90 / 431	0.10 (5)
E-S	-1691 / 532	0.63 (2)
S-F	-317 / 1419	0.32 (2)
F-R	-291 / 1379	0.22 (3)
R-G	-1036 / 310	0.61 (2)
R-H	-276 / 1305	0.21 (2)
Q-H	-398 / 1527	0.34 (3)
Q-I	-1843 / 561	0.68 (3)
O-I	-45 / 535	0.12 (3)
O-K	-582 / 249	0.52 (3)
N-K	0 / 259	0.06 (17)
B-V	-1131 / 6038	0.71 (1)
K-X	-4298 / 779	0.55 (1)
X-Y	-324 / 2848	0.00 (1)

DWG NO. TAM T1805365
STRUCTURAL
ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	61.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	75.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.83/1.00 (I-K:3) , BC=0.36/1.00 (N-O:3) , WB=0.71/0.90 (B-V:1) , SSI=0.39/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1657	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.99 (V) (INPUT = 1.00)

K

CONTINUED ON PAGE 2

TOWN OF INNISFIL 2018-0833-0840 REVIEWED Building Permit Ready

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T85	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:o4CzN5b4M0u2JhRQzBbNkBzMrUq-JL_Jd3ENuu3zyiFZGyAaht5W43Xqmb5CXurz20B0yIvcK

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (0.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. JONES
PROVINCE OF ONTARIO

DWG NO. TAM 11305363
STRUCTURAL
CITY OF OTTAWA 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G85	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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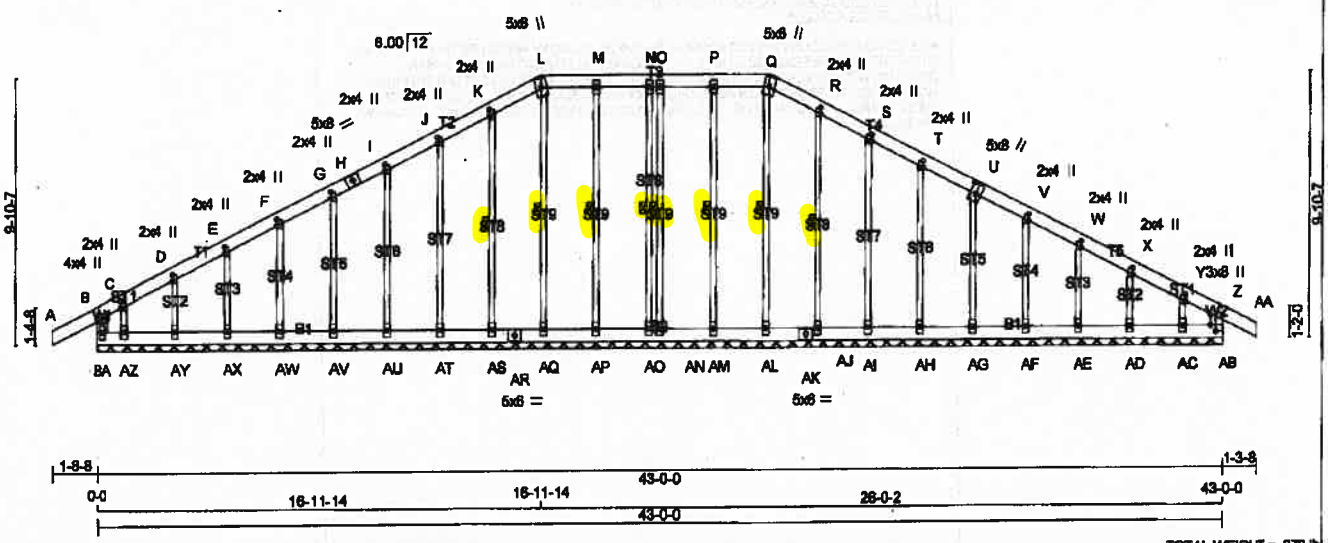

TOTAL WEIGHT = 273 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BA- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - Q	2x6	DRY	No.2	SPF
Q - U	2x6	DRY	No.2	SPF
U - AA	2x6	DRY	No.2	SPF
AB- Z	2x6	DRY	No.2	SPF
BA- AR	2x6	DRY	No.2	SPF
AR- AK	2x6	DRY	No.2	SPF
AK- AB	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0		
C, D, E, G, I, J, K, R, S, T, V, W, X, Y						
C	TMW+w	MT20	2.0	4.0	3.00	
F	TMW+w	MT20	2.0	4.0	2.75	
H	TS-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	6.0		
M, N, O, P						
M	TMW+w	MT20	3.0	6.0		
Q	TTW+m	MT20	5.0	6.0		
U	TSW+I	MT20	6.0	8.0	4.50	2.50
Z						
Z	TMBMV1+p	MT20	3.0	6.0	2.75	2.25
AB						
AC, AD, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ, AS, AT, AU, AV, AW, AX, AY, AZ						
AC	BMW1+w	MT20	3.0	6.0		
AK	BS-t	MT20	5.0	6.0		
AR	BS-t	MT20	5.0	6.0		
BA	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-AL, P-AM, O-AN, N-AO, M-AP, K-AS, R-AJ, L-AQ.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
BA- B	-463 / 0	0.0	0.0	0.05 (1)	7.61	
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00	
B- C	-116 / 0	-129.7	-129.7	0.12 (1)	6.25	
C- D	-38 / 0	-129.7	-129.7	0.03 (1)	6.25	
D- E	-31 / 0	-129.7	-129.7	0.03 (1)	6.25	
E- F	-22 / 0	-129.7	-129.7	0.03 (1)	6.25	
F- G	-16 / 0	-129.7	-129.7	0.03 (1)	6.25	
G- H	-10 / 0	-129.7	-129.7	0.03 (1)	6.25	
H- I	-10 / 0	-129.7	-129.7	0.03 (1)	6.25	
I- J	-6 / 0	-129.7	-129.7	0.06 (1)	10.00	
J- K	-1 / 0	-129.7	-129.7	0.03 (1)	10.00	
K- L	0 / 1	-129.7	-129.7	0.03 (1)	10.00	
L- M	0 / 5	-129.7	-129.7	0.03 (1)	10.00	
M- N	0 / 5	-129.7	-129.7	0.04 (1)	10.00	
N- O	0 / 5	-129.7	-129.7	0.04 (1)	10.00	
O- P	0 / 5	-129.7	-129.7	0.04 (1)	10.00	
P- Q	0 / 5	-129.7	-129.7	0.03 (1)	10.00	
Q- R	0 / 1	-129.7	-129.7	0.03 (1)	10.00	
R- S	-2 / 0	-129.7	-129.7	0.03 (1)	10.00	
S- T	-8 / 0	-129.7	-129.7	0.03 (1)	10.00	
T- U	0 / 0	-129.7	-129.7	0.03 (1)	10.00	
U- V	-30 / 0	-129.7	-129.7	0.03 (1)	6.25	
V- W	-29 / 0	-129.7	-129.7	0.03 (1)	6.25	
W- X	-35 / 0	-129.7	-129.7	0.03 (1)	6.25	
X- Y	-41 / 0	-129.7	-129.7	0.03 (1)	6.25	
Y- Z	-68 / 0	-129.7	-129.7	0.07 (1)	6.25	
Z-AA	0 / 40	-129.7	-129.7	0.09 (1)	10.00	
AB- Z	-363 / 0	0.0	0.0	0.04 (1)	7.61	
BA-AZ	0 / 40	-38.5	-38.5	0.06 (1)	10.00	
AZ-AY	0 / 39	-38.5	-38.5	0.02 (2)	10.00	
AY-AX	0 / 28	-38.5	-38.5	0.02 (2)	10.00	
AX-AW	0 / 20	-38.5	-38.5	0.01 (2)	10.00	
AW-AV	0 / 14	-38.5	-38.5	0.01 (2)	10.00	
AV-AU	0 / 9	-38.5	-38.5	0.01 (2)	10.00	
AU-AT	0 / 5	-38.5	-38.5	0.01 (3)	10.00	
AT-AS	0 / 1	-38.5	-38.5	0.01 (3)	10.00	
AS-AR	-2 / 0	-38.5	-38.5	0.01 (3)	10.00	
AR-AQ	-2 / 0	-38.5	-38.5	0.01 (3)	10.00	
AQ-AP	-5 / 0	-38.5	-38.5	0.01 (3)	10.00	
AP-AO	-5 / 0	-38.5	-38.5	0.01 (3)	10.00	
AO-AN	-5 / 0	-38.5	-38.5	0.01 (3)	10.00	
AN-AM	-5 / 0	-38.5	-38.5	0.01 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AL- Q	-269 / 0	0.16 (1)
AM- P	-253 / 0	0.15 (1)
AN- O	-166 / 0	0.10 (1)
AO- N	-166 / 0	0.10 (1)
AP- M	-253 / 0	0.15 (1)
AS- K	-260 / 0	0.12 (1)
AT- J	-257 / 0	0.27 (1)
AU- I	-257 / 0	0.16 (1)
AV- G	-257 / 0	0.12 (1)
AW- F	-256 / 0	0.08 (1)
AX- E	-255 / 0	0.05 (1)
AY- D	-266 / 0	0.04 (1)
AZ- C	-16 / 0	0.00 (1)
AJ- R	-259 / 0	0.12 (1)
AI- S	-258 / 0	0.27 (1)
AH- T	-268 / 0	0.21 (1)
AG- U	-209 / 0	0.10 (1)
AF- V	-275 / 0	0.09 (1)
AE- W	-253 / 0	0.05 (1)
AD- X	-269 / 0	0.04 (1)
AC- Y	-138 / 0	0.02 (1)
AQ- L	-269 / 0	0.16 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALLING
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2016

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.05/1.00 (AZ-BA:1) , WB=0.27/1.00 (J-AT:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (C) (INPUT = 0.90)
JSI METAL= 0.13 (Z) (INPUT = 1.00)

DWG NO. TAM 71805333
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G85	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AM-AL	-5 / 0	-38.5	-38.5	0.01 (3)	10.00
AL-AK	-2 / 0	-38.5	-38.5	0.01 (3)	10.00
AK-AJ	-2 / 0	-38.5	-38.5	0.01 (3)	10.00
AJ-AI	0 / 1	-38.5	-38.5	0.01 (3)	10.00
AI-AH	0 / 5	-38.5	-38.5	0.01 (3)	10.00
AH-AG	0 / 10	-38.5	-38.5	0.01 (2)	10.00
AG-AF	0 / 17	-38.5	-38.5	0.01 (2)	10.00
AF-AE	0 / 23	-38.5	-38.5	0.01 (2)	10.00
AE-AD	0 / 31	-38.5	-38.5	0.02 (2)	10.00
AD-AC	0 / 43	-38.5	-38.5	0.02 (2)	10.00
AC-AB	0 / 53	-38.5	-38.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MEBS
PROVINCE OF ONTARIO

DWG NO. TAM 17005333
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T86	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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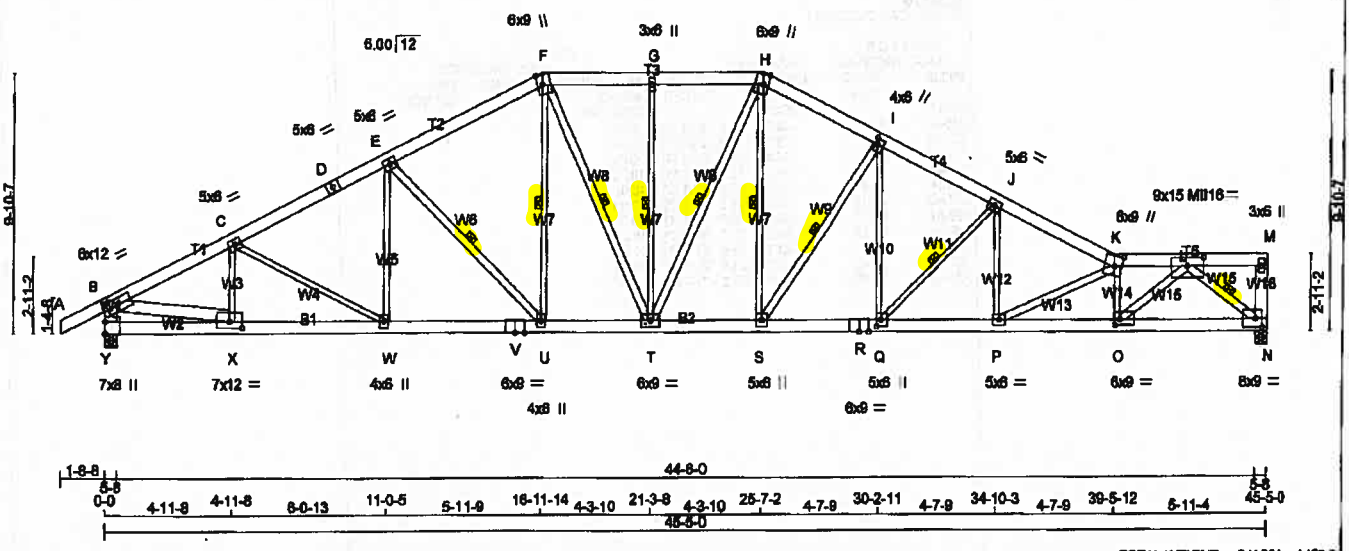

TOTAL WEIGHT = 5 X 291 = 1463 lb

LUMBER

N. L. G. A RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	2100F 1.8E	SPF
V - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - U	2x4	DRY	No.2	SPF
F - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
B - X	2x6	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	12.0	3.00	4.75
C, E, J						
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.25
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.25
I	TMWW+t	MT20	4.0	6.0	3.00	1.00
K	TTWW+m	MT20	6.0	9.0	4.75	3.25
L	TMWW-t	MII16	9.0	15.0	3.75	7.50
M	TMV+p	MT20	3.0	6.0		
N	BMVW1-t	MT20	8.0	9.0	4.00	3.26
O	BMWW-t	MT20	6.0	9.0	2.50	2.25
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0	3.00	2.25
R	BS-t	MT20	6.0	9.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMWWW-t	MT20	6.0	9.0		
U	BMWW+t	MT20	4.0	6.0		
V	BS-t	MT20	6.0	9.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMWW-t	MT20	5.0	12.0	2.75	6.00
Y	BMV1+p	MT20	7.0	8.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4664	0	4807	0	-1048	5-8	5-8
Y	4966	0	4966	325	-1098	5-8	5-8

2-TBE6
2 TBE6

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1048 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT Y FOR 1098 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDING DESIGNER

PROVIDE FOR 325 LBS FACTORED HORIZONTAL REACTION AT JOINT Y

UNFACTORED REACTIONS

MAX/MIN COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3362	2447 / 0	454 / 0	0 / 0	121 / -1099	545 / 0	0 / 0
Y	3657	2540 / 0	454 / 0	0 / 0	154 / -1148	563 / 0	0 / 0

HORIZONTAL REACTIONS

Y	—	0 / 0	0 / 0	0 / 0	232 / -149	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Y

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-U, F-T, G-T, H-T, H-S, I-S, J-Q, L-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 61	-167.9	-167.9	0.23 (2)	10.00
B- C	-5905 / 1528	-167.9	-167.9	0.58 (2)	2.86
C- D	-6896 / 1886	-167.9	-167.9	0.61 (2)	2.72
D- E	-6896 / 1886	-167.9	-167.9	0.61 (2)	2.72
E- F	-5943 / 1447	-167.9	-167.9	0.70 (2)	3.04
F- G	-5846 / 1448	-167.9	-167.9	0.35 (3)	3.44
G- H	-5846 / 1448	-167.9	-167.9	0.35 (3)	3.44
H- I	-6216 / 1560	-167.9	-167.9	0.44 (3)	3.23
I- J	-7500 / 1781	-167.9	-167.9	0.55 (3)	2.86
J- K	-8900 / 2016	-167.9	-167.9	0.70 (3)	2.42
K- L	-9875 / 2139	-167.9	-167.9	0.48 (3)	2.46
L- M	-20 / 52	-167.9	-167.9	0.12 (3)	6.25
N- M	-264 / 82	0.0	0.0	0.03 (14)	7.81
Y- B	-4664 / 1113	0.0	0.0	0.31 (1)	4.91
Y- X	-305 / 169	-37.5	-37.5	0.05 (2)	6.25
X- W	-1498 / 6205	-37.5	-37.5	0.38 (1)	6.25
W- V	-1339 / 6185	-37.5	-37.5	0.34 (1)	6.25
V- U	-1339 / 6185	-37.5	-37.5	0.34 (1)	6.25
U- T	-972 / 5284	-37.5	-37.5	0.29 (1)	6.25
T- S	-904 / 5547	-37.5	-37.5	0.30 (1)	6.25
S- R	-1240 / 6713	-37.5	-37.5	0.35 (1)	6.25
R- Q	-1240 / 6713	-37.5	-37.5	0.35 (1)	6.25
Q- P	-1633 / 7891	-37.5	-37.5	0.43 (1)	6.25
P- O	-2072 / 9829	-37.5	-37.5	0.56 (3)	6.25
O- N	-1181 / 5511	-37.5	-37.5	0.36 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-961 / 309	0.16 (1)
C- W	-269 / 490	0.24 (2)
W- E	-125 / 402	0.09 (5)
E- U	-1650 / 539	0.61 (2)
U- F	-321 / 1381	0.31 (2)
F- T	-356 / 1652	0.27 (3)
T- G	-1019 / 316	0.60 (2)
T- H	-380 / 1153	0.20 (3)
S- H	-514 / 2182	0.49 (3)
S- I	-2437 / 689	0.83 (3)
Q- I	-343 / 1667	0.38 (3)
Q- J	-2067 / 584	0.60 (3)
P- J	-167 / 1194	0.27 (3)
P- K	-2140 / 497	0.70 (3)
O- K	-3638 / 870	0.59 (1)
B- X	-1243 / 6304	0.74 (1)
O- L	-1150 / 5828	0.90 (1)
L- N	-7051 / 1578	0.82 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	51.6	PSF
	DL =	5.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	73.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(60 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/180 (3.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.46")

CSI: TC=0.61/1.00 (C-E:2) , BC=0.59/1.00 (O-P:3) , WB=0.90/1.00 (L-O:1) , SSI=0.39/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (O) (INPUT = 0.90)
MSI METAL= 0.98 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. T86 0536
STRUCTURAL

1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
jpenfold
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T86	5	1	TRUSS DESC.	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(9)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVIS
PROVINCE OF ONTARIO

DWG NO. TAM 71805364
STRUCTURAL
CHECKED BY JV 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G86	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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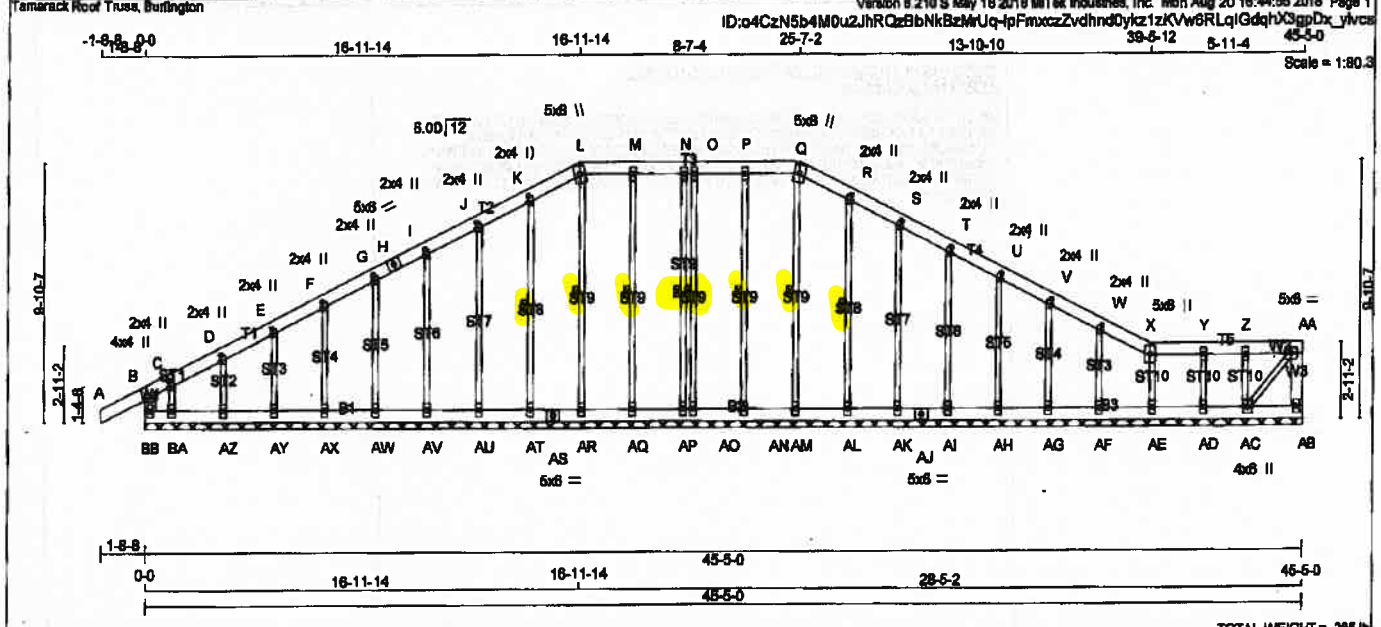

TOTAL WEIGHT = 285 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BB- B	2x6	DRY	No.2	SPF
A - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
L - Q	2x6	DRY	No.2	SPF
Q - X	2x6	DRY	No.2	SPF
X - AA	2x6	DRY	No.2	SPF
AB- AA	2x6	DRY	No.2	SPF
BB- AS	2x6	DRY	No.2	SPF
AS- AJ	2x6	DRY	No.2	SPF
AJ - AB	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0		
C, D, E, G, I, J, K, R, S, T, U						
C	TMW+w	MT20	2.0	4.0	3.00	
F, V, W						
F	TMW+w	MT20	2.0	4.0	2.75	
H	TB-t	MT20	5.0	6.0		
L	TTW+m	MT20	5.0	6.0		
M, N, O, P, Y, Z						
M	TMW+w	MT20	3.0	6.0		
Q	TTW+m	MT20	5.0	6.0		
X	TTW+p	MT20	5.0	6.0		
AA	TMVW-t	MT20	5.0	6.0		
AB	BMV1+p	MT20	3.0	6.0		
AC	BMWW1+t	MT20	4.0	6.0		
AD, AE, AF, AG, AH, AI, AK, AL, AM, AN, AO, AP, AQ, AR, AT, AU, AV, AW, AX, AY, AZ, BA						
AD	BMW1+w	MT20	3.0	6.0		
AJ	BS-t	MT20	5.0	6.0		
AS	BS-t	MT20	5.0	6.0		
BB	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-AM, P-AN, O-AO, N-AP, M-AQ, K-AT, R-AL, L-AR.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
BB- B	-504 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 53	-129.7	-129.7	0.15 (1)	10.00	
B- C	-82 / 0	-129.7	-129.7	0.14 (1)	6.25	
C- D	0 / 6	-129.7	-129.7	0.03 (1)	10.00	
D- E	0 / 10	-129.7	-129.7	0.03 (1)	10.00	
E- F	0 / 19	-129.7	-129.7	0.03 (1)	10.00	
F- G	0 / 26	-129.7	-129.7	0.03 (1)	10.00	
G- H	0 / 31	-129.7	-129.7	0.03 (1)	6.25	
H- I	0 / 31	-129.7	-129.7	0.03 (1)	6.25	
I- J	0 / 37	-129.7	-129.7	0.03 (1)	10.00	
J- K	0 / 41	-129.7	-129.7	0.04 (1)	10.00	
K- L	0 / 33	-129.7	-129.7	0.04 (1)	10.00	
L- M	0 / 42	-129.7	-129.7	0.04 (1)	10.00	
M- N	0 / 42	-129.7	-129.7	0.04 (1)	10.00	
N- O	0 / 42	-129.7	-129.7	0.04 (1)	10.00	
O- P	0 / 42	-129.7	-129.7	0.04 (1)	10.00	
P- Q	0 / 42	-129.7	-129.7	0.04 (1)	10.00	
Q- R	0 / 33	-129.7	-129.7	0.04 (1)	10.00	
R- S	0 / 41	-129.7	-129.7	0.04 (1)	10.00	
S- T	0 / 37	-129.7	-129.7	0.03 (1)	10.00	
T- U	0 / 31	-129.7	-129.7	0.03 (1)	10.00	
U- V	0 / 26	-129.7	-129.7	0.03 (1)	10.00	
V- W	0 / 19	-129.7	-129.7	0.03 (1)	10.00	
W- X	0 / 21	-129.7	-129.7	0.03 (1)	10.00	
X- Y	0 / 18	-129.7	-129.7	0.04 (1)	10.00	
Y- Z	0 / 18	-129.7	-129.7	0.03 (1)	10.00	
Z-AA	0 / 18	-129.7	-129.7	0.04 (1)	10.00	
AB-AA	-104 / 0	0.0	0.0	0.01 (1)	7.81	
BB-BA	0 / 0	-38.5	-38.5	0.01 (3)	10.00	
BA-AZ	0 / 2	-38.5	-38.5	0.01 (3)	10.00	
AZ-AY	-10 / 0	-38.5	-38.5	0.01 (3)	10.00	
AY-AX	-17 / 0	-38.5	-38.5	0.01 (3)	6.25	
AX-AW	-23 / 0	-38.5	-38.5	0.01 (3)	6.25	
AW-AV	-28 / 0	-38.5	-38.5	0.01 (3)	6.25	
AV-AU	-32 / 0	-38.5	-38.5	0.01 (3)	6.25	
AU-AT	-36 / 0	-38.5	-38.5	0.01 (3)	6.25	
AT-AS	-39 / 0	-38.5	-38.5	0.01 (3)	6.25	
AS-AR	-39 / 0	-38.5	-38.5	0.01 (3)	6.25	
AR-AQ	-42 / 0	-38.5	-38.5	0.01 (3)	6.25	
AQ-AP	-42 / 0	-38.5	-38.5	0.01 (3)	6.25	
AP-AO	-42 / 0	-38.5	-38.5	0.01 (3)	6.25	
AO-AN	-42 / 0	-38.5	-38.5	0.01 (3)	6.25	
AN-AM	-42 / 0	-38.5	-38.5	0.01 (3)	6.25	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
AM- Q	-245 / 0	0.15 (1)
AN- P	-272 / 0	0.16 (1)
AO- O	-166 / 0	0.10 (1)
AP- N	-166 / 0	0.10 (1)
AQ- M	-273 / 0	0.16 (1)
AT- K	-282 / 0	0.13 (1)
AU- J	-256 / 0	0.27 (1)
AV- I	-256 / 0	0.16 (1)
AW- G	-257 / 0	0.12 (1)
AX- F	-256 / 0	0.08 (1)
AY- E	-254 / 0	0.05 (1)
AZ- D	-274 / 0	0.04 (1)
BA- C	0 / 29	0.01 (1)
AL- R	-282 / 0	0.13 (1)
AK- S	-258 / 0	0.27 (1)
AI- T	-256 / 0	0.16 (1)
AH- U	-257 / 0	0.12 (1)
AG- V	-255 / 0	0.08 (1)
AF- W	-275 / 0	0.06 (1)
AE- X	-207 / 0	0.03 (1)
AD- Y	-263 / 0	0.04 (1)
AC- Z	-269 / 0	0.04 (1)
AC-AA	-27 / 0	0.00 (1)
AR- L	-345 / 0	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN OC

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(85 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (AC-AD:3) , WB=0.27/1.00 (J-AU:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) (INPUT = 0.90)
JSI METAL= 0.12 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 18005334
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G86	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AM-AL	-39 / 0	-38.5	-38.5	0.01	(3)	6.25
AL-AK	-38 / 0	-38.5	-38.5	0.01	(3)	6.25
AK-AJ	-32 / 0	-38.5	-38.5	0.01	(3)	6.25
AJ-AI	-32 / 0	-38.5	-38.5	0.01	(3)	6.25
AI-AH	-26 / 0	-38.5	-38.5	0.01	(3)	6.25
AH-AG	-23 / 0	-38.5	-38.5	0.01	(3)	6.25
AG-AF	-17 / 0	-38.5	-38.5	0.01	(3)	6.25
AF-AE	-9 / 0	-38.5	-38.5	0.01	(3)	10.00
AE-AD	-16 / 0	-38.5	-38.5	0.01	(3)	6.25
AD-AC	-16 / 0	-38.5	-38.5	0.02	(3)	6.25
AC-AB	0 / 0	-38.5	-38.5	0.02	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. JONES
PROVINCE OF ONTARIO

DWG NO. TAM H8305334
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T162	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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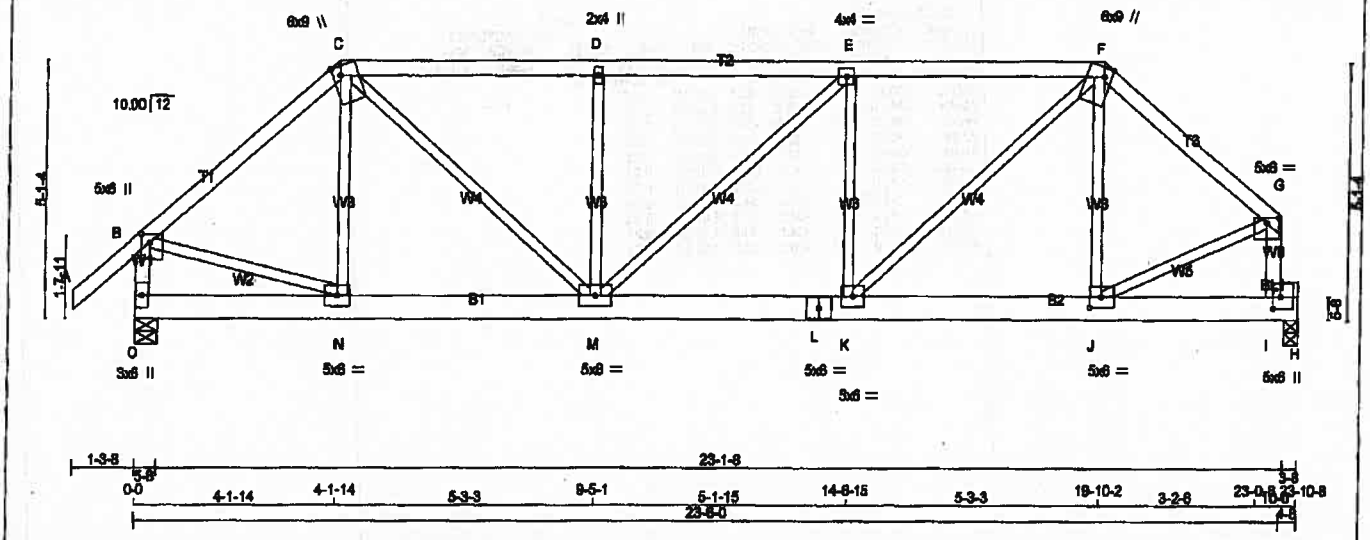

TOTAL WEIGHT = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES Gable is in inches

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	6.0	9.0	Edge	1.75
G	TMWW-p	MT20	5.0	6.0	1.25	3.00
I	BMVK+p	MT20	5.0	6.0	2.75	1.75
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	FACTORED BRG IN-SX	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2167	0	2167	0	0	5-8	5-8	5-8
H	1959	0	1959	0	0	3-8	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1599	1016 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
H	1441	884 / 0	251 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY REBTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B- C	-1943 / 0	-129.7	-129.7	0.48 (1)	4.30	
C- D	-2495 / 0	-129.7	-129.7	0.61 (1)	3.69	
D- E	-2498 / 0	-129.7	-129.7	0.62 (1)	3.66	
E- F	-2479 / 0	-129.7	-129.7	0.63 (1)	3.67	
F- G	-1951 / 0	-129.7	-129.7	0.37 (1)	4.43	
O- B	-2124 / 0	0.0	0.0	0.22 (1)	5.81	
I- G	-2238 / 0	0.0	0.0	0.25 (1)	5.69	
O- N	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
N- M	0 / 1464	-38.5	-38.5	0.23 (1)	10.00	
M- L	0 / 2479	-38.5	-38.5	0.37 (1)	10.00	
L- K	0 / 2479	-38.5	-38.5	0.37 (1)	10.00	
K- J	0 / 1491	-38.5	-38.5	0.37 (1)	10.00	
J- I	-198 / 0	-38.5	-38.5	0.68 (1)	6.26	
I- H	0 / 0	-38.5	-38.5	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
N- C	-214 / 91	0.08 (1)
C- M	0 / 1357	0.31 (1)
M- D	-727 / 0	0.26 (1)
M- E	0 / 23	0.01 (1)
K- E	-747 / 0	0.27 (1)
K- F	0 / 1326	0.30 (1)
J- F	-153 / 104	0.05 (1)
B- N	0 / 1548	0.35 (1)
J- G	0 / 1846	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.68/1.00 (I-J:1) , WB=0.42/1.00 (G-J:1) , SSI=0.72/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	364	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.47 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALWES
PROVINCE OF ONTARIO

DWG NO. TAM 71805240
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Ready

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T163	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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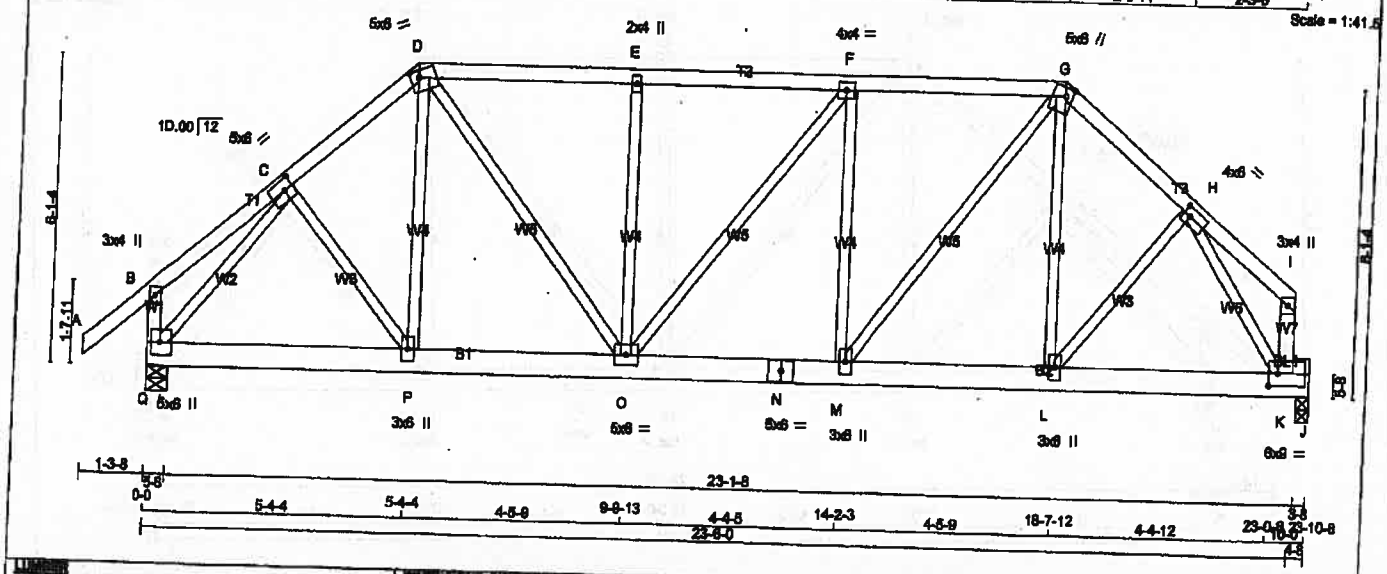

TOTAL WEIGHT = 128 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Middle is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.75
H	TMWW-t	MT20	4.0	6.0	2.00	1.75
I	TMV+p	MT20	3.0	4.0		
K	BMWK-t	MT20	6.0	9.0	3.00	2.25
L, M, P						
L	BMWW+t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
Q	BMWW1+p	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2187	0	2187	0	0	5-8	5-8
J	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1698	1016 / 0	261 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1441	864 / 0	261 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 57	-128.7	-128.7	0.18 (1)	10.00
B- C	0 / 21	-128.7	-128.7	0.12 (1)	10.00
C- D	-1951 / 0	-128.7	-128.7	0.17 (1)	4.68
D- E	-2088 / 0	-128.7	-128.7	0.40 (1)	4.28
E- F	-2088 / 0	-128.7	-128.7	0.41 (1)	4.25
F- G	-2054 / 0	-128.7	-128.7	0.41 (1)	4.27
G- H	-1972 / 0	-128.7	-128.7	0.18 (1)	4.67
H- I	0 / 33	-128.7	-128.7	0.10 (1)	10.00
Q- B	-318 / 0	0.0	0.0	0.08 (1)	7.81
K- I	-102 / 0	0.0	0.0	0.01 (1)	7.81
Q- P	0 / 1375	-38.5	-38.5	0.23 (1)	10.00
P- O	0 / 1478	-38.5	-38.5	0.23 (1)	10.00
O- N	0 / 2054	-38.5	-38.5	0.31 (1)	10.00
N- M	0 / 2054	-38.5	-38.5	0.31 (1)	10.00
M- L	0 / 1498	-38.5	-38.5	0.40 (1)	10.00
L- K	0 / 1187	-38.5	-38.5	0.82 (1)	10.00
K- J	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 171	0.04 (2)
P- D	0 / 173	0.04 (3)
L- G	0 / 198	0.05 (3)
L- H	0 / 494	0.11 (1)
Q- C	-2249 / 0	0.66 (1)
H- K	-2953 / 0	0.82 (1)
M- G	0 / 902	0.20 (1)
D- O	0 / 957	0.22 (1)
M- F	-637 / 0	0.35 (1)
O- E	-612 / 0	0.33 (1)
O- F	0 / 23	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.41/1.00-(F-G:1) , BC=0.82/1.00 (K-L:1) , WB=0.66/1.00 (C-Q:1) , SSI=0.72/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MIN	MAX	SECTION (PLI) MIN	MAX
MT20	518	354	1667	788	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17805241
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T164	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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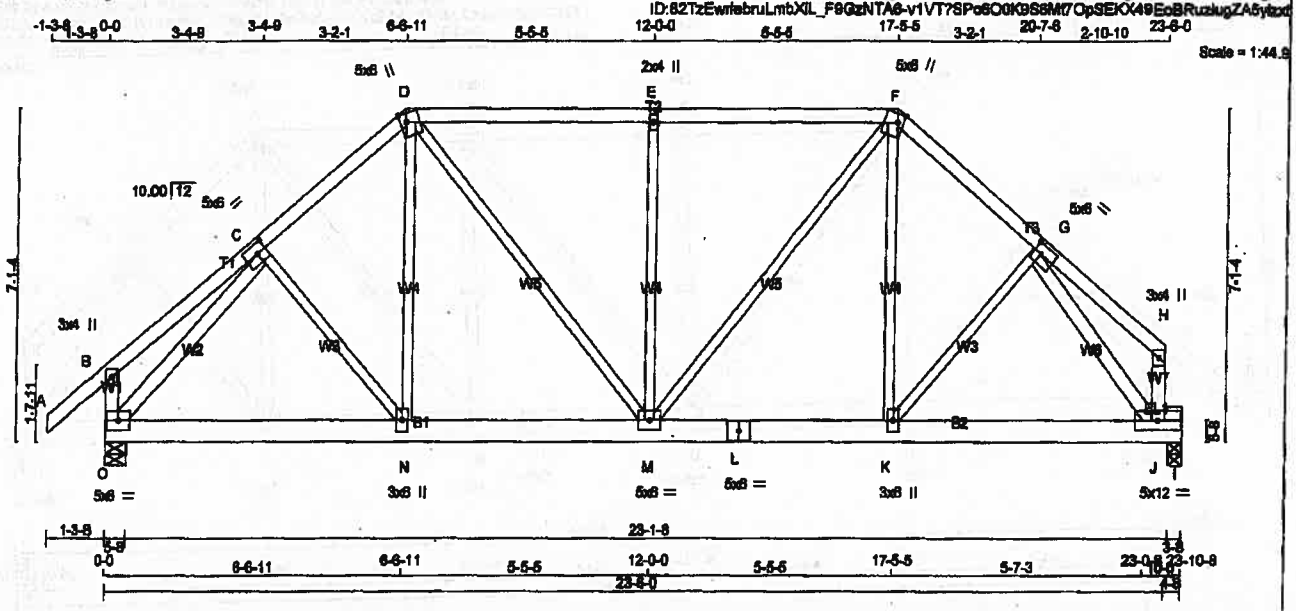

TOTAL WEIGHT = 127 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Article is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW+t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW+t	MT20	5.0	6.0	2.50	2.00
H	TMW+p	MT20	3.0	4.0		
J	BMWWK+t	MT20	5.0	12.0		
K	BMWW+t	MT20	3.0	6.0		
L	BS+t	MT20	5.0	6.0		
M	BMWWW+t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMWW+t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2167	0	2167	0	0	5-8	5-8
I	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1596	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
I	1441	884 / 0	251 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B- C	0 / 30	-129.7	-129.7	0.21 (1)	10.00	
C- D	-1918 / 0	-129.7	-129.7	0.27 (1)	4.60	
D- E	-1841 / 0	-129.7	-129.7	0.75 (1)	3.85	
E- F	-1841 / 0	-129.7	-129.7	0.75 (1)	3.85	
F- G	-1937 / 0	-129.7	-129.7	0.24 (1)	4.61	
G- H	0 / 48	-129.7	-129.7	0.19 (1)	10.00	
O- B	-343 / 0	0.0	0.0	0.04 (1)	7.81	
J- H	-121 / 0	0.0	0.0	0.01 (1)	7.81	
O- N	0 / 1438	-38.5	-38.5	0.28 (1)	10.00	
N- M	0 / 1449	-38.5	-38.5	0.28 (2)	10.00	
M- L	0 / 1469	-38.5	-38.5	0.44 (1)	10.00	
L- K	0 / 1469	-38.5	-38.5	0.44 (1)	10.00	
K- J	0 / 1273	-38.5	-38.5	0.63 (1)	10.00	
J- I	0 / 0	-38.5	-38.5	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 100	0.02 (5)
N- D	0 / 322	0.07 (2)
D- M	0 / 623	0.14 (1)
M- E	-661 / 0	0.71 (1)
M- F	0 / 592	0.13 (1)
K- F	0 / 348	0.08 (2)
K- G	0 / 290	0.07 (1)
O- C	-2287 / 0	0.69 (1)
G- J	-2325 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.63/1.00 (J-K:1) , WB=0.71/1.00 (E-M:1) , SSI=0.72/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1967	1558

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.80)
JSI METAL= 0.58 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 11805242
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T165	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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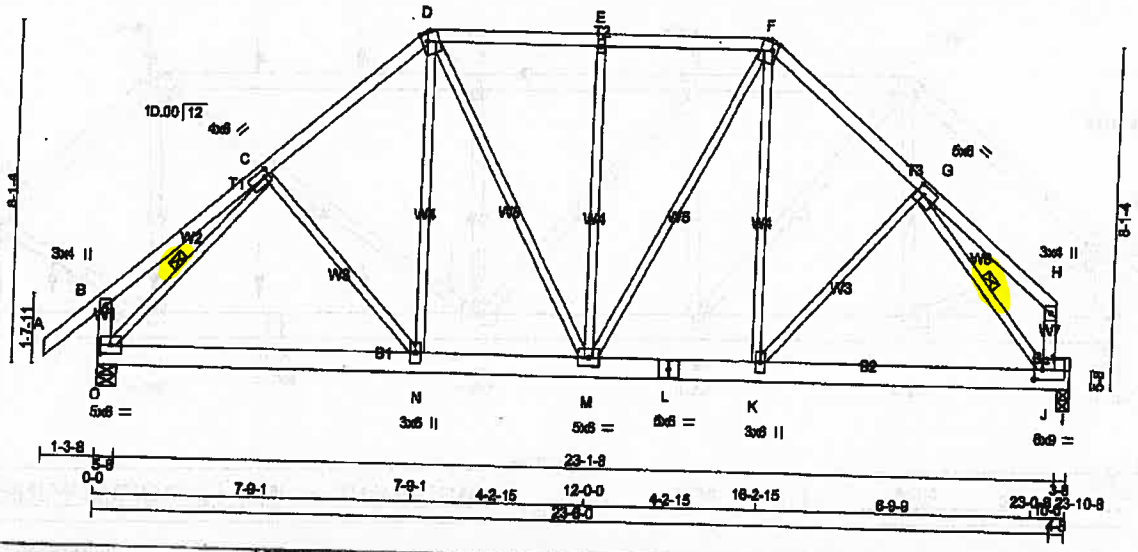

TOTAL WEIGHT = 130 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Units is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW+t	MT20	5.0	6.0	2.50	2.25
H	TMW+p	MT20	3.0	4.0		
J	BMWW+t	MT20	6.0	9.0	2.75	2.50
K	BMWW+t	MT20	3.0	6.0		
L	BS+t	MT20	5.0	6.0		
M	BMWWW+t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMWW+t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-6X	REQRD BRG IN-6X
O	2157	0	2157	0	0	5-8	5-8
I	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX/MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1596	1018 / 0	291 / 0	0 / 0	0 / 0	327 / 0	0 / 0
I	1441	884 / 0	251 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.15 (1)	10.00
B- C	0 / 37	-129.7	-129.7	0.31 (1)	10.00
C- D	-1845 / 0	-129.7	-129.7	0.38 (1)	4.53
D- E	-1571 / 0	-129.7	-129.7	0.42 (1)	4.72
E- F	-1571 / 0	-129.7	-129.7	0.42 (1)	4.72
F- G	-1874 / 0	-129.7	-129.7	0.34 (1)	4.55
G- H	0 / 54	-129.7	-129.7	0.27 (1)	10.00
O- B	-372 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-146 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 1468	-38.5	-38.5	0.32 (2)	10.00
N- M	0 / 1391	-38.5	-38.5	0.31 (2)	10.00
M- L	0 / 1417	-38.5	-38.5	0.49 (1)	10.00
L- K	0 / 1417	-38.5	-38.5	0.49 (1)	10.00
K- J	0 / 1332	-38.5	-38.5	0.84 (1)	10.00
J- I	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-122 / 68	0.07 (1)
N- D	0 / 445	0.10 (2)
D- M	0 / 373	0.08 (1)
M- E	-663 / 0	0.79 (1)
M- F	0 / 319	0.07 (1)
K- F	0 / 492	0.11 (2)
K- G	0 / 147	0.03 (2)
O- C	-2237 / 0	0.54 (1)
G- J	-2288 / 0	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2015
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.84/1.00 (J-K:1) , WB=0.79/1.00 (E-M:1) , SSI=0.72/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.59 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ADES
PROVINCE OF ONTARIO

DWG NO. TAM 17805243
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T170	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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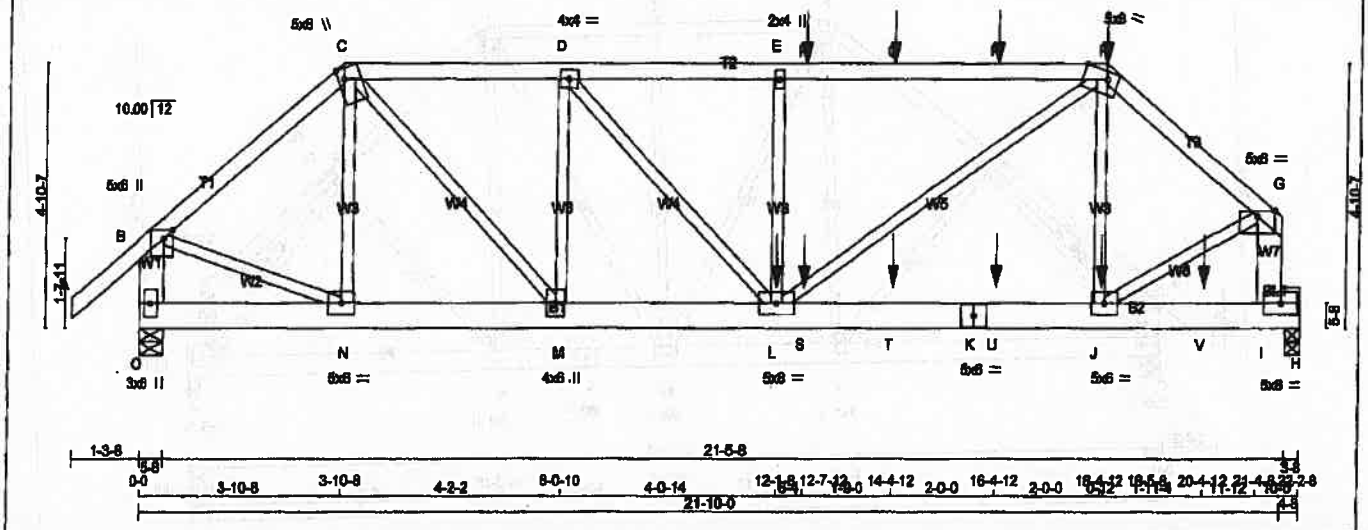

TOTAL WEIGHT = 2 X 109 = 219 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	SIDE(61.0)
F-G	1	12	SIDE(61.0)
O-B	2	12	TOP
I-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-K	2	12	SIDE(183.1)
K-H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	8	
E-L	1	8	SIDE(253.6)
2x4	1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	Edge 1.25	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge 5.50	
G	TMWW-p	MT20	5.0	6.0	Edge	
I	BMWK-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	3010	0	3010	0	0	5-8	5-8
H	3355	0	3355	0	0	3-8	3-8

UNFACTORED REACTIONS

MAX.MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2186	1435 / 0	327 / 0	0 / 0	0 / 0	424 / 0	0 / 0
H	2444	1590 / 0	368 / 0	0 / 0	0 / 0	486 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.10 (1)	10.00	
B-C	-2867 / 0	-129.7	-129.7	0.23 (1)	5.18	
C-D	-3891 / 0	-129.7	-129.7	0.24 (1)	4.57	
D-E	-4973 / 0	-129.7	-129.7	0.59 (1)	3.81	
E-P	-4972 / 0	-129.7	-129.7	0.75 (1)	3.53	
P-Q	-4972 / 0	-129.7	-129.7	0.75 (1)	3.53	
Q-R	-4972 / 0	-129.7	-129.7	0.75 (1)	3.53	
R-F	-4972 / 0	-129.7	-129.7	0.75 (1)	3.53	
F-G	-3476 / 0	-129.7	-129.7	0.19 (1)	4.85	
O-B	-2948 / 0	0.0	0.0	0.11 (1)	7.81	
I-G	-4008 / 0	0.0	0.0	0.16 (1)	7.10	
O-N	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
N-M	0 / 2185	-38.5	-38.5	0.16 (1)	10.00	
M-L	0 / 3891	-38.5	-38.5	0.33 (1)	10.00	
L-S	0 / 2653	-38.5	-38.5	0.38 (1)	10.00	
S-T	0 / 2653	-38.5	-38.5	0.38 (1)	10.00	
T-K	0 / 2653	-38.5	-38.5	0.38 (1)	10.00	
K-U	0 / 2653	-38.5	-38.5	0.38 (1)	10.00	
U-J	0 / 2653	-38.5	-38.5	0.38 (1)	10.00	
J-V	-555 / 0	-38.5	-38.5	0.74 (1)	6.25	
V-I	-555 / 0	-38.5	-38.5	0.74 (1)	6.25	
I-H	0 / 0	-168.2	-168.2	0.48 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-503 / 0	0.09 (1)
C-M	0 / 2500	0.31 (1)
M-D	-1606 / 0	0.27 (1)
D-L	0 / 1611	0.20 (1)
L-E	-1110 / 0	0.19 (1)
L-F	0 / 2639	0.35 (1)
J-F	-369 / 133	0.05 (1)
B-N	0 / 2239	0.29 (1)
J-G	0 / 3539	0.44 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	18-5-8	-195	-195	—	FRONT	VERT	TOTAL
J	18-4-12	-26	-26	—	FRONT	VERT	TOTAL
L	12-1-8	-1202	-1202	—	FRONT	VERT	TOTAL
P	12-7-12	-63	-63	—	FRONT	VERT	TOTAL
Q	14-4-12	-63	-63	—	FRONT	VERT	TOTAL
R	16-4-12	-63	-63	—	FRONT	VERT	TOTAL
S	12-7-12	-21	-21	—	FRONT	VERT	TOTAL
T	14-4-12	-21	-21	—	FRONT	VERT	TOTAL
U	16-4-12	-21	-21	—	FRONT	VERT	TOTAL
V	20-4-12	-11	-16	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN.O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.74/1.00 (I-J:1) , WB=0.44/1.00 (G-J:1) , SSI=0.88/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.00 (F) (INPUT = 0.90)
JSI METAL= 0.40 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805240
STRUCTURAL
COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
jpenfold
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T170	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.8 lbs FACTORED DOWN AT 12-7-12, 121.8 lbs FACTORED DOWN AT 14-4-12, AND 121.6 lbs FACTORED DOWN AT 16-4-12, AND 281.9 lbs FACTORED DOWN AT 18-5-8 ON TOP CHORD, AND 1874.1 lbs FACTORED DOWN AT 12-1-8, 30.9 lbs FACTORED DOWN AT 12-7-12, 30.9 lbs FACTORED DOWN AT 14-4-12, 30.6 lbs FACTORED DOWN AT 16-4-12, AND 38.0 lbs FACTORED DOWN AT 18-4-12, AND 23.7 lbs FACTORED DOWN AT 20-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/30/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805240
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T171	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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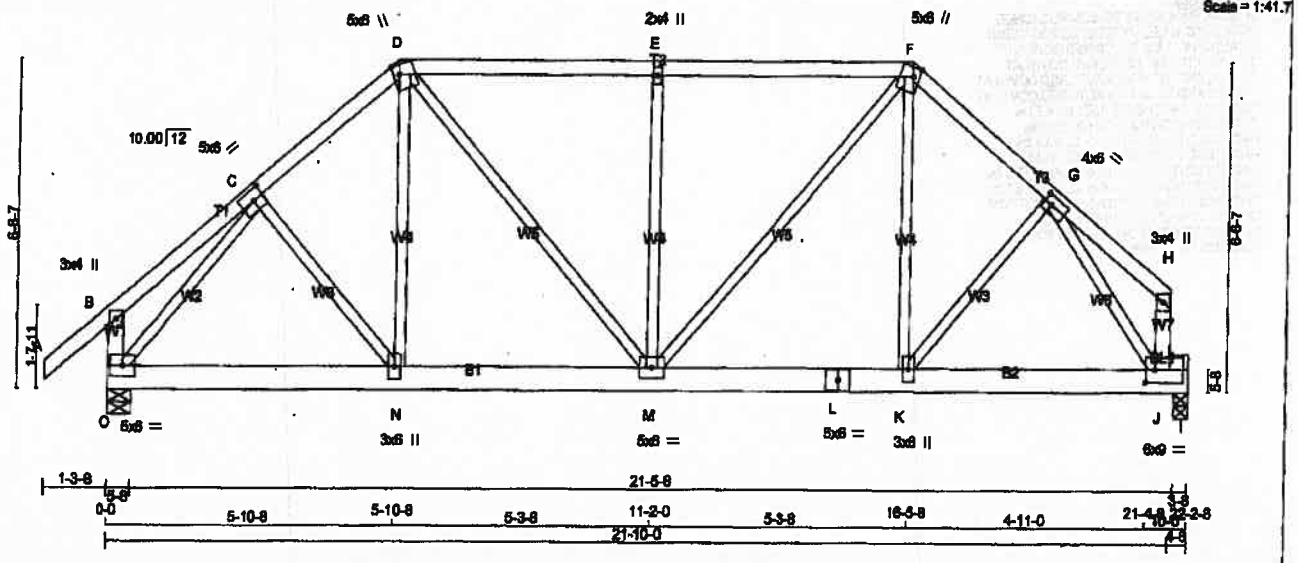

TOTAL WEIGHT = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMWW-t	MT20	6.0	9.0	3.00	2.25
K	BMWW+t	MT20	3.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWWW+t	MT20	3.0	6.0		
O	BMWW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
O	2046	0	2046	0	0	5-8	5-8
I	1819	0	1819	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LOADE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1493	854 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
I	1338	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B-C	0 / 25	-129.7	-129.7	0.16 (1)	10.00
C-D	-1768 / 0	-129.7	-129.7	0.20 (1)	4.83
D-E	-1747 / 0	-129.7	-129.7	0.70 (1)	4.04
E-F	-1747 / 0	-129.7	-129.7	0.70 (1)	4.04
F-G	-1784 / 0	-129.7	-129.7	0.17 (1)	4.84
G-H	0 / 38	-129.7	-129.7	0.14 (1)	10.00
O-B	-329 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-112 / 0	0.0	0.0	0.01 (1)	7.81
O-N	0 / 1298	-38.5	-38.5	0.24 (1)	10.00
N-M	0 / 1337	-38.5	-38.5	0.24 (1)	10.00
M-L	0 / 1354	-38.5	-38.5	0.39 (1)	10.00
L-K	0 / 1354	-38.5	-38.5	0.39 (1)	10.00
K-J	0 / 1133	-38.5	-38.5	0.73 (1)	10.00
J-I	0 / 0	-38.5	-38.5	0.39 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	0 / 107	0.03 (3)
N-D	0 / 262	0.08 (2)
D-M	0 / 629	0.14 (1)
M-E	-536 / 0	0.55 (1)
M-F	0 / 602	0.14 (1)
K-F	0 / 288	0.09 (2)
K-G	0 / 341	0.08 (1)
O-C	-2077 / 0	0.70 (1)
G-J	-2154 / 0	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	38.2	PSF
	DL.	=	6.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL.	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART B OF OBC 2012, OBC 2016
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.76/1.00 (J-K:1) , WB=0.70/1.00 (C-O:1) , SSI=0.67/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	615	354	1867	788	1867	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805249
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T172	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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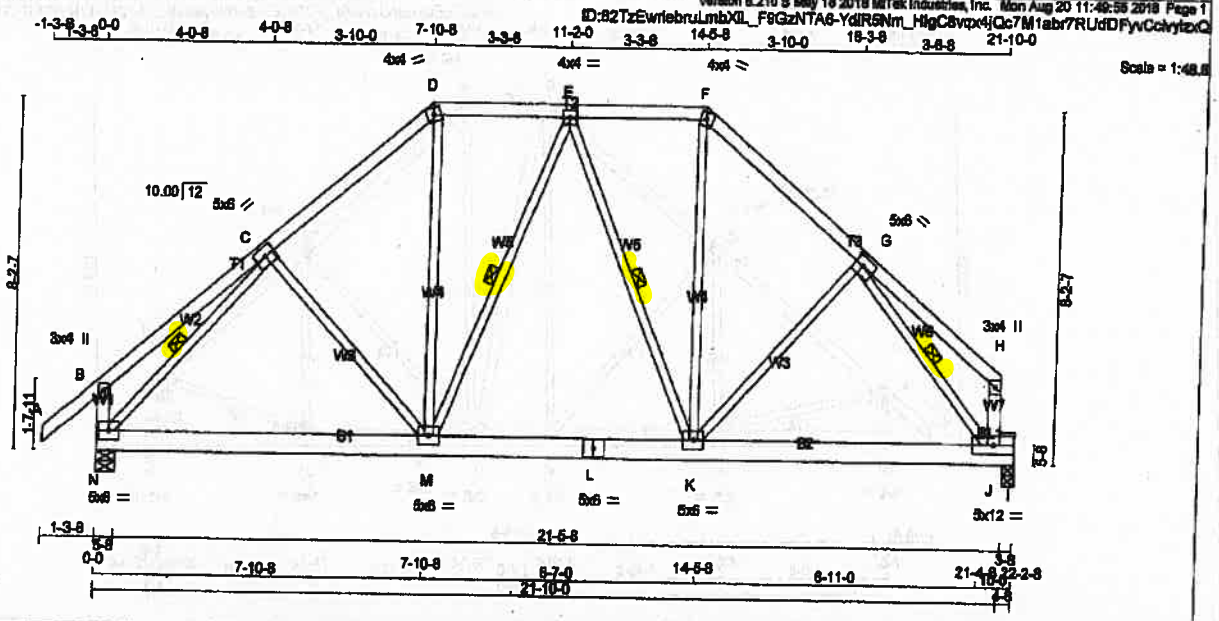

TOTAL WEIGHT = 119 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	12.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2046	0	2046	0	0	5-8	5-8
I	1619	0	1619	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1498	954 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
I	1338	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.16 (1)	10.00	
B- C	0 / 37	-129.7	-129.7	0.32 (1)	10.00	
C- 0	-1640 / 0	-129.7	-129.7	0.38 (1)	4.75	
D- E	-1238 / 0	-129.7	-129.7	0.23 (1)	5.49	
E- F	-1284 / 0	-129.7	-129.7	0.23 (1)	6.44	
F- G	-1671 / 0	-129.7	-129.7	0.34 (1)	4.77	
G- H	0 / 55	-129.7	-129.7	0.28 (1)	10.00	
N- B	-375 / 0	0.0	0.0	0.04 (1)	7.81	
J- H	-151 / 0	0.0	0.0	0.02 (1)	7.81	
N- M	0 / 1338	-38.5	-38.5	0.33 (2)	10.00	
M- L	0 / 1353	-38.5	-38.5	0.44 (1)	10.00	
L- K	0 / 1353	-38.5	-38.5	0.44 (1)	10.00	
K- J	0 / 1219	-38.5	-38.5	0.77 (1)	10.00	
J- I	0 / 0	-38.5	-38.5	0.39 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-168 / 54	0.10 (1)
M- D	0 / 868	0.15 (1)
M- E	-308 / 0	0.15 (1)
E- K	-235 / 0	0.11 (1)
K- F	0 / 671	0.15 (1)
K- G	0 / 121	0.03 (3)
N- C	-2037 / 0	0.50 (1)
G- J	-2094 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.77/1.00 (J-K:1) , WB=0.50/1.00 (C-N:1) , SSI=0.67/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 71805250
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T173	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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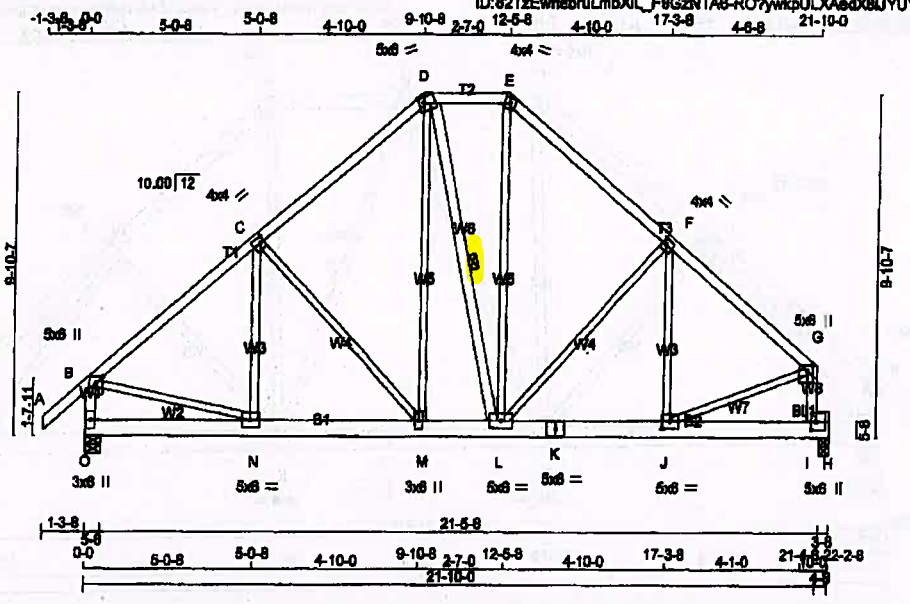

TOTAL WEIGHT = 180 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMWW+p	MT20	5.0	6.0	2.00	2.00
I	BMVK+p	MT20	5.0	6.0	2.75	1.75
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWWW+t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2046	0	2046	0	0	5-8	5-8
H	1616	0	1619	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
O	1493	954 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1338	820 / 0	233 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.26 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.16 (1)	10.00	
B- C	-1603 / 0	-129.7	-129.7	0.67 (1)	4.16	
C- D	-1459 / 0	-129.7	-129.7	0.62 (1)	4.66	
D- E	-1080 / 0	-129.7	-129.7	0.16 (1)	5.66	
E- F	-1449 / 0	-129.7	-129.7	0.55 (1)	4.71	
F- G	-1609 / 0	-129.7	-129.7	0.68 (1)	4.27	
O- B	-1980 / 0	0.0	0.0	0.20 (1)	6.01	
I- G	-1994 / 0	0.0	0.0	0.22 (1)	5.97	
O- N	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
N- M	0 / 1422	-38.5	-38.5	0.22 (1)	10.00	
M- L	0 / 1083	-38.5	-38.5	0.14 (1)	10.00	
L- K	0 / 1427	-38.5	-38.5	0.34 (1)	10.00	
K- J	0 / 1427	-38.5	-38.5	0.34 (1)	10.00	
J- I	-177 / 0	-38.5	-38.5	0.63 (1)	6.26	
I- H	0 / 0	-38.5	-38.5	0.39 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
N- C	-134 / 182	0.07 (1)
C- M	-512 / 0	0.53 (1)
M- D	0 / 512	0.12 (1)
D- L	-12 / 5	0.01 (1)
L- E	0 / 491	0.11 (1)
L- F	-523 / 0	0.54 (1)
J- F	-79 / 166	0.04 (1)
B- N	0 / 1463	0.33 (1)
J- G	0 / 1705	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.63/1.00 (I-J:1) , WB=0.54/1.00 (F-L:1) , SSI=0.67/1.00 (H-I:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.68 (J) (INPUT = 0.90)
MSI METAL= 0.40 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALTES
PROVINCE OF ONTARIO

DWG NO. TAN T1805251
STRUCTURAL
CONFIGURATION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T174	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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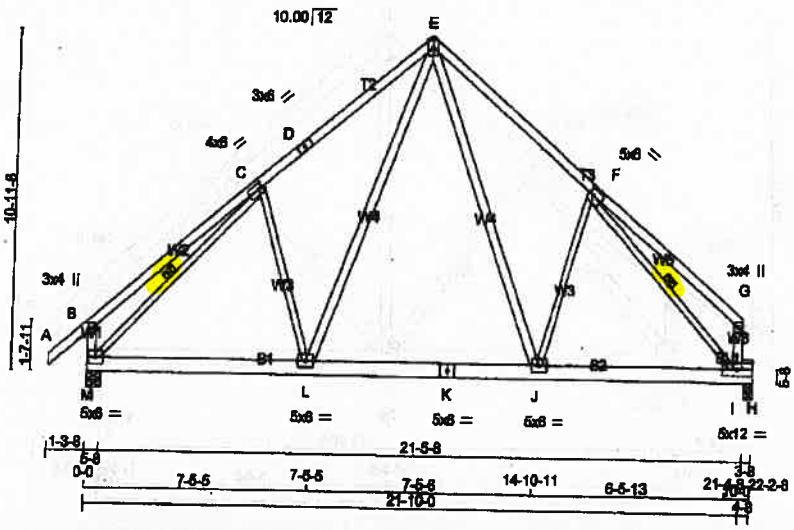

TOTAL WEIGHT = 124 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TB-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	5.0	6.0	2.50	2.00
G	TMV+p	MT20	3.0	4.0		
1	BMWW-t	MT20	5.0	12.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2046	0	2046	0	0	5-8	5-8
H	1819	0	1819	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1493	954 / 0	233 / 0	0 / 0	0 / 0	306 / 0	0 / 0
H	1336	820 / 0	233 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B-C	0 / 58	-129.7	-129.7	0.67 (1)	10.00
C-D	-1662 / 0	-129.7	-129.7	0.67 (1)	3.92
D-E	-1662 / 0	-129.7	-129.7	0.67 (1)	3.92
E-F	-1704 / 0	-129.7	-129.7	0.80 (1)	4.05
F-G	0 / 82	-129.7	-129.7	0.62 (1)	10.00
M-B	-451 / 0	0.0	0.0	0.05 (1)	7.81
I-G	-222 / 0	0.0	0.0	0.02 (1)	7.81
M-L	0 / 1396	-38.5	-38.5	0.30 (1)	10.00
L-K	0 / 982	-38.5	-38.5	0.37 (1)	10.00
K-J	0 / 982	-38.5	-38.5	0.37 (1)	10.00
J-I	0 / 1360	-38.5	-38.5	0.79 (1)	10.00
I-H	0 / 0	-38.5	-38.5	0.39 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-491 / 6	0.32 (1)
L-E	0 / 766	0.13 (1)
E-J	0 / 847	0.14 (1)
J-F	-302 / 67	0.20 (1)
M-C	-2028 / 0	0.88 (1)
F-I	-2148 / 0	0.84 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN.OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/240 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 699 (0.22")

CSI: TC=0.67/1.00 (C-E:1) , BC=0.79/1.00 (I-L:1) , WB=0.88/1.00 (C-M:1) , SSI=0.67/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.58 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AMES
PROVINCE OF ONTARIO

DWG NO. TAM T1805252
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plans

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T176	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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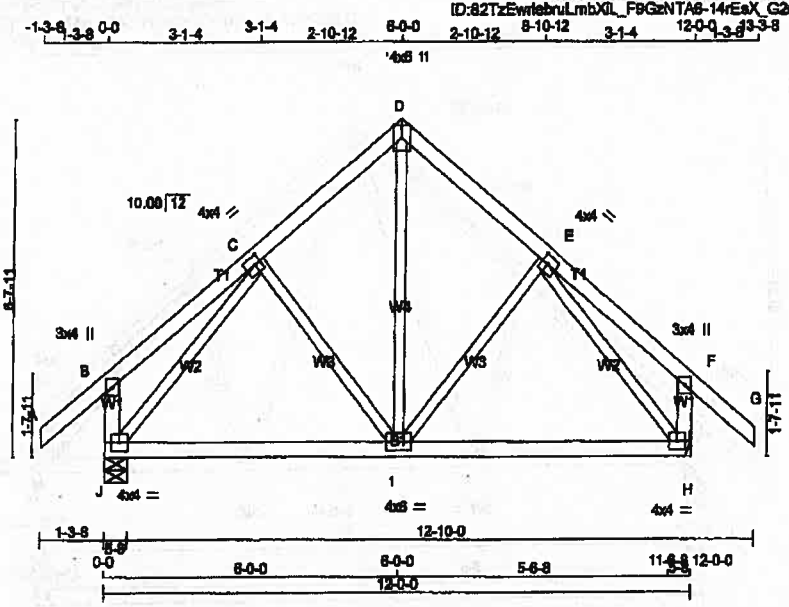

TOTAL WEIGHT = 2 X 58 = 116 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMW+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQ'D BRG
J	1189	0	1189	0	0	5-8	5-8
H	1189	0	1189	0	0	MECHANICAL	

— LS526DS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	893	595 / 0	128 / 0	0 / 0	0 / 0	173 / 0	0 / 0
H	893	595 / 0	128 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B- C	0 / 28	-129.7	-129.7	0.18 (1)	10.00	
C- D	-566 / 0	-129.7	-129.7	0.14 (1)	6.25	
D- E	-566 / 0	-129.7	-129.7	0.14 (1)	6.25	
E- F	0 / 28	-129.7	-129.7	0.18 (1)	10.00	
F- G	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
J- B	-329 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-329 / 0	0.0	0.0	0.03 (1)	7.81	
J-1	0 / 606	-38.5	-38.5	0.37 (2)	10.00	
I-H	0 / 606	-38.5	-38.5	0.37 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 535	0.12 (1)
I- E	-160 / 27	0.08 (1)
C- I	-160 / 27	0.08 (1)
J- C	-681 / 0	0.36 (1)
E- H	-681 / 0	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	39.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.37/1.00 (H-I:2) , WB=0.36/1.00 (C-J:1) , SSI=0.15/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	788	1987	1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALKES
PROVINCE OF ONTARIO

DWG NO. TAM 17605255
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stage

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T181X	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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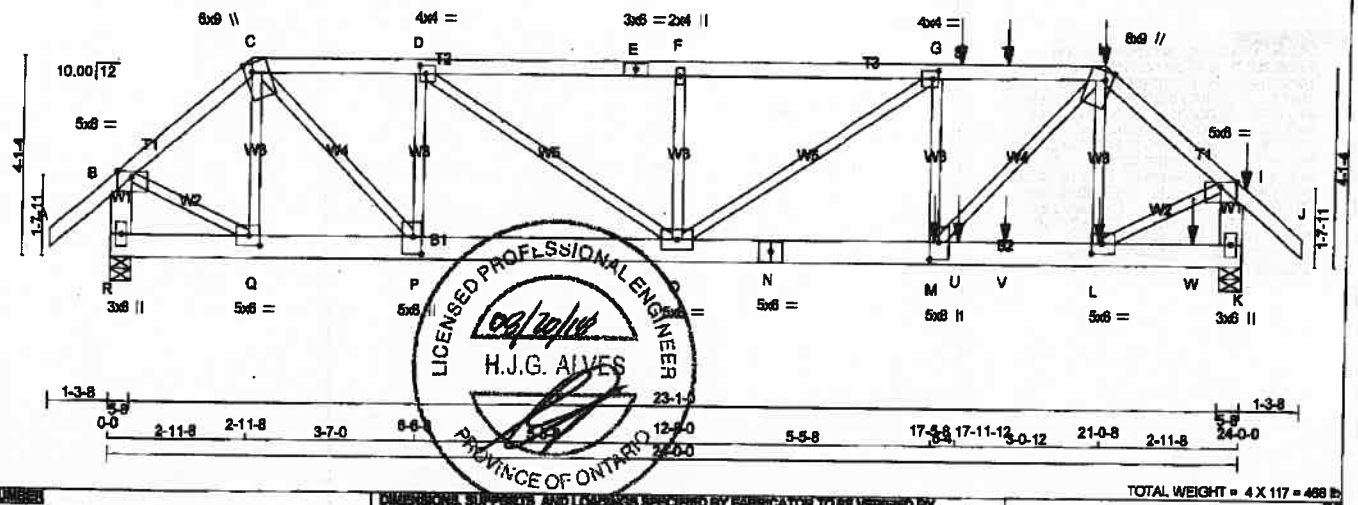

TOTAL WEIGHT = 4 X 117 = 468 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K-1	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	8	
G- M	1	4	SIDE(370.5)
D- P	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Units is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TTWWH-m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TTWWH-m	MT20	8.0	8.0	Edge	1.75
1	TMWW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWH-t	MT20	5.0	8.0	4.25	2.25
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWWH-t	MT20	5.0	6.0	4.25	2.25
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2939	0	2939	0	0	5-8	5-8
K	5012	0	5012	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2141	1378 / 0	328 / 0	0 / 0	0 / 0	438 / 0	0 / 0
K	3832	2398 / 0	512 / 0	0 / 0	0 / 0	723 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- S	0 / 57	-129.7	-129.7	0.10 (1)	10.00
B- C	-2665 / 0	-129.7	-129.7	0.13 (1)	5.44
C- D	-3952 / 0	-129.7	-129.7	0.26 (1)	4.51
D- E	-5710 / 0	-129.7	-129.7	0.42 (1)	3.75
E- F	-5710 / 0	-129.7	-129.7	0.42 (1)	3.75
F- G	-5710 / 0	-129.7	-129.7	0.42 (1)	3.75
G- S	-5094 / 0	-129.7	-129.7	0.37 (1)	3.63
S- T	-5094 / 0	-129.7	-129.7	0.37 (1)	3.63
T- H	-5094 / 0	-129.7	-129.7	0.37 (1)	3.63
H- I	-4369 / 0	-129.7	-129.7	0.18 (1)	4.43
I- J	0 / 57	-129.7	-129.7	0.10 (1)	10.00
R- B	-2901 / 0	0.0	0.0	0.11 (1)	7.61
K- I	-4958 / 0	0.0	0.0	0.18 (1)	8.53
R- Q	0 / 0	-38.5	-38.5	0.02 (2)	10.00
Q- P	0 / 2021	-38.5	-38.5	0.15 (1)	10.00
P- O	0 / 3953	-38.5	-38.5	0.31 (1)	10.00
O- N	0 / 6095	-38.5	-38.5	0.48 (1)	10.00
N- M	0 / 6095	-38.5	-38.5	0.48 (1)	10.00
M- U	0 / 3316	-38.5	-38.5	0.27 (1)	10.00
U- V	0 / 3316	-38.5	-38.5	0.27 (1)	10.00
V- L	0 / 3316	-38.5	-38.5	0.27 (1)	10.00
L- W	0 / 0	-38.5	-38.5	0.05 (2)	10.00
W- K	0 / 0	-38.5	-38.5	0.05 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-658 / 0	0.08 (1)
C- P	0 / 2778	0.34 (1)
P- D	-1827 / 0	0.23 (1)
D- O	0 / 2128	0.26 (1)
O- F	-716 / 0	0.09 (1)
O- G	-466 / 0	0.18 (1)
M- G	-591 / 0	0.07 (1)
M- H	0 / 3967	0.49 (1)
L- H	-992 / 0	0.13 (1)
B- Q	0 / 2196	0.27 (1)
L- I	0 / 3600	0.45 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	21-0-8	-28	-28	—	FRONT	VERT	DEAD
H	21-0-8	-112	-112	—	BACK	VERT	TOTAL
H	21-0-8	-178	-178	—	FRONT	VERT	SNOW
1	23-11-12	-303	-303	—	TOP	VERT	TOTAL
L	20-11-12	-50	-50	—	BACK	VERT	TOTAL
M	17-5-8	-1534	-1534	—	BACK	VERT	TOTAL
S	17-11-12	-106	-106	—	BACK	VERT	TOTAL
T	18-11-12	-106	-106	—	BACK	VERT	TOTAL
U	17-11-12	-50	-50	—	BACK	VERT	TOTAL
V	18-11-12	-50	-50	—	BACK	VERT	TOTAL
W	22-11-12	-50	-50	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.42/1.00 (F-G:1) , BC=0.48/1.00 (M-O:1) , WB=0.49/1.00 (H-M:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (B) (INPUT = 0.80)
JSI METAL= 0.60 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

TOWN OF INNISFIL 2018-0833-0840 REVIEWED Building Permit Stamp

ENGINEERING TAM T18005365 STRUCTURAL 1/2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stage

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T181X	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGER'S NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 269.0 lbs FACTORED DOWN AT 21-0-8, 156.1 lbs FACTORED DOWN AT 17-11-12, 156.1 lbs FACTORED DOWN AT 18-11-12, AND 163.1 lbs FACTORED DOWN AT 21-0-8, AND 440.4 lbs FACTORED DOWN AT 23-11-12 ON TOP CHORD, AND 2122.2 lbs FACTORED DOWN AT 17-5-8, 69.9 lbs FACTORED DOWN AT 17-11-12, 69.9 lbs FACTORED DOWN AT 18-11-12, AND 69.8 lbs FACTORED DOWN AT 20-11-12, AND 69.9 lbs FACTORED DOWN AT 22-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALDES
PROVINCE OF ONTARIO

PMS NO. TAM T1805365
STRUCTURAL
OF 2/2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T182	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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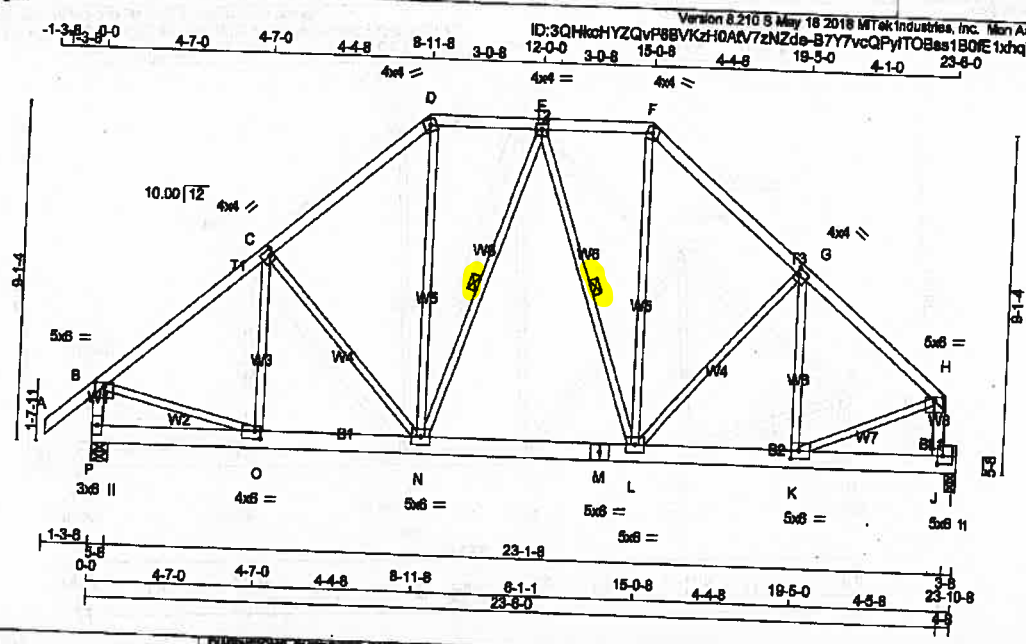

TOTAL WEIGHT = 2 X 134 = 268 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - 1	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-p	MT20	5.0	8.0	1.25	3.00
J	BMVK+p	MT20	5.0	8.0	2.75	1.75
K	BMWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	8.0	2.50	1.75
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2187	0	2187	0	0	5-8	5-8
I	1959	0	1959	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1596	1018 / 0	251 / 0	0 / 0	0 / 0	327 / 0	0 / 0
I	1441	684 / 0	251 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 67	-129.7	-129.7	0.18 (1)	10.00
B- C	-1959 / 0	-129.7	-129.7	0.66 (1)	4.21
C- D	-1738 / 0	-129.7	-129.7	0.52 (1)	4.44
D- E	-1305 / 0	-129.7	-129.7	0.19 (1)	5.44
E- F	-1290 / 0	-129.7	-129.7	0.19 (1)	5.46
F- G	-1714 / 0	-129.7	-129.7	0.46 (1)	4.55
G- H	-1968 / 0	-129.7	-129.7	0.47 (1)	4.28
P- B	-2101 / 0	0.0	0.0	0.22 (1)	5.84
J- H	-2190 / 0	0.0	0.0	0.24 (1)	5.74
P- O	0 / 0	-38.5	-38.5	0.07 (2)	10.00
O- N	0 / 1638	-38.5	-38.5	0.25 (1)	10.00
N- M	0 / 1375	-38.5	-38.5	0.23 (1)	10.00
M- L	0 / 1375	-38.5	-38.5	0.23 (1)	10.00
L- K	0 / 1544	-38.5	-38.5	0.34 (1)	10.00
K- J	-194 / 0	-38.5	-38.5	0.68 (1)	5.25
J- I	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-253 / 70	0.11 (1)
C- N	-363 / 0	0.30 (1)
N- D	0 / 707	0.18 (1)
N- E	-217 / 0	0.13 (1)
E- L	-283 / 0	0.16 (1)
L- F	0 / 687	0.15 (1)
L- G	-393 / 0	0.32 (1)
K- G	-173 / 88	0.07 (1)
B- O	0 / 1892	0.36 (1)
K- H	0 / 1873	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIED WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.69/1.00 (J-K:1) , WB=0.42/1.00 (H-K:1) , SSI=0.72/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

[illegible] 7180536 6
STRUCTURAL
[illegible] ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T183A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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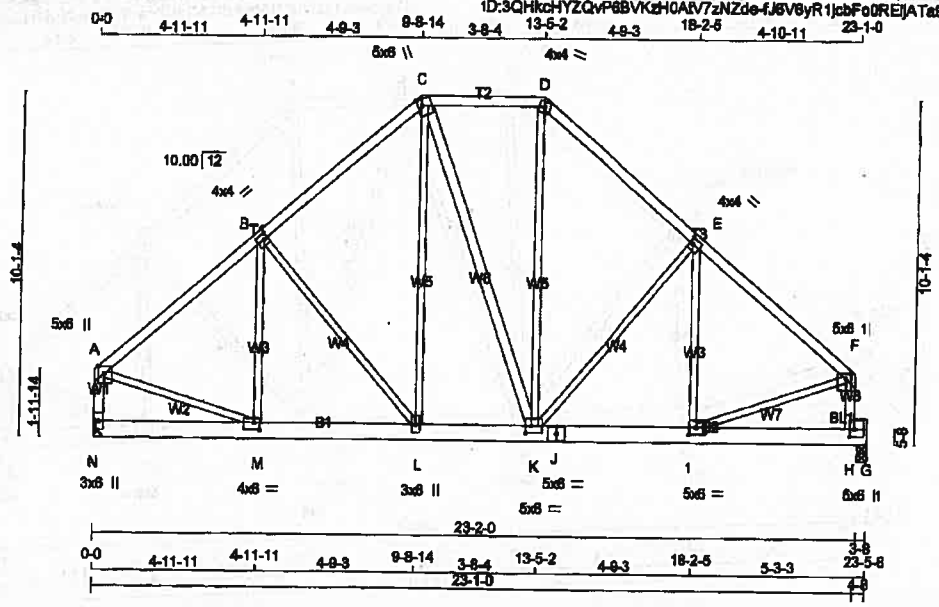

TOTAL WEIGHT = 2 X 134 = 268 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Units in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.50	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	6.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	5.0	6.0	2.00	2.00
H	BMVK+p	MT20	5.0	6.0	2.75	1.75
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	6.0	6.0		
K	BMWWW-t	MT20	6.0	6.0	2.50	2.00
L	BMWW+t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.50	2.00
N	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1972	0	1972	0	0	MECHANICAL	— LS526DS
G	1924	0	1924	0	0	3-8	3-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1448	896 / 0	248 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	1415	868 / 0	246 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1809 / 0	-129.7	-129.7	0.65 (1)	4.18	
B- C	-1559 / 0	-129.7	-129.7	0.61 (1)	4.47	
C- D	-1167 / 0	-129.7	-129.7	0.32 (1)	6.47	
D- E	-1564 / 0	-129.7	-129.7	0.60 (1)	4.49	
E- F	-1833 / 0	-129.7	-129.7	0.65 (1)	4.08	
N- A	-1867 / 0	0.0	0.0	0.21 (1)	8.11	
H- F	-2091 / 0	0.0	0.0	0.23 (1)	5.86	
N- M	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
M- L	0 / 1427	-38.5	-38.5	0.21 (1)	10.00	
L- K	0 / 1162	-38.5	-38.5	0.16 (1)	10.00	
K- J	0 / 1522	-38.5	-38.5	0.37 (1)	10.00	
J- I	0 / 1522	-38.5	-38.5	0.37 (1)	10.00	
I- H	-165 / 0	-38.5	-38.5	0.67 (1)	6.25	
H- G	0 / 0	-38.5	-38.5	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-247 / 104	0.14 (1)
B- L	-417 / 0	0.46 (1)
L- C	0 / 465	0.11 (1)
C- K	0 / 15	0.00 (1)
K- D	0 / 502	0.11 (1)
K- E	-555 / 0	0.61 (1)
I- E	-59 / 169	0.04 (3)
A- M	0 / 1498	0.34 (1)
I- F	0 / 1800	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.6	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.67/1.00 (H-I:1) , WB=0.61/1.00 (B-K:1) , SSI=0.71/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1667	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

T1805367
STRUCTURAL
ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T184	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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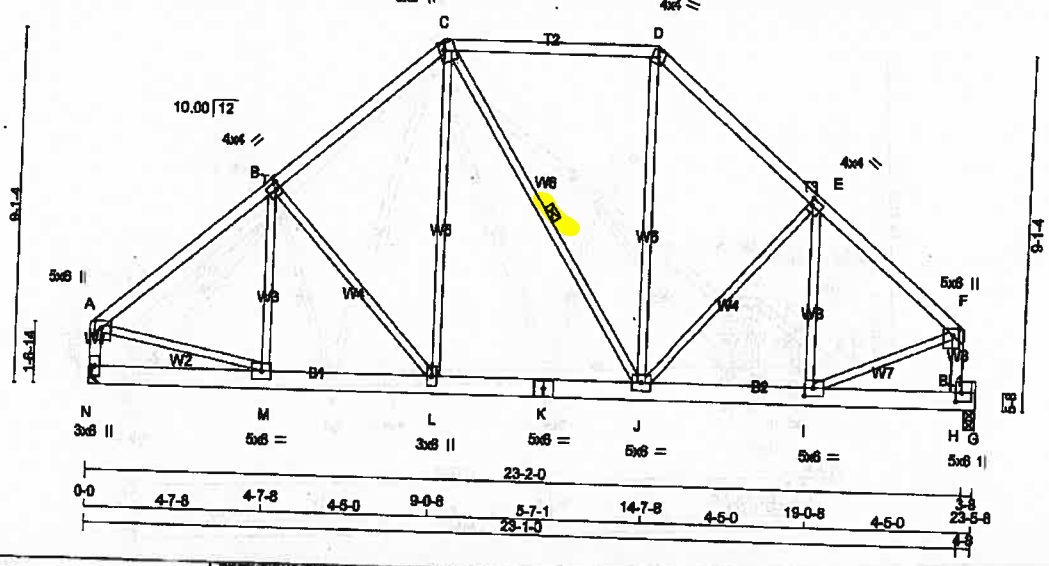

TOTAL WEIGHT = 12 X 124 = 1493 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	8.0	2.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	6.0	8.0	2.00	2.00
H	BMVK+p	MT20	5.0	8.0	2.75	1.75
I	BMWW-t	MT20	5.0	6.0	2.60	2.75
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1972	0	1972	0	0	MECHANICAL	
G	1924	0	1924	0	0	3-8	3-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1448	898 / 0	248 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	1415	868 / 0	248 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1947 / 0	-129.7	-129.7	0.56 (1)	4.20
B-C	-1893 / 0	-129.7	-129.7	0.53 (1)	4.47
C-D	-1282 / 0	-129.7	-129.7	0.77 (1)	4.39
D-E	-1687 / 0	-129.7	-129.7	0.48 (1)	4.60
E-F	-1925 / 0	-129.7	-129.7	0.47 (1)	4.32
N-A	-1887 / 0	0.0	0.0	0.19 (1)	8.11
H-F	-2153 / 0	0.0	0.0	0.24 (1)	5.78
N-M	0 / 0	-38.5	-38.5	0.07 (2)	10.00
M-L	0 / 1529	-38.5	-38.5	0.23 (1)	10.00
L-K	0 / 1268	-38.5	-38.5	0.21 (1)	10.00
K-J	0 / 1268	-38.5	-38.5	0.21 (1)	10.00
J-I	0 / 1512	-38.5	-38.5	0.33 (1)	10.00
I-H	-191 / 0	-38.5	-38.5	0.67 (1)	6.25
H-G	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-B	-215 / 92	0.09 (1)
B-L	-398 / 0	0.33 (1)
L-C	0 / 507	0.11 (1)
C-J	-30 / 0	0.02 (1)
J-D	0 / 463	0.10 (2)
J-E	-395 / 0	0.32 (1)
I-E	-171 / 92	0.07 (1)
A-M	0 / 1576	0.35 (1)
I-F	0 / 1838	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.77/1.00 (C-D:1) , BC=0.67/1.00 (H-I:1) , WB=0.41/1.00 (F-I:1) , SSI=0.71/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.90 (I) (INPUT = 0.90)
LS METAL= 0.43 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MORRIS
PROVINCE OF ONTARIO

#1005368
STRUCTURAL
ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T185	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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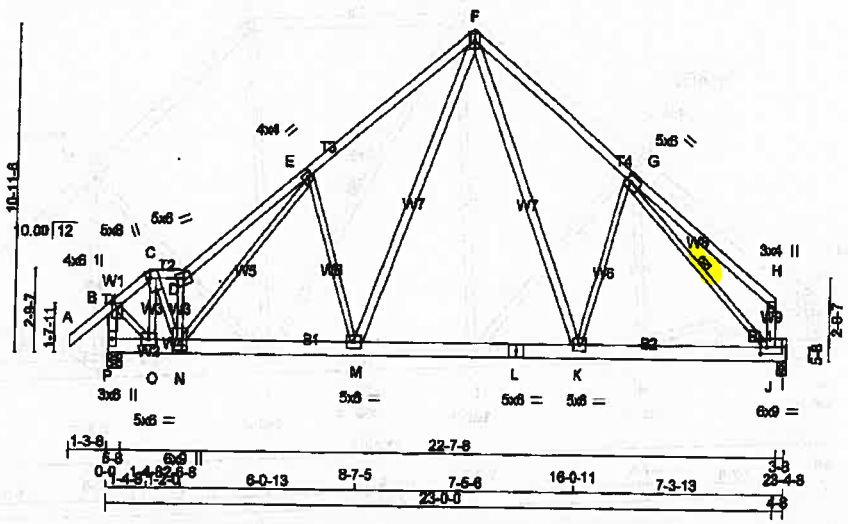

TOTAL WEIGHT = 8 X 134 = 1070 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - 1	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	1.76	2.00
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TTW+m	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	4.0	2.00	1.75
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	5.0	6.0	2.50	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMVWK+t	MT20	6.0	9.0	2.75	2.50
K, M, O						
K	BMWW+t	MT20	5.0	6.0		
L	BS+t	MT20	5.0	8.0		
N	BMWWW+t	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2150	0	2150	0	0	5-8	5-8
I	1912	0	1912	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	18T LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1569	1002 / 0	245 / 0	0 / 0	0 / 0	321 / 0	0 / 0
1	1406	892 / 0	245 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, 1

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B-C	-1509 / 0	-129.7	-129.7	0.19 (1)	5.13
C-D	-1685 / 0	-129.7	-129.7	0.06 (1)	5.08
D-E	-2221 / 0	-129.7	-129.7	0.70 (1)	3.89
E-F	-1908 / 0	-129.7	-129.7	0.77 (1)	3.98
F-G	-1824 / 0	-129.7	-129.7	0.62 (1)	3.92
G-H	0 / 62	-129.7	-129.7	0.62 (1)	10.00
P-B	-2211 / 0	0.0	0.0	0.23 (1)	5.72
J-H	-223 / 0	0.0	0.0	0.02 (1)	7.81
P-O	0 / 0	-38.5	-38.5	0.09 (1)	10.00
O-N	0 / 1084	-38.5	-38.5	0.23 (1)	10.00
N-M	0 / 1647	-38.5	-38.5	0.34 (1)	10.00
M-L	0 / 1080	-38.5	-38.5	0.39 (1)	10.00
L-K	0 / 1080	-38.5	-38.5	0.39 (1)	10.00
K-J	0 / 1445	-38.5	-38.5	0.83 (1)	10.00
J-1	0 / 0	-38.5	-38.5	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-485 / 0	0.08 (1)
C-N	0 / 1477	0.33 (1)
N-D	-1708 / 0	0.28 (1)
N-E	0 / 205	0.05 (2)
E-M	-726 / 0	0.48 (1)
M-F	0 / 1029	0.17 (1)
F-K	0 / 830	0.13 (1)
K-G	-276 / 77	0.18 (1)
B-O	0 / 1434	0.32 (1)
G-J	-2260 / 0	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.62/1.00 (F-G:1) , BC=0.63/1.00 (J-K:1) , WB=0.89/1.00 (G-J:1) , SSI=0.70/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.69 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. AMES
PROVINCE OF ONTARIO

DWG NO. T185 T1805369
PROJECTORAL
GLOBAL TRUSS ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Ready

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T186	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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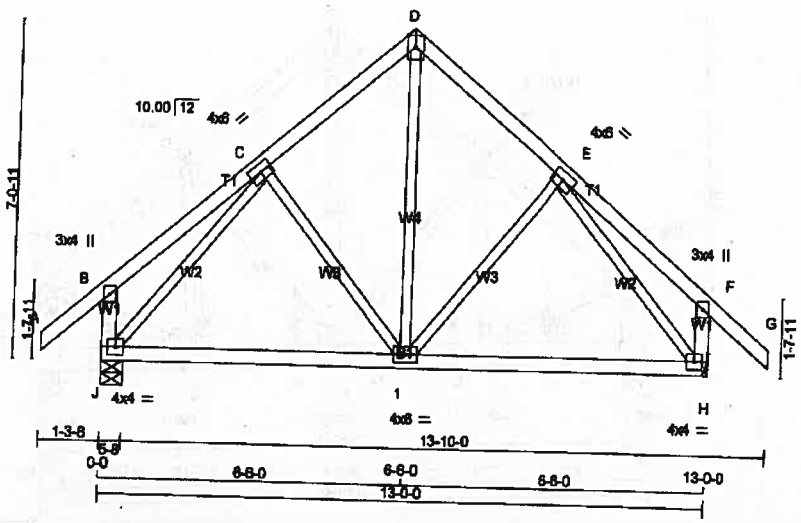

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-4	MT20	4.0	8.0		
D	TTW+p	MT20	4.0	8.0	Edge	
E	TMWW-4	MT20	4.0	8.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-1	MT20	4.0	4.0		
I	BMWWW-4	MT20	4.0	6.0		
J	BMWW1-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 4 X 82 = 248 lb [MP]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1273	0	1273	0	0	IN-SX	IN-SX
H	1273	0	1273	0	0	5-8	5-8
						MECHANICAL	

-LSS28DS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	925	803 / 0	137 / 0	0 / 0	0 / 0	188 / 0	0 / 0
H	925	803 / 0	137 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
B-C	0 / 31	-129.7	-129.7	0.21 (1)	10.00	
C-D	-755 / 0	-129.7	-129.7	0.17 (1)	6.25	
D-E	-755 / 0	-129.7	-129.7	0.17 (1)	6.25	
E-F	0 / 31	-129.7	-129.7	0.21 (1)	10.00	
F-G	0 / 57	-129.7	-129.7	0.18 (1)	10.00	
J-B	-341 / 0	0.0	0.0	0.04 (1)	7.81	
H-F	-341 / 0	0.0	0.0	0.04 (1)	7.81	
J-I	0 / 676	-38.5	-38.5	0.43 (2)	10.00	
I-H	0 / 676	-38.5	-38.5	0.43 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 597	0.13 (1)
I-E	-186 / 26	0.08 (1)
C-I	-186 / 26	0.08 (1)
J-C	-1075 / 0	0.46 (1)
E-H	-1075 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.43/1.00 (I-J:2) , WB=0.46/1.00 (E-H:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
Aug 20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

LICENCE NO. 71805370
STRUCTURAL
CIVIL ENGINEERING

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T187	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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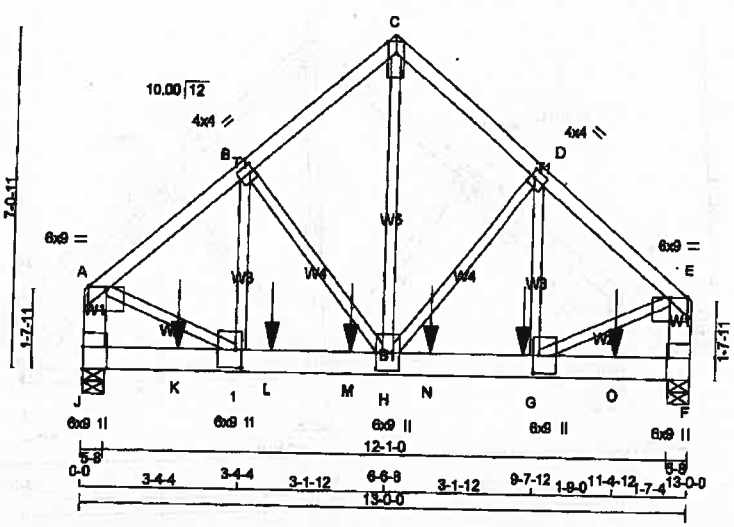

TOTAL WEIGHT = 4 X 71 = 284 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 1 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
J-A	2	12	TOP
F-E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-F	2	12	SIDE(163.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D-G	1	8	SIDE(321.3)
2x3	1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	6.0	9.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMWW-p	MT20	6.0	9.0	Edge	
F	BMV1+t	MT20	6.0	9.0	Edge	0.50
G	BMWW+t	MT20	6.0	9.0	4.50	1.50
H	BMWWW+1	MT20	8.0	9.0		
I	BMWW+t	MT20	6.0	9.0	4.50	1.50
J	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	6717	0	6717	0	0	5-8	5-8
F	7070	0	7070	0	0	5-8	5-8

UNFACTORED REACTIONS
1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J	4911	3104 / 0	782 / 0	0 / 0	0 / 0	1015 / 0	0 / 0
F	5169	3268 / 0	833 / 0	0 / 0	0 / 0	1068 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-6453 / 0	-129.7	-129.7	0.28 (1)	3.66
B-C	-5121 / 0	-129.7	-129.7	0.20 (1)	4.13
C-D	-5121 / 0	-129.7	-129.7	0.20 (1)	4.13
D-E	-6436 / 0	-129.7	-129.7	0.28 (1)	3.66
J-A	-6145 / 0	0.0	0.0	0.23 (1)	5.97
F-E	-6132 / 0	0.0	0.0	0.23 (1)	5.98
J-K	0 / 0	-38.5	-38.5	0.40 (1)	10.00
K-I	0 / 0	-38.5	-38.5	0.40 (1)	10.00
I-L	0 / 4966	-38.5	-38.5	0.66 (1)	10.00
L-M	0 / 4966	-38.5	-38.5	0.66 (1)	10.00
M-H	0 / 4966	-38.5	-38.5	0.66 (1)	10.00
H-N	0 / 4966	-38.5	-38.5	0.66 (1)	10.00
N-G	0 / 4966	-38.5	-38.5	0.66 (1)	10.00
G-O	0 / 0	-38.5	-38.5	0.54 (1)	10.00
O-F	0 / 0	-38.5	-38.5	0.54 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	0 / 6175	0.76 (1)
H-D	-1677 / 0	0.38 (1)
G-D	0 / 1718	0.21 (1)
B-H	-1695 / 0	0.37 (1)
I-B	0 / 1740	0.22 (1)
A-I	0 / 5287	0.65 (1)
G-E	0 / 5275	0.65 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	9-4-12	-1413	-1413	—	FRONT	VERT	TOTAL
K	2-0-4	-1413	-1413	—	FRONT	VERT	TOTAL
L	4-0-4	-1413	-1413	—	FRONT	VERT	TOTAL
M	5-8-8	-1413	-1413	—	FRONT	VERT	TOTAL
N	7-4-12	-1413	-1413	—	FRONT	VERT	TOTAL
O	11-4-12	-1413	-1413	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/240 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.66/1.00 (H-I:1) , WB=0.76/1.00 (C-H:1) , SSI=0.68/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SCI GRIP= 0.85 (C) (INPUT = 0.80)
ISI METAL= 0.78 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MOYES
PROVINCE OF ONTARIO

ENGINO.TAM T130 5371
STRUCTURAL
COMPONENT ENGINEERING
1/2

CONTINUED ON PAGE 2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stage

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T187	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1933.5 lbs FACTORED DOWN AT 2-0-4, 1933.5 lbs FACTORED DOWN AT 4-0-4, 1933.5 lbs FACTORED DOWN AT 5-8-8, 1933.5 lbs FACTORED DOWN AT 7-4-12, AND 1933.5 lbs FACTORED DOWN AT 9-4-12, AND 1933.5 lbs FACTORED DOWN AT 11-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. WOODS
PROVINCE OF ONTARIO

LICENSED TAM 91805371
STRUCTURAL
COMPONENTS ONLY

2/2

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	T188	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 16 2016 MiTek Industries, Inc. Mon Aug 20 18:45:40 2018 Page 1
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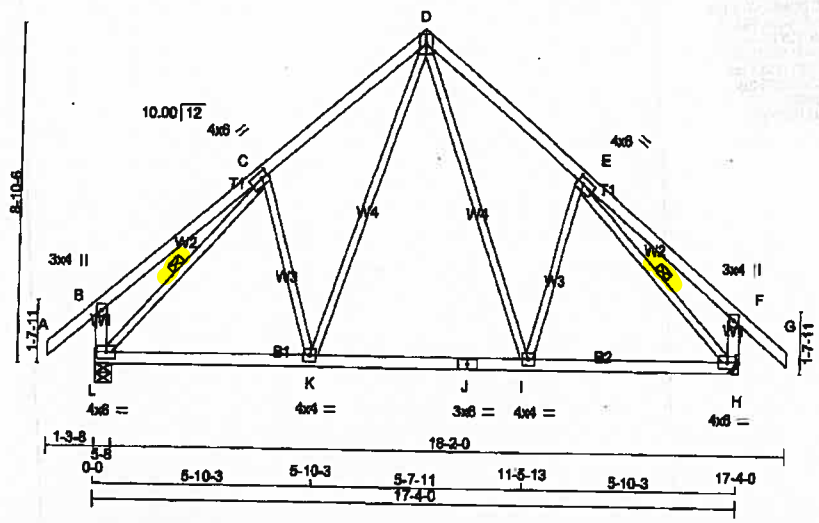

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1637	0	1637	0	0	5-8	5-8
H	1637	0	1637	0	0	MECHANICAL	- LSS26DS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1192	768 / 0	182 / 0	0 / 0	0 / 0	242 / 0	0 / 0
H	1192	768 / 0	182 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	0 / 42	-129.7	-129.7	0.39 (1)	10.00
C- D	-1239 / 0	-129.7	-129.7	0.32 (1)	5.40
D- E	-1239 / 0	-129.7	-129.7	0.32 (1)	5.40
E- F	0 / 42	-129.7	-129.7	0.39 (1)	10.00
F- G	0 / 57	-129.7	-129.7	0.18 (1)	10.00
L- B	-391 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-391 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 1015	-38.5	-38.5	0.35 (2)	10.00
K- J	0 / 738	-38.5	-38.5	0.31 (2)	10.00
J- I	0 / 738	-38.5	-38.5	0.31 (2)	10.00
I- H	0 / 1015	-38.5	-38.5	0.35 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 576	0.13 (1)
I- E	-320 / 28	0.14 (1)
K- D	0 / 576	0.13 (1)
C- K	-320 / 28	0.14 (1)
L- C	-1537 / 0	0.44 (1)
E- H	-1537 / 0	0.44 (1)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

TOTAL WEIGHT = 4 X 65 = 339 lb (MUF)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.35/1.00 (K-L:2) , WB=0.44/1.00 (C-L:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (H) (INPUT = 0.90)
SI METAL= 0.41 (E) (INPUT = 1.00)

LICENCE NUMBER T1805372
STRUCTURAL
COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	G188	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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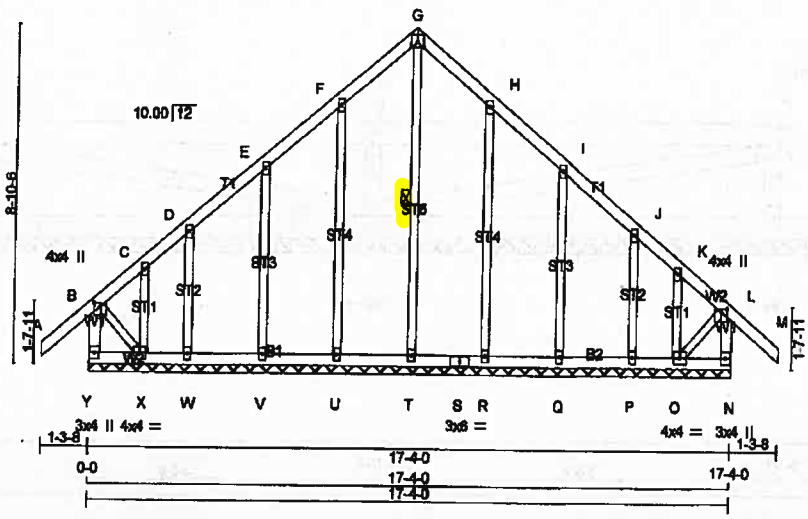

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	1.50	2.00
L	TMW+p	MT20	4.0	4.0	1.00	2.00
N	BMW1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMW1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-422 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.7	-129.7	0.18 (1)	10.00
B- C	-105 / 0	-129.7	-129.7	0.17 (1)	6.25
C- D	-20 / 0	-129.7	-129.7	0.05 (1)	6.25
D- E	-31 / 0	-129.7	-129.7	0.08 (1)	6.25
E- F	-24 / 0	-129.7	-129.7	0.07 (1)	6.25
F- G	-39 / 0	-129.7	-129.7	0.07 (1)	6.25
G- H	-39 / 0	-129.7	-129.7	0.07 (1)	6.25
H- I	-24 / 0	-129.7	-129.7	0.07 (1)	6.25
I- J	-31 / 0	-129.7	-129.7	0.08 (1)	6.25
J- K	-20 / 0	-129.7	-129.7	0.05 (1)	6.25
K- L	-105 / 0	-129.7	-129.7	0.17 (1)	6.25
L- M	0 / 57	-129.7	-129.7	0.18 (1)	10.00
N- L	-422 / 0	0.0	0.0	0.04 (1)	7.81
Y- X	0 / 0	-38.5	-38.5	0.01 (3)	10.00
X- W	0 / 32	-38.5	-38.5	0.02 (2)	10.00
W- V	0 / 26	-38.5	-38.5	0.03 (2)	10.00
V- U	0 / 21	-38.5	-38.5	0.03 (2)	10.00
U- T	0 / 17	-38.5	-38.5	0.02 (2)	10.00
T- S	0 / 17	-38.5	-38.5	0.02 (2)	10.00
S- R	0 / 17	-38.5	-38.5	0.02 (2)	10.00
R- Q	0 / 21	-38.5	-38.5	0.03 (2)	10.00
Q- P	0 / 26	-38.5	-38.5	0.03 (2)	10.00
P- O	0 / 32	-38.5	-38.5	0.02 (2)	10.00
O- N	0 / 0	-38.5	-38.5	0.01 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-190 / 0	0.09 (1)
U- F	-294 / 0	0.27 (1)
V- E	-254 / 0	0.12 (1)
W- D	-232 / 0	0.05 (1)
X- C	-36 / 0	0.01 (1)
R- H	-294 / 0	0.27 (1)
Q- I	-254 / 0	0.12 (1)
P- J	-232 / 0	0.05 (1)
O- K	-36 / 0	0.01 (1)
S- X	0 / 45	0.01 (1)
O- L	0 / 45	0.01 (1)

TOTAL WEIGHT = 2 X 90 = 180 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN.OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.03/1.00 (V-W:2) , WB=0.27/1.00 (H-R:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	765	1667	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (3) (INPUT = 0.90)
JSI METAL= 0.18 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/10/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 17805335
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Ready

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P31	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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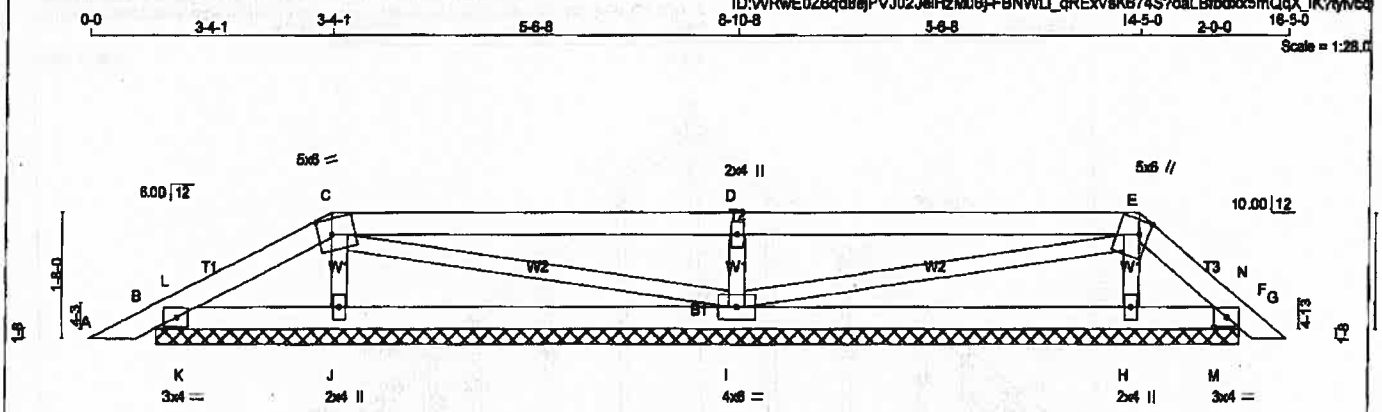

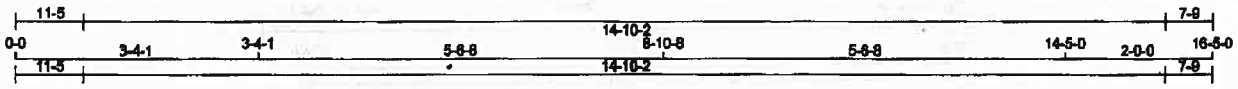

TOTAL WEIGHT = 2 X 46 = 98 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	314	0	314	0	0	14-10-2	14-10-2
F	180	0	180	0	-8	14-10-2	14-10-2
J	573	0	573	0	0	14-10-2	14-10-2
I	1123	0	1123	0	0	14-10-2	14-10-2
H	490	0	490	0	0	14-10-2	14-10-2

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	216	174 / 0	10 / 0	0 / 0	0 / 0	34 / 0	0 / 0
F	117	121 / 0	0 / -14	0 / 0	0 / 0	10 / 0	0 / 0
J	431	234 / 0	96 / 0	0 / 0	0 / 0	101 / 0	0 / 0
I	816	532 / 0	121 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	375	182 / 0	99 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- L	-52 / 0	-129.7	-129.7	0.01 (1)	6.25	
L- C	-65 / 0	-129.7	-129.7	-0.08 (1)	6.25	
C- D	-7 / 0	-129.7	-129.7	0.68 (1)	10.00	
D- E	-6 / 0	-129.7	-129.7	0.68 (1)	10.00	
E- N	-75 / 0	-129.7	-129.7	0.03 (2)	6.25	
N- F	-18 / 51	-129.7	-129.7	0.03 (3)	6.25	
F- G	0 / 20	-129.7	-129.7	0.03 (1)	10.00	
B- K	0 / 56	-38.5	-38.5	0.10 (1)	10.00	
K- J	0 / 68	-38.5	-38.5	0.14 (2)	10.00	
J- I	0 / 28	-38.5	-38.5	0.18 (3)	10.00	
I- H	-1 / 25	-38.5	-38.5	0.18 (3)	10.00	
H- M	0 / 49	-38.5	-38.5	0.14 (2)	10.00	
M- F	0 / 49	-38.5	-38.5	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J- C	-382 / 0	0.05 (1)
C- I	-18 / 0	0.01 (1)
I- D	-896 / 0	0.13 (1)
I- E	-19 / 0	0.01 (1)
H- E	-262 / 0	0.04 (1)
K- L	-181 / 0	0.00 (1)
M- N	-167 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.68/1.00 (C-D:1) , BC=0.19/1.00 (I-J:3) , WB=0.13/1.00 (D-I:1) , SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (D) (INPUT = 0.90)
JSI METAL= 0.18 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 16805336
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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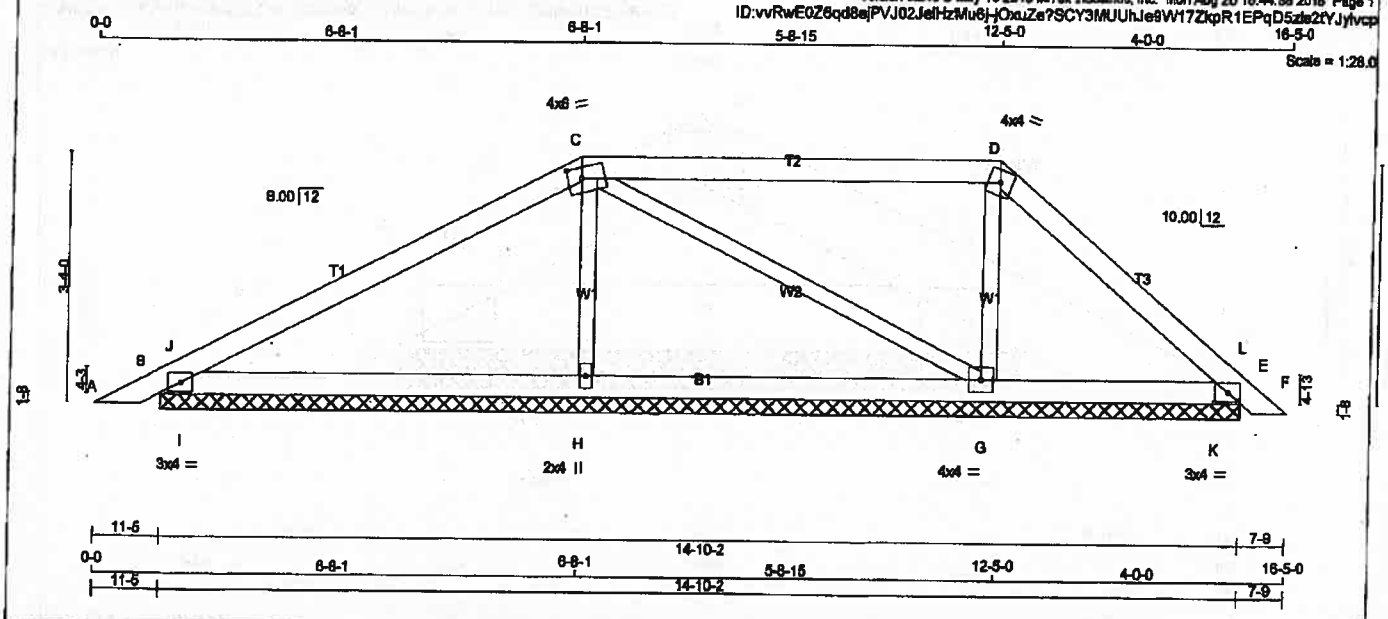

TOTAL WEIGHT = 2 X 47 = 94 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Grids is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	690	0	690	0	0	14-10-2	14-10-2
E	485	0	485	0	0	14-10-2	14-10-2
H	865	0	865	0	0	14-10-2	14-10-2
G	671	0	671	0	0	14-10-2	14-10-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	487	341 / 0	55 / 0	0 / 0	0 / 0	90 / 0	0 / 0
E	330	241 / 0	31 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	643	371 / 0	128 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	498	289 / 0	98 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B-J	-125 / 0	-129.7	-129.7	0.18 (1)	6.25
J-C	-271 / 0	-129.7	-129.7	0.55 (1)	6.25
C-D	-133 / 0	-129.7	-129.7	0.73 (1)	6.25
D-L	-205 / 0	-129.7	-129.7	0.18 (1)	6.25
L-E	-71 / 0	-129.7	-129.7	0.07 (1)	6.25
E-F	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B-I	0 / 232	-38.5	-38.5	0.43 (1)	10.00
I-H	0 / 232	-38.5	-38.5	0.44 (1)	10.00
H-G	0 / 213	-38.5	-38.5	0.32 (2)	10.00
G-K	0 / 149	-38.5	-38.5	0.17 (1)	10.00
K-E	0 / 149	-38.5	-38.5	0.16 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
H-C	-535 / 0	0.10 (1)
C-G	-90 / 0	0.08 (1)
G-D	-434 / 0	0.08 (1)
I-J	-695 / 77	0.00 (1)
K-L	-411 / 5	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.73/1.00 (C-D:1) , BC=0.44/1.00 (H-I:1) , WB=0.10/1.00 (C-H:1) , SSI=0.56/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.55 (B) (INPUT = 0.90)
SI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 18005337
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P33	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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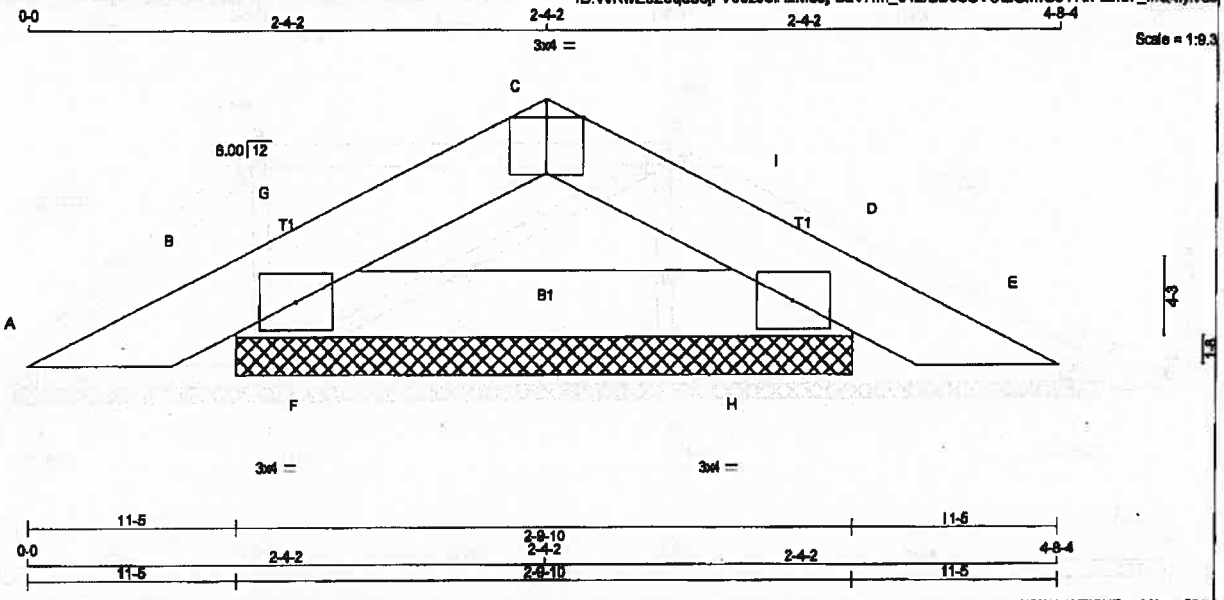

TOTAL WEIGHT = 6 X 8 = 55 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN.	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	348	0	349	0	0	2-9-10	2-9-10
D	349	0	349	0	0	2-9-10	2-9-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	251	174 / 0	29 / 0	0 / 0	0 / 0	47 / 0	0 / 0
D	251	174 / 0	29 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	-254 / 0	-129.7	-129.7	0.04 (2)	6.25
G- C	-199 / 0	-129.7	-129.7	0.05 (1)	6.25
C- I	-196 / 0	-129.7	-129.7	0.05 (1)	6.25
I- D	-254 / 0	-129.7	-129.7	0.04 (2)	6.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 71	0.00 (1)
H- I	0 / 71	0.00 (1)
B- F	0 / 187	-38.5 -38.5 0.04 (1) 10.00
F- H	0 / 187	-38.5 -38.5 0.07 (1) 10.00
H- D	0 / 187	-38.5 -38.5 0.04 (1) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (D-E:1) , BC=0.07/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1997	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T805338
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P41	29	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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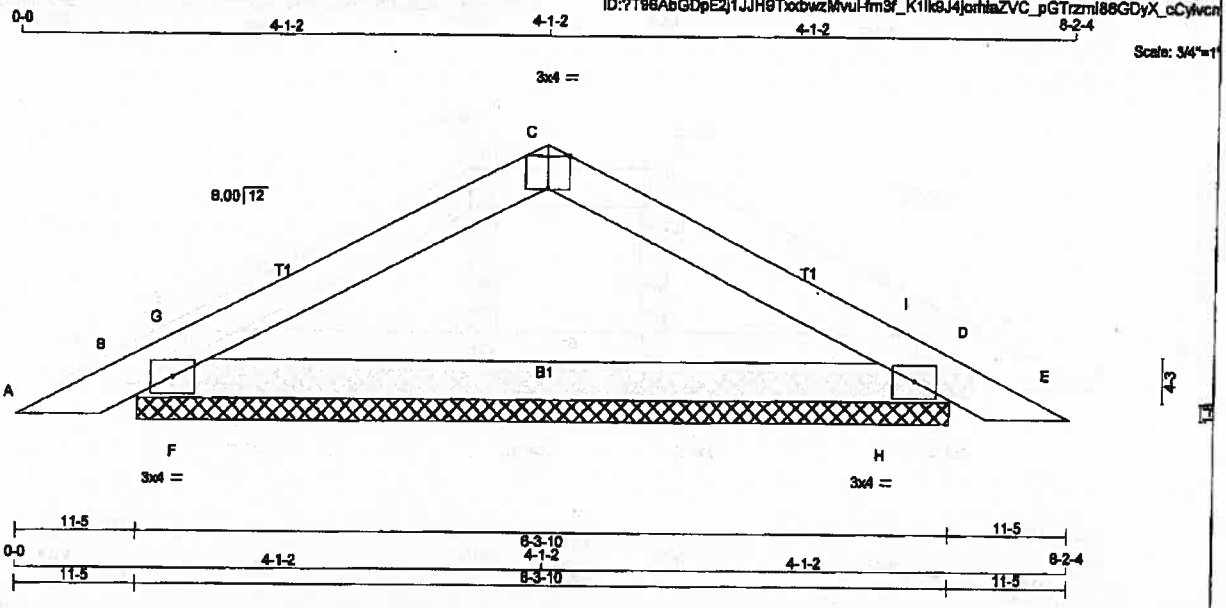

TOTAL WEIGHT = 29 X 18 = 511 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	644	0	644	0	0	6-3-10	6-3-10
D	644	0	644	0	0	6-3-10	6-3-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	467	308 / 0	66 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	467	308 / 0	66 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMITER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- G	-642 / 0	-129.7	-129.7	0.18 (2)	6.25
G- C	-468 / 0	-129.7	-129.7	0.25 (1)	6.25
C- I	-468 / 0	-129.7	-129.7	0.25 (1)	6.25
I- D	-642 / 0	-129.7	-129.7	0.18 (2)	6.25
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- F	0 / 435	-38.5	-38.5	0.12 (3)	10.00
F- H	0 / 435	-38.5	-38.5	0.25 (1)	10.00
H- D	0 / 435	-38.5	-38.5	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
F- G	0 / 317	0.00 (1)
H- I	0 / 317	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-G:1) , BC=0.25/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.22/1.00 (D-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (D) (INPUT = 0.90)
JSI METAL= 0.17 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805339
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Plan

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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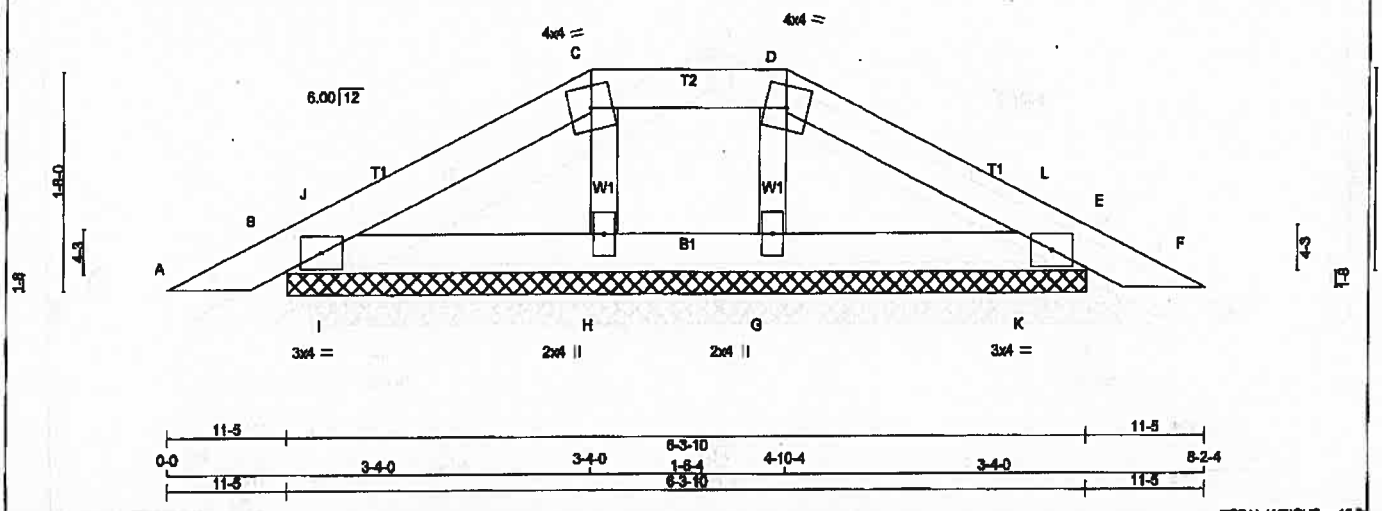

TOTAL WEIGHT = 19 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	334	0	334	0	0	6-3-10	6-3-10
E	334	0	334	0	0	6-3-10	6-3-10
H	310	0	310	0	0	6-3-10	6-3-10
G	310	0	310	0	0	6-3-10	6-3-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	238	170 / 0	25 / 0	0 / 0	0 / 0	43 / 0	0 / 0
E	238	170 / 0	25 / 0	0 / 0	0 / 0	43 / 0	0 / 0
H	229	138 / 0	42 / 0	0 / 0	0 / 0	49 / 0	0 / 0
G	229	138 / 0	42 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- J	-71 / 0	-129.7	-129.7	0.02 (3)	6.25	
J- C	-68 / 0	-129.7	-129.7	0.09 (1)	6.25	
C- D	-40 / 0	-129.7	-129.7	0.05 (1)	6.25	
D- L	-63 / 0	-129.7	-129.7	0.09 (1)	6.25	
L- E	-71 / 0	-129.7	-129.7	0.02 (3)	6.25	
E- F	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- I	0 / 57	-38.5	-38.5	0.09 (1)	10.00	
I- H	0 / 57	-38.5	-38.5	0.09 (1)	10.00	
H- G	0 / 40	-38.5	-38.5	0.05 (1)	10.00	
G- K	0 / 57	-38.5	-38.5	0.09 (1)	10.00	
K- E	0 / 57	-38.5	-38.5	0.09 (1)	10.00	

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
H- C	-201 / 0	0.03 (1)
G- D	-201 / 0	0.03 (1)
I- J	-139 / 25	0.00 (1)
K- L	-139 / 25	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (D-L:1) , BC=0.09/1.00 (E-K:1) , WB=0.03/1.00 (D-G:1) , SSI=0.12/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	768	1967	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 71805340
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P81	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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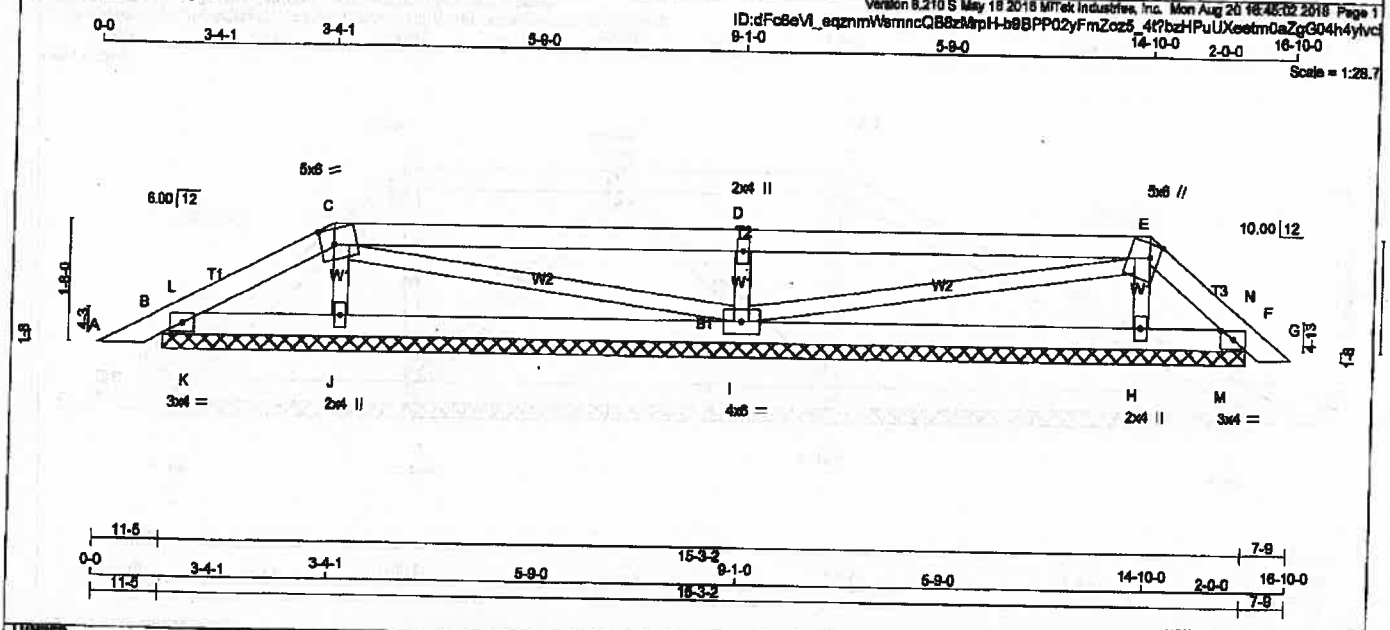

TOTAL WEIGHT = 2 X 49 = 98 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	8.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	312	0	312	0	0	15-3-2	15-3-2
F	176	0	176	0	-14	15-3-2	15-3-2
J	589	0	589	0	0	15-3-2	15-3-2
I	1165	0	1165	0	0	15-3-2	15-3-2
H	508	0	508	0	0	15-3-2	15-3-2

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOADE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	217	174 / 0	8 / 0	0 / 0	0 / 0	33 / 0	0 / 0
F	114	121 / 0	0 / -18	0 / 0	0 / 0	8 / 0	0 / 0
J	443	240 / 0	99 / 0	0 / 0	0 / 0	104 / 0	0 / 0
I	847	582 / 0	125 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	389	188 / 0	103 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00
B- L	-60 / 4	-129.7	-129.7	0.02 (1)	6.25
L- C	-65 / 0	-129.7	-129.7	0.09 (1)	6.25
C- D	-7 / 1	-129.7	-129.7	0.73 (1)	10.00
D- E	-6 / 1	-129.7	-129.7	0.73 (1)	10.00
E- N	-76 / 0	-129.7	-129.7	0.03 (2)	6.25
N- F	-14 / 57	-129.7	-129.7	0.04 (3)	6.25
F- G	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- K	0 / 56	-38.5	-38.5	0.10 (1)	10.00
K- J	0 / 56	-38.5	-38.5	0.15 (2)	10.00
J- I	0 / 25	-38.5	-38.5	0.21 (3)	10.00
I- H	-2 / 25	-38.5	-38.5	0.21 (3)	10.00
H- M	0 / 49	-38.5	-38.5	0.15 (3)	10.00
M- F	0 / 49	-38.5	-38.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-372 / 0	0.05 (1)
C- I	-18 / 0	0.01 (1)
I- D	-631 / 0	0.13 (1)
I- E	-19 / 0	0.01 (1)
H- E	-290 / 0	0.04 (1)
K- L	-186 / 0	0.00 (1)
M- N	-175 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.73/1.00 (C-D:1) , BC=0.21/1.00 (H-I:3) , WB=0.13/1.00 (D-I:1) , SSI=0.38/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.49 (D) (INPUT = 0.80)
CSI METAL= 0.20 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805341
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P82	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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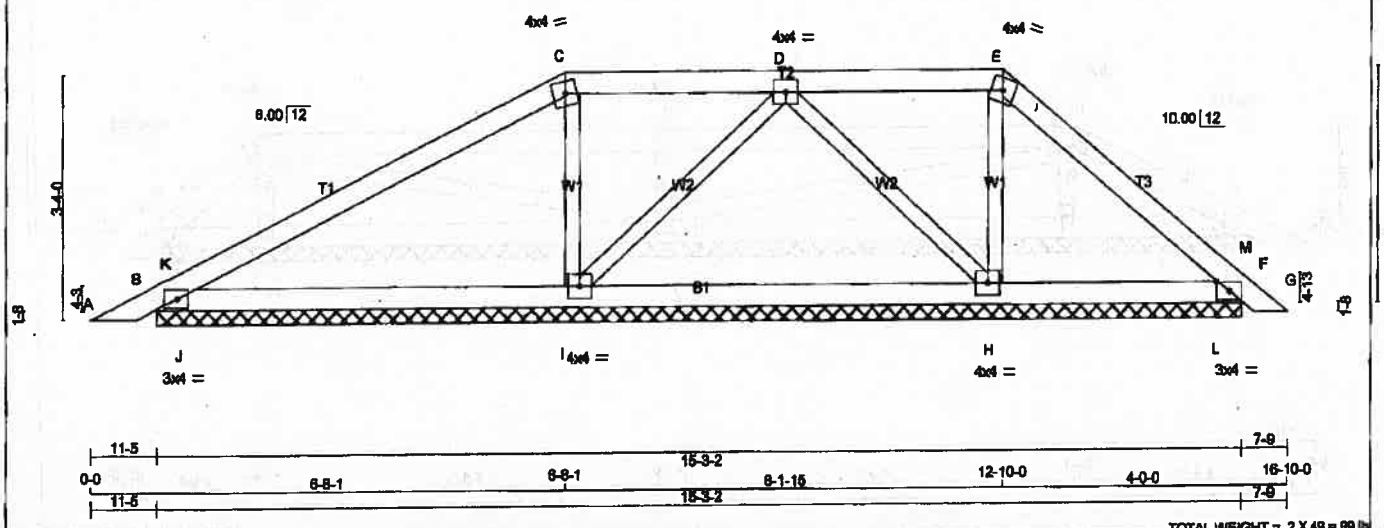

TOTAL WEIGHT = 2 X 49 = 99 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	SMWW1-t	MT20	4.0	4.0		
I	SMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	638	0	638	0	0	15-3-2	15-3-2
F	412	0	412	0	0	15-3-2	15-3-2
I	954	0	954	0	0	15-3-2	15-3-2
H	746	0	746	0	0	15-3-2	15-3-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	458	317 / 0	68 / 0	0 / 0	0 / 0	86 / 0	0 / 0
F	292	214 / 0	28 / 0	0 / 0	0 / 0	51 / 0	0 / 0
I	705	421 / 0	131 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	554	322 / 0	109 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- K	-101 / 0	-129.7	-129.7	0.18 (1)	6.25	
K- C	-160 / 0	-129.7	-129.7	0.55 (1)	6.25	
C- D	-136 / 0	-129.7	-129.7	0.20 (1)	6.25	
D- E	-82 / 0	-129.7	-129.7	0.20 (1)	6.25	
E- M	-131 / 0	-129.7	-129.7	0.18 (1)	6.25	
M- F	-33 / 24	-129.7	-129.7	0.08 (1)	6.25	
F- G	0 / 20	-129.7	-129.7	0.03 (1)	10.00	
B- J	0 / 151	-38.5	-38.5	0.42 (1)	10.00	
J- I	0 / 151	-38.5	-38.5	0.42 (1)	10.00	
I- H	0 / 382	-38.5	-38.5	0.35 (1)	10.00	
H- L	0 / 92	-38.5	-38.5	0.18 (3)	10.00	
L- F	0 / 92	-38.5	-38.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-397 / 0	0.07 (1)
I- D	-315 / 0	0.09 (1)
D- H	-391 / 0	0.12 (1)
H- E	-261 / 0	0.05 (1)
J- K	-703 / 70	0.00 (1)
L- M	-429 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55/1.00 (C-K:1) , BC=0.42/1.00 (I-J:1) , WB=0.12/1.00 (D-H:1) , SSI=0.57/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. MEYES
PROVINCE OF ONTARIO

DWG NO. TAM T1805342
STRUCTURAL
CONSENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P83	6	1	TRUSS DESC.	

Tamarack Roof Trusts, Burlington

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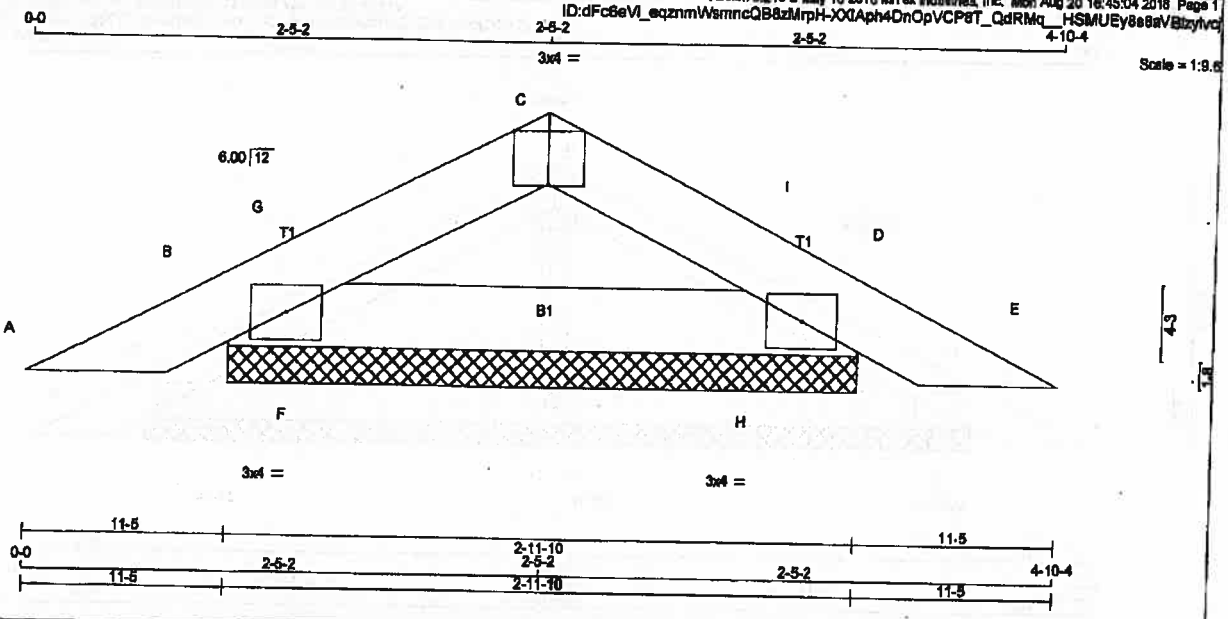

TOTAL WEIGHT = 6 X 10 = 57 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	363	0	363	0	0	2-11-10	2-11-10
D	363	0	363	0	0	2-11-10	2-11-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	281	181 / 0	31 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	281	181 / 0	31 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- G	-272 / 0	-129.7	-129.7	0.05 (2)	6.25	
G- C	-209 / 0	-129.7	-129.7	0.05 (1)	6.25	
C- I	-209 / 0	-129.7	-129.7	0.05 (1)	6.25	
I- D	-272 / 0	-129.7	-129.7	0.05 (2)	6.25	
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- F	0 / 199	-38.5	-38.5	0.04 (1)	10.00	
F- H	0 / 199	-38.5	-38.5	0.07 (1)	10.00	
H- D	0 / 199	-38.5	-38.5	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 79	0.00 (1)
H- I	0 / 79	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (A-B:1) , BC=0.07/1.00 (F-H:1) , WB=0.00/1.00 (F-G:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 BHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1957	1659

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805343
STRUCTURAL
COMPONENT ONLY

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Stamp

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P84	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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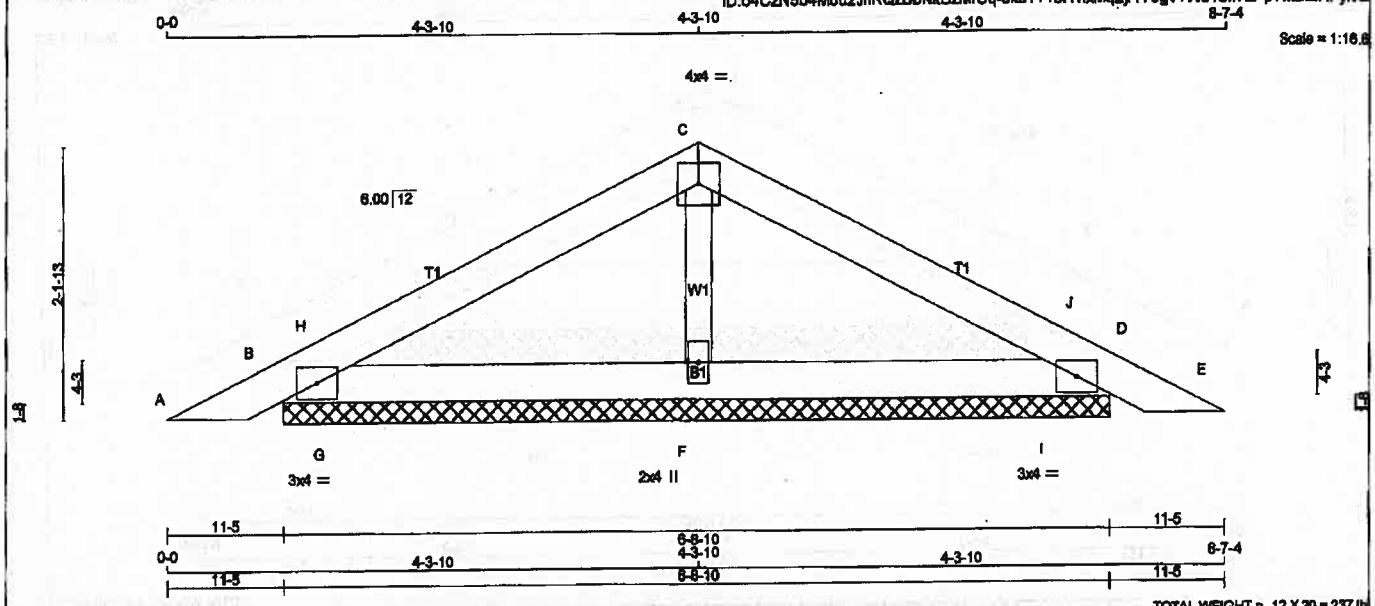

TOTAL WEIGHT = 12 X 20 = 237 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	416	0	416	0	0	6-8-10	6-8-10
D	416	0	416	0	0	6-8-10	6-8-10
F	524	0	524	0	0	6-8-10	6-8-10

UNFACTORED REACTIONS

JT	1ST LOADE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	297	212 / 0	31 / 0	0 / 0	0 / 0	54 / 0	0 / 0
D	297	212 / 0	31 / 0	0 / 0	0 / 0	54 / 0	0 / 0
F	380	224 / 0	79 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- H	-82 / 0	-129.7	-129.7	0.04 (1)	6.25	
H- C	-123 / 0	-129.7	-129.7	0.17 (1)	6.25	
C- J	-123 / 0	-129.7	-129.7	0.17 (1)	6.25	
J- D	-82 / 0	-129.7	-129.7	0.04 (1)	6.25	
D- E	0 / 24	-129.7	-129.7	0.07 (1)	10.00	
B- G	0 / 106	-38.5	-38.5	0.17 (1)	10.00	
G- F	0 / 106	-38.5	-38.5	0.17 (1)	10.00	
F- I	0 / 106	-38.5	-38.5	0.17 (1)	10.00	
I- D	0 / 106	-38.5	-38.5	0.17 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-265 / 0	0.04 (1)
G- H	-299 / 19	0.00 (1)
I- J	-299 / 19	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-H:1) , BC=0.17/1.00 (B-G:1) , WB=0.04/1.00 (C-F:1) , SSI=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1805344
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P164	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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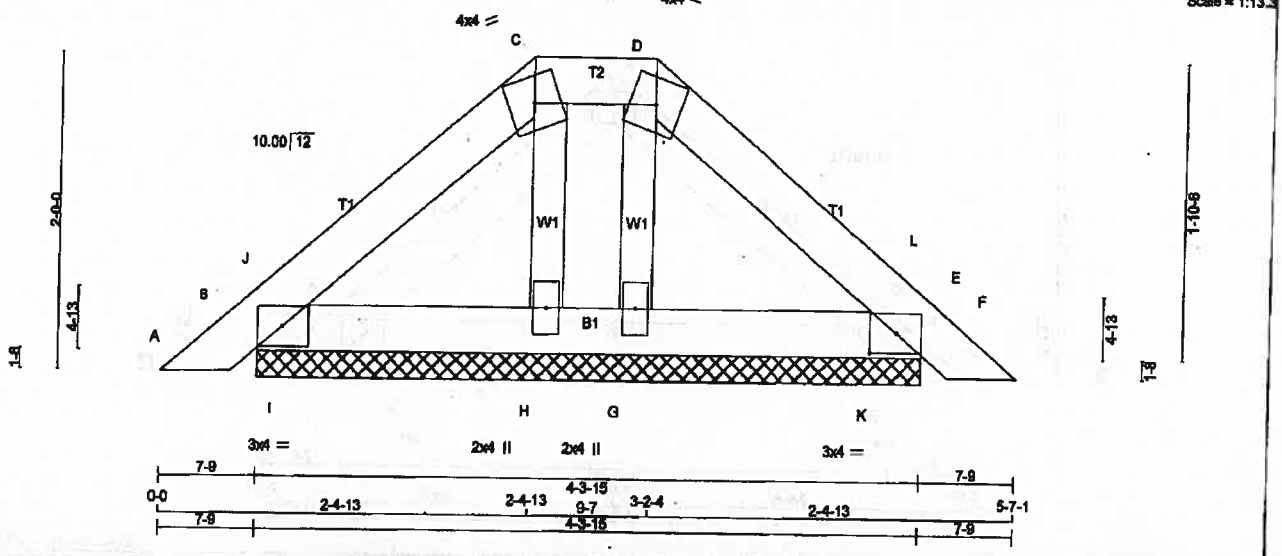


TOTAL WEIGHT = 2 X 15 = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	256	0	256	0	0	4-3-15	4-3-15
E	256	0	256	0	0	4-3-15	4-3-15
H	177	0	177	0	0	4-3-15	4-3-15
G	177	0	177	0	0	4-3-15	4-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	183	131 / 0	19 / 0	0 / 0	0 / 0	33 / 0	0 / 0
E	183	131 / 0	19 / 0	0 / 0	0 / 0	33 / 0	0 / 0
H	132	76 / 0	27 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	132	76 / 0	27 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- J	-62 / 0	-129.7	-129.7	0.01 (3)	6.25
J- C	-72 / 0	-129.7	-129.7	0.05 (1)	6.25
C- D	-46 / 0	-129.7	-129.7	0.01 (1)	6.25
D- L	-72 / 0	-129.7	-129.7	0.05 (1)	6.25
L- E	-62 / 0	-129.7	-129.7	0.01 (3)	6.25
E- F	0 / 20	-129.7	-129.7	0.03 (1)	10.00
B- I	0 / 53	-38.5	-38.5	0.05 (1)	10.00
I- H	0 / 53	-38.5	-38.5	0.05 (1)	10.00
H- G	0 / 46	-38.5	-38.5	0.04 (1)	10.00
G- K	0 / 53	-38.5	-38.5	0.05 (1)	10.00
K- E	0 / 53	-38.5	-38.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-102 / 0	0.01 (1)
G- D	-102 / 0	0.01 (1)
I- J	-127 / 17	0.00 (1)
K- L	-127 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-L:1) , BC=0.05/1.00 (G-K:1) , WB=0.01/1.00 (D-G:1) , SSI=0.11/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENB= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1657	788	1997	1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

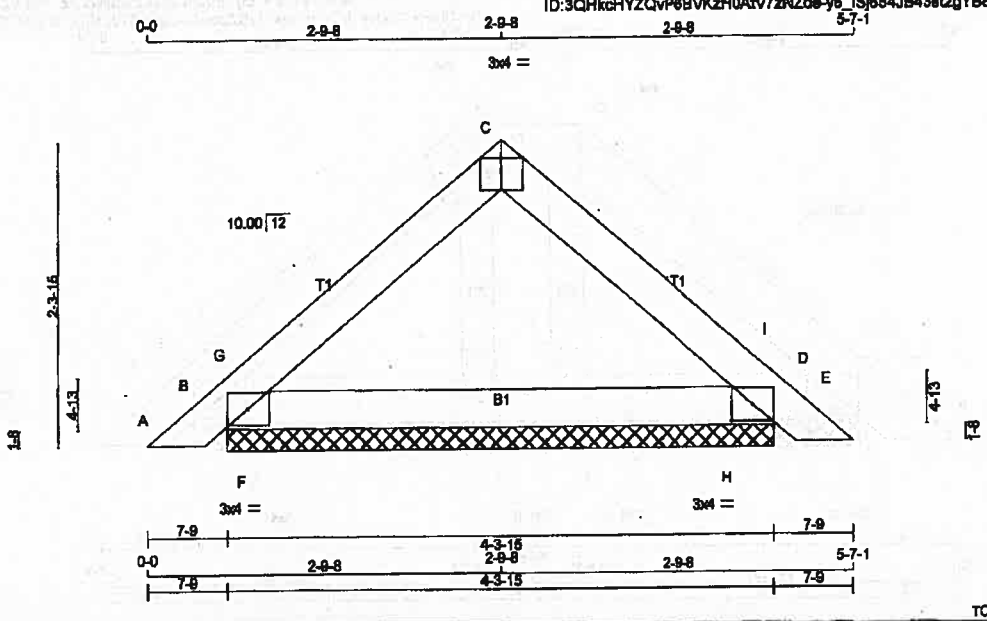
DWG NO. TAM T1805345
STRUCTURAL
CITY OF BRAMPTON

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P165	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 18:46:07 2018 Page 1
ID:3QHkcHYZQvP6BVKzH0AtV7zNZde-y8_ISj654JB43et2gYB8_ScVufOSRJtqYjrMlylvcq

Scale: 3/4"=1'



TOTAL WEIGHT = 6 X 13 = 79 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	433	0	433	0	0	4-3-15	4-3-15
D	433	0	433	0	0	4-3-15	4-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	315	208 / 0	45 / 0	0 / 0	0 / 0	63 / 0	0 / 0
D	315	208 / 0	45 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00	
B- G	-363 / 0	-129.7	-129.7	0.06 (3)	6.25	
G- C	-214 / 0	-129.7	-129.7	0.11 (1)	6.25	
C- I	-214 / 0	-129.7	-129.7	0.11 (1)	6.25	
I- D	-363 / 0	-129.7	-129.7	0.06 (3)	6.25	
D- E	0 / 20	-129.7	-129.7	0.03 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 232	0.00 (1)
H- I	0 / 232	0.00 (1)
B- F	0 / 175	-38.5 -38.5 0.06 (3) 10.00
F- H	0 / 175	-38.5 -38.5 0.12 (2) 10.00
H- D	0 / 175	-38.5 -38.5 0.06 (3) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-I:1) , BC=0.12/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.12/1.00 (D-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	768	1957	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

LOWS WILLIAM T1805346
STRUCTURAL
COMPONENT GROUP

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297083	P166	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.210 S May 16 2016 MiTek Industries, Inc. Mon Aug 20 16:45:09 2016 Page 1
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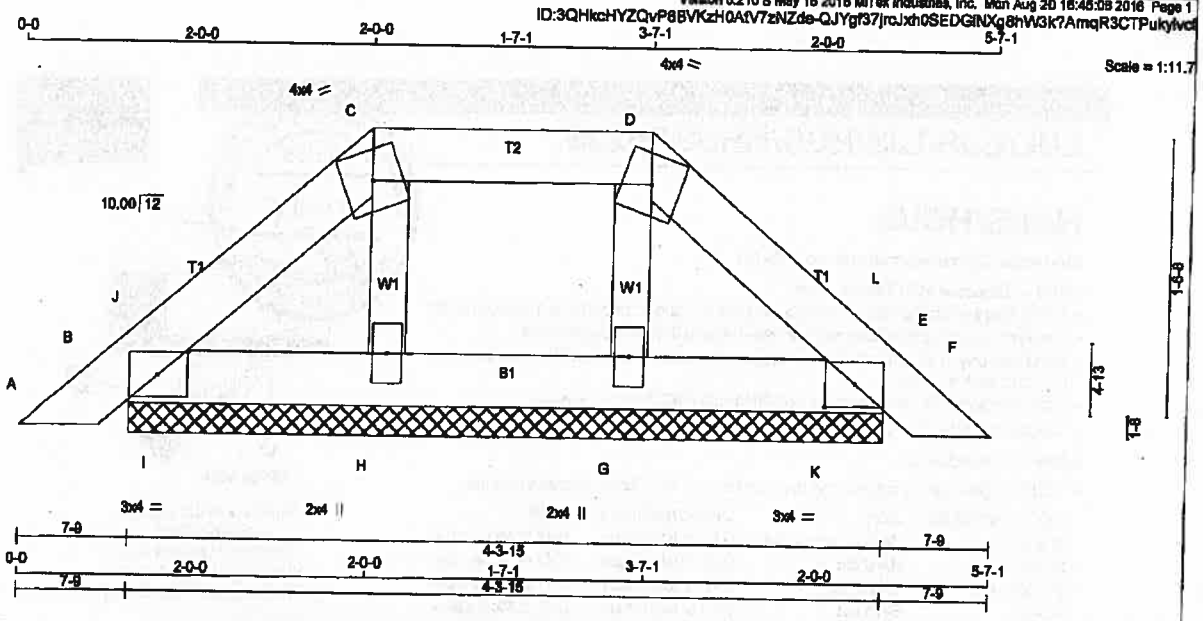


TOTAL WEIGHT = 2 X 14 = 28 lb.

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	222	0	222	0	0	4-3-15	4-3-15
E	222	0	222	0	0	4-3-15	4-3-15
H	212	0	212	0	0	4-3-15	4-3-15
G	212	0	212	0	0	4-3-15	4-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	157	116 / 0	14 / 0	0 / 0	0 / 0	27 / 0	0 / 0
E	157	116 / 0	14 / 0	0 / 0	0 / 0	27 / 0	0 / 0
H	158	91 / 0	32 / 0	0 / 0	0 / 0	35 / 0	0 / 0
G	158	91 / 0	32 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-129.7	-129.7	0.03 (1)	10.00	
B- J	-67 / 0	-129.7	-129.7	0.01 (3)	6.25	
J- C	-62 / 0	-129.7	-129.7	0.03 (1)	6.25	
C- D	-35 / 0	-129.7	-129.7	0.06 (1)	6.25	
D- L	-62 / 0	-129.7	-129.7	0.03 (1)	6.25	
L- E	-67 / 0	-129.7	-129.7	0.01 (3)	6.25	
E- F	0 / 20	-129.7	-129.7	0.03 (1)	10.00	
B- I	0 / 47	-38.5	-38.5	0.04 (1)	10.00	
I- H	0 / 47	-38.5	-38.5	0.04 (1)	10.00	
H- G	0 / 35	-38.5	-38.5	0.02 (1)	10.00	
G- K	0 / 47	-38.5	-38.5	0.04 (1)	10.00	
K- E	0 / 47	-38.5	-38.5	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-136 / 0	0.02 (1)
G- D	-136 / 0	0.02 (1)
I- J	-81 / 10	0.00 (1)
K- L	-81 / 10	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-D:1) , BC=0.04/1.00 (B-I:1) , WB=0.02/1.00 (C-H:1) , SSI=0.06/1.00 (C-D:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.18 (E) (INPUT = 0.90)
ISI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/20/16
H.J.G. MOES
PROVINCE OF ONTARIO

TAM 7805347
STRUCTURAL
COMPONENTS

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

SIMPSON Strong-Tie

LICENSED PROFESSIONAL ENGINEER
1/15/18
B. D. BUNTING
PROVINCE OF ONTARIO

HHUS/HGUS

See Hanger Options information on pp. 125–127.

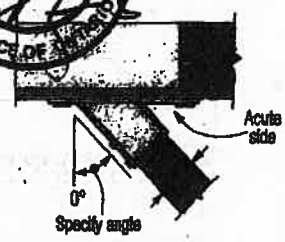
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-out to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions W	H	B	d_e	Fasteners Header	Joist	Allowable Uplift Loads (Lbs.)		Installed Weight (Lbs.)	
								[redacted]	[redacted]	[redacted]	[redacted]
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	[redacted]	(4) 10d	(2) 10d	710	1625	645	1155
LUS24L	22	1 9/16	3	1 3/4	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
[redacted]	22	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	720	1605	645	1140
[redacted]	18	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	1420	2170	1290	1630
[redacted]	18	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	2705	4940	2065	3875
[redacted]	18	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	2055	4285	1460	4115
HUS526	12	3 1/8	5 7/8	5	4 3/4	(28) 16d	(8) 16d	2885	6625	2885	5700
LUS28L	20	1 9/16	6 3/4	1 3/4	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
LUS28	18	1 9/16	6 1/8	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2520	1290	1790
HUS28	18	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605	5365	2875	4345
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7875	3310	6900
HUS210L	20	1 9/16	8	1 3/4	7 3/4	(10) 10d	(6) 16d x 1 1/2"	1140	2495	1020	1770
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

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TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

SIMPSON Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more Information.

Model No.	Ga.	Dimensions (in.) W	H	B	4_b[1]	Fasteners Header	Joist	Factored Resistance (lb.)		S-P-F Uplift (DOL = 1.33) lb. kN	S-P-F Normal (DOL = 1.00) lb. kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1¼	(4) 16d	(2) 16d	635	2020	580	1435
										2.62	6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
										6.87	8.54
HHUS26-2	14	3⅝	5⅝	3	3⅝	(14) 16d	(6) 16d	2850	7395	2065	5205
										9.20	23.15
HGUS26-2	12	3⅝	5⅝	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
										13.83	28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
										6.87	11.45
HHUS28-2	14	3⅝	7⅝	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
										11.90	28.22
HGUS28-2	12	3⅝	7⅝	4	6⅛	(30) 16d	(12) 16d	6070	12980	4310	9215
										19.17	40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
										10.32	14.21
HHUS210-2	14	3⅝	9⅝	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
										18.84	31.14
HGUS210-2	12	3⅝	9⅝	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
										21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4⅝	5⅛	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
										13.83	28.27
HGUS28-3	12	4⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
										19.17	40.99
HHUS210-3	14	4⅝	9	3	7⅝	(34) 16d	(10) 16d	4670	9670	4235	6865
										18.84	30.54
HGUS210-3	12	4⅝	9⅝	4	8⅛	(46) 16d	(16) 16d	6840	14845	4855	10400
										21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅝	5⅛	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
										13.83	28.27
HGUS28-4	12	6⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
										19.17	40.99
HHUS210-4	14	6⅝	8⅝	3	7⅝	(34) 16d	(10) 16d	4670	10155	4235	7210
										18.84	32.07
HGUS210-4	12	6⅝	9⅝	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
										21.60	46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7840	14995	5425	10845
										24.13	48.25
HGUS214-4	12	6⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
										32.00	51.80
4x Sizes											
LUS48	18	3⅝	4⅝	2	3⅝	(4) 16d	(4) 16d	1720	2595	1545	1920
										6.87	8.54
HHUS48	14	3⅝	5⅝	3	5⅝	(14) 16d	(6) 16d	2540	7395	2065	5205
										9.20	23.15
HGUS48	12	3⅝	5⅝	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
										13.83	28.27
LUS48	18	3⅝	6⅝	2	3⅝	(6) 16d	(4) 16d	1720	3325	1545	2575
										6.87	11.45
HHUS48	14	3⅝	7⅝	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
										11.90	28.22
HGUS48	12	3⅝	7⅝	4	6⅛	(30) 16d	(12) 16d	6070	12980	4310	9215
										19.17	40.99
LUS410	18	3⅝	8⅝	2	5⅝	(8) 16d	(6) 16d	2580	4500	2320	3195
										10.32	14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
										21.60	45.69
HGUS412	12	3⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7840	14995	5425	10845
										24.13	47.35
HGUS414	12	3⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11845
										32.00	51.80

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Plated Truss Connectors

See footnotes on p. 258.

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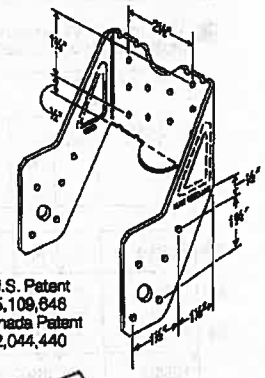
TBE – Truss Bearing Enhancers

SIMPSON Strong-Tie

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

Finish: G90 galvanized



TBE4
(TBE6 similar)

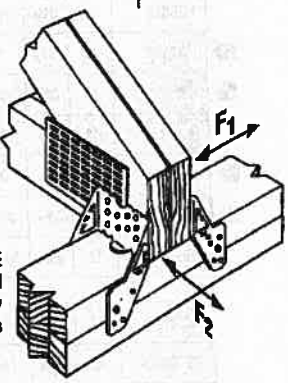
U.S. Patent 5,109,648
Canada Patent 2,044,440

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to with stand the loads imposed by the nails.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.



Two TBE installed with two ply girder truss

Model No.	No. of Plies	Fasteners: Truss	Fasteners: Plate	TBE Only Factored Resistance: Uplift (K_D = 1.15)	TBE Only Factored Resistance: Normal (K_D = 1.00)	Lateral (K_D = 1.15): F_1	Lateral (K_D = 1.15): F_2	Combined TBE and Wood Bearing Factored Resistance: Normal[2] (K_D = 1.00)	Combined TBE and Wood Bearing Factored Resistance: TBL[4]
				lb.	lb.	lb.	lb.	lb.	in.
D.Fir-L									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1605	3660	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3660	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3660	655	1415	20705	4.25
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3540	490	1745	10235	8.41
	2	(20) 10d	(20) 10d	1760	3860	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3860	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3860	490	1745	30640	6.29
S-P-F									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3220	615	1415	8445	6.99
	2	(20) 10d	(20) 10d	1605	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	16345	4.43
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q_r/A_b and Q'_r/A'_b = 812 psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.

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TS/LTS/MTS/HTS

SIMPSON Strong-Tie®

Twist Straps

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

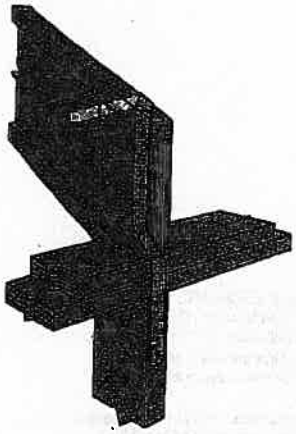
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV

TS

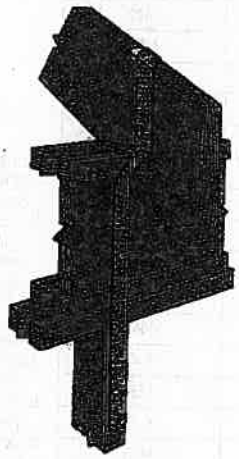
LTS12
(MTS and HTS similar)

MTS30
(HTS30 similar)

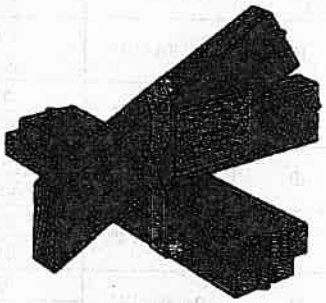
HTS30C
(MTS30C similar)



MTS30 Installation with I-Joist Rafter



Typical MTS30 Installation



MTS Installation as a Truss-to-Top Plate Tie

Typical TS Installation

Straps and Ties

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TS/LTS/MTS/HTS

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Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance (K_D = 1.15) D.Fir-L lb. kN	Factored Resistance (K_D = 1.15) S-P-F lb. kN
TS9	9	(8) 16d	1125 / 5.00	1040 / 4.63
TS12	11⅝	(10) 16d	1410 / 6.27	1300 / 5.78
TS18	17¾	(14) 16d	1970 / 8.76	1820 / 8.10
TS22	21⅝	(18) 16d	2125 / 9.45	2125 / 9.45
SS LTS12	12	(12) 10d x 1½"	1015 / 4.52	720 / 3.20
LTS16	16	(12) 10d x 1½"	1015 / 4.52	720 / 3.20
SS LTS18	18	(12) 10d x 1½"	1015 / 4.52	720 / 3.20
LTS20	20	(12) 10d x 1½"	1015 / 4.52	720 / 3.20
MTS12	12	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
MTS16	16	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
MTS18	18	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
SS MTS20	20	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
MTS30	30	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
MTS24C	24	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
MTS30C	30	(14) 10d x 1½"	1570 / 6.98	1180 / 5.25
HTS16	16	(16) 10d x 1½"	2050 / 9.12	1455 / 6.47
HTS20	20	(24) 10d x 1½"	2050 / 9.12	1455 / 6.47
HTS24	24	(24) 10d x 1½"	2050 / 9.12	1455 / 6.47
HTS28	28	(24) 10d x 1½"	2050 / 9.12	1455 / 6.47
HTS30	30	(24) 10d x 1½"	2050 / 9.12	1455 / 6.47
HTS30C	30	(24) 10d x 1½"	2050 / 9.12	1455 / 6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

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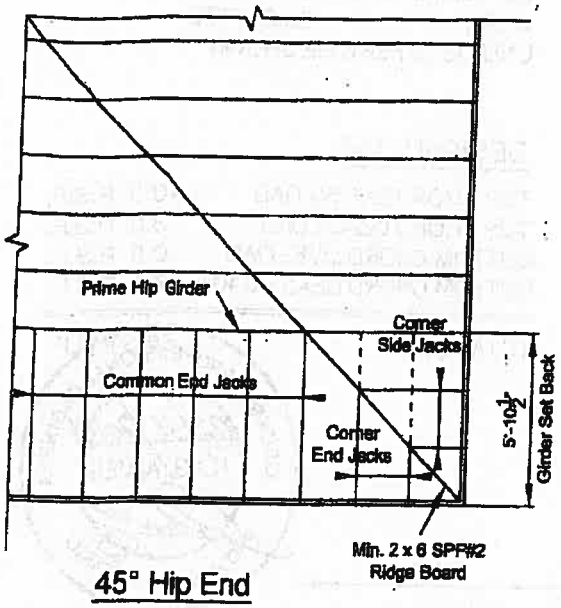

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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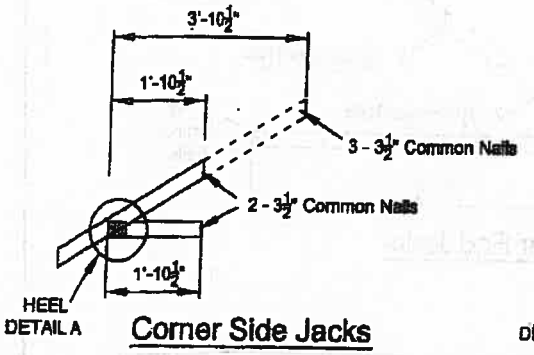
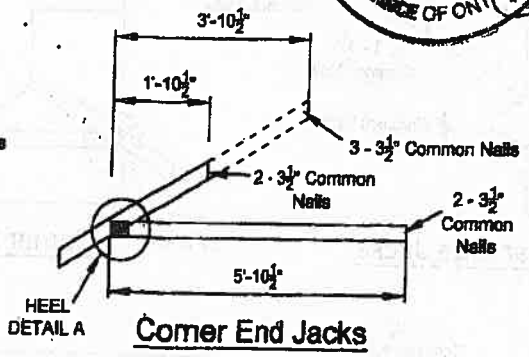
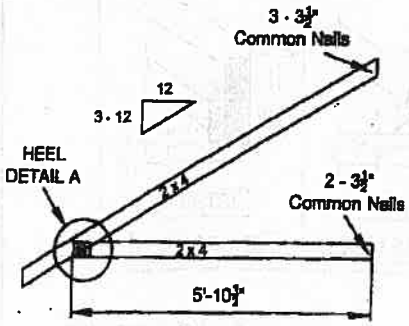


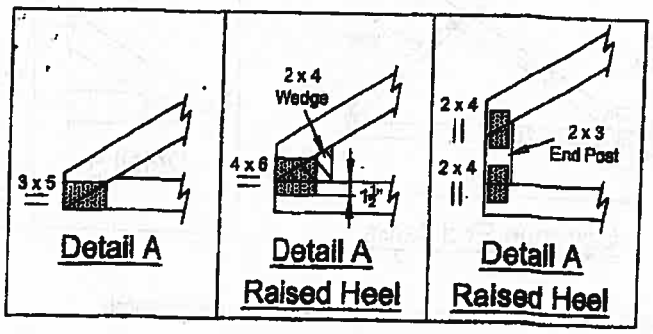
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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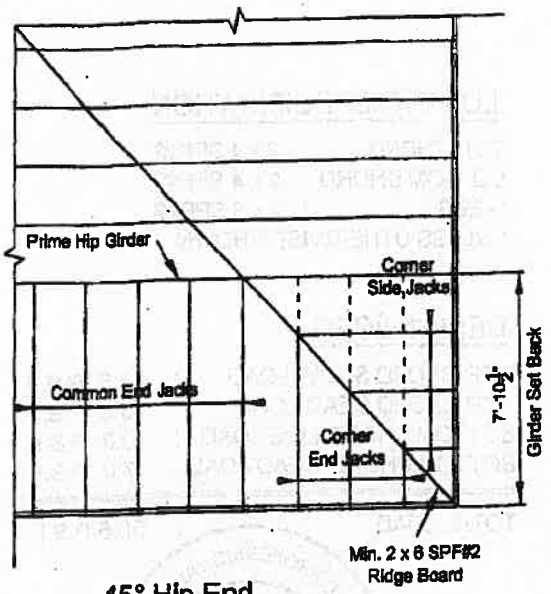

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

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06/11/2018
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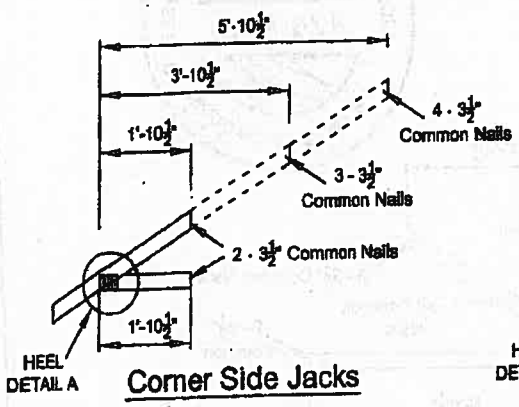
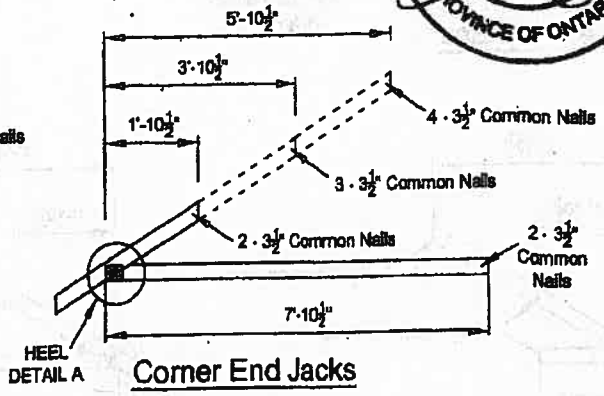
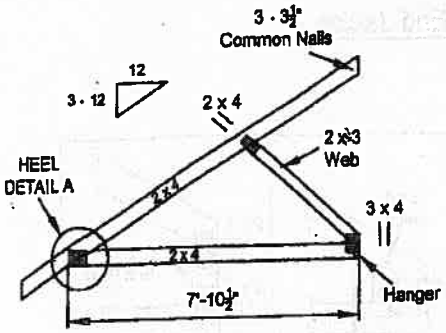


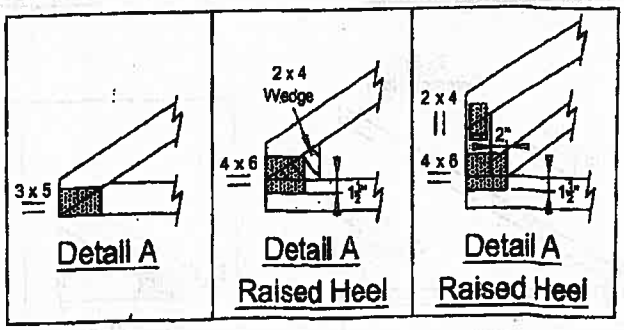
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

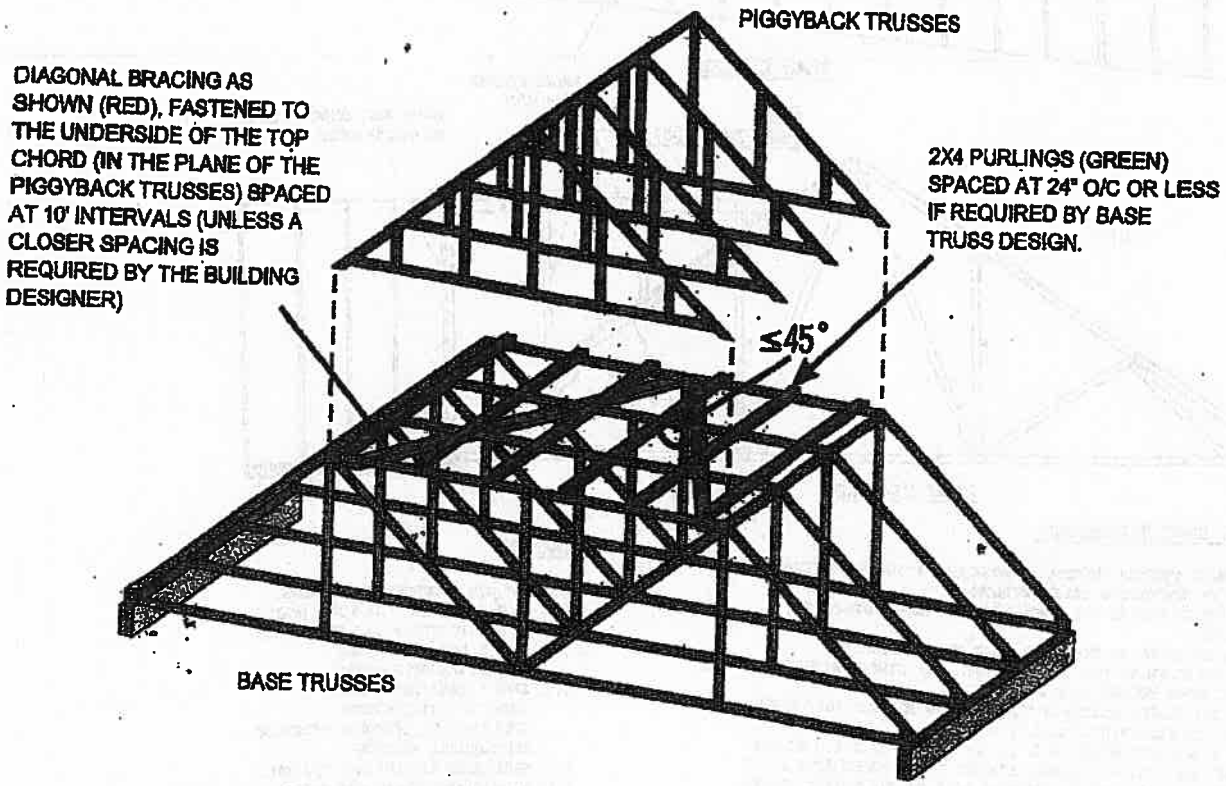
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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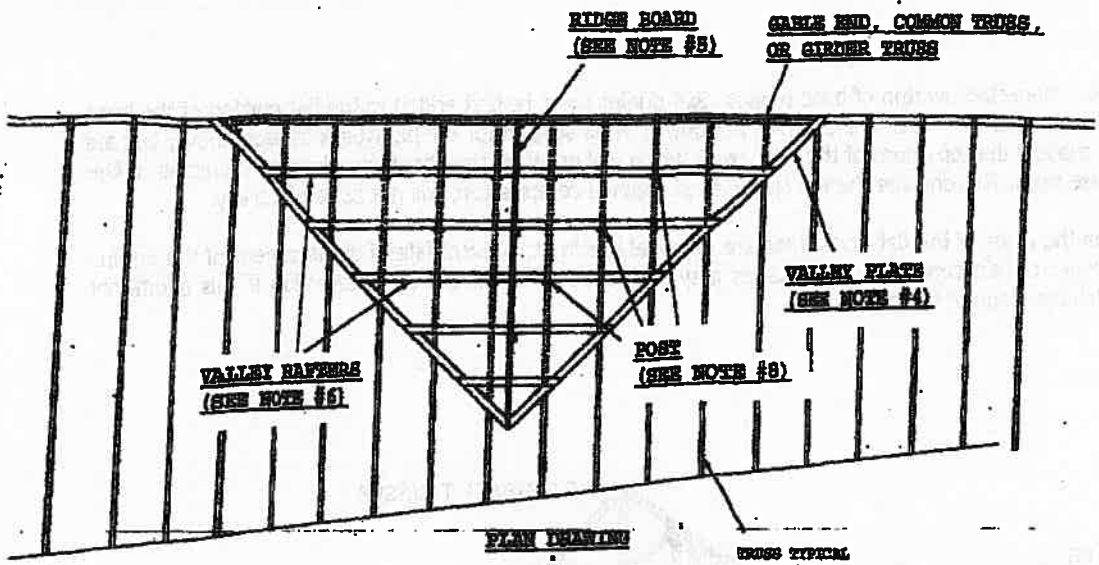
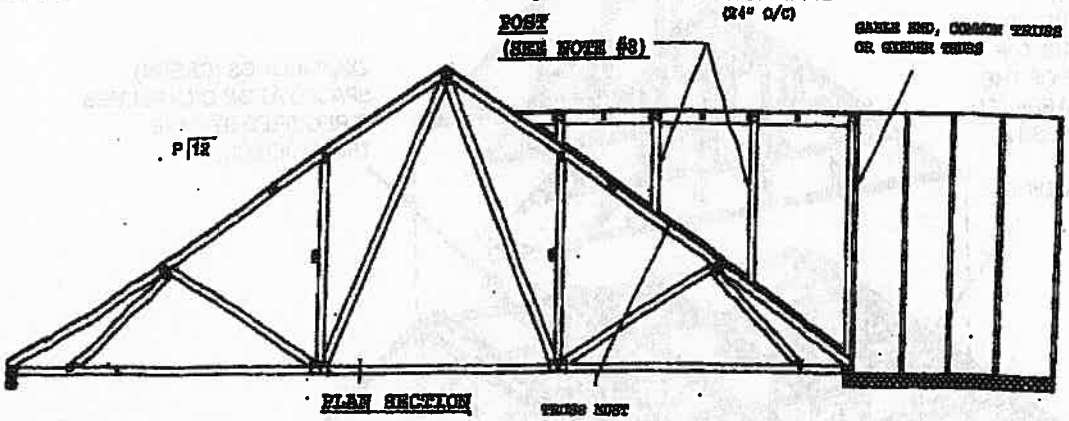
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY PUSHING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (3) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. LEVEL BOTTOM OF POST TO SIT EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 8" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 24.0 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

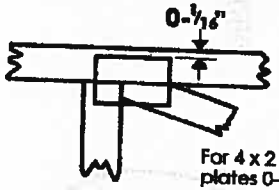
LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROV. OF ONTARIO

DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

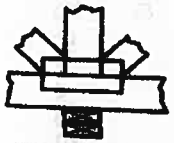
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T. I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

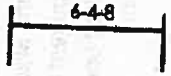
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

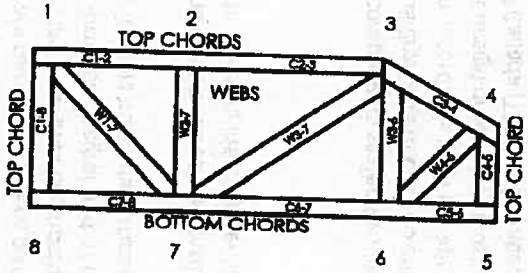
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T. I. or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

TOWN OF INNISFIL
2018-0833-0840
REVIEWED
Building Permit Staff

LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218 Feb 09, 2018