



Delivery Shiplist

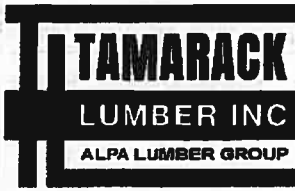
DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42087	LAYOUT ID: 301328	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: TH-6E	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	2	T105 HALF HIP	10.00 0.00	19-08-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	218.46 138.00		
	8	T106 PIGGYBACK	10.00 0.00	19-08-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	984.72 612.00		
	2	T107 HALF HIP	10.00 0.00	19-08-00	10-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	270.36 169.00		
	2	T110 COMMON	10.00 0.00	20-02-00	10-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	206.62 130.66		
	5	T110A COMMON	10.00 0.00	19-08-00	10-00-08	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	565.15 350.00		
	2	T111S SCISSOR	10.00 4.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	88.80 56.00		
	1	T112 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.11 39.50		
	1	G112 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	65.80 43.50		
	1 2 Ply	T120Z HIP GIRDER	10.00 0.00	19-08-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	203.18 132.66		
	1	T121 HIP	10.00 0.00	19-08-00	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	107.24 67.83		
	1	T122 HIP	10.00 0.00	19-08-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	109.81 70.50		
	1	T123 COMMON	10.00 0.00	19-08-00	10-00-06	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	113.03 70.00		
	1	T123C COMMON	10.00 0.00	19-08-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	112.77 69.33		
	2	P101 PIGGYBACK	10.00 0.00	10-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	69.60 47.34		
	2	P102 PIGGYBACK	10.00 0.00	10-00-15	02-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	72.24 47.66		
	2	P103 PIGGYBACK	10.00 0.00	10-00-15	03-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-06-08	75.80 49.00		
	3	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	45.69 29.01		
	1	J7 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	10.65 7.33		



Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 301328	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: TH-6E	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	8.37 6.00		
	6	J47 JACK-OPEN	4.00 0.00	07-01-00	02-08-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-07-12	127.50 79.98		

TOTAL # TRUSS= 46.00 TOTAL BFT OF ALL TRUSSES= 2215.30 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3517.90 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= 2.00

10/23/18	10/23/18	10/23/18	10/23/18
10/23/18	10/23/18	10/23/18	10/23/18



Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301329 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: TH-1 ELEVATION: A2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	10	T1 PIGGYBACK	8.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	2086.80 1263.30		
	2	G1 PIGGYBACK	8.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	435.38 267.00		
	1 4 Ply	T11 COMMON	10.00 0.00	10-08-00	06-03-08	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 00-07-04	332.08 208.00		
	1	G15 COMMON	10.00 0.00	11-02-00	06-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	54.28 37.00		
	2	T15A COMMON	10.00 0.00	10-08-00	06-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-03-08	105.28 70.34		
	12	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	204.24 135.96		
	6	J40 JACK-OPEN	4.00 0.00	03-11-00	01-07-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-07-02	65.04 40.02		
	7	J47 JACK-OPEN	4.00 0.00	07-01-00	02-08-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-07-12	148.75 93.31		

TOTAL # TRUSS= 44.00

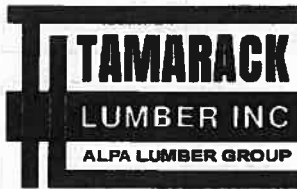
TOTAL BFT OF ALL TRUSSES=

2114.93 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3431.83 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= 5.00



Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK:42067	LAYOUT ID: 301330	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: TH-3	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	G1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-06 01-03-08	01-02-00 01-02-00	217.69 133.50		
	1	T21 HIP	6.00 0.00	43-10-00	08-02-07	2 X 6	2 X 6	01-03-06 01-03-08	01-02-00 01-02-00	257.69 152.50		
	1	T22 HIP	6.00 0.00	43-10-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	260.49 164.83		
	5	T23 PIGGYBACK	6.00 0.00	45-06-00	09-10-07	2 X 6	2 X 6	01-03-08 01-00-08	01-02-00 01-05-03	1465.70 880.85		
	1	G23 PIGGYBACK	6.00 0.00	45-06-00	09-10-07	2 X 6	2 X 6	01-03-06 01-00-08	01-02-00 01-05-03	285.27 179.00		
	3	T24 PIGGYBACK	6.00 0.00	43-10-00	09-04-03	2 X 6	2 X 6	01-03-06 01-03-08	01-02-00 01-02-00	822.27 491.49		
	4	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	68.08 45.32		
	1	P11 PIGGYBACK	6.00 0.00	07-01-10	01-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	22.64 15.17		
	4	P12 PIGGYBACK	6.00 0.00	07-01-10	02-01-09	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	83.44 57.32		
	1	P13 PIGGYBACK	6.00 0.00	07-01-10	00-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	20.07 14.67		
	6	J47 JACK-OPEN	4.00 0.00	07-01-00	02-08-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-07-12	127.50 79.98		

TOTAL # TRUSS= 28.00

TOTAL BFT OF ALL TRUSSES=

2214.63 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3650.84 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
40	Hangers	TBE6	

TOTAL # ITEMS= 40.00



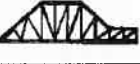
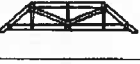
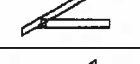
Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301331 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: TH-2 ELEVATION: A2 W/ FIREWALL

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-06	01-02-00 01-02-00	626.04 376.99		
	1	G1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	217.69 133.50		
	2	T2 PIGGYBACK	6.00 0.00	42-02-00	09-04-03	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	519.48 314.00		
	1	T3 ROOF	6.00 0.00	48-00-00	06-06-07	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 03-01-10	290.35 176.33		
	5	T4 PIGGYBACK	6.00 0.00	48-00-00	08-02-07	2 X 6	2 X 6	01-03-08 00-00-00	01-02-00 03-01-10	1441.65 881.65		
	6	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	102.12 67.98		
	1	P2 PIGGYBACK	6.00 0.00	12-01-10	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	38.75 24.83		
	1	P3 PIGGYBACK	6.00 0.00	12-01-10	03-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	35.92 23.00		
	2	P4 PIGGYBACK	6.00 0.00	12-01-10	03-04-09	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	67.92 41.66		
	2	J41 JACK-OPEN	4.00 0.00	04-01-00	01-08-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-08-04	22.44 14.66		
	4	J44 JACK-OPEN	4.00 0.00	05-07-00	02-02-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-12	58.96 37.32		
	6	J61 JACK-OPEN	6.00 0.00	04-05-00	02-09-04	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-09-04	78.84 52.02		

TOTAL # TRUSS= 34.00

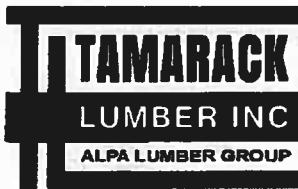
TOTAL BFT OF ALL TRUSSES=

2145.94 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3500.16 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
32	Hangers	TBE6	

TOTAL # ITEMS= 32.00



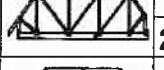
Delivery Shiplist

DATE	08/23/16
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301332 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: TH-6E ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	T104 HALF HIP	10.00 0.00	19-08-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	228.64 143.66		
	2	T105 HALF HIP	10.00 0.00	19-08-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	218.46 136.00		
	2	T106 PIGGYBACK	10.00 0.00	19-08-00	09-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	246.18 153.00		
	3	T107 HALF HIP	10.00 0.00	19-08-00	10-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	405.54 253.50		
	2	T110 COMMON	10.00 0.00	20-02-00	10-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	208.82 130.66		
	7	T110A COMMON	10.00 0.00	19-08-00	10-00-08	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	791.21 490.00		
	2	T111S SCISSOR	10.00 4.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	86.80 56.00		
	1 2 Ply	T115 HIP GIRDER	10.00 0.00	19-08-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	194.04 124.66		
	1	T116 HIP	10.00 0.00	19-08-00	05-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	95.24 61.00		
	1	T117 HIP	10.00 0.00	19-08-00	06-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	104.69 66.67		
	1	T118 HIP	10.00 0.00	19-08-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	105.65 68.00		
	1	T119 HIP	10.00 0.00	19-08-00	08-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	105.85 66.50		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	6	J47 JACK-OPEN	4.00 0.00	07-01-00	02-08-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-07-12	127.50 79.98		

TOTAL # TRUSS= 43.00

TOTAL BFT OF ALL TRUSSES=

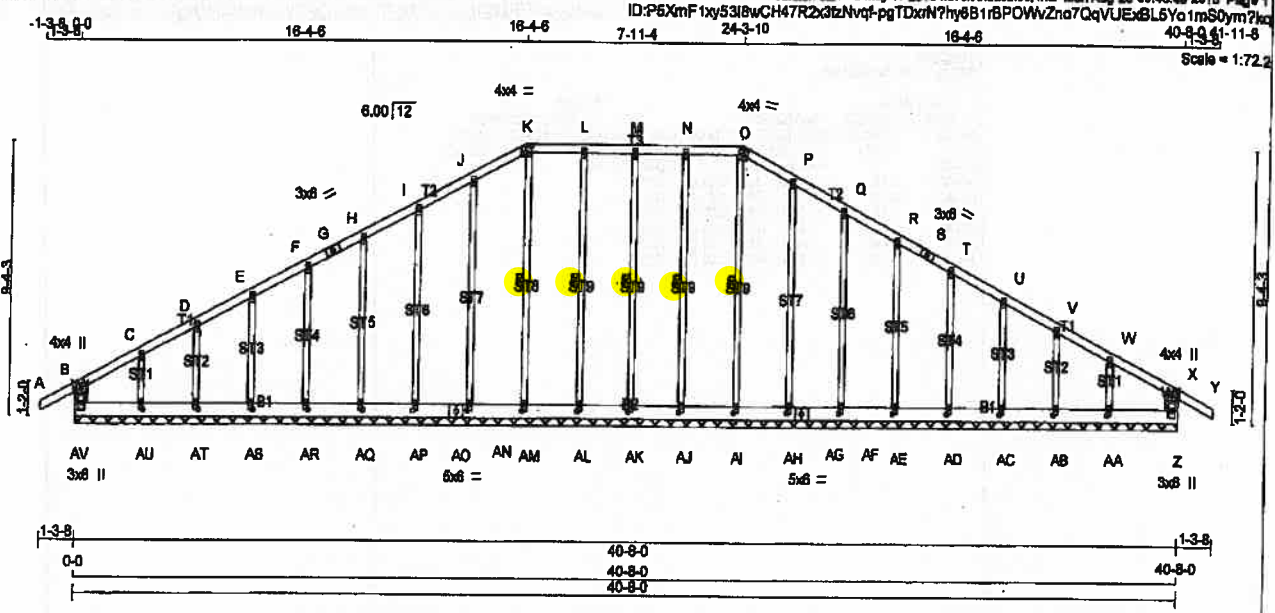
1951.00 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3101.11 LBS.

DWG NO. TAM 71805/74
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME G1	QUANTITY 8	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.110 S May 16 2018 Mitek Industries, Inc. Mon Aug 20 08:48:48 2018 Page 1



TOTAL WEIGHT = 8 X 216 = 1742 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AV - B	2x6	DRY No.2	SPF
A - G	2x4	DRY No.2	SPF
G - K	2x4	DRY No.2	SPF
K - O	2x4	DRY No.2	SPF
O - S	2x4	DRY No.2	SPF
S - Y	2x4	DRY No.2	SPF
Z - X	2x6	DRY No.2	SPF
AV - AO	2x6	DRY No.2	SPF
AO - AG	2x6	DRY No.2	SPF
AG - Z	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

PLATE TYPE	PLATES	W	LEN	Y	X
B TMW+P	MT20	4.0	4.0		
C, D, E, F, H, I, J, L, M, N, P, Q, R, T, U, V, W	MT20	2.0	4.0		
G TS4	MT20	3.0	6.0		
K TMW+P	MT20	4.0	4.0	2.50	2.00
O TMW+P	MT20	4.0	4.0		
S TS4	MT20	3.0	6.0		
X TMW+P	MT20	4.0	4.0		
Z BMV+P	MT20	3.0	6.0		
AA, AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU	MT20	2.0	4.0	2.50	1.00
AA BMV+P	MT20	5.0	6.0		
AG BS4	MT20	5.0	6.0		
AO BS4	MT20	5.0	6.0		
AV BMV+P	MT20	3.0	6.0		

DESIGNATION, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AI, N-AJ, M-AK, L-AL, K-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADINGS

TOTAL LOAD CASES: (4)

MEMB.	FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH (FT)	MEMB.	FACTORED	
		VERT. LOAD (LBS)	CS1 (LC)			FORCE (LBS)	MAX. CS1 (LC)
FR-TO							
AV-B	-367 / 0	0.0	0.0	0.02 (1)	AI-O	-253 / 0	0.14 (1)
A-B	0 / 39	-129.7	-129.7	0.17 (1)	AJ-N	-258 / 0	0.14 (1)
B-C	-68 / 0	-129.7	-129.7	0.06 (1)	AK-M	-235 / 0	0.13 (1)
C-D	-54 / 0	-129.7	-129.7	0.07 (1)	AL-L	-259 / 0	0.14 (1)
D-E	-45 / 0	-129.7	-129.7	0.06 (1)	AM-K	-253 / 0	0.14 (1)
E-F	-41 / 0	-129.7	-129.7	0.06 (1)	AN-J	-253 / 0	0.14 (1)
F-G	-37 / 0	-129.7	-129.7	0.06 (1)	AP-I	-259 / 0	0.14 (1)
G-H	-37 / 0	-129.7	-129.7	0.06 (1)	AQ-H	-258 / 0	0.16 (1)
H-I	-34 / 0	-129.7	-129.7	0.06 (1)	AR-F	-257 / 0	0.11 (1)
I-J	-32 / 0	-129.7	-129.7	0.06 (1)	AS-E	-259 / 0	0.07 (1)
J-K	-28 / 0	-129.7	-129.7	0.06 (1)	AT-D	-250 / 0	0.06 (1)
K-L	-23 / 0	-129.7	-129.7	0.06 (1)	AU-C	-279 / 0	0.04 (1)
L-M	-23 / 0	-129.7	-129.7	0.06 (1)	AV-P	-253 / 0	0.14 (1)
M-N	-23 / 0	-129.7	-129.7	0.06 (1)	AF-Q	-258 / 0	0.24 (1)
N-O	-23 / 0	-129.7	-129.7	0.06 (1)	AE-R	-258 / 0	0.16 (1)
O-P	-28 / 0	-129.7	-129.7	0.06 (1)	AD-T	-257 / 0	0.11 (1)
P-Q	-32 / 0	-129.7	-129.7	0.06 (1)	AC-U	-259 / 0	0.07 (1)
Q-R	-34 / 0	-129.7	-129.7	0.06 (1)	AB-V	-250 / 0	0.05 (1)
R-S	-37 / 0	-129.7	-129.7	0.06 (1)	AA-W	-279 / 0	0.04 (1)
S-T	-37 / 0	-129.7	-129.7	0.06 (1)			
T-U	-41 / 0	-129.7	-129.7	0.06 (1)			
U-V	-45 / 0	-129.7	-129.7	0.06 (1)			
V-W	-54 / 0	-129.7	-129.7	0.07 (1)			
W-X	-68 / 0	-129.7	-129.7	0.09 (1)			
X-Y	0 / 39	-129.7	-129.7	0.17 (1)			
Z-X	-367 / 0	0.0	0.0	0.02 (1)			
AV-AU	0 / 68	-38.5	-38.5	0.02 (1)			
AU-AT	0 / 46	-38.5	-38.5	0.02 (2)			
AT-AS	0 / 41	-38.5	-38.5	0.01 (2)			
AS-AR	0 / 37	-38.5	-38.5	0.01 (2)			
AR-AQ	0 / 33	-38.5	-38.5	0.01 (2)			
AQ-AP	0 / 31	-38.5	-38.5	0.01 (2)			
AP-AO	0 / 28	-38.5	-38.5	0.01 (2)			
AO-AN	0 / 28	-38.5	-38.5	0.01 (2)			
AN-AM	0 / 26	-38.5	-38.5	0.01 (2)			
AM-AL	0 / 23	-38.5	-38.5	0.01 (2)			
AL-AK	0 / 23	-38.5	-38.5	0.01 (2)			
AK-AJ	0 / 23	-38.5	-38.5	0.01 (2)			
AJ-AI	0 / 23	-38.5	-38.5	0.01 (2)			
AI-AH	0 / 26	-38.5	-38.5	0.01 (2)			
AH-AG	0 / 26	-38.5	-38.5	0.01 (2)			
AG-AF	0 / 26	-38.5	-38.5	0.01 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 81.7 PSF

SPACING = 24" IN GC

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.17/1.00 (X-Y:1), BC=0.02/1.00 (Z-AA:1)

WB=0.34/1.00 (J-AN:1), SS=0.12/1.00 (X-Y:1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	818	354	1667
	818	354	1667
	818	354	1667
	818	354	1667
	818	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.80 (O) (INPUT = 0.90)

JSI METAL=0.12 (W) (INPUT = 1.00)

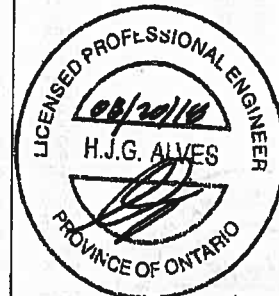
DWG NO. TAM 71005165
STRUCTURAL
COMPONENT ONLY 1/2



JOB NAME 297078	TRUSS NAME G1	QUANTITY 8	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Yamarack Roof Truss, Burlington				Version 8.210 8 May 18 2018 MITek Industries, Inc. Mon Aug 20 09:48:49 2018 Page 2 ID: P6XmF1w5318wCH47R237zNvc-paTDmN7hy8B1rBPOWwZno7QqVUExBL5Yo1m80ym77cp	

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AF-AE	0 / 31	-38.5	-38.5 0.01 (2)	10.00			
AE-AD	0 / 33	-38.5	-38.5 0.01 (2)	10.00			
AD-AC	0 / 37	-38.5	-38.5 0.01 (2)	10.00			
AC-AB	0 / 41	-38.5	-38.5 0.01 (2)	10.00			
AB-AA	0 / 46	-38.5	-38.5 0.02 (2)	10.00			
AA-Z	0 / 58	-38.5	-38.5 0.02 (1)	10.00			



DWG NO. TAM **T1005165**
STRUCTURAL
COMPONENT ONLY **3/2**



JOB NAME 297081	TRUSS NAME T2	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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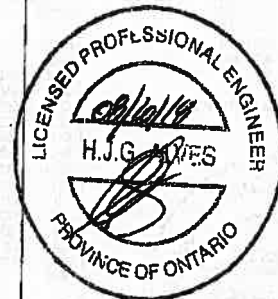
Tamersack Roof Truss, Burlington

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ID:81GT1TDT8XgHQV9ELneVWQzNtUl-Eu89Q2WenIZofNKhZQDOG7XQVEMossQainJ3ylyv8

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER MBCC 4.1.6.2(b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (0.6) PSF AT (40-0-0) FT-IN-6X REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-6X AWAY FROM EAVE.



DWG NO. TAM **11605273**
STRUCTURAL
COMPONENT ONLY **3/2**



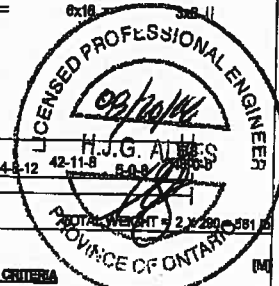
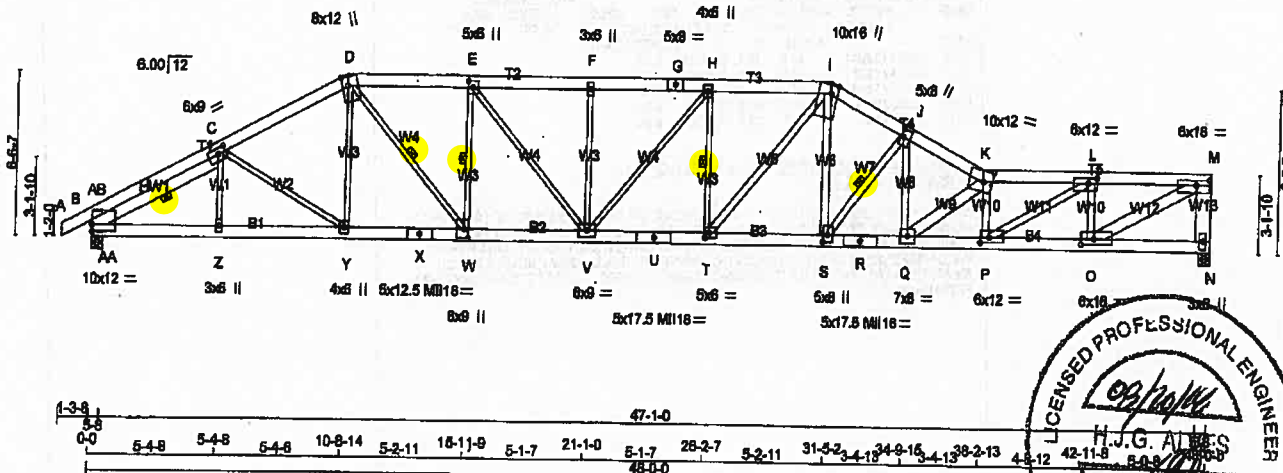
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297081	T3	2	1			

Tamarack Roof Truss, Burlington

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1-3-9-0 5-4-8 5-4-8 5-4-8 10-8-14 5-2-11 15-11-9 5-1-7 21-1-0 5-1-7 25-2-7 5-2-11 31-5-2 34-9-15 38-2-13 4-8-12 42-11-8 5-0-8 48-0-0

Scale = 1:84.0



LUMBER	SIZE	DESCR.
N.L.G.A. RULES		
CHORDS		
A - D	2x6 DRY	2100F 1.8E
D - G	2x6 DRY	2100F 1.8E
G - I	2x6 DRY	2100F 1.8E
I - K	2x6 DRY	2100F 1.8E
K - M	2x6 DRY	2100F 1.8E
N - M	2x6 DRY	No.2
B - X	2x6 DRY	2100F 1.8E
X - U	2x6 DRY	2100F 1.8E
U - R	2x6 DRY	2100F 1.8E
R - N	2x6 DRY	2100F 1.8E

REINFORCING MEMBERS	SIZE	DESCR.
HW1	2x6 DRY	No.2
ALL WEBS EXCEPT	2x3 DRY	No.2
T - I	2x4 DRY	No.2
S - I	2x4 DRY	No.2
P - K	2x4 DRY	No.2
P - L	2x4 DRY	2100F 1.8E
O - L	2x4 DRY	No.2
O - M	2x6 DRY	2100F 1.8E
Q - J	2x4 DRY	No.2
Q - K	2x6 DRY	No.2
S - J	2x4 DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
N	6013	0	5990	0
B	5251	0	5540	240

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1145 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1157 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES
SHALL BE PROVIDED BY BUILDING DESIGNER

PROVIDE FOR 240 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
N	3814	2857 / 0
B	3775	2824 / 0

HORIZONTAL REACTIONS	BEARING MATERIAL TO BE 6F NO.2 OR BETTER AT JOINT(S) N, B
B	0 / 0

BEARING MATERIAL TO BE 6F NO.2 OR BETTER AT JOINT(S) N, B

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.87 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-W, E-W, H-T, J-S, C-AA.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18).

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 2	-170.4 -170.4 0.07 (2)	C-Z	-20 / 226
B-AB	-8473 / 1280	-170.4 -170.4 0.28 (2)	C-Y	-445 / 618
AB-C	-3873 / 927	-170.4 -170.4 0.23 (2)	Y-D	-153 / 530
C-D	-7903 / 1702	-170.4 -170.4 0.33 (2)	D-W	-773 / 3727
D-E	-9048 / 2077	-170.4 -170.4 0.34 (3)	W-E	-2845 / 682
E-F	-10303 / 2325	-170.4 -170.4 0.38 (3)	E-V	-385 / 1853
F-G	-10303 / 2325	-170.4 -170.4 0.38 (3)	V-F	-478 / 281
G-H	-10303 / 2325	-170.4 -170.4 0.38 (3)	H-V	-482 / 503
H-I	-10820 / 2391	-170.4 -170.4 0.40 (3)	T-H	-1548 / 413
I-J	-10883 / 2391	-170.4 -170.4 0.27 (3)	S-I	-481 / 2215
J-K	-13401 / 2835	-170.4 -170.4 0.38 (3)	S-I	-633 / 8111
K-L	-15741 / 3277	-170.4 -170.4 0.54 (3)	P-K	-3688 / 625
L-M	-6518 / 2012	-170.4 -170.4 0.32 (3)	P-L	-1488 / 7258
N-M	-5482 / 1188	0.0 0.0 0.37 (3)	O-M	-2214 / 10774
B-AA	-852 / 3446	-38.5 -38.5 0.22 (2)	Q-J	-609 / 3183
AA-Z	-1818 / 7414	-38.5 -38.5 0.41 (2)	Q-K	-4889 / 1082
Z-Y	-1818 / 7412	-38.5 -38.5 0.42 (2)	S-J	-3558 / 839
Y-X	-1422 / 7045	-38.5 -38.5 0.40 (2)	AA-AB	-484 / 8187
X-W	-1422 / 7045	-38.5 -38.5 0.40 (2)	AA-C	-4810 / 828
W-V	-1813 / 8045	-38.5 -38.5 0.48 (3)		
V-U	-2032 / 10820	-38.5 -38.5 0.68 (3)		
U-T	-2032 / 10820	-38.5 -38.5 0.68 (3)		
T-S	-1848 / 9774	-38.5 -38.5 0.61 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 75.3 PSF

SPACING = 24.0 IN CG

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2012, CBC 2018
- CSA 088-08, CSA 088-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.80')
CALCULATED VERT. DEFL.(LL) = L/859 (0.87')
ALLOWABLE DEFL.(TL) = L/180 (3.20')
CALCULATED VERT. DEFL.(TL) = L/700 (0.82')

CS1 TO=0.54/1.00 (K-L-3), BC=0.82/1.00 (P-Q-3),
WB=0.84/0.90 (D-W-3), SS=0.38/1.00 (H-L-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354
MT18	488	274

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISL CRIP=0.90 (T) (INPUT = 0.90)
ISL CRIP=1.00 (Q) (INPUT = 1.00)

DRWG NO. TAM 774024-1
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2



JOB NAME 297081	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: SGT1TDTSXaHQN/BLneWQzNbuL-kv3tpy QIGRW6QVWhXY2NIQQ 6chQABInYevnd

LOADING
TOTAL LOAD CASES: (16)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
S-R	-2374 / 12011	-38.5	-38.5	0.85 (S)	8.25		
R-Q	-2374 / 12011	-38.5	-38.5	0.85 (S)	8.25		
Q-P	-3211 / 16841	-38.5	-38.5	0.92 (S)	3.67		
P-O	-1800 / 9818	-38.5	-38.5	0.82 (S)	6.25		
O-N	-22 / 58	-38.5	-38.5	0.04 (S)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.0) PSF AT (40-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-4) FT-IN-SX AWAY FROM EAVE.



DWG NO. TAM **11805275**
STRUCTURAL
COMPONENT ONLY **2/2**



JOB NAME 297081	TRUSS NAME T4	QUANTITY 10	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:8IGT1TDTSXgHQRV9BLnetWQzNtUL-9NR7QaSWHCo7Yabz3J18UmQW7bdeCvwOD4rEdYvddc

LOADING

TOTAL LOAD CASES: (18)

CHORDS					WEBB				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC LENGTH
FR-TO		FROM TO			FR-TO				
S-R	-2041 / 10452	-38.5	-38.5	0.54 (3)	6.25				
R-Q	-2041 / 10452	-38.5	-38.5	0.54 (3)	6.25				
Q-P	-3087 / 15820	-38.5	-38.5	0.91 (3)	5.75				
P-O	-1885 / 9172	-38.5	-38.5	0.82 (3)	6.25				
O-N	-22 / 58	-38.5	-38.5	0.05 (5)	6.25				

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NRCC 4.1 & 2.6**

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (6.6) PSF AT (40-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



DWG NO. TAM **11805276**
STRUCTURAL
COMPONENT ONLY **2/2**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297078	T11	2	4			

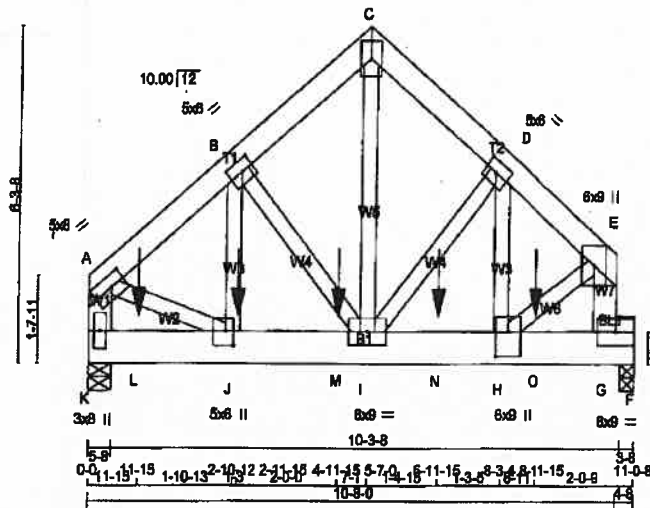
Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 08:47:27 2018 Page 1

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0-0 2-10-12 2-10-12 2-8-4 6-7-0 2-8-4 8-3-4 2-4-12 10-8-0

Scale = 1:39.4



TOTAL WEIGHT = 8 X 83 = 664 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	DESCR.
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
K - A	2x6	DRY	No.2
G - E	2x6	DRY	No.2
K - F	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2

ALL WEBS EXCEPT			
2x4	DRY	No.2	8PF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(P/LF)
TOP CHORDS : (0.122'x3') SPIRAL NAILS			
A-C	2	12	TOP
C-E	2	12	TOP
K-A	2	12	TOP
G-E	2	12	TOP
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS			
K-F	3	4	SIDE(1801.1)
WEBS : (0.122'x3') SPIRAL NAILS			
2x4	1	8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA088 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+1	MT20	5.0	8.0	2.50	3.25
B	TMWV+1	MT20	5.0	8.0	2.50	2.75
C	TTW+1p	MT20	5.0	8.0		
O	TMWV+1	MT20	5.0	8.0	2.50	2.75
E	TMWV+1p	MT20	8.0	9.0		
G	BMV+1c	MT20	8.0	9.0		
H	BMWV+1	MT20	8.0	9.0	5.50	3.00
I	BMWV+1	MT20	8.0	9.0		
J	BMWV+1	MT20	5.0	8.0	3.25	2.00
K	BMV+1p	MT20	3.0	8.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	VERT	JT	HORZ	JT	BRG	JT	BRG
K	10881	0	0	0	5-8	5-8	5-8
F	6837	0	0	0	3-8	3-8	3-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	JT	COMBINED
K	7779	4977 / 0	1231 / 0
F	6521	4174 / 0	1081 / 0

BEARING MATERIAL TO BE 8PF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	-8809 / 0	-129.7	-129.7	0.09 (1)	5.20
B-C	-8803 / 0	-129.7	-129.7	0.08 (1)	5.75
C-D	-8790 / 0	-129.7	-129.7	0.09 (1)	5.74
D-E	-8528 / 0	-129.7	-129.7	0.10 (1)	5.26
K-A	-8300 / 0	0.0	0.0	0.15 (1)	6.75
G-E	-10785 / 0	0.0	0.0	0.20 (1)	8.07
K-L	0 / 0	-38.5	-38.5	0.31 (1)	10.00
L-J	0 / 0	-38.5	-38.5	0.31 (1)	10.00
J-M	0 / 8717	-38.5	-38.5	0.31 (1)	10.00
M-I	0 / 8717	-38.5	-38.5	0.31 (1)	10.00
I-N	0 / 8480	-38.5	-38.5	0.34 (1)	10.00
N-H	0 / 8480	-38.5	-38.5	0.34 (1)	10.00
H-O	-1677 / 0	-38.5	-38.5	0.71 (1)	6.25
O-G	-1677 / 0	-38.5	-38.5	0.71 (1)	6.25
G-F	0 / 0	-168.2	-168.2	0.51 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE	
JT	LOC.	LC1	MAX.	JT	BACK	JT	VERT
J	2-11-15	-2583	-2583				TOTAL
L	11-15	-2583	-2583				TOTAL
M	4-11-15	-2585	-2585				TOTAL
N	6-11-15	-2583	-2583				TOTAL
O	8-11-15	-2583	-2583				TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 38.2 PSF
CL.	= 8.0 PSF
BOT CH.	LL = 10.5 PSF
CL.	= 7.0 PSF
TOTAL LOAD	= 61.7 PSF

SPACING = 24.0 IN. C.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 088-08, CSA 088-14
- TPC 2011, TPC 2014

(65 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: $TC=0.20/1.00$ (E-G-1), $BC=0.71/1.00$ (G-H-1), $WB=0.38/1.00$ (E-H-1), $SS=0.85/1.00$ (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

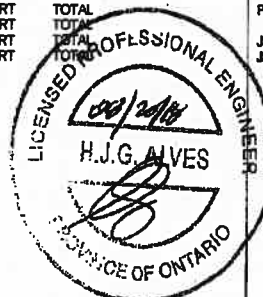
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	394	1687
	618	394	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (J) (INPUT = 0.80)
JSI METAL= 0.61 (H) (INPUT = 1.00)



DRWG NO. TAM 11805175
STRUCTURAL
COMPONENT ONLY

1/2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297078	T11	2	4			

Tamarack Roof Truss, Burlington

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ID:P5XmF1xy53BwCH47R2z6fzNvgfAAKzBA/TWx1fCqgv8C6ls0E3REdbagHqWM.LLFym7kD

HANDWRITTEN NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3541.8 lbs FACTORED DOWN AT 11-15, 3541.7 lbs FACTORED DOWN AT 9-11-15, 3544.8 lbs FACTORED DOWN AT 4-11-15, AND 3541.7 lbs FACTORED DOWN AT 8-11-15, AND 3541.7 lbs FACTORED DOWN AT 8-11-15 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 71805175
STRUCTURAL
COMPONENT ONLY 3/2

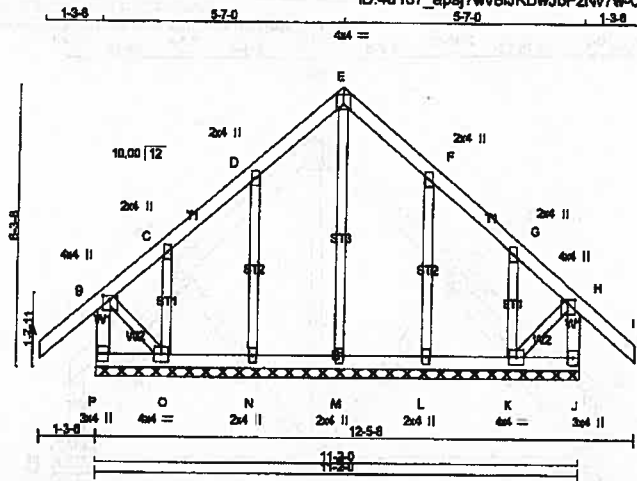
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:4u187_apej7wv8fJKDwJbPzNv7w-Qr6mc3x8_CFXohkAc9s_POlpYYHc3R1rVW7RvylbgX

Scale = 1:48



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (Plate is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR **CONTINUOUS BEARINGS**.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM TO			FR-TO				
P-B	-389 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-194 / 0	0.12 (1)	
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	N-O	-314 / 0	0.10 (1)	
B-C	-78 / 0	-129.7	-129.7	0.17 (1)	8.25	O-C	-122 / 0	0.02 (1)	
C-D	-8 / 0	-129.7	-129.7	0.08 (1)	10.00	L-F	-314 / 0	0.10 (1)	
D-E	-33 / 0	-129.7	-129.7	0.08 (1)	8.25	K-G	-122 / 0	0.02 (1)	
E-F	-33 / 0	-129.7	-129.7	0.08 (1)	8.25	B-O	0 / 29	0.01 (1)	
F-G	-8 / 0	-129.7	-129.7	0.08 (1)	10.00	K-H	0 / 29	0.01 (1)	
G-H	-78 / 0	-129.7	-129.7	0.17 (1)	8.25				
H-I	0 / 57	-129.7	-129.7	0.18 (1)	10.00				
J-H	-389 / 0	0.0	0.0	0.04 (1)	7.81				
P-D	0 / 0	-38.5	-38.5	0.02 (3)	10.00				
O-N	0 / 18	-38.5	-38.5	0.02 (3)	10.00				
N-M	0 / 11	-38.5	-38.5	0.02 (3)	10.00				
M-L	0 / 11	-38.5	-38.5	0.02 (3)	10.00				
L-K	0 / 18	-38.5	-38.5	0.02 (3)	10.00				
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00				

TOTAL WEIGHT = 2 X 54 = 108 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	38.2	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TO=0.18/1.00 (A-B:1), BC=0.02/1.00 (K-L:3), WB=0.12/1.00 (E-M:1), SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.80)
JSI METAL= 0.17 (D) (INPUT = 1.00)

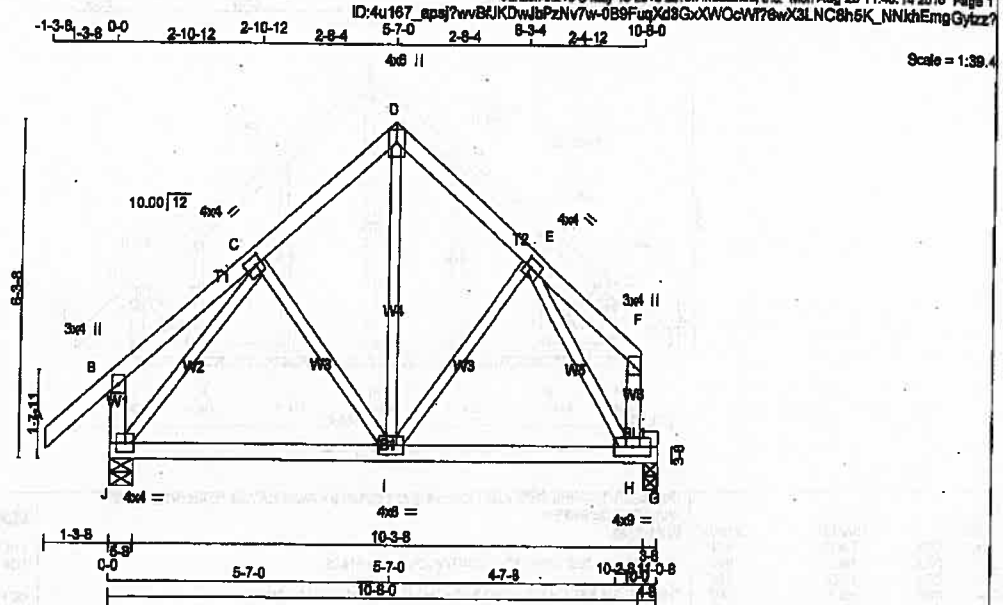


DRWG NO. TAM 11805545
STRUCTURAL
COMPONENT ONLY



JOB NAME 297079	TRUSS NAME T15A	QUANTITY 5	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 5 X 53 = 263 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - G	2x4	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0
C	TMVW+4	MT20	4.0	4.0 2.00 1.50
D	TTW+p	MT20	4.0	8.0 Edge
E	TMVW+4	MT20	4.0	4.0 2.00 1.50
F	TMV+p	MT20	3.0	4.0
H	BMVW+4	MT20	4.0	8.0
I	BMVW+4	MT20	4.0	8.0
J	BMVW+4	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SK	IN-SK	
G	881	0	881	0	3-8	3-8	
J	1107	0	1107	0	5-8	5-8	

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS	
1ST LCASE			
JT COMBINED	SNOW	LIVE	PERM. LIVE
G	649	394 / 0	116 / 0
J	803	827 / 0	116 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 57	C-I	-138 / 29
B-C	0 / 25	D	0 / 488
C-D	-634 / 0	E	0 / 70
D-E	-630 / 0	F	-909 / 0
E-F	0 / 37	G	-931 / 0
F-G	-319 / 0	H	0 / 0
G-H	-103 / 0	I	0 / 0
H-I	0 / 553	J	0 / 0
I-H	0 / 488		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TRC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (LL) = $L/988$ (0.12")
ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (TL) = $L/985$ (0.19")

CSI: TC=0.18/1.00 (A-B-1), BC=0.68/1.00 (H-I), WB=0.30/1.00 (C-D-1), SS=0.34/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 BHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
	(FBI)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.82 (H) (INPUT = 0.80)
JSI METAL= 0.33 (C) (INPUT = 1.00)



DWG NO. TAM **1185223**
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 7180522
STRUCTURAL
COMPONENT ONLY



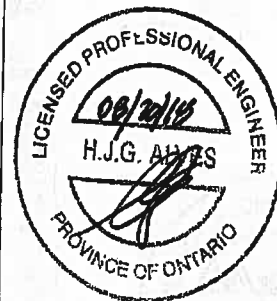
JOB NAME 297079	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tanarack Roof Truss, Burlington

Version 8.210 8 May 18 2018 Mitek Industries, Inc. Mon Aug 20 11:48:28 2018 Page 2
ID:4u187 epsi7wBUKDwJbPznV7wVmpPnsEYsGvEXaMWNk0brhzak8KedVZ8Q4Zvzr

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.8.2.6)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.5) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE.



DWG NO. TAM **11805226**
STRUCTURAL
COMPONENT ONLY **2/2**



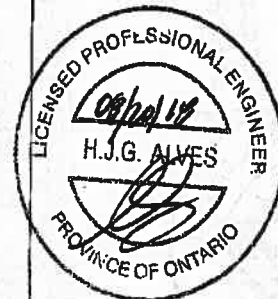
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T22	2	1	TRUSS DESC.	

Tamack Roof Truss, Burlington

Version 6.210 8 May 18 2015 MITek Industries, Inc. Mon Aug 20 11:48:30 2018 Page 2
ID:4u167_sps7wyBjKdWjPzNv7w-YG7IFJmAYGRabbMOCqBR7daEa47OkP88cDKv2r

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.9.2.6

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (0.6) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE.



DWG NO. TAM 11805227
STRUCTURAL
COMPONENT ONLY 3/4



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	T23	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 11:48:35 2018 Page 2
ID:4u167_apsi?wvBz/KDwJbPzNy7w-uEwB10oo7lbZYgZQv_ruUln8bu8MzTZSoNuYVyzg

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
V-U	-1081 / 8893	-38.5	-38.5	0.38 (1)	6.25		
U-T	-1081 / 8893	-38.5	-38.5	0.38 (1)	6.25		
T-S	-1412 / 7803	-38.5	-38.5	0.41 (1)	6.25		
S-R	-1718 / 8899	-38.5	-38.5	0.56 (3)	6.25		
R-Q	-887 / 3278	-38.5	-38.5	0.29 (3)	6.25		
Q-P	-9 / 19	-38.5	-38.5	0.07 (3)	10.00		

JSI GRIP= 0.80 (Y) (INPUT = 0.80)
JSI METAL= 0.85 (Y) (INPUT = 1.00)

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (6)**

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.9) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_{pe}$ BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN CATEGORY 2. BUILDING MAY BE LOCATED ON (OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

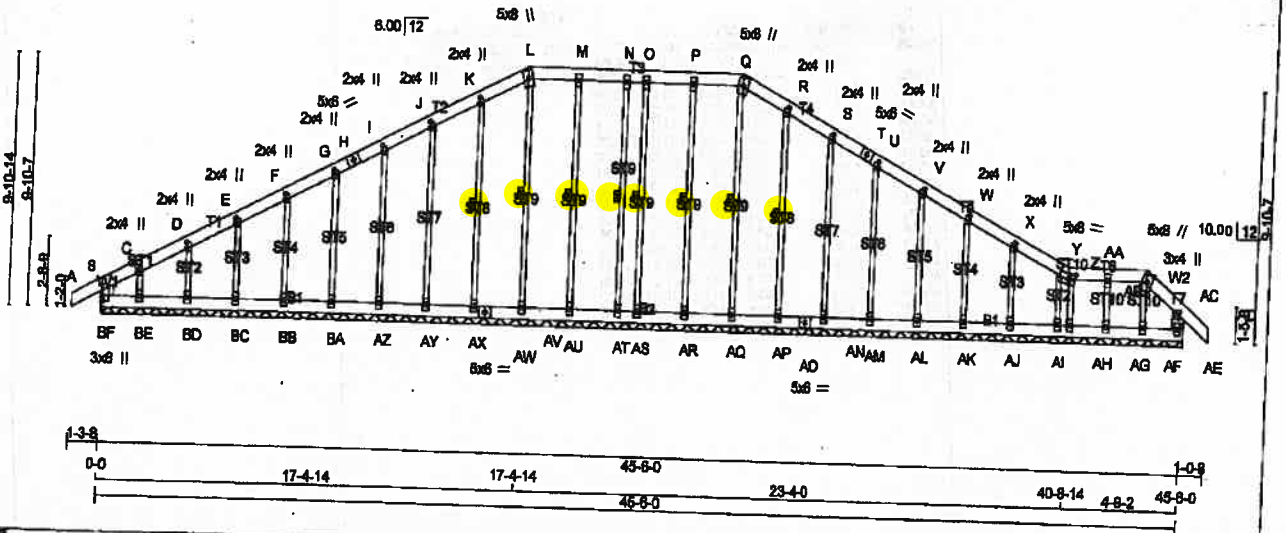


DRWG NO. TAM 11805228
STRUCTURAL
COMPONENT ONLY 3/2



JOB NAME 297081 Tamarack Roof Truss, Burlington	TRUSS NAME G23	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:07:30 2018 Page 1
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Scale = 1:82.7



LUMBER	
N.L.G.A. RULES	
CHORDS	
BF - B	2x6 DRY No.2
A - H	2x6 DRY No.2
H - L	2x6 DRY No.2
L - Q	2x6 DRY No.2
Q - T	2x6 DRY No.2
T - Z	2x6 DRY No.2
Z - AB	2x6 DRY No.2
AB - AD	2x6 DRY No.2
AD - AC	2x6 DRY No.2
AC - AW	2x6 DRY No.2
AW - AO	2x6 DRY No.2
AO - AE	2x6 DRY No.2
ALL WEBS	2x6 DRY No.2
ALL GABLE WEBS	2x6 DRY No.2
DRY: SEASONED LUMBER.	
GABLE STUDS SPACED AT	2-0-0 OC.

DESIGNER
BUILDING DESIGNER

DESIGNER'S NOTES

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-AQ, P-AR, O-AS, N-AT, M-AU, K-AX, R-AP, L-AV.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 6.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. GC

LOADING IN ALL PLATE SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2018
- CSA 086-08, CSA 086-14
- TFC 2011, TFC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. Q.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CB: TC=0.08/1.0 (A-B:1), BC=0.04/1.0 (BE-BF:1), WE=0.27/1.0 (S-AN:1), SB=0.10/1.0 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSD)	(PL)	(PL)	(PL)
MT20	518	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.29 (C) (INPUT = 0.80)

JSI METAL= 0.14 (AC) (INPUT = 1.00)

PLATES (Table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B, Y, Z, BF				
B				
C, D, E, F, G, I, J, K, R, S, U, V, W, X				
C	MT20	2.0	4.0	3.00
H	TS4	5.0	6.0	
I	TTWm	5.0	6.0	
M, N, O, P, AA				
M	MT20	3.0	8.0	
Q	TS4	5.0	6.0	
T	TS4	5.0	6.0	
Z	TTWm	5.0	8.0	2.75 3.00
AB	TTWm	5.0	8.0	Edges
AC	TMV+P	3.0	4.0	
AE	BMV+P	3.0	8.0	
AF, AG, AH, AI, AJ, AK, AL, AM, AN, AP, AQ, AR, AS, AT, AU, AV, AX, AY, AZ, BA, BB, BC, BD, BE				
AF	BMV+P	3.0	8.0	
AO	BS4	5.0	6.0	
AW	BS4	5.0	6.0	
BF	TMV+P	3.0	8.0	2.75 0.25

CHORDS		FACTORED		WEBS	
MEMB.	MAX FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX CBI (LC)	MEMB.	MAX FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
BF-B	-388 / 0	0.0 0.0 0.04 (1)	7.81	AQ-Q	-285 / 0
A-B	0 / 40	-128.7 -128.7 0.09 (1)	10.00	AR-P	-260 / 0
B-C	-95 / 0	-128.7 -128.7 0.08 (1)	6.25	AS-O	-188 / 0
C-D	-48 / 0	-128.7 -128.7 0.03 (1)	6.25	AT-N	-188 / 0
D-E	-42 / 0	-128.7 -128.7 0.03 (1)	6.25	AL-M	-260 / 0
E-F	-33 / 0	-128.7 -128.7 0.03 (1)	6.25	AX-K	-260 / 0
F-G	-26 / 0	-128.7 -128.7 0.03 (1)	6.25	AY-J	-267 / 0
G-H	-21 / 0	-128.7 -128.7 0.03 (1)	6.25	AZ-I	-267 / 0
H-I	-21 / 0	-128.7 -128.7 0.03 (1)	6.25	BA-G	-267 / 0
I-J	-18 / 0	-128.7 -128.7 0.03 (1)	6.25	BB-F	-260 / 0
J-K	-12 / 0	-128.7 -128.7 0.03 (1)	6.25	BC-E	-254 / 0
K-L	-9 / 0	-128.7 -128.7 0.03 (1)	10.00	BD-D	-270 / 0
L-M	-5 / 0	-128.7 -128.7 0.03 (1)	10.00	BE-C	-138 / 0
M-N	-5 / 0	-128.7 -128.7 0.03 (1)	10.00	AP-R	-258 / 0
N-O	-5 / 0	-128.7 -128.7 0.03 (1)	10.00	AN-S	-257 / 0
O-P	-5 / 0	-128.7 -128.7 0.03 (1)	10.00	AM-U	-257 / 0
P-Q	-5 / 0	-128.7 -128.7 0.03 (1)	10.00	AL-V	-257 / 0
Q-R	-9 / 0	-128.7 -128.7 0.03 (1)	10.00	AK-W	-258 / 0
R-S	-12 / 0	-128.7 -128.7 0.03 (1)	6.25	AJ-X	-266 / 0
S-T	-16 / 0	-128.7 -128.7 0.03 (1)	6.25	AI-Y	-148 / 0
T-U	-18 / 0	-128.7 -128.7 0.03 (1)	6.25	AG-AA	-204 / 0
U-V	-21 / 0	-128.7 -128.7 0.03 (1)	6.25	AF-AB	-114 / 0
V-W	-28 / 0	-128.7 -128.7 0.03 (1)	6.25	AH-Z	-157 / 0
W-X	-32 / 0	-128.7 -128.7 0.03 (1)	6.25	AV-L	-285 / 0
X-Y	-41 / 0	-128.7 -128.7 0.03 (1)	6.25		
Y-Z	-48 / 0	-128.7 -128.7 0.03 (1)	6.25		
Z-AA	-58 / 0	-128.7 -128.7 0.03 (1)	6.25		
AA-AB	-58 / 0	-128.7 -128.7 0.02 (1)	6.25		
AB-AC	-58 / 0	-128.7 -128.7 0.06 (1)	6.25		
AC-AD	0 / 49	-128.7 -128.7 0.07 (1)	10.00		
AE-AC	-339 / 0	0.0 0.0 0.03 (1)	7.81		
BF-BE	0 / 89	-38.5 -38.5 0.04 (1)	10.00		
BE-BD	0 / 49	-38.5 -38.5 0.02 (2)	10.00		
BD-BC	0 / 37	-38.5 -38.5 0.02 (2)	10.00		
BC-BB	0 / 29	-38.5 -38.5 0.01 (2)	10.00		
BB-BA	0 / 23	-38.5 -38.5 0.01 (2)	10.00		
BA-AZ	0 / 18	-38.5 -38.5 0.01 (2)	10.00		
AZ-AY	0 / 14	-38.5 -38.5 0.01 (2)	10.00		
AY-AX	0 / 11	-38.5 -38.5 0.01 (2)	10.00		
AX-AW	0 / 8	-38.5 -38.5 0.01 (3)	10.00		
AW-AV	0 / 8	-38.5 -38.5 0.01 (3)	10.00		
AV-AU	0 / 5	-38.5 -38.5 0.01 (3)	10.00		

DWG NO. TAM **71805264**

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2



JOB NAME 297081	TRUSS NAME G23	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DWG NO.
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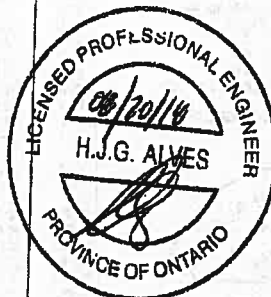
Tamarack Roof Truss, Burlington

Version 8.210 5 May 16 2016 Affek Industries, Inc. Mon Aug 20 13:07:30 2016 Page 2
ID: 81GT1TDTSXqHGV8BLneVQzNtUL-16XD-446v0Y818HBbG-RD1?KRMJswzHQNdJCabyvch

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO	0/8	-38.5	-38.5 0.01 (8)	10.00			
AIU-AT	0/8	-38.5	-38.5 0.01 (8)	10.00			
AT-AS	0/8	-38.5	-38.5 0.01 (8)	10.00			
AS-AR	0/8	-38.5	-38.5 0.01 (8)	10.00			
AR-AQ	0/8	-38.5	-38.5 0.01 (8)	10.00			
AQ-AP	0/8	-38.5	-38.5 0.01 (8)	10.00			
AP-AO	0/11	-38.5	-38.5 0.01 (2)	10.00			
AO-AN	0/11	-38.5	-38.5 0.01 (2)	10.00			
AN-AM	0/14	-38.5	-38.5 0.01 (2)	10.00			
AM-AL	0/16	-38.5	-38.5 0.01 (2)	10.00			
AL-AK	0/23	-38.5	-38.5 0.01 (2)	10.00			
AK-AJ	0/29	-38.5	-38.5 0.01 (2)	10.00			
AJ-AI	0/37	-38.5	-38.5 0.01 (2)	10.00			
AI-AH	0/43	-38.5	-38.5 0.01 (2)	10.00			
AH-AG	0/38	-38.5	-38.5 0.01 (1)	10.00			
AG-AF	0/36	-38.5	-38.5 0.01 (1)	10.00			
AF-AE	0/41	-38.5	-38.5 0.03 (1)	10.00			



DWG NO. TAM **11805264**
STRUCTURAL
COMPONENT ONLY
3/2

CONTINUED ON PAGE 2



JOB NAME 297079	TRUSS NAME T24	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

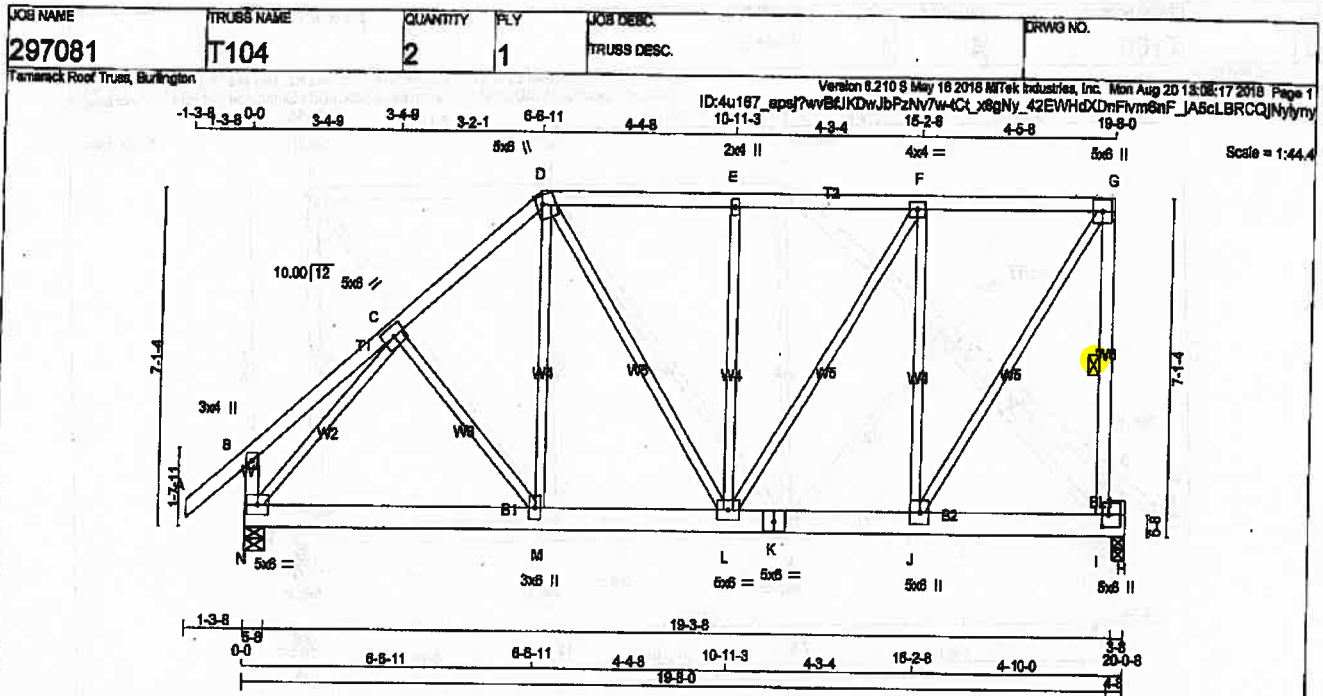
Version 8.210 8 May 18 2018 MTEK Industries, Inc. Mon Aug 20 11:48:38 2018 Page 2
ID:4u167_sps?wvBf/KDwbPzNv7w-n798OrJAx5_0E9Kd3n2KVQDEshBe3U4oe1JyZnd

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.14.2 (b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (8.6) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE.



DWG NO. TAM **7805229**
STRUCTURAL
COMPONENT ONLY **3/2**



TOTAL WEIGHT = 2 X 113 = 227 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
K - H	2x8	DRY	No.2	SPF	

BEARING BLOCKS	BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT		2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+H	MT20	5.0	6.0		
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	2.0	4.0		
F	TMVW+H	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0		
I	BMVW+p	MT20	5.0	6.0	3.00	1.75
J	BMVW+H	MT20	5.0	6.0		
K	BS4	MT20	5.0	6.0		
L	BMVW+H	MT20	8.0	8.0		
M	BMVW+H	MT20	2.0	6.0		
N	BMVW+H	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	JT	HORIZ	JT	BRG	JT	BRG
N	1884	0	1884	0	5-8	5-8	5-8
H	1637	0	1637	0	3-8	3-8	3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	1204	738 / 0	210 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 57	-128.7	-128.7 0.18 (1)	10.00	C-M	-88 / 70	0.03 (1)
B-C	0 / 30	-128.7	-128.7 0.21 (1)	10.00	M-D	0 / 353	0.06 (2)
C-D	-1498 / 0	-128.7	-128.7 0.25 (1)	5.09	N-C	-1834 / 0	0.75 (1)
D-E	-1287 / 0	-128.7	-128.7 0.25 (1)	5.27	J-G	0 / 1888	0.42 (1)
E-F	-1288 / 0	-128.7	-128.7 0.36 (1)	5.25	D-L	0 / 251	0.06 (1)
F-G	-892 / 0	-128.7	-128.7 0.35 (1)	5.77	J-F	-1058 / 0	0.87 (1)
G-H	-1781 / 0	0.0	0.0 0.39 (1)	4.99	L-E	-604 / 0	0.49 (1)
H-I	-343 / 0	0.0	0.0 0.04 (1)	7.61	L-F	0 / 814	0.12 (1)
N-M	0 / 1169	-38.5	-38.5 0.24 (2)	10.00			
M-L	0 / 1131	-38.5	-38.5 0.23 (2)	10.00			
L-K	0 / 982	-38.5	-38.5 0.29 (1)	10.00			
K-J	0 / 882	-38.5	-38.5 0.28 (1)	10.00			
J-I	-39 / 0	-38.5	-38.5 0.57 (1)	8.25			
I-H	0 / 0	-38.5	-38.5 0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2012, CBC 2018
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(85 % OF 84.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.67")
CALCULATED VERT. DEF. (LL) = L/969 (0.06")
ALLOWABLE DEF. (TL) = L/360 (0.67")
CALCULATED VERT. DEF. (TL) = L/969 (0.10")

CS: TO=0.39/1.00 (3:1), BO=0.57/1.00 (1:1), WB=0.87/1.00 (F:1), SS=0.60/1.00 (1:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

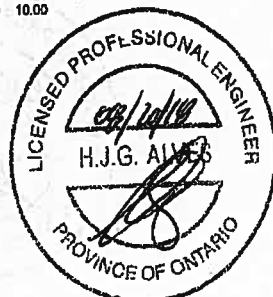
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1987 1888

PLATE PLACEMENT TOL = 0.280 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (3) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)



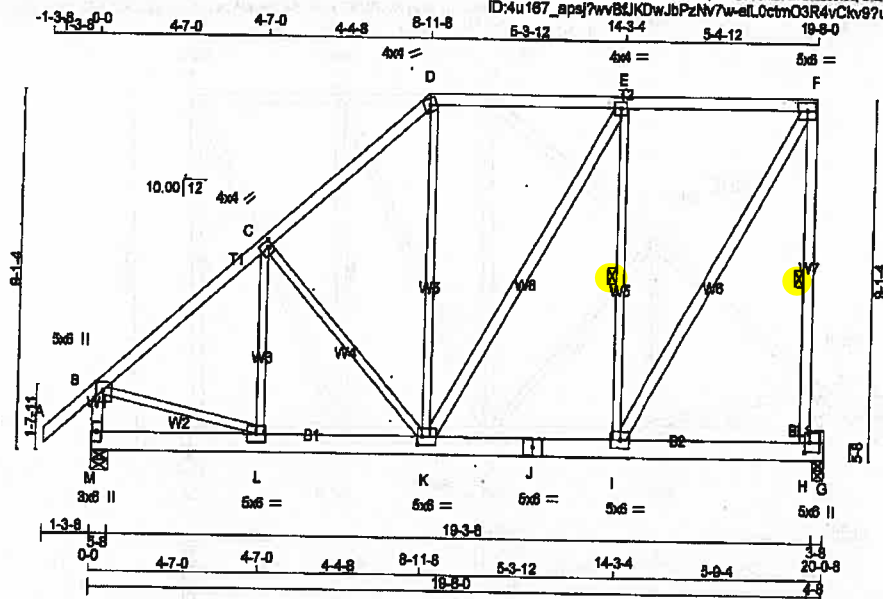
DWG NO. TAM 1160 5271
STRUCTURAL
COMPONENT ONLY

WSG NO. TAM **T1805278**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T106	10	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

Version 8.210 8 May 18 2018 MediaTek Industries, Inc. Mon Aug 20 13:08:25 2018 Page 1
ID:4u167_apsj7wv8UKDwJbPzNv7w-efL0ctmQ3R4vCkv89uw70b5PbUlb2a8W0h8R0vylyng
5.3-13 14-3-4 5.4-12 18-8-0

Scale = 1:53.4



N. L. Q. A. RULES			LUMBER
CHORDS	SIZE		
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS EXCEPT			
K - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (note in in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMWV+p	MT20	5.0	8.0	2.60 2.25
C	TMWV-1	MT20	4.0	4.0	2.00 1.25
D	TTW-m	MT20	4.0	4.0	
E	TMWV-1	MT20	4.0	4.0	
F	TMWV-t	MT20	8.0	8.0	
H	BMVW+p	MT20	5.0	6.0	3.00 1.75
I	BMWV-1	MT20	5.0	8.0	
J	BB-1	MT20	5.0	6.0	
K	BMWVW-1	MT20	5.0	6.0	2.50 1.50
L	BMWV-1	MT20	5.0	8.0	
M	BMV-1+p	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1884	0	1884	0	0	5-8	5-8
G	1637	0	1637	0	0	3-8	3-8

UNFACTORED REACTIONS

197 LCASE		MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1369	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0/57	-128.7	-128.7	0.18 (1)	10.00	L-C	-137 / 120	0.06 (1)
B-C	-1986 / 0	-128.7	-128.7	0.52 (1)	4.61	C-K	-453 / 0	0.37 (1)
C-D	-1286 / 0	-128.7	-128.7	0.49 (1)	8.03	K-E	0 / 332	0.07 (2)
D-E	-855 / 0	-128.7	-128.7	0.38 (1)	5.18	D-E	0 / 198	0.02 (1)
E-F	-678 / 0	-128.7	-128.7	0.85 (1)	5.38	E-E	-688 / 0	0.51 (1)
H-F	-1736 / 0	0.0	0.0	0.94 (1)	3.05	F-F	0 / 1727	0.28 (1)
M-B	-1788 / 0	0.0	0.0	0.19 (1)	8.25	B-L	0 / 1296	0.28 (1)
M-L	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
L-K	0 / 1252	-38.5	-38.5	0.20 (1)	10.00			
K-J	0 / 878	-38.5	-38.5	0.31 (1)	10.00			
J-I	0 / 878	-38.5	-38.5	0.31 (1)	10.00			
I-H	-28 / 0	-38.5	-38.5	0.57 (1)	8.25			
H-G	0 / 0	-38.5	-38.5	0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.68")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CS1 TC=0.66/1.00 (D-E:1), BC=0.57/1.00 (H-I:1),
WB=0.51/1.00 (E-I:1), SS1=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PL)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	518	354	1887	788	1987	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)



DWG NO. TAM 1805279
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM **71805280**
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T110A	12	1	TRUSS DESC.	

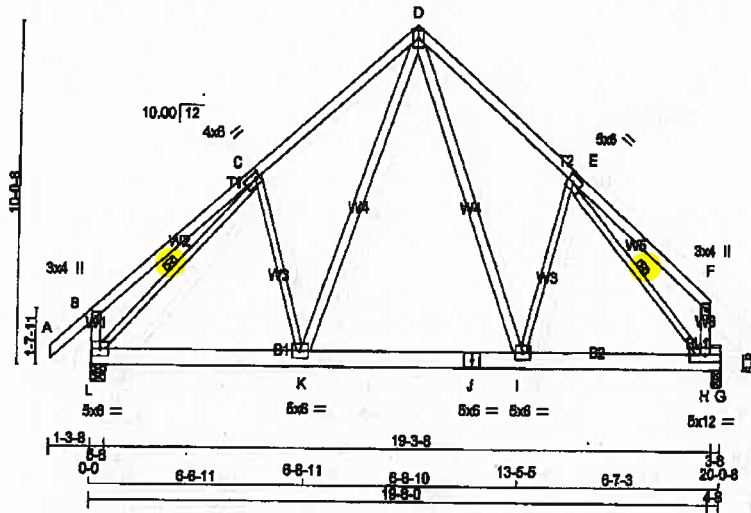
Tanarack Roof Truss, Burlington

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ID:3QHkcHYZQvP8BVkzHQAIV7zNZde-qeWAwesUqTMOCEG8tclzv3ltvQz7kc5YvJwumylm

1-3-8 0-0 5-1-12 5-1-12 4-11-4 10-1-0 4-11-4 15-0-4 4-7-12 19-8-0
4x8 II

Scale = 1:82.1



TOTAL WEIGHT = 12 X 113 = 1356 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS			
EXCEPT	2x3	DRY	No.2
K - D	2x4	DRY	No.2
D - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0
C	TMVW-1	MT20	4.0	8.0 2.00 1.75
D	TMVW-1	MT20	4.0	8.0 Edge
E	TMVW-1	MT20	5.0	8.0 2.80 2.25
F	TMV-p	MT20	3.0	4.0
H	BMVW-1	MT20	5.0	12.0
I	BMVW-1	MT20	5.0	6.0
J	BS-1	MT20	5.0	6.0
K	BMVW-1	MT20	5.0	6.0
L	BMVW-1	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX
L	1884	0	1884	0	5-8
G	1837	0	1837	0	3-8

UNFACTORED REACTIONS					
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	1369	872 / 0	210 / 0	0 / 0	277 / 0
G	1204	738 / 0	210 / 0	0 / 0	258 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	FORCE (LBS)

FR-TO	FROM	TO	CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 57	-129.7	-129.7	0.16 (1)	10.00	C-K	-424 / 14
B-C	0 / 50	-129.7	-129.7	0.54 (1)	10.00	K-D	0 / 668
C-D	-1489 / 0	-129.7	-129.7	0.65 (1)	4.49	D-I	0 / 745
D-E	-1510 / 0	-129.7	-129.7	0.80 (1)	4.58	E-H	-234 / 74
E-F	0 / 74	-129.7	-129.7	0.50 (1)	10.00	L-C	-1808 / 0
L-B	-425 / 0	0.0	0.0	0.04 (1)	7.81	E-H	-1926 / 0
H-F	-198 / 0	0.0	0.0	0.02 (1)	7.81		
L-K	0 / 1230	-38.5	-38.5	0.26 (1)	10.00		
K-J	0 / 875	-38.5	-38.5	0.32 (1)	10.00		
J-I	0 / 875	-38.5	-38.5	0.32 (1)	10.00		
I-H	0 / 1184	-38.5	-38.5	0.71 (1)	10.00		
H-G	0 / 0	-38.5	-38.5	0.35 (1)	10.00		

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL	=	38.2	PSF
DL	=	8.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	61.7	PSF

SPACING = 24.0 IN. GC
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

(65 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =	L / 360 (0.87)
CALCULATED VERT. DEFL. (LL) =	L / 999 (0.10)
ALLOWABLE DEFL. (TL) =	L / 360 (0.87)
CALCULATED VERT. DEFL. (TL) =	L / 999 (0.16)

CSL TC=0.69 / 1.0 (C-D:1), BC=0.71 / 1.0 (H-I:1), WB=0.84 / 1.0 (C-L:1), SB=0.80 / 1.0 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COSEP=1.10 SHEAR=1.10 TENB=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GROUP	DRY	SHEAR	SECTION	
(PS)	(PL)	(PL)			
MT20	818	354	1957	788	1957 1558

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)



DRWG NO. TAM 1105282
STRUCTURAL
COMPONENT ONLY



JOB NAME 297081	TRUSS NAME T112	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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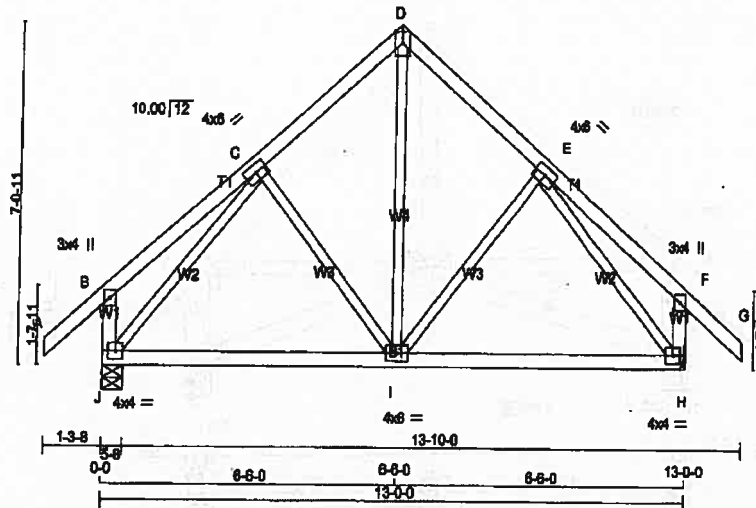
Tamarack Roof Truss, Burlington

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ID:3QHkczHYZQvP8BVkzH0AV7zNZde-bP78cN_JbHTD_fspcOmaticOmk8DD?OnJ08FLBJyJynX

1-3-8 3-8 0-0 3-4-4 3-4-4 3-1-12 6-8-0 3-1-12 9-7-12 3-4-4 13-0-0 1-3-8

Scale = 1:43.7



TOTAL WEIGHT = 62 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATE (Table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	4.0	8.0	
D	TTW+p	MT20	4.0	8.0	Edge
E	TMVW+1	MT20	4.0	8.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW+1	MT20	4.0	4.0	
I	BMVW+1	MT20	4.0	8.0	
J	BMVW+1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	1273	0	5-8	5-8
H	1273	0	MECHANICAL	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	2ND CASE SNOW	3RD CASE LIVE	4TH CASE PERM. LIVE	5TH CASE WIND	6TH CASE DEAD	7TH CASE SOIL
J	925	603 / 0	137 / 0	0 / 0	0 / 0	188 / 0	0 / 0
H	925	603 / 0	137 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO						FR-TO		
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	I-D	0 / 597	0.18 (1)
B-C	0 / 81	-129.7	-129.7	0.21 (1)	10.00	I-E	-168 / 26	0.08 (1)
C-D	-755 / 0	-129.7	-129.7	0.17 (1)	8.25	C-I	-168 / 26	0.08 (1)
D-E	-755 / 0	-129.7	-129.7	0.17 (1)	8.25	J-C	-1075 / 0	0.48 (1)
E-F	0 / 31	-129.7	-129.7	0.21 (1)	10.00	E-H	-1075 / 0	0.48 (1)
F-G	0 / 57	-129.7	-129.7	0.18 (1)	10.00			
J-B	-341 / 0	0.0	0.0	0.04 (1)	7.81			
H-F	-341 / 0	0.0	0.0	0.04 (1)	7.81			
J-I	0 / 675	-38.5	-38.5	0.48 (2)	10.00			
I-H	0 / 675	-38.5	-38.5	0.48 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, NBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43')
CALCULATED VERT. DEFL. (LL) = L/999 (0.06')
ALLOWABLE DEFL. (TL) = L/360 (0.43')
CALCULATED VERT. DEFL. (TL) = L/999 (0.06')

CS: TO=0.21/1.00 (B-C:1), BC=0.43/1.00 (I-J:2), WS=0.48/1.00 (E-H:1), SS=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP = 0.85 (J) (INPUT = 0.90)
JSI METAL = 0.25 (E) (INPUT = 1.00)



DRWG NO. TAM 11605284
STRUCTURAL
COMPONENT ONLY

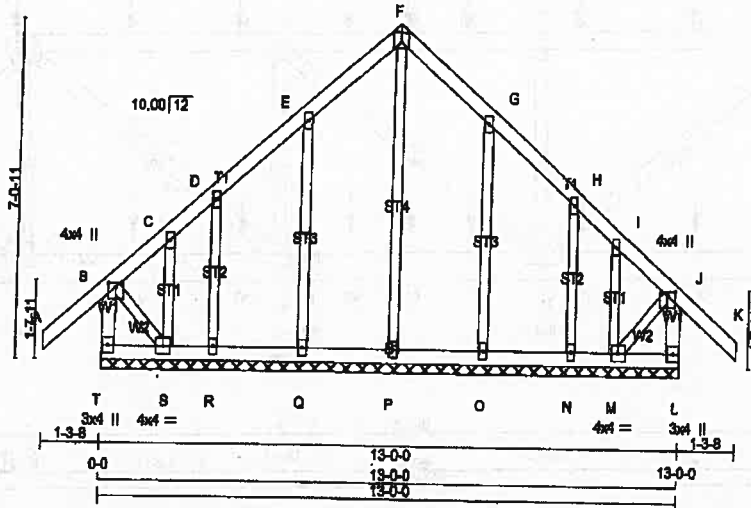


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	G112	1	1	TRUSS DESC.	

Terrace Roof Truss, Burlington

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1-3-8 3-8 0-0 6-6-0 6-6-0 6-2-8 12-8-8 13-0-0 14-3-8
4x4 = 13-0-0
Scale = 1:43.8



TOTAL WEIGHT = 68 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
T - B	2x4	DRY	No.2
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 24-0 OC.			

DESIGNER, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR **CONTINUOUS BEARINGS**.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (widths in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TWW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TWW+w	MT20	2.0	4.0		
F TWW+p	MT20	4.0	4.0	1.50	2.00
J TWW+p	MT20	4.0	4.0	1.00	2.00
L BMW1+p	MT20	3.0	4.0		
M BMWW1-i	MT20	4.0	4.0		
N, O, P, Q, R					
N BMW1+w	MT20	2.0	4.0		
S BMWW1-i	MT20	4.0	4.0		
T BMW1+p	MT20	3.0	4.0		

CHORDS				WEBS			
MEMBER	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMBER	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
T-B	-408 / 0	0.0	0.0 0.04 (1)	P-F	-195 / 0	0.18 (1)	7.81
A-B	0 / 57	-129.7	-129.7 0.15 (1)	Q-E	-303 / 0	0.13 (1)	10.00
B-C	-64 / 0	-129.7	-129.7 0.17 (1)	R-D	-213 / 0	0.05 (1)	8.25
C-D	-9 / 0	-129.7	-129.7 0.04 (1)	S-C	-28 / 0	0.00 (1)	10.00
D-E	-14 / 0	-129.7	-129.7 0.08 (1)	O-G	-303 / 0	0.13 (1)	8.25
E-F	-34 / 0	-129.7	-129.7 0.08 (1)	N-H	-213 / 0	0.05 (1)	6.25
F-G	-34 / 0	-129.7	-129.7 0.08 (1)	M-I	-23 / 0	0.00 (1)	6.25
G-H	-14 / 0	-129.7	-129.7 0.08 (1)	B-S	0 / 33	0.01 (1)	8.25
H-I	-8 / 0	-129.7	-129.7 0.04 (1)	M-J	0 / 33	0.01 (1)	6.25
I-J	-64 / 0	-129.7	-129.7 0.17 (1)				
J-K	0 / 57	-129.7	-129.7 0.18 (1)				
L-J	-408 / 0	0.0	0.0 0.04 (1)				
T-S	0 / 0	-38.5	-38.5 0.01 (3)				
S-R	0 / 24	-38.5	-38.5 0.02 (3)				
R-Q	0 / 19	-38.5	-38.5 0.03 (3)				
Q-P	0 / 13	-38.5	-38.5 0.03 (3)				
P-O	0 / 13	-38.5	-38.5 0.03 (3)				
O-N	0 / 18	-38.5	-38.5 0.03 (3)				
N-M	0 / 24	-38.5	-38.5 0.02 (3)				
M-L	0 / 0	-38.5	-38.5 0.01 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 38.2 PSF
DL = 8.0 PSF

BOT CH. LL = 10.8 PSF
DL = 7.0 PSF

TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2015
- CSA 088-08, CSA 088-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TO=0.18/1.00 (A-B-1), BC=0.03/1.00 (C-R-3),
WB=0.16/1.00 (F-P-1), SS=0.11/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)

MAX MIN MAX MIN MAX MIN

MT20 818 354 1887 798 1887 1888

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (F) (INPUT = 0.80)
JSI METAL= 0.16 (E) (INPUT = 1.00)

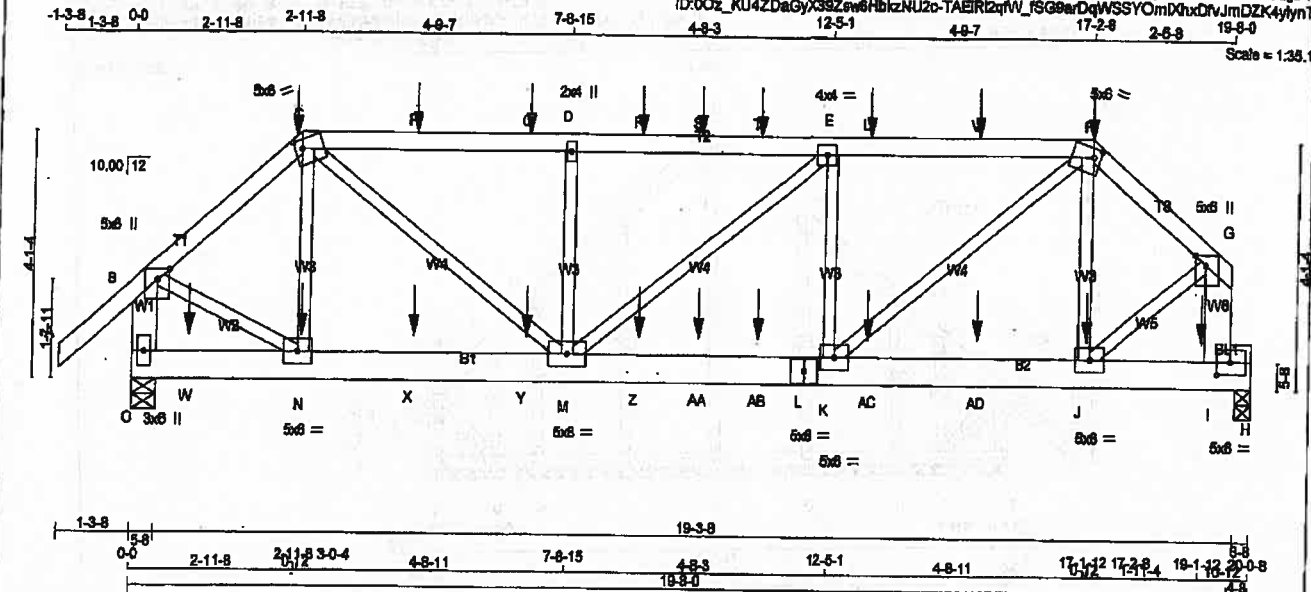


DRWG NO. TAM 1005265
STRUCTURAL
COMPONENT ONLY



JOB NAME 297081	TRUSS NAME T115	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Scale = 1:38.1



TOTAL WEIGHT = 2 X 97 = 194 lb

N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
O - B	2x6	DRY	No.2
I - G	2x6	DRY	No.2
O - L	2x6	DRY	No.2
L - H	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS EXCEPT	2x8	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'x3') SPIRAL NAILS		
A-C	12	SIDE (81.0)
C-F	12	SIDE (81.0)
F-G	12	SIDE (81.0)
O-B	2	TOP
I-G	2	TOP
BOTTOM CHORDS: (0.122'x3') SPIRAL NAILS		
O-L	2	SIDE (183.1)
L-H	2	SIDE (183.1)
WEBS: (0.122'x3') SPIRAL NAILS		
N-C	1	SIDE (12.6)
2x3	1	
2x4	1	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES table is in inches					
JT TYPE	PLATES	W	LEN	Y	X
B TMWW+p	MT20	5.0	8.0	2.00	2.25
C TTMWW-m	MT20	5.0	8.0	1.75	1.50
D TMWW-w	MT20	2.0	4.0		
E TMWW-l	MT20	4.0	4.0		
F TTMWW-m	MT20	5.0	8.0	1.75	1.50
G TMWW+p	MT20	5.0	8.0	2.00	2.25
I BMWV-t	MT20	5.0	8.0	2.50	2.75
J, K, N					
J BMWW-l	MT20	3.0	6.0		
L BS-t	MT20	3.0	8.0		
M BMWWV-t	MT20	5.0	8.0		
O BMV-l+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARING			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
JT	VERT	DOWN	BRG
O	3136	0	5-8
H	2871	0	3-8

UNFACTORED REACTIONS			
	1ST CASE	MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
O	2287	1489 / 0	374 / 0
H	2176	1382 / 0	378 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 57	-129.7 -129.7	0.10 (1)
B-C	-3891 / 0	-129.7 -129.7	0.14 (1)
C-P	-3845 / 0	-129.7 -129.7	0.48 (1)
P-Q	-3845 / 0	-129.7 -129.7	0.48 (1)
Q-D	-3845 / 0	-129.7 -129.7	0.48 (1)
D-R	-3845 / 0	-129.7 -129.7	0.47 (1)
R-S	-3845 / 0	-129.7 -129.7	0.47 (1)
S-T	-3845 / 0	-129.7 -129.7	0.47 (1)
T-E	-3845 / 0	-129.7 -129.7	0.47 (1)
E-U	-3808 / 0	-129.7 -129.7	0.47 (1)
U-V	-3808 / 0	-129.7 -129.7	0.47 (1)
V-F	-3808 / 0	-129.7 -129.7	0.47 (1)
F-G	-2828 / 0	-129.7 -129.7	0.10 (1)
O-B	-3066 / 0	0.0	0.11 (1)
I-G	-3730 / 0	0.0	0.15 (1)
O-W	0 / 0	-38.5 -38.5	0.04 (2)
W-N	0 / 0	-38.5 -38.5	0.04 (2)
N-X	0 / 2174	-38.5 -38.5	0.18 (1)
X-Y	0 / 2174	-38.5 -38.5	0.18 (1)
Y-M	0 / 2174	-38.5 -38.5	0.19 (1)
M-Z	0 / 3808	-38.5 -38.5	0.34 (1)
Z-AA	0 / 3808	-38.5 -38.5	0.34 (1)
AA-AB	0 / 3808	-38.5 -38.5	0.34 (1)
AB-L	0 / 3808	-38.5 -38.5	0.34 (1)
L-K	0 / 3808	-38.5 -38.5	0.34 (1)
K-AC	0 / 2147	-38.5 -38.5	0.28 (1)
AC-AD	0 / 2147	-38.5 -38.5	0.28 (1)
AD-J	0 / 2147	-38.5 -38.5	0.28 (1)
J-I	-517 / 0	-38.5 -38.5	0.85 (1)
I-H	0 / 0	-168.2 -168.2	0.40 (1)

SPECIFIED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX+
C	2-11-8	-28	-28
C	2-11-8	-148	-148
C	2-11-8	-178	-178
F	17-2-8	-28	-28
F	17-2-8	-112	-112
F	17-2-8	-178	-178
I	19-1-12	-43	-43

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:			
TOP CH.	LL	=	38.2 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	81.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 088-14
- TPC 2011, TPC 2014

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.67")
CALCULATED VERT. DEFL. (LL) = L/699 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.67")
CALCULATED VERT. DEFL. (TL) = L/699 (0.11")

CSL TO=0.47/1.00 (E-F-1), BC=0.65/1.00 (G-J-1), WB=0.38/1.00 (G-J-1), SB=0.60/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVEHEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (PS)	SHEAR (PL)	SECTION (PL)	
MAX	MIN	MAX	MIN
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.36 (L) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T115	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

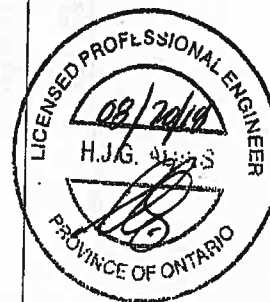
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HANGERS NOTES

1) SPECIAL HANGERS(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S). 915.8 lbs FACTORED DOWN AT 2-11-8,
482.1 lbs FACTORED DOWN AT 17-3-8, 158.1 lbs
FACTORED DOWN AT 5-0-4, 158.1 lbs
FACTORED DOWN AT 7-0-4, 158.1 lbs
FACTORED DOWN AT 8-0-4, 158.1 lbs
FACTORED DOWN AT 10-1-0, 158.1 lbs
FACTORED DOWN AT 11-1-12, AND 158.1 lbs
FACTORED DOWN AT 13-1-12, AND 158.1 lbs
FACTORED DOWN AT 15-1-12 ON TOP CHORD,
AND 54.9 lbs FACTORED DOWN AT 1-0-4, 61.8
lbs FACTORED DOWN AT 3-0-4, 54.9 lbs
FACTORED DOWN AT 5-0-4, 54.9 lbs
FACTORED DOWN AT 7-0-4, 54.9 lbs
FACTORED DOWN AT 9-0-4, 54.9 lbs
FACTORED DOWN AT 10-1-0, 54.9 lbs
FACTORED DOWN AT 11-1-12, 54.9 lbs
FACTORED DOWN AT 13-1-12, 54.9 lbs
FACTORED DOWN AT 15-1-12, AND 61.8 lbs
FACTORED DOWN AT 17-1-12, AND 61.1 lbs
FACTORED DOWN AT 19-1-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX*	FACE	DIR.	TYPE
J	17-1-12	-44	-44	-	FRONT	VERT	TOTAL
N	3-0-4	-44	-44	-	FRONT	VERT	TOTAL
P	5-0-4	-108	-108	-	FRONT	VERT	TOTAL
Q	7-0-4	-108	-108	-	FRONT	VERT	TOTAL
R	8-0-4	-108	-108	-	FRONT	VERT	TOTAL
S	10-1-0	-108	-108	-	FRONT	VERT	TOTAL
T	11-1-12	-108	-108	-	FRONT	VERT	TOTAL
U	13-1-12	-108	-108	-	FRONT	VERT	TOTAL
V	15-1-12	-108	-108	-	FRONT	VERT	TOTAL
W	1-0-4	-38	-38	-	FRONT	VERT	TOTAL
X	5-0-4	-38	-38	-	FRONT	VERT	TOTAL
Y	7-0-4	-38	-38	-	FRONT	VERT	TOTAL
Z	9-0-4	-38	-38	-	FRONT	VERT	TOTAL
AA	10-1-0	-38	-38	-	FRONT	VERT	TOTAL
AB	11-1-12	-38	-38	-	FRONT	VERT	TOTAL
AC	13-1-12	-38	-38	-	FRONT	VERT	TOTAL
AD	15-1-12	-38	-38	-	FRONT	VERT	TOTAL

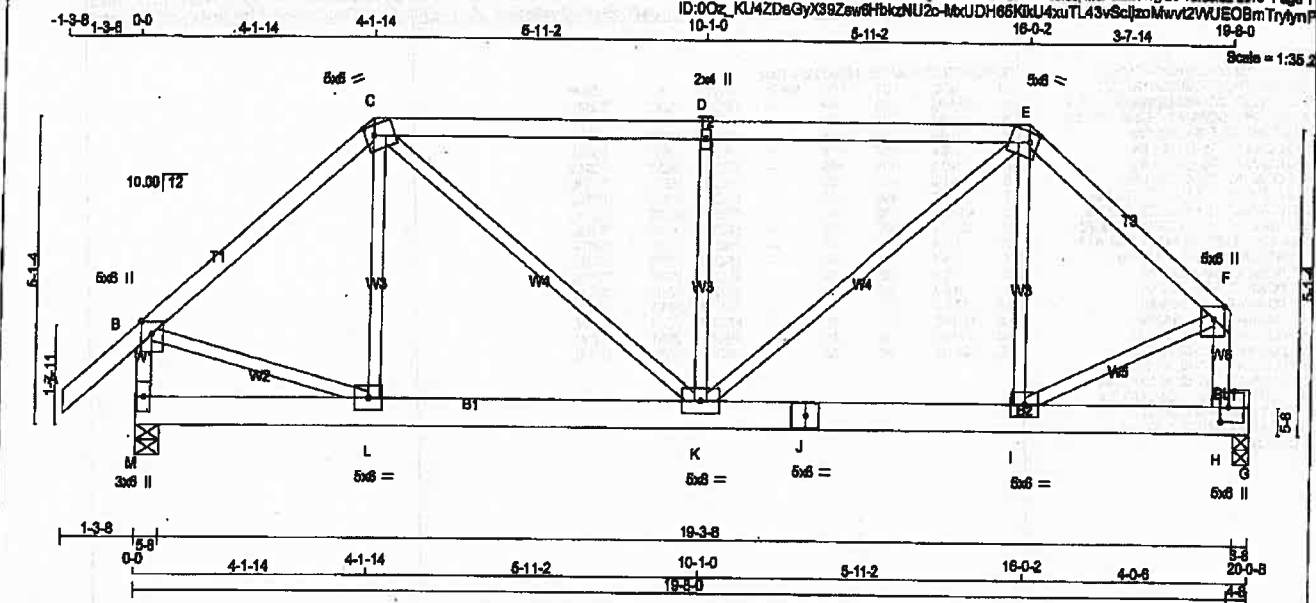


DRWG NO. TAM 71805285
STRUCTURAL
COMPONENT ONLY
3/2



JOB NAME 297081	TRUSS NAME T116	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x6 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES - Edges in in. inches				
JT	TYPE	PLATES	W	LEN Y X
B	TWVW+p	MT20	5.0	8.0 Edge
C	TWVW-m	MT20	5.0	8.0 2.00 1.75
D	TWVW-w	MT20	2.0	4.0
E	TWVW-m	MT20	5.0	8.0 2.00 1.75
F	TWVW-p	MT20	5.0	8.0 2.50 2.25
H	BMVW+p	MT20	5.0	8.0 3.00 1.75
I	BMVW-t	MT20	5.0	8.0
J	BB-t	MT20	5.0	8.0
K	BMVW-w	MT20	5.0	8.0
L	BMVW-t	MT20	5.0	8.0
M	BMVW-p	MT20	3.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1884	0	1884	0	0	5-8	5-8
G	1837	0	1837	0	0	3-8	3-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS					
JT	18T LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1389	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MAX. UNBRAC. LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 57	-129.7	-129.7 0.18 (1)	10.00	I-C	-108 / 141	0.04 (1)
B-C	-1578 / 0	-129.7	-129.7 0.45 (1)	4.71	C-K	0 / 688	0.20 (1)
C-D	-1800 / 0	-129.7	-129.7 0.91 (1)	3.37	K-D	-938 / 0	0.34 (1)
D-E	-1900 / 0	-129.7	-129.7 0.91 (1)	3.37	E-K	0 / 682	0.20 (1)
E-F	-1881 / 0	-129.7	-129.7 0.35 (1)	4.85	I-E	-88 / 146	0.08 (3)
M-B	-1808 / 0	0.0	0.0 0.18 (1)	6.22	B-L	0 / 1258	0.28 (1)
H-F	-1890 / 0	0.0	0.0 0.21 (1)	8.15	I-F	0 / 1501	0.34 (1)
M-L	0 / 0	-38.5	-38.5 0.09 (2)	10.00			
L-K	0 / 1206	-38.5	-38.5 0.22 (1)	10.00			
K-J	0 / 1210	-38.5	-38.5 0.32 (1)	10.00			
J-I	0 / 1210	-38.5	-38.5 0.32 (1)	10.00			
I-H	-185 / 0	-38.5	-38.5 0.57 (1)	8.25			
H-G	0 / 0	-38.5	-38.5 0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2016

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, OBC 2016
- CSA 088-08, CSA 088-14
- TRC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = 1/899 (0.10")

CSL TC=0.91/1.00 (D-E-I), BC=0.57/1.00 (H-I-I), WB=0.34/1.00 (F-I-I), SS=0.60/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY), SHEAR SECTION (PSD) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 254 1887 788 1887 1638

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.82 (B) (INPUT = 1.00)



DRWG NO. TAM 17605286
STRUCTURAL
CONSULTANT P.L.L.C.

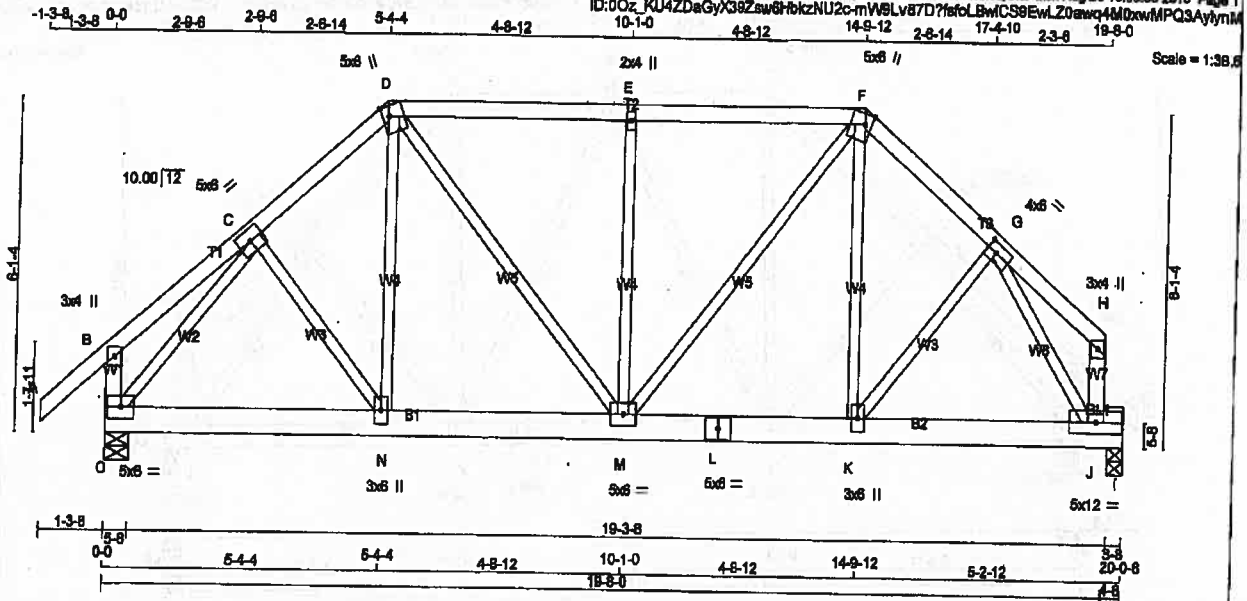


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297081	T117	1	1			

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TOTAL WEIGHT = 105 lb

LUMBER

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

BEARING BLOCKS	SIZE	DRY	LUMBER	DESCR.
BL1	2x4	DRY	No.2	SPF

ALL WEBB 2x4 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+H	MT20	5.0	6.0		
D TTWV+M	MT20	6.0	6.0	2.25	1.50
E TMV+H	MT20	2.0	4.0		
F TTWV+M	MT20	5.0	6.0	2.25	1.50
G TMVW+H	MT20	4.0	6.0	2.00	2.25
H TMV+p	MT20	3.0	4.0		
J BMVW+H	MT20	5.0	12.0		
K BMVW+H	MT20	3.0	6.0		
L BS+H	MT20	5.0	6.0		
M BMVW+H	MT20	5.0	6.0		
N BMVW+H	MT20	3.0	6.0		
O BMVW+H	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	DOWN	0	5-8
O	HORZ	0	0	3-8
I	UP/LIFT	0	0	3-8

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
O	1359	872 / 0 210 / 0 0 / 0 0 / 0 277 / 0 0 / 0
I	1204	738 / 0 210 / 0 0 / 0 0 / 0 258 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLP)	MAX. FACTORED HORIZ. LOAD (LC)	MAX. FACTORED LENGTH FR-TO	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)
FR-TO							
A-B	0 / 57	-129.7	-129.7 0.18 (1)	10.00	C-N	0 / 104	0.02 (3)
B-C	0 / 22	-129.7	-129.7 0.13 (1)	10.00	H-D	0 / 225	0.05 (2)
C-D	-1598 / 0	-129.7	-129.7 0.16 (1)	5.13	D-M	0 / 545	0.12 (1)
D-E	-1528 / 0	-129.7	-129.7 0.53 (1)	4.80	M-E	-744 / 0	0.41 (1)
E-F	-1528 / 0	-129.7	-129.7 0.53 (1)	4.80	M-F	0 / 516	0.12 (1)
F-G	-1576 / 0	-129.7	-129.7 0.14 (1)	5.12	K-F	0 / 248	0.08 (2)
G-H	0 / 33	-129.7	-129.7 0.11 (1)	10.00	K-G	0 / 185	0.08 (1)
O-B	-317 / 0	0.0	0.0 0.03 (1)	7.81	O-C	-1848 / 0	0.55 (1)
J-H	-101 / 0	0.0	0.0 0.01 (1)	7.81	G-J	-1828 / 0	0.51 (1)
O-N	0 / 1129	-38.5	-38.5 0.20 (1)	10.00			
N-M	0 / 1178	-38.5	-38.5 0.21 (1)	10.00			
M-L	0 / 1185	-38.5	-38.5 0.34 (1)	10.00			
L-K	0 / 1185	-38.5	-38.5 0.34 (1)	10.00			
K-J	0 / 872	-38.5	-38.5 0.88 (1)	10.00			
J-I	0 / 0	-38.5	-38.5 0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 51.7 PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2012, CBC 2016
- CSA 088-09, CSA 088-14
- TRC 2011, TRC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/889 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/889 (0.11")

CSI: TC=0.53/1.00 (E-F), BC=0.88/1.00 (H-K),
WB=0.56/1.00 (O-C), SB=0.60/1.00 (J-I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 768 1867 1868

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)



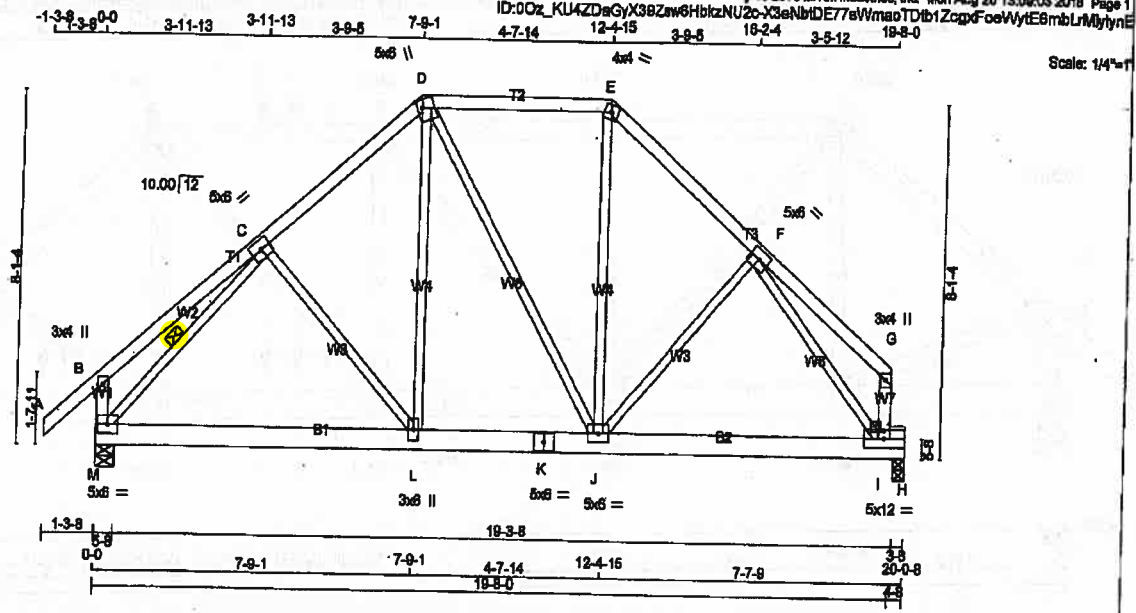
DRWG NO. 11805287
STRUCTURAL
CONSTRUCTION

DWG NO. TAM 71805288
STRUCTURAL
COMPONENTS ONLY



JOB NAME 297081	TRUSS NAME T119	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Version 8.310 5 May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:06:03 2018 Page 1
ID:00Z_KU4ZDaGyX38Zw6HbtzNU2c-X3eNbIDE77sVWmaTDt1ZagpFoeVytE8mbLmMytynE



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY EXCEPT			
DRY: SEASONED LUMBER			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMW+p	MT20	3.0	4.0	
C TMWV+1	MT20	5.0	6.0	
D TTWV+m	MT20	5.0	6.0	2.25 1.50
E TTW-m	MT20	4.0	4.0	
F TMWV+1	MT20	5.0	6.0	
G TMW+p	MT20	3.0	4.0	
I BMWV+1	MT20	5.0	12.0	
J BMWV+1	MT20	5.0	6.0	
K BS-1	MT20	5.0	6.0	
L BMWV+1	MT20	3.0	6.0	
M BMWV+1	MT20	5.0	6.0	

FACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
M	1399	672 / 0	210 / 0
H	1204	738 / 0	210 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 57	C-L	-103 / 41
B-C	0 / 37	D-D	0 / 382
C-D	-1407 / 0	D-J	0 / 62
D-E	-1088 / 0	J-E	0 / 408
E-F	-1443 / 0	J-F	0 / 101
F-G	0 / 85	M-C	-1787 / 0
M-B	-371 / 0	F-I	-1888 / 0
I-G	-147 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.8 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.87")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (0.87")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CS: TC=0.51/1.00 (D-E), BC=0.88/1.00 (J-I), WB=0.91/1.00 (F-I), SS=0.60/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

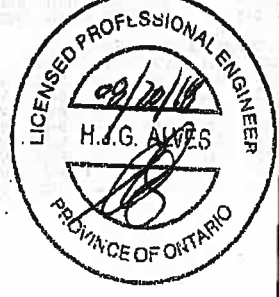
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSD) (PLI) (PLD)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

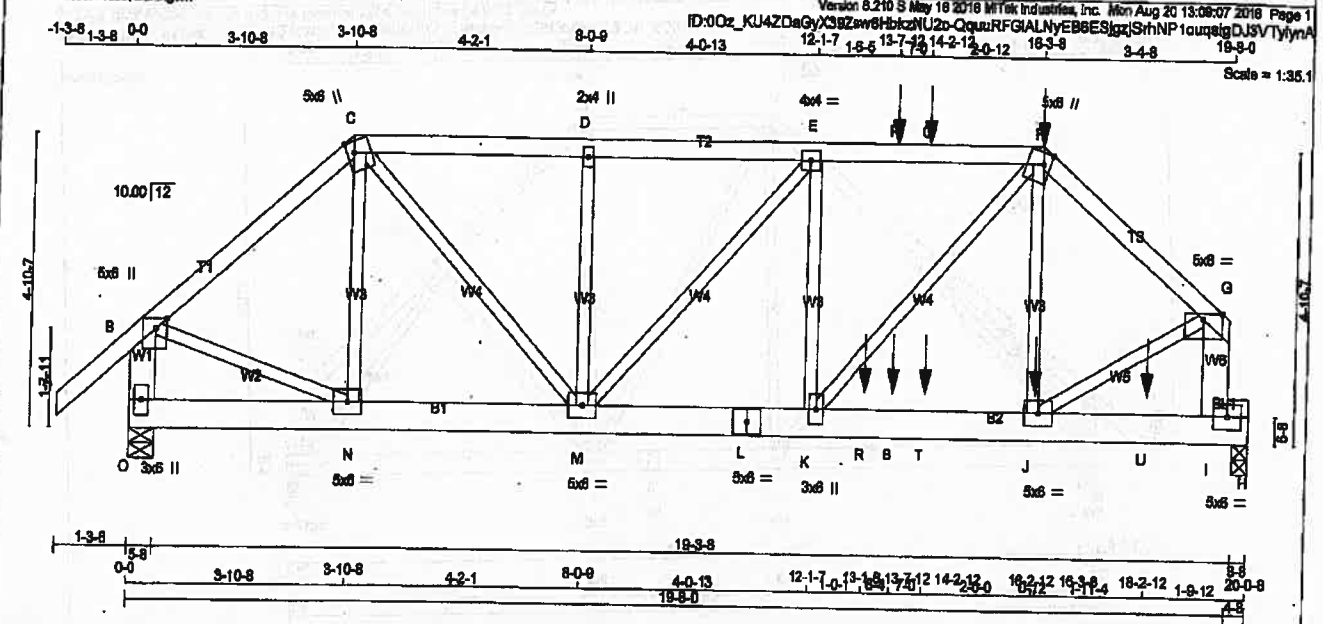
JSI GRIP= 0.88 (F) (INPUT = 0.80)
JSI METAL= 0.45 (F) (INPUT = 1.00)

DRWG NO. TAM **11605289**
STRUCTURAL
COMPONENT ONLY





JOB NAME: 297081 TRUSS NAME: T120Z QUANTITY: 1 PLY: 2 JOB DESC: TRUSS DESC. DRWG NO. Tamarack Roof Truss, Burlington



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
O - B	2x6	DRY	No.2
I - G	2x6	DRY	No.2
O - L	2x6	DRY	No.2
L - H	2x6	DRY	No.2

BEARING BLOCKS

BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE BRACING (IN) LOAD (PLF)

TOP CHORDS	SIZE	LOAD (PLF)
A - C	12	TOP
C - F	12	SIDE (81.0)
F - G	12	SIDE (81.0)
O - B	12	TOP
I - G	12	TOP

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

CHORDS	SIZE	LOAD (PLF)
O - L	12	TOP
L - H	12	SIDE (183.1)

WEBS: (0.122"x3") SPIRAL NAILS

WEBS	SIZE	LOAD (PLF)
2x3	8	TOP
2x4	8	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW+L	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.25	1.50
G	TMVW+p	MT20	5.0	8.0	Edge	
H	BMVW-L	MT20	5.0	8.0		
I	BMVW-H	MT20	5.0	8.0		
J	BMVW-H	MT20	3.0	8.0		
K	BB-4	MT20	5.0	8.0		
L	BMVW-H	MT20	5.0	8.0		
M	BMVW-H	MT20	5.0	8.0		
N	BMVW-H	MT20	5.0	8.0		
O	BMVW+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	FACTORED
O	GROSS REACTION
H	DOWN

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	FACTORED
O	GROSS REACTION
H	DOWN

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	FACTORED	MAX. FACTORED
FR-TO	0/57	FROM	TO	FR-TO	0/57	FROM	TO
A-B	-129.7	-129.7	0.10 (1)	10.00	N-C	-284/50	0.06 (1)
B-C	-129.7	-129.7	0.21 (1)	5.98	C-M	0/1561	0.16 (1)
C-D	-129.7	-129.7	0.18 (1)	5.38	M-D	-533/0	0.08 (1)
D-E	-129.7	-129.7	0.28 (1)	8.27	M-E	-734/0	0.23 (1)
E-F	-129.7	-129.7	0.31 (1)	4.86	K-E	-213/74	0.04 (1)
F-G	-129.7	-129.7	0.31 (1)	4.86	K-F	0/1436	0.18 (1)
G-H	-129.7	-129.7	0.31 (1)	4.86	J-F	-38/100	0.02 (3)
H-I	-129.7	-129.7	0.18 (1)	5.28	B-N	0/1684	0.21 (1)
I-J	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
J-K	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
K-L	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
L-M	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
M-N	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
N-O	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
O-P	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
P-Q	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
Q-R	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
R-S	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
S-T	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
T-U	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
U-V	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
V-W	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
W-X	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
X-Y	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
Y-Z	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)
Z-A	-129.7	-129.7	0.18 (1)	7.81	J-G	0/1684	0.21 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
F	18-3-8	-195	-195		BACK	VERT	TOTAL
J	18-2-12	-26	-26		BACK	VERT	TOTAL
P	13-7-12	-83	-83		BACK	VERT	TOTAL
Q	14-2-12	-83	-83		BACK	VERT	TOTAL
R	13-1-8	-866	-866		BACK	VERT	TOTAL
S	13-7-12	-21	-21		BACK	VERT	TOTAL
T	14-2-12	-21	-21		BACK	VERT	TOTAL
U	18-2-12	-11	-11		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = 1/360 (0.67)
CALCULATED VERT. DEF. (LL) = 1/999 (0.05)
ALLOWABLE DEF. (TL) = 1/360 (0.67)
CALCULATED VERT. DEF. (TL) = 1/999 (0.05)

CSI: TO=0.31/1.00 (E-F-1), BC=0.61/1.00 (H-I-1), WS=0.38/1.00 (3-I-1), SS=0.58/1.00 (H-I-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	816	354
	1887	788
		1887
		1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.80)
JSI METAL= 0.33 (J) (INPUT = 1.00)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
F	18-3-8	-195	-195		BACK	VERT	TOTAL
J	18-2-12	-26	-26		BACK	VERT	TOTAL
P	13-7-12	-83	-83		BACK	VERT	TOTAL
Q	14-2-12	-83	-83		BACK	VERT	TOTAL
R	13-1-8	-866	-866		BACK	VERT	TOTAL
S	13-7-12	-21	-21		BACK	VERT	TOTAL
T	14-2-12	-21	-21		BACK	VERT	TOTAL
U	18-2-12	-11	-11		BACK	VERT	TOTAL

DWG NO. TAM 11805290
STRUCTURAL
COMPONENT ONLY



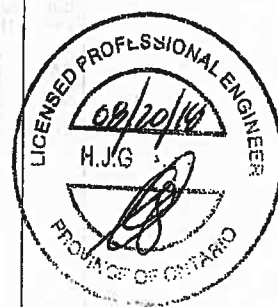
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297081	T120Z	1	2			

Temerack Roof Truss, Burlington

Version 8.210 8 May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:02:07 2018 Page 2
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HANGER NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.6 lbs FACTORED DOWN AT 13-7-12, AND 121.6 lbs FACTORED DOWN AT 14-2-12, AND 281.9 lbs FACTORED DOWN AT 16-3-6 ON TOP CHORD, AND 828.0 lbs FACTORED DOWN AT 13-1-8, 30.8 lbs FACTORED DOWN AT 13-7-12, 30.8 lbs FACTORED DOWN AT 14-2-12, AND 38.0 lbs FACTORED DOWN AT 16-2-12, AND 23.7 lbs FACTORED DOWN AT 18-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

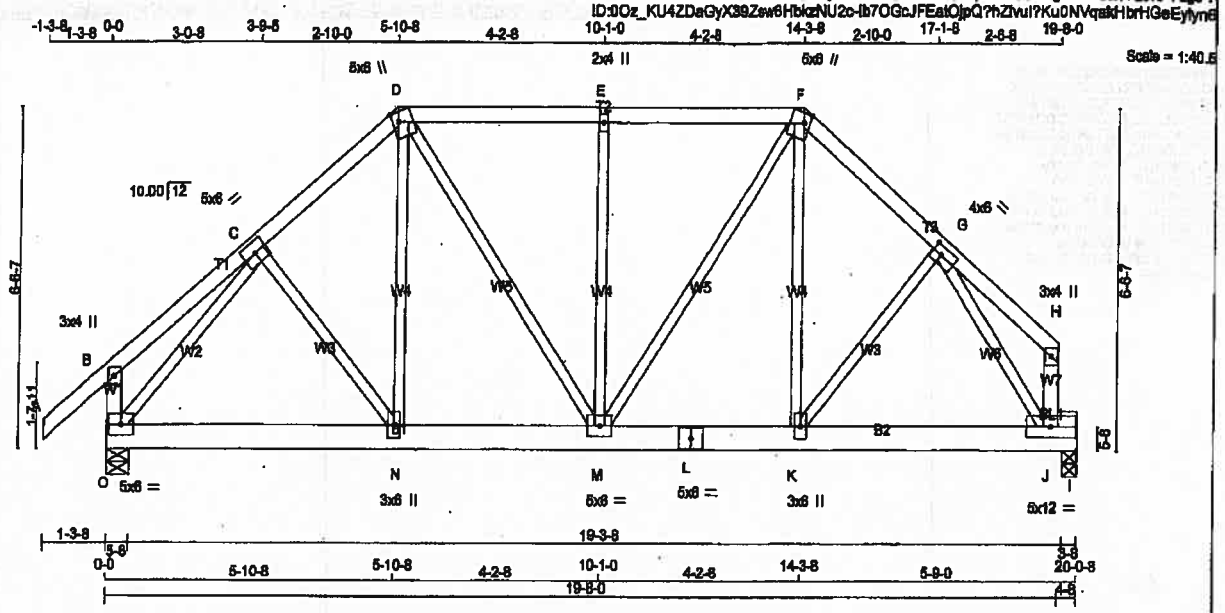


DWG NO. TAM 11805291
STRUCTURAL
COMPONENT ONLY 7/2



JOB NAME 297081	TRUSS NAME T121	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.210.5 May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:08:11 2018 Page 1



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - A	2x4	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW+H	MT20	5.0	6.0
D	TTWVW+m	MT20	5.0	6.0
E	TMVW+w	MT20	2.0	4.0
F	TTWVW+m	MT20	5.0	6.0
G	TMVW+H	MT20	4.0	6.0
H	TMV+p	MT20	3.0	4.0
J	BMVW+H	MT20	5.0	12.0
K	BMVW+H	MT20	3.0	6.0
L	BS-4	MT20	5.0	6.0
M	BMVW+H	MT20	5.0	6.0
N	BMVW+H	MT20	3.0	6.0
O	BMVW+H	MT20	5.0	6.0

REARINGS

FACTORED			
JT	VERT	HORZ	UPLIFT
O	1884	0	0
I	1837	0	0

MAX. MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE
O	1359	872 / 0	210 / 0
I	1204	738 / 0	210 / 0

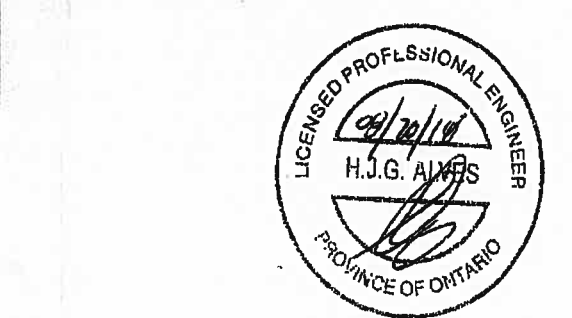
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 57	-128.7	0.18 (1)
B-C	0 / 28	-128.7	0.18 (1)
C-D	-1538 / 0	-128.7	0.18 (1)
D-E	-1403 / 0	-128.7	0.41 (1)
E-F	-1557 / 0	-128.7	0.41 (1)
F-G	0 / 98	-128.7	0.17 (1)
G-H	-328 / 0	-128.7	0.14 (1)
H-I	-111 / 0	0.0	0.08 (1)
I-J		0.0	0.01 (1)
J-K		0.0	0.01 (1)
K-L		0.0	0.01 (1)
L-M		0.0	0.01 (1)
M-N		0.0	0.01 (1)
N-O		0.0	0.01 (1)
O-A		0.0	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24 IN. G.C.

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 54.8 P.S.F. G.B.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.87")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.87")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.12")

CS: TC=0.41/1.00 (D-E:1), BC=0.69/1.00 (J-K:1), WB=0.82/1.00 (C-D:1), SB=0.60/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.80)
JSI METAL= 0.45 (G) (INPUT = 1.00)

DRWG NO. TAM **71805292**
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	T122	1	1	TRUSS DESC.	

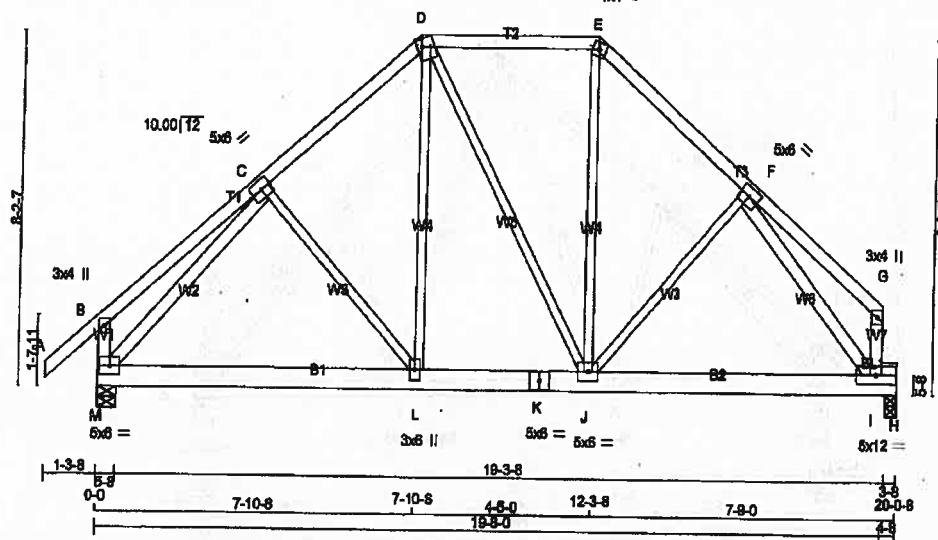
Tamarack Roof Truss, Burlington

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ID:00x_KU4ZDgGyX38ZawGhbczNU2c-IApXveMAXV/FysG8aHhVxqHfEP31vbKHpVbCEZyln3

1-3-8 3-8-0 4-0-8 4-0-8 3-10-0 7-10-8 4-8-0 12-3-8 3-10-0 15-1-8 3-8-8 19-8-0

Scale = 1:48.8



TOTAL WEIGHT = 110 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	SIZE	DRY	LUMBER	DESCR.
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x8	DRY	No.2	SPF
EXCEPT				
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+M	MT20	3.0	6.0		
D TTWW+M	MT20	3.0	6.0	2.25	1.50
E TTW+M	MT20	4.0	4.0		
F TMVW+M	MT20	3.0	6.0		
G TMV+p	MT20	3.0	4.0		
I BMVW+M	MT20	5.0	12.0		
J BMVW+M	MT20	5.0	6.0		
K BS+M	MT20	3.0	6.0		
L BMVW+M	MT20	3.0	6.0		
M BMVW+M	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

REACTIONS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
M	1864	0	1864	0
H	1637	0	1637	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
M	1359
H	1204

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	VERT. LOAD	LC1 MAX	MAX. MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	FROM TO	(PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE
FR-TO	(LBS)				LENGTH	FR-TO	(LBS)
A-B	0/57	-129.7	-129.7	0.16 (1)	10.00	C-L	-205/38
B-C	0/39	-129.7	-129.7	0.33 (1)	10.00	D-D	0/383
C-D	-1400/0	-129.7	-129.7	0.37 (1)	8.05	D-J	0/88
D-E	-1080/0	-129.7	-129.7	0.46 (1)	6.38	E-E	0/413
E-F	-1438/0	-129.7	-129.7	0.34 (1)	5.08	F-F	-7/38
F-G	0/56	-129.7	-129.7	0.29 (1)	10.00	M-C	-1797/0
M-B	-372/0	0.0	0.0	0.04 (1)	7.81	F-I	-1855/0
I-G	-148/0	0.0	0.0	0.02 (1)	7.81		
M-L	0/1181	-38.5	-38.5	0.31 (2)	10.00		
L-K	0/1048	-38.5	-38.5	0.40 (1)	10.00		
K-J	0/1048	-38.5	-38.5	0.40 (1)	10.00		
J-I	0/1078	-38.5	-38.5	0.68 (1)	10.00		
I-H	0/0	-38.5	-38.5	0.35 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	36.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.87)
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12)
ALLOWABLE DEFL.(TL) = $L/360$ (0.87)
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.21)

CS1: TO=0.48/1.00 (D-E=1), BC=0.69/1.00 (I-J=1),
WB=0.75/1.00 (C-A=1), SB=0.80/1.00 (F-I=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.80)
JSI METAL= 0.46 (F) (INPUT = 1.00)



DRWG NO. TAM 71805293
STRUCTURAL
CONSTRUCTION ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297081	T123	1	1			

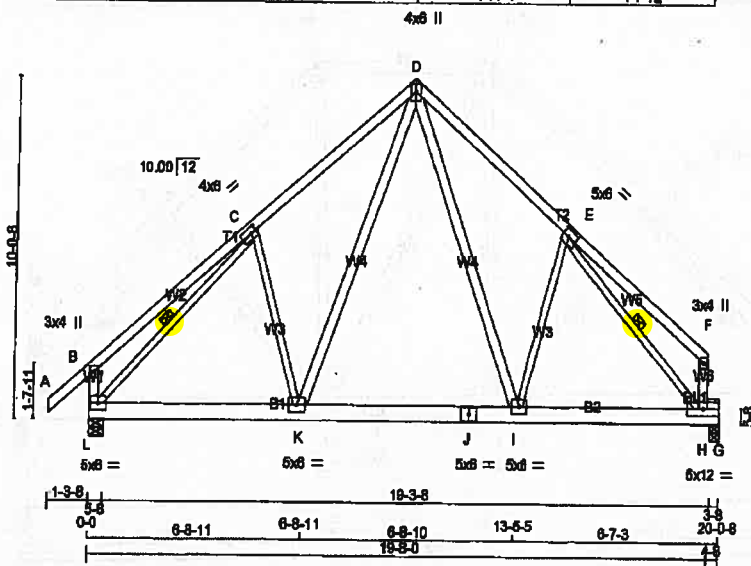
Tamack Roof Truss, Burlington

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ID:3QHkCHYZQvP8BVKGH0AN7zNZde-bx22k7Pabk03SLbXNYgnoThrozkDJCRU8NKym7

-1-3-8 0-0 5-1-12 5-1-12 4-11-4 10-1-0 4-11-4 15-0-4 4-7-12 19-8-0

Scale = 1:82.1



TOTAL WEIGHT = 113 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2

BEARING BLOCKS

BL1	2x4	DRY	No.2
ALL WEBS EXCEPT	2x6	DRY	No.2
K - D	2x4	DRY	No.2
D - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Plate is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
C	TMVW-p	MT20	4.0	6.0	2.00 1.75
D	TTWV-p	MT20	4.0	6.0	Edge
E	TMVW-p	MT20	5.0	6.0	2.50 2.25
F	TMV-p	MT20	3.0	4.0	
H	BMVW-p	MT20	5.0	12.0	
I	BMVW-p	MT20	5.0	6.0	
J	BB-p	MT20	5.0	6.0	
K	BMVW-p	MT20	5.0	6.0	
L	BMVW-p	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	1884	0	0	5-8
G	1837	0	0	3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1359	872 / 0	210 / 0	0 / 0	0 / 0	277 / 0	0 / 0
G	1204	738 / 0	210 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	C-K	-424 / 14	0.23 (1)		
B-C	0 / 50	-129.7	-129.7	0.54 (1)	10.00	K-D	0 / 686	0.11 (1)		
C-D	-1489 / 0	-129.7	-129.7	0.65 (1)	4.49	D-I	0 / 745	0.12 (1)		
D-E	-1610 / 0	-129.7	-129.7	0.80 (1)	4.58	I-E	-234 / 74	0.13 (1)		
E-F	0 / 74	-129.7	-129.7	0.50 (1)	10.00	L-C	-1806 / 0	0.64 (1)		
L-B	-425 / 0	0.0	0.0	0.04 (1)	7.81	E-H	-1828 / 0	0.63 (1)		
H-F	-198 / 0	0.0	0.0	0.02 (1)	7.81					
L-K	0 / 1230	-38.5	-38.5	0.28 (1)	10.00					
K-J	0 / 875	-38.5	-38.5	0.32 (1)	10.00					
J-I	0 / 875	-38.5	-38.5	0.32 (1)	10.00					
I-H	0 / 1194	-38.5	-38.5	0.71 (1)	10.00					
H-G	0 / 0	-38.5	-38.5	0.35 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CS1: TC=0.68/1.00 (C-D:1), BC=0.71/1.00 (H-I:1),
WB=0.84/1.00 (C-L:1), SS=0.60/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	818 354 1687 788 1957 1636	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)



DRWG NO. TAM 71605294
STRUCTURAL
COMPONENT ONLY



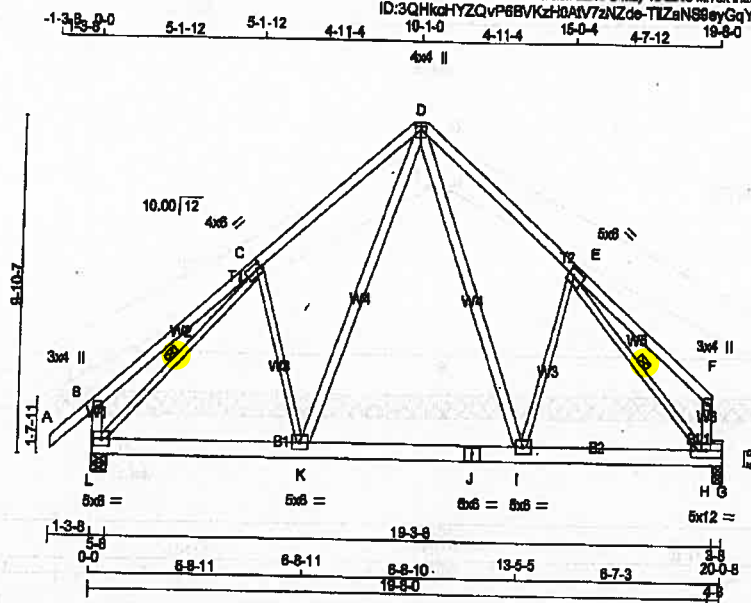
JOB NAME 297081	TRUSS NAME T123C	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:81.8



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - F	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS EXCEPT			
K - D	2x4	DRY	No.2
D - I	2x4	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+L	MT20	4.0	6.0	2.00	1.75
D TTWV+p	MT20	4.0	4.0	1.25	2.00
E TMVW+L	MT20	5.0	6.0	2.50	2.25
F TMV+p	MT20	3.0	4.0		
H BMVW+L	MT20	5.0	12.0		
I BMVW+L	MT20	5.0	6.0		
J BS-L	MT20	5.0	6.0		
K BMVW+L	MT20	5.0	6.0		
L BMVW+L	MT20	5.0	6.0		

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	IN-SK	BRG	IN-SK
L	1864 0	L	1864 0	0/0	5-8	5-8	5-8
G	1637 0	G	1637 0	0/0	3-8	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	COMBINED
L	1389 872/0	L	1389 872/0
G	1204 738/0	G	1204 738/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/57	C-K	-424/14
B-C	0/60	K-D	0/688
C-D	-1489/0	D-I	0/745
D-E	-1510/0	I-E	-234/74
E-F	0/74	L-C	-1808/0
F-G	-425/0	E-H	-1928/0
H-F	-188/0		
L-K	0/1230		
K-J	0/878		
J-I	0/876		
I-H	0/1194		
H-G	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.8 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 6, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 6 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

(55 % OF 64.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/860 (0.67")
CALCULATED VERT. DEFL.(LL) = L/868 (0.10")
ALLOWABLE DEFL.(TL) = L/380 (0.67")
CALCULATED VERT. DEFL.(TL) = L/868 (0.16")

CSI: TO=0.65/1.00 (C-D-1), 8C=0.71/1.00 (H-1),
WB=0.84/1.00 (C-L-1), 8B=0.80/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

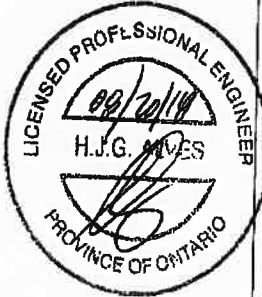
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1867 788 1987 1895

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.50 (E) (INPUT = 1.00)



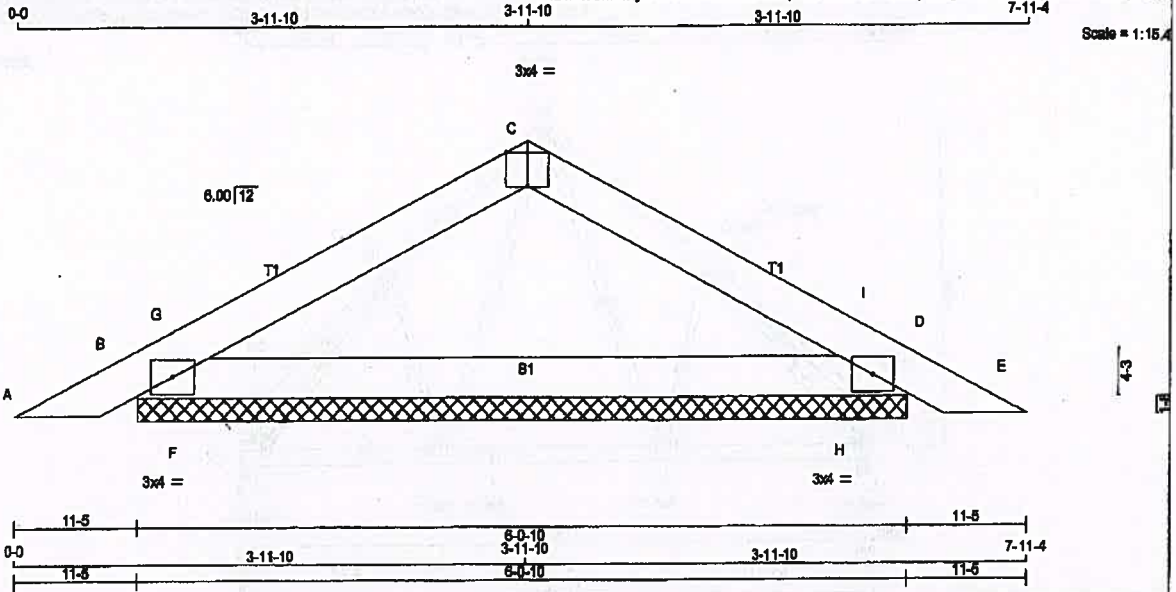
DRG NO. TAM 1105295
STRUCTURAL
CALCULATION ONLY

JOB NAME 297078	TRUSS NAME P1	QUANTITY 56	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:15.4

TOTAL WEIGHT = 58 X 17 = 983 LB

LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (kxh in in inches)	W	LEN	Y	X
JT TYPE				
B TMB1-1	MT20	3.0	4.0	
C TT-2	MT20	3.0	4.0	Edge 2.00
D TMB1-1	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	623	0	623	0	0	6-0-10	6-0-10
D	623	0	623	0	0	6-0-10	6-0-10

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
B	451	298 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0
D	451	298 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBB				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 24	-129.7	-129.7	0.07 (1)	10.00	F-G	0 / 294	0.00 (1)	
B-G	-613 / 0	-129.7	-129.7	0.16 (2)	6.25	H-I	0 / 294	0.00 (1)	
G-C	-448 / 0	-129.7	-129.7	0.23 (1)	6.25				
C-I	-448 / 0	-129.7	-129.7	0.23 (1)	6.25				
I-D	-613 / 0	-129.7	-129.7	0.16 (2)	6.25				
D-E	0 / 24	-129.7	-129.7	0.07 (1)	10.00				
B-F	0 / 417	-38.5	-38.5	0.11 (3)	10.00				
F-H	0 / 417	-38.5	-38.5	0.23 (1)	10.00				
H-D	0 / 417	-38.5	-38.5	0.11 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2012, CBC 2018
- CSA 088-08, CSA 088-14
- TPC 2011, TPC 2014

(55 % OF 64.3 P.S.F. Q.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.23/1.00 (C-1), BC=0.23/1.00 (F-1), WB=0.00/1.00 (F-G-1), GS1=0.21/1.00 (B-G-2)

DOL LUMBER=1.00 NAIL=1.00 L8 BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(P8)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	384	1087
	788	1987	1688

PLATE PLACEMENT TOL = 0.250 inches.

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.45 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



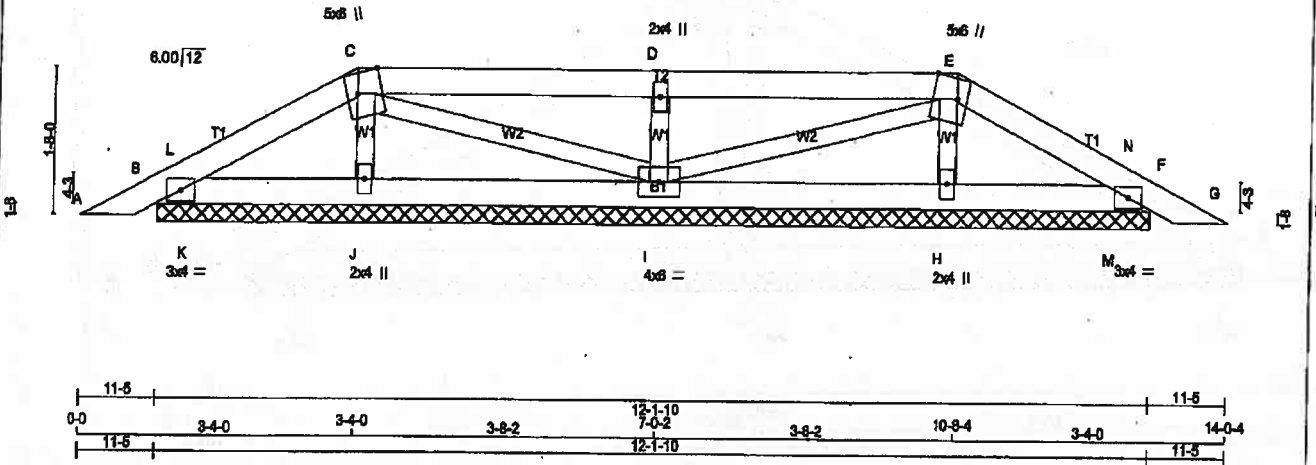
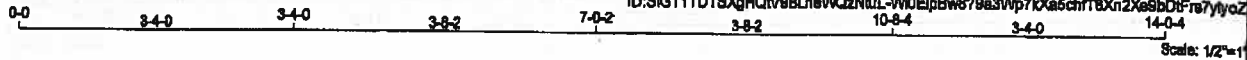
DWG NO. TAM 7100969
STRUCTURAL
COMPONENT ONLY



JOB NAME 297081	TRUSS NAME P2	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
E - G	2x4	DRY No.2
G - I	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table in in/inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	8.0	Edge 3.50
D	TWW+m	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	8.0	Edge 3.50
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+m	MT20	2.0	4.0	
I	BMW1+m	MT20	4.0	8.0	
J	BMW1+m	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	328 0	328 0	12-1-10	12-1-10
F	328 0	328 0	12-1-10	12-1-10
J	496 0	496 0	12-1-10	12-1-10
I	744 0	744 0	12-1-10	12-1-10
H	496 0	496 0	12-1-10	12-1-10

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
B	230	171/0 18/0 0/0 0/0 40/0 0/0
F	230	171/0 18/0 0/0 0/0 40/0 0/0
J	328	183/0 69/0 0/0 0/0 75/0 0/0
I	540	354/0 79/0 0/0 0/0 109/0 0/0
H	328	183/0 69/0 0/0 0/0 75/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CBI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CBI (LC)	
FR-TO				FR-TO			
A-B	0/24	-129.7 -129.7	0.07 (1)	J-C	-272/0	0.04 (1)	
B-L	-61/0	-129.7 -129.7	0.01 (5)	C-I	-29/0	0.01 (1)	
L-C	-61/0	-129.7 -129.7	0.09 (1)	I-D	-298/0	0.08 (1)	
C-D	-3/0	-129.7 -129.7	0.30 (1)	D-E	-29/0	0.01 (1)	
D-E	-3/0	-129.7 -129.7	0.30 (1)	E-F	-272/0	0.04 (1)	
E-F	-61/0	-129.7 -129.7	0.09 (1)	F-G	-193/8	0.00 (1)	
F-G	-61/0	-129.7 -129.7	0.01 (5)	G-H	-193/8	0.00 (1)	
G-H	0/24	-129.7 -129.7	0.07 (1)	H-I			
H-I				I-J			
I-J				J-K			
J-K	0/54	-38.5 -38.5	0.09 (1)	K-L			
K-L	0/54	-38.5 -38.5	0.09 (1)	L-M			
L-M	0/31	-38.5 -38.5	0.08 (3)	M-N			
M-N	0/31	-38.5 -38.5	0.08 (3)	N-O			
N-O	0/31	-38.5 -38.5	0.08 (3)	O-P			
O-P	0/54	-38.5 -38.5	0.09 (1)	P-Q			
P-Q	0/54	-38.5 -38.5	0.09 (1)	Q-R			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-08, CSA 086-14
- TRC 2011, TRC 2014

(35 % OF 54.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CBI TO=0.30/1.00 (D-E-1), BC=0.08/1.00 (J-K-1), WB=0.08/1.00 (D-E-1), SSB=0.23/1.00 (C-D-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PLI)
MT20	818	354	1887

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)



DRWG NO. TAM **T1805266**
STRUCTURAL
COMPONENT ONLY



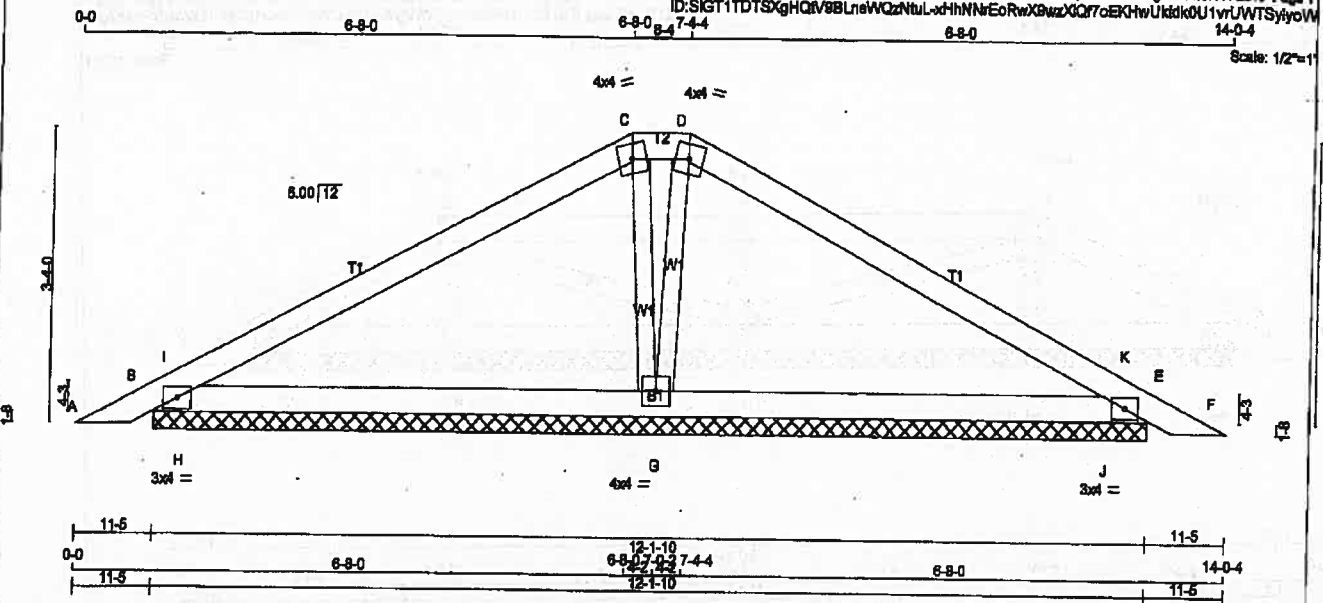
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	P3	2	1	TRUSS DESC.	

Remarck Roof Truss, Burlington

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ID:SIGT1TDTSXgHCIV8BLrWQZnUuLxHhNNrEoRwXQwzXQ77cEKHwUkdkOU1yrUWTSyiyovM

Scale: 1/2"=1'



TOTAL WEIGHT = 2 X 38 = 72 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x8	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (Table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B-TMB1-I	MT20	3.0	4.0		
C-TTW-m	MT20	4.0	4.0		
D-TTW-m	MT20	4.0	4.0		
E-TMB1-I	MT20	3.0	4.0		
G-BMW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
B	680	0	12-1-10	12-1-10
E	685	0	12-1-10	12-1-10
G	924	0	12-1-10	12-1-10

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	489	335 / 0	81 / 0	0 / 0	0 / 0	83 / 0	0 / 0
E	478	328 / 0	59 / 0	0 / 0	0 / 0	91 / 0	0 / 0
G	685	399 / 0	135 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CBI (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CBI (LBS)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 24	-129.7	-129.7 0.07 (1)	10.00	C-G	-242 / 0	0.05 (1)
B-I	-142 / 0	-129.7	-129.7 0.19 (1)	6.25	G-D	-246 / 0	0.05 (1)
I-C	-288 / 0	-129.7	-129.7 0.54 (1)	6.25	H-I	-720 / 80	0.00 (1)
C-D	-210 / 0	-129.7	-129.7 0.01 (1)	6.25	J-K	-749 / 57	0.00 (1)
D-K	-271 / 0	-129.7	-129.7 0.54 (1)	6.25			
K-E	-129 / 0	-129.7	-129.7 0.21 (1)	6.25			
E-F	0 / 24	-129.7	-129.7 0.07 (1)	10.00			
B-H	0 / 247	-38.5	-38.5 0.44 (1)	10.00			
H-G	0 / 247	-38.5	-38.5 0.48 (1)	10.00			
G-J	0 / 231	-38.5	-38.5 0.48 (1)	10.00			
J-E	0 / 231	-38.5	-38.5 0.45 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 51.7 PSF

SPACING = 24.0 IN. G.C.

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2012, CBC 2016
- CSA 088-06, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 84.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 58.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CBI: TC=0.54/1.00 (C-I:1), BC=0.48/1.00 (G-H:1), WB=0.05/1.00 (D-G:1), SB=0.58/1.00 (E-J:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1667 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.58 (E) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)



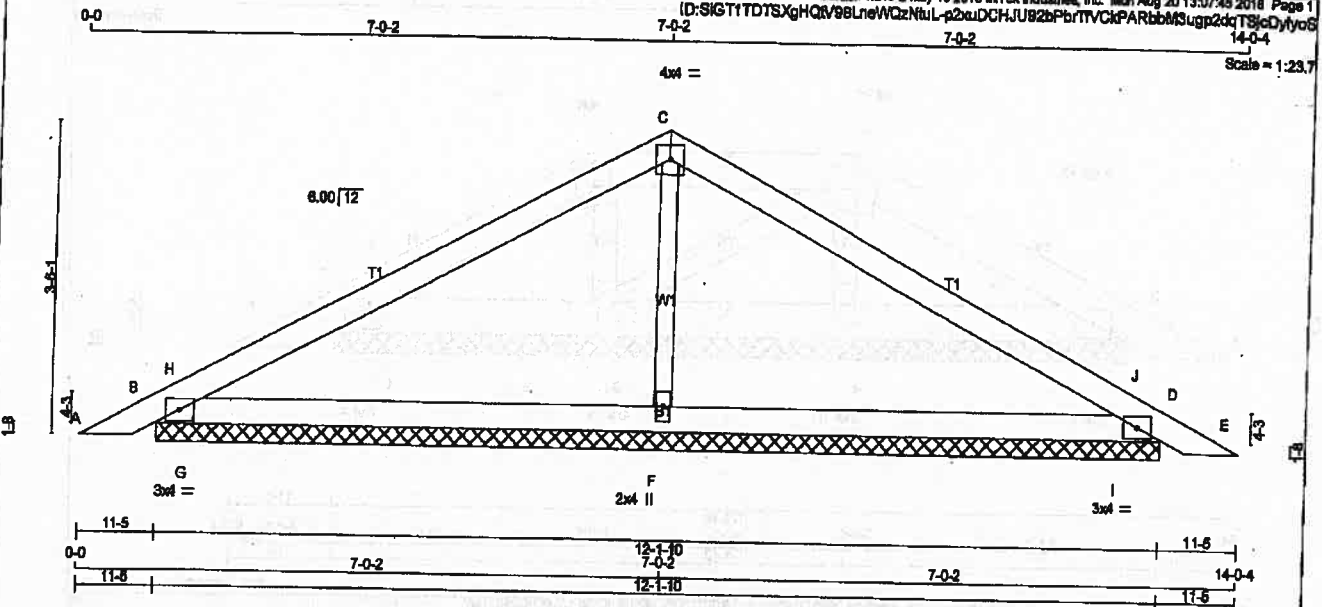
DWG NO. TAM 11605267
STRUCTURAL
COMPONENT ONLY



JOB NAME 297081	TRUSS NAME P4	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Temerack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 34 = 136 lb

LUMBER

N.L.G.A. RULES	SIZE	LUMBER
CHORDS		
A - C	2x4 DRY	No.2
C - E	2x4 DRY	No.2
B - D	2x4 DRY	No.2
ALL WEBS	2x2 DRY	No.2
DRY, SEASONED LUMBER		

DESCR

SPF
SPF
SPF

UNFACTORED REACTIONS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION UP	INPUT BRG	REQD BRG
B	711	0	711	0	12-1-10	12-1-10
D	711	0	711	0	12-1-10	12-1-10
F	846	0	846	0	12-1-10	12-1-10

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH	LL = 38.2 PSF
	DL = 8.0 PSF
BOT CH	LL = 10.3 PSF
	DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF	

PLATES (note is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0		
C	TTW-P	MT20	4.0	4.0		
D	TMB1-H	MT20	3.0	4.0		
F	BMW1-W	MT20	2.0	4.0		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	WIND	PERM. LIVE	WIND	DEAD	SOIL
B	510	354/0	80/0	0/0	0/0	88/0	0/0
D	510	354/0	80/0	0/0	0/0	88/0	0/0
F	623	354/0	134/0	0/0	0/0	148/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (I.C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (I.C)	
FR-TO	0/24	-129.7	-129.7 0.07 (1)	FR-TO	0/24	-129.7	-129.7 0.07 (1)
A-B	-133/0	-129.7	-129.7 0.28 (1)	G-H	-886/57	-886/57	0.07 (1)
B-H	-335/0	-129.7	-129.7 0.00 (1)	H-J	-886/57	-886/57	0.00 (1)
C-H	-335/0	-129.7	-129.7 0.00 (1)				
C-J	-335/0	-129.7	-129.7 0.00 (1)				
J-D	-133/0	-129.7	-129.7 0.28 (1)				
D-E	0/24	-129.7	-129.7 0.07 (1)				
B-G	0/287	-38.5	-38.5 0.52 (1)				
G-F	0/287	-38.5	-38.5 0.53 (1)				
F-I	0/287	-38.5	-38.5 0.53 (1)				
I-D	0/287	-38.5	-38.5 0.52 (1)				

SPACING = 24" IN G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2015
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(85 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.80/1.00 (C-F-1), BC=0.83/1.00 (F-G-1), WB=0.07/1.00 (C-F-1), SB=0.88/1.00 (B-G-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

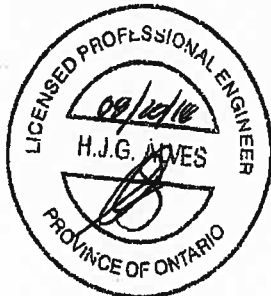
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	818	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.80)

JSI METAL= 0.18 (D) (INPUT = 1.00)



DWG NO. TAM 1805268
STRUCTURAL
COMPONENT ONLY

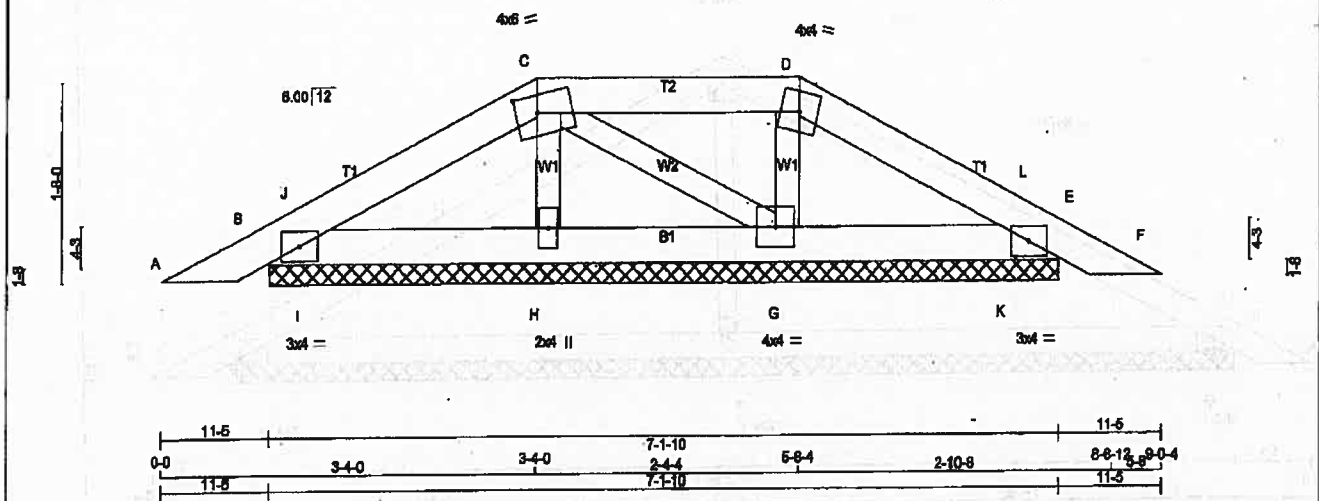


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	P11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 5 May 18 2018 Mitek Industries, Inc. Mon Aug 20 11:47:46 2018 Page 1
ID:4u167_apsj?wvBLJKDwJbPzNv7w-qoSg0GC3RQW85T807oMP9Bb8rtbmObFKSHgtyzzR

0-0 3-4-0 3-4-0 2-4-4 5-8-4 3-4-0 9-0-4
Scale = 1:17.5



TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMS1-H	MT20	3.0	4.0	
C	TTWW-m	MT20	4.0	8.0	1.75 2.25
D	TTW-m	MT20	4.0	4.0	
E	TMS1-H	MT20	3.0	4.0	
G	BMWV1-H	MT20	4.0	4.0	
H	BMWV1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
B	341	0	341	0
C	329	0	329	0
D	358	0	358	0
E	399	0	399	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
B	243	178/0	24/0	0/0	0/0	43/0	0/0	
E	234	188/0	23/0	0/0	0/0	42/0	0/0	
H	265	198/0	51/0	0/0	0/0	58/0	0/0	
G	294	180/0	52/0	0/0	0/0	63/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/24	-128.7 -128.7	0.07 (1)	10.00	H-C	-234/0	0.09 (1)
B-J	-87/0	-128.7 -128.7	0.02 (3)	6.25	C-D	-30/0	0.00 (1)
J-C	-79/0	-128.7 -128.7	0.09 (1)	6.25	G-D	-260/0	0.04 (1)
C-D	-28/0	-128.7 -128.7	0.12 (1)	6.25	I-J	-138/22	0.00 (1)
D-L	-52/0	-128.7 -128.7	0.06 (1)	6.25	K-L	-138/22	0.00 (1)
L-E	-60/0	-128.7 -128.7	0.02 (3)	6.25			
E-F	0/24	-128.7 -128.7	0.07 (1)	10.00			
B-I	0/71	-38.5 -38.5	0.09 (1)	10.00			
I-H	0/71	-38.5 -38.5	0.09 (1)	10.00			
H-G	0/52	-38.5 -38.5	0.06 (1)	10.00			
G-K	0/48	-38.5 -38.5	0.09 (1)	10.00			
K-E	0/48	-38.5 -38.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2015
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 64.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.12/1.00 (C-D:1), BC=0.09/1.00 (B-I), WB=0.04/1.00 (D-G:1), BB=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

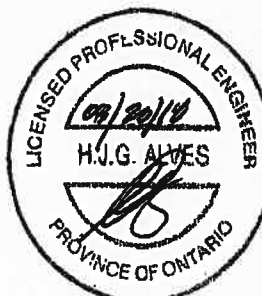
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	516	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



DWG NO. TAM 11005215
STRUCTURAL
COMPONENT ONLY

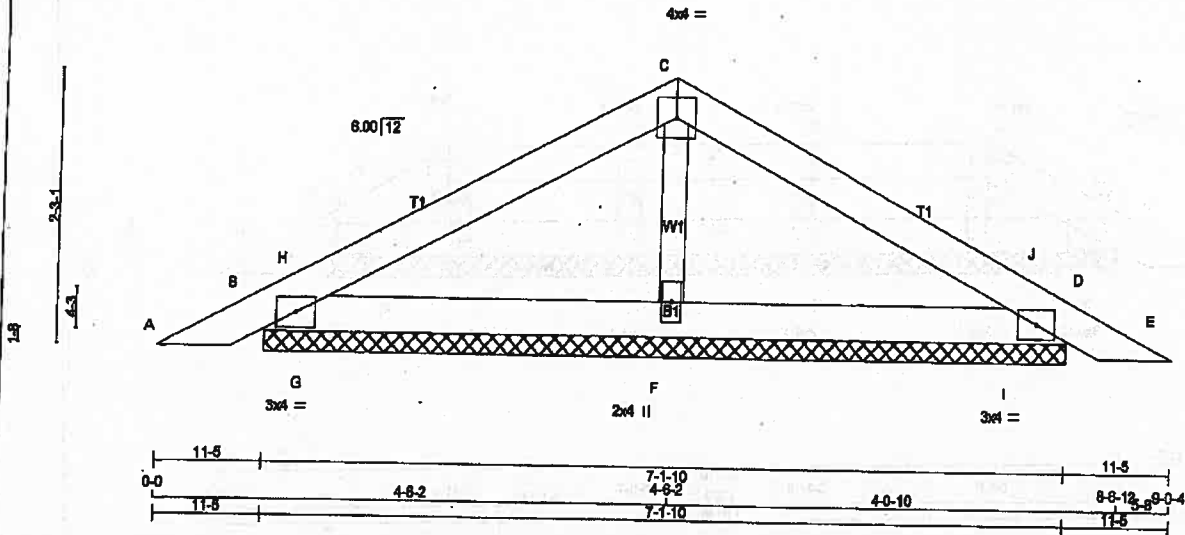


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297079	P12	10	1			

Tamrack Roof Truss, Burlington

Version 8.210 8 May 18 2018 Mitek Industries, Inc. Mon Aug 20 11:47:50 2018 Page 1
ID:4u167_sps7wBfJKDwJbPzNv7wJZiAdFajwvxeirvFygiZ7LF0ScFIBXrDyQUpfytzzN

Scale = 1:17.4



LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
B - D 2x4 DRY No.2
ALL WEBS 2x8 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB1-I M720 3.0 4.0
C TTW-p M720 4.0 4.0
D TMB1-I M720 3.0 4.0
F BMW1+rw M720 2.0 4.0

DESIGNER
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG			
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SK	IN-SK		
B	438	0	438	0	0	7-1-10	7-1-10		
D	438	0	438	0	0	7-1-10	7-1-10		
F	552	0	552	0	0	7-1-10	7-1-10		

UNFACTORED REACTIONS									
1ST CASE MAX/MIN COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
B	312	222 / 0	33 / 0	0 / 0	0 / 0	57 / 0	0 / 0		
D	312	222 / 0	33 / 0	0 / 0	0 / 0	57 / 0	0 / 0		
F	411	235 / 0	33 / 0	0 / 0	0 / 0	92 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS									
MEMB.		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (LBS)	WEBS		MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO			FROM TO			FR-TO			
A-B		0 / 24	-129.7 -129.7		0.07 (1)	F-C		-288 / 0	0.04 (1)
B-H		-98 / 0	-129.7 -129.7		0.05 (1)	G-H		-331 / 21	0.00 (1)
H-C		-136 / 0	-129.7 -129.7		0.19 (1)	I-J		-331 / 21	0.00 (1)
C-J		-136 / 0	-129.7 -129.7		0.19 (1)				
J-D		-86 / 0	-129.7 -129.7		0.05 (1)				
D-E		0 / 24	-129.7 -129.7		0.07 (1)				
B-G		0 / 117	-38.5 -38.5		0.19 (1)				
G-F		0 / 117	-38.5 -38.5		0.19 (1)				
F-I		0 / 117	-38.5 -38.5		0.19 (1)				
I-D		0 / 117	-38.5 -38.5		0.19 (1)				

TOTAL WEIGHT = 10 X 21 = 209 lb

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 51.7 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2015
- CSA 086-08, CSA 086-14
- TBC 2011, TBC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TO=0.18/1.00 (C-F-1), BC=0.18/1.00 (F-G-1), WB=0.04/1.00 (C-F-1), BB=0.25/1.00 (D-I-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1957 1659

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.38 (D) (INPUT = 0.80)
JSI METAL = 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 71805216
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	P13	2	1	TRUSS DESC.	

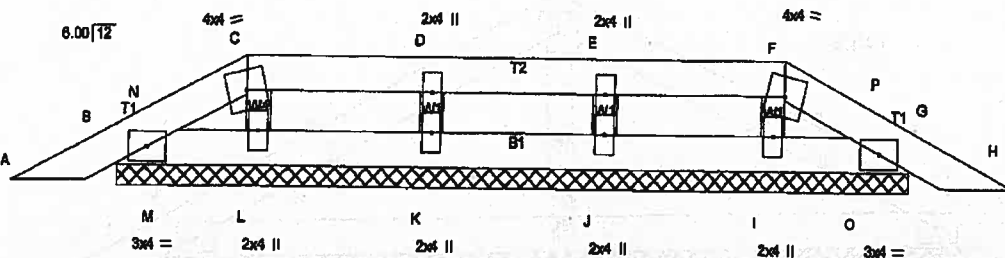
Tamereck Roof Truss, Burlington

Version 8.210 8 May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:07:48 2018 Page 1

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0-0 2-1-0 2-1-0 1-7-13 3-8-13 1-8-8 5-3-7 1-7-13 6-11-4 2-1-0 9-0-4

Scale = 1:17.6



11-5 2-1-0 2-1-0 1-7-13 3-8-13 1-8-8 5-3-7 1-7-13 6-11-4 2-1-0 9-0-4

TOTAL WEIGHT = 2 X 20 = 40 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - F	2x4 DRY	No.2
F - H	2x4 DRY	No.2
B - G	2x4 DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

DESIGN

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	DOWN	UP	DOWN	UP		
B	250	0	250	0	7-1-10	7-1-10
G	250	0	250	0	7-1-10	7-1-10
L	185	0	185	0	7-1-10	7-1-10
K	290	0	290	0	7-1-10	7-1-10
J	290	0	290	0	7-1-10	7-1-10
I	185	0	185	0	7-1-10	7-1-10

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	181	142/0	10/0	0/0	0/0	29/0	0/0
G	181	142/0	10/0	0/0	0/0	29/0	0/0
L	128	83/0	32/0	0/0	0/0	31/0	0/0
K	211	135/0	33/0	0/0	0/0	43/0	0/0
J	211	135/0	33/0	0/0	0/0	43/0	0/0
I	128	83/0	32/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS B, G, L, K, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LCG (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO			FR-TO		
A-B	0/24	-129.7 -129.7	0.07 (1)	10.00	L-C	-123/0	0.02 (1)
B-N	-75/0	-129.7 -129.7	0.07 (1)	6.25	K-D	-228/0	0.08 (1)
C-D	-20/0	-129.7 -129.7	0.01 (1)	6.25	J-E	-228/0	0.08 (1)
C-D	0/5	-129.7 -129.7	0.05 (1)	10.00	I-F	-123/0	0.02 (1)
D-E	0/5	-129.7 -129.7	0.05 (1)	10.00	M-N	0/34	0.00 (1)
E-F	0/5	-129.7 -129.7	0.06 (1)	10.00	C-P	0/34	0.00 (1)
F-P	-20/0	-129.7 -129.7	0.01 (1)	6.25			
P-G	-75/0	-129.7 -129.7	0.07 (1)	6.25			
G-H	0/24	-129.7 -129.7	0.07 (1)	10.00			
B-M	0/15	-38.5 -38.5	0.01 (1)	10.00			
M-L	0/15	-38.5 -38.5	0.01 (3)	10.00			
L-K	-5/0	-38.5 -38.5	0.01 (3)	10.00			
K-J	-5/0	-38.5 -38.5	0.01 (3)	10.00			
J-I	-5/0	-38.5 -38.5	0.01 (3)	10.00			
I-O	0/15	-38.5 -38.5	0.01 (3)	10.00			
O-G	0/15	-38.5 -38.5	0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 38.2 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 61.7 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00H/2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, NBC 2015
- CSA S8S-08, CSA S8S-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TC=0.071/0.0 (G-H-1), SC=0.01/0.0 (J-K-2), WS=0.03/1.00 (D-K-1), SS=0.10/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

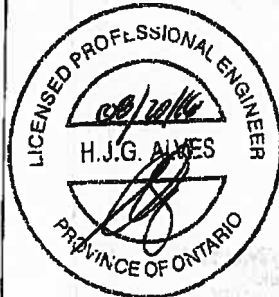
PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1867 788	1987 1698

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.80)
JSI METAL= 0.05 (F) (INPUT = 1.00)

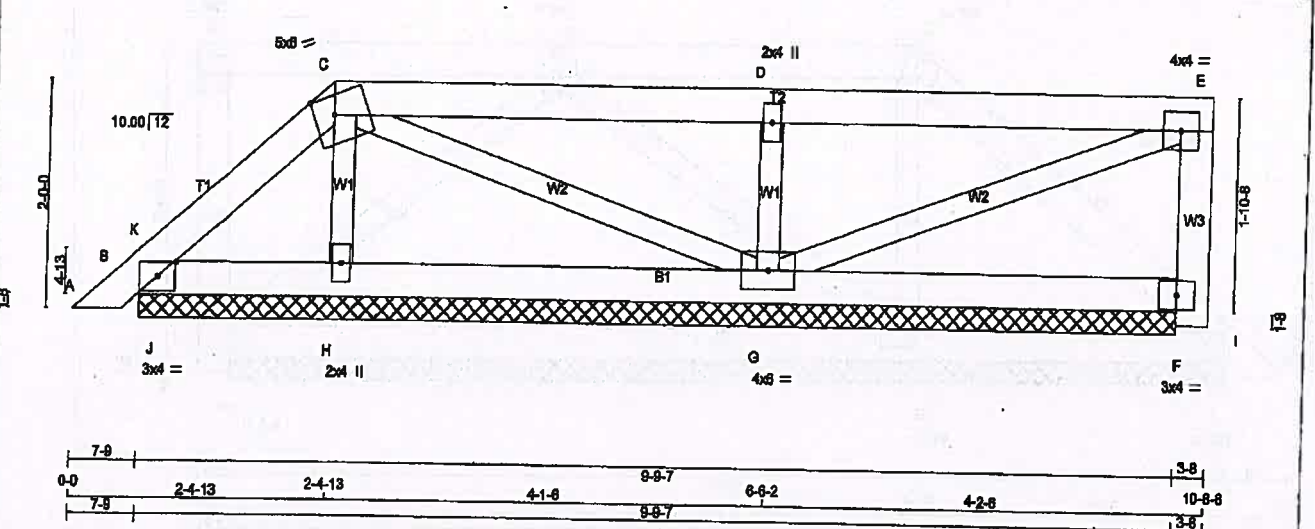
DWG NO. TAM 11805269
STRUCTURAL
COMPONENT ONLY





JOB NAME 297081	TRUSS NAME P101	QUANTITY 2	PLY 1	JOB DESIG. TRUSS DESC.	DRWG NO.
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Version 8.210 5 May 18 2018 MTK Industries, Inc. Mon Aug 20 13:07:32 2018 Page 1
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10-8-8
Scale = 1:18.8



LUMBER

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
I - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.25
D	TMMW-w	MT20	2.0	4.0		
E	TMMW-t	MT20	4.0	4.0		
F	BVM-I	MT20	3.0	4.0		
G	BMMWMM-I	MT20	4.0	8.0		
H	BMMW-I-w	MT20	2.0	4.0		

FACTORED REACTIONS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
B	11	0	11	0	9-9-7	9-9-7
H	442	0	442	0	9-9-7	9-9-7
G	1812	0	1812	0	9-9-7	9-9-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 160 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	-1	28/0	0/20	0/0	0/0	0/9	0/0
H	332	160/0	74/0	0/0	0/0	78/0	0/0
G	880	803/0	158/0	0/0	0/0	200/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS) CSI (LC)
A-B	0/120	-128.7	-128.7 0.03 (1)	10.00	H-C -304/0 0.04 (1)
B-K	0/330	-128.7	-128.7 0.08 (1)	10.00	C-G -478/0 0.16 (1)
K-C	0/288	-128.7	-128.7 0.10 (1)	10.00	G-D -687/0 0.10 (1)
C-D	0/688	-128.7	-128.7 0.62 (1)	10.00	D-E -733/0 0.23 (1)
D-E	0/688	-128.7	-128.7 0.62 (1)	10.00	E-F -177/0 0.00 (1)
F-F	0/0	0.0	0.0 0.00 (1)	10.00	
F-E	0/79	0.0	0.0 0.01 (3)	10.00	
B-J	-218/0	-38.5	-38.5 0.06 (1)	6.25	
J-H	-218/0	-38.5	-38.5 0.06 (3)	6.25	
H-G	-239/0	-38.5	-38.5 0.15 (2)	6.25	
G-F	0/0	-38.5	-38.5 0.15 (2)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 38.2 PSF
DL = 8.0 PSF
BOT CH LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 54.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.32/1.00 (C-D-1), BC=0.15/1.00 (G-H-2), WB=0.23/1.00 (E-G-1), SB=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 516 354 1687 788 1987 1688

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.50)
JSI METAL= 0.26 (E) (INPUT = 1.00)



DRWG NO. TAM 11805270
STRUCTURAL
COMPONENT ONLY

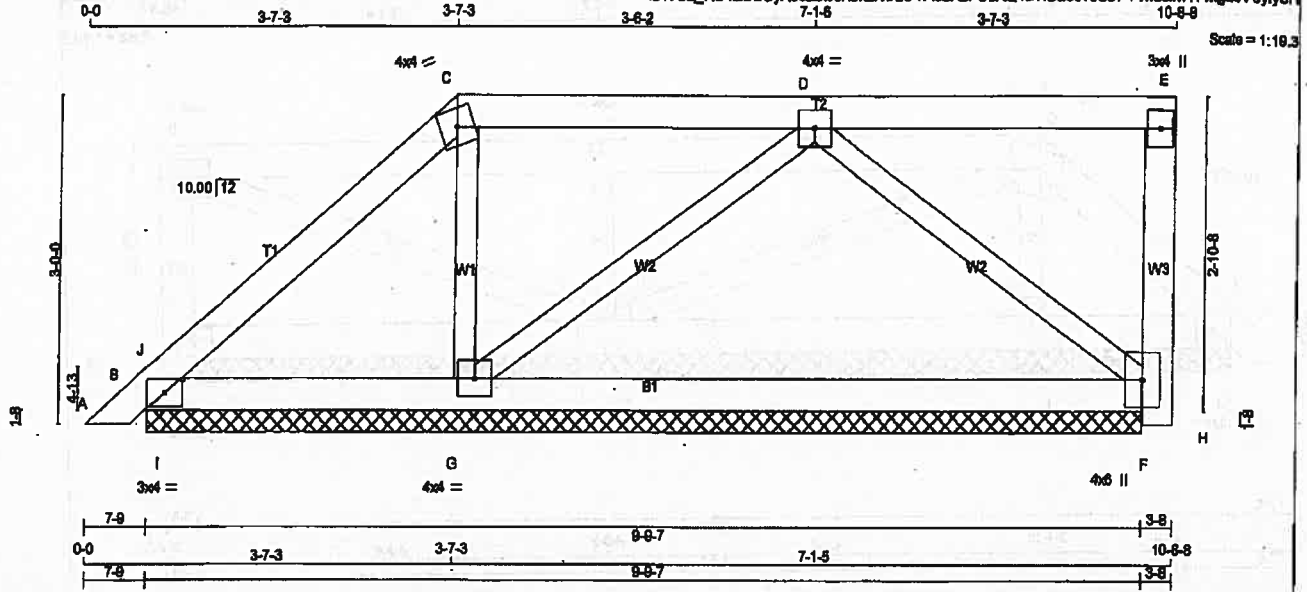


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297081	P102	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 8 May 18 2018 MITek Industries, Inc. Mon Aug 20 13:07:58 2018 Page 1

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LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS 2x4 DRY No.2			
DRY: SEASONED LUMBER			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMB-1	MT20	4.0	1.50 2.00
C	TTW-m	MT20	4.0	4.0
D	TMMW-1	MT20	4.0	4.0
E	TMB-1	MT20	4.0	4.0
F	BVMW-1	MT20	4.0	8.0
G	BMMW-1	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	284	0	284	0	0	9-9-7	9-9-7
F	581	0	581	0	0	9-9-7	9-9-7
G	940	0	940	0	0	9-9-7	9-9-7

UNFACTORED REACTIONS							
1ST CASE							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	178	181/0	0/-5	0/0	0/0	22/0	0/0
F	408	289/0	57/0	0/0	0/0	80/0	0/0
G	707	382/0	159/0	0/0	0/0	169/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX.	LC1
	(LBS)	(PLF)	(LC)		(LBS)	(LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/20	-128.7	-128.7 0.05 (1)	10.00	G-C	-320/0	0.05 (1)
B-J	0/183	-128.7	-128.7 0.12 (1)	10.00	G-D	-490/0	0.15 (1)
J-C	-23/7	-128.7	-128.7 0.12 (1)	6.25	D-F	-471/0	0.15 (1)
C-D	0/13	-128.7	-128.7 0.27 (1)	10.00	F-J	-487/0	0.00 (1)
D-E	0/0	-128.7	-128.7 0.27 (1)	10.00			
E-F	0/0	0.0	0.0 0.00 (1)	10.00			
F-E	-179/0	0.0	0.0 0.05 (1)	7.81			
B-I	-11/6	-38.5	-38.5 0.15 (1)	6.25			
I-G	-11/6	-38.5	-38.5 0.33 (3)	6.25			
G-F	0/383	-38.5	-38.5 0.37 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 6, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OSC 2012, OSC 2016
- CSA 086-08, CSA 086-14
- TRC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(85 % OF 64.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CE: TC=0.27/1.00 (C-D-1), BC=0.37/1.00 (F-G-2), WB=0.19/1.00 (D-G-1), SB=0.35/1.00 (B-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(FBI)	(PL)	(PL)	(PL)
MT20	618	354	1657
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.44 (3) (INPUT = 0.80)
JSI METAL= 0.14 (3) (INPUT = 1.00)

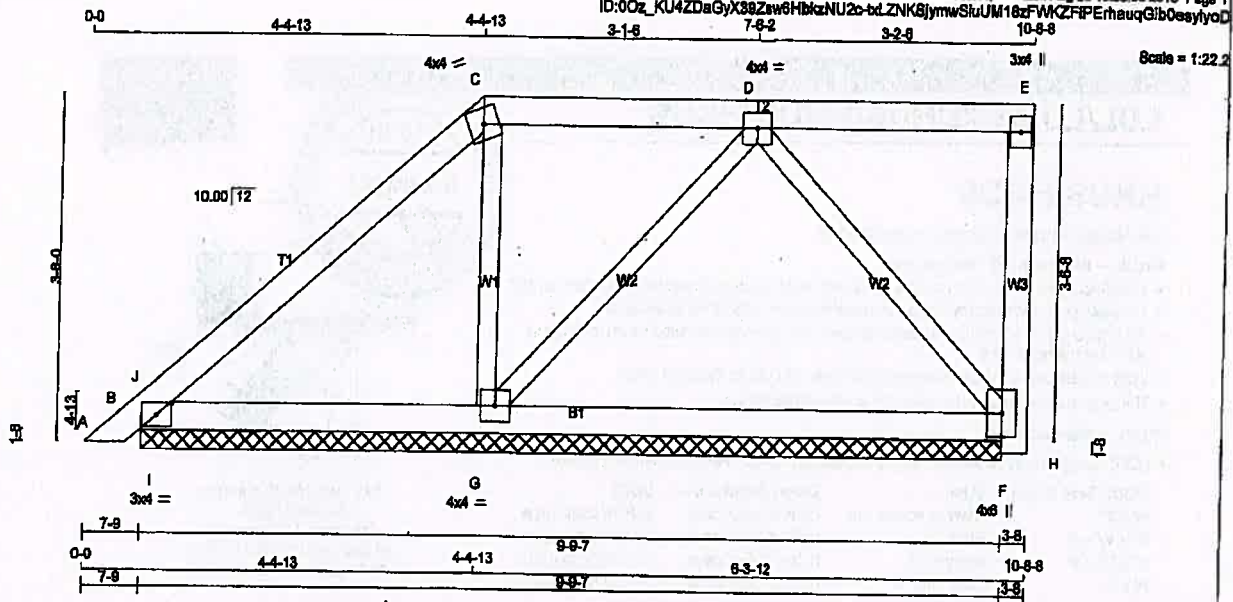


DRWG NO. TAM 11605271
STRUCTURAL
COMPONENT ONLY



JOB NAME 297081	TRUSS NAME P103	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.210.8 May 18 2018 Mitek Industries, Inc. Mon Aug 20 13:08:00 2018 Page 1
ID:00z_KU4ZDaGyX38Zaw6HbzNU2o-bLZNK8jymwStuUM18zFWKZFP ErhaugGib0easyfy0D



LUMBER

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
H - E	2x4	DRY No.2
B - F	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
DRY: SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMAWV-t	MT20	4.0	4.0		
E	TMAV-p	MT20	3.0	4.0		
F	BV/MW1+p	MT20	4.0	8.0		
G	BMWV1-t	MT20	4.0	4.0		

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	411	0	411	0	9-9.7	9-9.7
F	531	0	531	0	9-9.7	9-9.7
G	822	0	822	0	9-9.7	9-9.7

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	288	220 / 0	21 / 0	0 / 0	0 / 0	48 / 0	0 / 0
F	384	256 / 0	83 / 0	0 / 0	0 / 0	75 / 0	0 / 0
G	618	336 / 0	138 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		FROM	TO				
FR-TO							
A-B	0 / 20	-129.7	-129.7	0.08 (1)	G-C	-295 / 0	0.08 (1)
B-J	0 / 151	-129.7	-129.7	0.14 (1)	G-D	-308 / 0	0.10 (1)
J-C	-115 / 0	-129.7	-129.7	0.22 (1)	D-F	-394 / 0	0.13 (1)
C-D	-68 / 0	-129.7	-129.7	0.21 (1)	I-J	-608 / 0	0.00 (1)
D-E	0 / 0	-129.7	-129.7	0.21 (1)			
E-F	0 / 0	0.0	0.0	0.00 (1)			
F-G	-168 / 0	0.0	0.0	0.04 (1)			
B-I	0 / 76	-38.5	-38.5	0.21 (1)			
I-G	0 / 76	-38.5	-38.5	0.28 (2)			
G-F	0 / 278	-38.5	-38.5	0.30 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 38.2 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.8 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 61.7 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TO=0.22/1.00 (C-J-1), BC=0.30/1.00 (F-G-2), WB=0.13/1.00 (D-F-1), SS=0.47/1.00 (B-I-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1858	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)



DWG NO. TAM **T1805272**
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

HHUS/HGUS

See Hanger Options Information on pp. 125-127.

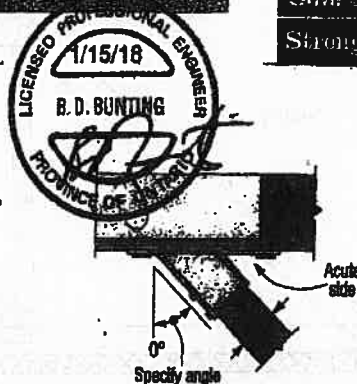
HHUS — Skewed and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or skewed to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For skewed only or skewed and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for skewed/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 8"	Bevel cut	0.75 of table value	0.41 of table value



Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

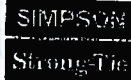
Hanger	SL	Bearing					Embedment		Resistance		Design	
		W	F	R	JA	Resistor	Joist	lb.			lb.	
		Single 2x Sizes										
LUS24	18	1 1/4	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1825	645	1158	
LU24L	22	1 1/4	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/4"	390	1020	320	725	
LUS22	22	1 1/4	6	1 1/4	4 1/4	(6) 10d	(2) 10d x 1 1/4"	720	1805	645	1140	
LUS18	18	1 1/4	4 1/4	1 1/4	3 1/4	(4) 10d	(6) 10d	1420	2170	1280	1830	
LUS16	16	1 1/4	5 1/4	3	3 1/4	(4) 10d	(6) 10d	2705	4940	2085	3875	
LUS10S	18	1 1/4	5	3 1/4	4 1/4	(6) 10d	(6) 10d	2055	4285	1480	4115	
LUS10L	12	3/4	5 1/4	5	4 1/4	(2) 10d	(6) 10d	2885	6625	2885	6700	
LU28L	20	1 1/4	6 1/4	1 1/4	5 1/4	(6) 10d	(6) 10d x 1 1/4"	1140	2185	1020	1650	
LUS28	18	1 1/4	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1280	1780	
HUS28	16	1 1/4	7 1/4	3	5 1/4	(22) 16d	(6) 16d	3605	6365	2875	4945	
HUS28	12	1 1/4	7 1/4	5	5 1/4	(36) 16d	(12) 16d	3310	7875	3310	6900	
LU210L	20	1 1/4	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/4"	1140	2485	1020	1770	
LUS210	18	1 1/4	7 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2765	1280	2210	

1. Factored uplift resistances have been increased 15% for wind or earthquake loadings; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_o is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girdler truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nail: 16d = 0.162" dia. x 3 1/4" long. See pp. 27-28 for other nail sizes and information.

C-C-042018 ©2017 SIMPSON STRONG-TIE COMPANY INC.



Face-Mount Hangers



These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 33-34 for more information.

Model No.	Gp.	Dimensions (in.)				Fasteners		Factored Resistance		S-P-F	
		W	H	S	d _h	Hanger	Joints	lb.		kN	
lb.	lb.	lb.	lb.								

Double 2x Sizes													
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 18d	(2) 18d	835	2020	590	1435		
LUS26-2	18	3 1/4	4 1/4	2	4	(4) 18d	(4) 18d	1720	2595	1645	1920		
HHUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 18d	(8) 18d	2850	7385	2065	6205		
HGUS26-2	12	3 1/4	5 1/4	4	4 1/4	(20) 18d	(8) 18d	4385	8950	3110	6355		
LUS28-2	18	3 1/4	7	2	4	(8) 18d	(4) 18d	1720	3325	1645	2575		
HHUS28-2	14	3 1/4	7 1/4	3	6 1/4	(22) 18d	(8) 18d	3765	8940	2875	6345		
HGUS28-2	12	3 1/4	7 1/4	4	8 1/4	(30) 18d	(12) 18d	6070	12980	4310	9215		
LUS210-2	18	3 1/4	9	2	6	(8) 18d	(8) 18d	2580	4500	2320	3195		
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 18d	(10) 18d	4670	9650	4235	7000		
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(48) 18d	(18) 18d	6840	14015	4855	10270		

Triple 2x Sizes													
HGUS28-3	12	4 1/4	5 1/4	4	4 1/4	(30) 18d	(8) 18d	4385	8950	3110	6355		
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 18d	(12) 18d	6070	12980	4310	9215		
HHUS28-3	14	4 1/4	9	3	7 1/4	(36) 18d	(10) 18d	4670	9670	4235	6885		
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(48) 18d	(18) 18d	6840	14645	4855	10400		

Quadruple 2x Sizes													
HGUS28-4	12	6 1/4	5 1/4	4	4 1/4	(30) 18d	(8) 18d	4385	8950	3110	6355		
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 18d	(12) 18d	6070	12980	4310	9215		
HHUS28-4	14	6 1/4	9	3	7 1/4	(36) 18d	(10) 18d	4670	10155	4235	7210		
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(48) 18d	(18) 18d	6840	14645	4855	10400		
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(36) 18d	(20) 18d	7840	14995	5425	10845		
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(36) 18d	(22) 18d	10130	18400	7195	11845		

4x Sizes													
LUS48	18	3 1/4	4 1/4	2	3 1/4	(4) 18d	(4) 18d	1720	2595	1645	1920		
HHUS48	14	3 1/4	5 1/4	3	3 1/4	(14) 18d	(8) 18d	2850	7385	2065	6205		
HGUS48	12	3 1/4	5 1/4	4	4 1/4	(20) 18d	(8) 18d	4385	8950	3110	6355		
LUS48	18	3 1/4	6 1/4	2	3 1/4	(6) 18d	(4) 18d	1720	3325	1645	2575		
HHUS48	14	3 1/4	7 1/4	3	6 1/4	(22) 18d	(8) 18d	3765	8940	2875	6345		
HGUS48	12	3 1/4	7 1/4	4	6 1/4	(30) 18d	(12) 18d	6070	12980	4310	9215		
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 18d	(8) 18d	2580	4500	2320	3195		
HGUS410	12	3 1/4	9	4	8 1/4	(48) 18d	(18) 18d	6840	14015	4855	10270		
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(36) 18d	(20) 18d	7840	14995	5425	10845		
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(36) 18d	(22) 18d	10130	18400	7195	11845		



Plated Truss Connectors

TBE - Truss Bearing Enhancers

SIMPSON
Strong-Tie

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to withstand the loads imposed by the nails.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.

TBE4
(TBE6 similar)

U.S. Patent
5,108,848
Canada Patent
2,044,440

Two TBE
installed
with two ply
girder truss

Model No.	No. of Plys	Fasteners		TBE Only Factored Resistance				Combined TBE and Wood Bearing Factored Resistance	
		Truss	Plate	Uplift (K _o = 1.15)	Normal (K _o = 1.00)	Lateral (K _o = 1.15)		Normal ¹ (K _o = 1.00)	TBL ¹
						F ₁	F ₂		
				lb.	lb.	lb.	lb.	lb.	in.
D-Fir-L									
TBE4	1	(20) 10d x 1 1/2"	(20) 10d x 1 1/2"	1805	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1805	3860	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3860	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3860	655	1415	20705	4.25
TBE6	1	(20) 10d x 1 1/2"	(20) 10d x 1 1/2"	1760	3540	490	1745	10235	6.41
	2	(20) 10d	(20) 10d	1760	3860	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3860	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3860	490	1745	30840	6.29
S-P-F									
TBE4	1	(20) 10d x 1 1/2"	(20) 10d x 1 1/2"	1805	3220	615	1415	6445	6.99
	2	(20) 10d	(20) 10d	1805	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	18345	4.43
TBE6	1	(20) 10d x 1 1/2"	(20) 10d x 1 1/2"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q_s/A_o and $Q_u/A_o = 612$ psi (5.60 MPa) for D-Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.



(800) 999-5099
strongtie.com

TS/LTS/MTS/HTS

SIMPSON
Strong-Tie

Twist Straps

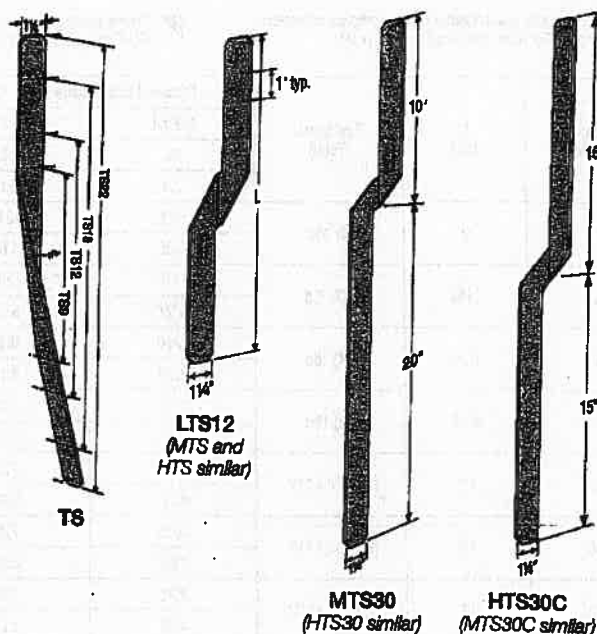
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

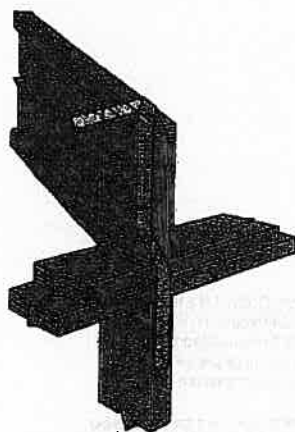
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

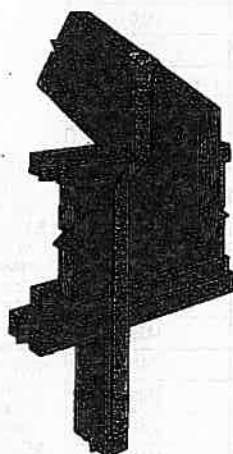
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



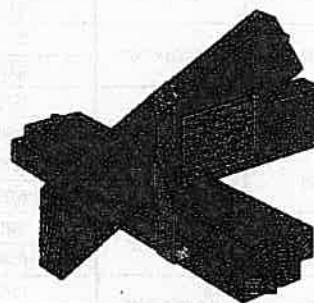
Straps and Ties



MTS30 Installation
with I-Joist Rafter



Typical MTS30
Installation

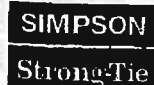


MTS Installation as a
Truss-to-Top Plate Tie



Typical TS
Installation

TS/LTS/MTS/HTS



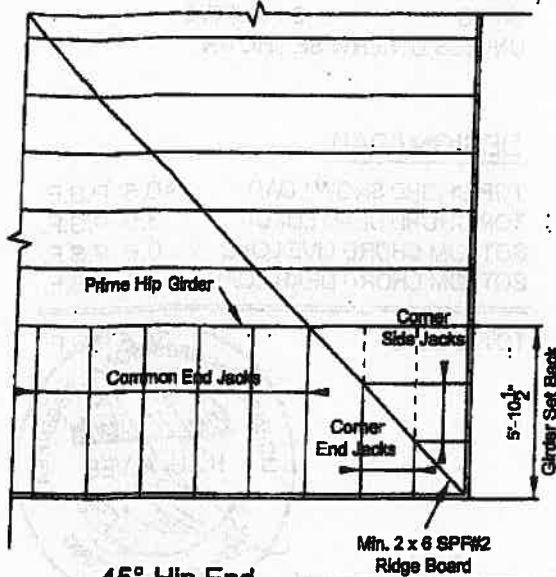
Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance ($K_D = 1.15$)	
			S-P-L	S-P-F
			lb.	kg
TS9	9	(8) 16d	1125	1040
			5.00	4.63
TS12	11½	(10) 16d	1410	1300
			6.27	5.78
TS18	17¾	(14) 16d	1970	1820
			8.78	8.10
TS22	21½	(18) 16d	2125	2125
			9.45	9.45
SS LTS12	12	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS16	16	(12) 10d x 1½"	1015	720
			4.52	3.20
SS LTS18	18	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS20	20	(12) 10d x 1½"	1015	720
			4.52	3.20
MTS12	12	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS16	16	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS18	18	(14) 10d x 1½"	1570	1180
			6.98	5.25
SS MTS20	20	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS24C	24	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30C	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
HTS16	16	(16) 10d x 1½"	2050	1455
			9.12	6.47
HTS20	20	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS24	24	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS28	28	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30	30	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30C	30	(24) 10d x 1½"	2050	1455
			9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.85 for LTS and 0.74 for MTS, HTS cannot be used in this application.
8. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.



45° Hip End

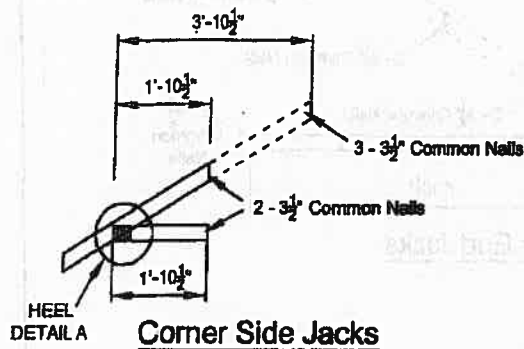
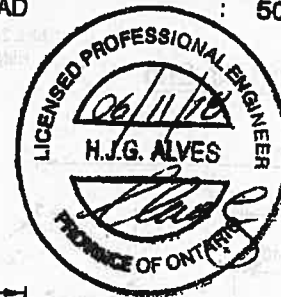
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

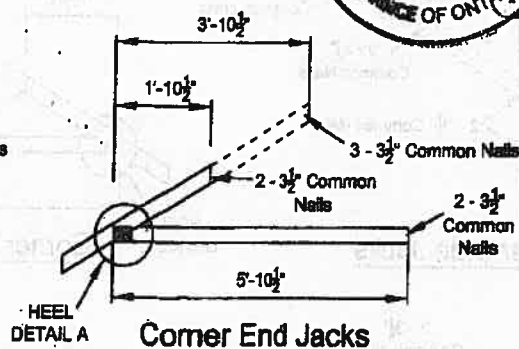
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

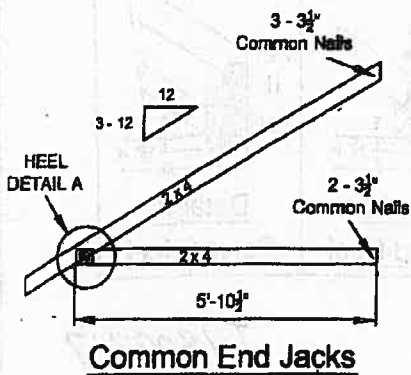
TOTAL LOAD : 50.5 P.S.F



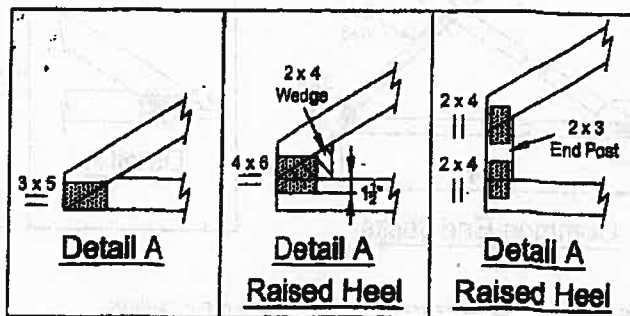
Corner Side Jacks



Corner End Jacks

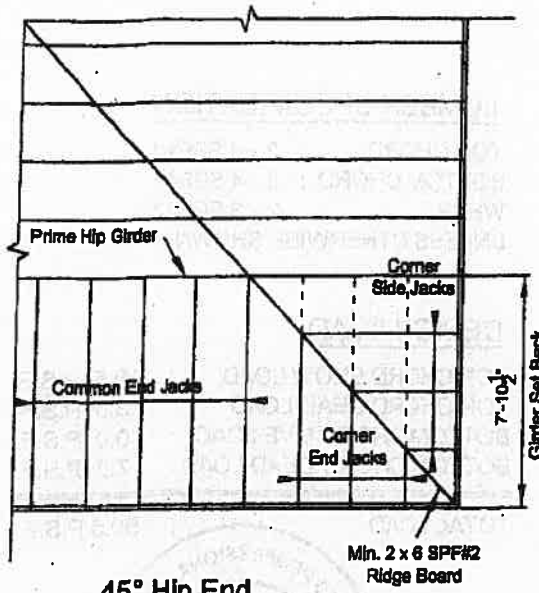


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

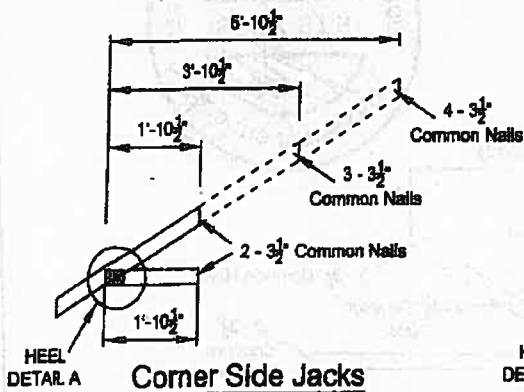
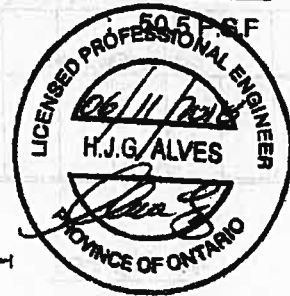
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

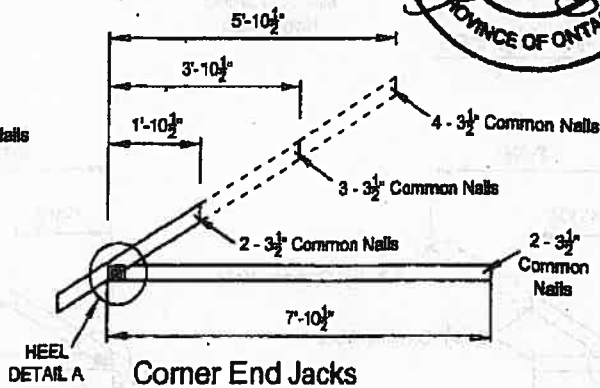
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

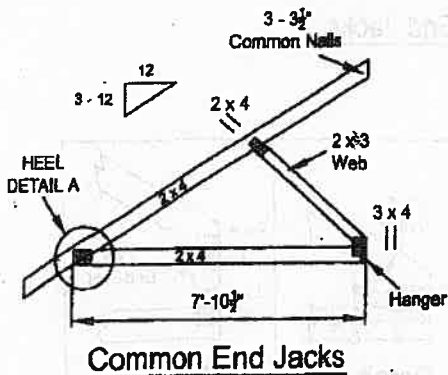
TOTAL LOAD



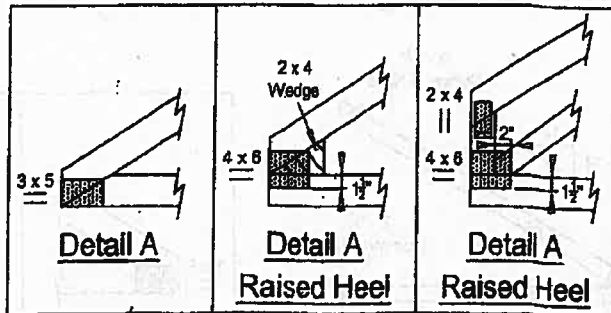
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

TECH-NOTES

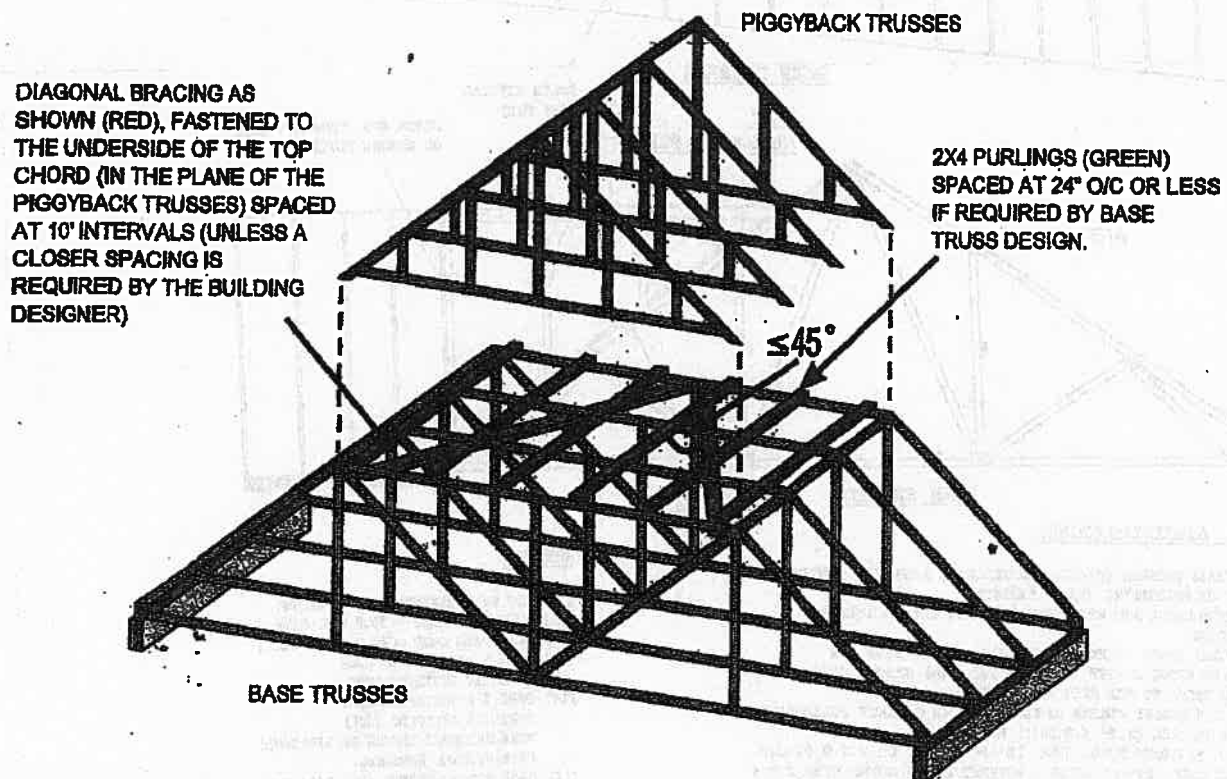
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

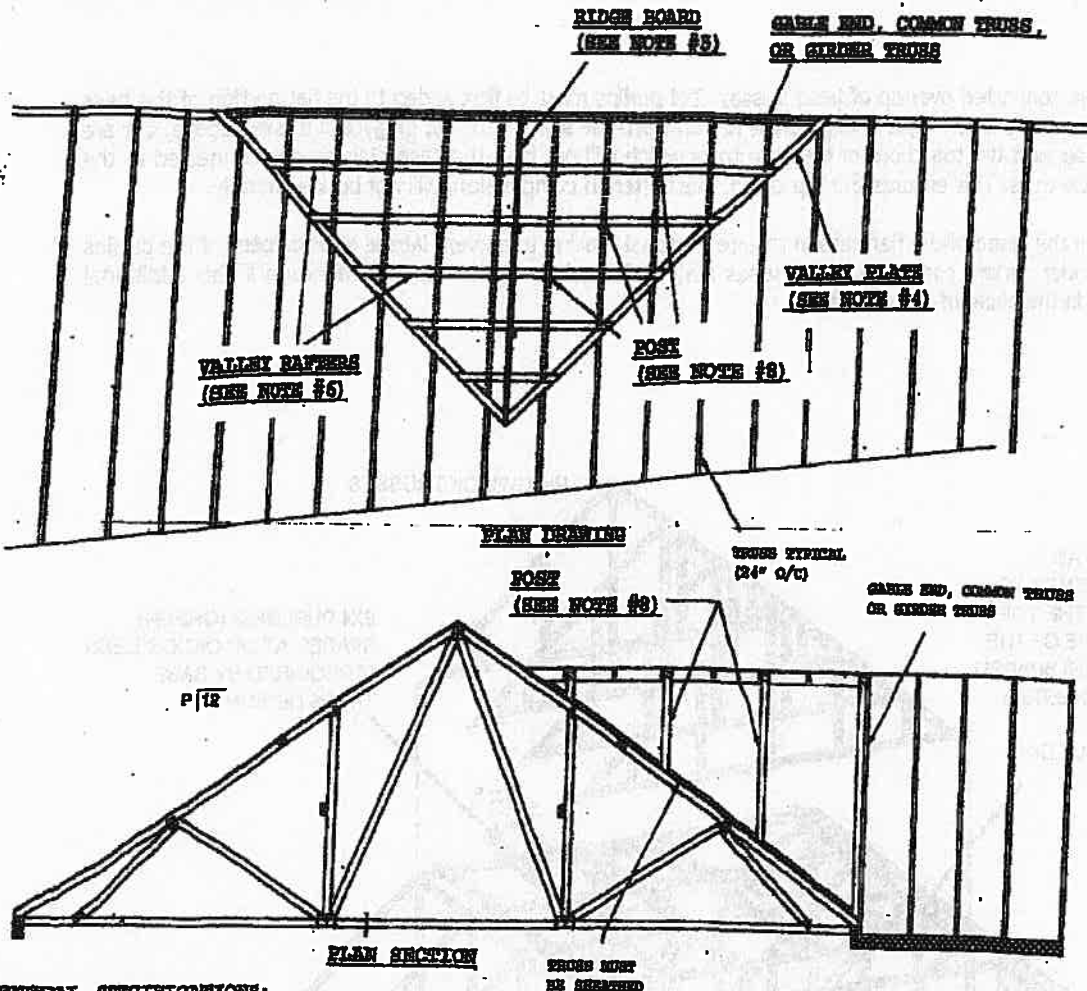


MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R.#1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE RAFTER THUSSES BRACKETED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (RAFT) THUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNER.
- (3) DEFINE VALLEY RIDGE BY HORIZING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOR-NAIIS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BOARD WITH (2) 16d (3.5" X 0.131") TOR-NAIIS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOR-NAIIS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH THUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING THUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) 2 X 4 #2 SPF OR BETTER PASTERED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LARGER ROOF DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

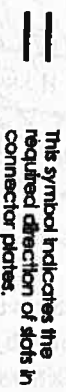
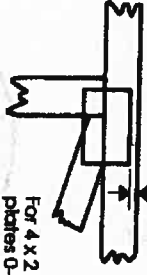
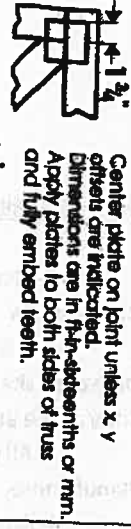
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 24.0 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) RACE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED RACE TRUSS COMPONENTS TO BE SEALD BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY NAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL RACE THUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in Mittek software or upon request.

PLATE SIZE

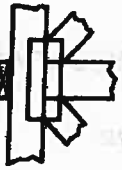
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



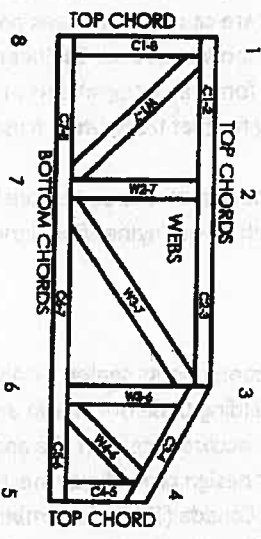
BEARING



Industry Standards:
TRC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in 1/8-in.-increments or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TRC.
7. Design assumes trusses will be suitably protected from the environment in accord with TRC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is finished, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TRC Quality Criteria.

MITTEK
POWER TO PERFORMANCE™
Mittek Engineering Reference Sheet: MIT-7472C rev. 10-08



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. Is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an Integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for Information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018