

Delivery Shiplist

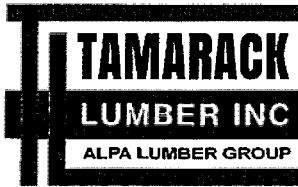
DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067	LAYOUT ID: 301360	LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO	SUB-BUILDER:	
MODEL: TH-7E	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T131 HIP GIRDER	10.00	21-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	203.62		
	2 Ply		0.00					00-00-00	00-05-08	128.00		
	1	T132 HIP	10.00 0.00	21-00-00	05-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	105.77 66.33		
	1	T133 HIP	10.00 0.00	21-00-00	06-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	109.41 69.33		
	1	T134 HIP	10.00 0.00	21-00-00	07-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	115.19 73.17		
	2	T136 HALF HIP	10.00 0.00	21-00-00	07-04-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	239.06 148.00		
	2	T137 HALF HIP	10.00 0.00	21-00-00	08-04-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	252.76 156.34		
	2	T138 HALF HIP	10.00 0.00	21-00-00	09-04-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	258.60 159.00		
	3	T139 HALF HIP	10.00 0.00	21-00-00	10-04-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	424.35 260.01		
	1	T140 HIP GIRDER	10.00	21-00-00	04-10-07	2 X 4	2 X 6	01-03-08	01-07-11	212.52		
	2 Ply		0.00					00-00-00	00-05-08	134.00		
	1	T141 HIP	10.00 0.00	21-00-00	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	111.88 70.50		
	1	T142 HIP	10.00 0.00	21-00-00	08-02-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	116.24 72.67		
	1	T143 HIP	10.00 0.00	21-00-00	09-10-07	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	124.06 78.33		
	2	T144 COMMON	10.00 0.00	21-00-00	10-07-03	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-05-08	239.88 148.00		
	1	T145 HIP GIRDER	10.00	10-06-00	04-10-07	2 X 4	2 X 4	01-03-08	01-07-11	50.89		
			0.00					01-03-08	01-07-11	32.83		
	1	T146 COMMON	10.00	10-06-00	08-00-03	2 X 4	2 X 4	01-03-08	03-07-11	54.75		
			0.00					01-03-08	03-07-11	34.67		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	7	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	106.61 67.69		
	3	J7 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	31.95 21.99		



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SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301360 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-7E ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	J8 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	25.11 18.00		

TOTAL # TRUSS= 47.00

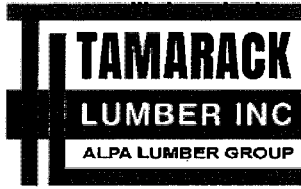
TOTAL BFT OF ALL TRUSSES=

1856.23 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2967.34 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

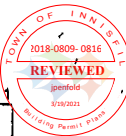
TOTAL # ITEMS= 2.00



Delivery Shiplist

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DATE	08/23/18
SALES REP	Mario



JOB TRACK: 42067 LAYOUT ID: 301361 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: TH-1 ELEVATION: B2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	10	T1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	2086.80 1263.30		
	2	G1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	435.38 267.00		
	1 4 Ply	T13 MONOPITCH	10.00 0.00	10-08-00	10-06-06	2 X 6 2 X 8	00-00-00 00-00-00	01-07-11 00-07-04	395.20 249.32		
	2	T14 MONOPITCH	10.00 0.00	10-08-00	10-06-06	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 00-03-08	116.62 74.66		
	1	G14 MONOPITCH	10.00 0.00	10-08-00	10-06-06	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 10-06-06	67.19 42.50		
	12	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4 2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	204.24 135.96		
	6	J50 JACK-OPEN	5.00 0.00	03-11-00	02-00-15	2 X 4 2 X 4	01-03-08 00-00-00	00-05-06 02-00-05	66.24 40.02		

TOTAL # TRUSS= 37.00

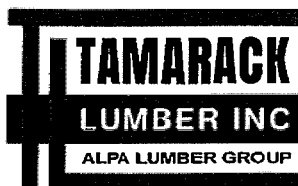
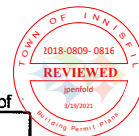
TOTAL BFT OF ALL TRUSSES=

2072.76 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3371.67 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

TOTAL # ITEMS= 5.00



Delivery Shiplist

DATE	08/23/18
SALES REP	Mario

JOB TRACK: 42067 LAYOUT ID: 301362 LOCATION: INNISFIL
BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
MODEL: TH-2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	10	T1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	2086.80 1263.30		
	2	G1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	435.38 267.00		
	1 4 Ply	T13 MONOPITCH	10.00 0.00	10-08-00	10-06-06	2 X 6	2 X 8	00-00-00 00-00-00	01-07-11 00-07-04	395.20 249.32		
	3	T14 MONOPITCH	10.00 0.00	10-08-00	10-06-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-03-08	174.93 111.99		
	1	G15 COMMON	10.00 0.00	11-02-00	06-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	54.26 37.00		
	12	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	204.24 135.96		
	6	J51 JACK-OPEN	5.00 0.00	05-07-00	02-09-05	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-11	90.00 55.98		

TOTAL # TRUSS= 38.00

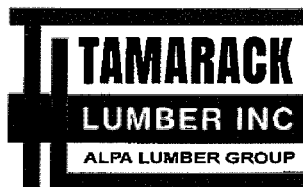
TOTAL BFT OF ALL TRUSSES=

2120.55 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3440.81 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	

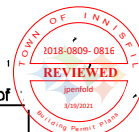
TOTAL # ITEMS= 5.00



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DATE	08/23/18
SALES REP	Mario

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JOB TRACK: 42067 LAYOUT ID: 301363 LOCATION: INNISFIL
 BUILDER: BAYVIEW WELLINGTON/ALCONA SHO SUB-BUILDER:
 MODEL: TH-3 ELEVATION: B W/ FIREWALL

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	G1 PIGGYBACK	6.00 0.00	40-08-00	09-04-03	2 X 4 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	217.69 133.50		
	1	T31A HIP	6.00 0.00	45-01-00	08-02-07	2 X 6 2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	264.47 159.33		
	6	T32 PIGGYBACK	6.00 0.00	45-06-00	09-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1723.98 1021.02		
	1	T32A HIP	6.00 0.00	45-01-00	09-10-07	2 X 6 2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	281.51 169.17		
	3	T33A PIGGYBACK	6.00 0.00	45-01-00	09-04-03	2 X 6 2 X 6	01-03-08 00-00-00	01-02-00 03-02-07	835.83 500.49		
	1 4 Ply	T35 COMMON	10.00 0.00	10-08-00	05-10-08	2 X 6 2 X 6	01-03-08 01-00-08	01-05-03 01-05-03	320.48 201.32		
	1	G36 COMMON	10.00 0.00	10-08-00	05-10-08	2 X 4 2 X 4	01-00-08 01-00-08	01-05-03 01-05-03	49.49 31.67		
	4	P1 PIGGYBACK	6.00 0.00	06-00-10	01-10-05	2 X 4 2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	68.08 45.32		
	1	P21 PIGGYBACK	6.00 0.00	08-09-10	01-06-08	2 X 4 2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	27.72 17.67		
	5	P22 PIGGYBACK	6.00 0.00	08-09-10	02-06-09	2 X 4 2 X 4	00-00-00 00-00-00	00-04-03 00-04-03	126.15 75.00		

TOTAL # TRUSS= 27.00

TOTAL BFT OF ALL TRUSSES=

2354.49 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3915.40 LBS.

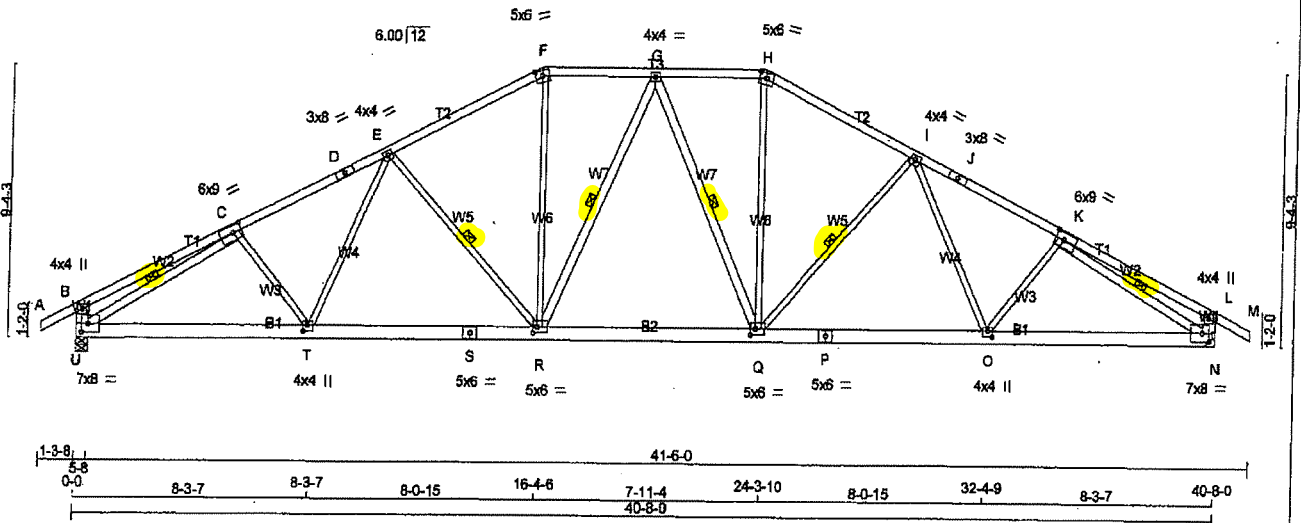
HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	HGUS26	
34	Hangers	TBE6	
4	Hangers	TS22	

TOTAL # ITEMS= 43.00



JOB NAME 297078	TRUSS NAME T1	QUANTITY 42	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 06:58:41 2018 Page 1					
ID: P5XmF1xy5318wCH47R2x3fzNvqf-zfUOMf_c0ipASGdF62hXeycw0Wkym7RuHZVSSkym?Z					
Scale = 1:71.8					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
U - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - G	2x4	DRY	No.2
G - C	2x4	DRY	No.2
U - Q	2x4	DRY	2100F 1.8E
K - N	2x4	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	4.0	4.0	
C TMWW-t	MT20	6.0	9.0	3.00 3.75
O TS-t	MT20	3.0	8.0	
E, G, I				
F TMWW-t	MT20	4.0	4.0	
H TTW-m	MT20	5.0	6.0	2.25 3.00
I TTW-m	MT20	5.0	6.0	2.25 3.00
J TS-t	MT20	3.0	8.0	
K TMWW-t	MT20	6.0	9.0	3.00 3.75
L TMV+p	MT20	4.0	4.0	
N BMWW-t	MT20	7.0	8.0	3.50 3.00
O BMWW-t	MT20	4.0	4.0	2.50 1.75
P BS-t	MT20	5.0	6.0	
Q BMWW-t	MT20	5.0	6.0	2.50 2.00
R BMWW-t	MT20	5.0	6.0	2.50 1.50
S BS-t	MT20	4.0	4.0	2.50 1.75
T BMWW-t	MT20	7.0	8.0	3.50 3.00
U BMWW-t	MT20	7.0	8.0	3.50 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
U	3595 0	3595 0	0	5-8	5-8
N	3595 0	3595 0	0	MECHANICAL	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

UNFACTORED REACTIONS

JT	1ST LG CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	2530	1858 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0
N	2630	1858 / 0	427 / 0	0 / 0	0 / 0	545 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)	
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 39	-129.7	-129.7 0.17 (1)	10.00	C-T	-95 / 144	0.03 (3)
B-C	0 / 28	-129.7	-129.7 0.47 (1)	10.00	T-E	0 / 468	0.11 (2)
C-D	-5144 / 0	-129.7	-129.7 0.88 (1)	2.43	E-R	-1025 / 0	0.48 (1)
D-E	-5144 / 0	-129.7	-129.7 0.88 (1)	2.43	R-F	0 / 1378	0.31 (1)
E-F	-4215 / 0	-129.7	-129.7 0.78 (1)	2.78	F-G	-332 / 0	0.16 (1)
F-G	-3758 / 0	-129.7	-129.7 0.40 (1)	3.31	G-Q	-332 / 0	0.16 (1)
G-H	-3758 / 0	-129.7	-129.7 0.40 (1)	3.31	Q-H	0 / 1378	0.31 (1)
H-I	-4215 / 0	-129.7	-129.7 0.78 (1)	2.78	H-I	-1025 / 0	0.48 (1)
I-J	-5144 / 0	-129.7	-129.7 0.88 (1)	2.43	I-Q	0 / 468	0.11 (2)
J-K	-5144 / 0	-129.7	-129.7 0.88 (1)	2.43	Q-K	-95 / 144	0.03 (3)
K-L	0 / 28	-129.7	-129.7 0.47 (1)	10.00	U-C	-5537 / 0	0.67 (1)
L-M	0 / 39	-129.7	-129.7 0.17 (1)	10.00	K-N	-5537 / 0	0.67 (1)
U-B	-468 / 0	0.0	0.0 0.03 (1)	7.81			
N-L	-468 / 0	0.0	0.0 0.03 (1)	7.81			
U-T	0 / 4661	-38.5	-38.5 0.72 (1)	10.00			
T-S	0 / 4426	-38.5	-38.5 0.65 (1)	10.00			
S-R	0 / 4426	-38.5	-38.5 0.65 (1)	10.00			
R-Q	0 / 3867	-38.5	-38.5 0.59 (1)	10.00			
Q-P	0 / 4426	-38.5	-38.5 0.65 (1)	10.00			
P-O	0 / 4426	-38.5	-38.5 0.65 (1)	10.00			
O-N	0 / 4661	-38.5	-38.5 0.72 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.38")
CALCULATED VERT. DEFL. (LL) = L/999 (0.28")
ALLOWABLE DEFL. (TL) = L/360 (1.38")
CALCULATED VERT. DEFL. (TL) = L/999 (0.42")

CSI: TC=0.88/1.00 (H-K:1), SC=0.72/1.00 (N-O:1), WB=0.87/1.00 (C-U:1), SS=0.30/1.00 (H-I:1)

DQL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

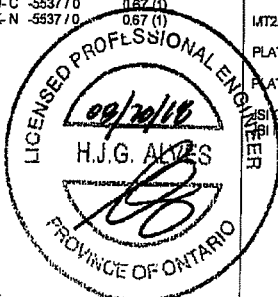
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN I MAX MIN I
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SLIP GRIP = 0.88 (R) (INPUT = 0.90)
SLIP METAL = 0.91 (C) (INPUT = 1.00)



DRWG NO. TAM 11005174
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME G1	QUANTITY 8	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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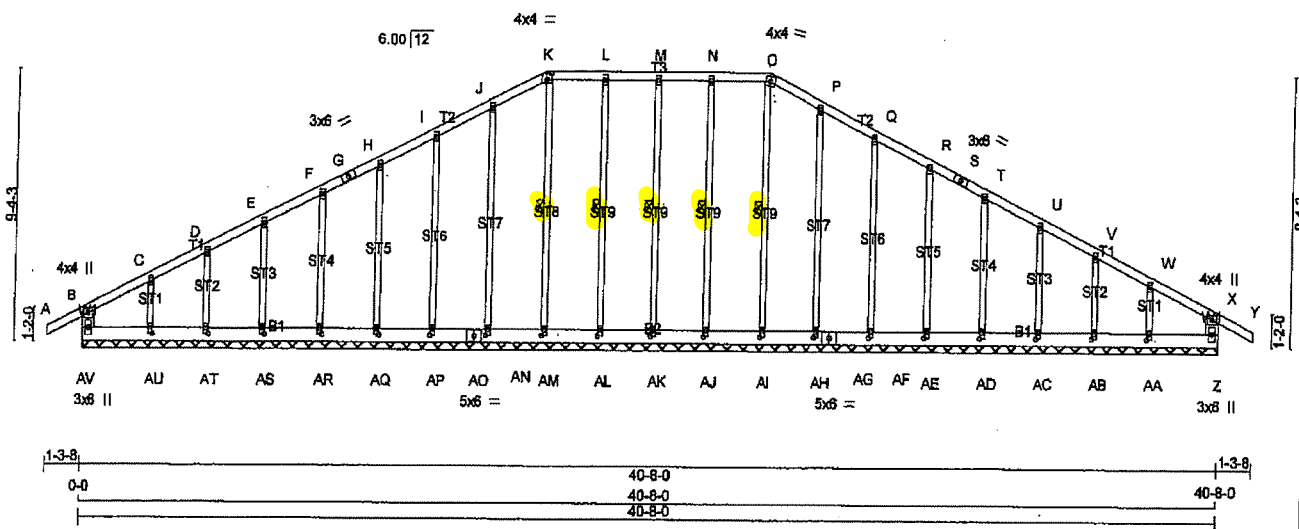
Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 09:46:49 2018 Page 1

ID: P5XmF1xy53l8wCH47R2x3fzNvqfpgTDxrn7hy6B1rBPOVWZno7QqVUEXBL5Yo1mS0ym?kq

1-3-8-0 16-4-6 16-4-6 7-11-4 24-3-10 16-4-6 40-8-0-11-8

Scale = 1:7.2



TOTAL WEIGHT = 8 X 218 = 1742 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
AV - B	2x6 DRY	No.2	SPF
A - G	2x4 DRY	No.2	SPF
G - K	2x4 DRY	No.2	SPF
K - O	2x4 DRY	No.2	SPF
O - S	2x4 DRY	No.2	SPF
S - Y	2x4 DRY	No.2	SPF
Z - X	2x6 DRY	No.2	SPF
AV - AO	2x6 DRY	No.2	SPF
AO - AG	2x6 DRY	No.2	SPF
AG - Z	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-AI, A-J, M-AK, L-AL, K-AM.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO			
AV-B	-367 / 0	0.0 0.0 0.02 (1)	7.81	AI-O	-253 / 0	0.14 (1)	
A-B	0 / 39	-129.7 -129.7 0.17 (1)	10.00	AI-N	-259 / 0	0.14 (1)	
B-C	-68 / 0	-129.7 -129.7 0.09 (1)	8.25	AK-M	-235 / 0	0.13 (1)	
C-D	-54 / 0	-129.7 -129.7 0.07 (1)	6.25	AL-L	-259 / 0	0.14 (1)	
D-E	-45 / 0	-129.7 -129.7 0.08 (1)	8.25	AM-K	-253 / 0	0.14 (1)	
E-F	-41 / 0	-129.7 -129.7 0.08 (1)	8.25	AN-J	-253 / 0	0.34 (1)	
F-G	-37 / 0	-129.7 -129.7 0.08 (1)	8.25	AP-I	-259 / 0	0.24 (1)	
G-H	-37 / 0	-129.7 -129.7 0.08 (1)	6.25	AQ-H	-258 / 0	0.16 (1)	
H-I	-34 / 0	-129.7 -129.7 0.06 (1)	6.25	AR-F	-257 / 0	0.11 (1)	
I-J	-32 / 0	-129.7 -129.7 0.06 (1)	6.25	AS-E	-259 / 0	0.07 (1)	
J-K	-28 / 0	-129.7 -129.7 0.05 (1)	6.25	AT-D	-250 / 0	0.05 (1)	
K-L	-23 / 0	-129.7 -129.7 0.06 (1)	6.25	AU-C	-279 / 0	0.04 (1)	
L-M	-23 / 0	-129.7 -129.7 0.08 (1)	8.25	AV-P	-253 / 0	0.34 (1)	
M-N	-23 / 0	-129.7 -129.7 0.06 (1)	6.25	AF-Q	-259 / 0	0.24 (1)	
N-O	-23 / 0	-129.7 -129.7 0.05 (1)	6.25	AE-R	-258 / 0	0.16 (1)	
O-P	-28 / 0	-129.7 -129.7 0.06 (1)	8.25	AD-T	-257 / 0	0.11 (1)	
P-Q	-32 / 0	-129.7 -129.7 0.05 (1)	6.25	AC-U	-259 / 0	0.07 (1)	
Q-R	-34 / 0	-129.7 -129.7 0.06 (1)	6.25	AB-V	-260 / 0	0.05 (1)	
R-S	-37 / 0	-129.7 -129.7 0.08 (1)	8.25	AA-W	-279 / 0	0.04 (1)	
S-T	-37 / 0	-129.7 -129.7 0.05 (1)	6.25				
T-U	-41 / 0	-129.7 -129.7 0.06 (1)	6.25				
U-V	-45 / 0	-129.7 -129.7 0.06 (1)	6.25				
V-W	-54 / 0	-129.7 -129.7 0.07 (1)	6.25				
W-X	-68 / 0	-129.7 -129.7 0.09 (1)	8.25				
X-Y	0 / 39	-129.7 -129.7 0.17 (1)	10.00				
Z-X	-367 / 0	0.0 0.0 0.02 (1)	7.81				
AV-AU	0 / 56	-38.5 -38.5 0.02 (1)	10.00				
AU-AT	0 / 46	-38.5 -38.5 0.02 (2)	10.00				
AT-AS	0 / 41	-38.5 -38.5 0.01 (2)	10.00				
AS-AR	0 / 37	-38.5 -38.5 0.01 (2)	10.00				
AR-AQ	0 / 33	-38.5 -38.5 0.01 (2)	10.00				
AQ-AP	0 / 31	-38.5 -38.5 0.01 (2)	10.00				
AP-AO	0 / 28	-38.5 -38.5 0.01 (2)	10.00				
AO-AN	0 / 28	-38.5 -38.5 0.01 (2)	10.00				
AN-AM	0 / 25	-38.5 -38.5 0.01 (2)	10.00				
AM-AL	0 / 23	-38.5 -38.5 0.01 (2)	10.00				
AL-AK	0 / 23	-38.5 -38.5 0.01 (2)	10.00				
AK-AJ	0 / 23	-38.5 -38.5 0.01 (2)	10.00				
AJ-AI	0 / 23	-38.5 -38.5 0.01 (2)	10.00				
AI-AH	0 / 28	-38.5 -38.5 0.01 (2)	10.00				
AH-AG	0 / 28	-38.5 -38.5 0.01 (2)	10.00				
AG-AF	0 / 28	-38.5 -38.5 0.01 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 36.2 PSF
DL = 5.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 81.7 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (X-Y:1), BC=0.02/1.00 (Z-AA:1)
WB=0.34/1.00 (J-AN:1), SS=0.12/1.00 (X-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	818	354	1867
	818	354	1867

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

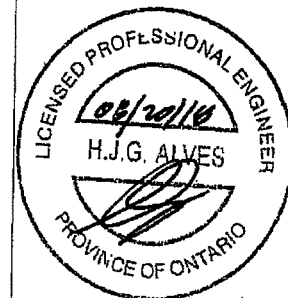
JSI GRIP= 0.60 (O) (INPUT= 0.90)
JSI METAL= 0.12 (W) (INPUT= 1.00)

DWG NO. TAM 71805165
STRUCTURAL
COMPONENT ONLY 1/2



JOB NAME 297078	TRUSS NAME G1	QUANTITY 8	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 09:48:49 2018 Page 2 ID:P5XmF1xy53l8wCH47R2x3fzNvgf-pgTDxtN?hy6B1rBPOVwZno7QqVUEXBL5Yo1mS0vm?ko					

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX
FR-TO		FROM TO			FR-TO				
AF-AE	0 / 31	-38.5	-38.5	0.01 (2)	10.00				
AE-AD	0 / 33	-38.5	-38.5	0.01 (2)	10.00				
AD-AC	0 / 37	-38.5	-38.5	0.01 (2)	10.00				
AC-AB	0 / 41	-38.5	-38.5	0.01 (2)	10.00				
AB-AA	0 / 46	-38.5	-38.5	0.02 (2)	10.00				
AA-Z	0 / 56	-38.5	-38.5	0.02 (1)	10.00				



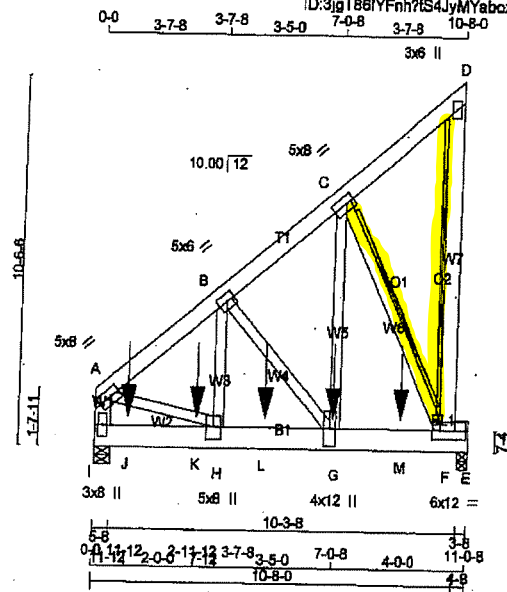
DWG NO. TAM **71805165**
STRUCTURAL
COMPONENT ONLY **2/2**



JOB NAME 297080	TRUSS NAME T13	QUANTITY 4	PLY 4	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 16 2018 Mitek Industries, Inc. Mon Aug 20 12:58:25 2018 Page 1
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TOTAL WEIGHT = 16 X 89 = 1581 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	DESCR
A - D	2x6	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
I - A	2x6	DRY	No.2		SPF
I - E	2x6	DRY	No.2		SPF

BEARING BLOCKS					
BL1	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT					
	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS		
A-D 2	12	TOP
D-F 2	12	TOP
I-A 2	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS		
I-E 3	4	SIDE (1548.0)
WEBS: (0.122'X3') SPIRAL NAILS		
2x4 1	8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS. BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.25
B	TMVW-t	MT20	5.0	8.0	2.50	2.75
C	TMVW-t	MT20	5.0	8.0	2.50	2.75
D	TMV-p	MT20	3.0	8.0		
F	BMVW-t	MT20	8.0	12.0	3.00	2.75
G	BMVW-t	MT20	4.0	12.0		
H	BMVW-t	MT20	5.0	8.0	4.25	2.50
I	BMV-t	MT20	3.0	8.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	10672	0	10672	0	5-8	5-8
E	8920	0	8920	0	3-8	3-8

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	7791	4984 / 0	1228 / 0	0 / 0	0 / 0	1598 / 0	0 / 0
E	6514	4141 / 0	1034 / 0	0 / 0	0 / 0	1339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT D-F, C-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FACTORED VERT. LOAD (LBS)	MAX. FACTORED (LC1) (LBS)	MAX. FACTORED (LC1) (LBS)	MAX. FACTORED (LC1) (LBS)
FR-TO		FROM	TO	FR-TO	TO
A-B	-8748 / 0	-129.7	-129.7	0.11 (1)	5.21
B-C	-5488 / 0	-129.7	-129.7	0.07 (1)	8.25
C-D	-32 / 0	-129.7	-129.7	0.04 (1)	6.25
D-F	-180 / 0	0.0	0.0	0.01 (1)	7.81
I-A	-7795 / 0	0.0	0.0	0.14 (1)	6.92
I-J	0 / 0	-38.5	-38.5	0.37 (1)	10.00
J-K	0 / 0	-38.5	-38.5	0.37 (1)	10.00
K-H	0 / 0	-38.5	-38.5	0.37 (1)	10.00
H-L	0 / 6684	-38.5	-38.5	0.39 (1)	10.00
L-G	0 / 6684	-38.5	-38.5	0.39 (1)	10.00
G-M	0 / 4269	-38.5	-38.5	0.82 (1)	10.00
M-F	0 / 4269	-38.5	-38.5	0.82 (1)	10.00
F-E	0 / 0	-38.5	-38.5	0.51 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)						
	LOC.	LC1	MAX- MAX+	FACE	DIR.	TYPE
	8-11-12	-2595	-2595	—	BACK	VERT TOTAL
	11-12	-2595	-2595	—	BACK	VERT TOTAL
	2-11-12	-2595	-2595	—	BACK	VERT TOTAL
	4-11-12	-2595	-2595	—	BACK	VERT TOTAL
	8-11-12	-2595	-2595	—	BACK	VERT TOTAL

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.14/1.00 (A-1), BC=0.82/1.00 (F-G-1), WB=0.47/1.00 (C-G-1), SSI=0.85/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LBS SEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MIN (PL)
MT20	818	354	1687
	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (C) (INPUT = 0.90)
JSI METAL=0.80 (H) (INPUT = 1.00)

DWG NO. TAM 11805262
STRUCTURAL
COMPONENT ONLY 1/2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297080	T13	4	4	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 8.210 S May 18 2018 MITek Industries, Inc. Mon Aug 20 12:58:25 2018 Page 2 ID:3lgT86iYFnh7cS4JvMYabozN43t-IEpVh2mdDM4g5spph?HmHVNEv8MAaXSgkklv4yzd					

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 3557.1 lbs FACTORED DOWN AT 11-12,
3558.7 lbs FACTORED DOWN AT 2-11-12, 3556.7
lbs FACTORED DOWN AT 4-11-12, AND 3558.7
lbs FACTORED DOWN AT 8-11-12, AND 3558.7
lbs FACTORED DOWN AT 8-11-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

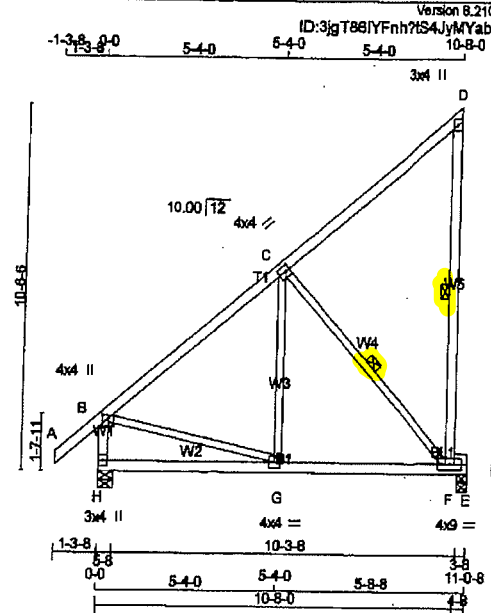


DWG NO. TAM 71805262
STRUCTURAL
COMPONENT ONLY 3/2



JOB NAME 297080	TRUSS NAME T14	QUANTITY 10	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Trusses, Burlington



Scale = 1:80.3

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ID:3jgT86IYFnh7tS4JyMYabozNu3tJwh8RUDYKX74VEwTedFZPjzmZuEdX1bgr7UEqlyys

TOTAL WEIGHT = 10 X 58 = 583 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
F - B	2x4	DRY	No.2
H - E	2x4	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1107	0	1107	0	0	5-8	5-8
E	881	0	881	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
COMBINED		SNOW		LIVE		PERM.LIVE		WIND	
H	803	527/0	118/0	0/0	0/0	180/0	0/0	0/0	0/0
E	649	394/0	118/0	0/0	0/0	139/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

TOTAL LOAD CASES: (4)									
CHDRDS					WBS				
MEMB.		MAX. FACTORED	FACTORED		MEMB.		MAX. FACTORED	MAX. FACTORED	
		FORCE	VERT. LOAD	LC1 MAX.			FORCE	MAX. CS1 (LC)	
		(LBS)	(PLF)	CS1 (LC)			(LBS)		
FR-TO			FROM TO		FR-TO				
A-B	0/57		-128.7 -128.7	0.18 (1)	10.00	G-C	0/318	0.08 (3)	
B-C	-880/0		-128.7 -128.7	0.87 (1)	5.91	C-F	-848/0	0.34 (1)	
C-D	-50/0		-128.7 -128.7	0.82 (1)	8.25	B-G	0/581	0.13 (1)	
F-D	-284/0		0.0 0.0	0.14 (1)	8.25				
H-B	-1048/0		0.0 0.0	0.11 (1)	7.68				
H-G	0/0		-38.5 -38.5	0.34 (2)	10.00				
G-F	0/584		-38.5 -38.5	0.71 (1)	10.00				
F-E	0/0		-38.5 -38.5	0.28 (1)	10.00				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.25
C	TMVW+p	MT20	4.0	4.0	2.00 1.25
D	TMVW+p	MT20	3.0	4.0	
F	BMVW+k	MT20	4.0	9.0	
G	BMVW+k	MT20	4.0	4.0	
H	BMV1+p	MT20	3.0	4.0	

SPECIFIED LOADS:	
TOP CH.	LL = 38.2 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 61.7 PSF

SPACING = 24.0 IN/OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/380 (0.37")
CALCULATED VERT. DEFL.(TL) = L/821 (0.21")

CS1: TC=0.97/1.00 (B-G:1), BC=0.71/1.00 (F-G:1), WB=0.34/1.00 (C-F:1), SB=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

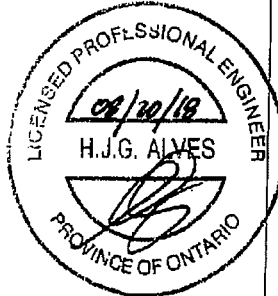
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.80)
JSI METAL= 0.25 (B) (INPUT = 1.00)



DWG NO. TAM 11805263
STRUCTURAL
COMPONENT ONLY

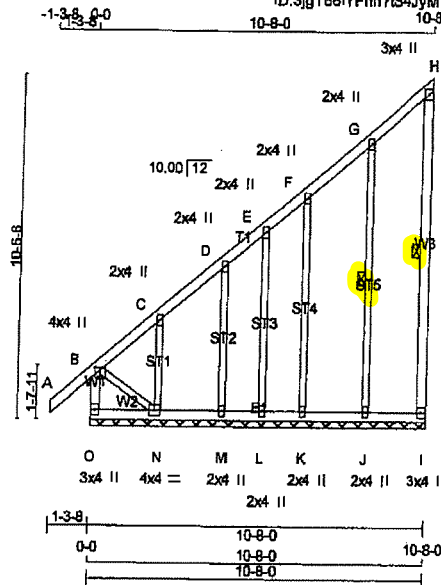


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
297080	G14	2	1			

Tamarack Roof Truss, Burlington

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Scale: 3/16"=1'

TOTAL WEIGHT = 2 X 67 = 134 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - B	2x4	DRY	No.2
A - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G					
C TMW+w	MT20	2.0	4.0		
H TMW+p	MT20	3.0	4.0		
I BMV1+p	MT20	3.0	4.0		
J, K, L, M					
J BMV1+w	MT20	2.0	4.0		
N BMWV1-t	MT20	4.0	4.0		
O BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
O-B	-316/0	0.0	0.03 (1)	J-G	-299/0	0.15 (1)	
A-B	0/57	-129.7	-129.7 0.18 (1)	K-F	-209/0	0.19 (1)	
B-C	-10/0	-129.7	-129.7 0.08 (1)	L-E	-141/0	0.08 (1)	
C-D	-28/0	-129.7	-129.7 0.08 (1)	M-D	-202/0	0.08 (1)	
D-E	-17/0	-129.7	-129.7 0.04 (1)	N-C	-299/0	0.08 (1)	
E-F	0/0	-129.7	-129.7 0.04 (1)	B-N	0/25	0.01 (1)	
F-G	0/3	-129.7	-129.7 0.07 (1)				
G-H	-18/0	-129.7	-129.7 0.07 (1)				
I-H	-110/0	0.0	0.0 0.06 (1)				
O-N	0/0	-38.5	-38.5 0.03 (3)				
N-M	0/13	-38.5	-38.5 0.03 (3)				
M-L	0/8	-38.5	-38.5 0.02 (2)				
L-K	0/8	-38.5	-38.5 0.02 (3)				
K-J	0/3	-38.5	-38.5 0.03 (3)				
J-I	0/0	-38.5	-38.5 0.03 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF \$4.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.18/1.00 (A-B:1), BC=0.03/1.00 (M-N:3), WB=0.19/1.00 (F-K:1), SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

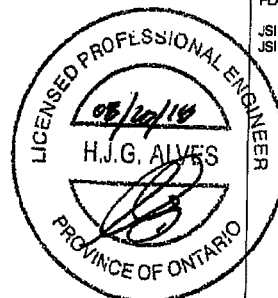
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667	788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)



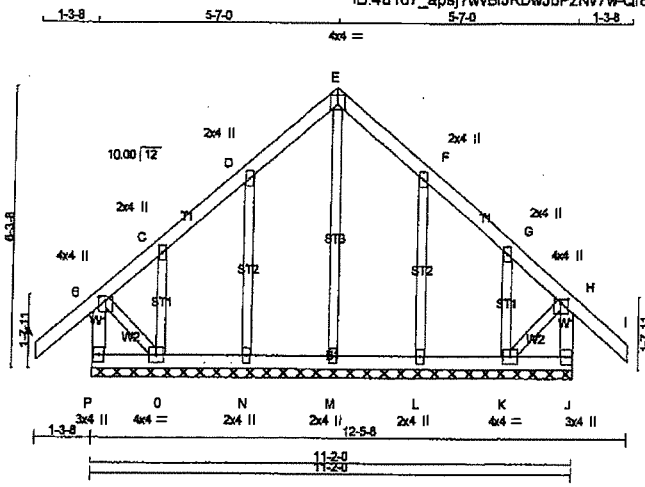
DRWG NO. TAM 11805261
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297079	G15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 54 = 108 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
GABLE STUDS SPACED AT 2'-0" O.C.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVV+p	MT20	4.0	4.0	1.00 2.00
C, D, F, G					
C	TMV+w	MT20	2.0	4.0	
E	TTV+p	MT20	4.0	4.0	1.50 2.00
H	TMV+p	MT20	4.0	4.0	1.00 2.00
J	BMV1+p	MT20	3.0	4.0	
K	BMVV1-t	MT20	4.0	4.0	
L, M, N					
L	BMV1+w	MT20	2.0	4.0	
O	BMVV1-t	MT20	4.0	4.0	
P	BMV1+p	MT20	3.0	4.0	

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-389 / 0	0.0 0.0 0.04 (1)	7.81	M-E	-194 / 0	0.12 (1)	
A-B	0 / 57	-129.7 -129.7 0.18 (1)	10.00	N-D	-314 / 0	0.10 (1)	
B-C	-78 / 0	-129.7 -129.7 0.17 (1)	8.25	C-C	-122 / 0	0.02 (1)	
C-D	-8 / 0	-129.7 -129.7 0.08 (1)	10.00	L-F	-314 / 0	0.10 (1)	
D-E	-33 / 0	-129.7 -129.7 0.08 (1)	8.25	K-G	-122 / 0	0.02 (1)	
E-F	-33 / 0	-129.7 -129.7 0.08 (1)	8.25	B-O	0 / 29	0.01 (1)	
F-G	-8 / 0	-129.7 -129.7 0.08 (1)	10.00	K-H	0 / 29	0.01 (1)	
G-H	-78 / 0	-129.7 -129.7 0.17 (1)	8.25				
H-I	0 / 57	-129.7 -129.7 0.18 (1)	10.00				
J-H	-389 / 0	0.0 0.0 0.04 (1)	7.81				
P-O	0 / 0	-38.5 -38.5 0.02 (3)	10.00				
O-N	0 / 18	-38.5 -38.5 0.02 (3)	10.00				
N-M	0 / 11	-38.5 -38.5 0.02 (3)	10.00				
M-L	0 / 11	-38.5 -38.5 0.02 (3)	10.00				
L-K	0 / 18	-38.5 -38.5 0.02 (3)	10.00				
K-J	0 / 0	-38.5 -38.5 0.02 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1), BC=0.02/1.00 (K-L:3), WB=0.12/1.00 (E-M:1), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354 1687 788	1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



DRWG NO. TAM 77505845
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T31A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

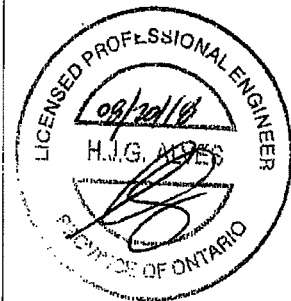
Version 8.210 S May 18 2018 MTEK Industries, Inc. Mon Aug 20 09:47:43 2018 Page 2
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LOADING
TOTAL LOAD CASES: (18)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRAC LENGTH
R-Q	-993 / 5995	-38.5	-38.5	0.34 (3)	8.25				
Q-P	-993 / 5995	-38.5	-38.5	0.34 (3)	8.25				
P-O	-1231 / 6681	-38.5	-38.5	0.38 (3)	6.25				
O-N	-1240 / 6218	-38.5	-38.5	0.37 (3)	8.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.8.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

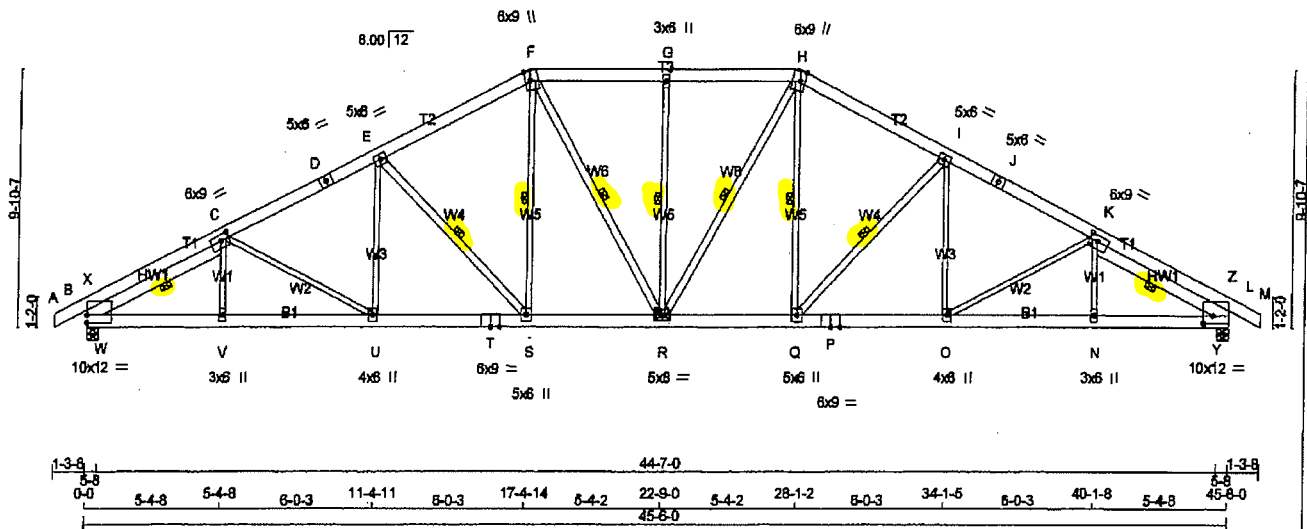


DWG NO. TAM 11805179
STRUCTURAL
COMPONENT ONLY 2/2



JOB NAME 297078	TRUSS NAME T32	QUANTITY 10	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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1-3-8 0-0 5-4-8 5-4-8 6-0-3 11-4-11 8-0-3 17-4-14 5-4-2 22-9-0 5-4-2 28-1-2 6-0-3 34-1-5 6-0-3 40-1-8 5-4-8 45-8-0
Scale = 1:80.1



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No. 2
D - F	2x6	DRY	No. 2
F - H	2x6	DRY	No. 2
H - J	2x6	DRY	No. 2
J - M	2x6	DRY	No. 2
B - T	2x6	DRY	2100F 1.8E
T - P	2x6	DRY	2100F 1.8E
P - L	2x6	DRY	2100F 1.8E

REINFORCING MEMBERS			
HW1	2x6	DRY	No. 2
HW2	2x6	DRY	No. 2

ALL WEBS EXCEPT			
E - S	2x4	DRY	No. 2
F - R	2x4	DRY	No. 2
R - H	2x4	DRY	No. 2
Q - I	2x4	DRY	No. 2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBWW1-I	MT20	10.0	12.0	3.50
C	TMBWW1-I	MT20	8.0	9.0	2.75 4.00
D	TS-I	MT20	6.0	6.0	
E	TMBWW1-I	MT20	5.0	6.0	
F	TTWW1-M	MT20	8.0	9.0	Edge 2.25
G	TMBWW1-I	MT20	3.0	6.0	
H	TTWW1-M	MT20	6.0	9.0	Edge 2.25
I	TMBWW1-I	MT20	5.0	6.0	
J	TS-I	MT20	5.0	6.0	
K	TMBWW1-I	MT20	8.0	9.0	2.75 4.00
L	TMBWW1-I	MT20	10.0	12.0	3.50 4.50
N	BMWW1-I	MT20	3.0	6.0	
O	BMWW1-I	MT20	4.0	8.0	
P	BS-I	MT20	6.0	9.0	
Q	BMWW1-I	MT20	5.0	8.0	
R	BMWW1-I	MT20	5.0	8.0	
S	BMWW1-I	MT20	5.0	6.0	
T	BS-I	MT20	6.0	9.0	
U	BMWW1-I	MT20	4.0	6.0	
V	BMWW1-I	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE

TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	DOWN	HORIZ
L	4990	0	5151
B	4990	0	5151

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1066 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 263 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS					
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS			
B	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	3587	2808 / 0	478 / 0	0 / 0	172 / -1152
B	3587	2808 / 0	478 / 0	0 / 0	82 / -1152

HORIZONTAL REACTIONS

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) B, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W, K-Y.

LOADING

TOTAL LOAD CASES: (18)

CHORDS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)
FR-TO			
A-B	0 / 2	-170.4	-170.4 0.13 (2)
B-X	-5735 / 1091	-170.4	-170.4 0.48 (2)
X-C	-3413 / 835	-170.4	-170.4 0.40 (2)
C-D	-7208 / 1812	-170.4	-170.4 0.88 (2)
D-E	-7208 / 1812	-170.4	-170.4 0.88 (2)
E-F	-8122 / 1461	-170.4	-170.4 0.77 (2)
F-G	-5806 / 1425	-170.4	-170.4 0.57 (2)
G-H	-5806 / 1425	-170.4	-170.4 0.57 (2)
H-I	-6122 / 1461	-170.4	-170.4 0.77 (3)
I-J	-7208 / 1812	-170.4	-170.4 0.80 (3)
J-K	-7208 / 1812	-170.4	-170.4 0.89 (3)
K-Z	-3413 / 834	-170.4	-170.4 0.40 (3)
Z-L	-5735 / 1091	-170.4	-170.4 0.48 (3)
L-M	0 / 2	-170.4	-170.4 0.13 (3)

WEBS			
MEMB.	FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH
FR-TO			
V-C	0 / 257	0.08 (17)	
C-U	-509 / 313	0.44 (2)	
U-E	-46 / 509	0.11 (5)	
E-S	-1807 / 555	0.87 (2)	
S-F	-330 / 1554	0.34 (2)	
F-R	-327 / 1554	0.25 (3)	
R-Q	-1321 / 1386	0.78 (2)	
Q-H	-328 / 1554	0.25 (2)	
H-C	-330 / 1554	0.34 (3)	
C-I	-1807 / 555	0.87 (3)	
I-O	-42 / 509	0.11 (5)	
O-K	-509 / 305	0.44 (5)	
K-N	0 / 257	0.08 (17)	
N-W	-367 / 2828	0.00 (1)	
W-X	-4802 / 850	0.58 (1)	
X-Y	-4803 / 853	0.58 (1)	
Y-Z	-367 / 2828	0.00 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 75.3 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2012, CBC 2018
- CSA 088-09, CSA 088-14
- TRC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.52")
CALCULATED VERT. DEFL. (LL) = L/999 (0.27")
ALLOWABLE DEFL. (TL) = L/180 (3.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.33")

CSI: TC=0.89/1.00 (H-C3), BC=0.39/1.00 (N-C3), WB=0.78/0.90 (G-R2), SSI=0.40/1.00 (G-H2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLU) (PLU)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1658

PLATE ROTATION TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.85 (C) (INPUT = 0.90)
JSI METAL=0.95 (T) (INPUT = 1.00)

DWG NO. TAM 11805180
STRUCTURAL
COMPONENT ONLY

1/2

CONTINUED ON PAGE 2



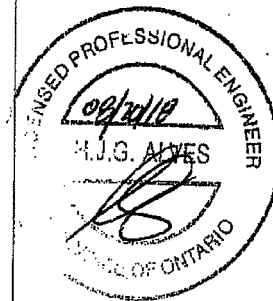
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 09:47:47 2018 Page 2
ID: P5XmF1xy5318vCH47R2x3fzNvqf-6qz86p4O1mQz_idYoiy571W9vuB1XJpduo4iVeym7W

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE.

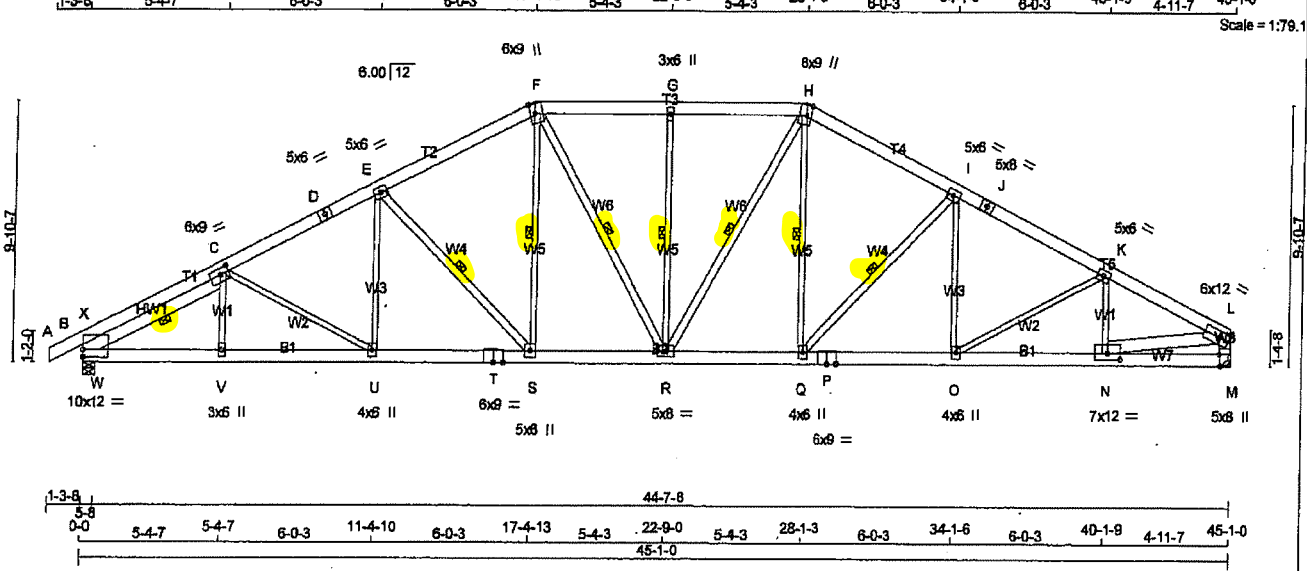


DWG NO. TAM 71805780
STRUCTURAL
COMPONENT ONLY 2/2



JOB NAME 297078	TRUSS NAME T32A	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.210 S May 18 2018 MTek Industries, Inc. Mon Aug 20 09:47:51 2018 Page 1
ID: P5XmF1xy5318wCH47R2x3fzNvqf-7bCh27u57wOTNwK1Y08IhrzVY7T7oDpR22eQym7je



TOTAL WEIGHT = 2 X 282 = 563 lb

LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
M - L	2x6	DRY	No.2
B - T	2x6	DRY	2100F 1.8E
T - P	2x6	DRY	2100F 1.8E
P - M	2x6	DRY	2100F 1.8E
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
ALL WEBS			
EXCEPT	2x4	DRY	No.2
E - S	2x4	DRY	No.2
F - R	2x4	DRY	No.2
R - H	2x4	DRY	No.2
Q - I	2x4	DRY	No.2
N - L	2x6	DRY	No.2

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMBW1-H	MT20	10.0	12.0	3.50
C TMBWW1-H	MT20	6.0	9.0	2.75 4.00
D TS-t	MT20	5.0	6.0	
E, I, K				
E TMBWW-t	MT20	5.0	8.0	
F TTWW+m	MT20	6.0	9.0	Edge 2.25
G TMBW-w	MT20	3.0	8.0	
H TTWW+m	MT20	6.0	9.0	Edge 2.25
J TS-t	MT20	5.0	6.0	
L TMBW-t	MT20	6.0	12.0	Edge
M BMV1-H	MT20	5.0	8.0	Edge 0.50
N BMVWW-t	MT20	7.0	12.0	2.75 8.00
O, Q, U				
O BMVWW-t	MT20	4.0	8.0	
P BS-t	MT20	6.0	9.0	
R BMVWW-t	MT20	5.0	8.0	
S BMVWW-t	MT20	5.0	6.0	
T BS-t	MT20	6.0	9.0	
V BMVW-w	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		DOWN		BRG		BRG	
JT	VERT	4946	0	5125	266	-1059	5-8		
B	HORIZ	4709	0	4812	0	-992	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1059 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 992 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDING DESIGNER

PROVIDE FOR 288 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
B	3556	2598 / 0	473 / 0	0 / 0	171 / -1144
M	3395	2404 / 0	473 / 0	0 / 0	98 / -1085

HORIZONTAL REACTIONS		DEAD		SOIL	
B	0 / 0	0 / 0	0 / 0	180 / -168	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q, C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 2	-170.4 -170.4	0.13 (2)	10.00	V-C	0 / 258	0.08 (17)	
B-X	-5704 / 1083	-170.4 -170.4	0.48 (2)	3.25	C-U	-514 / 304	0.45 (2)	
X-C	-3395 / 1830	-170.4 -170.4	0.40 (2)	4.24	U-E	-41 / 511	0.11 (5)	
C-D	-7122 / 1598	-170.4 -170.4	0.89 (2)	2.48	E-S	-1899 / 555	0.87 (2)	
D-E	-7122 / 1598	-170.4 -170.4	0.89 (2)	2.48	S-F	-330 / 1528	0.34 (8)	
E-F	-6033 / 1446	-170.4 -170.4	0.77 (2)	2.92	F-R	-317 / 1514	0.24 (3)	
F-G	-5702 / 1408	-170.4 -170.4	0.57 (2)	3.21	R-G	-1323 / 386	0.78 (3)	
G-H	-5702 / 1408	-170.4 -170.4	0.57 (2)	3.21	R-H	-338 / 1594	0.28 (2)	
H-I	-5978 / 1429	-170.4 -170.4	0.74 (3)	2.98	Q-H	-308 / 1413	0.32 (3)	
I-J	-6953 / 1557	-170.4 -170.4	0.85 (3)	2.58	Q-I	-1649 / 521	0.81 (3)	
J-K	-6953 / 1557	-170.4 -170.4	0.85 (3)	2.58	O-I	-119 / 398	0.09 (6)	
K-L	-7019 / 1485	-170.4 -170.4	0.60 (3)	2.87	O-K	-218 / 480	0.19 (3)	
M-L	-4704 / 1008	0.0	0.0	0.30 (3)	N-K	-971 / 310	0.20 (3)	
					N-L	-1255 / 8414	0.75 (3)	
B-W	-852 / 3024	-38.5	-38.5	0.16 (2)	6.25	W-X	-382 / 2797	0.00 (1)
W-V	-1547 / 8770	-38.5	-38.5	0.37 (2)	6.25	W-C	-4551 / 842	0.58 (1)
V-U	-1548 / 8767	-38.5	-38.5	0.39 (2)	8.25			
U-T	-1349 / 6395	-38.5	-38.5	0.34 (1)	8.25			
T-S	-1349 / 6395	-38.5	-38.5	0.34 (1)	8.25			
S-R	-968 / 5384	-38.5	-38.5	0.28 (1)	8.25			
R-Q	-824 / 5315	-38.5	-38.5	0.28 (1)	8.25			

DWG NO. TAM
STRUCT
COMPONEN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 75.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL) = L/180 (3.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.89/1.00 (C-E2), BC=0.39/1.00 (U-V2), WS=0.78/0.90 (G-R3), SS=0.40/1.00 (G-H3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	518	354	1657

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.94 (T) (INPUT = 1.00)

DWG NO. TAM 11005781
STRUCTURAL
COMPONENT ONLY

1/2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T32A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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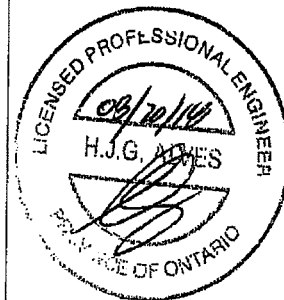
LOADING

TOTAL LOAD CASES: (18)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	
FR-TO		FRDM TO			FR-TO				
Q-P	-1072 / 6238	-38.5	-38.5	0.34 (1)	8.25				
P-O	-1072 / 6238	-38.5	-38.5	0.34 (1)	6.25				
O-N	-1217 / 8313	-38.5	-38.5	0.37 (3)	6.25				
N-M	-10 / 20	-38.5	-38.5	0.06 (3)	10.00				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBC 2.4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (40.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p/C_g , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



DWG NO. TAM 1805181
STRUCTURAL
COMPONENT ONLY 3/2

CONTINUED ON PAGE 2



JOB NAME 297078	TRUSS NAME T33A	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.210 S May 18 2018 MITek Industries, Inc. Mon Aug 20 08:47:58 2018 Page 2
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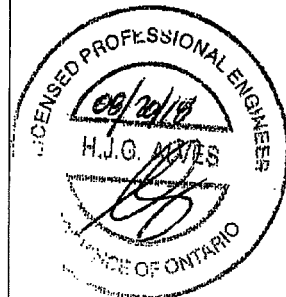
LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	FORCE	FACTOR	VERT. LOAD	MEMB.	FORCE	FACTOR	VERT. LOAD
(LBS)				(LBS)			
FR-TO				FR-TO			
R-Q	-1740 / 8886	-38.5	-38.5 0.48 (1)	8.25			
Q-P	-2383 / 11838	-38.5	-38.5 0.72 (3)	8.25			
P-O	-1398 / 7039	-38.5	-38.5 0.49 (3)	8.25			
O-N	-22 / 58	-38.5	-38.5 0.04 (5)	8.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.8.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



DWG NO. TAM **T1805182**
STRUCTURAL
COMPONENT ONLY **7/2**



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T35	2	4	TRUSS DESC.	

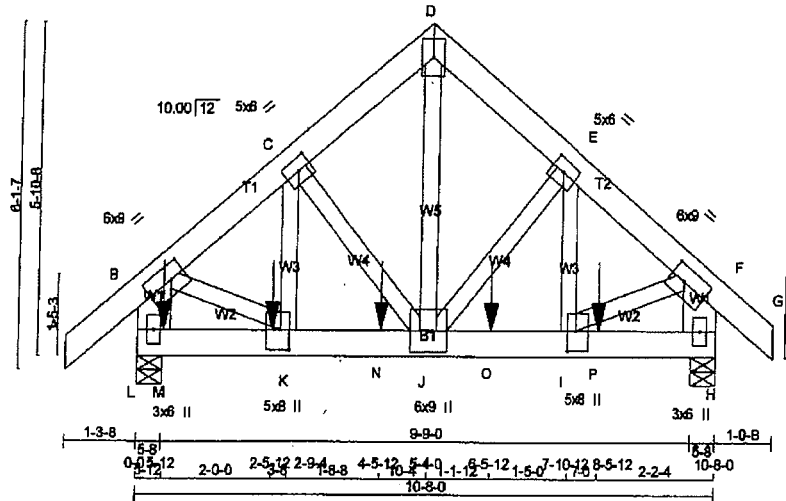
Tamarack Roof Truss, Burlington

Version 8.210.9 May 16 2018 MTEK Industries, Inc. Mon Aug 20 09:46:00 2018 Page 1

ID: P5XmF1xy5318wCH47R2x3fzNvqf-EKF3q7EXzm372623xdD9mZyV7H4G1Y1k1SOym7J

1-3-8 1-3-8 0-0 2-9-4 2-9-4 2-6-12 5-4-0 2-8-12 7-10-12 2-9-4 10-8-0 1-8-8
5x8 II

Scale = 1:37.0



TOTAL WEIGHT = 8 X 80 = 641 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - O	2x6	DRY	No.2
D - G	2x6	DRY	No.2
L - 8	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - H	2x6	DRY	2100F 1.8E

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3") SPIRAL NAILS		
A-D	12	TOP
C-G	12	TOP
L-B	12	TOP
H-F	12	TOP

BOTTOM CHORDS: MITRSLK-G006 SCREWS

WEBS: (0.122'X3") SPIRAL NAILS

2x4 1 6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLYS. SCREWS TO BE DRIVEN FROM ONE SIDE ONLY. DO NOT SINK SCREW HEADS INTO LUMBER.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES, FOR OTHER BOLT TYPES SEE CSA085 3.3.2.

BOLTING MAY BE OMITTED IN PANELS THAT ARE CONNECTED WITH SCREWS.

GIRDER NAILING ASSUMES NAILO HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-1	MT20	8.0	9.0	3.00 3.25
C	TMWV-1	MT20	5.0	8.0	2.50 2.75
D	TTW-2	MT20	6.0	8.0	
E	TMWV-1	MT20	5.0	8.0	2.50 2.75
F	TMWV-1	MT20	6.0	9.0	3.00 3.25
H	BMV-1+P	MT20	3.0	6.0	
I	BMVWV-1	MT20	5.0	8.0	4.25 2.00
J	BMVWV-1	MT20	8.0	9.0	
K	BMVWV-1	MT20	5.0	8.0	4.25 2.00
L	BMV-1+P	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	15331	0	-3183	5-8
H	11859	0	-2417	5-8

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 3183 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT H FOR 2417 LBS. FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 280 LBS. FACTORED HORIZONTAL REACTION AT JOINT L OK

UNFACTORED REACTIONS

	1ST LGASE	MAX. MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	10993	7773/0 1417/0 0/0 388/-3433 1803/0 0/0
H	8359	5815/0 1074/0 0/0 341/-2807 1370/0 0/0

HORIZONTAL REACTIONS	L	H
	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.52 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0/93	-170.4 -170.4 0.05 (2)	10.00	J-D	-2528 / 11405 0.46 (1)
B-C	-12181 / 2581	-170.4 -170.4 0.17 (3)	4.52	J-E	-2682 / 719 0.13 (3)
C-O	-9409 / 2103	-170.4 -170.4 0.15 (3)	5.03	I-E	-695 / 3331 0.13 (2)
D-E	-9414 / 2104	-170.4 -170.4 0.15 (2)	5.04	C-J	-3250 / 839 0.15 (2)
E-F	-11589 / 2482	-170.4 -170.4 0.18 (2)	4.80	K-C	-853 / 4111 0.17 (3)
F-G	0/77	-170.4 -170.4 0.03 (3)	10.00	B-K	-1982 / 9758 0.39 (1)
L-B	-11407 / 2384	0.0 -0.0 0.15 (1)	8.71	I-F	-1889 / 9372 0.38 (1)
H-F	-10932 / 2286	0.0 0.0 0.15 (1)	8.82		

L-M	-271 / 278	-38.5 -38.5 0.17 (2)	6.25
M-K	-271 / 278	-38.5 -38.5 0.17 (2)	8.25
K-N	-1973 / 9278	-38.5 -38.5 0.27 (1)	8.25
N-J	-1973 / 9278	-38.5 -38.5 0.27 (1)	8.25
J-O	-1777 / 8909	-38.5 -38.5 0.27 (1)	10.00
O-I	-1777 / 8909	-38.5 -38.5 0.27 (1)	10.00
I-P	-9/19	-38.5 -38.5 0.14 (3)	10.00
P-H	-9/19	-38.5 -38.5 0.14 (3)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
K	2-5-12	-3574	-3574	543	BACK	VERT	TOTAL
M	5-12	-3437	-3437	509	BACK	VERT	TOTAL
N	4-5-12	-3477	-3477	525	BACK	VERT	TOTAL
O	6-5-12	-3477	-3477	525	BACK	VERT	TOTAL
P	8-5-12	-3477	-3477	525	BACK	VERT	TOTAL

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(b)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 51.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 75.3 PSF

SPACING = 240 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 51.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.38")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/180 (0.71")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.17/1.00 (B-C3), BC=0.27/1.00 (J-K1),
WB=0.46/1.00 (D-J-1), SS=0.84/1.00 (H-K2)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.80)
JSI METAL= 0.84 (I) (INPUT = 1.00)

DWG NO. TAM 11805183
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T35	2	4	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 8.210 S May 18 2018 MTEK Industries, Inc. Mon Aug 20 09:48:00 2018 Page 2					
ID: P5XmF1xy53l8wCH47R2x3fzNvqf-EKF3g7EXzm372l623xhD9mZYv7R4G1YlLk1SOym?i					

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4783.3 lbs FACTORED DOWN AND 1001.0 lbs FACTORED UP AT 5-12, 4891.9 lbs FACTORED DOWN AND 1045.7 lbs FACTORED UP AT 2-5-12, 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 4-5-12, AND 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 8-5-12, AND 4846.3 lbs FACTORED DOWN AND 1020.4 lbs FACTORED UP AT 8-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



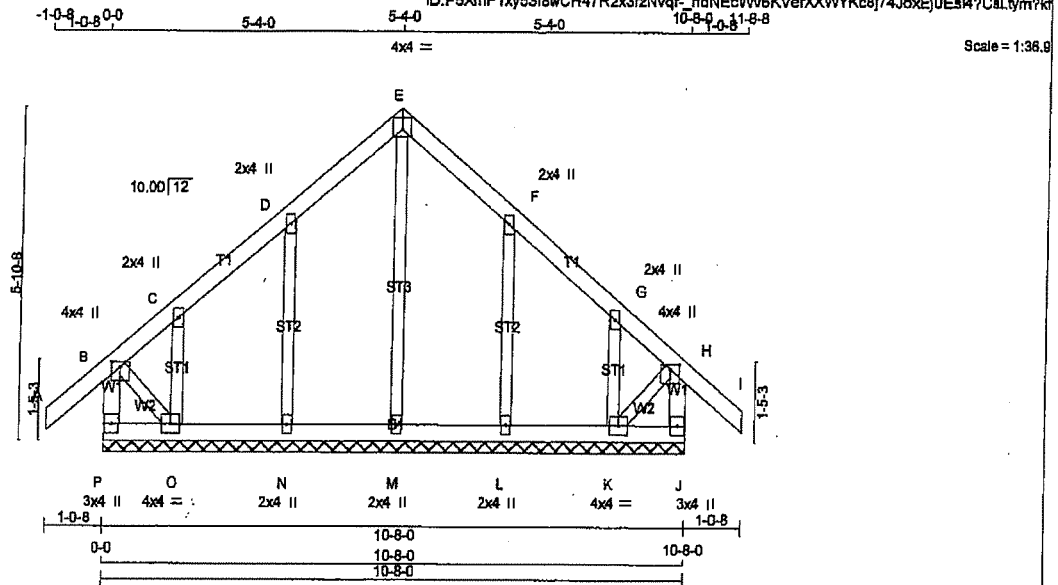
DWG NO. TAM 71805183
STRUCTURAL
COMPONENT ONLY *YH*



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	G36	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: P5XmF1xy53l8wCH47R2x3fzNvqf-ndNEcVv6KVerXXVYKc8j74JoxEj0Ea47Csl.tym?k



Scale = 1:36.9

TOTAL WEIGHT = 2 X 49 = 98 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMVW+p	MT20	2.0	4.0		
E TMVW+p	MT20	4.0	4.0	1.50	2.00
H TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMVW1+p	MT20	3.0	4.0		
K BMVW1-1	MT20	4.0	4.0		
L, M, N					
L BMVW1+p	MT20	2.0	4.0		
O BMVW1-1	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LC1 MAX (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
P-B	-332 / 0	0.0	0.0 0.03 (1)	M-E	-184 / 0	0.09 (1)	7.81
A-B	0 / 47	-129.7	-129.7 0.12 (1)	N-D	-309 / 0	0.08 (1)	10.00
B-C	-75 / 0	-129.7	-129.7 0.11 (1)	O-C	-131 / 0	0.02 (1)	6.25
C-D	-20 / 0	-129.7	-129.7 0.08 (1)	L-F	-309 / 0	0.08 (1)	6.25
D-E	-42 / 0	-129.7	-129.7 0.08 (1)	K-G	-131 / 0	0.02 (1)	6.25
E-F	-42 / 0	-129.7	-129.7 0.08 (1)	B-D	0 / 40	0.01 (1)	8.25
F-G	-20 / 0	-129.7	-129.7 0.08 (1)	K-H	0 / 40	0.01 (1)	8.25
G-H	-75 / 0	-129.7	-129.7 0.11 (1)				8.25
H-I	0 / 47	-129.7	-129.7 0.12 (1)				10.00
J-H	-332 / 0	0.0	0.0 0.03 (1)				7.81
P-O	0 / 0	-38.5	-38.5 0.02 (3)				10.00
O-N	0 / 26	-38.5	-38.5 0.03 (2)				10.00
N-M	0 / 18	-38.5	-38.5 0.03 (3)				10.00
M-L	0 / 18	-38.5	-38.5 0.03 (3)				10.00
L-K	0 / 26	-38.5	-38.5 0.03 (2)				10.00
K-J	0 / 0	-38.5	-38.5 0.02 (3)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
CL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 81.7 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (H-I), BC=0.03/1.00 (K-L2), WB=0.09/1.00 (E-M1), SS=0.10/1.00 (E-F1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1987 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

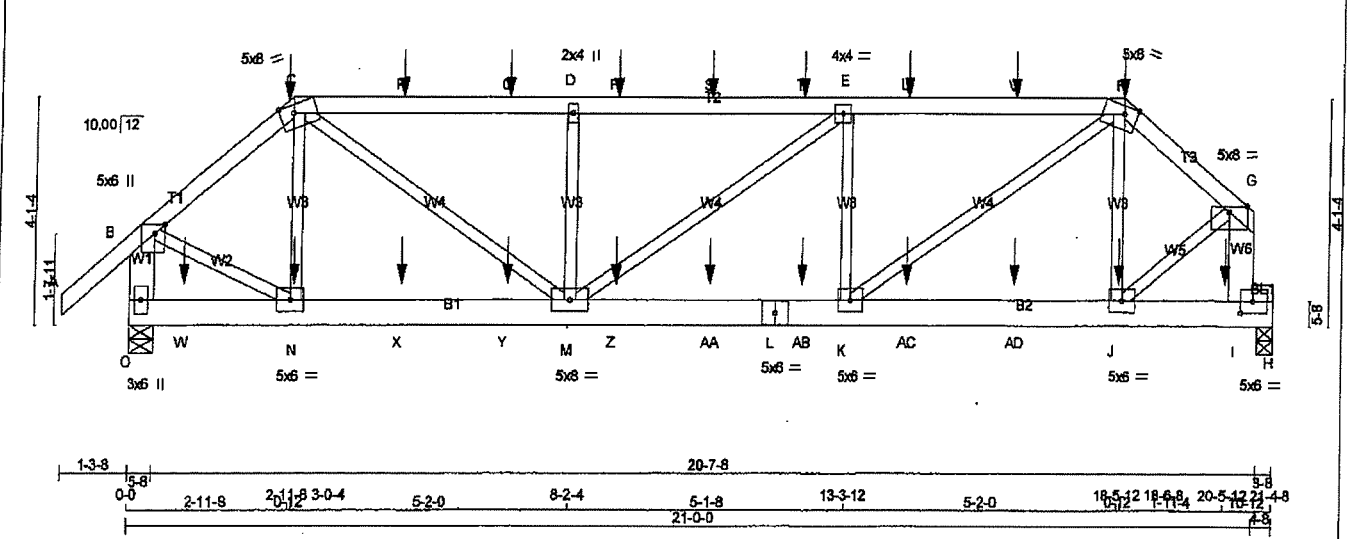


DRWG NO. TAM 71805168
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME T131	QUANTITY 1	PLY 2	JOB DESC. TRUSS OESC.	DRWG NO.
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ID: P5XmF1xy5318wCH47R2x3fzNvgf-65VagUH21_ZYXNqpm9JcJ82kwZ05m7oyEbbym7
1-3-8 1-3-8 0-0 2-11-8 2-11-8 5-2-12 8-2-4 5-1-8 13-3-12 5-2-12 18-5-8 2-5-8 21-0-0
Scale = 1:37.7



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
O - B	2x6	DRY	No.2
I - G	2x6	DRY	No.2
O - L	2x6	DRY	No.2
L - H	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	1	12
C-F	1	12
F-G	1	12
O-B	2	12
I-G	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
O-L	2	12
L-H	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	8
2x4	1	8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TTWW+p	MT20	5.0	6.0	2.00 2.25
C	TTWW-m	MT20	5.0	8.0	Edge 3.00
D	TMWW-w	MT20	2.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTWW-m	MT20	5.0	8.0	Edge 3.00
G	TMWW-p	MT20	5.0	8.0	Edge
I	BMVK-t	MT20	5.0	6.0	2.50 2.75
J, K, N					
J	BMWW-t	MT20	5.0	6.0	
L	BS-t	MT20	5.0	6.0	
M	BMWW-w	MT20	5.0	8.0	
O	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	
O	3251	0	3251	0	5-8	5-8	
H	3082	0	3082	0	3-8	3-8	

UNFACTORED REACTIONS		1ST CASE		MAX./MIN. COMPONENT REACTIONS	
GROSS REACTION		COMBINED	SNOW	LIVE	PERM.LIVE
JT	3251	1541	0	388	0
O	3082	1432	0	392	0
H					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 57	-129.7 -129.7	0.10 (1)	10.00	N-C	-598 / 38	0.08 (1)
B-C	-2688 / 0	-129.7 -129.7	0.14 (1)	5.20	C-M	0 / 2470	0.31 (1)
C-P	-4265 / 0	-129.7 -129.7	0.53 (1)	4.02	M-D	-1171 / 0	0.15 (1)
P-Q	-4265 / 0	-129.7 -129.7	0.53 (1)	4.02	D-E	0 / 49	0.01 (1)
Q-D	-4265 / 0	-129.7 -129.7	0.53 (1)	4.02	E-F	-1208 / 0	0.15 (1)
D-R	-4265 / 0	-129.7 -129.7	0.54 (1)	3.99	K-F	0 / 2460	0.30 (1)
R-S	-4265 / 0	-129.7 -129.7	0.54 (1)	3.99	J-F	-527 / 48	0.07 (1)
S-T	-4265 / 0	-129.7 -129.7	0.54 (1)	3.99	B-N	0 / 2460	0.30 (1)
T-E	-4265 / 0	-129.7 -129.7	0.54 (1)	3.99	J-G	0 / 2460	0.41 (1)
E-U	-4245 / 0	-129.7 -129.7	0.54 (1)	4.01			
U-V	-4245 / 0	-129.7 -129.7	0.54 (1)	4.01			
V-F	-4245 / 0	-129.7 -129.7	0.54 (1)	4.01			
F-G	-2948 / 0	-129.7 -129.7	0.10 (1)	5.27			
O-B	-3207 / 0	0.0 0.0	0.12 (1)	7.73			
I-G	-3880 / 0	0.0 0.0	0.15 (1)	7.19			

CHORDS		FACTORED		WEBS	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
Q-W	0 / 0	-38.5 -38.5	0.05 (2)	10.00	
W-N	0 / 0	-38.5 -38.5	0.05 (2)	10.00	
N-X	0 / 2270	-38.5 -38.5	0.19 (1)	10.00	
X-Y	0 / 2270	-38.5 -38.5	0.19 (1)	10.00	
Y-M	0 / 2270	-38.5 -38.5	0.19 (1)	10.00	
M-Z	0 / 4248	-38.5 -38.5	0.38 (1)	10.00	
Z-AA	0 / 4246	-38.5 -38.5	0.36 (1)	10.00	
AA-L	0 / 4246	-38.5 -38.5	0.38 (1)	10.00	
L-AB	0 / 4246	-38.5 -38.5	0.36 (1)	10.00	
AB-K	0 / 4246	-38.5 -38.5	0.38 (1)	10.00	
K-AC	0 / 2238	-38.5 -38.5	0.29 (1)	10.00	
AC-AD	0 / 2238	-38.5 -38.5	0.29 (1)	10.00	
AD-J	0 / 2238	-38.5 -38.5	0.29 (1)	10.00	
J-I	-538 / 0	-38.5 -38.5	0.87 (1)	8.25	
I-H	0 / 0	-168.2 -168.2	0.42 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE	
JT	LOC.	MAX-	MAX+	FRONT	VERT	DEAD	
C	2-11-8	-28	-	BACK	VERT	TOTAL	
C	2-11-8	-148	-	FRONT	VERT	TOTAL	
C	2-11-8	-176	-	FRONT	VERT	SNOW	
F	18-6-8	-28	-	FRONT	VERT	DEAD	
F	18-6-8	-112	-	BACK	VERT	TOTAL	
F	18-6-8	-176	-	FRONT	VERT	SNOW	
I	20-5-12	-43	-	BACK	VERT	TOTAL	

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
CL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.71")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.71")
CALCULATED VERT. DEFL. (TL) = L/999 (0.13")

CS1: TC=0.54/1.00 (E-F-1), BC=0.87/1.00 (I-J-1), WB=0.41/1.00 (G-I-1), SS=0.62/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	MAX MIN	MAX MIN	MAX MIN
	818 354	1657 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.44 (L) (INPUT = 1.00)



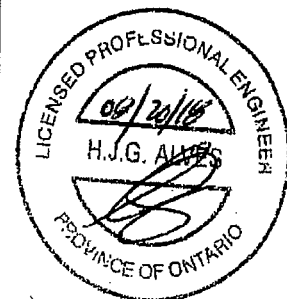
JOB NAME 297078	TRUSS NAME T131	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					Version 6.210 S May 18 2018 Mittek Industries, Inc. Mon Aug 20 09:48:04 2018 Page 2 ID:P5XmF1xy53l8wCH47R2x3fzNvgf-65VagUH21 ZYXNQpImI9JcI82kwZ05m7cylEb9ym2H

HANGER NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 289.0 lbs FACTORED DOWN AT 18-8-8,
289.0 lbs FACTORED DOWN AT 2-11-8, 216.8 lbs
FACTORED DOWN AT 2-11-8, 158.1 lbs
FACTORED DOWN AT 5-0-4, 158.1 lbs
FACTORED DOWN AT 7-0-4, 158.1 lbs
FACTORED DOWN AT 9-0-4, 158.1 lbs
FACTORED DOWN AT 10-9-0, 158.1 lbs
FACTORED DOWN AT 12-5-12, 158.1 lbs
FACTORED DOWN AT 14-5-12, AND 158.1 lbs
FACTORED DOWN AT 18-5-12, AND 183.1 lbs
FACTORED DOWN AT 18-5-8 ON TOP CHORD,
AND 54.9 lbs FACTORED DOWN AT 1-0-4, 61.9
lbs FACTORED DOWN AT 3-0-4, 54.9 lbs
FACTORED DOWN AT 5-0-4, 54.9 lbs
FACTORED DOWN AT 7-0-4, 54.9 lbs
FACTORED DOWN AT 9-0-4, 54.9 lbs
FACTORED DOWN AT 10-9-0, 54.9 lbs
FACTORED DOWN AT 12-5-12, 54.9 lbs
FACTORED DOWN AT 14-5-12, 54.9 lbs
FACTORED DOWN AT 18-5-12, AND 81.9 lbs
FACTORED DOWN AT 18-5-12, AND 81.1 lbs
FACTORED DOWN AT 20-5-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LCI	MAX-	MAX+	FACE	DIR.	TYPE
J	18-5-12	-44	-44	---	BACK	VERT	TOTAL
N	3-0-4	-44	-44	---	BACK	VERT	TOTAL
P	5-0-4	-106	-106	---	BACK	VERT	TOTAL
Q	7-0-4	-106	-106	---	BACK	VERT	TOTAL
R	9-0-4	-106	-106	---	BACK	VERT	TOTAL
S	10-9-0	-106	-106	---	BACK	VERT	TOTAL
T	12-5-12	-106	-106	---	BACK	VERT	TOTAL
U	14-5-12	-106	-106	---	BACK	VERT	TOTAL
V	18-5-12	-106	-106	---	BACK	VERT	TOTAL
W	1-0-4	-38	-38	---	BACK	VERT	TOTAL
X	5-0-4	-38	-38	---	BACK	VERT	TOTAL
Y	7-0-4	-38	-38	---	BACK	VERT	TOTAL
Z	9-0-4	-38	-38	---	BACK	VERT	TOTAL
AA	10-9-0	-38	-38	---	BACK	VERT	TOTAL
AB	12-5-12	-38	-38	---	BACK	VERT	TOTAL
AC	14-5-12	-38	-38	---	BACK	VERT	TOTAL
AD	18-5-12	-38	-38	---	BACK	VERT	TOTAL

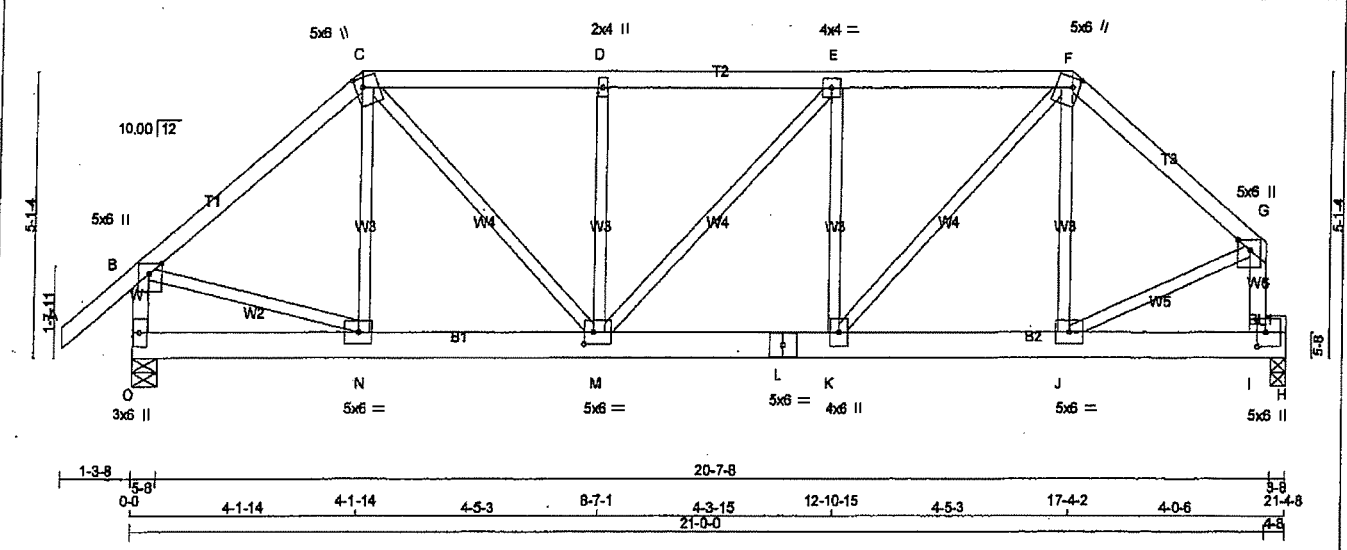


DWG NO. TAM **1805184**
STRUCTURAL
COMPONENT ONLY **2/2**



JOB NAME 297078	TRUSS NAME T132	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 09:48:07 2018 Page 1
ID: P5XmF1xy53l8wCH47R2x3fzNvqf-XgAllWJwKvx7Qg9OzvJsfLgYyZKDEZUwvVCUym7j0
1-3-8 1-3-8 0-0 4-1-14 4-1-14 4-5-3 8-7-1 4-3-15 12-10-16 4-5-3 17-4-2 3-7-14 21-0-0
Scale = 1:37.3



TOTAL WEIGHT = 108 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	QESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x6	DRY	No.2
L - H	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge 2.75
C	TTVW+m	MT20	5.0	6.0	2.00 1.50
D	TMVW+p	MT20	2.0	4.0	
E	TMVW+p	MT20	4.0	4.0	
F	TTVW+m	MT20	5.0	6.0	2.00 1.50
G	TMVW+p	MT20	5.0	8.0	Edge 2.75
I	BMVW+p	MT20	5.0	8.0	3.00 1.75
J	BMVW+p	MT20	5.0	8.0	
K	BMVW+p	MT20	4.0	6.0	
L	BS-1	MT20	5.0	8.0	
M	BMVW+p	MT20	5.0	6.0	2.50 2.00
N	BMVW+p	MT20	5.0	8.0	
O	BMVW+p	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RECD
	VERT	DOWN	BRG
O	1978	0	5-8
H	1749	0	3-8

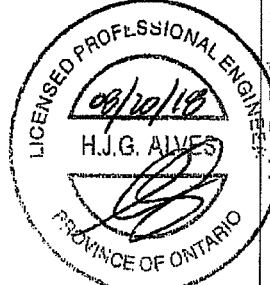
UNFACTORED REACTIONS			
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE
O	1441	923 / 0	224 / 0
H	1286	789 / 0	224 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO			
A-B	0 / 57	-129.7	-129.7 0.18 (1)
B-C	-1694 / 0	-129.7	-129.7 0.48 (1)
C-D	-2801 / 0	-129.7	-129.7 0.39 (1)
D-E	-2001 / 0	-129.7	-129.7 0.40 (1)
E-F	-1987 / 0	-129.7	-129.7 0.40 (1)
F-G	-1707 / 0	-129.7	-129.7 0.38 (1)
G-H	-1910 / 0	0.0	0.0 0.20 (1)
H-I	-1989 / 0	0.0	0.0 0.22 (1)
O-N	0 / 0	-38.5	-38.5 0.06 (3)
N-M	0 / 1294	-38.5	-38.5 0.20 (1)
M-L	0 / 1987	-38.5	-38.5 0.30 (1)
L-K	0 / 1987	-38.5	-38.5 0.30 (1)
K-J	0 / 1305	-38.5	-38.5 0.33 (1)
J-I	-176 / 0	-38.5	-38.5 0.61 (1)
I-H	0 / 0	-38.5	-38.5 0.38 (1)



DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	38.2 PSF
	DL	=	8.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD = 81.7 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, NBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 34.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/380 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.46/1.00 (B-C:1), BC=0.81/1.00 (J-I:1), WB=0.36/1.00 (G-J:1), SS=0.84/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN
MT20	618	354	1667 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS GRIP= 0.87 (C) (INPUT = 0.60)

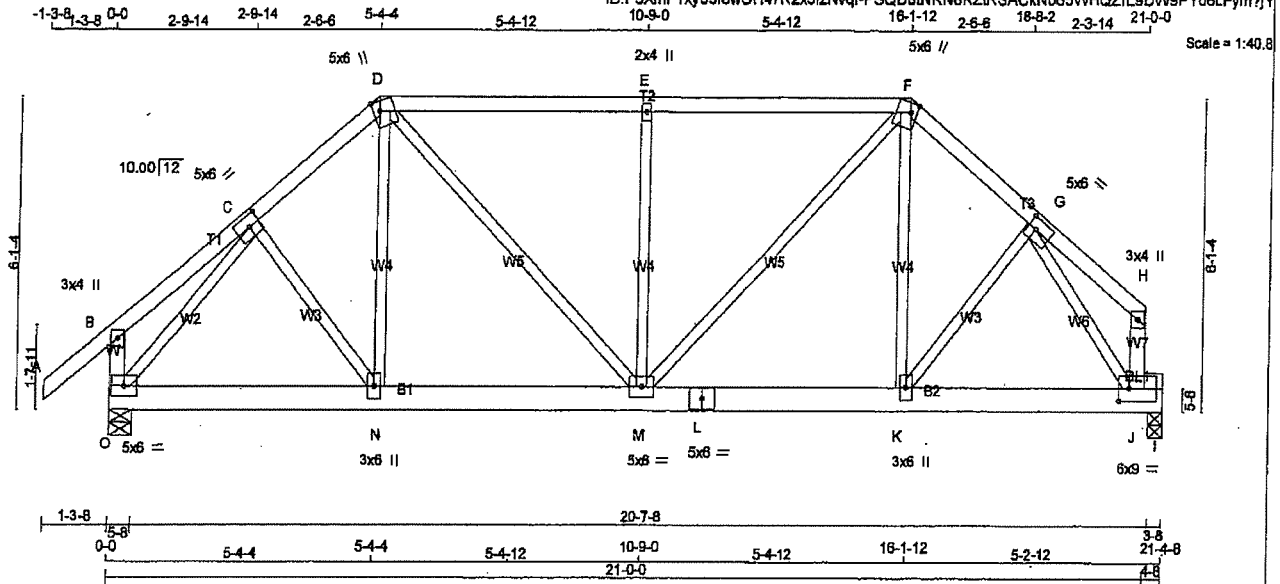
JS METAL= 0.41 (L) (INPUT = 1.00)

DRWG NO. TAM **11805185**
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME T133	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.210 S May 18 2018 MITek Industries, Inc. Mon Aug 20 09:48:11 2018 Page 1



TOTAL WEIGHT = 109 lb

LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.50	2.75
D	TTVW+m	MT20	5.0	8.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	8.0	2.25	1.50
G	TMVW+1	MT20	5.0	6.0	2.50	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVW+1	MT20	8.0	8.0	3.00	2.50
K	BMVW+1	MT20	3.0	6.0		
L	BS-1	MT20	5.0	6.0		
M	BMVW+1	MT20	5.0	6.0		
N	BMVW+1	MT20	3.0	6.0		
O	BMVW+1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	VERT	HORZ	UPLIFT
I	1749	0	1749
O	1978	0	1976

UNFACTORED REACTIONS			
JT	COMBINED	SNOW	LIVE
I	1286	789 / 0	224 / 0
O	1441	923 / 0	224 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)
FR-TO	0 / 57	-129.7	-129.7
A-B	0 / 22	-129.7	-129.7
B-C	-1898 / 0	-129.7	-129.7
C-D	-1750 / 0	-129.7	-129.7
D-E	-1750 / 0	-129.7	-129.7
E-F	-1713 / 0	-129.7	-129.7
F-G	0 / 33	-129.7	-129.7
G-H	-320 / 0	0.0	0.0
H-I	-101 / 0	0.0	0.0
O-N	0 / 1221	-38.5	-38.5
N-M	0 / 1286	-38.5	-38.5
M-L	0 / 1301	-38.5	-38.5
L-K	0 / 1301	-38.5	-38.5
K-J	0 / 1046	-38.5	-38.5
J-I	0 / 0	-38.5	-38.5

WEBS			
MEMB.	FORCE (LBS)	MAX. FACTORED (LC1)	MAX. FACTORED (LC2)
FR-TO	0 / 219	0.05 (3)	0.15 (1)
A-B	0 / 677	0.15 (1)	0.47 (1)
B-C	-853 / 0	0.15 (1)	0.15 (1)
C-D	0 / 855	0.15 (1)	0.05 (2)
D-E	0 / 235	0.05 (2)	0.03 (3)
E-F	0 / 121	0.03 (3)	0.80 (1)
F-G	-1990 / 0	0.80 (1)	0.08 (1)
G-H	0 / 401	0.08 (1)	0.58 (1)
H-I	-2078 / 0	0.58 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
BOT CH. LL = 6.0 PSF
LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 2x6 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/960$ (0.71")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/960$ (0.71")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.73/1.00 (D-E-1), BC=0.73/1.00 (J-K-1),
WB=0.80/1.00 (C-D-1), SSI=0.64/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

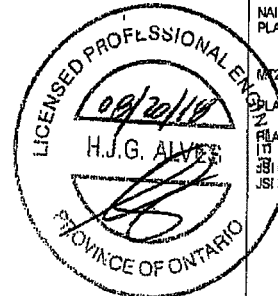
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1887

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (J) (INPUT = 0.80)
JSI METAL=0.49 (G) (INPUT = 1.00)

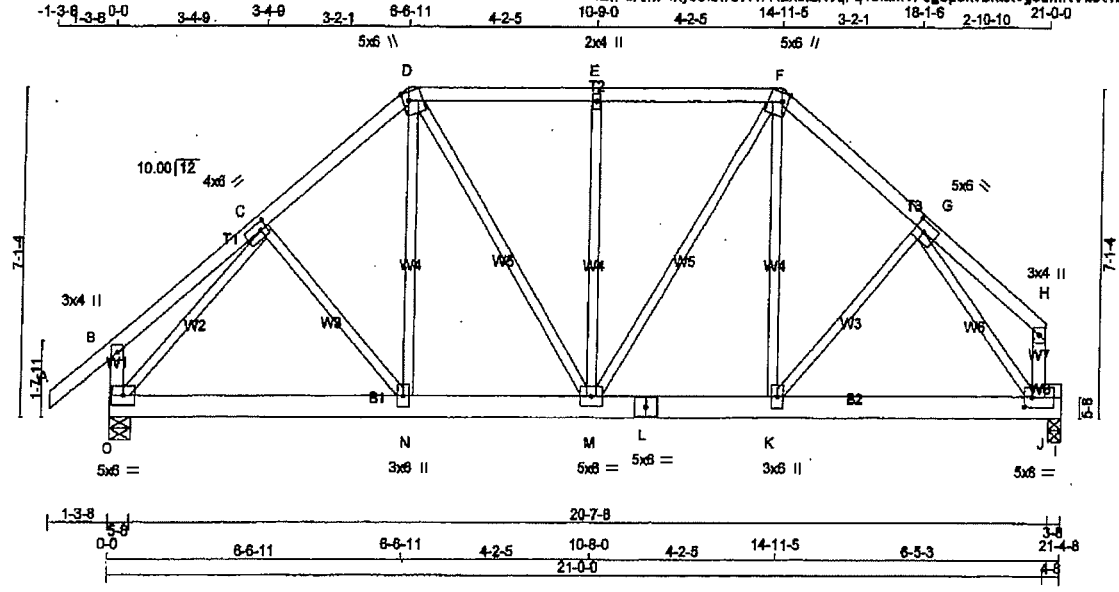


DRWG NO. TAM 17005786
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME T134	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.210 S May 18 2018 MITek Industries, Inc. Mon Aug 20 09:48:14 2018 Page 1
ID: P5XmF1xy53IbwCH47R2x3IzNvqf-q15MmVpJg3p8kvBktcvJ8thmNVMDX4b5VW7myaym7JW



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - O	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
J - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMVW+4	MT20	4.0	6.0	2.00 1.75
D TTWW+m	MT20	5.0	6.0	2.25 1.50
E TMV+w	MT20	2.0	4.0	
F TTWW+m	MT20	5.0	8.0	2.25 1.50
G TMVW+4	MT20	5.0	8.0	2.50 2.50
H TMV+p	MT20	3.0	4.0	
J BMVW+4	MT20	5.0	8.0	2.50 2.00
K BMVW+4	MT20	3.0	6.0	
L BS+4	MT20	5.0	8.0	
M BMVW+4	MT20	5.0	8.0	
N BMVW+4	MT20	3.0	6.0	
O BMVW+4	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
		DOWN		DOWN		IN-SX		IN-SX	
		VERT		HORIZ		UPLIFT		IN-SX	
JT		1978	0	1978	0	0	5.8	5.8	
O		1748	0	1748	0	0	3.8	3.8	
I									
UNFACTORED REACTIONS									
1ST CASE									
MAX./MIN. COMPONENT REACTIONS									
		COMBINED		SNOW		LIVE		PERM. LIVE	
		O		O		O		O	
JT		1441	923 / 0	224 / 0		0 / 0		284 / 0	0 / 0
O		1295	789 / 0	224 / 0		0 / 0		273 / 0	0 / 0
I									

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
		MAX. FACTORED				MAX. FACTORED	
		FORCE				FORCE	
		(LBS)				(LBS)	
		VERT. LOAD		MAX. FACTORED		MAX. FACTORED	
		(PLF)		CS1 (LC)		CS1 (LC)	
		FROM TO		LENGTH		FR-TO	
A-B	0 / 57	-129.7	-129.7 0.18 (1)	10.00		C-N	-40 / 80
B-C	0 / 30	-129.7	-129.7 0.21 (1)	10.00		N-D	0 / 334
C-O	-1842 / 0	-129.7	-129.7 0.25 (1)	4.91		D-M	0 / 410
D-E	-1457 / 0	-129.7	-129.7 0.41 (1)	4.90		M-E	-858 / 0
E-F	-1457 / 0	-129.7	-129.7 0.41 (1)	4.90		F-F	0 / 396
F-G	-1854 / 0	-129.7	-129.7 0.22 (1)	4.93		K-F	0 / 352
G-H	0 / 30	-129.7	-129.7 0.18 (1)	10.00		K-G	0 / 187
O-B	-344 / 0	0.0	0.0 0.04 (1)	7.81		C-C	-1979 / 0
J-H	-132 / 0	0.0	0.0 0.01 (1)	7.81		G-J	-2003 / 0
O-N	0 / 1261	-38.5	-38.5 0.25 (1)	10.00			
N-M	0 / 1240	-38.5	-38.5 0.24 (2)	10.00			
M-L	0 / 1253	-38.5	-38.5 0.36 (1)	10.00			
L-K	0 / 1253	-38.5	-38.5 0.36 (1)	10.00			
K-J	0 / 1148	-38.5	-38.5 0.57 (1)	10.00			
J-I	0 / 0	-38.5	-38.5 0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.09/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CS1: TC=0.41/1.00 (E-F:1), BC=0.57/1.00 (J-K:1), WB=0.81/1.00 (C-O:1), SS=0.84/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

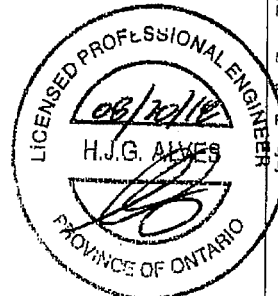
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 768 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

CS1 GRIP= 0.88 (C) (INPUT = 0.90)
CS1 METAL= 0.49 (C) (INPUT = 1.00)

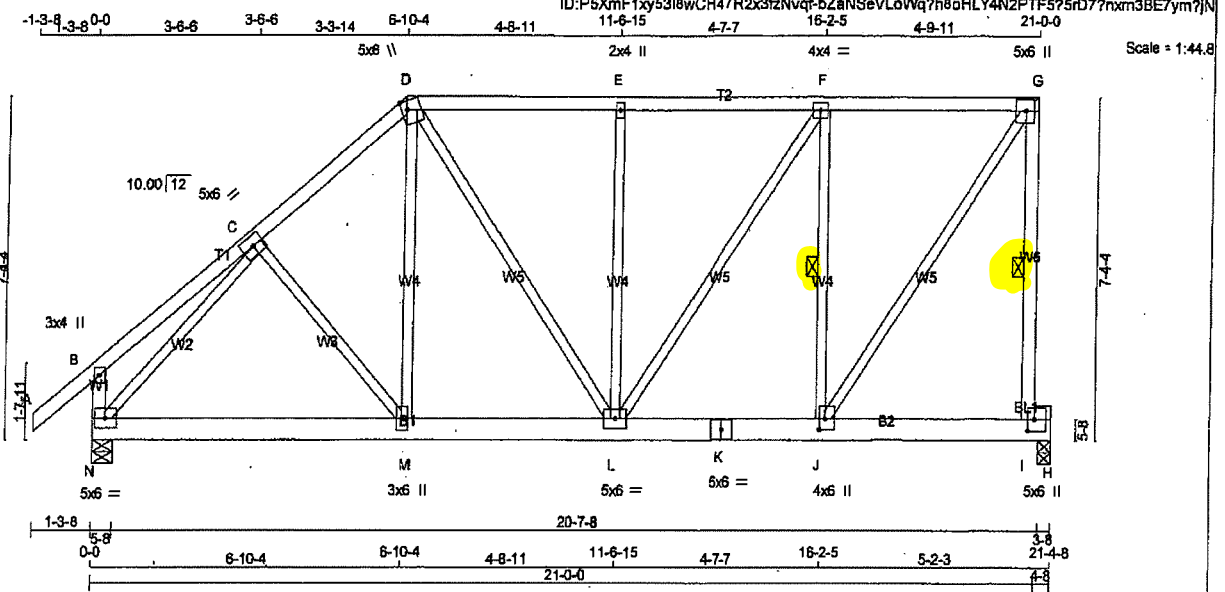
DWG NO. TAM 11805187
STRUCTURAL
COMPONENT ONLY





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T136	2	1	TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
I - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-1	MT20	5.0	8.0	
D	TTWW+m	MT20	6.0	8.0	2.25 1.50
E	TMVW-w	MT20	2.0	4.0	
F	TMVW-1	MT20	4.0	4.0	
G	TMVW+p	MT20	5.0	8.0	3.00 2.25
I	BMVW+p	MT20	5.0	8.0	3.00 1.75
J	BMVW-1	MT20	4.0	8.0	2.75 1.50
K	BS-1	MT20	5.0	6.0	
L	BMVW-1	MT20	5.0	6.0	
M	BMVW-1	MT20	3.0	6.0	
N	BMVW-1	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1975	0	1975	0	5-8
H	1749	0	1749	0	3-8

UNFACTORED REACTIONS		1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE				
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0	0 / 0
H	1286	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAO CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0 / 57	-129.7	-129.7 0.18 (1)	C-M	-72 / 73	0.03 (1)	
B-C	0 / 32	-129.7	-129.7 0.23 (1)	M-D	0 / 372	0.08 (2)	
C-D	-1827 / 0	-129.7	-129.7 0.28 (1)	N-C	-1975 / 0	0.87 (1)	
D-E	-1395 / 0	-129.7	-129.7 0.41 (1)	J-G	0 / 1995	0.45 (1)	
E-F	-1395 / 0	-129.7	-129.7 0.43 (1)	D-L	0 / 300	0.07 (1)	
F-G	-1082 / 0	-129.7	-129.7 0.42 (1)	J-F	-1130 / 0	0.37 (1)	
I-G	-1897 / 0	0.0	0.0 0.44 (1)	L-E	-553 / 0	0.59 (1)	
N-B	-351 / 0	0.0	0.0 0.04 (1)	L-F	0 / 550	0.12 (1)	
N-M	0 / 1270	-38.5	-38.5 0.28 (2)				
M-L	0 / 1227	-38.5	-38.5 0.28 (2)				
L-K	0 / 1062	-38.5	-38.5 0.32 (1)				
K-J	0 / 1062	-38.5	-38.5 0.32 (1)				
J-I	-40 / 0	-38.5	-38.5 0.61 (1)				
I-H	0 / 0	-38.5	-38.5 0.38 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 38.2 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAO	= 61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 64.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.44/1.00 (G-I:1), BC=0.61/1.00 (I-K:1), WB=0.87/1.00 (C-N:1), SS=0.64/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAO FACTOR = 1.00

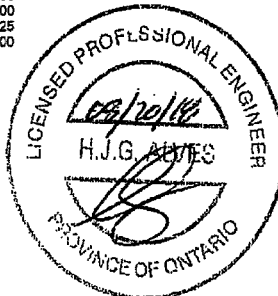
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (FLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (G) (INPUT = 0.90)
JSI METAL=0.51 (J) (INPUT = 1.00)



DRWG NO. TAM 11805199
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 11805790
STRUCTURAL
COMPONENT ONLY

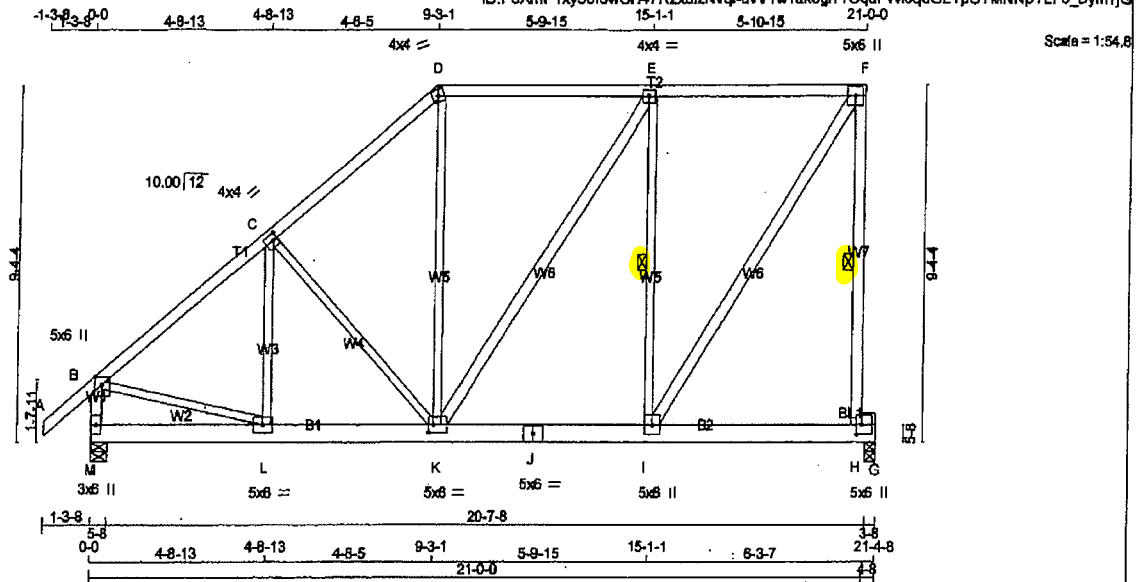


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T138	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 129 = 258 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
K - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	5.0	8.0	2.50	2.25
C TMWV-t	MT20	4.0	4.0	2.00	1.25
O TMW-m	MT20	4.0	4.0		
E TMWV-t	MT20	4.0	4.0		
F TMWV+p	MT20	5.0	8.0		
H BMVK+p	MT20	5.0	8.0	3.00	1.75
I BMWV-t	MT20	5.0	6.0		
J BS-t	MT20	5.0	8.0		
K BMWV-t	MT20	5.0	8.0	2.50	1.50
L BMWV-t	MT20	5.0	6.0		
M BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
				BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1978	0	1978	0	5-8
G	1749	0	1749	0	3-8

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS		
M	1441	823 / 0	224 / 0	0 / 0	0 / 0	294 / 0
G	1288	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MAX	MAX	MEMB.	MAX. FACTORED
	FORCE	VERT. LOAD	LC1	MAX	UNBRAC		FORCE
	(LBS)	(PLF)	CSI (LC)	CS1 (LC)	CS1 (LC)		(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	L-C	-147 / 121
B-C	-1720 / 0	-129.7	-129.7	0.57 (1)	4.39	C-K	-483 / 0
C-D	-1413 / 0	-129.7	-129.7	0.53 (1)	4.78	K-D	0 / 376
D-E	-1051 / 0	-129.7	-129.7	0.81 (1)	4.53	K-E	0 / 119
E-F	-987 / 0	-129.7	-129.7	0.81 (1)	4.85	I-E	-1043 / 0
F-H	-1838 / 0	0.0	0.0	0.72 (1)	4.94	I-F	0 / 1849
H-B	-1895 / 0	0.0	0.0	0.20 (1)	8.10	B-L	0 / 1400
M-L	0 / 0	-38.5	-38.5	0.07 (2)	10.00		
L-K	0 / 1355	-38.5	-38.5	0.22 (1)	10.00		
K-J	0 / 967	-38.5	-38.5	0.35 (1)	10.00		
J-I	0 / 967	-38.5	-38.5	0.35 (1)	10.00		
I-H	-30 / 0	-38.5	-38.5	0.81 (1)	8.25		
H-G	0 / 0	-38.5	-38.5	0.38 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2016

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 088-08, CSA 088-14
- TRC 2011, TPC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.81/1.00 (D-E-1), BC=0.61/1.00 (H-I-1), WB=0.58/1.00 (E-I-1), SS=0.64/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

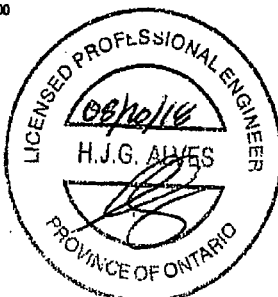
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	618	354	1667
	786	1887	1635

PLATE PLACEMENT TOL = 0.250 inches

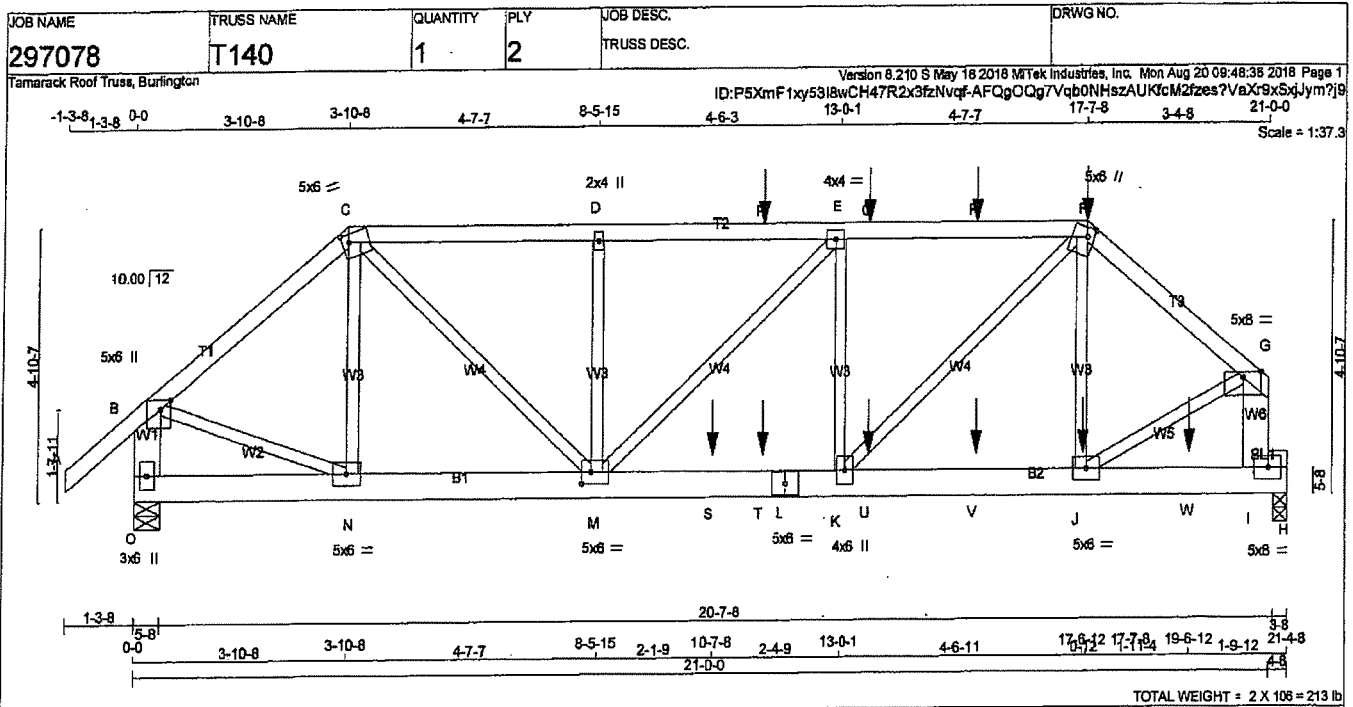
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.90 (B) (INPUT=0.90)
JSI METAL=0.38 (I) (INPUT=1.00)



DRWG NO. TAM 17005791
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 11805192
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF		
C - F	2x4	DRY	No.2	SPF		
F - G	2x4	DRY	No.2	SPF		
O - B	2x6	DRY	No.2	SPF		
I - G	2x6	DRY	No.2	SPF		
O - L	2x6	DRY	No.2	SPF		
L - H	2x6	DRY	No.2	SPF		

BEARING BLOCKS
BL1 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12	TOP SIDE(61.0)	
C-F 1 12	TOP SIDE(61.0)	
F-G 1 12	TOP	
O-B 2 12	TOP	
I-G 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
C-L 2 12	SIDE(0.0)	
L-H 2 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 8		
2x4 1 8		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIROE NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWW+p	MT20	5.0	8.0	2.00	2.25	
C TTWW+m	MT20	5.0	8.0	1.75	1.50	
D TMWW+w	MT20	2.0	4.0			
E TMWW+L	MT20	4.0	4.0			
F TTWW+m	MT20	5.0	8.0	2.00	1.50	
G TMWW+p	MT20	5.0	8.0	Edge		
I BMVW-1	MT20	5.0	6.0			
J BMVW-4	MT20	5.0	6.0			
K BMVW+4	MT20	4.0	6.0			
L BS-1	MT20	3.0	6.0			
M BMVW+M-1	MT20	5.0	6.0	2.50	2.00	
N BMVW-1	MT20	5.0	6.0			
O BMV+P	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRO BRG
JT VERT	2830	2830	5-8	5-8
O	2830	2830	5-8	5-8
H	2804	2804	3-8	3-8

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
O	1911	1254 / 0 288 / 0 0 / 0 368 / 0 0 / 0
H	2045	1325 / 0 332 / 0 0 / 0 368 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UPLIFT (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UPLIFT (LBS)
FR-TO					FR-TO			
A-B	0 / 57	-129.7	-129.7	0.10 (1)	N-C	-468 / 2	0.08 (1)	0.27 (1)
B-C	-2419 / 0	-129.7	-129.7	0.22 (1)	C-M	0 / 2171	0.27 (1)	0.11 (1)
C-D	-3402 / 0	-129.7	-129.7	0.25 (1)	M-D	-541 / 0	0.11 (1)	0.07 (1)
D-E	-3402 / 0	-129.7	-129.7	0.34 (1)	E-K	-209 / 0	0.07 (1)	0.13 (1)
E-F	-3402 / 0	-129.7	-129.7	0.34 (1)	K-F	-781 / 0	0.13 (1)	0.24 (1)
F-G	-3549 / 0	-129.7	-129.7	0.35 (1)	G-J	0 / 1963	0.24 (1)	0.07 (1)
G-H	-3549 / 0	-129.7	-129.7	0.35 (1)	J-I	-389 / 78	0.07 (1)	0.24 (1)
H-I	-3549 / 0	-129.7	-129.7	0.35 (1)	I-H	0 / 1939	0.24 (1)	0.24 (1)
I-G	-2807 / 0	-129.7	-129.7	0.18 (1)	G-F	0 / 12678	0.24 (1)	0.24 (1)
O-B	-2555 / 0	0.0	0.0	0.09 (1)	B-N	0 / 1939	0.24 (1)	0.24 (1)
I-G	-3282 / 0	0.0	0.0	0.13 (1)	N-C	0 / 1939	0.24 (1)	0.24 (1)
O-N	0 / 0	-38.5	-38.5	0.03 (2)	C-M	0 / 1939	0.24 (1)	0.24 (1)
N-M	0 / 1842	-38.5	-38.5	0.23 (1)	M-D	0 / 1939	0.24 (1)	0.24 (1)
M-S	0 / 3550	-38.5	-38.5	0.54 (1)	D-E	0 / 1939	0.24 (1)	0.24 (1)
S-T	0 / 3550	-38.5	-38.5	0.54 (1)	E-K	0 / 1939	0.24 (1)	0.24 (1)
T-L	0 / 3550	-38.5	-38.5	0.54 (1)	K-F	0 / 1939	0.24 (1)	0.24 (1)
L-K	0 / 3550	-38.5	-38.5	0.54 (1)	F-G	0 / 1939	0.24 (1)	0.24 (1)
K-U	0 / 2140	-38.5	-38.5	0.26 (1)	G-J	0 / 1939	0.24 (1)	0.24 (1)
U-V	0 / 2140	-38.5	-38.5	0.26 (1)	J-I	0 / 1939	0.24 (1)	0.24 (1)
V-W	0 / 2140	-38.5	-38.5	0.26 (1)	I-H	0 / 1939	0.24 (1)	0.24 (1)
J-W	-455 / 0	-38.5	-38.5	0.81 (1)	H-G	0 / 1939	0.24 (1)	0.24 (1)
W-I	-455 / 0	-38.5	-38.5	0.81 (1)	G-F	0 / 1939	0.24 (1)	0.24 (1)
I-H	0 / 0	-168.2	-168.2	0.38 (1)	F-G	0 / 1939	0.24 (1)	0.24 (1)

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
F	17-7-8	-202	-202	-	FRONT	VERT	TOTAL
J	17-6-12	-26	-26	-	FRONT	VERT	TOTAL
P	11-6-12	-83	-83	-	FRONT	VERT	TOTAL
Q	13-6-12	-83	-83	-	FRONT	VERT	TOTAL
R	15-6-12	-83	-83	-	FRONT	VERT	TOTAL
S	10-7-8	-623	-623	-	FRONT	VERT	TOTAL
T	11-6-12	-21	-21	-	FRONT	VERT	TOTAL
U	13-6-12	-21	-21	-	FRONT	VERT	TOTAL
V	15-6-12	-21	-21	-	FRONT	VERT	TOTAL
W	19-6-12	-11	-16	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2016
- CSA 084-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.7")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.7")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSL TC=0.35/1.00 (E-F-1), BC=0.61/1.00 (I-J-1), WB=0.38/1.00 (G-K-1), SS=0.57/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MIN. VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)

JSI METAL = 0.43 (L) (INPUT = 1.00)

DRWG NO. TAM 11805193
STRUCTURAL
COMPONENT ONLY
1/2

CONTINUED ON PAGE 2



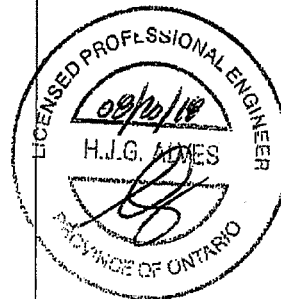
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T140	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:P5XmF1xy53I8wCH47R2x3fzNvaf-AFQgOQg7Vqb0NHszAUKfcM2fzes7VaX9xSxJym79

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 121.6 lbs FACTORED DOWN AT
11-6-12, 121.6 lbs FACTORED DOWN AT
13-6-12, AND 121.6 lbs FACTORED DOWN AT
15-6-12, AND 282.9 lbs FACTORED DOWN AT
17-7-8 ON TOP CHORD, AND 872.4 lbs
FACTORED DOWN AT 10-7-8, 30.9 lbs
FACTORED DOWN AT 11-8-12, 30.9 lbs
FACTORED DOWN AT 13-6-12, 30.9 lbs
FACTORED DOWN AT 15-6-12, AND 38.0 lbs
FACTORED DOWN AT 17-6-12, AND 23.7 lbs
FACTORED DOWN AT 19-6-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

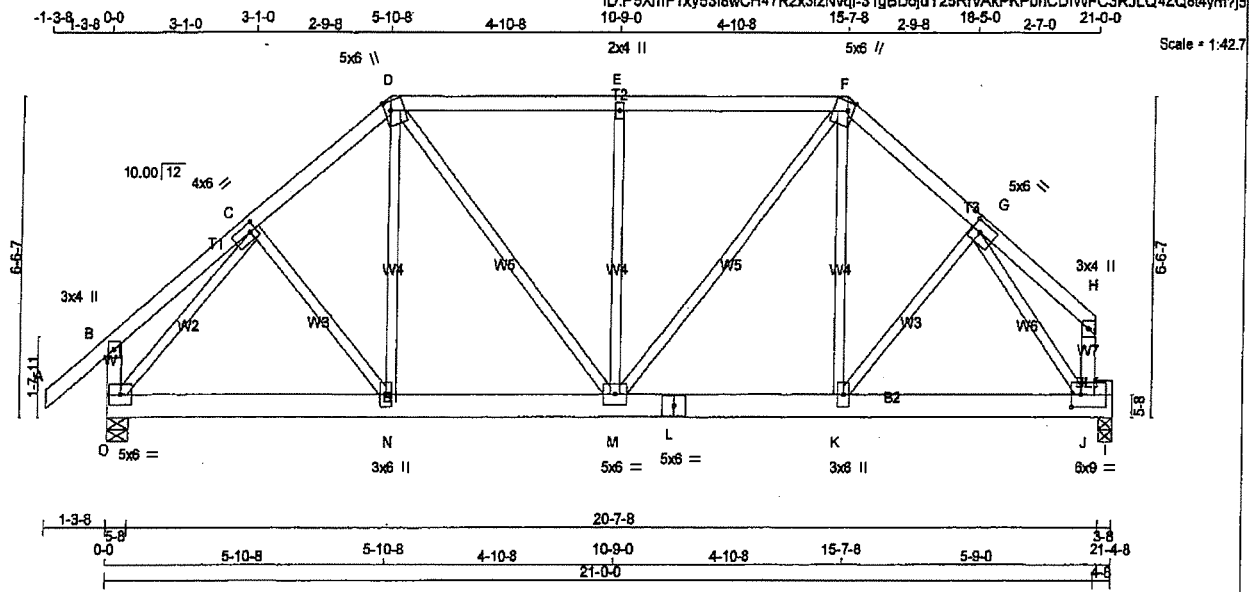


DWG NO. TAM 1105193
STRUCTURAL
COMPONENT ONLY 3/2



JOB NAME 297078	TRUSS NAME T141	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.210 S May 18 2018 MTEK Industries, Inc. Mon Aug 20 09:48:40 2018 Page 1
ID: P5XmF1xy5318wCH47R2x3fzNvqf-31gBDojdY25RrvAkPKPbnCDIWFC3RJLQ4ZQ8Mym7J5



TOTAL WEIGHT = 112 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2

BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMWW+I	MT20	4.0	8.0	2.00 1.50
D TTWW+m	MT20	5.0	8.0	2.25 1.50
E TMW+w	MT20	2.0	4.0	
F TTWW+m	MT20	5.0	8.0	2.25 1.50
G TMWW+I	MT20	5.0	8.0	2.50 2.25
H TMV+p	MT20	3.0	4.0	
J BMVWK+I	MT20	6.0	9.0	3.00 2.50
K BMVWK+I	MT20	3.0	6.0	
L BS-I	MT20	5.0	8.0	
M BMVWW+I	MT20	5.0	6.0	
N BMVWW+I	MT20	3.0	6.0	
O BMVW1-I	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED		MAXIMUM FACTORED		INPUT	
GROSS REACTION		GROSS REACTION		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	1749	0	1749	0	3-8
O	1978	0	1978	0	5-8

UNFACTORED REACTIONS						
1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1286	789 / 0	224 / 0	0 / 0	273 / 0	0 / 0
O	1441	923 / 0	224 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 57	-129.7	-129.7 0.18 (1)	10.00	N-D	0 / 270	0.06 (2)
B-C	0 / 25	-129.7	-129.7 0.16 (1)	10.00	D-M	0 / 550	0.12 (1)
C-O	-1877 / 0	-129.7	-129.7 0.20 (1)	4.93	M-E	-788 / 0	0.50 (1)
D-E	-1810 / 0	-129.7	-129.7 0.57 (1)	4.43	M-F	0 / 520	0.12 (1)
E-F	-1810 / 0	-129.7	-129.7 0.57 (1)	4.43	K-F	0 / 294	0.07 (2)
F-G	-1697 / 0	-129.7	-129.7 0.17 (1)	4.84	C-N	0 / 68	0.02 (3)
G-H	0 / 38	-129.7	-129.7 0.14 (1)	10.00	O-C	-1988 / 0	0.69 (1)
O-B	-332 / 0	0.0	0.0 0.03 (1)	7.81	K-G	0 / 312	0.07 (1)
J-H	-112 / 0	0.0	0.0 0.01 (1)	7.81	G-J	-2082 / 0	0.83 (1)
O-N	0 / 1242	-38.5	-38.5 0.23 (1)	10.00			
N-M	0 / 1268	-38.5	-38.5 0.23 (2)	10.00			
M-L	0 / 1287	-38.5	-38.5 0.38 (1)	10.00			
L-K	0 / 1287	-38.5	-38.5 0.38 (1)	10.00			
K-J	0 / 1084	-38.5	-38.5 0.73 (1)	10.00			
J-I	0 / 0	-38.5	-38.5 0.38 (1)	10.00			

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 81.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.71")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.71")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.13")

CSI: TC=0.57/1.00 (E-F:1), BC=0.73/1.00 (J-K:1),
WB=0.69/1.00 (C-D:1), SI=0.64/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

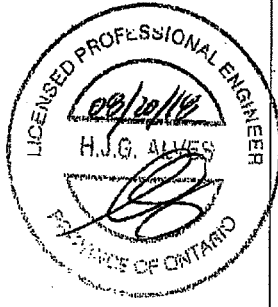
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.88 (C) (INPUT = 0.90)
SI METAL=0.49 (G) (INPUT = 1.00)



DWG NO. TAM **11805194**
STRUCTURAL
COMPONENT ONLY

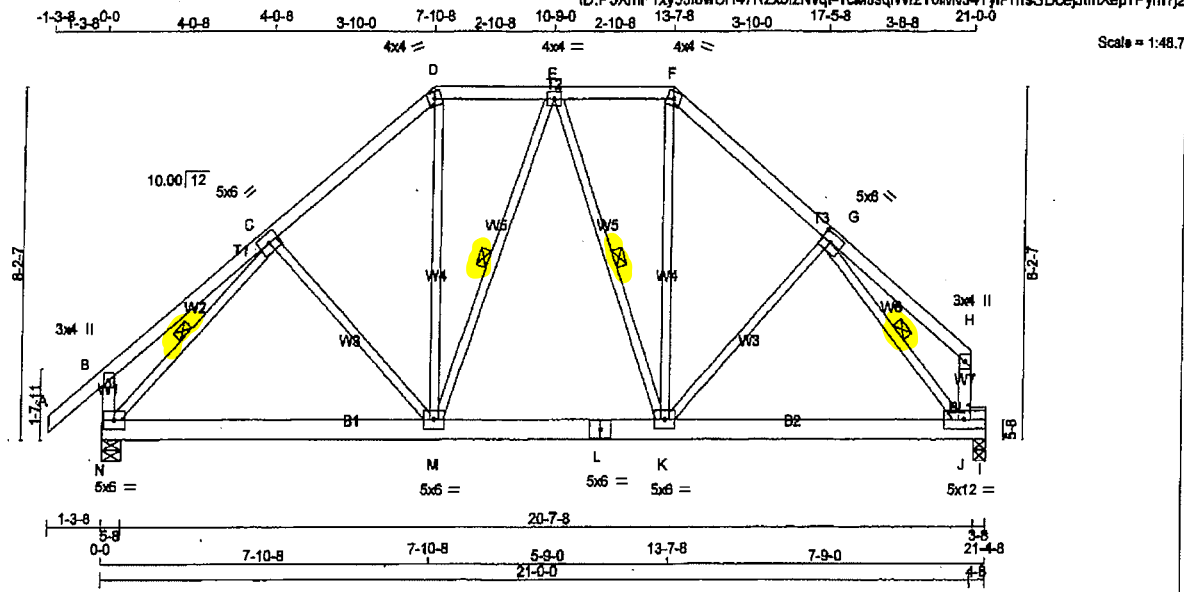


JOB NAME 297078	TRUSS NAME T142	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 09:48:43 2018 Page 1

ID: P5Xmf1xy53l8wCH47R2x3fzNvgf-TcMJsqiWzT0IMvJ4TyPmrsSDoeJlrmXepTPym7J2



TOTAL WEIGHT = 116 lb

LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW+p	MT20	8.0	8.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-m	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	5.0	6.0	2.50 2.75
H	TMV+p	MT20	3.0	4.0	
J	BMVWW-t	MT20	5.0	12.0	
K	BMVWW-t	MT20	5.0	8.0	
L	BS-t	MT20	5.0	8.0	
M	BMVWW-t	MT20	5.0	8.0	
N	BMVWW-t	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1978	0	1978	0	0	5-8	5-8
I	1749	0	1749	0	0	3-8	3-8

UNFACTORED REACTIONS						
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
N	1441	923 / 0	224 / 0	0 / 0	0 / 0	294 / 0
I	1296	789 / 0	224 / 0	0 / 0	0 / 0	273 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	C-M	-150 / 48 0.11 (1)
B-C	0 / 38	-129.7	-129.7	0.32 (1)	10.00	M-D	0 / 613 0.14 (1)
C-D	-1547 / 0	-129.7	-129.7	0.37 (1)	4.66	M-E	-270 / 0 0.12 (1)
D-E	-1164 / 0	-129.7	-129.7	0.18 (1)	5.71	E-K	-186 / 0 0.09 (1)
E-F	-1192 / 0	-129.7	-129.7	0.18 (1)	5.66	K-F	0 / 830 0.14 (1)
F-G	-1570 / 0	-129.7	-129.7	0.34 (1)	4.66	K-G	0 / 112 0.03 (3)
G-H	0 / 56	-129.7	-129.7	0.29 (1)	10.00	N-C	-1943 / 0 0.48 (1)
H-B	-374 / 0	0.0	0.0	0.04 (1)	7.81	G-J	-2030 / 0 0.46 (1)
J-H	-151 / 0	0.0	0.0	0.02 (1)	7.81		
N-M	0 / 1278	-38.5	-38.5	0.32 (2)	10.00		
M-L	0 / 1265	-38.5	-38.5	0.42 (1)	10.00		
L-K	0 / 1255	-38.5	-38.5	0.42 (1)	10.00		
K-J	0 / 1164	-38.5	-38.5	0.74 (1)	10.00		
J-I	0 / 0	-38.5	-38.5	0.38 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.71")
CALCULATED VERT. DEFL. (LL) = L/989 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (0.71")
CALCULATED VERT. DEFL. (TL) = L/990 (0.23")

CSI: TC=0.37/1.00 (C-D:1), BC=0.74/1.00 (J-K:1), WB=0.48/1.00 (C-N:1), SS=0.84/1.00 (N-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

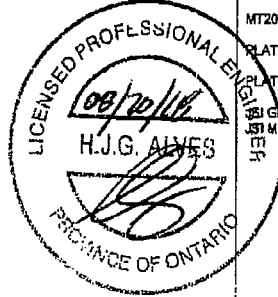
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 816 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.86 (G) (INPUT = 0.90)
JST METAL= 0.48 (G) (INPUT = 1.00)



DRWG NO. TAM 1185795
STRUCTURAL
COMPONENT ONLY



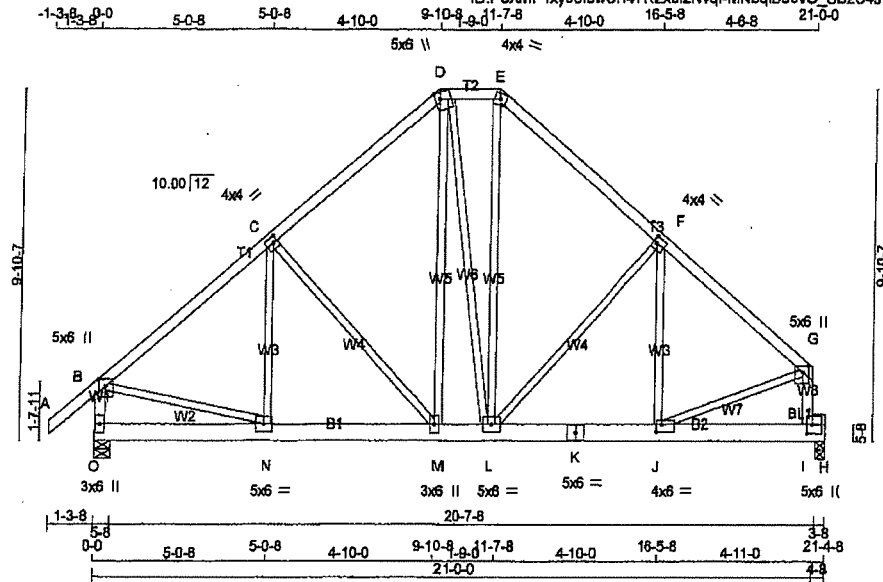
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T143	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 09:48:47 2018 Page 1

ID: P5XmF1xy53IBwCH47R2x3fzNvqfMNBqib0vC_S8zC4JH1EZ7SL4dhaWJSh8c0cAym7J

Scale = 1:58.8



TOTAL WEIGHT = 124 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
O - B	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
D - K	2x6	DRY	No.2	SPF		
K - H	2x6	DRY	No.2	SPF		

BEARING BLOCKS	SIZE	DRY	LUMBER	DESCR.
BL1	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.50 2.25
C	TMWW+t	MT20	4.0	4.0	2.00 1.25
D	TTWW+m	MT20	5.0	8.0	2.25 1.50
E	TTWW-m	MT20	4.0	4.0	
F	TMWW-t	MT20	4.0	4.0	2.00 1.25
G	TMWW+p	MT20	5.0	8.0	2.25 2.00
I	BMV+p	MT20	5.0	8.0	3.00 1.75
J	BMWW-t	MT20	4.0	6.0	2.50 1.75
K	BS-t	MT20	5.0	6.0	
L	BMWWW-t	MT20	5.0	6.0	
M	BMWW+t	MT20	3.0	8.0	
N	BMWW-t	MT20	5.0	8.0	
O	BMV+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	1976	0	1976	0
H	1749	0	1749	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
O	1441	823 / 0 224 / 0 0 / 0 0 / 0 294 / 0 0 / 0
H	1286	789 / 0 224 / 0 0 / 0 0 / 0 273 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	VERT. LOAD	LC1	MAX
FR-TO															
A-B	0 / 57	-129.7	-129.7	0.18 (1)	10.00	N-C	-112 / 160	0.05 (1)							
B-C	-1720 / 0	-128.7	-129.7	0.66 (1)	4.25	C-M	-529 / 0	0.54 (1)							
C-D	-1358 / 0	-129.7	-129.7	0.81 (1)	4.71	M-D	0 / 479	0.11 (1)							
D-E	-1010 / 0	-129.7	-129.7	0.08 (1)	6.14	D-L	0 / 20	0.00 (3)							
E-F	-1356 / 0	-129.7	-129.7	0.55 (1)	4.83	L-E	0 / 486	0.11 (1)							
F-G	-1724 / 0	-129.7	-129.7	0.58 (1)	4.36	L-F	-530 / 0	0.54 (1)							
O-B	-1892 / 0	0.0	0.0	0.20 (1)	8.10	J-F	-70 / 158	0.04 (3)							
I-G	-1913 / 0	0.0	0.0	0.21 (1)	6.07	B-N	0 / 1368	0.31 (1)							
						J-G	0 / 1627	0.37 (1)							
O-N	0 / 0	-38.5	-38.5	0.08 (3)	10.00										
N-M	0 / 1358	-38.5	-38.5	0.22 (1)	10.00										
M-L	0 / 1007	-38.5	-38.5	0.14 (1)	10.00										
L-K	0 / 1361	-38.5	-38.5	0.32 (1)	10.00										
K-J	0 / 1381	-38.5	-38.5	0.32 (1)	10.00										
J-I	-170 / 0	-38.5	-38.5	0.51 (1)	6.25										
I-H	0 / 0	-38.5	-38.5	0.38 (1)	10.00										

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 81.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/969 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/969 (0.11")

CSI: TC=0.65/1.00 (B-C:1), BC=0.81/1.00 (H-J:1), WB=0.54/1.00 (F-L:1), SSI=0.64/1.00 (I-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION UVE LOAD FACTOR = 1.00

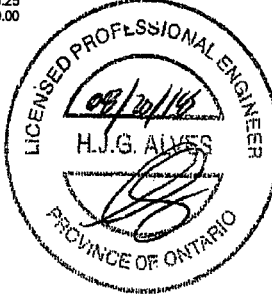
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.28 (J) (INPUT = 1.00)



DWG NO. TAM 11805196
STRUCTURAL
COMPONENT ONLY

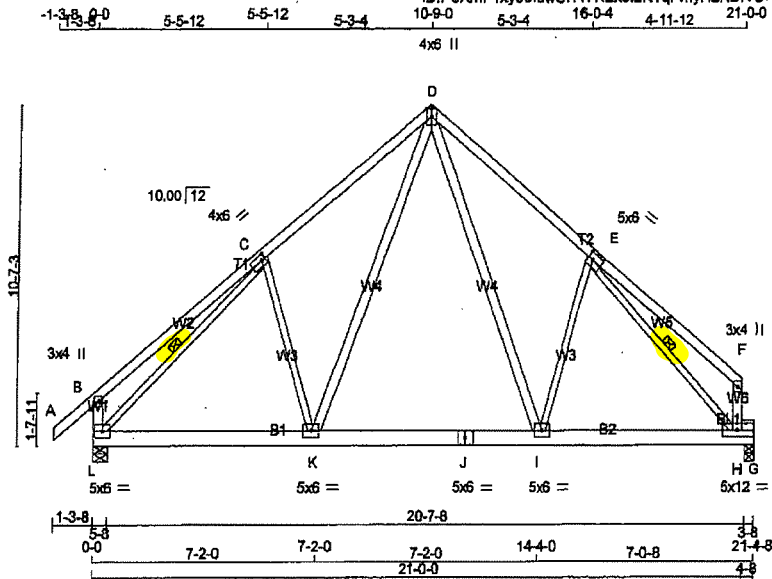


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	T144	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:P5Xmf1xy53l8wCH47R2x3fzNvqf-myHzKDrvc7M12Rxf_RaxBJdxnHc0npXvN8rgCVym7b



Scale = 1:65.2

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2

BEARING BLOCKS

BL1	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - D	2x4	DRY	No.2
D - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0	2.00	1.50
D TTWW+p	MT20	4.0	8.0	Edge	
E TMWW-t	MT20	5.0	8.0	2.50	2.00
F TMV+p	MT20	3.0	4.0		
H BMVW-t	MT20	5.0	12.0		
I BMWW-t	MT20	5.0	6.0		
J BS-t	MT20	5.0	8.0		
K BMWW-t	MT20	5.0	6.0		
L BMWW-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1978	0	1978	0
G	1749	0	1749	0

UNFACTORED REACTIONS

1ST LOASE	MAX /MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
L	1441	923 / 0 224 / 0 0 / 0 0 / 0 294 / 0 0 / 0
G	1286	789 / 0 224 / 0 0 / 0 0 / 0 273 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 57	-128.7 -128.7 0.18 (1)	C-K	-485 / 10 0.28 (1)
B-C	0 / 54	-128.7 -128.7 0.82 (1)	K-D	0 / 748 0.12 (1)
C-D	-1608 / 0	-128.7 -128.7 0.78 (1)	D-I	0 / 807 0.13 (1)
D-E	-1629 / 0	-128.7 -128.7 0.72 (1)	I-E	-275 / 69 0.17 (1)
E-F	0 / 79	-128.7 -128.7 0.57 (1)	L-C	-1943 / 0 0.77 (1)
L-B	-441 / 0	0.0 0.0 0.05 (1)	E-H	-2063 / 0 0.75 (1)
H-F	-213 / 0	0.0 0.0 0.02 (1)		
L-K	0 / 1332	-38.5 -38.5 0.28 (1)		
K-J	0 / 941	-38.5 -38.5 0.35 (1)		
J-I	0 / 941	-38.5 -38.5 0.35 (1)		
I-H	0 / 1298	-38.5 -38.5 0.78 (1)		
H-G	0 / 0	-38.5 -38.5 0.39 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2012, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.71")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.78/1.00 (C-D:1), BC=0.76/1.00 (H-I:1), WB=0.77/1.00 (C-L:1), SSI=0.84/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 788 1967 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (C) (INPUT = 0.90)
JSI METAL=0.53 (E) (INPUT = 1.00)



DRWG NO. TAM 11805197
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 1180519B
STRUCTURAL
COMPONENT ONLY

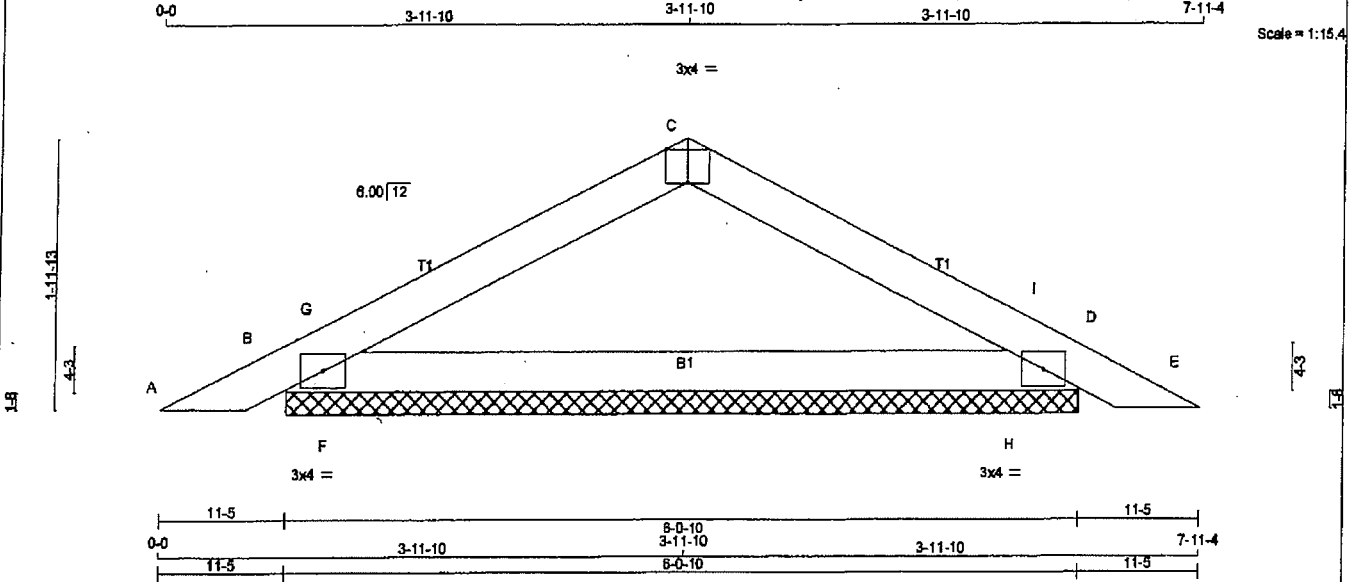
DWG NO. TAM 71805799
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	P1	56	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: P5XmF1xy53l8wCH47R2x3fzNvqf-tZu4zZP9Z73K8qHnAg4uzEz2YZQy2KI?dA3Ueym?kb



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB14	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	623 0	623 0	6-0-10	6-0-10
D	623 0	623 0	6-0-10	6-0-10

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	451	298/0	64/0	0/0	0/0	89/0	0/0
D	451	298/0	64/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0/24	-129.7	-129.7 0.07 (1)	10.00	F-G	0/294	0.00 (1)
B-G	-613/0	-129.7	-129.7 0.16 (2)	8.25	H-I	0/294	0.00 (1)
G-C	-449/0	-129.7	-129.7 0.23 (1)	6.25			
C-I	-449/0	-129.7	-129.7 0.23 (1)	6.25			
I-D	-613/0	-129.7	-129.7 0.16 (2)	8.25			
D-E	0/24	-129.7	-129.7 0.07 (1)	10.00			
B-F	0/417	-38.5	-38.5 0.11 (3)	10.00			
F-H	0/417	-38.5	-38.5 0.23 (1)	10.00			
H-D	0/417	-38.5	-38.5 0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(35 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-I), BC=0.23/1.00 (F-H), WB=0.00/1.00 (F-G), SSI=0.21/1.00 (B-G-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1967 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.45 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

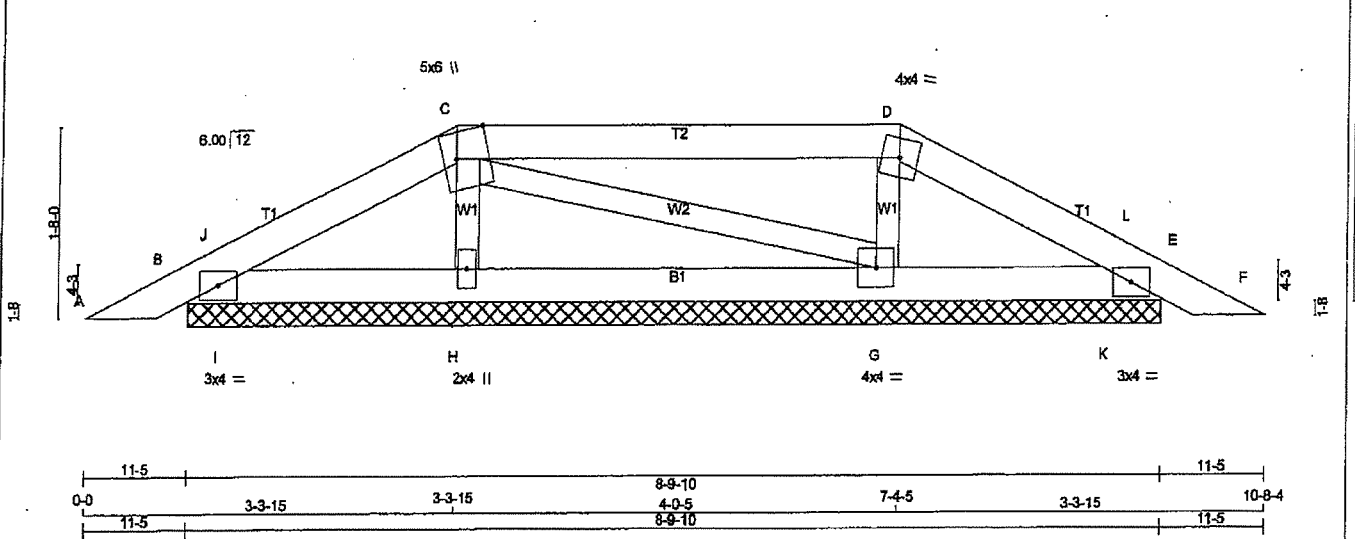


DWG NO. TAM 11809169
STRUCTURAL
COMPONENT ONLY



JOB NAME 297078	TRUSS NAME P21	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.210 S May 18 2018 MiTek Industries, Inc. Mon Aug 20 09:47:08 2018 Page 1
ID: P5XmF1xy5318wCH47R2x3fzNvqfHk8PwLcwDnVVpm8300102pPo4AyYusWuwF8HdPym?kX
Scale = 1:18.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMS1-I	MT20	3.0	4.0
C	TTWW+m	MT20	5.0	8.0 Edge 3.50
D	TTW-m	MT20	4.0	4.0
E	TMB1-I	MT20	3.0	4.0
G	BMWW1-t	MT20	4.0	4.0
H	BMW1+w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	335	0	335	0	0	8-9-10	8-9-10	8-9-10	8-9-10
E	326	0	326	0	0	8-9-10	8-9-10	8-9-10	8-9-10
H	514	0	514	0	0	8-9-10	8-9-10	8-9-10	8-9-10
G	533	0	533	0	0	8-9-10	8-9-10	8-9-10	8-9-10

UNFACTORED REACTIONS							
JT	1ST CASE	DOMINANT	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	238	178 / 0	18 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	230	173 / 0	18 / 0	0 / 0	0 / 0	39 / 0	0 / 0
H	381	222 / 0	75 / 0	0 / 0	0 / 0	85 / 0	0 / 0
G	395	233 / 0	75 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 24	-129.7 -129.7	0.07 (1)	10.00	H-C	-348 / 0	0.05 (1)
B-J	-82 / 0	-129.7 -129.7	0.01 (3)	8.25	C-G	-18 / 0	0.00 (1)
J-C	-81 / 0	-129.7 -129.7	0.09 (1)	8.25	G-D	-362 / 0	0.05 (1)
C-D	-26 / 0	-129.7 -129.7	0.38 (1)	6.25	I-J	-153 / 0	0.00 (1)
D-L	-63 / 0	-129.7 -129.7	0.09 (1)	6.25	K-L	-154 / 0	0.00 (1)
L-E	-63 / 0	-129.7 -129.7	0.01 (3)	8.25			
E-F	0 / 24	-129.7 -129.7	0.07 (1)	10.00			
B-I	0 / 72	-38.5 -38.5	0.09 (1)	10.00			
I-H	0 / 72	-38.5 -38.5	0.09 (1)	10.00			
H-G	0 / 43	-38.5 -38.5	0.09 (2)	10.00			
G-K	0 / 56	-38.5 -38.5	0.09 (1)	10.00			
K-E	0 / 56	-38.5 -38.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2' PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-D:1), BC=0.09/1.00 (B-I:1), WB=0.05/1.00 (D-G:1), SB=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

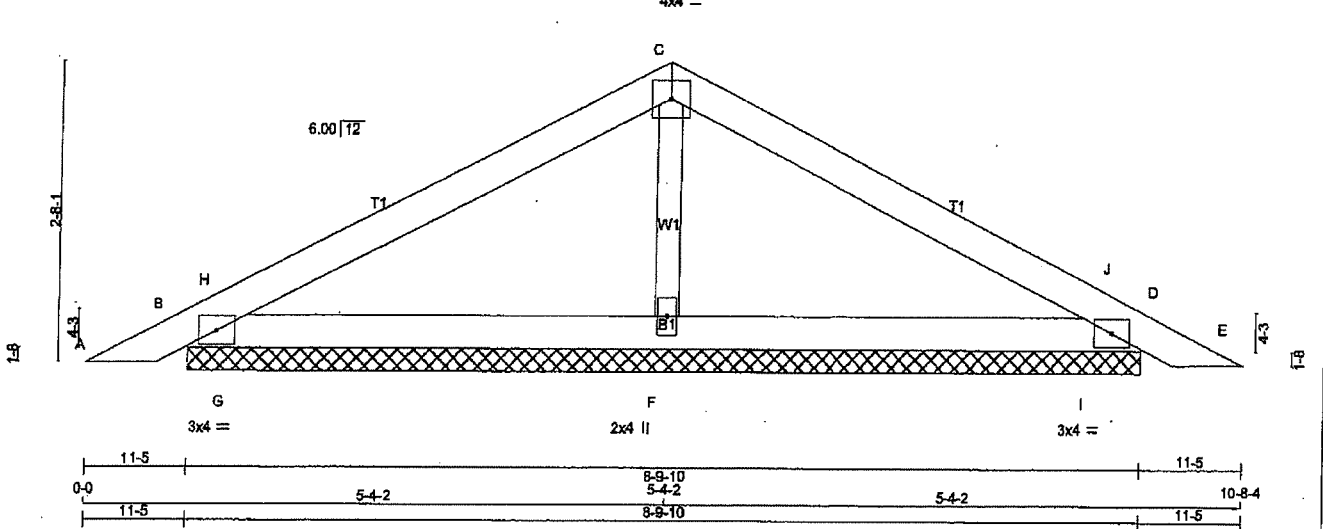
JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

DWG NO. TAM **11605770**
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
297078	P22	8	1	TRUSS DESC.	

Version 6.210 S May 18 2018 Mitek Industries, Inc. Mon Aug 20 09:47:11 2018 Page 1
ID: P5XmF1xy5318wCH47R2x3fzNvgf-AvoXYMeoVt4gDteH8lgR18ANXDS5DHKCDMxEkym?KJ
Tamarack Roof Truss, Burlington
Scale = 1:18.6



TOTAL WEIGHT = 8 X 25 = 202 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-p	MT20	4.0	4.0
O	TMB1-I	MT20	3.0	4.0
F	BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	DOWN	HORIZ	IN-SX	IN-SX
B	525	0	525	0	8-9-10
D	525	0	525	0	8-9-10
F	658	0	658	0	8-9-10

UNFACTORED REACTIONS						
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	375	264/0	42/0	0/0	0/0	69/0
D	375	264/0	42/0	0/0	0/0	69/0
F	491	279/0	101/0	0/0	0/0	111/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	FACTORED	MAX
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX		FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/24	-129.7	-129.7	0.07 (1)	10.00	F-C	-313/0
B-H	-99/0	-129.7	-129.7	0.11 (1)	5.25	G-H	-481/31
H-C	-184/0	-129.7	-129.7	0.30 (1)	8.25	I-J	-481/31
C-J	-184/0	-129.7	-129.7	0.30 (1)	5.25		
J-D	-99/0	-129.7	-129.7	0.11 (1)	5.25		
D-E	0/24	-129.7	-129.7	0.07 (1)	10.00		
B-G	0/166	-38.5	-38.5	0.29 (1)	10.00		
G-F	0/166	-38.5	-38.5	0.29 (1)	10.00		
F-I	0/166	-38.5	-38.5	0.29 (1)	10.00		
I-D	0/166	-38.5	-38.5	0.29 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.2 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2018
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-J:1), BC=0.28/1.00 (D-I:1),

WB=0.05/1.00 (C-F:1), SSI=0.37/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

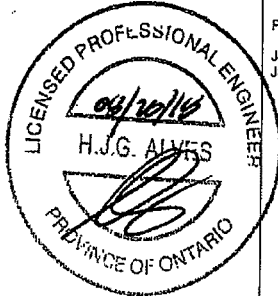
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	518	354	1867 788 1987 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)

JSI METAL = 0.10 (B) (INPUT = 1.00)



DRWG NO. TAM 11805171
STRUCTURAL
COMPONENT ONLY

Simpson Strong-Tie, LLC, 3645 Conestoga Connector, Columbus, OH 43219-1099

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.48 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	GA	Dimensions (in.)					Fasteners		Factored Resistances			
		W	H	B	d _h ^a	Bearing	Joist	S-P-F				
								T ₁	T ₂	U ₁ (K ₁ = 1.75)	Normal (K ₂ = 1.00)	
												lb.
Single 2x Sizes												
SS LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1158	
								267	534			
								360	1020	320	725	
								720	1605	645	1140	
SS LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1830	
								2705	4940	2065	3875	
								2055	4285	1460	4115	
								2685	6825	2685	6700	
SS LUS28	18	1 1/8	5 1/4	3 1/4	4 1/4	(15) 16d	(6) 16d	1140	2185	1020	1550	
								3605	5385	2675	4345	
								3310	7875	3310	6900	
								1140	2495	1020	1770	
SS LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210	
								574	983			

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- Designer must ensure that hanger is compatible with truss when reduced heel height is used.
- d_h is the distance from the bearing seat to the top joist nail.
- Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
- Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

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Simpson Strong-Tie Wood Construction Connectors - Canadian Limited Slotted Design

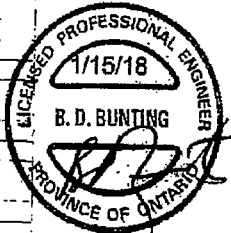
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d ₁	Header	Joist	S-P-F		Uplift (K _U = 1.15) lb. kN	Normal (K _N = 1.00) lb. kN
Double 2x Sizes											
LUS24-2	18	3 1/4	8 1/4	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS28-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(8) 16d	2850	7385	2085	5205
HGUS28-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	8355
LUS28-2	18	3 1/4	7	2	4	(8) 16d	(4) 16d	1720	3325	1545	2575
HHUS28-2	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3785	8940	2875	6345
HGUS28-2	12	3 1/4	7 1/4	4	8 1/4	(28) 16d	(12) 16d	6070	12980	4310	9215
LUS210-2	18	3 1/4	9	2	6	(8) 16d	(8) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 16d	(10) 16d	4670	9680	4235	7000
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS28-3	12	4 1/4	5 1/4	4	4 1/4	(28) 16d	(8) 16d	4385	8950	3110	8355
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(38) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d	4670	9670	4235	6865
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14045	4855	10400
Quadruple 2x Sizes											
HGUS28-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	8355
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(38) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 1/4	8 1/4	3	7 1/4	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(68) 16d	(22) 16d	10130	16400	7195	11845
4x Sizes											
LUS48	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS48	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(8) 16d	2540	7385	2085	5205
HGUS48	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	8355
LUS48	18	3 1/4	8 1/4	2	3 1/4	(8) 16d	(4) 16d	1720	3325	1545	2575
HHUS48	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3785	8940	2875	6345
HGUS48	12	3 1/4	7 1/4	4	8 1/4	(28) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(8) 16d	2580	4500	2320	3195
HGUS410	12	3 1/4	9	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(68) 16d	(22) 16d	10130	16400	7195	11845



Plated Truss Connectors

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See footnotes on p. 268.

TBE – Truss Bearing Enhancers

SIMPSON

Strong-Tie

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

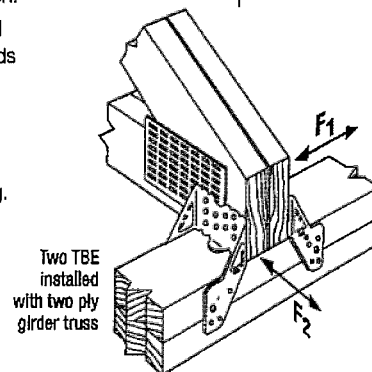
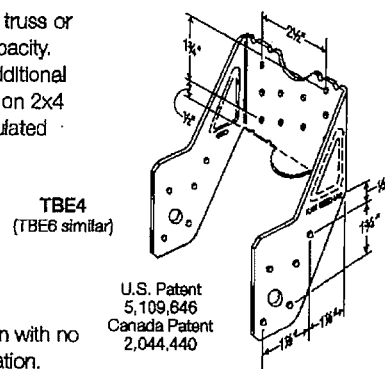
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to withstand the loads imposed by the nails.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.



Model No.	No. of Piles	Fasteners		TBE Only Factored Resistance				Combined TBE and Wood Bearing Factored Resistance	
		Truss	Plate	Uplift ($K_0 = 1.15$)	Normal ($K_0 = 1.00$)	Lateral ($K_0 = 1.15$)		Normal ² ($K_0 = 1.00$)	TBL ⁴
						F ₁	F ₂		
				lb.	lb.	lb.	lb.	lb.	in.
D.Fir-L									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1605	3660	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3660	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3660	655	1415	20705	4.25
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3540	490	1745	10235	8.41
	2	(20) 10d	(20) 10d	1760	3660	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3660	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3660	490	1745	30640	6.29
S-P-F									
TBE4	1	(20) 10d x 1½"	(20) 10d x 1½"	1605	3220	615	1415	6445	6.99
	2	(20) 10d	(20) 10d	1605	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	16345	4.43
TBE6	1	(20) 10d x 1½"	(20) 10d x 1½"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q/A_p and $Q/A'_p = 812$ psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. $TBE4 = 3 \frac{1}{2} \times$ plate width; $TBE6 = 5 \frac{1}{2} \times$ plate width.

LIMIT
STATES
DESIGN

This technical bulletin is subject to change until June 30, 2019, and related information available as of March 1, 2019. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for more information and limited warranty or see simsimpson.com.

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TSPECIBET 317 EXT 619

(800) 999-5099
strongtie.com

TS/LTS/MTS/HTS

SIMPSON
Strong-Tie

Twist Straps

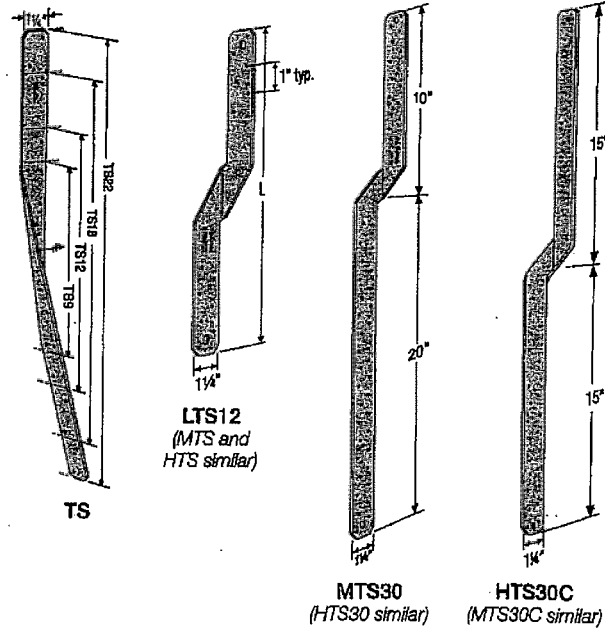
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

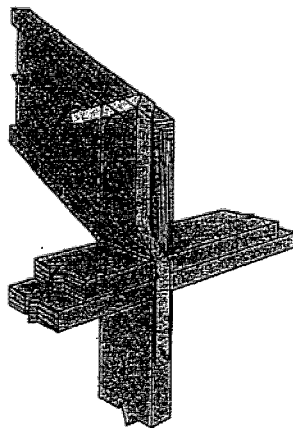
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

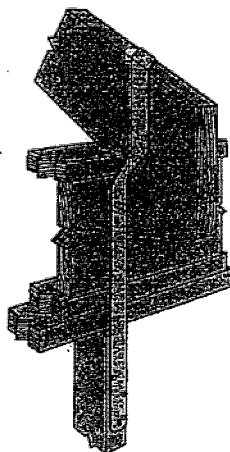
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



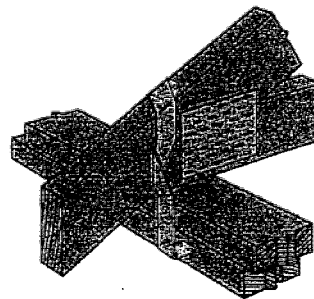
Straps and Ties



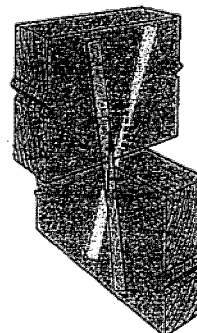
MTS30 Installation
with I-Joist Rafter



Typical MTS30
Installation



MTS Installation as a
Truss-to-Top Plate Tie



Typical TS
Installation

Simpson Strong-Tie® Wood Construction Connectors -- Canadian Limit States Design

TS/LTS/MTS/HTS

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Twist Straps (cont.)

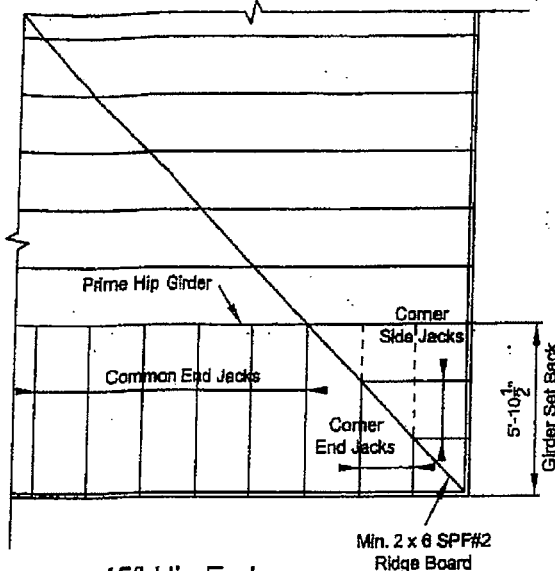
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance ($K_F = 1.15$)	
			D.Hr-L	S-P-F
			lb.	lb.
			kN	kN
TS9	9	(8) 16d	1125	1040
			5.00	4.63
TS12	11½	(10) 16d	1410	1300
			6.27	5.78
TS18	17¾	(14) 16d	1970	1820
			8.76	8.10
TS22	21¾	(18) 16d	2125	2125
			9.45	9.45
SS LTS12	12	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS16	16	(12) 10d x 1½"	1015	720
			4.52	3.20
SS LTS18	18	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS20	20	(12) 10d x 1½"	1015	720
			4.52	3.20
MTS12	12	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS16	18	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS18	18	(14) 10d x 1½"	1570	1180
			6.98	5.25
SS MTS20	20	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS24C	24	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30C	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
HTS16	16	(16) 10d x 1½"	2050	1455
			9.12	6.47
HTS20	20	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS24	24	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS28	28	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30	30	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30C	30	(24) 10d x 1½"	2050	1455
			9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. Nails: 10d = 0.148" dia. x 3' long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27-28 for other nail sizes and information.

Straps and Ties



45° Hip End

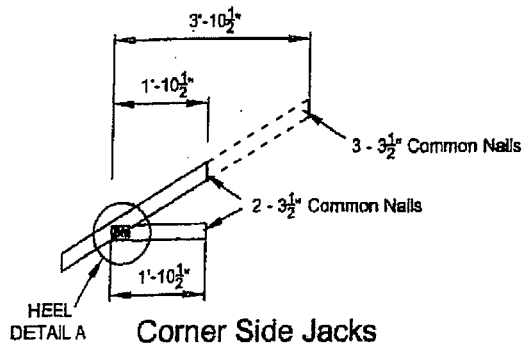
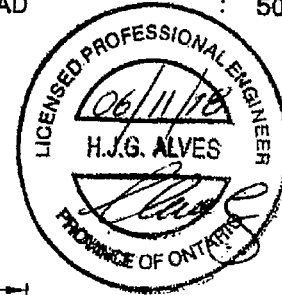
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

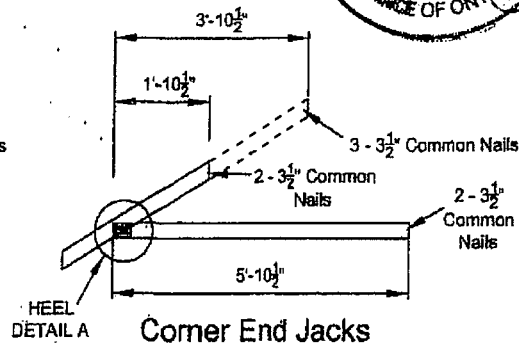
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

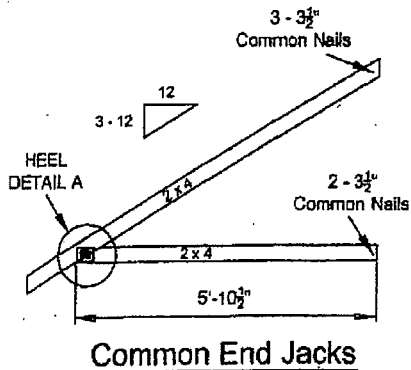
TOTAL LOAD : 50.5 P.S.F



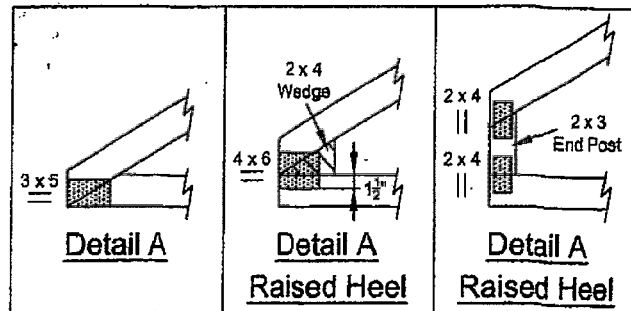
Corner Side Jacks



Corner End Jacks

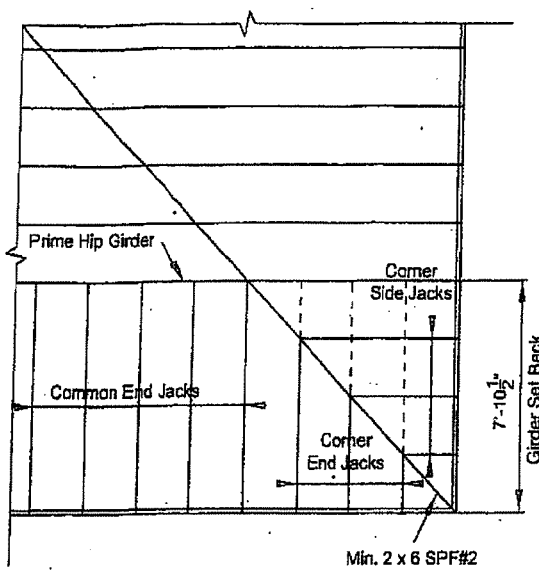


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

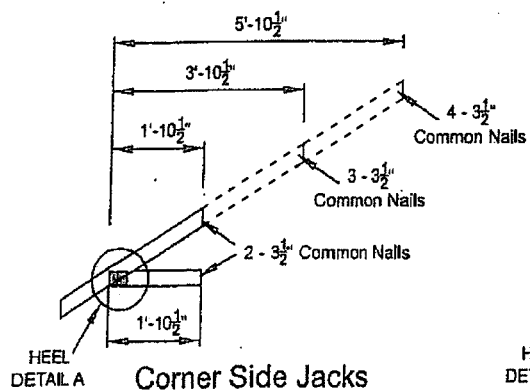
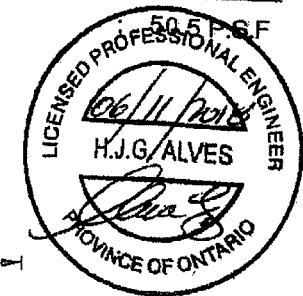
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

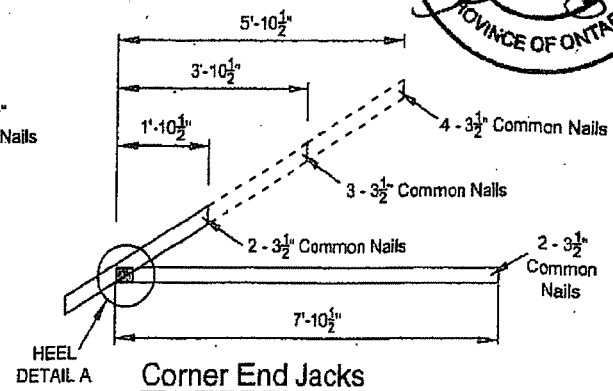
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

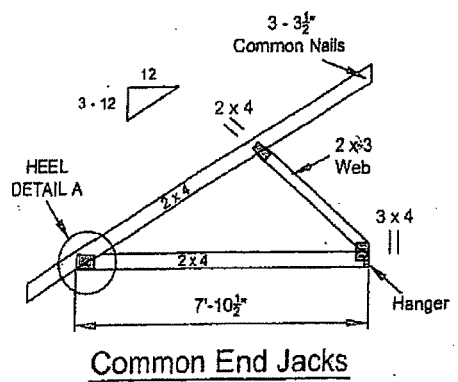
TOTAL LOAD



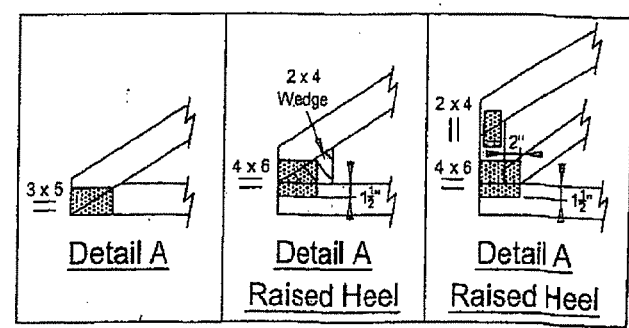
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



ONTARIO WOOD TRUSS
 FABRICATORS ASSOCIATION

TECH-NOTES

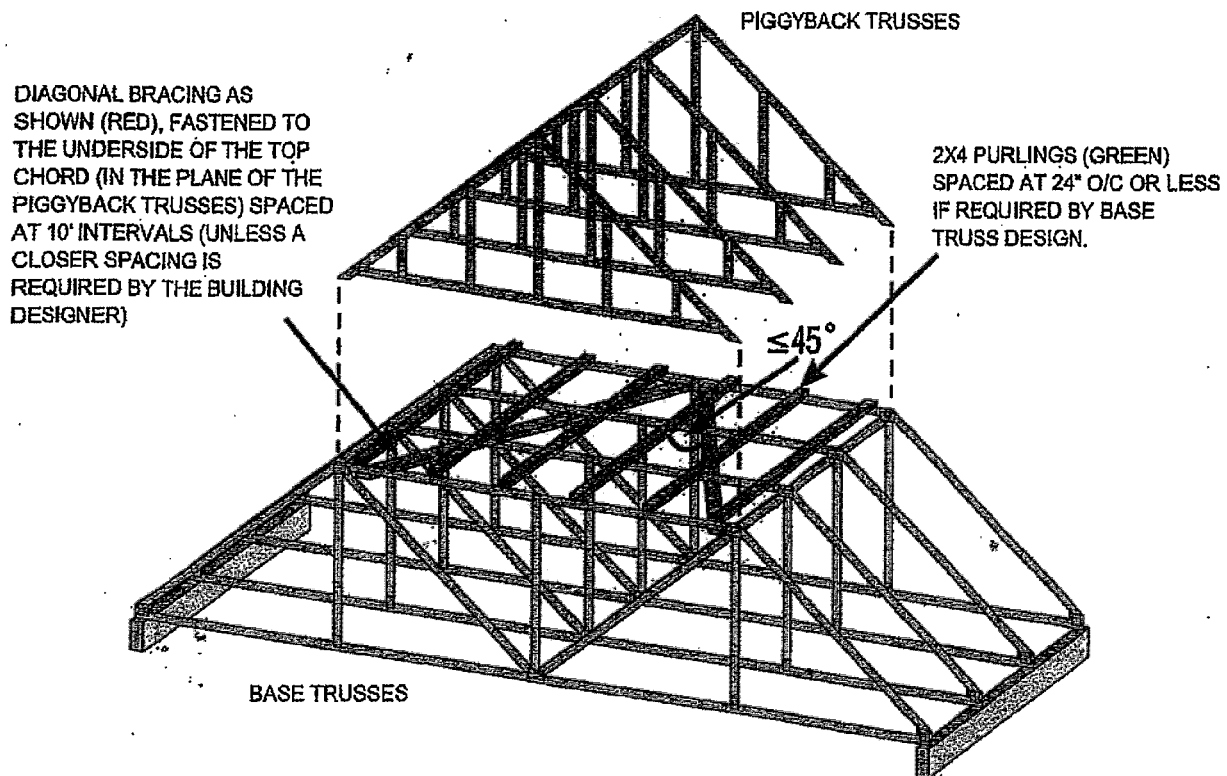
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP
 CHORD OF THE BASE TRUSS AND PIGGYBACK
 TRUSS IN THIS SKETCH IS ASSUMED TO BE
 SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

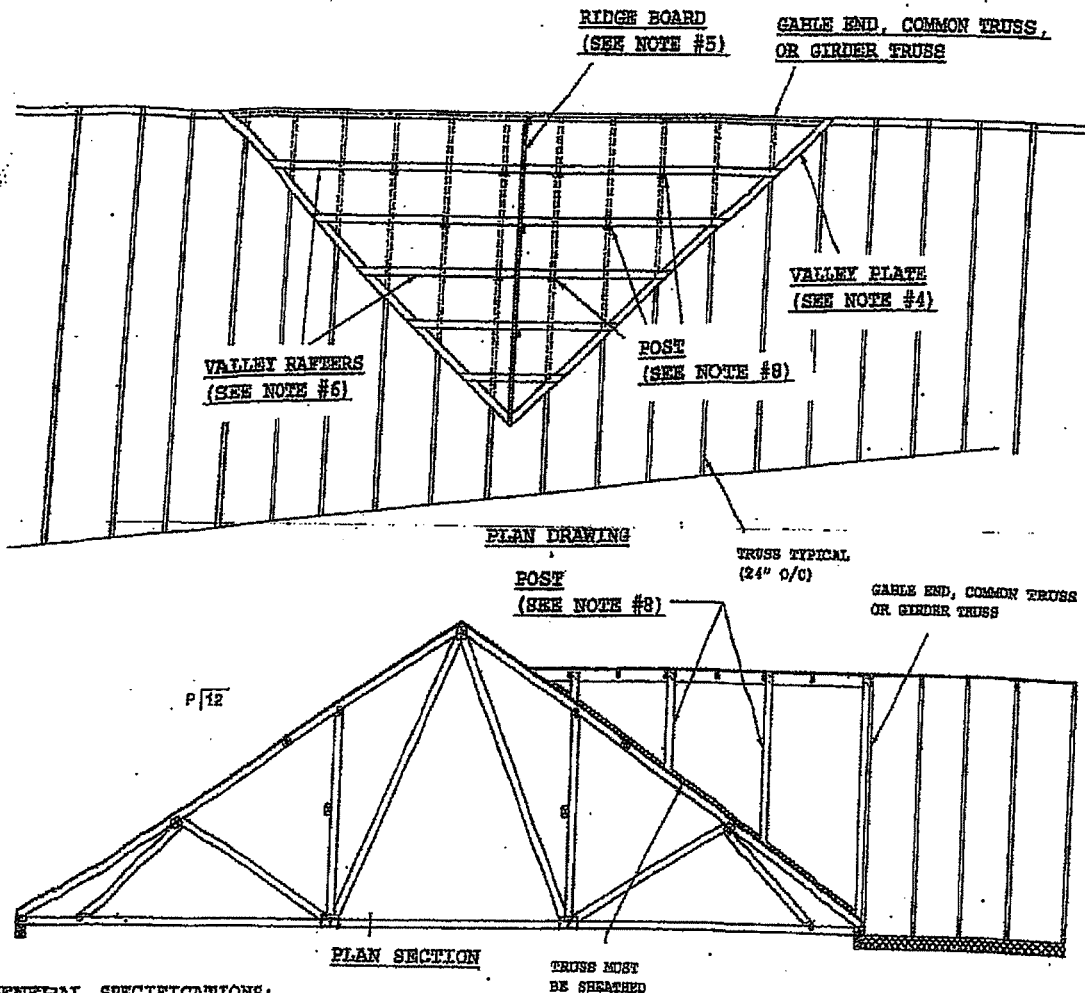
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL

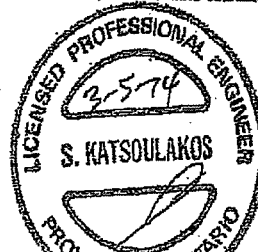


GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) CABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. REVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAIIS.
- (6) BRACE VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAIIS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAIIS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AS ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

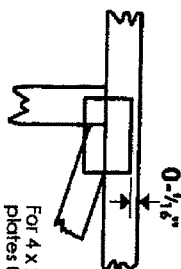


DWG NO T&M 6305.14
 STRUCTURAL
 COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-steeenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

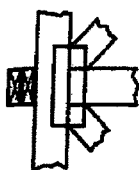
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING



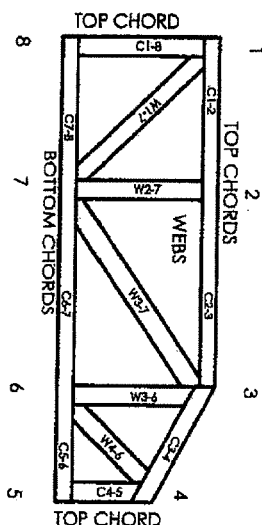
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-steeenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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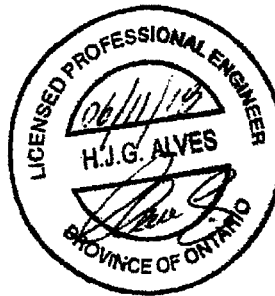
MITEK
POWER TO PERFORM™

Mitek Engineering Reference Sheet: MLE-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative L, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 17% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

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Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018