ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS26-2 -(U)
LUS24 - (O)

NOTE:

ALL B-2-2X10 (FLUSH)

SD25-5C
EL:A (STD/OPT)
286832

SD25-3
EL:A (STD/OPT)(REV)
286834

Center line of wood party wall

10/12 PITCHES (TYP.)
UNLESS NOTED

PLATE HEIGHT 8'-4"-0
U/S OF SOFFIT
@ PLATE LEVEL (A.P.P)

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-10-23
INSPECTOR: BG

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"@16"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'.

DESIGN LOADS:

SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.0 PSF
BC DEAD 7 PSF

FLAT ROOF

PLATE HEIGHT 7'-10"-0
U/S OF SOFFIT
@ PLATE LEVEL (A.P.P)

PRELIMINARY
LAYOUT ONLY
T-170587

DENOTES:
CONVENTIONAL
FRAMING

SITE COPY

M11,436

TAMARACK LUMBER INC. AEPA LUMBER GROUP

Job Track:	44755
Layout ID:	286831
Plan Log:	93746

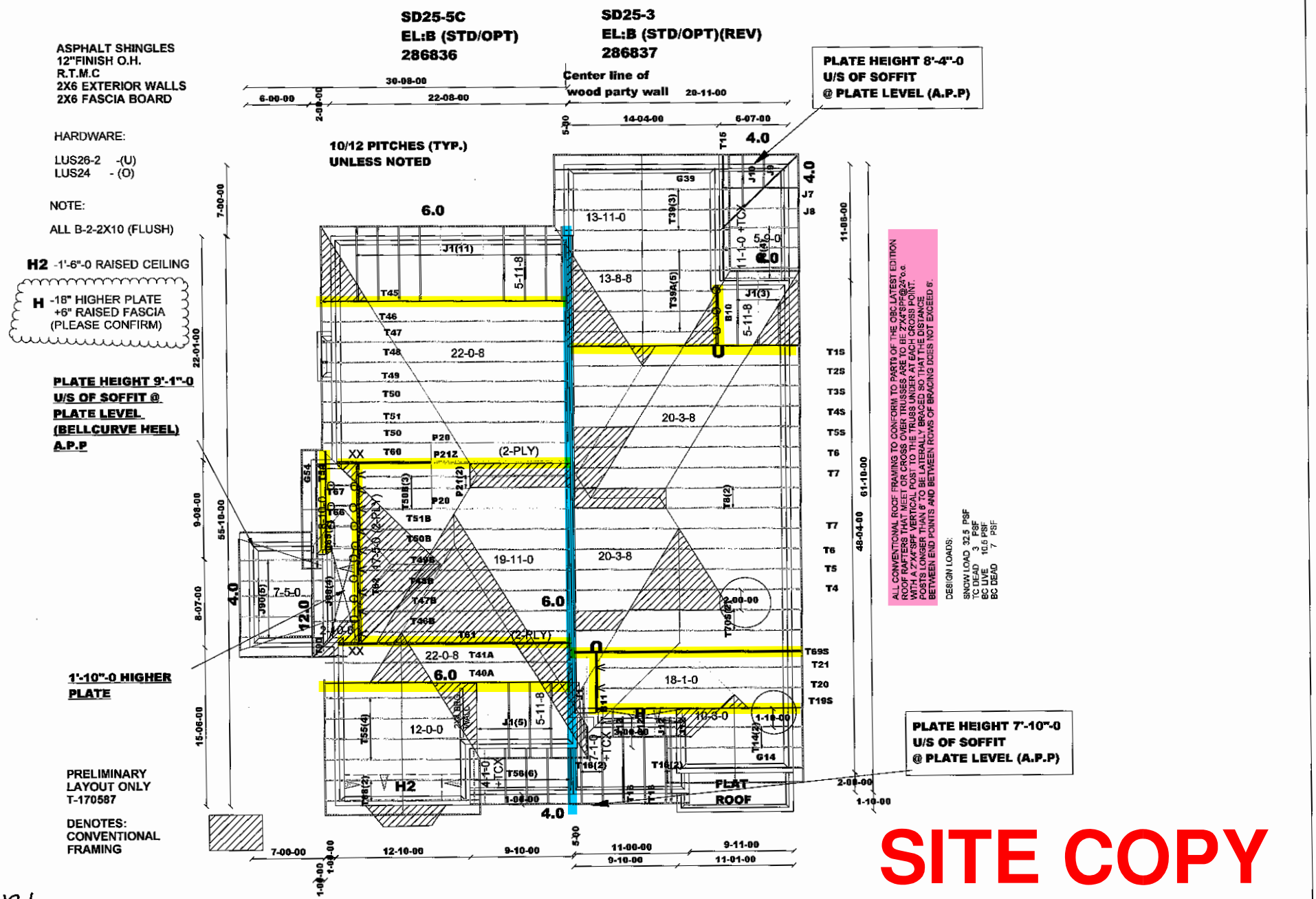
Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: 06-Feb-18 Designer: sonny

Model / Elevation: SD25-5C SONOMA 5/ EL:A(STD/OPT)
PAIRED WITH SD25-3 SONOMA 3 EL:A(STD/OPT.)(REV.)

Mitek ver 7.5.0



SITE COPY

M11,436

TAMARACK LUMBER INC.	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: SD25-5C SONOMA5/ EL:B(STD/OPT) PAIRED WITH SD25-3 SONOMA 3 EL:B(STD/OPT.)(REV.)
	Layout ID: 286835	Project: GREEN VALLEY EAST	
	Plan Log: 93746	Date: 06-Feb-18 Designer: sonny	

Mitek ver 7.5.0

ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS26-2 -(U)
LUS24 - (O)
LJS26DS -(V)

CONFIRM DIM. NOT GIVEN FOR EL:C

NOTE:

ALL B-2-2X10 (FLUSH)

H2 -1'-4"-0 RAISED PLATE

X -11" HIGHER FASCIA ONLY (MINIMUM)

* -STUCCO

PLATE HEIGHT 7'-10"-0 FROM FIN. GR. FLOOR U/S OF SOFFIT @ PLATE LEVEL

PLEASE CONFIRM MINIMUM 11" HIGHER FASCIA RAISED ONLY. IF NOT WILL OVER SHOOT THIS RIDGE

CONFIRM DIM. NOT GIVEN FOR EL:C

PRELIMINARY LAYOUT ONLY T-170587

DENOTES: CONVENTIONAL FRAMING

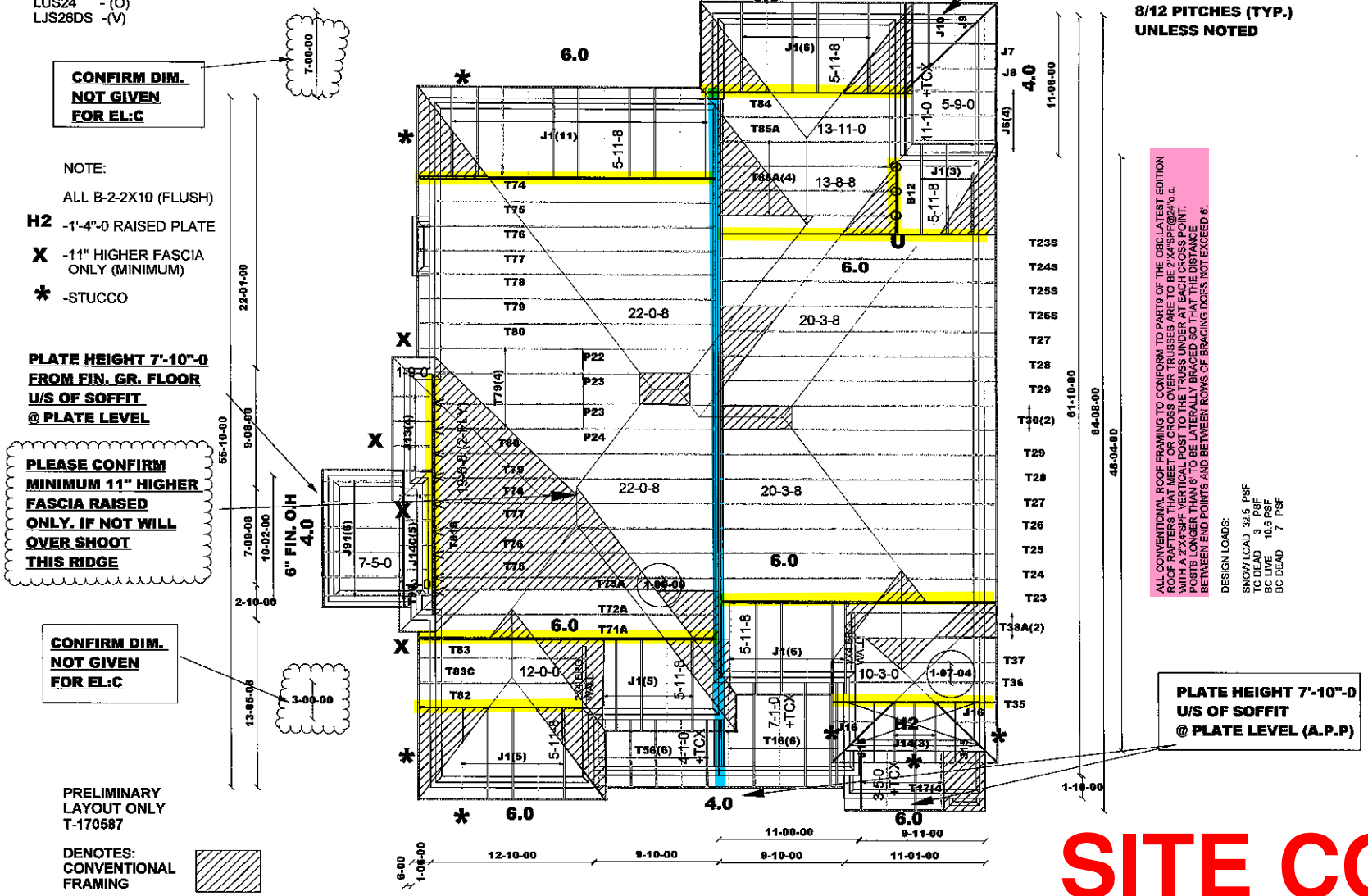
SD25-5C EL:C (STD/OPT) 286839

SD25-3 EL:C (STD/OPT)(REV) 286841

Center line of wood party wall

PLATE HEIGHT 8'-4"-0 U/S OF SOFFIT @ PLATE LEVEL (A.P.P)

8/12 PITCHES (TYP.) UNLESS NOTED



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"@24"o.c.
WITH A 2"X4"@8'- VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	32.5 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

PLATE HEIGHT 7'-10"-0 U/S OF SOFFIT @ PLATE LEVEL (A.P.P)

M11,436

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	286838
Plan Log:	93746

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: 06-Feb-18 Designer: sonny

Model / Elevation: SD25-5C SONOMA5/ EL:C(STD/OPT) PAIRED
SD25-3 SONOMA3 WITH EL:C (STD/OPT.)(REV.)

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

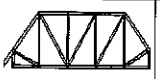
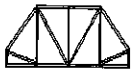
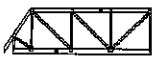
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286832 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-5C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T40A HIP GIRDER	10.00 0.00	22-00-08	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 01-09-12	213.50 137.34		
	1	T41A HIP	10.00 0.00	22-00-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-09-12	95.33 61.67		
	1	T42A HIP	10.00 0.00	22-00-08	06-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-09-12	92.10 59.67		
	1	T43A HIP	10.00 0.00	22-00-08	07-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-09-12	102.84 65.17		
	1	T44A HIP	10.00 0.00	22-00-08	08-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-09-12	103.07 66.33		
	1 2 Ply	T45 HALF HIP	10.00 0.00	22-00-08	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-12	201.04 124.66		
	1	T46 HALF HIP	10.00 0.00	22-00-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-12	92.71 59.33		
	1	T47 HALF HIP	10.00 0.00	22-00-08	06-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-12	98.59 62.67		
	2	T48 HALF HIP	10.00 0.00	22-00-08	07-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-12	215.10 136.66		
	2	T49 HALF HIP	10.00 0.00	22-00-08	08-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-12	224.48 141.34		
	7	T50 PIGGYBACK	10.00 0.00	22-00-08	09-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-12	842.17 525.00		
	2	T51 HALF HIP	10.00 0.00	22-00-08	10-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-12	253.00 158.34		
	1 2 Ply	T52 COMMON	10.00 0.00	17-05-00	08-10-12	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 01-07-11	236.94 143.34		
	1	T53 COMMON	10.00 0.00	17-05-00	08-10-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	85.11 54.33		
	5	T55 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	290.55 185.00		
	6	T56 MONOPITCH	4.00 0.00	04-01-00	02-06-10	2 X 4	2 X 4	01-03-08 00-09-04	00-11-03 02-06-10	115.20 82.02		
	1	G54 COMMON	10.00 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	42.96 28.83		
	1	G55 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.38 38.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

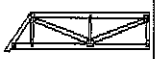
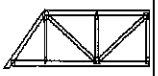
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286832 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-5C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P20 PIGGYBACK	10.00 0.00	12-04-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	83.52 56.00		
	3	P21 PIGGYBACK	10.00 0.00	12-04-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	133.89 86.01		
	16	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	271.84 170.72		

TOTAL # TRUSS= **60.00** TOTAL BFT OF ALL TRUSSES= **2442.43** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3852.32** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	LJS26DS	

TOTAL # ITEMS= **9.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

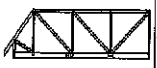
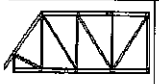
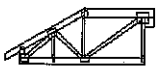
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286834 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	20-03-08	06-04-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 06-04-12	226.42 142.00		
	1 2 Ply	T1Z HALF HIP	10.00 0.00	20-03-08	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-12	188.34 120.00		
	1	T2 HALF HIP	10.00 0.00	20-03-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-12	87.64 55.83		
	1	T2S ROOF	10.00 0.00	20-03-08	07-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-04-12	102.06 65.33		
	1	T3 HALF HIP	10.00 0.00	20-03-08	06-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-12	93.66 59.33		
	1	T3S ROOF	10.00 0.00	20-03-08	08-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-04-12	104.49 66.50		
	1	T4 HALF HIP	10.00 0.00	20-03-08	07-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-12	93.32 58.67		
	1	T4S ROOF	10.00 0.00	20-03-08	09-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-04-12	114.62 72.83		
	1	T5 HALF HIP	10.00 0.00	20-03-08	08-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-12	100.74 63.33		
	1	T5S ROOF	10.00 0.00	20-03-08	10-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-04-12	125.88 80.00		
	2	T6 HALF HIP	10.00 0.00	20-03-08	09-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-12	230.42 144.66		
	2	T7 HALF HIP	10.00 0.00	20-03-08	10-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-12	243.14 151.66		
	2	T8 HALF HIP	10.00 0.00	20-03-08	11-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 11-01-12	263.12 162.34		
	5	T14 COMMON	10.00 0.00	10-03-00	05-10-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	231.35 150.00		
	1	T15 HALF HIP	4.00 0.00	11-01-00	02-10-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-10-03	48.93 32.83		
	6	T16 MONOPITCH	4.00 0.00	07-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-10-08	00-11-03 03-07-00	192.60 129.00		
	5	T17 MONOPITCH	6.00 0.00	03-05-00	03-03-12	2 X 4	2 X 4	01-03-08 00-10-08	01-02-00 02-10-08	100.35 68.35		
	3	T39 COMMON	10.00 0.00	13-11-00	07-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	197.34 126.51		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

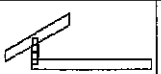
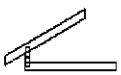
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286834 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T39A COMMON	10.00 0.00	13-08-08	07-05-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-09-12	317.00 205.00		
	1	G14 COMMON	10.00 0.00	10-03-00	05-10-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	49.87 33.00		
	1	G39 COMMON	10.00 0.00	13-11-00	07-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	68.63 44.17		
	9	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	152.91 96.03		
	4	J6 JACK-OPEN	4.00 0.00	05-09-00	02-10-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-10-03	62.52 40.00		
	1	J7 JACK-OPEN	4.00 0.00	05-09-00	01-06-05	2 X 4	2 X 4	01-03-08 -03-11-09	00-11-03 00-03-08	10.86 7.33		
	1	J8 JACK-OPEN	4.00 0.00	05-09-00	02-02-05	2 X 4	2 X 4	01-03-08 -01-11-09	00-11-03 00-03-08	13.26 8.67		
	1	J9 JACK-OPEN	4.00 0.00	01-10-08	01-06-05	2 X 4	2 X 4	01-03-08 -00-01-01	00-11-03 00-03-08	6.44 4.67		
	1	J10 JACK-OPEN	4.00 0.00	01-10-08	02-02-05	2 X 4	2 X 4	01-03-08 01-10-15	00-11-03 00-04-03	8.84 6.00		

TOTAL # TRUSS= **62.00** TOTAL BFT OF ALL TRUSSES= **2194.04** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3434.75** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	A23	
1	Hangers	HGUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

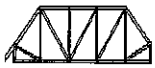
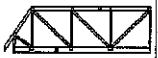
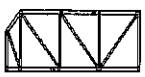
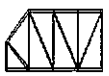
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 286836 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: SD25-5C **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T40A HIP GIRDER	10.00 0.00	22-00-08	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 01-09-12	213.50 137.34		
	1	T41A HIP	10.00 0.00	22-00-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-09-12	95.33 61.67		
	1 2 Ply	T45 HALF HIP	10.00 0.00	22-00-08	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-12	201.04 124.66		
	1	T46 HALF HIP	10.00 0.00	22-00-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-12	92.71 59.33		
	1	T46B HALF HIP	10.00 0.00	19-11-00	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 05-01-12	86.74 54.17		
	1	T47 HALF HIP	10.00 0.00	22-00-08	06-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-12	98.59 62.67		
	1	T47B HALF HIP	10.00 0.00	19-11-00	06-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 06-01-12	92.28 57.50		
	2	T48 HALF HIP	10.00 0.00	22-00-08	07-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-12	215.10 136.66		
	1	T48B HALF HIP	10.00 0.00	19-11-00	07-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 07-01-12	100.37 63.50		
	2	T49 HALF HIP	10.00 0.00	22-00-08	08-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-12	224.48 141.34		
	1	T49B HALF HIP	10.00 0.00	19-11-00	08-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 08-01-12	107.01 66.00		
	7	T50 PIGGYBACK	10.00 0.00	22-00-08	09-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-12	842.17 525.00		
	4	T50B PIGGYBACK	10.00 0.00	19-11-00	09-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 09-01-12	450.48 278.00		
	2	T51 HALF HIP	10.00 0.00	22-00-08	10-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-12	253.00 158.34		
	1	T51B HALF HIP	10.00 0.00	19-11-00	10-01-12	2 X 4	2 X 4	00-00-00 00-00-00	03-04-15 10-01-12	121.68 75.00		
	1	T54 COMMON	10.00 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	40.96 26.83		
	5	T55 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	290.55 185.00		
	6	T56 MONOPITCH	4.00 0.00	04-01-00	02-06-10	2 X 4	2 X 4	01-03-08 00-09-04	00-11-03 02-06-10	115.20 82.02		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

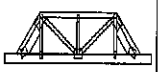
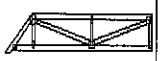
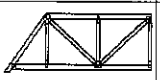
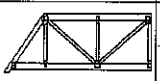
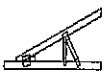
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286836 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-5C ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T60 PIGGYBACK	10.00 0.00	22-00-08	09-01-12	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 09-01-12	274.04 172.66		
	1 2 Ply	T61 HIP GIRDER	10.00 0.00	22-00-08	06-00-08	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 01-09-12	232.22 148.66		
	1 2 Ply	T62 HIP GIRDER	10.00 0.00	17-05-00	06-07-00	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 03-05-11	204.22 129.34		
	1	T66 FLAT	0.00 0.00	02-10-08	04-11-11	2 X 4	2 X 4	00-00-00 00-00-00	04-11-11 04-11-11	20.60 13.17		
	1	T67 FLAT	0.00 0.00	02-10-08	03-03-11	2 X 4	2 X 4	00-00-00 00-00-00	03-03-11 03-03-11	15.67 10.00		
	2	T68 ROOF	10.00 10.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	115.52 74.34		
	1	T90 HIP GIRDER	12.00 0.00	09-05-00	02-05-00	2 X 4	2 X 6	01-03-08 01-03-08	00-05-08 00-05-08	52.33 33.67		
	2	P20 PIGGYBACK	10.00 0.00	12-04-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	83.52 56.00		
	2	P21 PIGGYBACK	10.00 0.00	12-04-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	89.26 57.34		
	1 2 Ply	P21Z PIGGYBACK	10.00 0.00	12-04-14	02-09-12	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-09-12	88.86 57.34		
	16	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	271.84 170.72		
	2	J65 JACK-OPEN	12.00 0.00	02-10-08	06-07-00	2 X 4	2 X 4	00-00-00 00-00-00	03-08-08 06-07-00	30.86 21.00		
	4	J66 JACK-OPEN	12.00 0.00	02-07-00	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	83.68 56.68		
	5	J90 JACK-OPEN	4.00 0.00	05-05-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 02-05-00	88.80 60.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3355.95** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5292.61** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
12	Hangers	LUS24	

TOTAL # ITEMS= **23.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

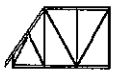
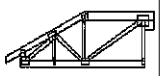
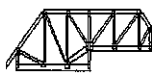
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286837 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S ROOF	10.00 0.00	20-03-08	06-04-12	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 06-04-12	226.42 142.00		
	1	T2S ROOF	10.00 0.00	20-03-08	07-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-04-12	102.06 65.33		
	1	T3S ROOF	10.00 0.00	20-03-08	08-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-04-12	104.49 66.50		
	1	T4 HALF HIP	10.00 0.00	20-03-08	07-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-12	93.32 58.67		
	1	T4S ROOF	10.00 0.00	20-03-08	09-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-04-12	114.62 72.83		
	1	T5 HALF HIP	10.00 0.00	20-03-08	08-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-12	100.74 63.33		
	1	T5S ROOF	10.00 0.00	20-03-08	10-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-04-12	125.88 80.00		
	2	T6 HALF HIP	10.00 0.00	20-03-08	09-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-12	230.42 144.66		
	2	T7 HALF HIP	10.00 0.00	20-03-08	10-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-12	243.14 151.66		
	2	T8 HALF HIP	10.00 0.00	20-03-08	11-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 11-01-12	263.12 162.34		
	5	T14 COMMON	10.00 0.00	10-03-00	05-10-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	231.35 150.00		
	1	T15 HALF HIP	4.00 0.00	11-01-00	02-10-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-10-03	48.93 32.83		
	6	T16 MONOPITCH	4.00 0.00	07-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-10-08	00-11-03 03-07-00	192.60 129.00		
	1	T19S HIP GIRDER	10.00 0.00	18-04-00	04-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 01-11-03	99.56 65.83		
	1	T20 HIP	10.00 0.00	18-01-00	06-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-11	88.51 57.83		
	1	T21 HIP	10.00 0.00	18-01-00	08-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-07-11	86.66 55.50		
	3	T39 COMMON	10.00 0.00	13-11-00	07-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	197.34 126.51		
	5	T39A COMMON	10.00 0.00	13-08-08	07-05-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-09-12	317.00 205.00		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

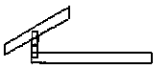
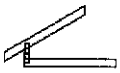
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286837 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T69S ROOF	10.00 0.00	20-03-08	09-05-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-12	254.06 160.66		
	2	T70S ROOF	10.00 0.00	20-03-08	10-02-02	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-12	207.68 133.34		
	1	G14 COMMON	10.00 0.00	10-03-00	05-10-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	49.87 33.00		
	1	G39 COMMON	10.00 0.00	13-11-00	07-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	68.63 44.17		
	4	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	67.96 42.68		
	4	J6 JACK-OPEN	4.00 0.00	05-09-00	02-10-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-10-03	62.52 40.00		
	1	J7 JACK-OPEN	4.00 0.00	05-09-00	01-06-05	2 X 4	2 X 4	01-03-08 -03-11-09	00-11-03 00-03-08	10.86 7.33		
	1	J8 JACK-OPEN	4.00 0.00	05-09-00	02-02-05	2 X 4	2 X 4	01-03-08 -01-11-09	00-11-03 00-03-08	13.26 8.67		
	1	J9 JACK-OPEN	4.00 0.00	01-10-08	01-06-05	2 X 4	2 X 4	01-03-08 -00-01-01	00-11-03 00-03-08	6.44 4.67		
	1	J10 JACK-OPEN	4.00 0.00	01-10-08	02-02-05	2 X 4	2 X 4	01-03-08 01-10-15	00-11-03 00-04-03	8.84 6.00		
	4	J12 JACK-OPEN	10.00 0.00	01-04-00	03-03-00	2 X 4	2 X 4	01-03-08 00-00-00	02-01-11 03-03-00	30.60 21.32		

TOTAL # TRUSS= **59.00** TOTAL BFT OF ALL TRUSSES= **2331.66** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3646.88** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	
3	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

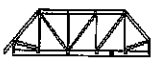
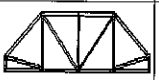
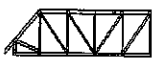
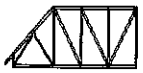
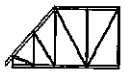
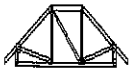
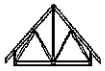
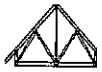
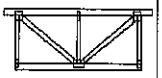
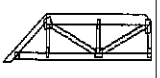
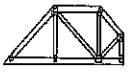
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286839 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-5C ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	T56 MONOPITCH	4.00 / 0.00	04-01-00	02-06-10	2 X 4	2 X 4	01-03-08 / 00-09-04	00-11-03 / 02-03-08	117.18 / 79.98		
	1 / 2 Ply	T71A HIP GIRDER	8.00 / 0.00	22-00-08	04-01-12	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 01-06-08	202.98 / 126.66		
	1	T72A HIP	8.00 / 0.00	22-00-08	05-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-13 / 01-06-08	86.67 / 55.17		
	1	T73A HIP	8.00 / 0.00	22-00-08	06-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-13 / 01-06-08	95.76 / 61.33		
	1 / 2 Ply	T74 HALF HIP	8.00 / 0.00	22-00-08	04-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-01-12	183.64 / 117.34		
	2	T75 HALF HIP	8.00 / 0.00	22-00-08	05-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-12	184.50 / 115.34		
	2	T76 HALF HIP	8.00 / 0.00	22-00-08	06-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-12	200.46 / 126.66		
	2	T77 HALF HIP	8.00 / 0.00	22-00-08	07-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-01-12	202.90 / 128.66		
	2	T78 HALF HIP	8.00 / 0.00	22-00-08	08-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-01-12	219.00 / 135.66		
	6	T79 HALF HIP	8.00 / 0.00	22-00-08	09-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 09-01-12	677.76 / 420.00		
	2	T80 HALF HIP	8.00 / 0.00	22-00-08	10-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-13 / 10-01-12	248.12 / 153.34		
	1 / 2 Ply	T81S ROOF	8.00 / 0.00	19-05-08	03-05-13	2 X 6	2 X 6	00-00-00 / 00-00-00	02-03-13 / 02-03-13	223.36 / 145.34		
	1	T82 HIP GIRDER	8.00 / 0.00	12-00-00	04-01-12	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	52.33 / 33.17		
	1	T83 COMMON	8.00 / 0.00	12-00-00	05-04-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	53.00 / 31.83		
	1	T83C HIP	8.00 / 0.00	12-00-00	05-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 01-04-13	50.65 / 32.17		
	1	T90 FLAT GIRDER	0.00 / 0.00	09-04-00	02-07-00	2 X 4	2 X 4	00-09-08 / 00-09-08	02-07-00 / 02-07-00	36.97 / 22.67		
	1	P22 PIGGYBACK	8.00 / 0.00	09-08-03	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 01-06-09	37.33 / 24.67		
	2	P23 PIGGYBACK	8.00 / 0.00	10-02-07	02-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-04 / 01-06-09	68.88 / 47.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

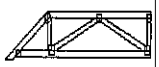
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286839 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-5C ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P24 PIGGYBACK	8.00 0.00	09-08-03	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	32.52 21.33		
	21	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	356.79 224.07		
	4	J13 JACK-OPEN	8.00 0.00	01-09-00	03-05-13	2 X 4	2 X 4	01-03-08 00-00-00	02-03-13 03-05-13	41.04 27.32		
	5	J14C JACK-OPEN	8.00 0.00	01-03-00	03-01-13	2 X 4	2 X 4	01-03-08 00-00-00	02-03-13 03-01-13	36.30 26.65		
	6	J91 JACK-OPEN	4.00 0.00	05-05-08	02-07-00	2 X 4	2 X 4	00-09-08 00-00-00	00-09-03 02-07-00	84.90 55.98		

TOTAL # TRUSS= **74.00** TOTAL BFT OF ALL TRUSSES= **2212.68** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3493.04** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	LJS26DS	

TOTAL # ITEMS= **9.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

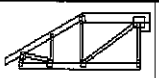
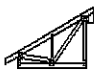
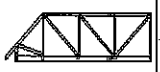
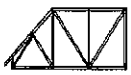
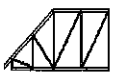
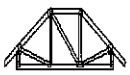
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 286841 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: SD25-3 **ELEVATION:** C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T15 HALF HIP	4.00 / 0.00	11-01-00	02-10-03	2 X 4	2 X 4	01-03-08 / 00-10-08	00-11-03 / 02-10-03	46.31 / 31.00		
	6	T16 MONOPITCH	4.00 / 0.00	07-01-00	03-06-10	2 X 4	2 X 4	01-03-08 / 00-09-04	00-11-03 / 03-06-10	190.38 / 129.00		
	4	T17 MONOPITCH	6.00 / 0.00	03-05-00	03-03-02	2 X 4	2 X 4	01-03-08 / 00-09-04	01-02-00 / 03-03-02	78.48 / 54.68		
	1 / 2 Ply	T23 HALF HIP	8.00 / 0.00	20-03-08	04-01-12	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 04-01-12	188.44 / 118.34		
	1	T23S ROOF	8.00 / 0.00	20-03-08	06-04-12	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 06-04-12	115.30 / 72.83		
	1	T24 HALF HIP	8.00 / 0.00	20-03-08	05-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-12	87.22 / 55.00		
	1	T24S ROOF	8.00 / 0.00	20-03-08	07-04-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-04-12	98.59 / 62.67		
	1	T25 HALF HIP	8.00 / 0.00	20-03-08	06-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-12	89.34 / 56.33		
	1	T25S ROOF	8.00 / 0.00	20-03-08	08-04-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-04-12	108.36 / 69.17		
	1	T26 HALF HIP	8.00 / 0.00	20-03-08	07-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-01-12	92.31 / 58.33		
	1	T26S ROOF	8.00 / 0.00	20-03-08	09-04-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 09-04-12	116.26 / 74.17		
	2	T27 HALF HIP	8.00 / 0.00	20-03-08	08-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-01-12	194.86 / 121.66		
	2	T28 HALF HIP	8.00 / 0.00	20-03-08	09-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 09-01-12	205.60 / 128.66		
	2	T29 HALF HIP	8.00 / 0.00	20-03-08	10-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 10-01-12	220.68 / 138.66		
	2	T30 HALF HIP	8.00 / 0.00	20-03-08	11-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 11-01-12	216.18 / 133.34		
	1	T35 HIP GIRDER	8.00 / 0.00	10-03-00	03-08-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	45.08 / 29.83		
	1	T36 COMMON	8.00 / 0.00	10-03-00	06-01-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-08-13 / 02-08-13	47.98 / 31.00		
	1	T37 HIP	8.00 / 0.00	10-03-00	05-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	02-08-13 / 02-08-13	52.13 / 34.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

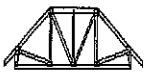
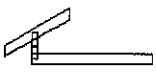
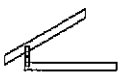
Delivery Shiplist

DATE	02/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 286841 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: SD25-3 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T38A COMMON	8.00 0.00	10-03-00	05-05-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-08-13	91.38 60.34		
	1	T84 HIP GIRDER	8.00 0.00	13-11-00	04-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.03 39.00		
	1	T85A HIP	8.00 0.00	13-08-08	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	58.60 38.00		
	4	T86A COMMON	8.00 0.00	13-08-08	06-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	231.76 150.68		
	15	J1 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	254.85 160.05		
	4	J6 JACK-OPEN	4.00 0.00	05-09-00	02-10-03	2 X 4	2 X 4	01-03-08 00-00-00	00-11-03 02-10-03	62.52 40.00		
	1	J7 JACK-OPEN	4.00 0.00	05-09-00	01-06-05	2 X 4	2 X 4	01-03-08 -03-11-09	00-11-03 00-03-08	10.86 7.33		
	1	J8 JACK-OPEN	4.00 0.00	05-09-00	02-02-05	2 X 4	2 X 4	01-03-08 -01-11-09	00-11-03 00-03-08	13.26 8.67		
	1	J9 JACK-OPEN	4.00 0.00	01-10-08	01-06-05	2 X 4	2 X 4	01-03-08 -00-01-01	00-11-03 00-03-08	6.44 4.67		
	1	J10 JACK-OPEN	4.00 0.00	01-10-08	02-02-05	2 X 4	2 X 4	01-03-08 01-10-15	00-11-03 00-04-03	8.84 6.00		
	3	J14 JACK-OPEN	8.00 0.00	03-05-08	03-08-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-08-08	39.63 24.99		
	2	J15 JACK-OPEN	8.00 0.00	03-05-08	02-07-02	2 X 4	2 X 4	01-03-08 -01-08-01	01-04-13 00-03-08	18.94 12.00		
	2	J16 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	15.34 9.34		

TOTAL # TRUSS= **69.00** TOTAL BFT OF ALL TRUSSES= **1959.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3067.95** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

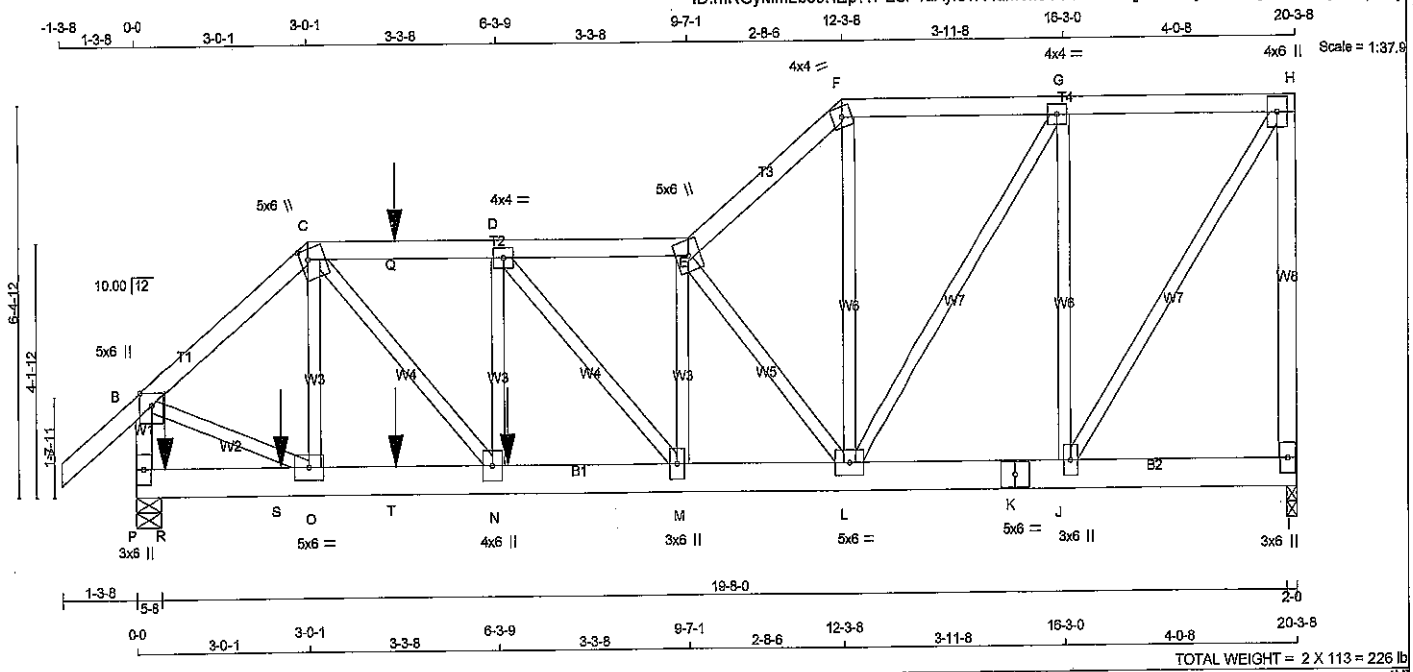
TOTAL # ITEMS= **4.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Thu Feb 1 09:43:22 2018 Page 1

ID:mRGyMmLb3JNZp??PESP4uAyieTX-ldm1h3VcaEsH?Vqjz9Cza3jHkRGNyXa4dVZy2EzpZ7p



TOTAL WEIGHT = 2 X 113 = 226 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
A- C	1	12		TOP
C- E	1	12		SIDE(0.0)
E- F	1	12		TOP
F- H	1	12		TOP
H- I	1	12		TOP
P- B	1	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
P- K	2	12		SIDE(183.1)
K- I	2	12		TOP
WEBS : (0.122"X3") SPIRAL NAILS				
D- N	1	6		SIDE(177.7)
2x3	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW+t	MT20	3.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1822	0	1822	0	0	2-0	2-0
P	2662	0	2662	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1452	891 / 0	267 / 0	0 / 0	0 / 0	274 / 0	0 / 0
P	2134	1287 / 0	437 / 0	0 / 0	0 / 0	410 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	O- C	-488 / 3	0.06 (1)
B- C	-2433 / 0	-104.9	-104.9	0.11 (1)	5.67	C- N	0 / 2227	0.28 (1)
C- Q	-3318 / 0	-104.9	-104.9	0.18 (1)	4.94	N- D	-254 / 57	0.03 (1)
Q- D	-3318 / 0	-104.9	-104.9	0.18 (1)	4.94	D- M	-252 / 0	0.05 (1)
D- E	-3155 / 0	-104.9	-104.9	0.14 (1)	5.09	M- E	0 / 265	0.03 (1)
E- F	-2190 / 0	-104.9	-104.9	0.09 (1)	5.93	E- L	-2447 / 0	0.44 (1)
F- G	-1701 / 0	-104.9	-104.9	0.15 (1)	6.25	L- F	0 / 1090	0.13 (1)
G- H	-1079 / 0	-104.9	-104.9	0.15 (1)	6.25	B- O	0 / 2001	0.25 (1)
I- H	-1773 / 0	0.0	0.0	0.69 (1)	7.81	L- G	0 / 1153	0.14 (1)
P- B	-2600 / 0	0.0	0.0	0.15 (1)	7.05	J- G	-1475 / 0	0.46 (1)
						J- H	0 / 1936	0.24 (1)
P- R	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
R- S	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
S- O	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
O- T	0 / 1849	-28.0	-28.0	0.16 (1)	10.00			
T- N	0 / 1849	-28.0	-28.0	0.16 (1)	10.00			
N- M	0 / 3318	-28.0	-28.0	0.27 (1)	10.00			
M- L	0 / 3148	-28.0	-28.0	0.24 (1)	10.00			
L- K	0 / 1079	-28.0	-28.0	0.09 (1)	10.00			
K- J	0 / 1079	-28.0	-28.0	0.09 (1)	10.00			
J- I	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
N	6-5-8	-1383	-1383	—	FRONT	VERT	TOTAL
Q	4-6-4	-130	-130	—	FRONT	VERT	TOTAL
R	6-4	-48	-83	—	FRONT	VERT	TOTAL
S	2-6-4	-41	-71	—	FRONT	VERT	TOTAL
T	4-6-4	-41	-71	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.69/1.00 (H-I:1) , BC=0.27/1.00 (M-N:1) , WB=0.46/1.00 (G-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

PROFESSIONAL ENGINEER PROVINCE OF ONTARIO S KATSOULAKOS

DWG NO. TAM 7228.18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 4-6-4 ON TOP CHORD, AND 83.2 lbs FACTORED DOWN AT 6-4, 71.5 lbs FACTORED DOWN AT 2-6-4, AND 71.5 lbs FACTORED DOWN AT 4-6-4, AND 1382.9 lbs FACTORED DOWN AT 6-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7228-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Thu Feb 1 09:43:22 2018 Page 1

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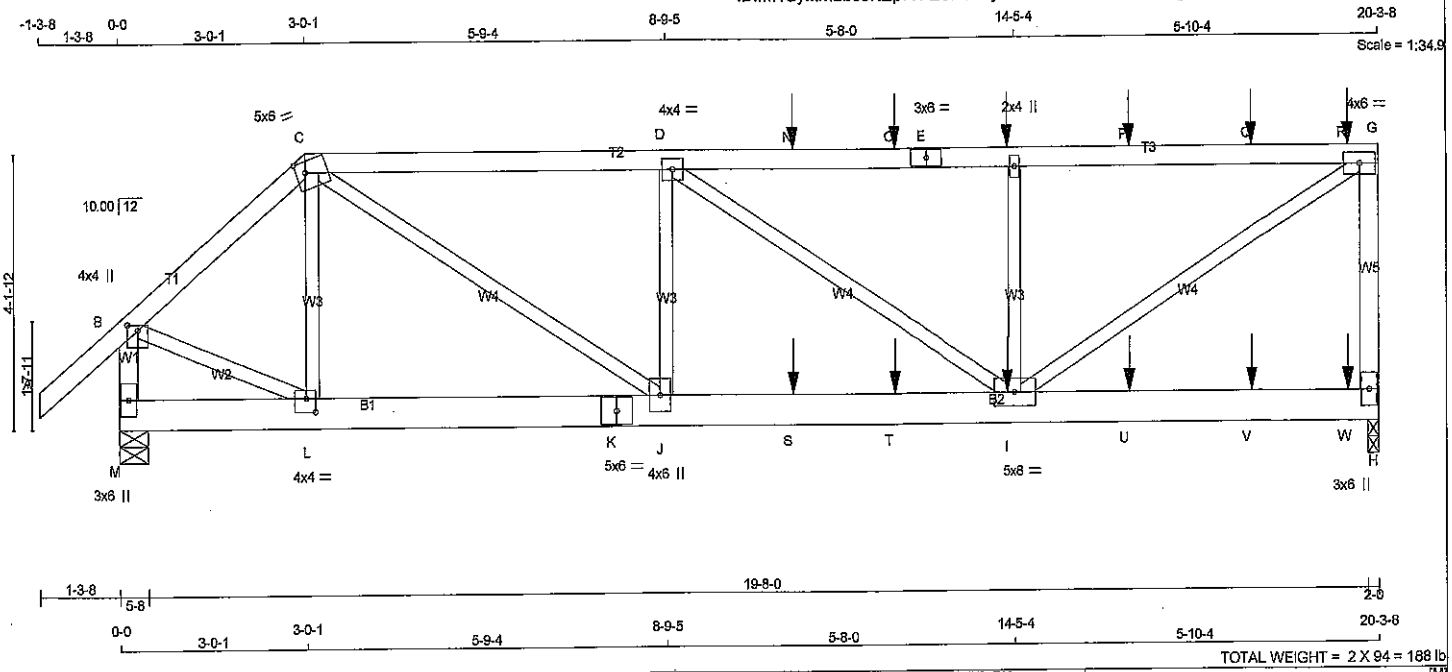

TOTAL WEIGHT = 2 X 94 = 188 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(61.0)
G- H	1	12	TOP
M- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- K	2	12	TOP
K- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW+t	MT20	4.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.50	1.75
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	2156	0	2156	0	0	2-0	2-0
M	1744	0	1744	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL.
H	1732	1040 / 0	358 / 0	0 / 0	0 / 0	334 / 0	0 / 0
M	1376	866 / 0	258 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	L- C	-299 / 37	0.04 (1)
B- C	-1505 / 0	-104.9	-104.9	0.10 (1)	6.25	B- L	0 / 1238	0.15 (1)
C- D	-2475 / 0	-104.9	-104.9	0.35 (1)	5.32	C- J	0 / 1588	0.20 (1)
D- N	-2362 / 0	-104.9	-104.9	0.49 (1)	5.16	J- D	-679 / 0	0.09 (1)
N- O	-2362 / 0	-104.9	-104.9	0.49 (1)	5.16	D- I	-135 / 0	0.06 (1)
O- E	-2362 / 0	-104.9	-104.9	0.49 (1)	5.16	I- F	-1141 / 0	0.15 (1)
E- F	-2362 / 0	-104.9	-104.9	0.49 (1)	5.16	I- G	0 / 2810	0.35 (1)
F- P	-2362 / 0	-104.9	-104.9	0.50 (1)	5.16			
P- Q	-2362 / 0	-104.9	-104.9	0.50 (1)	5.16			
Q- R	-2362 / 0	-104.9	-104.9	0.50 (1)	5.16			
R- G	-2362 / 0	-104.9	-104.9	0.50 (1)	5.16			
H- G	-2002 / 0	0.0	0.0	0.25 (1)	7.78			
M- B	-1724 / 0	0.0	0.0	0.10 (1)	7.81			
M- L	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
L- K	0 / 1144	-28.0	-28.0	0.10 (1)	10.00			
K- J	0 / 1144	-28.0	-28.0	0.10 (1)	10.00			
J- S	0 / 2475	-28.0	-28.0	0.21 (1)	10.00			
S- T	0 / 2475	-28.0	-28.0	0.21 (1)	10.00			
T- I	0 / 2475	-28.0	-28.0	0.21 (1)	10.00			
I- U	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
U- V	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
V- W	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
W- H	0 / 0	-28.0	-28.0	0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	14-3-12	-130	-130	---	BACK	VERT	TOTAL
I	14-3-12	-41	-71	---	BACK	VERT	TOTAL
N	10-9-12	-130	-130	---	BACK	VERT	TOTAL
O	12-5-4	-130	-130	---	BACK	VERT	TOTAL
P	16-3-12	-130	-130	---	BACK	VERT	TOTAL
Q	18-3-12	-130	-130	---	BACK	VERT	TOTAL
R	19-9-12	-157	-157	---	BACK	VERT	TOTAL
S	10-9-12	-41	-71	---	BACK	VERT	TOTAL
T	12-5-4	-41	-71	---	BACK	VERT	TOTAL
U	16-3-12	-41	-71	---	BACK	VERT	TOTAL
V	18-3-12	-41	-71	---	BACK	VERT	TOTAL
W	19-9-12	-46	-84	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.50/1.00 (F-G:1) , BC=0.21/1.00 (I-J:1) , WB=0.35/1.00 (G-I:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (L) (INPUT = 0.90)
SI METAL= 0.30 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7229 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T1Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 10-9-12, 129.6 lbs FACTORED DOWN AT 12-5-4, 129.6 lbs FACTORED DOWN AT 14-3-12, 129.6 lbs FACTORED DOWN AT 16-3-12, AND 129.6 lbs FACTORED DOWN AT 18-3-12, AND 156.9 lbs FACTORED DOWN AT 19-9-12 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 10-9-12, 71.5 lbs FACTORED DOWN AT 12-5-4, 71.5 lbs FACTORED DOWN AT 14-3-12, 71.5 lbs FACTORED DOWN AT 16-3-12, AND 71.5 lbs FACTORED DOWN AT 18-3-12, AND 84.3 lbs FACTORED DOWN AT 19-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7229-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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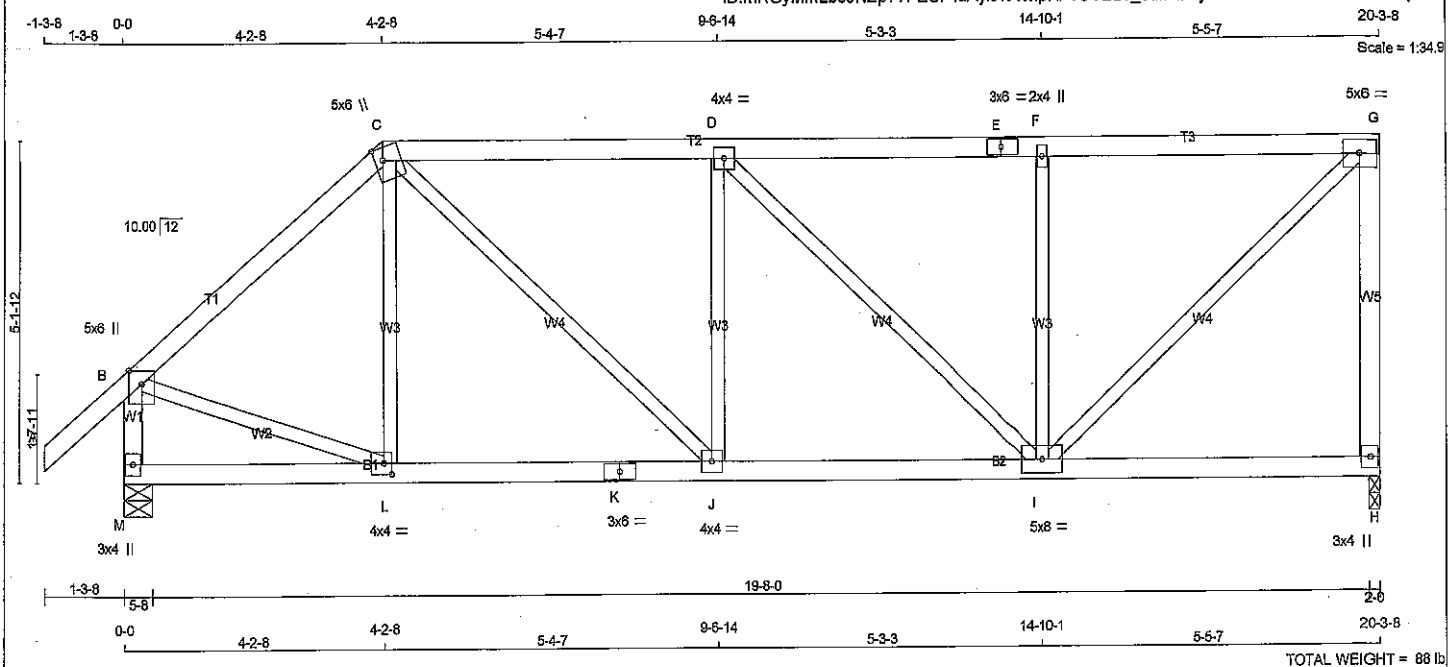

TOTAL WEIGHT = 88 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	2-0	2-0
M	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
M	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- C	-136 / 137	0.05 (1)	
B- C	-1238 / 0	-104.9	-104.9	0.36 (1)	5.32	B- L	0 / 991	0.22 (1)	
C- D	-1474 / 0	-104.9	-104.9	0.46 (1)	4.82	C- J	0 / 712	0.16 (1)	
D- E	-1183 / 0	-104.9	-104.9	0.44 (1)	5.26	J- D	-336 / 67	0.13 (1)	
E- F	-1183 / 0	-104.9	-104.9	0.44 (1)	5.26	D- I	-397 / 0	0.39 (1)	
F- G	-1183 / 0	-104.9	-104.9	0.45 (1)	5.26	I- F	-617 / 0	0.24 (1)	
H- G	-1286 / 0	0.0	0.0	0.58 (1)	7.11	I- G	0 / 1584	0.36 (1)	
M- B	-1448 / 0	0.0	0.0	0.15 (1)	6.79				
M- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00				
L- K	0 / 945	-28.0	-28.0	0.26 (2)	10.00				
K- J	0 / 945	-28.0	-28.0	0.26 (2)	10.00				
J- I	0 / 1474	-28.0	-28.0	0.35 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.58/1.00 (G-H:1) , BC=0.35/1.00 (I-J:2) , WB=0.39/1.00 (D-I:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.35 (L) (INPUT = 1.00)

DWG NO. TAM 7230-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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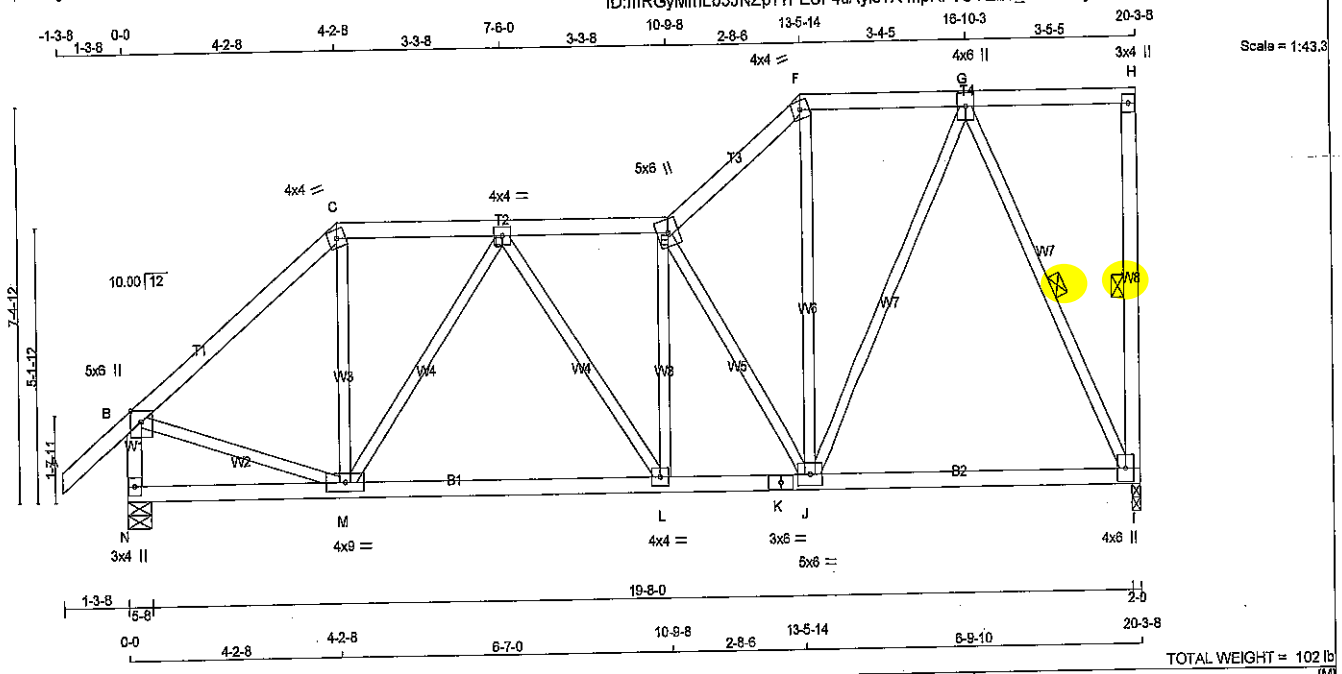

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
N	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
N	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1245 / 0	-104.9	-104.9	0.36 (1)	5.30
C- D	-963 / 0	-104.9	-104.9	0.19 (1)	6.09
D- E	-1418 / 0	-104.9	-104.9	0.20 (1)	5.25
E- F	-1121 / 0	-104.9	-104.9	0.15 (1)	5.62
F- G	-867 / 0	-104.9	-104.9	0.20 (1)	6.25
G- H	0 / 0	-104.9	-104.9	0.19 (1)	10.00
I- H	-139 / 0	0.0	0.0	0.03 (1)	6.25
N- B	-1454 / 0	0.0	0.0	0.15 (1)	6.78
N- M	0 / 0	-28.0	-28.0	0.22 (3)	10.00
M- L	0 / 1324	-28.0	-28.0	0.36 (2)	10.00
L- K	0 / 1419	-28.0	-28.0	0.41 (2)	10.00
K- J	0 / 1419	-28.0	-28.0	0.41 (2)	10.00
J- I	0 / 548	-28.0	-28.0	0.31 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	0 / 439	0.10 (1)
L- E	-41 / 55	0.02 (1)
E- J	-1111 / 0	0.58 (1)
J- F	0 / 436	0.10 (1)
B- M	0 / 996	0.22 (1)
J- G	0 / 766	0.17 (1)
G- I	-1257 / 0	0.52 (1)
M- D	-658 / 0	0.38 (1)
D- L	0 / 181	0.04 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.41/1.00 (J-L:2) , WB=0.58/1.00 (E-J:1) , SSI=0.17/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)

DWG NO. TAM73-31-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T3	1	1	TRUSS DESC.	

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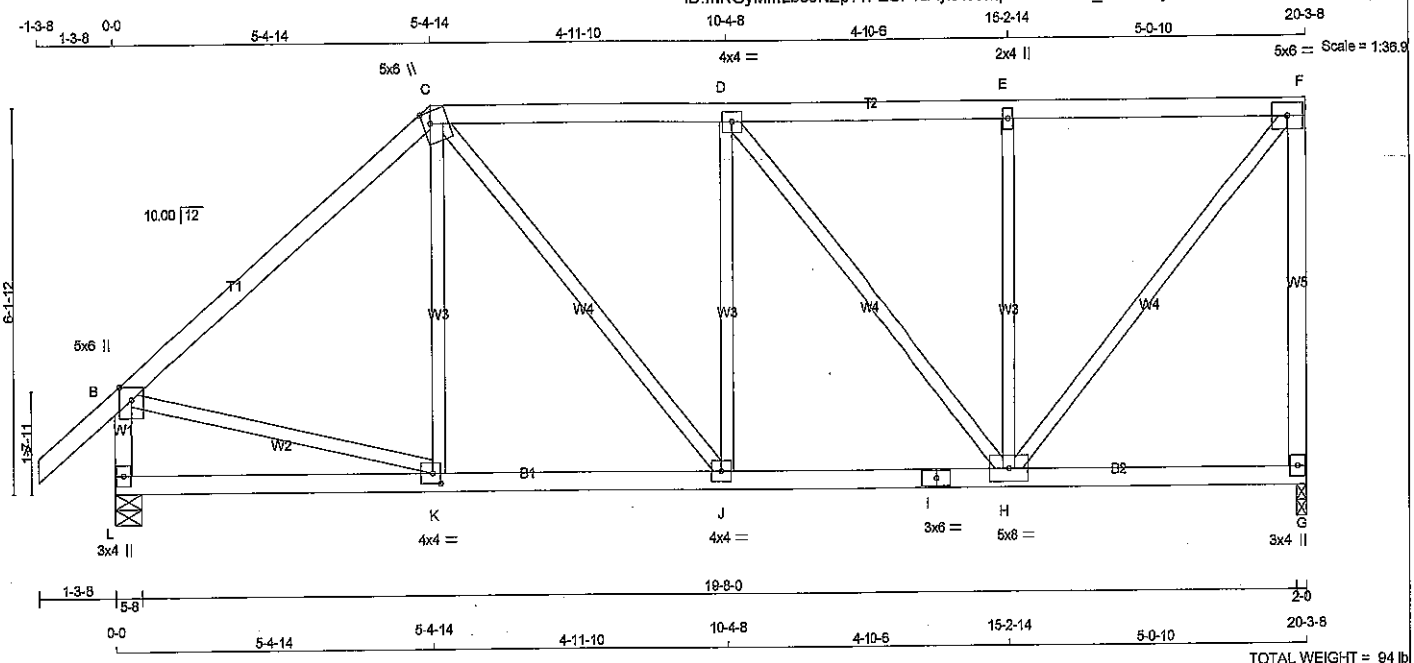

TOTAL WEIGHT = 94 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		K- C	-52 / 198	0.04 (3)
B- C	-1217 / 0	-104.9	-104.9	0.62 (1)	4.93		B- K	0 / 959	0.22 (1)
C- D	-1216 / 0	-104.9	-104.9	0.38 (1)	5.32		C- J	0 / 438	0.10 (1)
D- E	-928 / 0	-104.9	-104.9	0.37 (1)	5.90		J- D	-212 / 101	0.13 (1)
E- F	-928 / 0	-104.9	-104.9	0.37 (1)	5.90		D- H	-452 / 0	0.52 (1)
G- F	-1291 / 0	0.0	0.0	0.95 (1)	7.10		H- E	-570 / 0	0.34 (1)
L- B	-1431 / 0	0.0	0.0	0.15 (1)	6.82		H- F	0 / 1420	0.32 (1)
L- K	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
K- J	0 / 932	-28.0	-28.0	0.30 (2)	10.00				
J- I	0 / 1217	-28.0	-28.0	0.30 (2)	10.00				
I- H	0 / 1217	-28.0	-28.0	0.30 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.95/1.00 (F-G:1) , BC=0.30/1.00 (H-J:2) , WB=0.52/1.00 (D-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.35 (K) (INPUT = 1.00)

DWG NO. TAM 72326

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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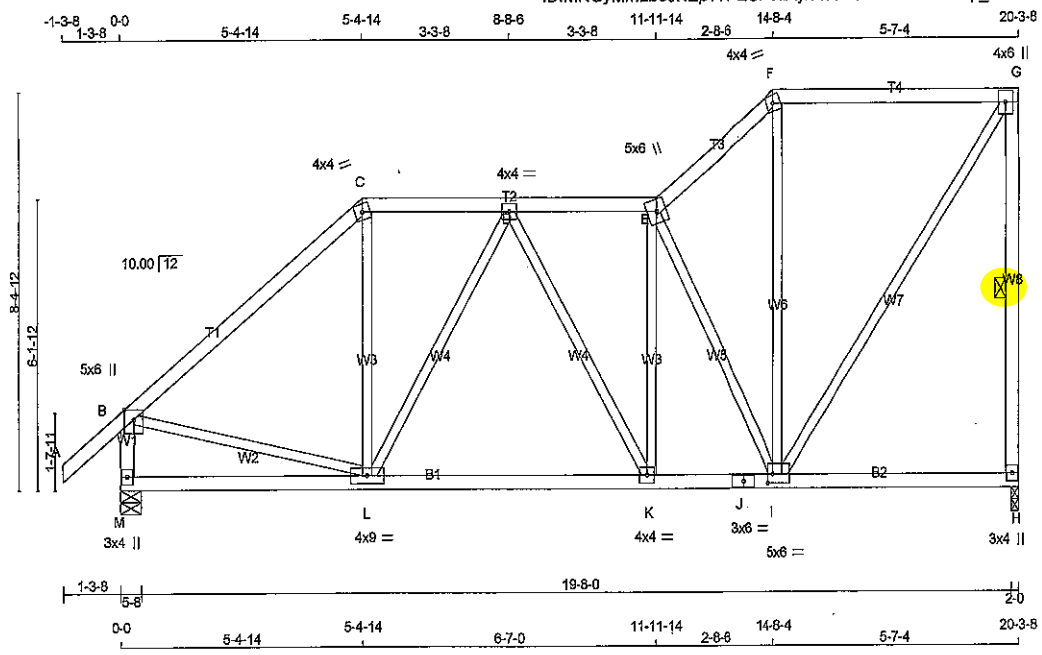

TOTAL WEIGHT = 104 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.25	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	2-0	2-0
M	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
M	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1224 / 0	-104.9	-104.9	0.62 (1)	4.92	
C- D	-949 / 0	-104.9	-104.9	0.19 (1)	6.13	
D- E	-1143 / 0	-104.9	-104.9	0.19 (1)	5.71	
E- F	-874 / 0	-104.9	-104.9	0.14 (1)	6.25	
F- G	-673 / 0	-104.9	-104.9	0.59 (1)	6.12	
H- G	-1283 / 0	0.0	0.0	0.41 (1)	5.69	
M- B	-1435 / 0	0.0	0.0	0.15 (1)	6.82	
M- L	0 / 0	-28.0	-28.0	0.26 (3)	10.00	
L- K	0 / 1156	-28.0	-28.0	0.38 (2)	10.00	
K- J	0 / 1141	-28.0	-28.0	0.28 (2)	10.00	
J- I	0 / 1141	-28.0	-28.0	0.28 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 377	0.08 (2)
L- D	-439 / 0	0.36 (1)
D- K	-26 / 62	0.02 (1)
K- E	0 / 163	0.04 (2)
E- I	-1084 / 0	0.81 (1)
I- F	0 / 214	0.05 (3)
I- G	0 / 1199	0.27 (1)
B- L	0 / 964	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.38/1.00 (K-L:2) , WB=0.81/1.00 (E-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (I) (INPUT = 0.90)
JSI METAL= 0.36 (J) (INPUT = 1.00)

DWG NO. TAM7233 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T4	1	1	TRUSS DESC.	

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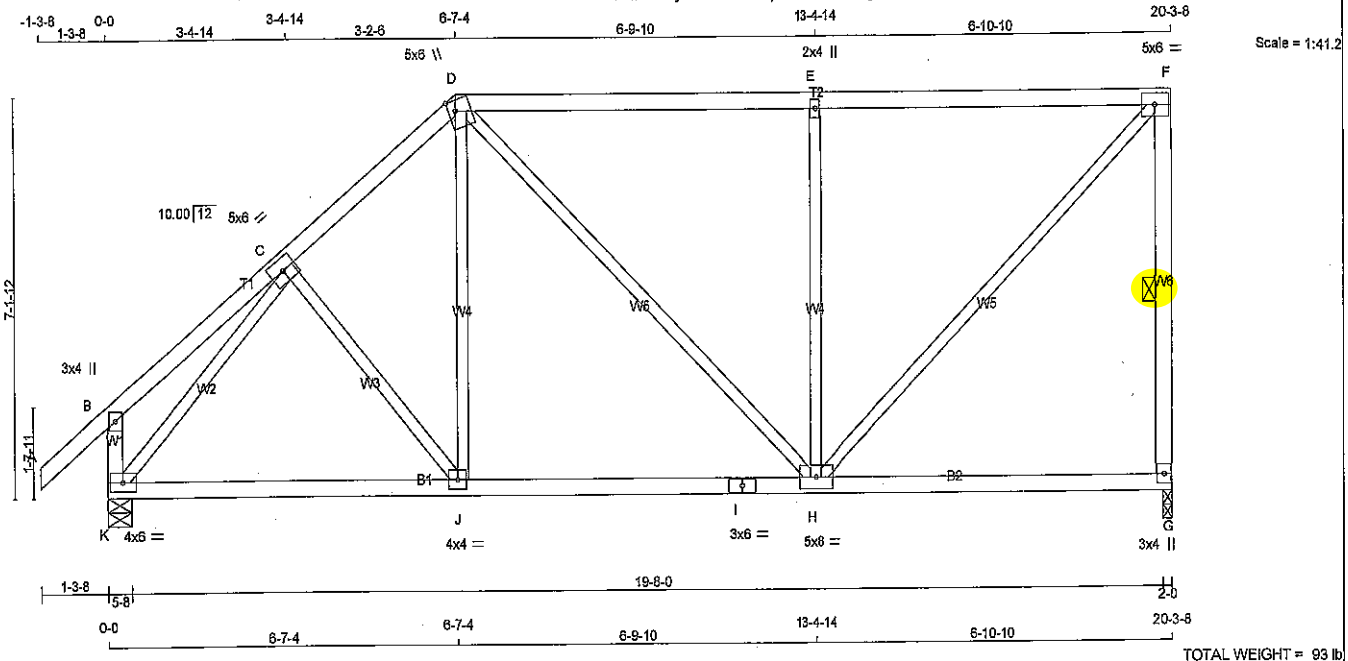

TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
K	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL.
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
K	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 24	-104.9	-104.9	0.17 (1)	10.00
C- D	-1193 / 0	-104.9	-104.9	0.20 (1)	5.63
D- E	-1000 / 0	-104.9	-104.9	0.91 (1)	4.25
E- F	-1000 / 0	-104.9	-104.9	0.92 (1)	4.25
G- F	-1270 / 0	0.0	0.0	0.29 (1)	5.71
K- B	-279 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 924	-28.0	-28.0	0.38 (2)	10.00
J- I	0 / 898	-28.0	-28.0	0.40 (2)	10.00
I- H	0 / 898	-28.0	-28.0	0.40 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-46 / 85	0.02 (1)
J- D	0 / 318	0.07 (2)
K- C	-1467 / 0	0.64 (1)
D- H	0 / 144	0.03 (1)
H- E	-890 / 0	0.79 (1)
H- F	0 / 1411	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.92/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.79/1.00 (E-H:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM-7234-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T4S	1	1	TRUSS DESC.	

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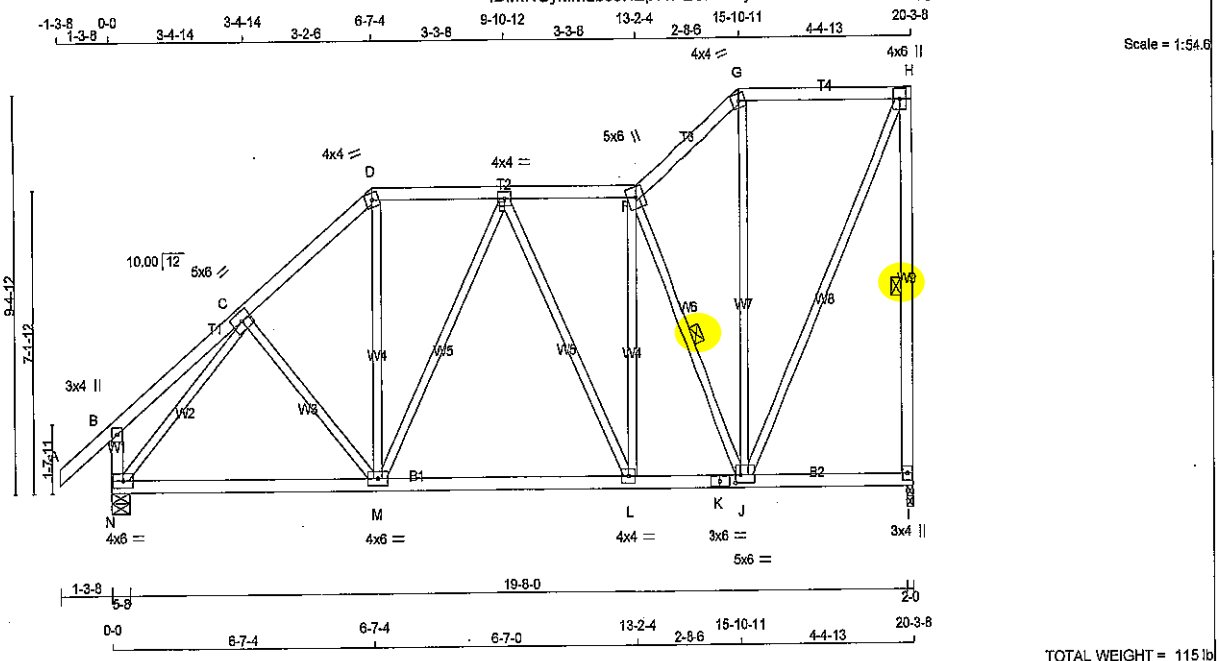

TOTAL WEIGHT = 115 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	6.0	2.25	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
N	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
N	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 24	-104.9	-104.9	0.17 (1)	10.00
C- D	-1195 / 0	-104.9	-104.9	0.19 (1)	5.63
D- E	-904 / 0	-104.9	-104.9	0.18 (1)	6.24
E- F	-921 / 0	-104.9	-104.9	0.18 (1)	6.20
F- G	-655 / 0	-104.9	-104.9	0.14 (1)	6.25
G- H	-504 / 0	-104.9	-104.9	0.36 (1)	6.25
I- H	-1296 / 0	0.0	0.0	0.54 (1)	5.67
N- B	-279 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 926	-28.0	-28.0	0.42 (2)	10.00
M- L	0 / 1008	-28.0	-28.0	0.43 (2)	10.00
L- K	0 / 917	-28.0	-28.0	0.22 (2)	10.00
K- J	0 / 917	-28.0	-28.0	0.22 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.11 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-45 / 85	0.02 (1)
M- D	0 / 475	0.11 (1)
M- E	-247 / 0	0.28 (1)
E- L	-205 / 0	0.24 (1)
L- F	0 / 306	0.07 (1)
F- J	-1089 / 0	0.40 (1)
J- G	0 / 155	0.03 (3)
J- H	0 / 1160	0.27 (1)
N- C	-1469 / 0	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.54/1.00 (H-I:1) , BC=0.43/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 72358

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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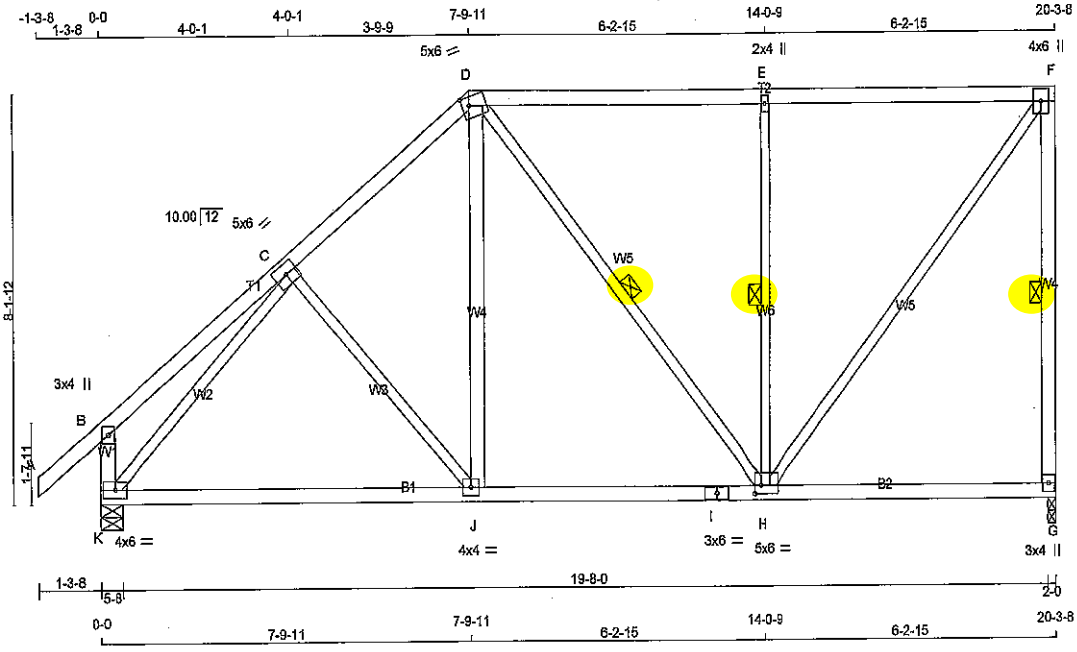

TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
K	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
K	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00
B-C	0 / 30	-104.9	-104.9	1	0.25 (1)	10.00
C-D	-1130 / 0	-104.9	-104.9	1	0.28 (1)	5.62
D-E	-816 / 0	-104.9	-104.9	1	0.73 (1)	5.30
E-F	-816 / 0	-104.9	-104.9	1	0.73 (1)	5.30
G-F	-1275 / 0	0.0	0.0	1	0.38 (1)	5.71
K-B	-301 / 0	0.0	0.0	1	0.03 (1)	7.61
K-J	0 / 941	-28.0	-28.0	2	0.47 (2)	10.00
J-I	0 / 849	-28.0	-28.0	2	0.47 (2)	10.00
I-H	0 / 849	-28.0	-28.0	2	0.47 (2)	10.00
H-G	0 / 0	-28.0	-28.0	2	0.23 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-147 / 61	0.09 (1)
J-D	0 / 397	0.06 (2)
D-H	-83 / 0	0.06 (2)
H-E	-812 / 0	0.34 (1)
H-F	0 / 1312	0.30 (1)
K-C	-1448 / 0	0.87 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 914 (0.27")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.47/1.00 (H-J:2) , WB=0.87/1.00 (C-K:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 7236-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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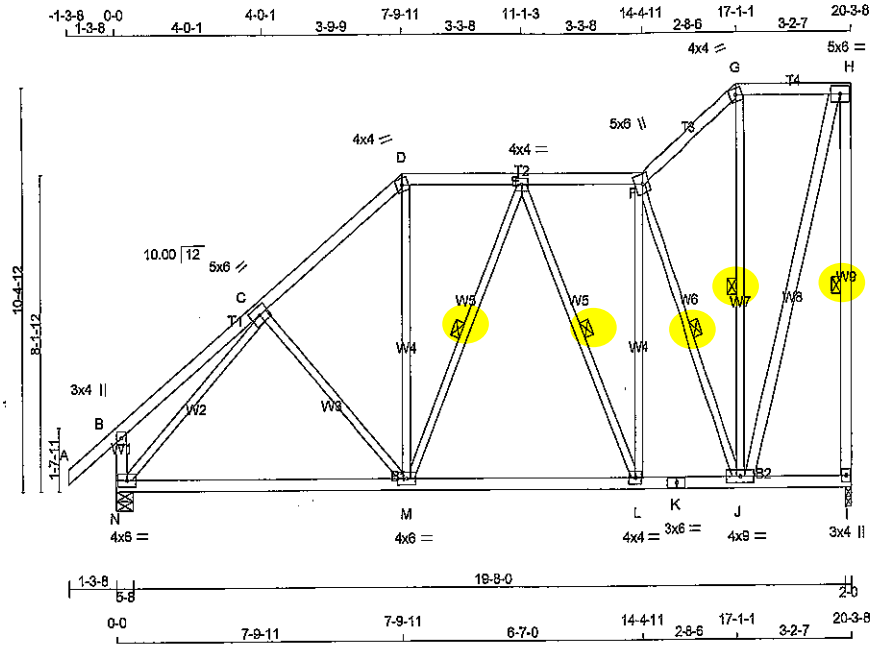


TOTAL WEIGHT = 126 lb

[M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTWW+m	MT20	5.0	6.0	
G	TTW-m	MT20	4.0	4.0	
H	TMVW-t	MT20	5.0	6.0	
I	BMV1+p	MT20	3.0	4.0	
J	BMWWW-t	MT20	4.0	9.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW-t	MT20	4.0	4.0	
M	BMWWW-t	MT20	4.0	6.0	
N	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
N	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
N	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

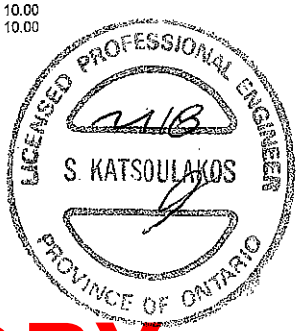
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, E-M, E-L, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 30	-104.9	-104.9	0.25 (1)	10.00
C- D	-1134 / 0	-104.9	-104.9	0.21 (1)	5.74
D- E	-851 / 0	-104.9	-104.9	0.14 (1)	6.25
E- F	-730 / 0	-104.9	-104.9	0.14 (1)	6.25
F- G	-457 / 0	-104.9	-104.9	0.10 (1)	6.25
G- H	-351 / 0	-104.9	-104.9	0.14 (1)	6.25
I- H	-1309 / 0	0.0	0.0	0.70 (1)	5.65
N- B	-301 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 943	-28.0	-28.0	0.51 (2)	10.00
M- L	0 / 874	-28.0	-28.0	0.50 (2)	10.00
L- K	0 / 724	-28.0	-28.0	0.17 (2)	10.00
K- J	0 / 724	-28.0	-28.0	0.17 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-148 / 62	0.09 (1)
M- D	0 / 409	0.09 (1)
M- E	-61 / 86	0.03 (1)
E- L	-382 / 0	0.19 (1)
L- F	0 / 469	0.11 (1)
F- J	-1108 / 0	0.52 (1)
J- G	-20 / 103	0.02 (3)
J- H	0 / 1198	0.19 (1)
N- C	-1451 / 0	0.87 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 986 (0.25")

CSI: TC=0.70/1.00 (H-I:1) , BC=0.51/1.00 (M-N:2) , WB=0.87/1.00 (C-N:1) , SSI=0.18/1.00 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 7237 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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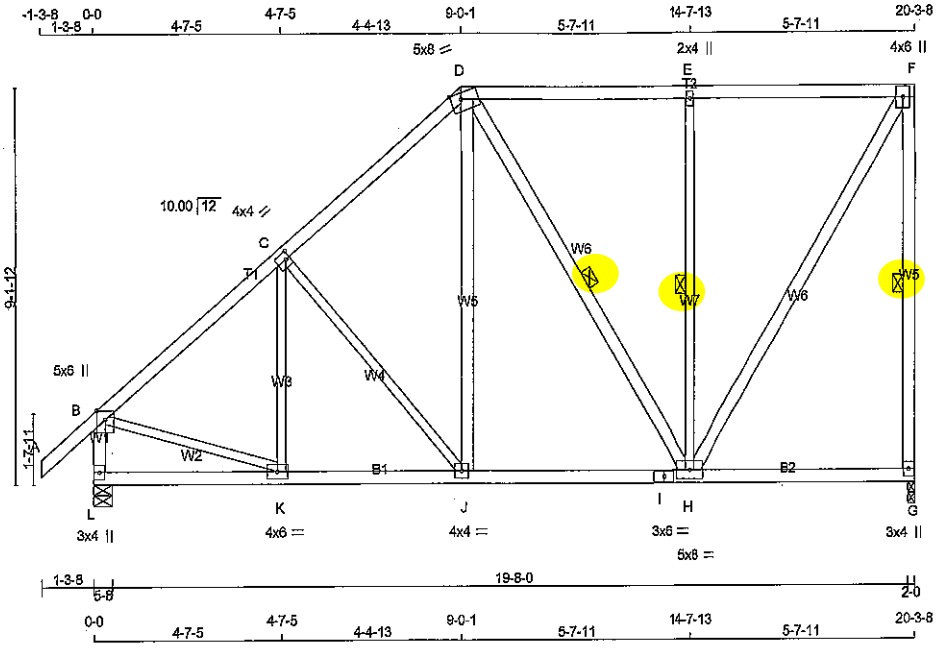

TOTAL WEIGHT = 2 X 115 = 230 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K- C	-143 / 130	0.06 (1)
B- C	-1257 / 0	-104.9	-104.9	0.41 (1)	5.22	C- J	-344 / 0	0.30 (1)
C- D	-1036 / 0	-104.9	-104.9	0.39 (1)	5.64	J- D	0 / 394	0.06 (2)
D- E	-676 / 0	-104.9	-104.9	0.58 (1)	6.13	D- H	-178 / 0	0.10 (1)
E- F	-676 / 0	-104.9	-104.9	0.58 (1)	6.13	H- E	-733 / 0	0.40 (1)
G- F	-1285 / 0	0.0	0.0	0.50 (1)	5.69	H- F	0 / 1258	0.20 (1)
L- B	-1440 / 0	0.0	0.0	0.15 (1)	6.81	B- K	0 / 1032	0.23 (1)
L- K	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
K- J	0 / 993	-28.0	-28.0	0.24 (2)	10.00			
J- I	0 / 772	-28.0	-28.0	0.31 (2)	10.00			
I- H	0 / 772	-28.0	-28.0	0.31 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.31/1.00 (H-J:2) , WB=0.40/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 723 G18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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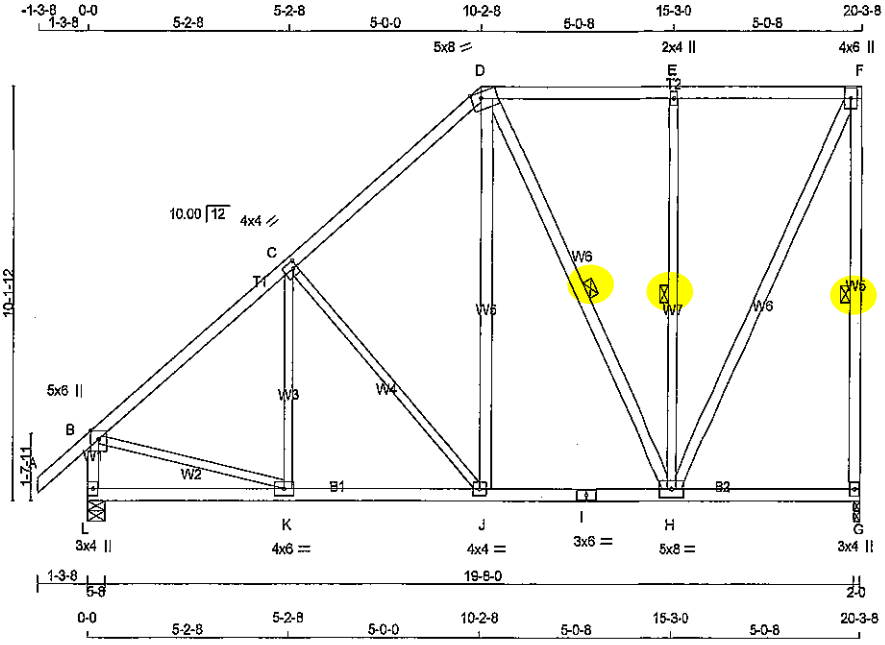

Scale = 1:56.7

TOTAL WEIGHT = 2 X 122 = 243 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K- C	-87 / 184	0.05 (1)	
B- C	-1256 / 0	-104.9	-104.9	0.54 (1)	5.02	C- J	-447 / 0	0.52 (1)	
C- D	-949 / 0	-104.9	-104.9	0.51 (1)	5.62	J- D	0 / 464	0.07 (1)	
D- E	-557 / 0	-104.9	-104.9	0.45 (1)	6.25	D- H	-316 / 0	0.21 (1)	
E- F	-558 / 0	-104.9	-104.9	0.45 (1)	6.25	H- E	-654 / 0	0.46 (1)	
G- F	-1291 / 0	0.0	0.0	0.65 (1)	5.68	H- F	0 / 1224	0.20 (1)	
L- B	-1435 / 0	0.0	0.0	0.15 (1)	6.82	B- K	0 / 1027	0.23 (1)	
L- K	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
K- J	0 / 996	-28.0	-28.0	0.29 (2)	10.00				
J- I	0 / 702	-28.0	-28.0	0.25 (2)	10.00				
I- H	0 / 702	-28.0	-28.0	0.25 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.29/1.00 (J-K:2) , WB=0.52/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (K) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

DWG NO. TAM 7239 -1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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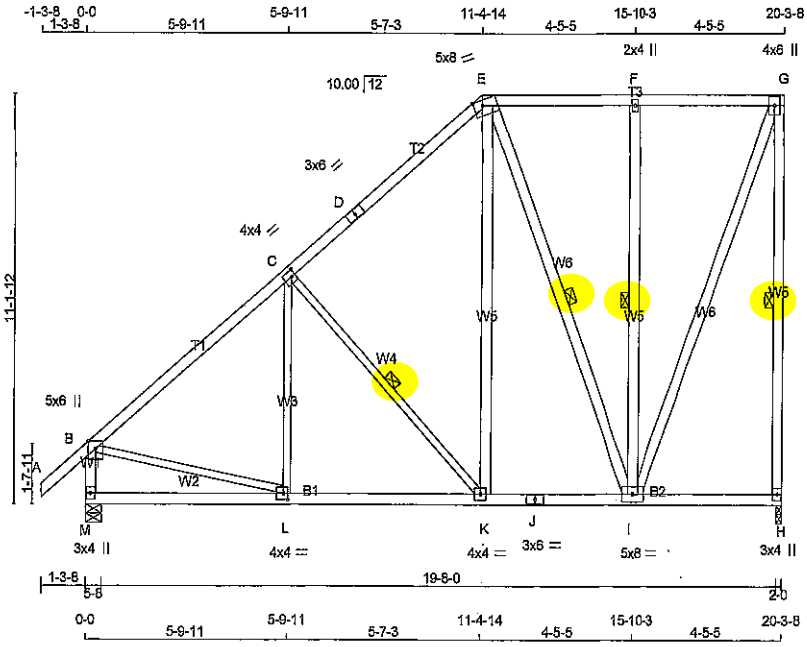

Scale = 1:62.9

TOTAL WEIGHT = 2 X 132 = 263 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1349	0	1349	0	0	2-0	2-0
M	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
M	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1247 / 0	-104.9	-104.9	0.49 (1)		5.13
C- D	-857 / 0	-104.9	-104.9	0.48 (1)		5.96
D- E	-857 / 0	-104.9	-104.9	0.48 (1)		5.96
E- F	-458 / 0	-104.9	-104.9	0.26 (1)		6.25
F- G	-458 / 0	-104.9	-104.9	0.26 (1)		6.25
H- G	-1296 / 0	0.0	0.0	0.83 (1)		5.67
M- B	-1429 / 0	0.0	0.0	0.15 (1)		6.83
M- L	0 / 0	-28.0	-28.0	0.24 (3)		10.00
L- K	0 / 993	-28.0	-28.0	0.34 (2)		10.00
K- J	0 / 627	-28.0	-28.0	0.19 (2)		10.00
J- I	0 / 627	-28.0	-28.0	0.19 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.13 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-38 / 234	0.05 (3)
C- K	-546 / 0	0.26 (1)
K- E	0 / 534	0.09 (1)
E- I	-447 / 0	0.34 (1)
I- F	-575 / 0	0.37 (1)
I- G	0 / 1209	0.19 (1)
B- L	0 / 1018	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.83/1.00 (G-H:1) , BC=0.34/1.00 (K-L:2) , WB=0.37/1.00 (F-I:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.37 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7240 -T8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T14	5	1	TRUSS DESC. .	

Tamarack Roof Truss, Burlington

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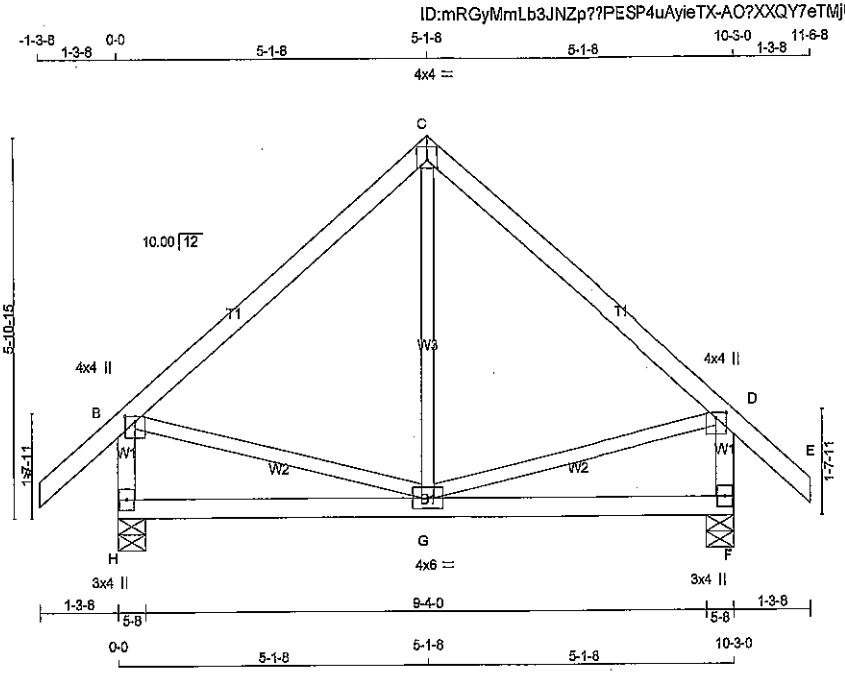

TOTAL WEIGHT = 5 X 46 = 231 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	827	0	827	0	0	5-8	5-8
F	827	0	827	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	641	423 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0
F	641	423 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO			
A- B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	G- C	0 / 230	0.05 (3)	
B- C	-428 / 0	-104.9 -104.9	0.36 (1)	6.25	B- G	0 / 339	0.08 (1)	
C- D	-428 / 0	-104.9 -104.9	0.36 (1)	6.25	G- D	0 / 339	0.08 (1)	
D- E	0 / 47	-104.9 -104.9	0.14 (1)	10.00				
H- B	-772 / 0	0.0 0.0	0.08 (1)	7.81				
F- D	-772 / 0	0.0 0.0	0.08 (1)	7.81				
H- G	0 / 0	-28.0 -28.0	0.22 (3)	10.00				
G- F	0 / 0	-28.0 -28.0	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.22/1.00 (F-G:3) , WB=0.08/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

DWG NO. TAM 7241 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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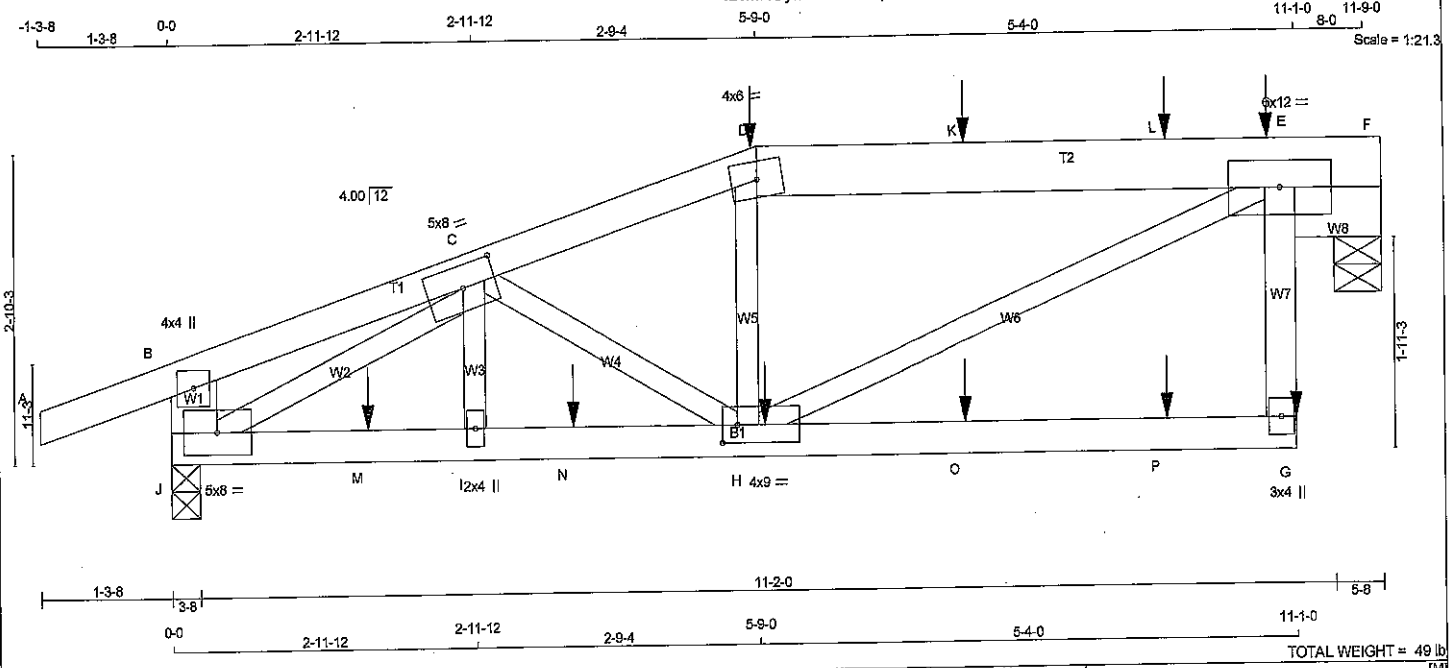

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	1650F 1.5E	SPF
E - F	2x6	DRY	1650F 1.5E	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWWW-t	MT20	5.0	8.0	2.50	3.75
D	TTW-m	MT20	4.0	6.0		
E	TMVWW-t	MT20	6.0	12.0		
G	BMV+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0	2.00	1.75
I	BMW+w	MT20	2.0	4.0		
J	BMVW1-t	MT20	5.0	8.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 587.3 lbs FACTORED DOWN AT 5-9-0, 131.0 lbs FACTORED DOWN AT 7-9-12, AND 131.0 lbs FACTORED DOWN AT 9-9-12, AND 131.0 lbs FACTORED DOWN AT 10-9-12 ON TOP CHORD, AND 80.7 lbs FACTORED DOWN AT 1-11-4, 61.3 lbs FACTORED DOWN AT 3-11-4, 118.2 lbs FACTORED DOWN AT 5-9-12, 118.2 lbs FACTORED DOWN AT 7-9-12, AND 118.2 lbs FACTORED DOWN AT 9-9-12, AND 132.7 lbs FACTORED DOWN AT 11-1-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1744	0	1744	0	0	5-8	5-8
J	1539	0	1539	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1379	865 / 0	260 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1214	765 / 0	227 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-104.9	-104.9	0.14 (1)	10.00
B- C	-2 / 6	-104.9	-104.9	0.14 (1)	10.00
C- D	-2370 / 0	-104.9	-104.9	0.19 (1)	4.26
D- K	-2276 / 0	-104.9	-104.9	0.82 (1)	4.39
K- L	-2276 / 0	-104.9	-104.9	0.82 (1)	4.39
L- E	-2276 / 0	-104.9	-104.9	0.82 (1)	4.39
E- F	0 / 0	-104.9	-104.9	0.30 (1)	10.00
G- E	0 / 354	0.0	0.0	0.06 (2)	10.00
J- B	-279 / 0	0.0	0.0	0.02 (1)	7.81
J- M	0 / 2160	-28.0	-28.0	0.55 (1)	10.00
M- I	0 / 2160	-28.0	-28.0	0.55 (1)	10.00
I- N	0 / 2159	-28.0	-28.0	0.65 (1)	10.00
N- H	0 / 2159	-28.0	-28.0	0.65 (1)	10.00
H- O	0 / 0	-28.0	-28.0	0.39 (2)	10.00
O- P	0 / 0	-28.0	-28.0	0.39 (2)	10.00
P- G	0 / 0	-28.0	-28.0	0.39 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 131	0.03 (3)
C- H	0 / 108	0.03 (1)
H- D	-633 / 33	0.12 (1)
H- E	0 / 2503	0.62 (1)
J- C	-2459 / 0	0.51 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-9-0	-587	-587	---	BACK	VERT	TOTAL
E	10-9-12	-131	-131	---	BACK	VERT	TOTAL
G	11-1-0	-126	-133	---	BACK	VERT	TOTAL
H	5-9-12	-118	-118	---	BACK	VERT	TOTAL
K	7-9-12	-131	-131	---	BACK	VERT	TOTAL
L	9-9-12	-131	-131	---	BACK	VERT	TOTAL
M	1-11-4	-81	-81	---	BACK	VERT	TOTAL
N	3-11-4	-38	-61	---	BACK	VERT	TOTAL
O	7-9-12	-118	-118	---	BACK	VERT	TOTAL
P	9-9-12	-118	-118	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 814 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 494 (0.29")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.65/1.00 (H-I:1) , WB=0.62/1.00 (E-H:1) , SSI=0.58/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.58 (E) (INPUT = 1.00)

DWG NO. TAM7242-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T16	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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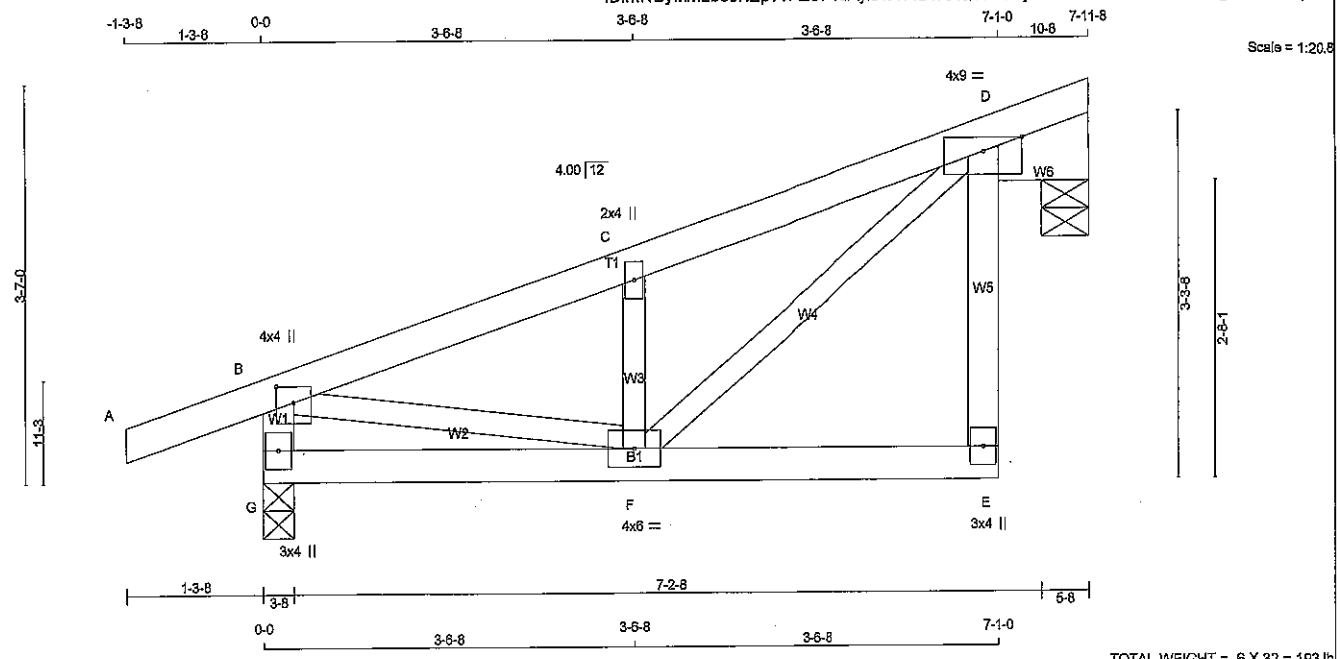

TOTAL WEIGHT = 6 X 32 = 193 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	9.0	Edge	
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	471	0	471	0	0	5-8	5-8
G	611	0	611	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	375	230 / 0	74 / 0	0 / 0	0 / 0	71 / 0	0 / 0
G	470	317 / 0	74 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-104.9	-104.9	0.13 (1)	10.00	F- C	-443 / 0	0.07 (1)
B- C	-575 / 0	-104.9	-104.9	0.15 (1)	6.25	F- D	0 / 696	0.16 (1)
C- D	-568 / 0	-104.9	-104.9	0.15 (1)	6.25	B- F	0 / 566	0.13 (1)
E- D	0 / 67	0.0	0.0	0.01 (2)	10.00			
G- B	-567 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-28.0	-28.0	0.09 (3)	10.00			
F- E	0 / 0	-28.0	-28.0	0.09 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.09/1.00 (E-F:3) , WB=0.16/1.00 (D-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7243 -T8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T17	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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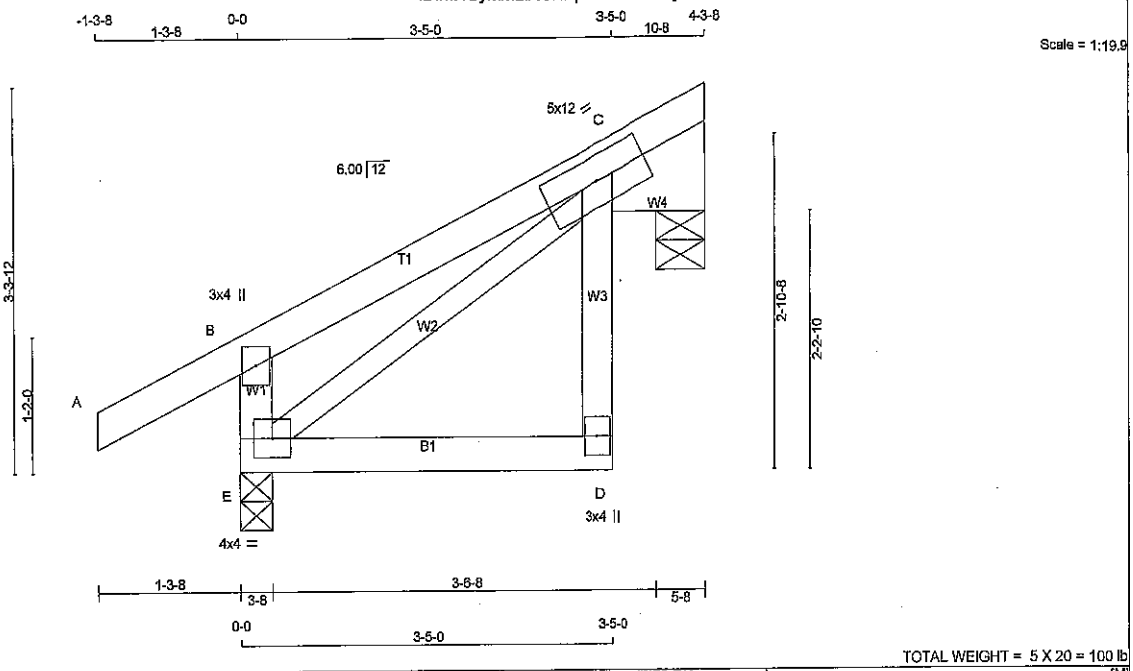

TOTAL WEIGHT = 5 X 20 = 100 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVWW1-t	MT20	5.0	12.0	
D	BMV+p	MT20	3.0	4.0	
E	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	227	0	227	0	0	5-8	5-8
E	369	0	369	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	181	111 / 0	36 / 0	0 / 0	0 / 0	34 / 0	0 / 0
E	277	199 / 0	36 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-104.9	-104.9	0.21 (1)	10.00			
D- C	0 / 84	0.0	0.0	0.01 (3)	10.00			
E- B	-322 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.10/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7244-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T19S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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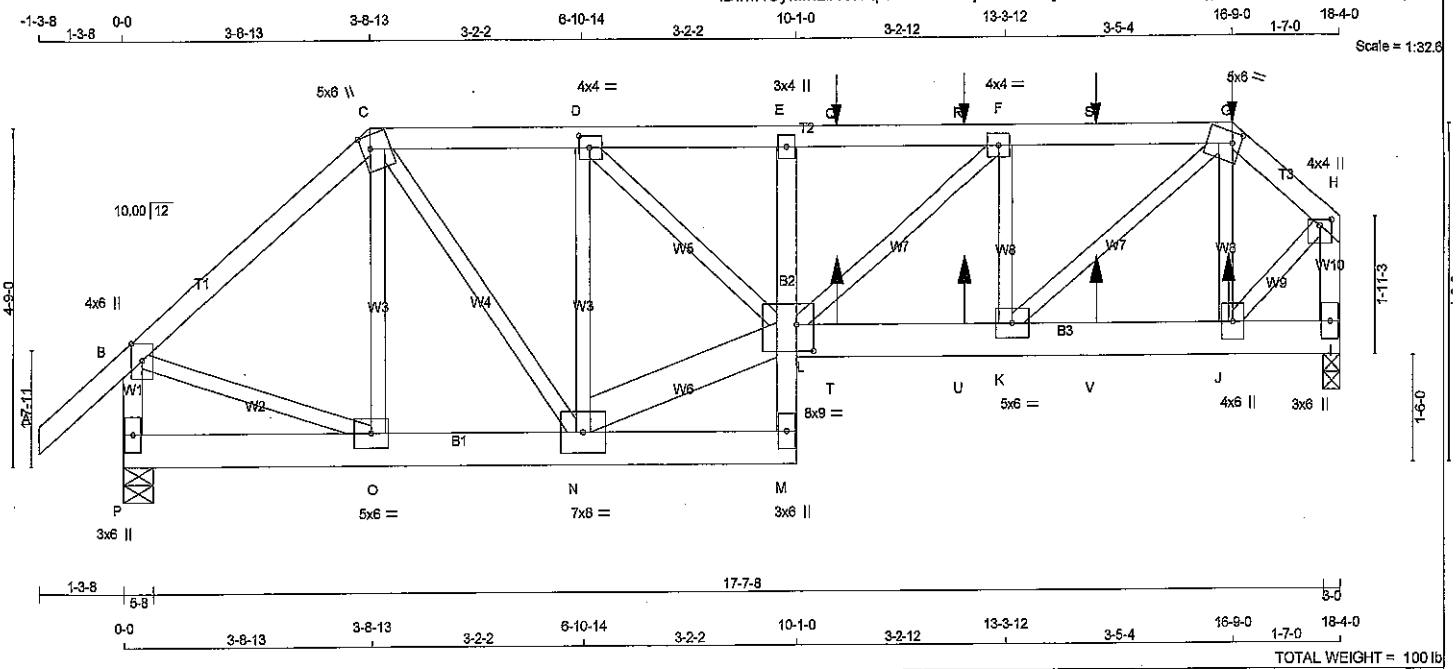

TOTAL WEIGHT = 100 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	1.50
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BVMWWW-l	MT20	8.0	9.0	4.50	3.00
M	BMV+p	MT20	3.0	6.0		
N	BMWWW-t	MT20	7.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.0 lbs FACTORED DOWN AND 152.6 lbs FACTORED UP AT 10-8-4, 1.0 lbs FACTORED DOWN AND 152.6 lbs FACTORED UP AT 12-8-4, AND 1.0 lbs FACTORED DOWN AND 152.6 lbs FACTORED UP AT 14-8-4, AND 1.0 lbs FACTORED DOWN AND 152.6 lbs FACTORED UP AT 16-9-0 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 14.9 lbs FACTORED UP AT 10-8-4, 1.0 lbs FACTORED DOWN AND 14.9 lbs FACTORED UP AT 12-8-4, AND 1.0 lbs FACTORED DOWN AND 14.9 lbs FACTORED UP AT 14-8-4, AND 1.0 lbs FACTORED DOWN AND 14.9 lbs FACTORED UP AT 16-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1352	0	1352	0	0	5-8	5-8
I	1184	0	1184	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1062	677 / 0	193 / 0	0 / 0	0 / 0	192 / 0	0 / 0
I	950	572 / 0	194 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	
B- C	-1087 / 0	-104.9	-104.9	0.31 (1)	5.63	
C- D	-1199 / 0	-104.9	-104.9	0.18 (5)	5.62	
D- E	-1890 / 0	-104.9	-104.9	0.20 (1)	4.66	
E- Q	-1905 / 0	-104.9	-104.9	0.24 (1)	4.60	
Q- R	-1905 / 0	-104.9	-104.9	0.24 (1)	4.60	
R- F	-1905 / 0	-104.9	-104.9	0.24 (1)	4.60	
F- S	-1532 / 0	-104.9	-104.9	0.22 (1)	5.04	
S- G	-1532 / 0	-104.9	-104.9	0.22 (1)	5.04	
G- H	-834 / 0	-104.9	-104.9	0.06 (1)	6.25	
P- B	-1305 / 0	0.0	0.0	0.15 (1)	7.03	
I- H	-1193 / 0	0.0	0.0	0.14 (1)	7.28	
P- O	0 / 0	-28.0	-28.0	0.05 (7)	10.00	
O- N	0 / 829	-28.0	-28.0	0.13 (1)	10.00	
N- M	0 / 28	-28.0	-28.0	0.03 (7)	10.00	
M- L	0 / 63	0.0	0.0	0.07 (1)	10.00	
L- E	-299 / 0	0.0	0.0	0.07 (1)	7.81	
L- T	0 / 1532	-28.0	-28.0	0.24 (1)	10.00	
T- U	0 / 1532	-28.0	-28.0	0.24 (1)	10.00	
U- K	0 / 1532	-28.0	-28.0	0.24 (1)	10.00	
K- V	0 / 620	-28.0	-28.0	0.12 (1)	10.00	
V- J	0 / 620	-28.0	-28.0	0.12 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.04 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-159 / 70	0.05 (1)
C- N	0 / 637	0.16 (1)
N- D	-1007 / 0	0.33 (1)
J- G	-456 / 0	0.09 (1)
B- O	0 / 874	0.22 (1)
J- H	0 / 855	0.21 (1)
N- L	0 / 1318	0.17 (1)
D- L	0 / 932	0.23 (1)
L- F	0 / 517	0.13 (5)
K- F	-723 / 0	0.14 (1)
K- G	0 / 1186	0.29 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	16-9-0	-0	-1	153	BACK	VERT	TOTAL
J	16-8-4	12	1	15	BACK	VERT	TOTAL
Q	10-8-4	-0	-1	153	BACK	VERT	TOTAL
R	12-8-4	-0	-1	153	BACK	VERT	TOTAL
S	14-8-4	-0	-1	153	BACK	VERT	TOTAL
T	10-8-4	12	1	15	BACK	VERT	TOTAL
U	12-8-4	12	1	15	BACK	VERT	TOTAL
V	14-8-4	12	1	15	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.24/1.00 (K-L:1) , WB=0.33/1.00 (D-N:1) , SSI=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7269 -19
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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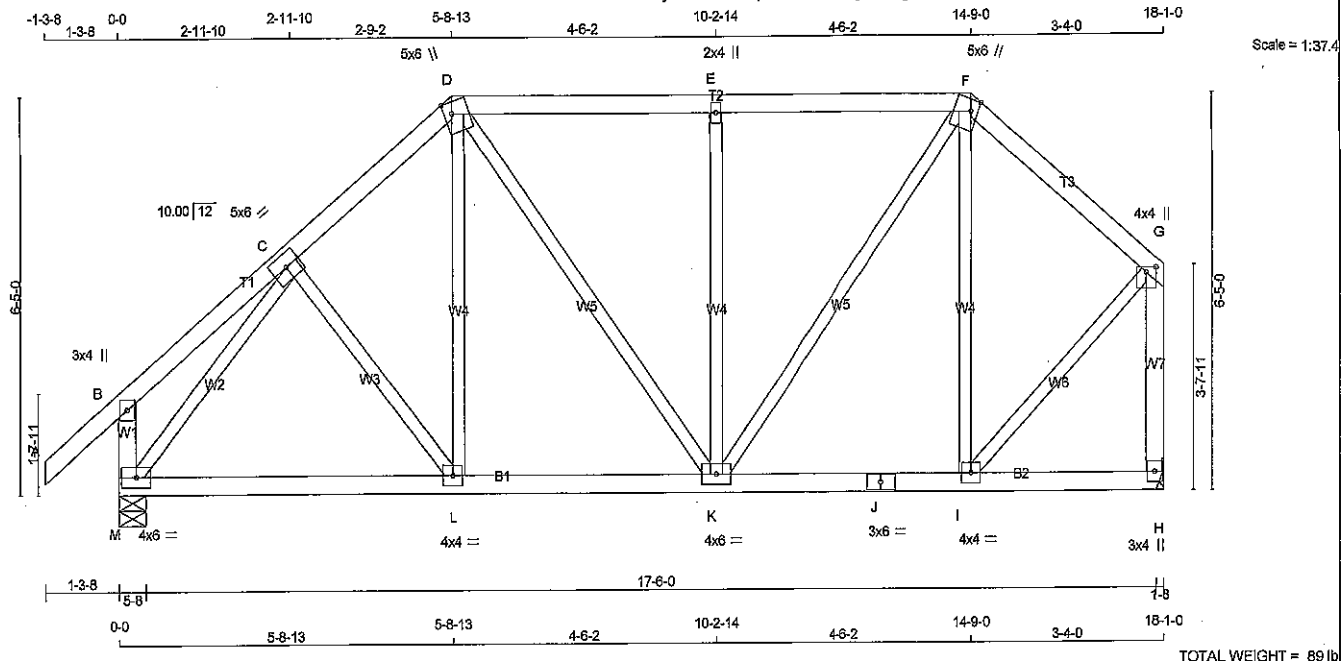

TOTAL WEIGHT = 89 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1347	0	1347	0	0	IN-SX	IN-SX
H	1202	0	1202	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1056	677 / 0	190 / 0	0 / 0	0 / 0	189 / 0	0 / 0
H	958	587 / 0	190 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 20	-104.9	-104.9	0.13 (1)	10.00	
C- D	-1043 / 0	-104.9	-104.9	0.14 (1)	5.99	
D- E	-916 / 0	-104.9	-104.9	0.37 (1)	5.92	
E- F	-916 / 0	-104.9	-104.9	0.37 (1)	5.92	
F- G	-720 / 0	-104.9	-104.9	0.21 (1)	6.25	
M- B	-262 / 0	0.0	0.0	0.03 (1)	7.81	
H- G	-1168 / 0	0.0	0.0	0.26 (1)	7.37	
M- L	0 / 787	-28.0	-28.0	0.29 (2)	10.00	
L- K	0 / 786	-28.0	-28.0	0.29 (2)	10.00	
K- J	0 / 544	-28.0	-28.0	0.17 (2)	10.00	
J- I	0 / 544	-28.0	-28.0	0.17 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- L	-6 / 86	0.02 (3)
L- D	0 / 246	0.06 (2)
D- K	0 / 220	0.05 (1)
K- E	-576 / 0	0.38 (1)
K- F	0 / 626	0.14 (1)
I- F	-401 / 0	0.27 (1)
M- C	-1266 / 0	0.44 (1)
I- G	0 / 765	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.29/1.00 (K-L:2) , WB=0.44/1.00 (C-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7270-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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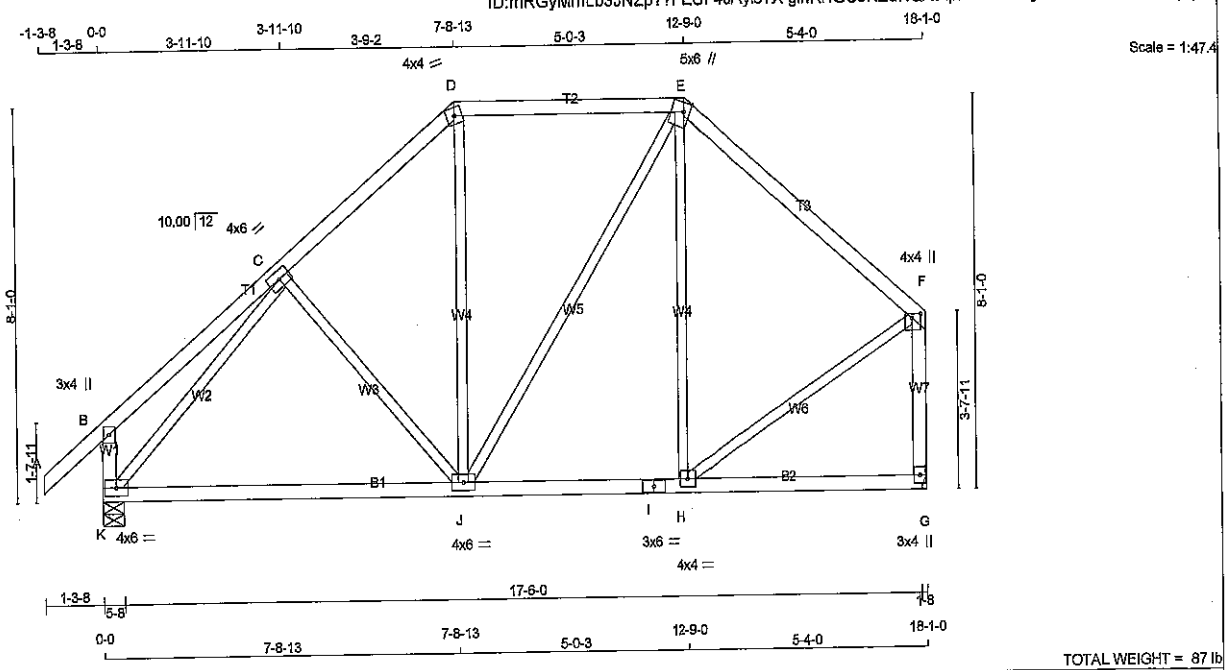

TOTAL WEIGHT = 87 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1347	0	1347	0	0	5-8	5-8
G	1202	0	1202	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1056	677 / 0	190 / 0	0 / 0	0 / 0	189 / 0	0 / 0
G	958	587 / 0	190 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B-C	0 / 30	-104.9	-104.9	0.25 (1)	10.00	
C-D	-943 / 0	-104.9	-104.9	0.20 (1)	6.15	
D-E	-702 / 0	-104.9	-104.9	0.34 (1)	6.25	
E-F	-773 / 0	-104.9	-104.9	0.39 (1)	6.25	
K-B	-299 / 0	0.0	0.0	0.03 (1)	7.81	
G-F	-1136 / 0	0.0	0.0	0.25 (1)	7.45	
K-J	0 / 815	-28.0	-28.0	0.44 (2)	10.00	
J-I	0 / 589	-28.0	-28.0	0.42 (2)	10.00	
I-H	0 / 589	-28.0	-28.0	0.42 (2)	10.00	
H-G	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-174 / 49	0.11 (1)
J-D	0 / 249	0.06 (2)
J-E	0 / 211	0.05 (1)
H-E	-225 / 76	0.28 (1)
K-C	-1257 / 0	0.74 (1)
H-F	0 / 893	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 869 (0.25")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.44/1.00 (J-K:2) , WB=0.74/1.00 (C-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM7271-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T23	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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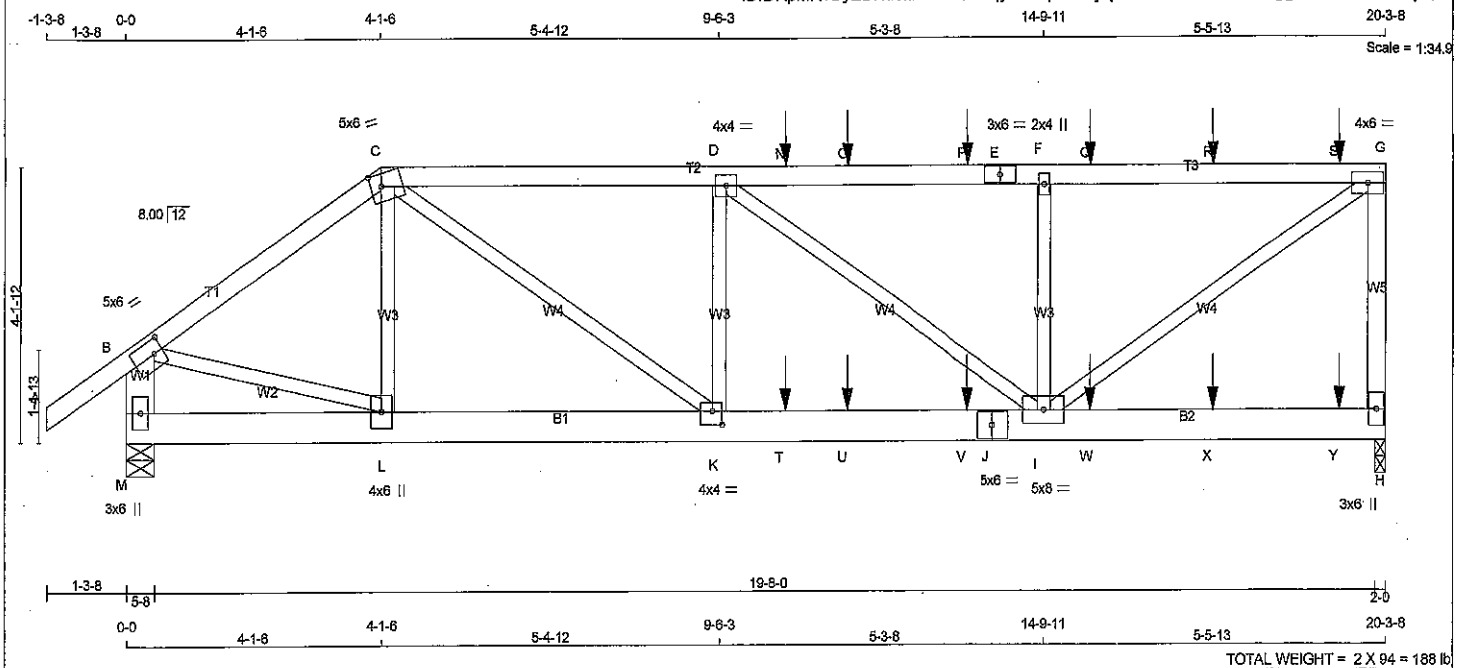


TOTAL WEIGHT = 2 X 94 = 188 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(0.0)
G- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.50	1.75
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	2108	0	2108	0	0	2-0	2-0
M	1774	0	1774	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1693	1017 / 0	349 / 0	0 / 0	0 / 0	327 / 0	0 / 0
M	1401	880 / 0	264 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)		10.00
B- C	-1816 / 0	-104.9	-104.9	0.19 (1)		6.22
C- D	-2624 / 0	-104.9	-104.9	0.33 (1)		5.22
D- N	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
N- O	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
O- P	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
P- E	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
E- F	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
F- Q	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
Q- R	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
R- S	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
S- G	-2297 / 0	-104.9	-104.9	0.45 (1)		5.29
H- G	-1978 / 0	0.0	0.0	0.25 (1)		7.81
M- B	-1727 / 0	0.0	0.0	0.05 (1)		7.81
M- L	0 / 0	-28.0	-28.0	0.03 (3)		10.00
L- K	0 / 1502	-28.0	-28.0	0.12 (1)		10.00
K- T	0 / 2624	-28.0	-28.0	0.22 (1)		10.00
T- U	0 / 2624	-28.0	-28.0	0.22 (1)		10.00
U- V	0 / 2624	-28.0	-28.0	0.22 (1)		10.00
V- J	0 / 2624	-28.0	-28.0	0.22 (1)		10.00
J- I	0 / 2624	-28.0	-28.0	0.22 (1)		10.00
I- W	0 / 0	-28.0	-28.0	0.09 (3)		10.00
W- X	0 / 0	-28.0	-28.0	0.09 (3)		10.00
X- Y	0 / 0	-28.0	-28.0	0.09 (3)		10.00
Y- H	0 / 0	-28.0	-28.0	0.09 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
L- C	-207 / 89	0.03 (1)
B- L	0 / 1550	0.19 (1)
C- K	0 / 1366	0.17 (1)
K- D	-574 / 37	0.07 (1)
D- I	-402 / 0	0.15 (1)
I- F	-1063 / 0	0.14 (1)
I- G	0 / 2788	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	10-6-12	-130	-130	—	BACK	VERT	TOTAL
O	11-6-12	-130	-130	—	BACK	VERT	TOTAL
P	13-6-12	-130	-130	—	BACK	VERT	TOTAL
Q	15-6-12	-130	-130	—	BACK	VERT	TOTAL
R	17-6-12	-130	-130	—	BACK	VERT	TOTAL
S	19-6-12	-144	-144	—	BACK	VERT	TOTAL
T	10-6-12	-41	-71	—	BACK	VERT	TOTAL
U	11-6-12	-41	-71	—	BACK	VERT	TOTAL
V	13-6-12	-41	-71	—	BACK	VERT	TOTAL
W	15-6-12	-41	-71	—	BACK	VERT	TOTAL
X	17-6-12	-41	-71	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.45/1.00 (F-G:1) , BC=0.22/1.00 (I-K:1) , WB=0.35/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

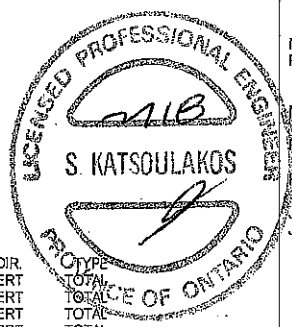
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.29 (G) (INPUT = 1.00)



DWG NO. TAM 7293-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T23	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 10-6-12, 129.6 lbs FACTORED DOWN AT 11-6-12, 129.6 lbs FACTORED DOWN AT 13-6-12, 129.6 lbs FACTORED DOWN AT 15-6-12, AND 129.6 lbs FACTORED DOWN AT 17-6-12, AND 143.8 lbs FACTORED DOWN AT 19-6-12 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 10-6-12, 71.5 lbs FACTORED DOWN AT 11-6-12, 71.5 lbs FACTORED DOWN AT 13-6-12, 71.5 lbs FACTORED DOWN AT 15-6-12, AND 71.5 lbs FACTORED DOWN AT 17-6-12, AND 78.1 lbs FACTORED DOWN AT 19-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	19-6-12	-45	-78	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7293-R8
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T23S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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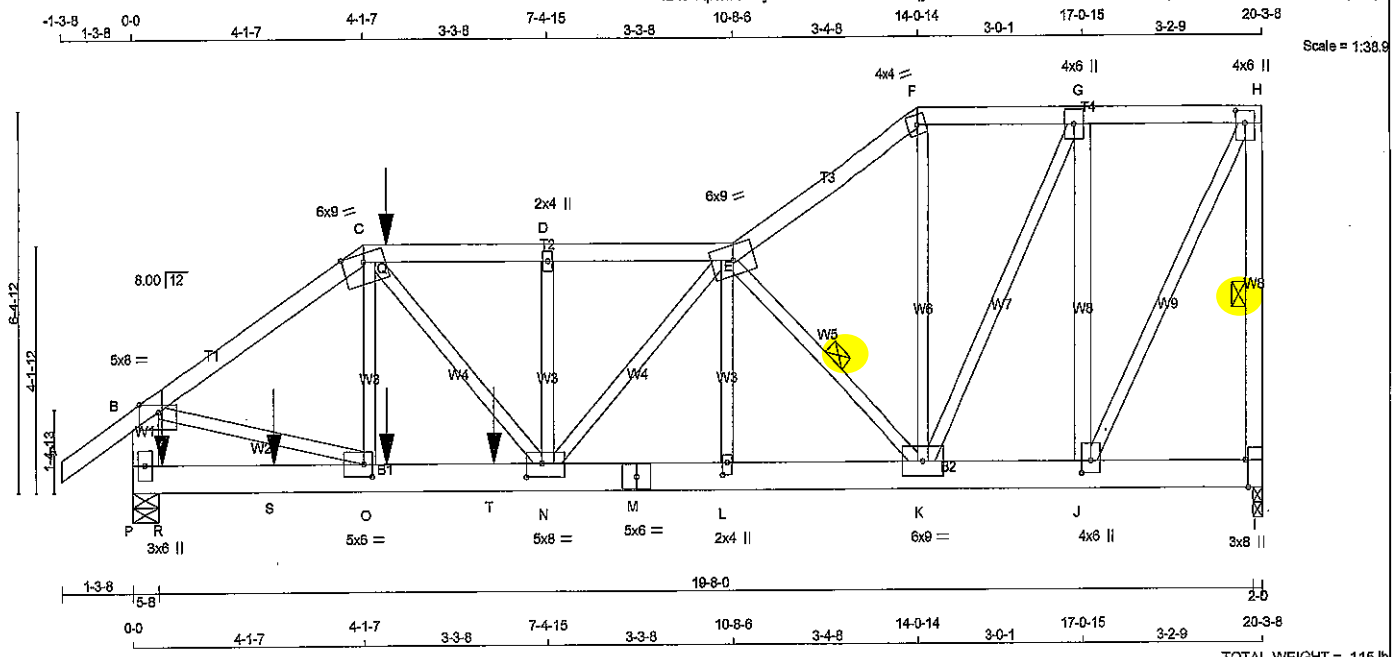

TOTAL WEIGHT = 115 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TTWWW-m	MT20	6.0	9.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMW+p	MT20	4.0	6.0	2.50	2.00
I	BMV1+t	MT20	3.0	8.0	Edge	0.50
J	BMWW+t	MT20	4.0	6.0	2.50	2.00
K	BMWWW-t	MT20	6.0	9.0		
L	BMW+w	MT20	2.0	4.0	2.50	1.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.75	3.25
O	BMWW-t	MT20	5.0	6.0	2.50	1.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 160.9 lbs FACTORED DOWN AT 4-6-4 ON TOP CHORD, AND 63.2 lbs FACTORED DOWN AT 6-4, 71.5 lbs FACTORED DOWN AT 2-6-4, AND 71.5 lbs FACTORED DOWN AT 4-6-4, AND 1359.7 lbs FACTORED DOWN AT 6-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1832	0	1832	0	0	2-0	2-0
P	2659	0	2659	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1459	897 / 0	287 / 0	0 / 0	0 / 0	274 / 0	0 / 0
P	2130	1288 / 0	434 / 0	0 / 0	0 / 0	408 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)		10.00
B- C	-2963 / 0	-104.9	-104.9	0.53 (1)		3.50
C- Q	-3429 / 0	-104.9	-104.9	0.37 (1)		3.42
Q- D	-3429 / 0	-104.9	-104.9	0.37 (1)		3.42
D- E	-3428 / 0	-104.9	-104.9	0.34 (1)		3.45
E- F	-1724 / 0	-104.9	-104.9	0.28 (1)		4.74
F- G	-1445 / 0	-104.9	-104.9	0.16 (1)		5.21
G- H	-881 / 0	-104.9	-104.9	0.17 (1)		6.25
I- H	-1788 / 0	0.0	0.0	0.34 (1)		4.96
P- B	-2591 / 0	0.0	0.0	0.19 (1)		6.41
P- R	0 / 0	-28.0	-28.0	0.15 (1)		10.00
R- S	0 / 0	-28.0	-28.0	0.15 (1)		10.00
S- O	0 / 0	-28.0	-28.0	0.15 (1)		10.00
O- T	0 / 2463	-28.0	-28.0	0.84 (1)		10.00
T- N	0 / 2463	-28.0	-28.0	0.84 (1)		10.00
N- M	0 / 3020	-28.0	-28.0	0.57 (1)		10.00
M- L	0 / 3020	-28.0	-28.0	0.57 (1)		10.00
L- K	0 / 3021	-28.0	-28.0	0.48 (1)		10.00
K- J	0 / 881	-28.0	-28.0	0.14 (1)		10.00
J- I	0 / 0	-28.0	-28.0	0.04 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 268	0.07 (3)
C- N	0 / 1466	0.36 (1)
N- D	-405 / 0	0.10 (1)
N- E	0 / 620	0.15 (1)
L- E	-38 / 111	0.03 (3)
E- K	-2335 / 0	0.52 (1)
K- F	0 / 648	0.16 (1)
K- G	0 / 1300	0.32 (1)
J- G	-1556 / 0	0.70 (1)
J- H	0 / 1672	0.46 (1)
B- O	0 / 2529	0.63 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
O	4-6-4	-41	-71	—	FRONT	VERT	TOTAL
Q	4-6-4	-161	-161	—	FRONT	VERT	TOTAL
R	6-4	-48	-83	—	FRONT	VERT	TOTAL
S	2-6-4	-41	-71	—	FRONT	VERT	TOTAL
T	6-5-8	-1360	-1360	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.84/1.00 (N-O:1) , WB=0.70/1.00 (G-J:1) , SSI=0.67/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7294 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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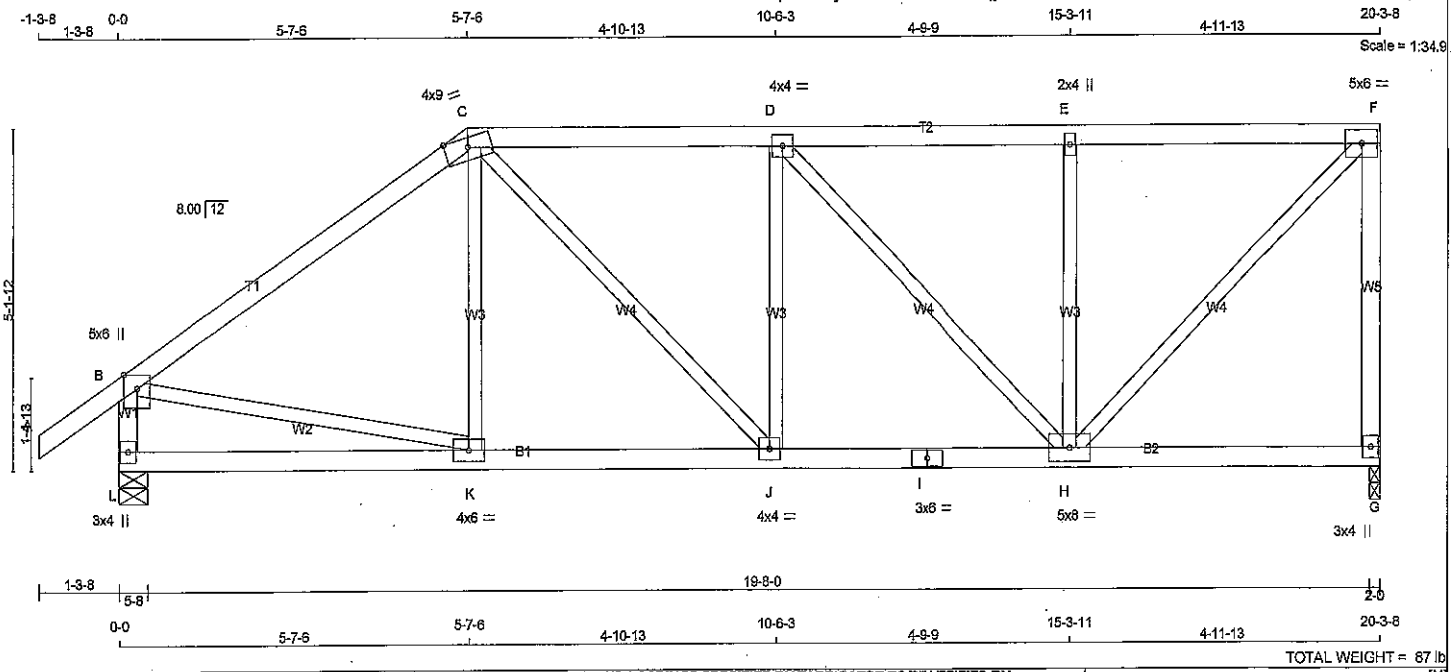

TOTAL WEIGHT = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00		K- C	-45 / 206	0.05 (3)
B- C	-1386 / 0	-104.9	-104.9	0.68 (1)	4.57		B- K	0 / 1172	0.28 (1)
C- D	-1463 / 0	-104.9	-104.9	0.38 (1)	4.96		C- J	0 / 439	0.10 (1)
D- E	-1107 / 0	-104.9	-104.9	0.36 (1)	5.53		J- D	-191 / 107	0.07 (1)
E- F	-1107 / 0	-104.9	-104.9	0.36 (1)	5.53		D- H	-508 / 0	0.43 (1)
G- F	-1291 / 0	0.0	0.0	0.58 (1)	7.10		H- E	-562 / 0	0.22 (1)
L- B	-1427 / 0	0.0	0.0	0.15 (1)	6.84		H- F	0 / 1545	0.35 (1)
L- K	0 / 0	-28.0	-28.0	0.21 (3)	10.00				
K- J	0 / 1151	-28.0	-28.0	0.33 (2)	10.00				
J- I	0 / 1463	-28.0	-28.0	0.32 (2)	10.00				
I- H	0 / 1463	-28.0	-28.0	0.32 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.68/1.00 (B-C:1) , BC=0.33/1.00 (J-K:2) , WB=0.43/1.00 (D-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7295-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T24S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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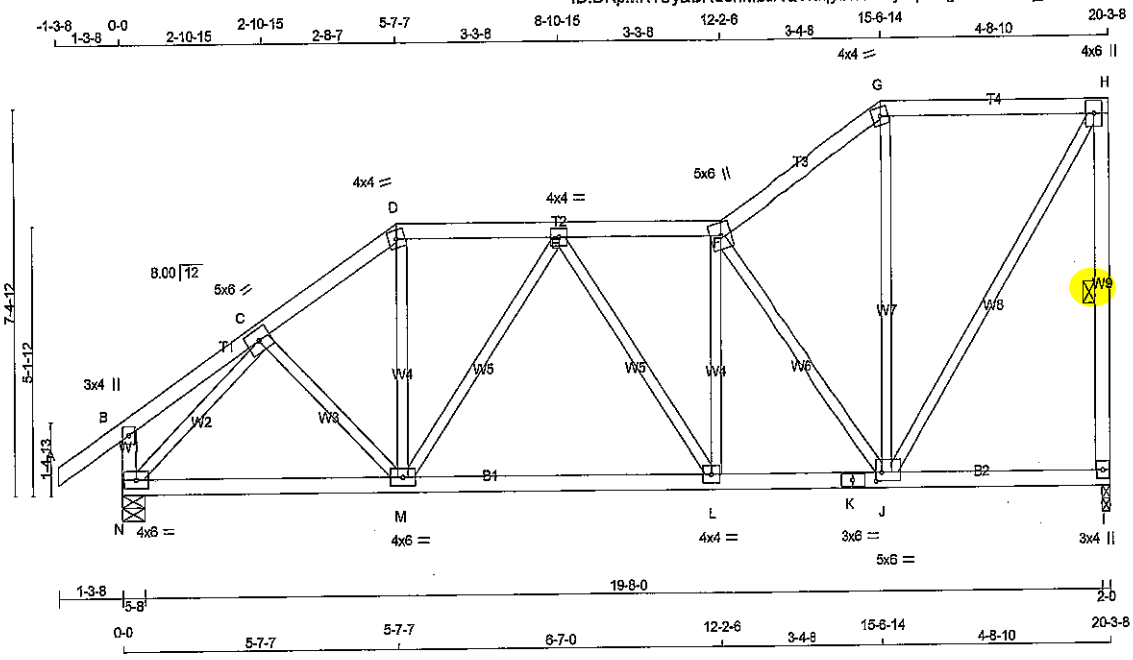

TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	6.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
N	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
N	1172	746 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00		C- M	0 / 123	0.03 (3)
B- C	0 / 15	-104.9	-104.9	0.11 (1)	10.00		M- D	0 / 514	0.12 (1)
C- D	-1404 / 0	-104.9	-104.9	0.13 (1)	5.37		M- E	-433 / 0	0.25 (1)
D- E	-1164 / 0	-104.9	-104.9	0.19 (1)	5.68		E- L	-59 / 54	0.03 (1)
E- F	-1369 / 0	-104.9	-104.9	0.20 (1)	5.33		L- F	0 / 215	0.05 (2)
F- G	-817 / 0	-104.9	-104.9	0.22 (1)	6.25		F- J	-1175 / 0	0.72 (1)
G- H	-680 / 0	-104.9	-104.9	0.41 (1)	6.25		J- G	0 / 167	0.04 (3)
I- H	-1293 / 0	0.0	0.0	0.32 (1)	5.68		J- H	0 / 1249	0.28 (1)
N- B	-263 / 0	0.0	0.0	0.03 (1)	7.81		N- C	-1610 / 0	0.43 (1)
N- M	0 / 1109	-28.0	-28.0	0.38 (2)	10.00				
M- L	0 / 1401	-28.0	-28.0	0.41 (2)	10.00				
L- K	0 / 1365	-28.0	-28.0	0.29 (1)	10.00				
K- J	0 / 1365	-28.0	-28.0	0.29 (1)	10.00				
J- I	0 / 0	-28.0	-28.0	0.13 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.41/1.00 (G-H:1) , BC=0.41/1.00 (L-M:2) , WB=0.72/1.00 (F-J:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7296-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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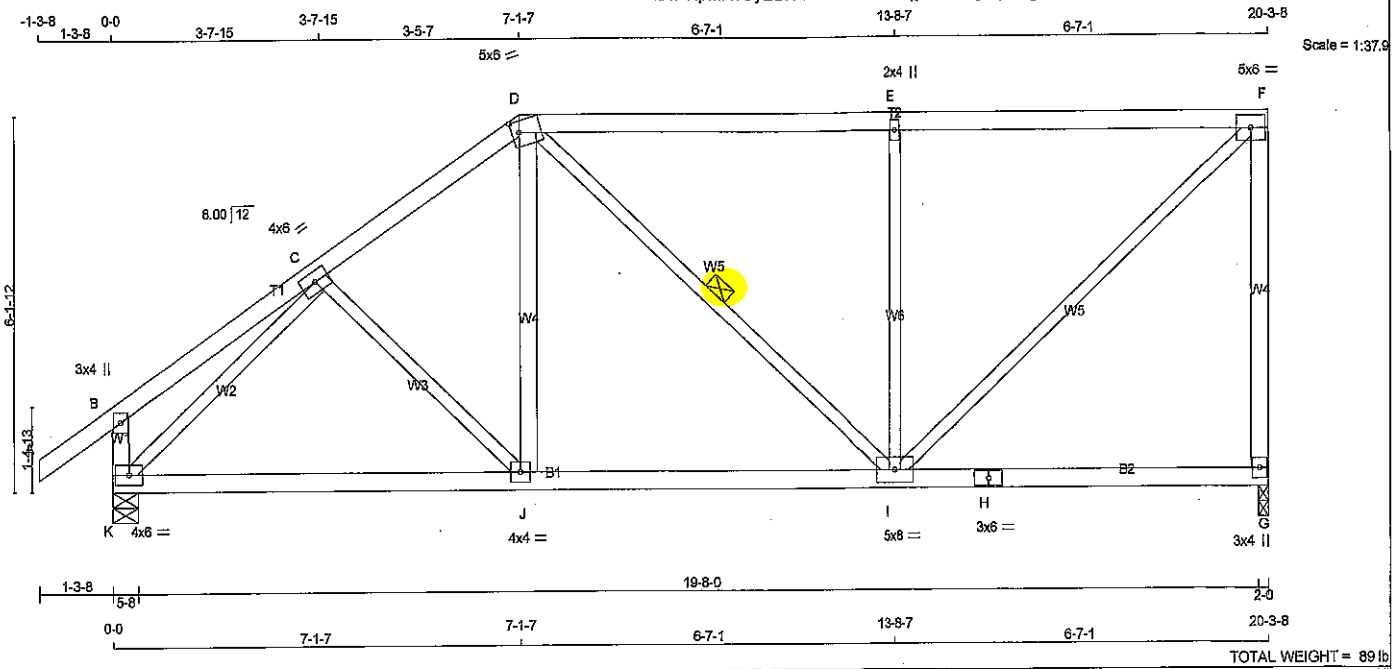

TOTAL WEIGHT = 89 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1349	0	1349	0	0	2-0	2-0
K	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
K	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- J	-93 / 85	0.04 (1)
B- C	0 / 23	-104.9	-104.9	0.20 (1)	10.00	J- D	0 / 349	0.06 (2)
C- D	-1330 / 0	-104.9	-104.9	0.23 (1)	5.35	D- I	-32 / 60	0.02 (3)
D- E	-1137 / 0	-104.9	-104.9	0.86 (1)	4.32	I- E	-857 / 0	0.51 (1)
E- F	-1137 / 0	-104.9	-104.9	0.86 (1)	4.32	I- F	0 / 1521	0.34 (1)
G- F	-1272 / 0	0.0	0.0	0.94 (1)	7.14	K- C	-1608 / 0	0.63 (1)
K- B	-289 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 1153	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 1092	-28.0	-28.0	0.45 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.27 (3)	10.00			
H- G	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (F-G:1) , BC=0.45/1.00 (I-J:2) , WB=0.63/1.00 (C-K:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7297-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T25S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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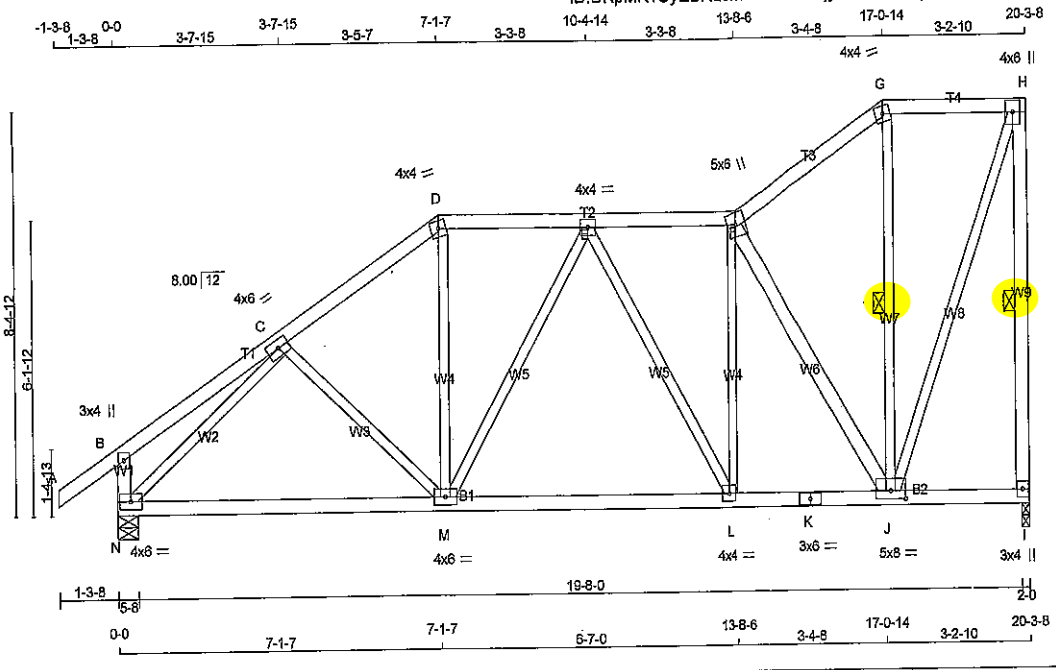

TOTAL WEIGHT = 108 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	4.00
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
N	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
N	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 22	-104.9	-104.9	0.20 (1)		10.00
C- D	-1333 / 0	-104.9	-104.9	0.17 (1)		5.44
D- E	-1096 / 0	-104.9	-104.9	0.15 (1)		5.88
E- F	-1042 / 0	-104.9	-104.9	0.14 (1)		6.00
F- G	-527 / 0	-104.9	-104.9	0.16 (1)		6.25
G- H	-438 / 0	-104.9	-104.9	0.14 (1)		6.25
I- H	-1307 / 0	0.0	0.0	0.42 (1)		5.85
N- B	-290 / 0	0.0	0.0	0.03 (1)		7.81
N- M	0 / 1155	-28.0	-28.0	0.47 (2)		10.00
M- L	0 / 1182	-28.0	-28.0	0.48 (2)		10.00
L- K	0 / 1035	-28.0	-28.0	0.24 (2)		10.00
K- J	0 / 1035	-28.0	-28.0	0.24 (2)		10.00
J- I	0 / 0	-28.0	-28.0	0.06 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- M	-92 / 86	0.04 (1)
M- D	0 / 444	0.10 (1)
M- E	-178 / 34	0.15 (1)
E- L	-291 / 0	0.24 (1)
L- F	0 / 393	0.09 (1)
F- J	-1166 / 0	0.71 (1)
J- G	-55 / 101	0.02 (1)
J- H	0 / 1222	0.27 (1)
N- C	-1610 / 0	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.42/1.00 (H-I:1) , BC=0.48/1.00 (L-M:2) , WB=0.71/1.00 (F-J:1) , SSI=0.17/1.00 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 729B-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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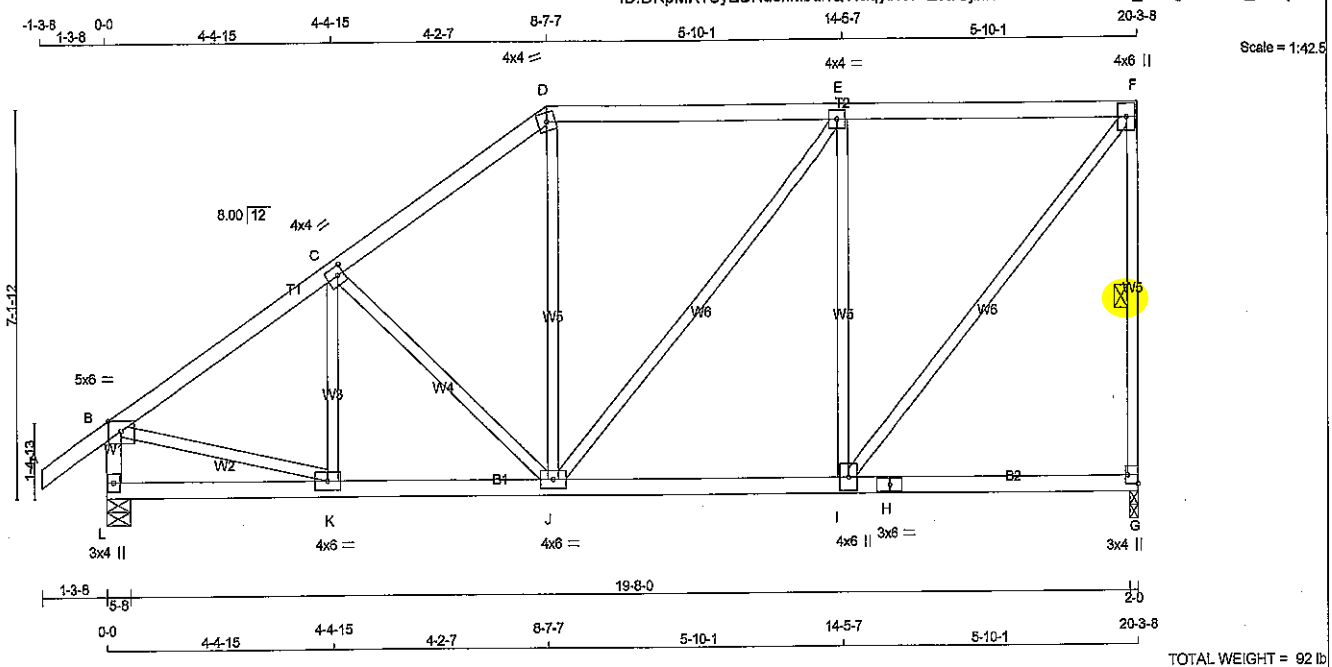

TOTAL WEIGHT = 92 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x3	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0	2.00	Edge
H	BS-t	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14	(1)	10.00	K- C	-172 / 105	0.05 (1)
B- C	-1443 / 0	-104.9	-104.9	0.37	(1)	5.00	C- J	-334 / 0	0.20 (1)
C- D	-1211 / 0	-104.9	-104.9	0.35	(1)	5.36	J- D	0 / 290	0.07 (2)
D- E	-984 / 0	-104.9	-104.9	0.65	(1)	5.19	J- E	0 / 139	0.03 (1)
E- F	-895 / 0	-104.9	-104.9	0.64	(1)	5.36	I- E	-866 / 0	0.77 (1)
G- F	-1283 / 0	0.0	0.0	0.41	(1)	4.99	I- F	0 / 1381	0.31 (1)
L- B	-1440 / 0	0.0	0.0	0.15	(1)	6.81	B- K	0 / 1261	0.28 (1)
L- K	0 / 0	-28.0	-28.0	0.12	(3)	10.00			
K- J	0 / 1224	-28.0	-28.0	0.27	(1)	10.00			
J- I	0 / 895	-28.0	-28.0	0.33	(2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.24	(3)	10.00			
H- G	0 / 0	-28.0	-28.0	0.24	(3)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.33/1.00 (I-J:2) , WB=0.77/1.00 (E-I:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7299 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T26S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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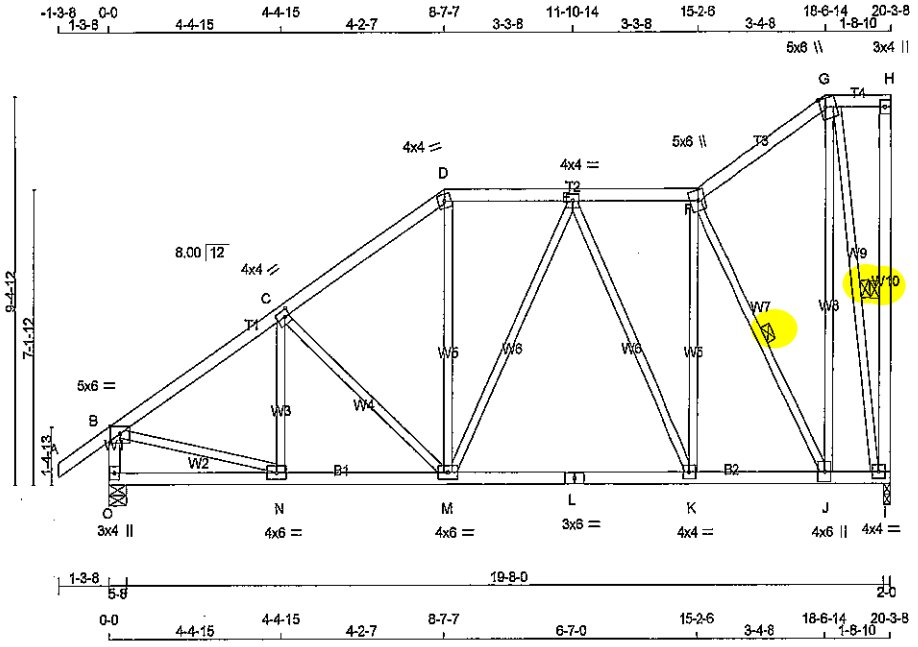

TOTAL WEIGHT = 116 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1349	0	1349	0	0	2-0	2-0
O	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
O	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, F-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1441 / 0	-104.9	-104.9	0.27 (1)	5.16
C- D	-1215 / 0	-104.9	-104.9	0.26 (1)	5.52
D- E	-989 / 0	-104.9	-104.9	0.14 (1)	6.12
E- F	-772 / 0	-104.9	-104.9	0.14 (1)	6.25
F- G	-258 / 0	-104.9	-104.9	0.15 (1)	6.25
G- H	0 / 0	-104.9	-104.9	0.05 (1)	10.00
I- H	-90 / 0	0.0	0.0	0.04 (1)	6.25
O- B	-1438 / 0	0.0	0.0	0.15 (1)	6.81
O- N	0 / 0	-28.0	-28.0	0.12 (2)	10.00
N- M	0 / 1223	-28.0	-28.0	0.31 (2)	10.00
M- L	0 / 977	-28.0	-28.0	0.29 (2)	10.00
L- K	0 / 977	-28.0	-28.0	0.29 (2)	10.00
K- J	0 / 763	-28.0	-28.0	0.28 (2)	10.00
J- I	0 / 227	-28.0	-28.0	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-180 / 92	0.05 (1)
C- M	-327 / 0	0.20 (1)
M- D	0 / 354	0.08 (1)
M- E	0 / 96	0.02 (3)
E- K	-486 / 0	0.56 (1)
K- F	0 / 619	0.14 (1)
F- J	-1209 / 0	0.48 (1)
J- G	0 / 1078	0.24 (1)
G- I	-1223 / 0	0.73 (1)
B- N	0 / 1259	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.31/1.00 (M-N:2) , WB=0.73/1.00 (G-I:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.42 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7300-S8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T27	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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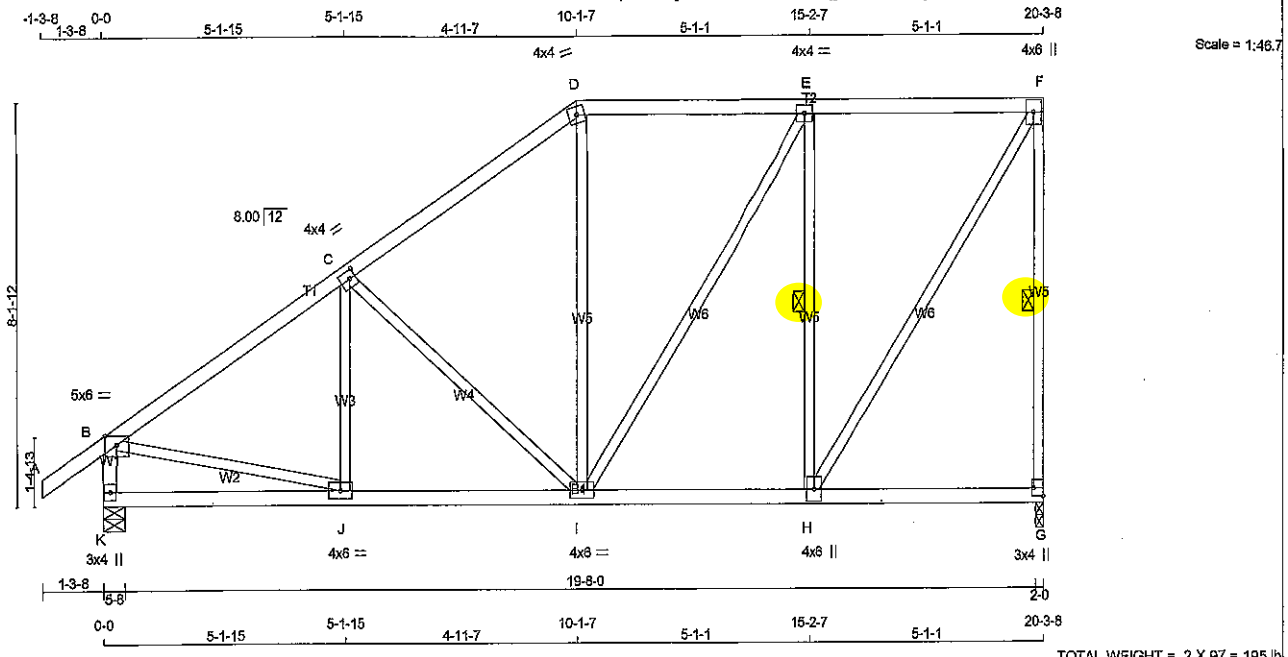

TOTAL WEIGHT = 2 X 97 = 195 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x3	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0	2.00	Edge
H	BMWW+t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1349	0	1349	0	0	2-0	2-0
K	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
K	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	J- C	-99 / 176	0.04 (3)	
B- C	-1448 / 0	-104.9	-104.9	0.53 (1)	4.76	C- I	-483 / 0	0.41 (1)	
C- D	-1088 / 0	-104.9	-104.9	0.49 (1)	5.35	I- D	0 / 241	0.05 (2)	
D- E	-876 / 0	-104.9	-104.9	0.47 (1)	5.81	I- E	0 / 320	0.07 (1)	
E- F	-705 / 0	-104.9	-104.9	0.47 (1)	6.25	H- E	-929 / 0	0.39 (1)	
G- F	-1291 / 0	0.0	0.0	0.54 (1)	4.97	H- F	0 / 1296	0.29 (1)	
K- B	-1433 / 0	0.0	0.0	0.15 (1)	6.82	B- J	0 / 1261	0.28 (1)	
K- J	0 / 0	-28.0	-28.0	0.18 (3)	10.00				
J- I	0 / 1234	-28.0	-28.0	0.31 (2)	10.00				
I- H	0 / 705	-28.0	-28.0	0.25 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.54/1.00 (F-G:1) , BC=0.31/1.00 (I-J:2) , WB=0.41/1.00 (C-I:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.31 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7230/-T8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T28	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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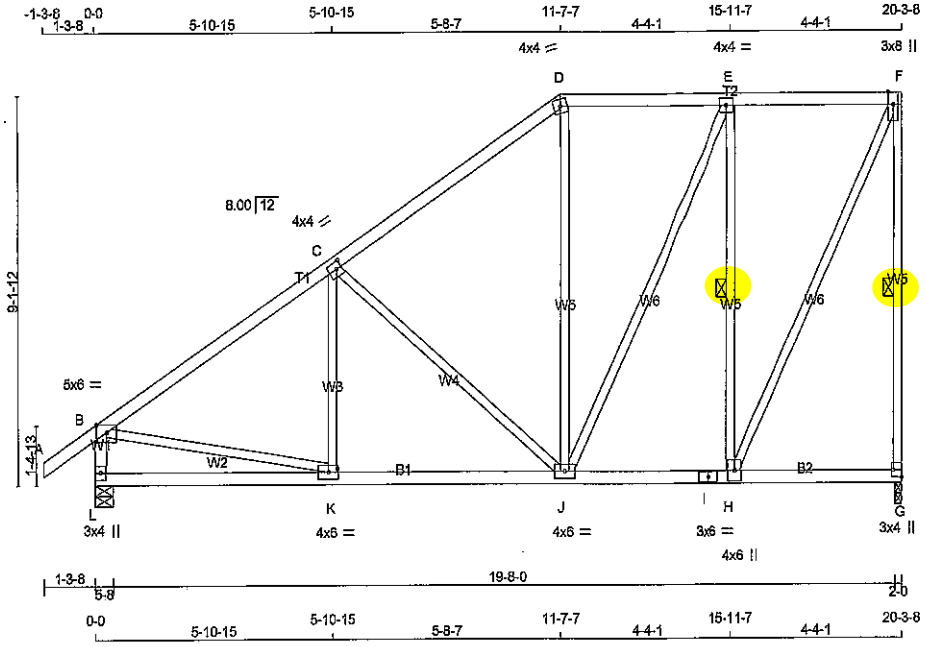

TOTAL WEIGHT = 2 X 103 = 206 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x3	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	3.0	8.0	Edge	
G	BMV1+p	MT20	3.0	4.0	2.00	Edge
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	2-0	2-0
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1172	746 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-1438 / 0	-104.9	-104.9	0.51 (1)		4.83
C- D	-954 / 0	-104.9	-104.9	0.47 (1)		5.69
D- E	-759 / 0	-104.9	-104.9	0.25 (1)		6.25
E- F	-550 / 0	-104.9	-104.9	0.25 (1)		6.25
G- F	-1298 / 0	0.0	0.0	0.71 (1)		4.97
L- B	-1426 / 0	0.0	0.0	0.15 (1)		6.84
L- K	0 / 0	-28.0	-28.0	0.25 (3)		10.00
K- J	0 / 1230	-28.0	-28.0	0.37 (2)		10.00
J- I	0 / 550	-28.0	-28.0	0.18 (2)		10.00
I- H	0 / 550	-28.0	-28.0	0.18 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.12 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-37 / 238	0.05 (3)
C- J	-624 / 0	0.75 (1)
J- D	0 / 189	0.04 (3)
J- E	0 / 483	0.11 (1)
H- E	-995 / 0	0.54 (1)
H- F	0 / 1251	0.28 (1)
B- K	0 / 1250	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.71/1.00 (F-G:1) , BC=0.37/1.00 (J-K:2) , WB=0.75/1.00 (C-J:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

DWG NO. TAM) 302-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T29	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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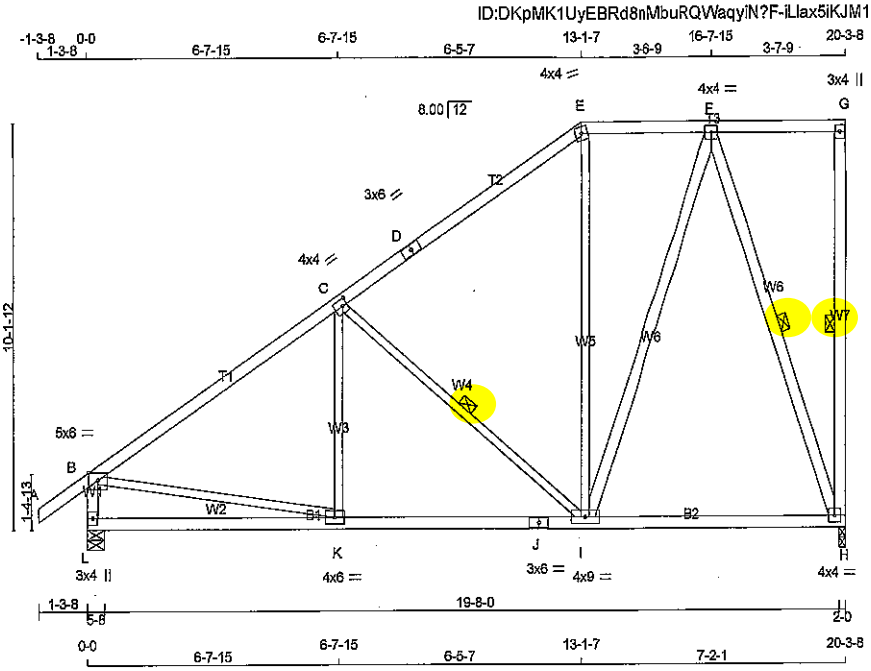

TOTAL WEIGHT = 2 X 110 = 221 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	2-0	2-0
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1411 / 0	-104.9	-104.9	0.66 (1)	4.57	
C- D	-821 / 0	-104.9	-104.9	0.61 (1)	5.71	
D- E	-821 / 0	-104.9	-104.9	0.61 (1)	5.71	
E- F	-643 / 0	-104.9	-104.9	0.18 (1)	6.25	
F- G	0 / 0	-104.9	-104.9	0.22 (1)	10.00	
H- G	-145 / 0	0.0	0.0	0.07 (1)	6.25	
L- B	-1415 / 0	0.0	0.0	0.15 (1)	6.85	
L- K	0 / 0	-28.0	-28.0	0.28 (3)	10.00	
K- J	0 / 1213	-28.0	-28.0	0.45 (2)	10.00	
J- I	0 / 1213	-28.0	-28.0	0.45 (2)	10.00	
I- H	0 / 414	-28.0	-28.0	0.38 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 267	0.06 (3)
C- I	-745 / 0	0.37 (1)
I- E	0 / 164	0.04 (3)
B- K	0 / 1229	0.28 (1)
I- F	0 / 897	0.11 (1)
F- H	-1197 / 0	0.69 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.45/1.00 (I-K:2) , WB=0.69/1.00 (F-H:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 73035-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T30	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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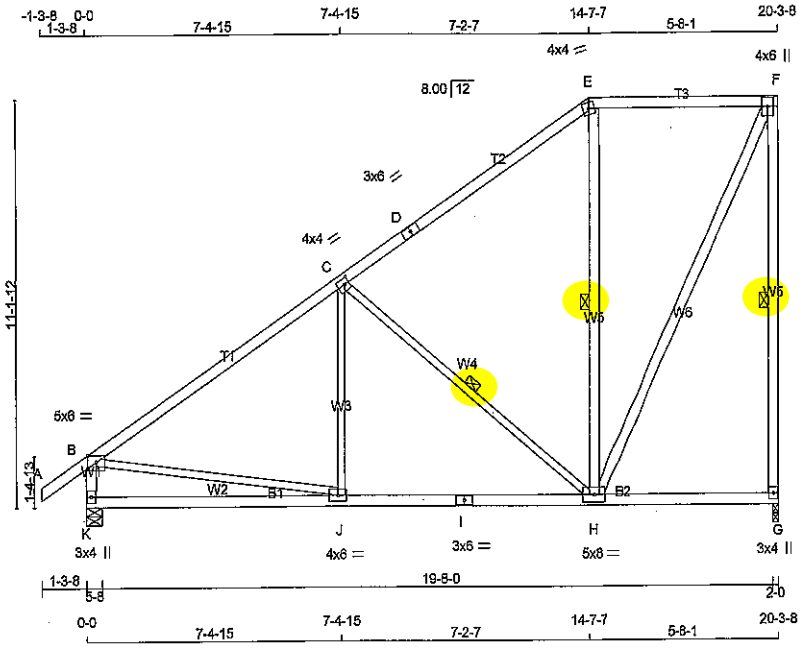

TOTAL WEIGHT = 2 X 108 = 216 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1349	0	1349	0	0	2-0	2-0
K	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
K	1172	748 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1383 / 0	-104.9	-104.9	0.84 (1)	4.13	
C- D	-666 / 0	-104.9	-104.9	0.76 (1)	5.66	
D- E	-666 / 0	-104.9	-104.9	0.76 (1)	5.66	
E- F	-506 / 0	-104.9	-104.9	0.44 (1)	6.25	
G- F	-1290 / 0	0.0	0.0	0.83 (1)	5.68	
K- B	-1409 / 0	0.0	0.0	0.15 (1)	6.87	
K- J	0 / 0	-28.0	-28.0	0.38 (3)	10.00	
J- I	0 / 1194	-28.0	-28.0	0.51 (2)	10.00	
I- H	0 / 1194	-28.0	-28.0	0.51 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 339	0.08 (3)
C- H	-865 / 0	0.56 (1)
H- E	-240 / 110	0.15 (1)
H- F	0 / 1115	0.18 (1)
B- J	0 / 1207	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.51/1.00 (H-J:2) , WB=0.56/1.00 (C-H:1) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7304 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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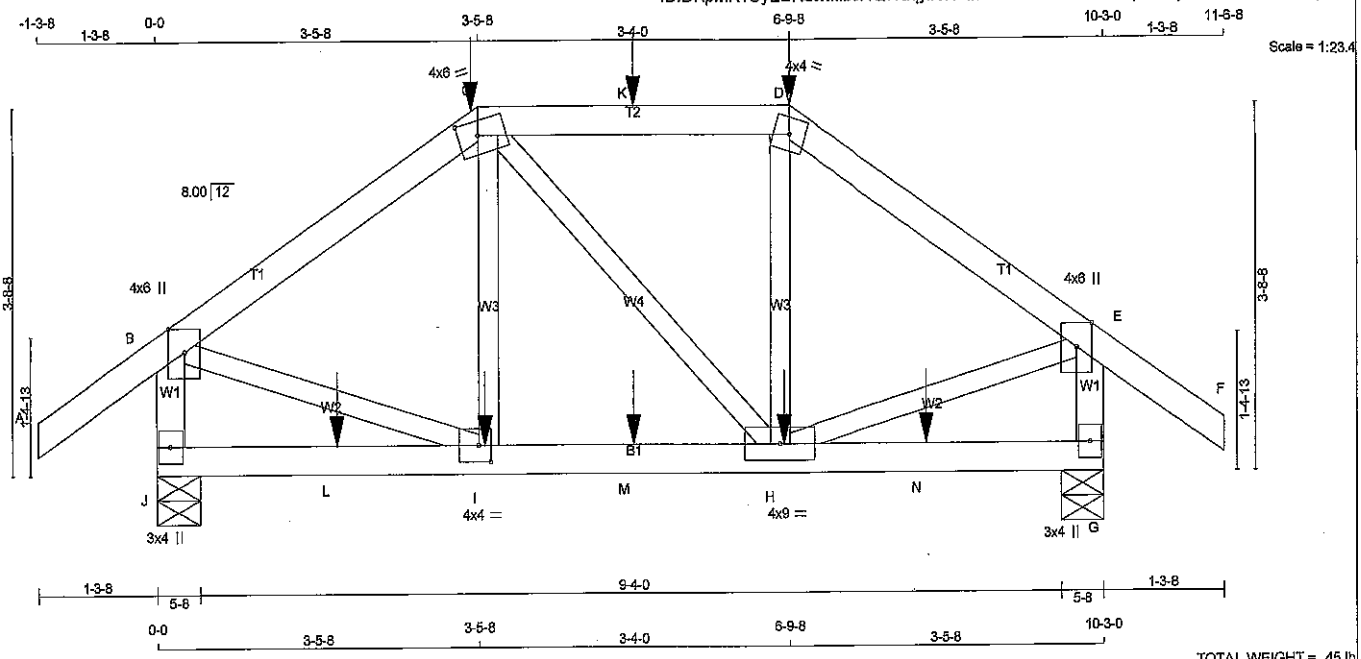

TOTAL WEIGHT = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 277.5 lbs FACTORED DOWN AT 3-5-8, AND 76.5 lbs FACTORED DOWN AT 5-1-8, AND 277.5 lbs FACTORED DOWN AT 6-9-8 ON TOP CHORD, AND 31.3 lbs FACTORED DOWN AT 1-11-4, 31.3 lbs FACTORED DOWN AT 3-6-4, 31.3 lbs FACTORED DOWN AT 5-1-8, AND 31.3 lbs FACTORED DOWN AT 6-8-12, AND 31.3 lbs FACTORED DOWN AT 8-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1186	0	1186	0	0	5-8	5-8
G	1186	0	1186	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	910	615 / 0	141 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	910	615 / 0	141 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	I- C	-113 / 153	0.04 (3)
B- C	-1003 / 0	-104.9	-104.9	0.26 (1)	5.88	C- H	0 / 0	0.00 (1)
C- K	-829 / 0	-104.9	-104.9	0.33 (1)	6.18	H- D	-114 / 154	0.04 (3)
K- D	-829 / 0	-104.9	-104.9	0.33 (1)	6.18	B- I	0 / 870	0.22 (1)
D- E	-1003 / 0	-104.9	-104.9	0.26 (1)	5.88	H- E	0 / 870	0.22 (1)
E- F	0 / 40	-104.9	-104.9	0.15 (1)	10.00			
J- B	-1137 / 0	0.0	0.0	0.13 (1)	7.42			
G- E	-1138 / 0	0.0	0.0	0.13 (1)	7.42			
J- L	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
L- I	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
I- M	0 / 829	-28.0	-28.0	0.19 (1)	10.00			
M- H	0 / 829	-28.0	-28.0	0.19 (1)	10.00			
H- N	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
N- G	0 / 0	-28.0	-28.0	0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-5-8	-278	-278	---	BACK	VERT	TOTAL
D	6-9-8	-278	-278	---	BACK	VERT	TOTAL
H	6-8-12	-18	-31	---	BACK	VERT	TOTAL
I	3-6-4	-18	-31	---	BACK	VERT	TOTAL
K	5-1-8	-77	-77	---	BACK	VERT	TOTAL
L	1-11-4	-18	-31	---	BACK	VERT	TOTAL
M	5-1-8	-18	-31	---	BACK	VERT	TOTAL
N	8-3-12	-18	-31	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	53.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.19/1.00 (H-I:1) , WB=0.22/1.00 (B-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

LICENCED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7305-M
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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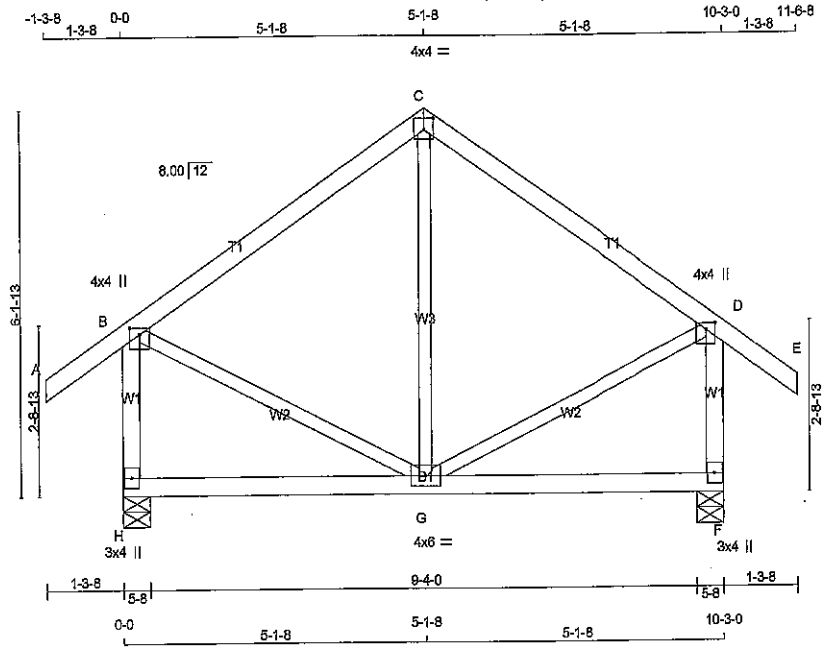

TOTAL WEIGHT = 48 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	825	0	825	0	0	5-8	5-8
F	825	0	825	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	640	422 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0
F	640	422 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	G- C	-119 / 167	0.07 (1)
B- C	-378 / 0	-104.9	-104.9	0.35 (1)	6.25	B- G	0 / 347	0.08 (1)
C- D	-378 / 0	-104.9	-104.9	0.35 (1)	6.25	G- D	0 / 347	0.08 (1)
D- E	0 / 40	-104.9	-104.9	0.14 (1)	10.00			
H- B	-770 / 0	0.0	0.0	0.11 (1)	7.81			
F- D	-770 / 0	0.0	0.0	0.11 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.22/1.00 (G-H:3) , WB=0.08/1.00 (B-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (D) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

DWG NO. TAM7306-R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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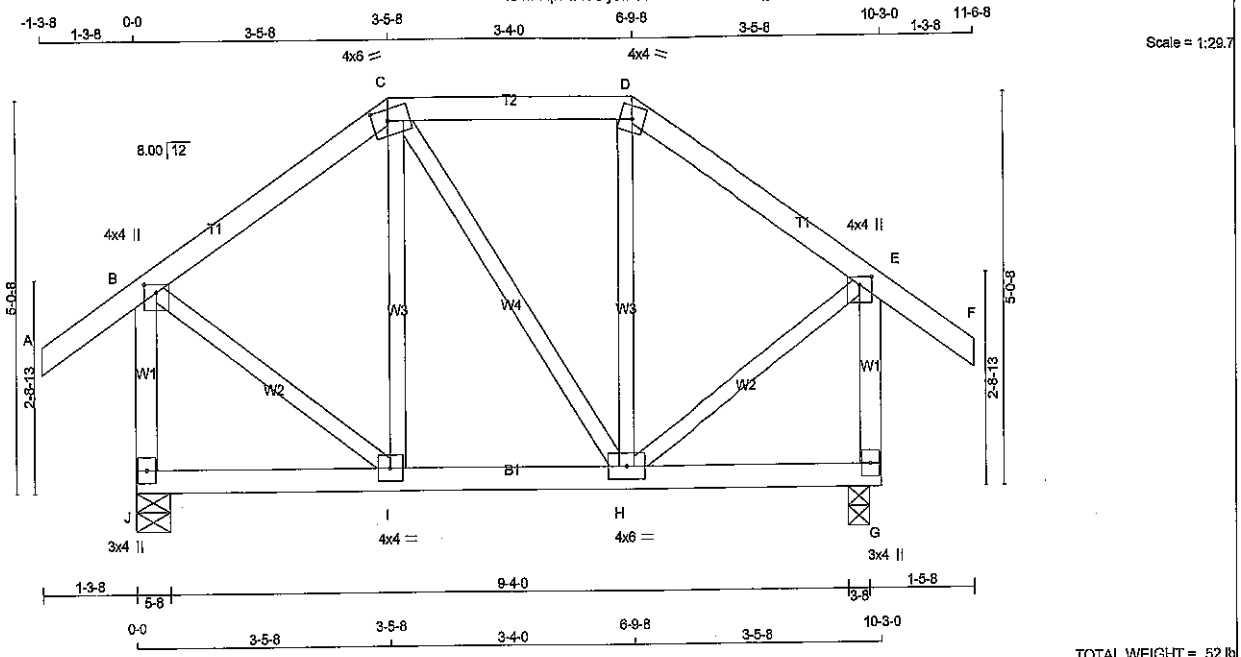

TOTAL WEIGHT = 52 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	825	0	825	0	0	5-8	5-8
G	825	0	825	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	640	422 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0
G	640	422 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00		I- C	-128 / 76	0.05 (1)
B- C	-409 / 0	-104.9	-104.9	0.16 (1)	6.25		C- H	0 / 0	0.00 (1)
C- D	-336 / 0	-104.9	-104.9	0.15 (1)	6.25		H- D	-129 / 76	0.05 (1)
D- E	-409 / 0	-104.9	-104.9	0.16 (1)	6.25		B- I	0 / 410	0.09 (1)
E- F	0 / 40	-104.9	-104.9	0.14 (1)	10.00		H- E	0 / 410	0.09 (1)
J- B	-784 / 0	0.0	0.0	0.11 (1)	7.81				
G- E	-783 / 0	0.0	0.0	0.11 (1)	7.81				
J- I	0 / 0	-28.0	-28.0	0.07 (3)	10.00				
I- H	0 / 337	-28.0	-28.0	0.11 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.08 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.11/1.00 (H-I:2) , WB=0.09/1.00 (B-I:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

DWG NO. TAM 7307 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T38A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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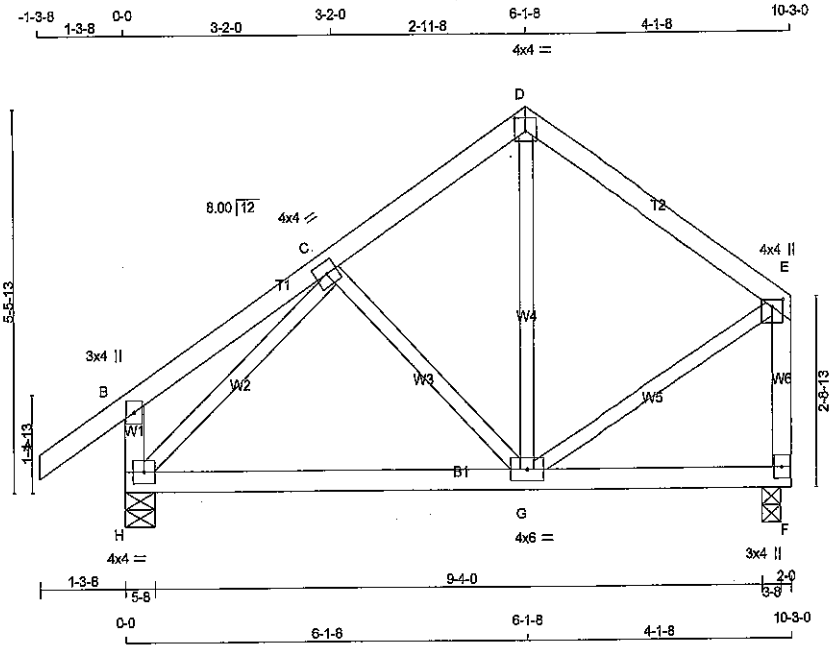

TOTAL WEIGHT = 2 X 46 = 91 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	681	0	681	0	0	3-8	3-8
H	825	0	825	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	543	333 / 0	108 / 0	0 / 0	0 / 0	102 / 0	0 / 0
H	640	422 / 0	108 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 20	-104.9	-104.9	0.18 (1)		10.00
C- D	-433 / 0	-104.9	-104.9	0.12 (1)		6.25
D- E	-413 / 0	-104.9	-104.9	0.23 (1)		6.25
H- B	-267 / 0	0.0	0.0	0.03 (1)		7.81
F- E	-647 / 0	0.0	0.0	0.09 (1)		7.81
H- G	0 / 476	-28.0	-28.0	0.28 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-191 / 25	0.06 (1)
G- D	0 / 204	0.05 (2)
H- C	-683 / 0	0.21 (1)
G- E	0 / 396	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.28/1.00 (G-H:2) , WB=0.21/1.00 (C-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (C) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

DWG NO. TAM 7308-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T39	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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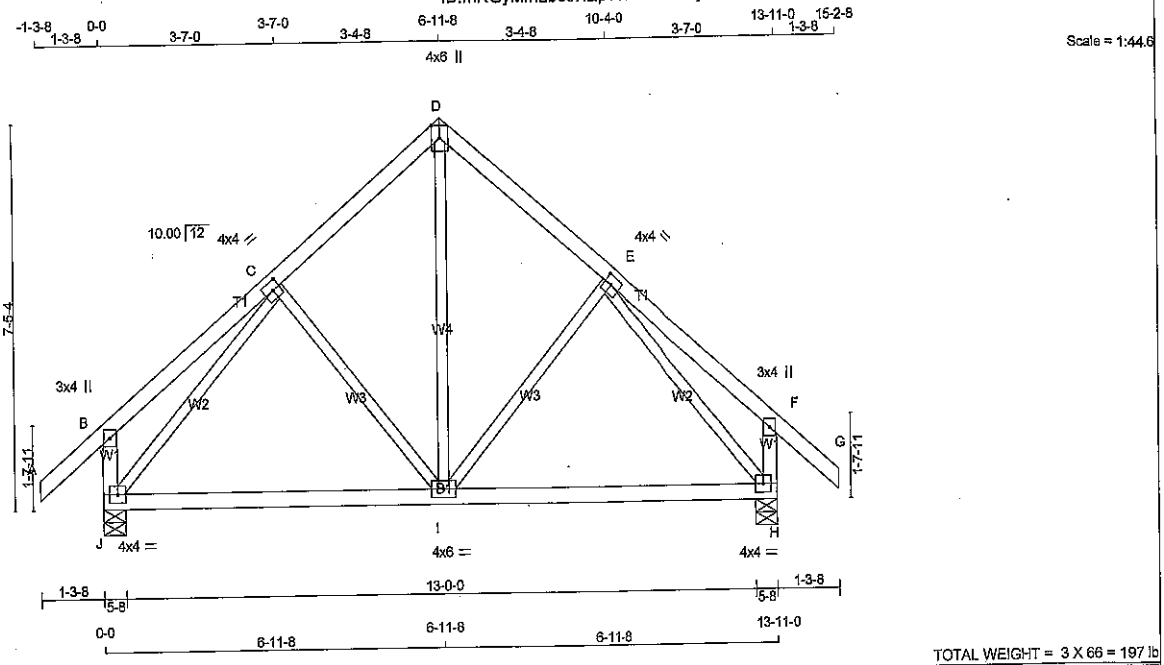

TOTAL WEIGHT = 3 X 66 = 197 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1070	0	1070	0	0	5-8	5-8
H	1070	0	1070	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	836	542 / 0	146 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	836	542 / 0	146 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		I- D	0 / 507	0.11 (1)
B- C	0 / 27	-104.9	-104.9	0.20 (1)	10.00		I- E	-174 / 38	0.09 (1)
C- D	-646 / 0	-104.9	-104.9	0.16 (1)	6.25		C- I	-174 / 38	0.09 (1)
D- E	-646 / 0	-104.9	-104.9	0.16 (1)	6.25		J- C	-923 / 0	0.44 (1)
E- F	0 / 27	-104.9	-104.9	0.20 (1)	10.00		E- H	-923 / 0	0.44 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00				
J- B	-284 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-284 / 0	0.0	0.0	0.03 (1)	7.81				
J- I	0 / 587	-28.0	-28.0	0.45 (2)	10.00				
I- H	0 / 587	-28.0	-28.0	0.45 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.45/1.00 (I-J:2) , WB=0.44/1.00 (C-J:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

DWG NO. TAM 7245-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T39A	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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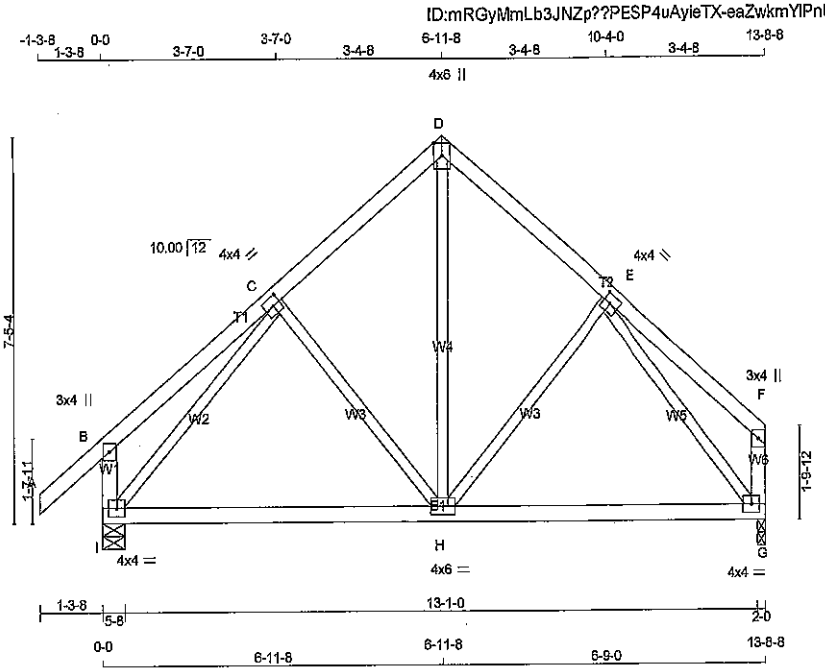


TOTAL WEIGHT = 5 X 63 = 317 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1056	0	1056	0	0	5-8	5-8
G	911	0	911	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	824	535 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	726	445 / 0	144 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 27	-104.9	-104.9	0.20 (1)		10.00
C- D	-627 / 0	-104.9	-104.9	0.16 (1)		6.25
D- E	-625 / 0	-104.9	-104.9	0.15 (1)		6.25
E- F	0 / 27	-104.9	-104.9	0.19 (1)		10.00
I- B	-284 / 0	0.0	0.0	0.03 (1)		7.81
G- F	-128 / 0	0.0	0.0	0.01 (1)		7.81
I- H	0 / 575	-28.0	-28.0	0.44 (2)		10.00
H- G	0 / 552	-28.0	-28.0	0.44 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- H	-177 / 36	0.09 (1)
H- D	0 / 481	0.11 (1)
H- E	-141 / 47	0.07 (1)
I- C	-904 / 0	0.43 (1)
E- G	-901 / 0	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.44/1.00 (H-I:2) , WB=0.43/1.00 (C-I:1) , SSI=0.16/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

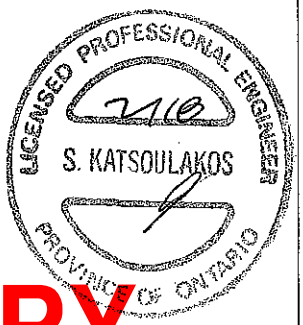
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)



DWG NO. TAM 7246-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T40A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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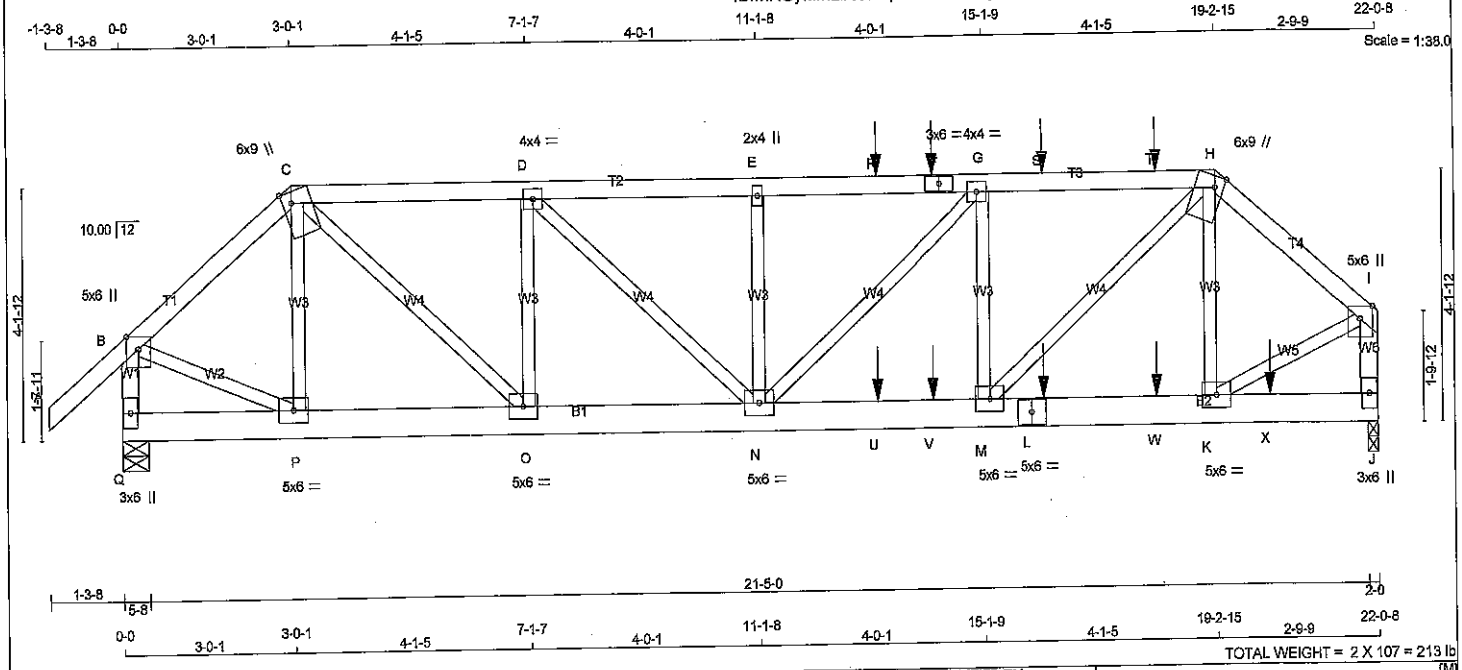

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	SIDE(0.0)
F-H	1	12	SIDE(0.0)
H-I	1	12	TOP
Q-B	1	12	TOP
J-I	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-L	2	12	SIDE(0.0)
L-J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMWW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K, M, O, P						
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1818	0	1818	0	0	5-8	5-8
J	1980	0	1980	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1436	901 / 0	271 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1603	941 / 0	345 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B-C	-1574 / 0	-104.9	-104.9	0.10 (1)	6.25
C-D	-2305 / 0	-104.9	-104.9	0.15 (1)	5.74
D-E	-2780 / 0	-104.9	-104.9	0.16 (1)	5.33
E-R	-2780 / 0	-104.9	-104.9	0.28 (1)	5.16
R-F	-2780 / 0	-104.9	-104.9	0.28 (1)	5.16
F-G	-2780 / 0	-104.9	-104.9	0.28 (1)	5.16
G-S	-2635 / 0	-104.9	-104.9	0.28 (1)	5.27
S-T	-2635 / 0	-104.9	-104.9	0.28 (1)	5.27
T-H	-2635 / 0	-104.9	-104.9	0.28 (1)	5.27
H-I	-1785 / 0	-104.9	-104.9	0.09 (1)	6.25
Q-B	-1789 / 0	0.0	0.0	0.10 (1)	7.81
J-I	-1951 / 0	0.0	0.0	0.11 (1)	7.81
Q-P	0 / 0	-28.0	-28.0	0.02 (2)	10.00
P-O	0 / 1195	-28.0	-28.0	0.09 (1)	10.00
O-N	0 / 2305	-28.0	-28.0	0.18 (1)	10.00
N-U	0 / 2636	-28.0	-28.0	0.22 (1)	10.00
U-V	0 / 2636	-28.0	-28.0	0.22 (1)	10.00
V-M	0 / 2636	-28.0	-28.0	0.22 (1)	10.00
M-L	0 / 1353	-28.0	-28.0	0.12 (1)	10.00
L-W	0 / 1353	-28.0	-28.0	0.12 (1)	10.00
W-K	0 / 1353	-28.0	-28.0	0.12 (1)	10.00
K-X	0 / 0	-28.0	-28.0	0.04 (2)	10.00
X-J	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-345 / 0	0.04 (1)
C-O	0 / 1502	0.19 (1)
O-D	-907 / 0	0.12 (1)
D-N	0 / 653	0.08 (1)
N-E	-457 / 0	0.06 (1)
N-G	0 / 199	0.02 (1)
M-G	-970 / 0	0.12 (1)
M-H	0 / 1736	0.21 (1)
K-H	-449 / 11	0.06 (1)
B-P	0 / 1294	0.16 (1)
K-I	0 / 1515	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	14-2-4	-130	-130	—	FRONT	VERT	TOTAL
L	16-2-4	-41	-71	—	FRONT	VERT	TOTAL
R	13-2-4	-130	-130	—	FRONT	VERT	TOTAL
S	16-2-4	-130	-130	—	FRONT	VERT	TOTAL
T	18-2-4	-130	-130	—	FRONT	VERT	TOTAL
U	13-2-4	-41	-71	—	FRONT	VERT	TOTAL
V	14-2-4	-41	-71	—	FRONT	VERT	TOTAL
W	18-2-4	-41	-71	—	FRONT	VERT	TOTAL
X	20-2-4	-41	-71	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28/1.00 (E-G:1) , BC=0.22/1.00 (M-N:1) , WB=0.21/1.00 (H-M:1) , SSI=0.18/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (D) (INPUT = 0.90)
JSI METAL= 0.17 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 72247-S8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T40A	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 13-2-4, 129.6 lbs FACTORED DOWN AT 14-2-4, AND 129.6 lbs FACTORED DOWN AT 16-2-4, AND 129.6 lbs FACTORED DOWN AT 18-2-4 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 13-2-4, 71.5 lbs FACTORED DOWN AT 14-2-4, 71.5 lbs FACTORED DOWN AT 16-2-4, AND 71.5 lbs FACTORED DOWN AT 18-2-4, AND 71.5 lbs FACTORED DOWN AT 20-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
7148
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7247-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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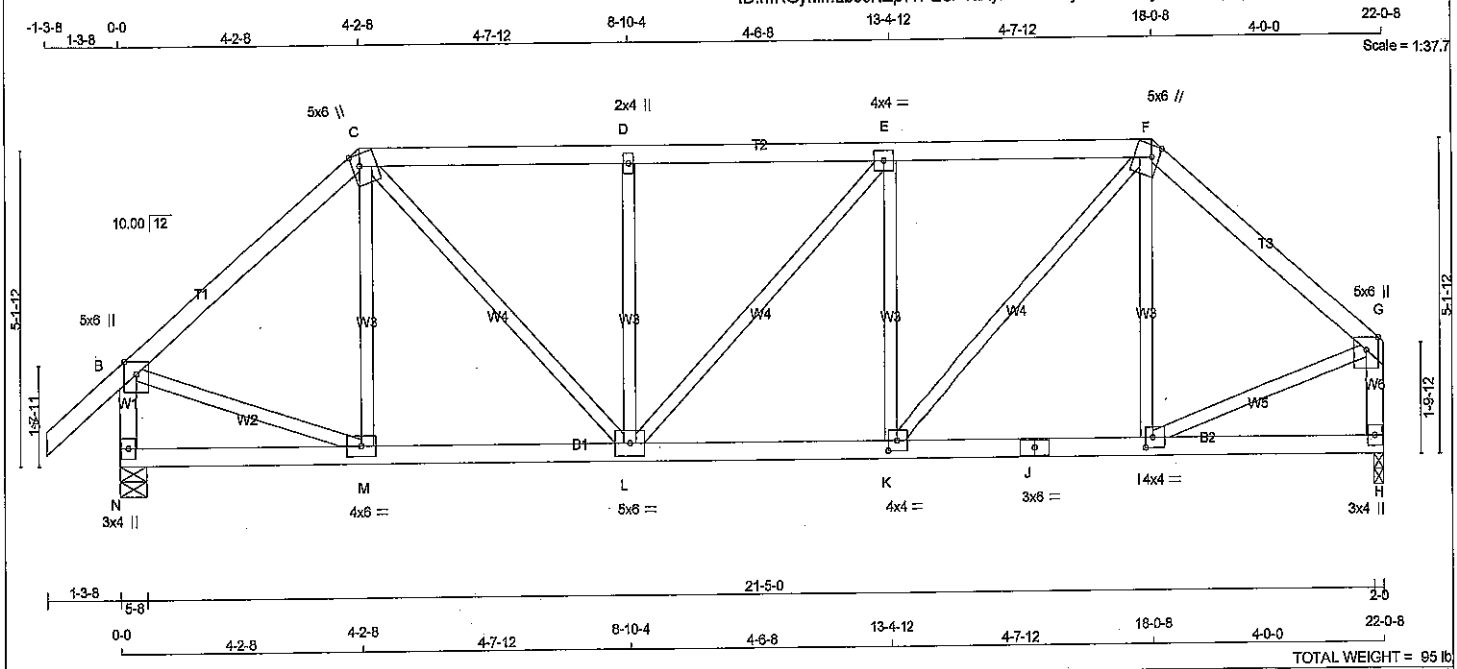

TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1610	0	1610	0	0	5-8	5-8
H	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1266	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0
H	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1369 / 0	-104.9	-104.9	0.37 (1)	5.10	
C- D	-1648 / 0	-104.9	-104.9	0.34 (1)	4.78	
D- E	-1648 / 0	-104.9	-104.9	0.34 (1)	4.77	
E- F	-1637 / 0	-104.9	-104.9	0.34 (1)	4.79	
F- G	-1317 / 0	-104.9	-104.9	0.33 (1)	5.24	
N- B	-1563 / 0	0.0	0.0	0.17 (1)	6.59	
H- G	-1420 / 0	0.0	0.0	0.16 (1)	6.84	
N- M	0 / 0	-28.0	-28.0	0.13 (2)	10.00	
M- L	0 / 1045	-28.0	-28.0	0.25 (2)	10.00	
L- K	0 / 1638	-28.0	-28.0	0.32 (1)	10.00	
K- J	0 / 1004	-28.0	-28.0	0.23 (2)	10.00	
J- I	0 / 1004	-28.0	-28.0	0.23 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.12 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-176 / 105	0.07 (1)
C- L	0 / 871	0.20 (1)
L- D	-518 / 0	0.20 (1)
L- E	0 / 16	0.00 (1)
K- E	-533 / 0	0.21 (1)
K- F	0 / 913	0.21 (1)
I- F	-227 / 78	0.09 (1)
B- M	0 / 1096	0.25 (1)
I- G	0 / 1073	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.32/1.00 (K-L:1) , WB=0.25/1.00 (B-M:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

DWG NO. TAM7 2486

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T42A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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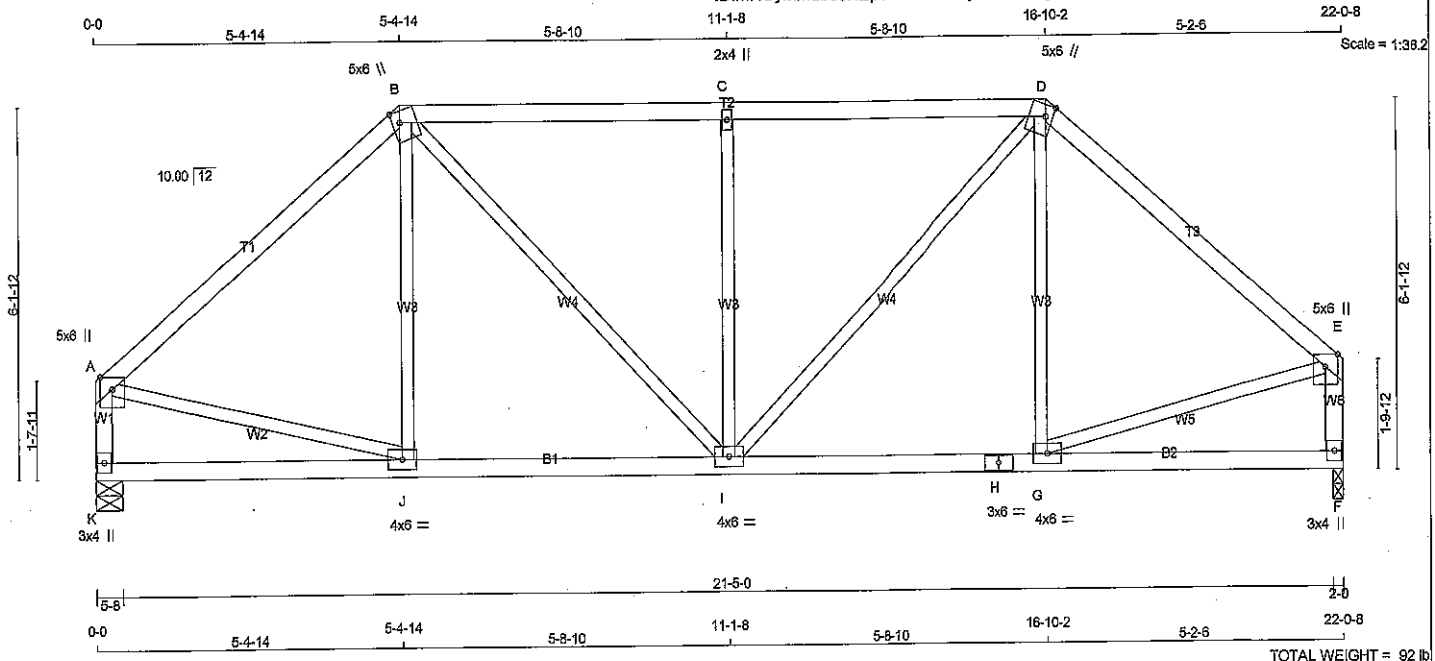

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1465	0	1465	0	0	5-8	5-8
F	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
F	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1360 / 0	-104.9	-104.9	0.64 (1)		4.71
B- C	-1458 / 0	-104.9	-104.9	0.65 (1)		4.47
C- D	-1458 / 0	-104.9	-104.9	0.65 (1)		4.47
D- E	-1325 / 0	-104.9	-104.9	0.58 (1)		4.85
K- A	-1404 / 0	0.0	0.0	0.15 (1)		6.87
F- E	-1407 / 0	0.0	0.0	0.15 (1)		6.87
K- J	0 / 0	-28.0	-28.0	0.21 (3)		10.00
J- I	0 / 1042	-28.0	-28.0	0.33 (2)		10.00
I- H	0 / 1014	-28.0	-28.0	0.32 (2)		10.00
H- G	0 / 1014	-28.0	-28.0	0.32 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.20 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-69 / 200	0.04 (3)
B- I	0 / 598	0.13 (1)
I- C	-733 / 0	0.44 (1)
I- D	0 / 635	0.14 (1)
G- D	-109 / 177	0.06 (1)
A- J	0 / 1072	0.24 (1)
G- E	0 / 1055	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.65/1.00 (B-C:1) , BC=0.33/1.00 (I-J:2) , WB=0.44/1.00 (C-I:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)

DWG NO. TAM 72499-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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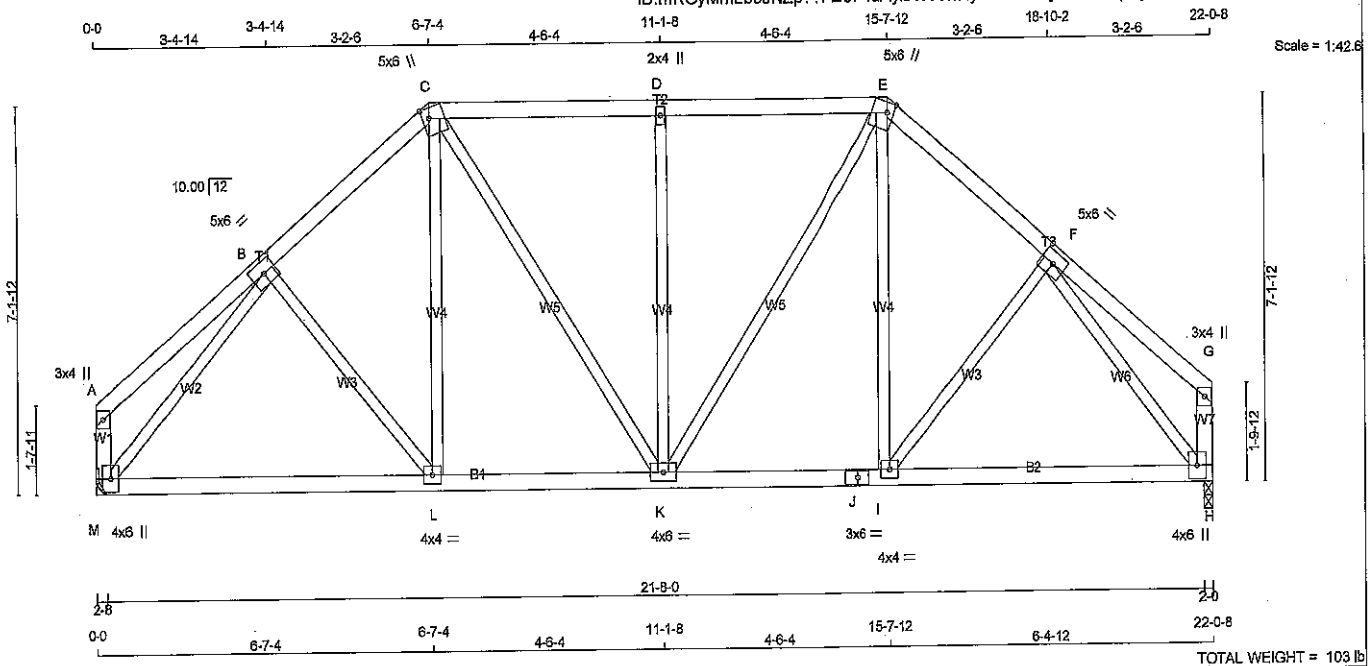

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1465	0	1465	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-8	
H	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1166	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
H	1166	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-104.9	-104.9	0.17 (1)	10.00
B- C	-1338 / 0	-104.9	-104.9	0.15 (1)	5.46
C- D	-1215 / 0	-104.9	-104.9	0.28 (1)	5.48
D- E	-1215 / 0	-104.9	-104.9	0.28 (1)	5.48
E- F	-1313 / 0	-104.9	-104.9	0.15 (1)	5.51
F- G	0 / 24	-104.9	-104.9	0.16 (1)	10.00
M- A	-134 / 0	0.0	0.0	0.01 (1)	7.81
H- G	-123 / 0	0.0	0.0	0.01 (1)	7.81
M- L	0 / 1018	-28.0	-28.0	0.38 (2)	10.00
L- K	0 / 1010	-28.0	-28.0	0.39 (2)	10.00
K- J	0 / 991	-28.0	-28.0	0.37 (2)	10.00
J- I	0 / 991	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 963	-28.0	-28.0	0.36 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	-17 / 96	0.02 (3)
L- C	0 / 289	0.07 (2)
C- K	0 / 373	0.08 (1)
K- D	-574 / 0	0.51 (1)
K- E	0 / 407	0.09 (1)
I- E	0 / 256	0.06 (2)
I- F	0 / 114	0.03 (3)
M- B	-1615 / 0	0.70 (1)
F- H	-1592 / 0	0.66 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.39/1.00 (K-L:2) , WB=0.70/1.00 (B-M:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7250-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T44A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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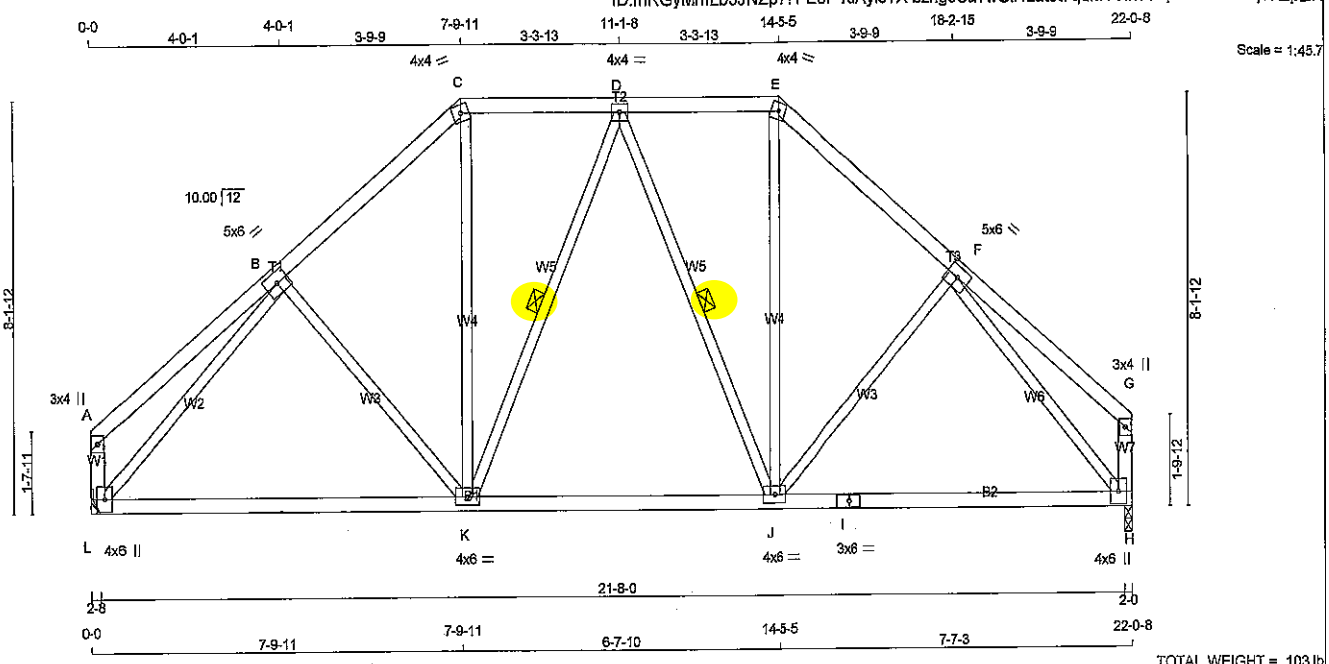

TOTAL WEIGHT = 103 lb

[M]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1465	0	1465	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-8	
H	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
H	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 30	-104.9	-104.9	0.25 (1)	10.00	B-K	-121 / 72	0.08 (1)
B-C	-1281 / 0	-104.9	-104.9	0.21 (1)	5.47	K-C	0 / 502	0.11 (1)
C-D	-965 / 0	-104.9	-104.9	0.14 (1)	6.17	K-D	-206 / 0	0.10 (1)
D-E	-953 / 0	-104.9	-104.9	0.14 (1)	6.20	D-J	-239 / 0	0.12 (1)
E-F	-1263 / 0	-104.9	-104.9	0.20 (1)	5.51	J-E	0 / 489	0.11 (1)
F-G	0 / 30	-104.9	-104.9	0.24 (1)	10.00	J-F	-72 / 88	0.05 (1)
L-A	-157 / 0	0.0	0.0	0.02 (1)	7.81	L-B	-1600 / 0	0.96 (1)
H-G	-146 / 0	0.0	0.0	0.02 (1)	7.81	F-H	-1581 / 0	0.90 (1)
L-K	0 / 1039	-28.0	-28.0	0.46 (2)	10.00			
K-J	0 / 1043	-28.0	-28.0	0.47 (2)	10.00			
J-I	0 / 994	-28.0	-28.0	0.44 (2)	10.00			
I-H	0 / 994	-28.0	-28.0	0.44 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 980 (0.27")

CSI: TC=0.25/1.00 (A-B:1) , BC=0.47/1.00 (J-K:2) , WB=0.96/1.00 (B-L:1) , SSI=0.18/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7251-R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T45	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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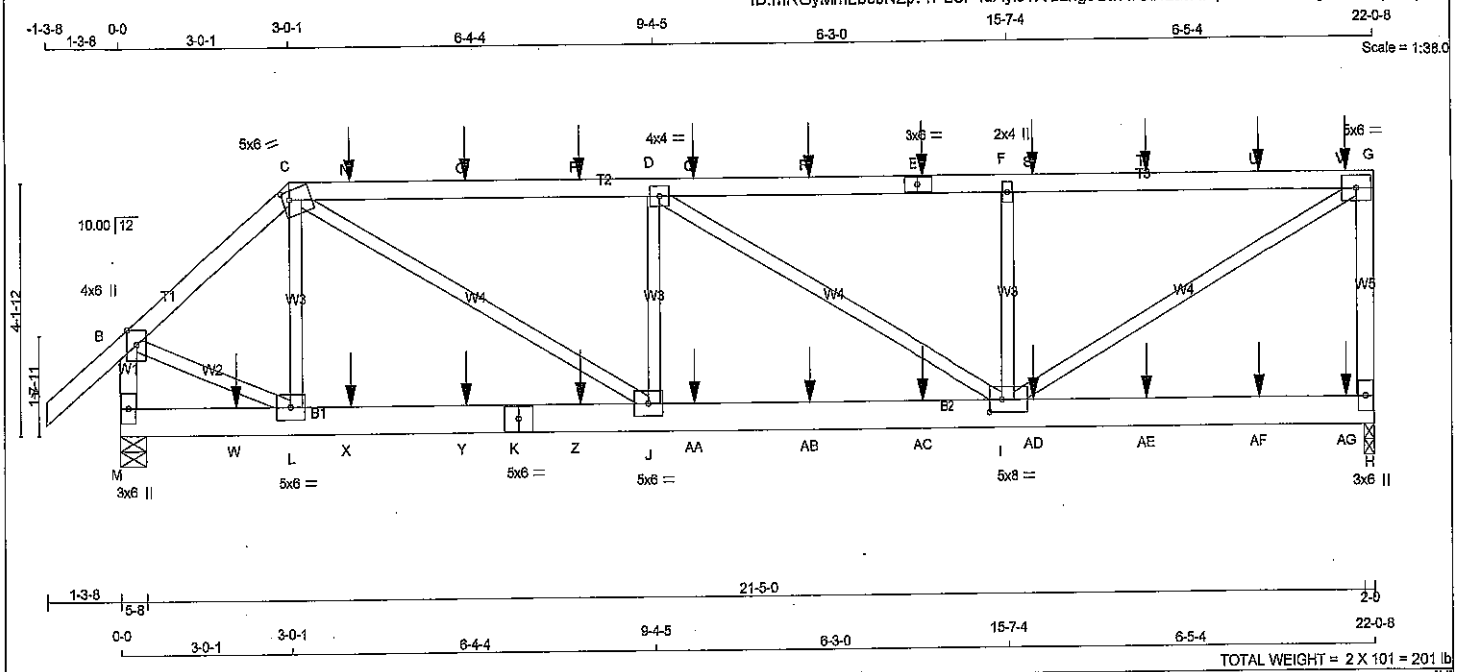

TOTAL WEIGHT = 2 X 101 = 201 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(61.0)
G- H	1	12	TOP
M- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- K	2	12	SIDE(0.0)
K- H	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.50
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2508	0	2508	0	0	2-0	2-0
M	2347	0	2347	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2018	1206 / 0	421 / 0	0 / 0	0 / 0	392 / 0	0 / 0
M	1881	1135 / 0	385 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-2159 / 0	-104.9	-104.9	0.11 (1)	5.93	
C- N	-3582 / 0	-104.9	-104.9	0.64 (1)	4.19	
N- O	-3582 / 0	-104.9	-104.9	0.64 (1)	4.19	
O- P	-3582 / 0	-104.9	-104.9	0.64 (1)	4.19	
P- D	-3582 / 0	-104.9	-104.9	0.64 (1)	4.19	
D- Q	-3096 / 0	-104.9	-104.9	0.62 (1)	4.45	
Q- R	-3096 / 0	-104.9	-104.9	0.62 (1)	4.45	
R- E	-3096 / 0	-104.9	-104.9	0.62 (1)	4.45	
E- F	-3096 / 0	-104.9	-104.9	0.62 (1)	4.45	
F- S	-3096 / 0	-104.9	-104.9	0.63 (1)	4.45	
S- T	-3096 / 0	-104.9	-104.9	0.63 (1)	4.45	
T- U	-3096 / 0	-104.9	-104.9	0.63 (1)	4.45	
U- V	-3096 / 0	-104.9	-104.9	0.63 (1)	4.45	
V- G	-3096 / 0	-104.9	-104.9	0.63 (1)	4.45	
H- G	-2341 / 0	0.0	0.0	0.30 (1)	7.34	
M- B	-2342 / 0	0.0	0.0	0.13 (1)	7.34	
M- W	0 / 0	-28.0	-28.0	0.09 (2)	10.00	
W- L	0 / 0	-28.0	-28.0	0.09 (2)	10.00	
L- X	0 / 1643	-28.0	-28.0	0.17 (1)	10.00	
X- Y	0 / 1643	-28.0	-28.0	0.17 (1)	10.00	
Y- K	0 / 1643	-28.0	-28.0	0.17 (1)	10.00	
K- Z	0 / 1643	-28.0	-28.0	0.17 (1)	10.00	
Z- J	0 / 1643	-28.0	-28.0	0.17 (1)	10.00	
J-AA	0 / 3582	-28.0	-28.0	0.30 (1)	10.00	
AA-AB	0 / 3582	-28.0	-28.0	0.30 (1)	10.00	
AB-AC	0 / 3582	-28.0	-28.0	0.30 (1)	10.00	
AC- I	0 / 3582	-28.0	-28.0	0.30 (1)	10.00	
I-AD	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
AD-AE	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
AE-AF	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
AF-AG	0 / 0	-28.0	-28.0	0.12 (2)	10.00	
AG- H	0 / 0	-28.0	-28.0	0.12 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-357 / 151	0.05 (1)
B- L	0 / 1776	0.22 (1)
C- J	0 / 2252	0.28 (1)
J- D	-865 / 47	0.11 (1)
D- I	-568 / 0	0.29 (1)
I- F	-1191 / 0	0.15 (1)
I- G	0 / 3589	0.44 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC	MAX	MAX+	FACE	DIR	TYPE
E	14-0-12	-130	-130	---	BACK	VERT	TOTAL
N	4-0-12	-130	-130	---	BACK	VERT	TOTAL
O	6-0-12	-130	-130	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.64/1.00 (C-D:1) , BC=0.30/1.00 (I-J:1) , WB=0.44/1.00 (G-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUMBER GRIP= 0.87 (I) (INPUT = 0.90)
PLATE/METAL= 0.39 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 72528

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T45	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 4-0-12, 129.6 lbs FACTORED DOWN AT 6-0-12, 129.6 lbs FACTORED DOWN AT 8-0-12, 129.6 lbs FACTORED DOWN AT 10-0-12, 129.6 lbs FACTORED DOWN AT 12-0-12, 129.6 lbs FACTORED DOWN AT 14-0-12, 129.6 lbs FACTORED DOWN AT 16-0-12, 129.6 lbs FACTORED DOWN AT 18-0-12, AND 129.6 lbs FACTORED DOWN AT 20-0-12, AND 156.8 lbs FACTORED DOWN AT 21-6-12 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 2-0-12, 71.5 lbs FACTORED DOWN AT 4-0-12, 71.5 lbs FACTORED DOWN AT 6-0-12, 71.5 lbs FACTORED DOWN AT 8-0-12, 71.5 lbs FACTORED DOWN AT 10-0-12, 71.5 lbs FACTORED DOWN AT 12-0-12, 71.5 lbs FACTORED DOWN AT 14-0-12, 71.5 lbs FACTORED DOWN AT 16-0-12, 71.5 lbs FACTORED DOWN AT 18-0-12, AND 71.5 lbs FACTORED DOWN AT 20-0-12, AND 84.3 lbs FACTORED DOWN AT 21-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	8-0-12	-130	-130	—	BACK	VERT	TOTAL
Q	10-0-12	-130	-130	—	BACK	VERT	TOTAL
R	12-0-12	-130	-130	—	BACK	VERT	TOTAL
S	16-0-12	-130	-130	—	BACK	VERT	TOTAL
T	18-0-12	-130	-130	—	BACK	VERT	TOTAL
U	20-0-12	-130	-130	—	BACK	VERT	TOTAL
V	21-6-12	-157	-157	—	BACK	VERT	TOTAL
W	2-0-12	-41	-71	—	BACK	VERT	TOTAL
X	4-0-12	-41	-71	—	BACK	VERT	TOTAL
Y	6-0-12	-41	-71	—	BACK	VERT	TOTAL
Z	8-0-12	-41	-71	—	BACK	VERT	TOTAL
AA	10-0-12	-41	-71	—	BACK	VERT	TOTAL
AB	12-0-12	-41	-71	—	BACK	VERT	TOTAL
AC	14-0-12	-41	-71	—	BACK	VERT	TOTAL
AD	16-0-12	-41	-71	—	BACK	VERT	TOTAL
AE	18-0-12	-41	-71	—	BACK	VERT	TOTAL
AF	20-0-12	-41	-71	—	BACK	VERT	TOTAL
AG	21-6-12	-48	-84	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7252-58
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286831	T46	1	1	44755 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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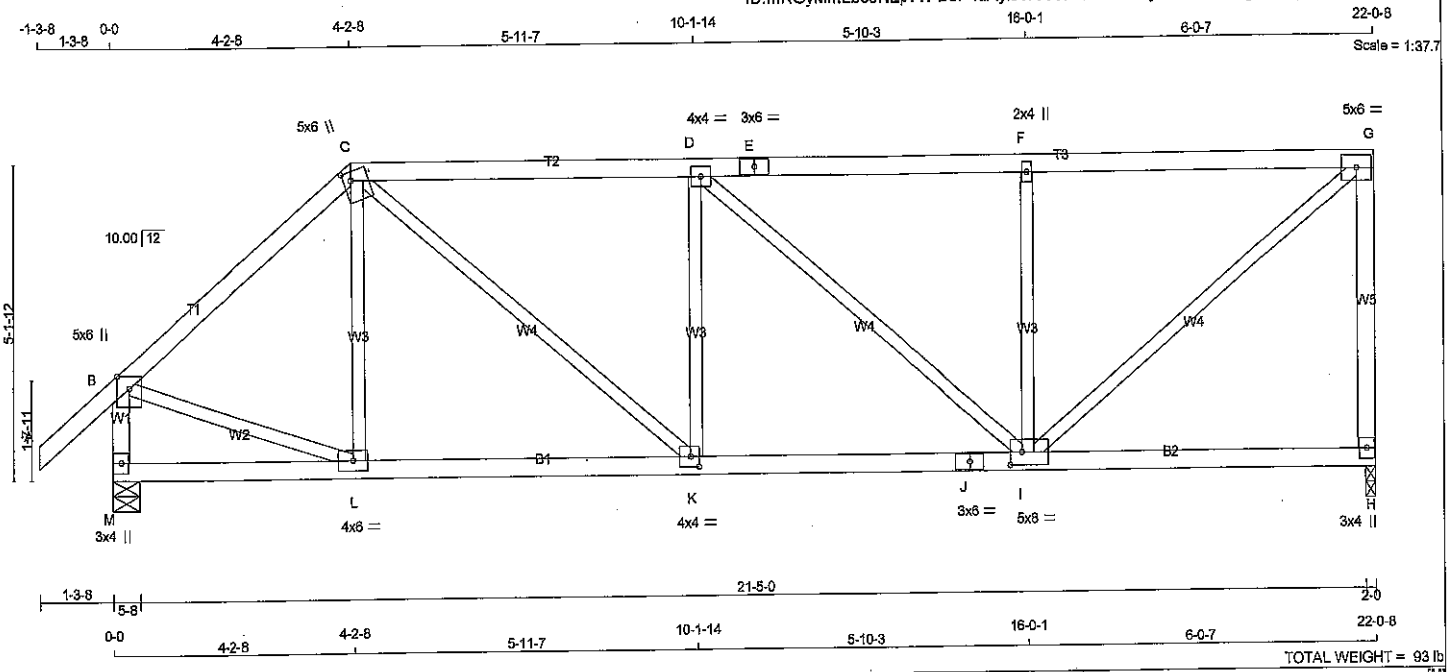

TOTAL WEIGHT = 93 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1465	0	1465	0	0	2-0	2-0
M	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
M	1266	806 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO				FR-TO		
A- B	0 / 47	-104.9 -104.9	0.14 (1)	10.00		L- C	-158 / 138	0.06 (1)
B- C	-1373 / 0	-104.9 -104.9	0.37 (1)	5.10		B- L	0 / 1099	0.25 (1)
C- D	-1739 / 0	-104.9 -104.9	0.60 (1)	4.32		C- K	0 / 889	0.20 (1)
D- E	-1416 / 0	-104.9 -104.9	0.57 (1)	4.70		K- D	-403 / 62	0.16 (1)
E- F	-1416 / 0	-104.9 -104.9	0.57 (1)	4.70		D- I	-420 / 0	0.48 (1)
F- G	-1416 / 0	-104.9 -104.9	0.58 (1)	4.70		I- F	-684 / 0	0.27 (1)
H- G	-1396 / 0	0.0 0.0	0.63 (1)	6.89		I- G	0 / 1617	0.41 (1)
M- B	-1567 / 0	0.0 0.0	0.17 (1)	6.58				
M- L	0 / 0	-28.0 -28.0	0.18 (3)	10.00				
L- K	0 / 1049	-28.0 -28.0	0.29 (2)	10.00				
K- J	0 / 1739	-28.0 -28.0	0.42 (2)	10.00				
J- I	0 / 1739	-28.0 -28.0	0.42 (2)	10.00				
I- H	0 / 0	-28.0 -28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.63/1.00 (G-H:1) , BC=0.42/1.00 (I-K:2) , WB=0.48/1.00 (D-I:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7253-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T46B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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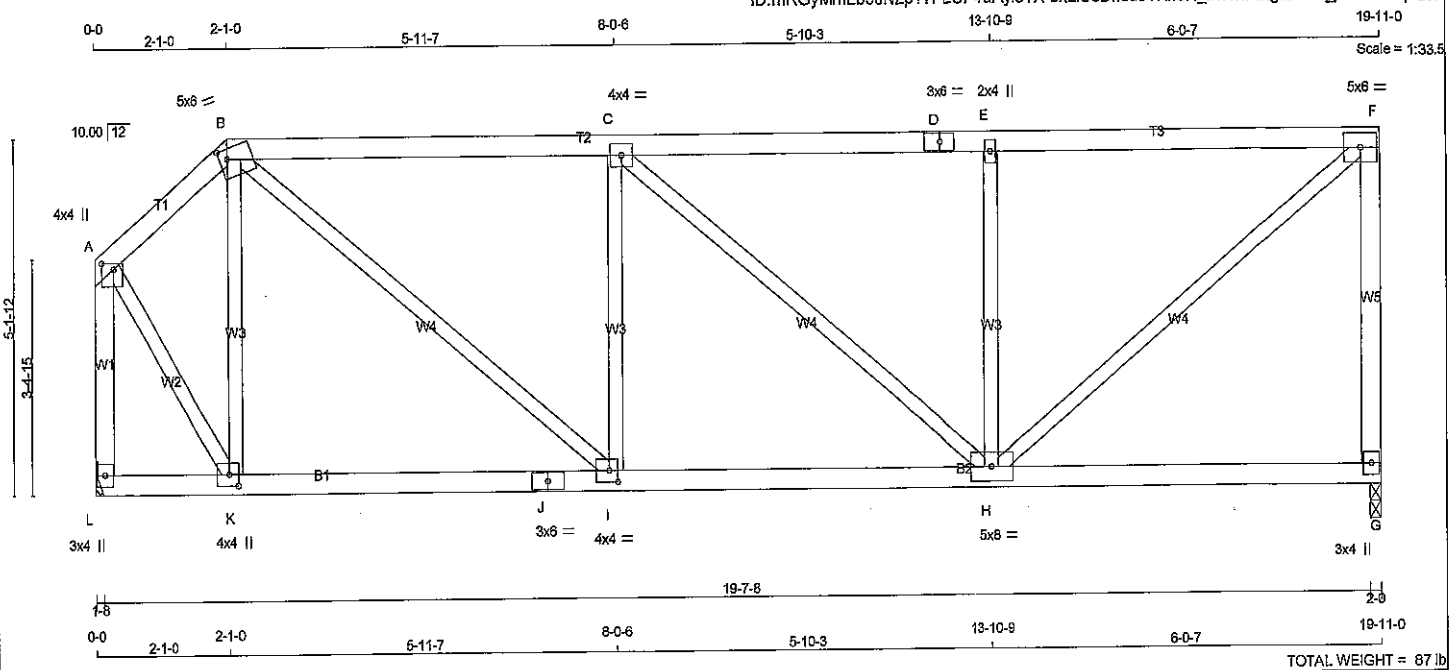

TOTAL WEIGHT = 87 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TTWW-m	MT20	5.0	6.0	1.75	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1324	0	1324	0	0	2-0	2-0
L	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-706 / 0	-104.9	-104.9	0.08 (1)	6.25	K- B	-616 / 0	0.24 (1)	
B- C	-1394 / 0	-104.9	-104.9	0.57 (1)	4.75	A- K	0 / 932	0.21 (1)	
C- D	-1240 / 0	-104.9	-104.9	0.56 (1)	4.97	B- I	0 / 1116	0.25 (1)	
D- E	-1240 / 0	-104.9	-104.9	0.56 (1)	4.97	I- C	-545 / 3	0.21 (1)	
E- F	-1240 / 0	-104.9	-104.9	0.56 (1)	4.97	C- H	-200 / 0	0.23 (1)	
G- F	-1254 / 0	0.0	0.0	0.57 (1)	7.18	H- E	-684 / 0	0.27 (1)	
L- A	-1324 / 0	0.0	0.0	0.26 (1)	7.03	H- F	0 / 1592	0.36 (1)	
L- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
K- J	0 / 526	-28.0	-28.0	0.23 (2)	10.00				
J- I	0 / 526	-28.0	-28.0	0.23 (2)	10.00				
I- H	0 / 1394	-28.0	-28.0	0.39 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.39/1.00 (H-I:2) , WB=0.36/1.00 (F-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

DWG NO. TAM 7222-R8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T47B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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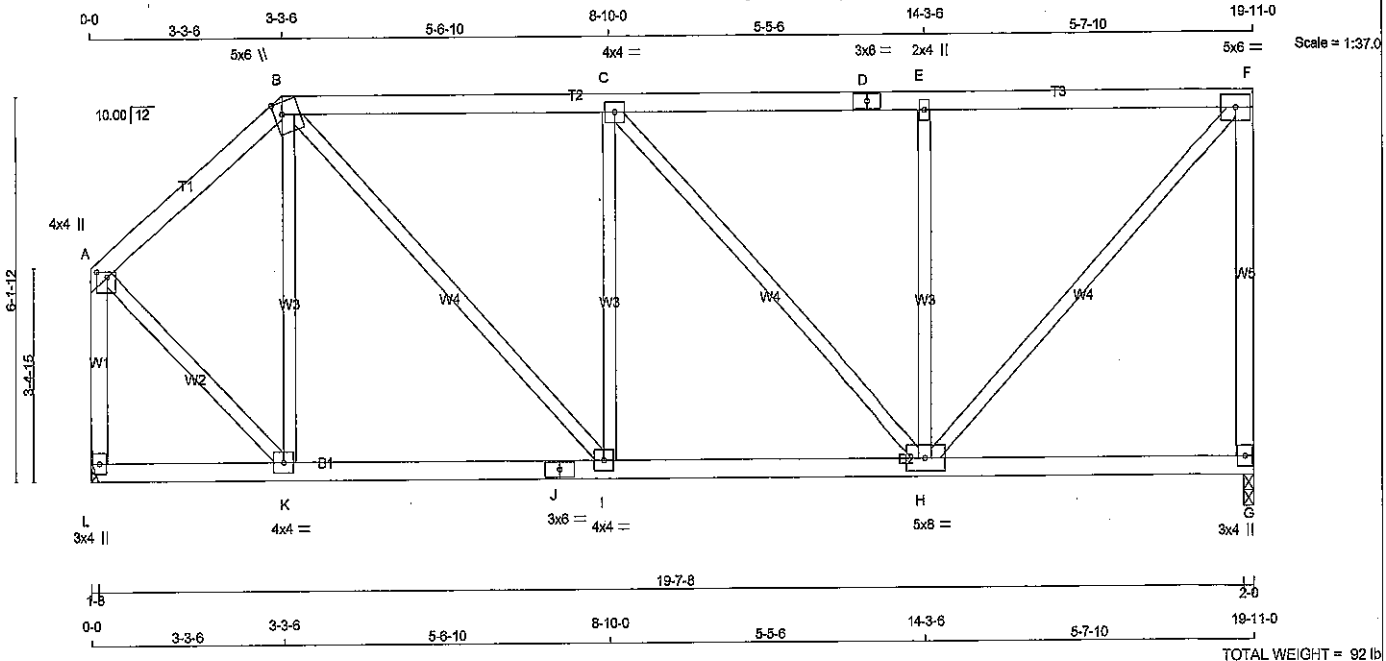

TOTAL WEIGHT = 92 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1324	0	1324	0	0	2-0	2-0
L	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-838 / 0	-104.9	-104.9	0.21 (1)	6.25	K- B	-441 / 0	0.26 (1)
B- C	-1174 / 0	-104.9	-104.9	0.47 (1)	5.22	A- K	0 / 867	0.20 (1)
C- D	-981 / 0	-104.9	-104.9	0.47 (1)	5.60	B- I	0 / 784	0.18 (1)
D- E	-981 / 0	-104.9	-104.9	0.47 (1)	5.60	I- C	-420 / 40	0.25 (1)
E- F	-981 / 0	-104.9	-104.9	0.47 (1)	5.60	C- H	-284 / 0	0.38 (1)
G- F	-1259 / 0	0.0	0.0	0.93 (1)	7.17	H- E	-838 / 0	0.38 (1)
L- A	-1294 / 0	0.0	0.0	0.26 (1)	7.10	H- F	0 / 1414	0.32 (1)
L- K	0 / 0	-28.0	-28.0	0.14 (2)	10.00			
K- J	0 / 634	-28.0	-28.0	0.22 (2)	10.00			
J- I	0 / 634	-28.0	-28.0	0.22 (2)	10.00			
I- H	0 / 1174	-28.0	-28.0	0.33 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.93/1.00 (F-G:1) , BC=0.33/1.00 (H-I:2) , WB=0.38/1.00 (E-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)

DWG NO. TAM 7223 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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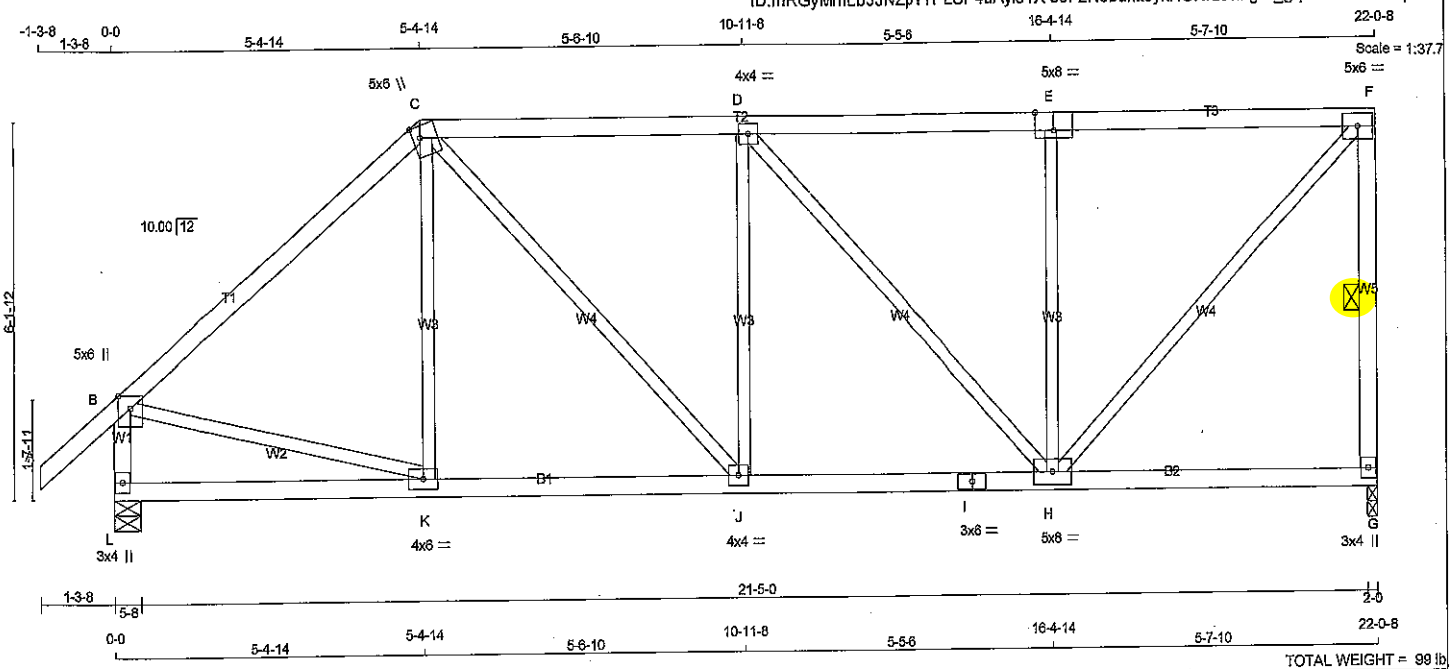

TOTAL WEIGHT = 99 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TSW-t	MT20	5.0	8.0	Edge	4.00
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
L	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1166	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1266	806 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1360 / 0	-104.9	-104.9	0.64 (1)	4.71
C- D	-1441 / 0	-104.9	-104.9	0.50 (1)	4.80
D- E	-1114 / 0	-104.9	-104.9	0.47 (1)	5.33
E- F	-1120 / 0	-104.9	-104.9	0.47 (1)	5.32
G- F	-1400 / 0	0.0	0.0	0.25 (1)	5.51
L- B	-1549 / 0	0.0	0.0	0.16 (1)	6.61
L- K	0 / 0	-28.0	-28.0	0.21 (3)	10.00
K- J	0 / 1041	-28.0	-28.0	0.33 (2)	10.00
J- I	0 / 1441	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 1441	-28.0	-28.0	0.37 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-71 / 198	0.04 (3)
B- K	0 / 1071	0.24 (1)
C- J	0 / 580	0.13 (1)
J- D	-260 / 97	0.17 (1)
D- H	-480 / 0	0.63 (1)
H- E	-637 / 0	0.38 (1)
H- F	0 / 1615	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.37/1.00 (H-J:2) , WB=0.63/1.00 (D-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

DWG NO. TAM 7254-19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T48	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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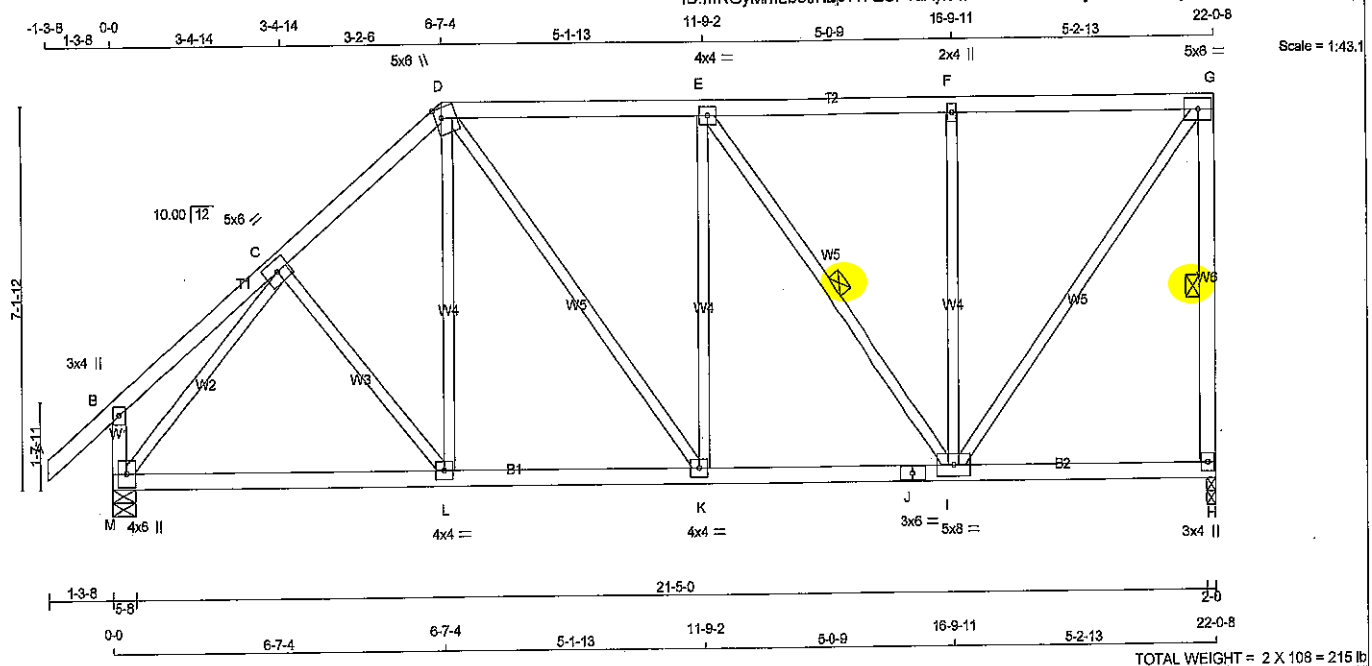

TOTAL WEIGHT = 2 X 108 = 215 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1465	0	1465	0	0	2-0	2-0
M	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
M	1266	808 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- L	-17 / 96	0.02 (3)
B- C	0 / 24	-104.9	-104.9	0.17 (1)	10.00	L- D	0 / 290	0.07 (2)
C- D	-1338 / 0	-104.9	-104.9	0.20 (1)	5.38	M- C	-1615 / 0	0.70 (1)
D- E	-1217 / 0	-104.9	-104.9	0.41 (1)	5.26	D- K	0 / 344	0.08 (1)
E- F	-903 / 0	-104.9	-104.9	0.39 (1)	5.91	K- E	-155 / 130	0.14 (1)
F- G	-903 / 0	-104.9	-104.9	0.39 (1)	5.91	E- J	-530 / 0	0.26 (1)
H- G	-1405 / 0	0.0	0.0	0.32 (1)	5.50	J- F	-591 / 0	0.52 (1)
M- B	-279 / 0	0.0	0.0	0.03 (1)	7.81	I- G	0 / 1487	0.33 (1)
M- L	0 / 1018	-28.0	-28.0	0.38 (2)	10.00			
L- K	0 / 1010	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1217	-28.0	-28.0	0.32 (2)	10.00			
J- I	0 / 1217	-28.0	-28.0	0.32 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.19 (3)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.39/1.00 (K-L:2) , WB=0.70/1.00 (C-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7255-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T48B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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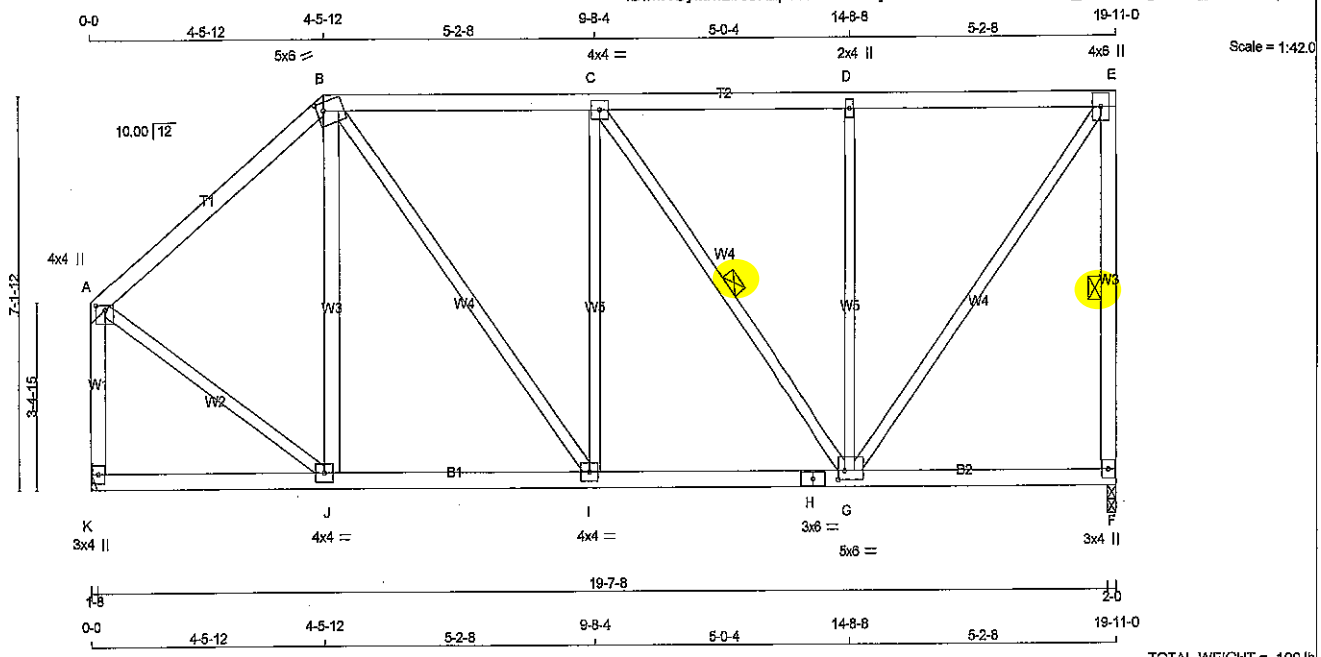

TOTAL WEIGHT = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TTWW-m	MT20	5.0	6.0	1.75	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1324	0	1324	0	0	2-0	2-0
K	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
K	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-902 / 0	-104.9	-104.9	0.39 (1)	5.93
B- C	-1007 / 0	-104.9	-104.9	0.40 (1)	5.66
C- D	-791 / 0	-104.9	-104.9	0.39 (1)	6.20
D- E	-791 / 0	-104.9	-104.9	0.38 (1)	6.22
F- E	-1264 / 0	0.0	0.0	0.29 (1)	5.73
K- A	-1274 / 0	0.0	0.0	0.25 (1)	7.14
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00
J- I	0 / 684	-28.0	-28.0	0.24 (2)	10.00
I- H	0 / 1007	-28.0	-28.0	0.29 (2)	10.00
H- G	0 / 1007	-28.0	-28.0	0.29 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-305 / 66	0.19 (1)
A- J	0 / 830	0.19 (1)
B- I	0 / 533	0.12 (1)
I- C	-291 / 78	0.26 (1)
C- G	-366 / 0	0.18 (1)
G- D	-586 / 0	0.52 (1)
G- E	0 / 1308	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.29/1.00 (G-I:2) , WB=0.52/1.00 (D-G:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7224-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T49	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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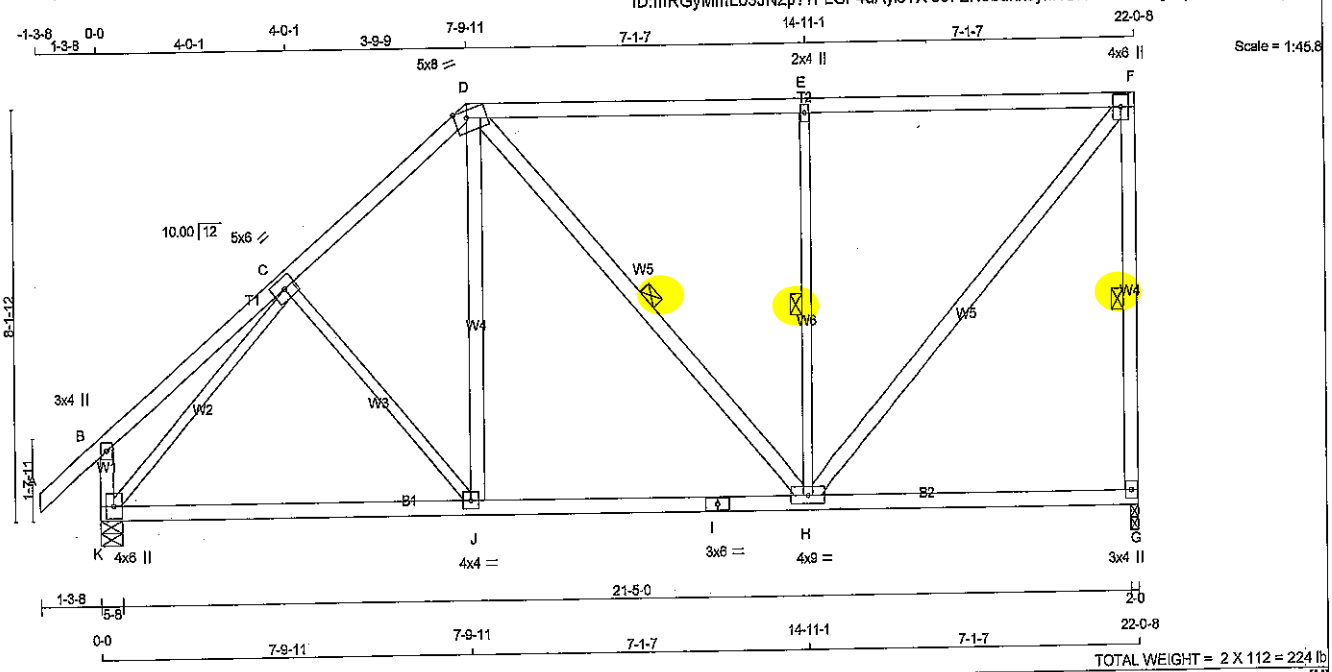

TOTAL WEIGHT = 2 X 112 = 224 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1+p	MT20	4.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
K	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1166	718 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
K	1256	806 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 30	-104.9	-104.9	0.25 (1)	10.00
C- D	-1282 / 0	-104.9	-104.9	0.29 (1)	5.35
D- E	-998 / 0	-104.9	-104.9	1.00 (1)	3.71
E- F	-998 / 0	-104.9	-104.9	1.00 (1)	3.71
G- F	-1383 / 0	0.0	0.0	0.41 (1)	5.53
K- B	-302 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1041	-28.0	-28.0	0.49 (2)	10.00
J- I	0 / 965	-28.0	-28.0	0.50 (2)	10.00
I- H	0 / 985	-28.0	-28.0	0.50 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-121 / 73	0.08 (1)
J- D	0 / 395	0.06 (2)
D- H	-36 / 48	0.02 (3)
H- E	-927 / 0	0.39 (1)
H- F	0 / 1486	0.24 (1)
K- C	-1602 / 0	0.95 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=1.00/1.00 (E-F:1) , BC=0.50/1.00 (H-J:2) , WB=0.98/1.00 (C-K:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7256-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T49B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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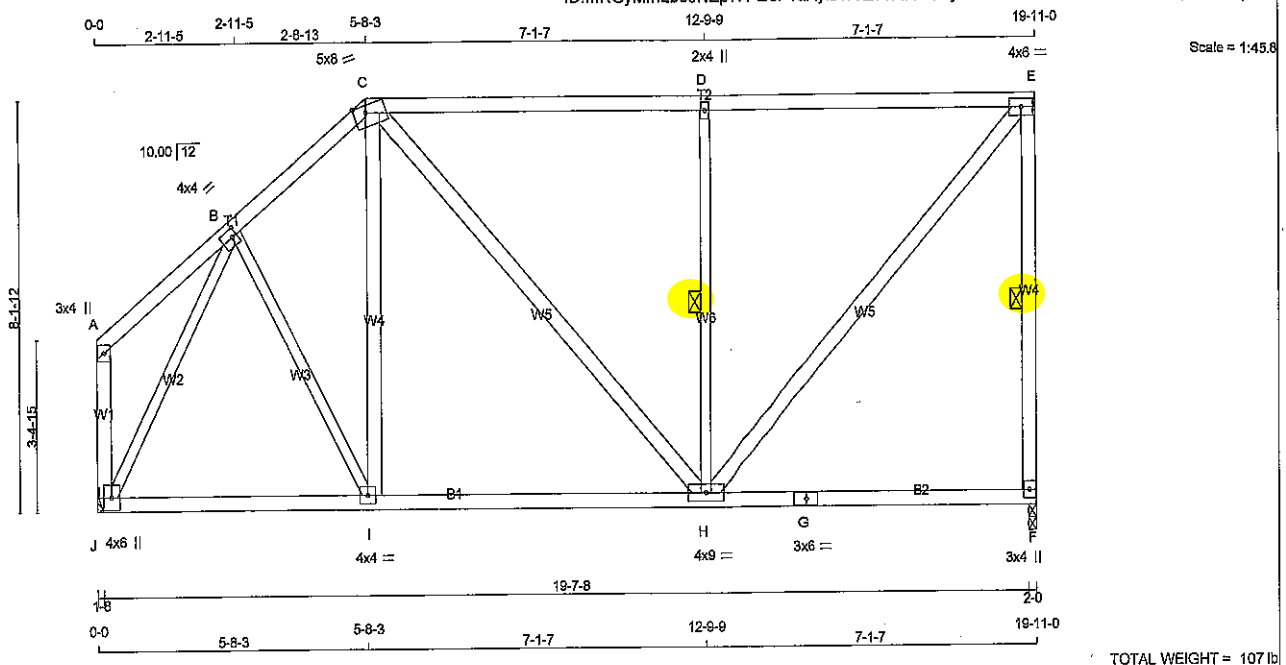

TOTAL WEIGHT = 107 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - I	2x3	DRY	No.2	SPF
H - D	2x3	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1324	0	1324	0	0	2-0	2-0
J	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
J	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-104.9	-104.9	0.13 (1)	10.00
B- C	-937 / 0	-104.9	-104.9	0.14 (1)	6.23
C- D	-871 / 0	-104.9	-104.9	0.62 (1)	6.25
D- E	-872 / 0	-104.9	-104.9	0.62 (1)	6.25
F- E	-1244 / 0	0.0	0.0	0.37 (1)	5.76
J- A	-119 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 600	-28.0	-28.0	0.31 (2)	10.00
I- H	0 / 701	-28.0	-28.0	0.42 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.35 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.35 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	0 / 214	0.05 (1)
I- C	0 / 181	0.03 (3)
C- H	0 / 253	0.04 (1)
H- D	-927 / 0	0.39 (1)
H- E	0 / 1298	0.21 (1)
J- B	-1289 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.42/1.00 (H-I:2) , WB=0.82/1.00 (B-J:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO. TAM 7275-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T50	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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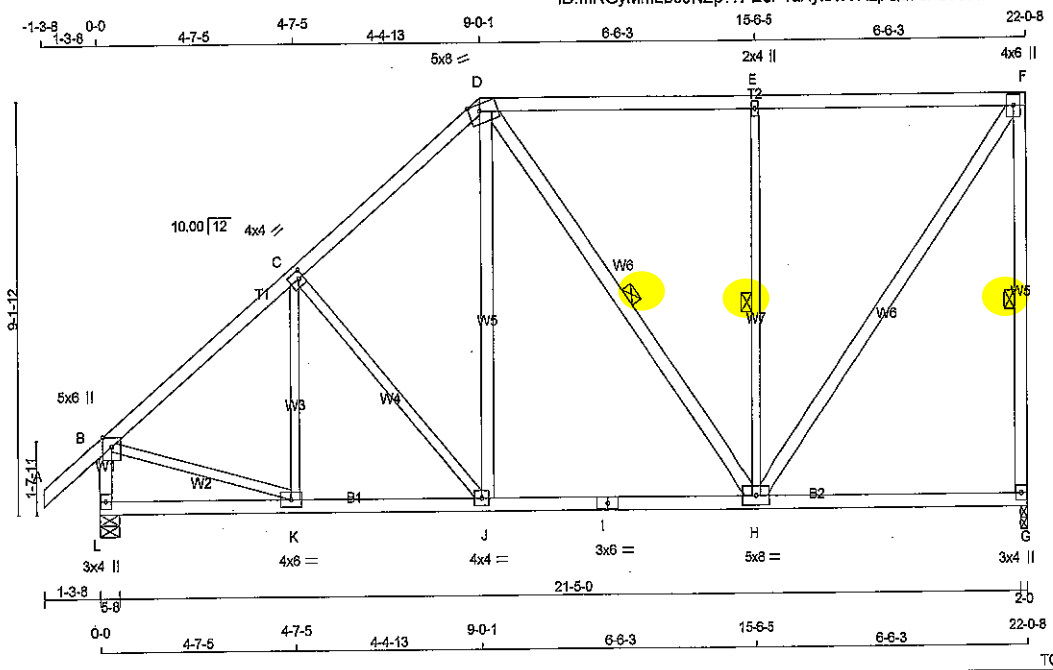

TOTAL WEIGHT = 7 X 120 = 842 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
L	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1266	806 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K- C	-177 / 110	0.06 (1)	
B- C	-1391 / 0	-104.9	-104.9	0.42 (1)	5.02	C- J	-318 / 0	0.26 (1)	
C- D	-1193 / 0	-104.9	-104.9	0.40 (1)	5.33	J- D	0 / 403	0.06 (2)	
D- E	-835 / 0	-104.9	-104.9	0.81 (1)	4.99	D- H	-96 / 0	0.06 (1)	
E- F	-835 / 0	-104.9	-104.9	0.81 (1)	4.98	H- E	-848 / 0	0.46 (1)	
G- F	-1392 / 0	0.0	0.0	0.54 (1)	5.52	H- F	0 / 1408	0.23 (1)	
L- B	-1556 / 0	0.0	0.0	0.17 (1)	6.61	B- K	0 / 1139	0.26 (1)	
L- K	0 / 0	-28.0	-28.0	0.13 (3)	10.00				
K- J	0 / 1096	-28.0	-28.0	0.27 (2)	10.00				
J- I	0 / 892	-28.0	-28.0	0.40 (2)	10.00				
I- H	0 / 892	-28.0	-28.0	0.40 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.31 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.46/1.00 (E-H:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7257 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T50B	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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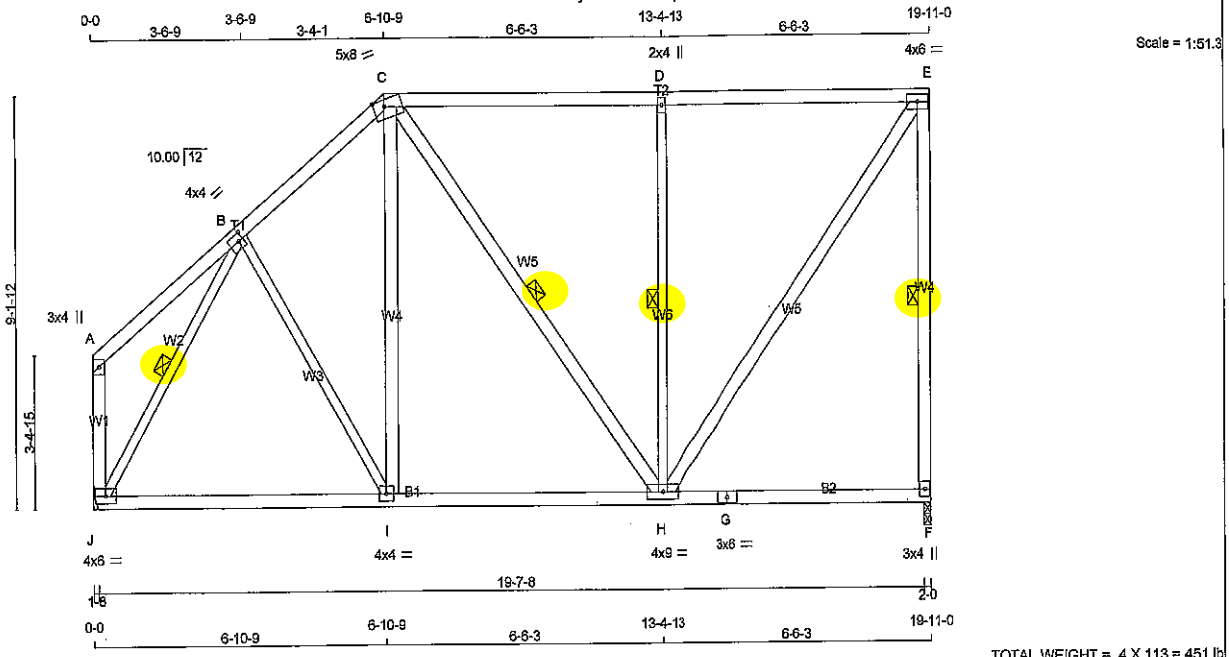

TOTAL WEIGHT = 4 X 113 = 451 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - I	2x3	DRY	No.2	SPF
H - D	2x3	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1324	0	1324	0	0	2-0	2-0
J	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
J	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-H, D-H, B-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-104.9	-104.9	0.20 (1)	10.00	B- I	0 / 145	0.03 (2)
B- C	-926 / 0	-104.9	-104.9	0.21 (1)	6.15	I- C	0 / 245	0.04 (3)
C- D	-729 / 0	-104.9	-104.9	0.80 (1)	5.27	C- H	-18 / 84	0.01 (3)
D- E	-729 / 0	-104.9	-104.9	0.80 (1)	5.27	H- D	-849 / 0	0.46 (1)
F- E	-1249 / 0	0.0	0.0	0.49 (1)	5.75	H- E	0 / 1230	0.20 (1)
J- A	-141 / 0	0.0	0.0	0.03 (1)	7.81	J- B	-1275 / 0	0.40 (1)
J- I	0 / 841	-28.0	-28.0	0.37 (2)	10.00			
I- H	0 / 691	-28.0	-28.0	0.39 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.27 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.39/1.00 (H-I:2) , WB=0.46/1.00 (D-H:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (J) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7226-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T51	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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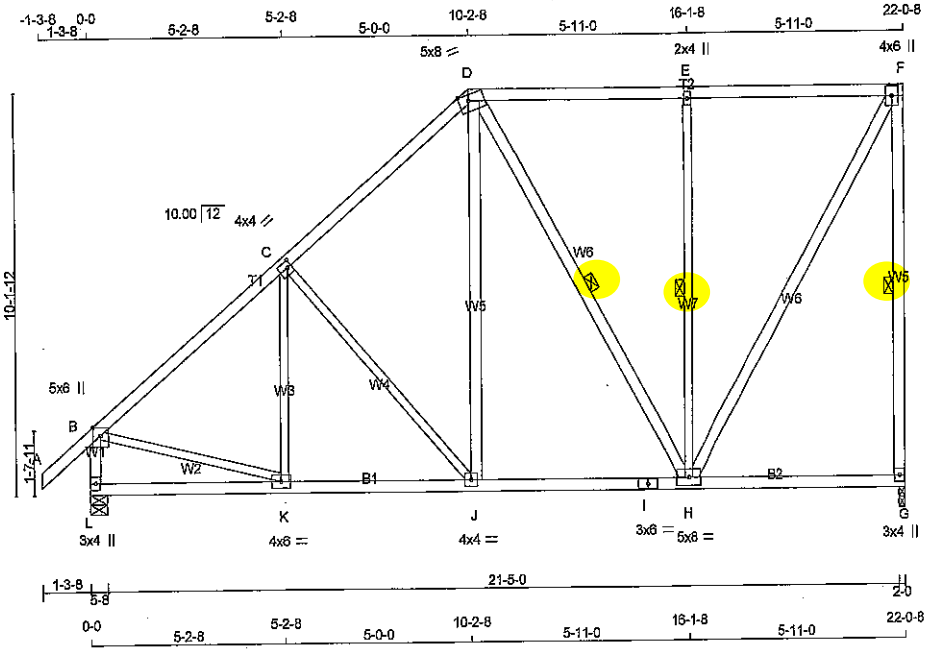


TOTAL WEIGHT = 2 X 127 = 253 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
L	1610	0	1610	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1266	806 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1395 / 0	-104.9	-104.9	0.55 (1)	4.82
C- D	-1108 / 0	-104.9	-104.9	0.52 (1)	5.29
D- E	-698 / 0	-104.9	-104.9	0.65 (1)	5.88
E- F	-699 / 0	-104.9	-104.9	0.65 (1)	5.88
G- F	-1398 / 0	0.0	0.0	0.70 (1)	5.51
L- B	-1550 / 0	0.0	0.0	0.16 (1)	6.81
L- K	0 / 0	-28.0	-28.0	0.18 (3)	10.00
K- J	0 / 1103	-28.0	-28.0	0.29 (2)	10.00
J- I	0 / 824	-28.0	-28.0	0.33 (2)	10.00
I- H	0 / 824	-28.0	-28.0	0.33 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-117 / 168	0.07 (1)
C- J	-424 / 0	0.49 (1)
J- D	0 / 459	0.07 (1)
D- H	-242 / 0	0.17 (1)
H- E	-769 / 0	0.54 (1)
H- F	0 / 1357	0.22 (1)
B- K	0 / 1137	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.70/1.00 (F-G:1) , BC=0.33/1.00 (H-J:2) , WB=0.54/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

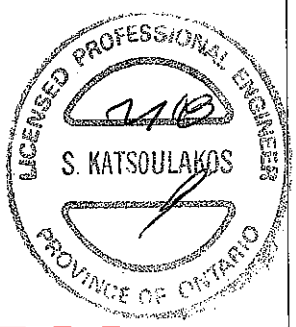
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (K) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)



DWG NO. TAM7258-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T51B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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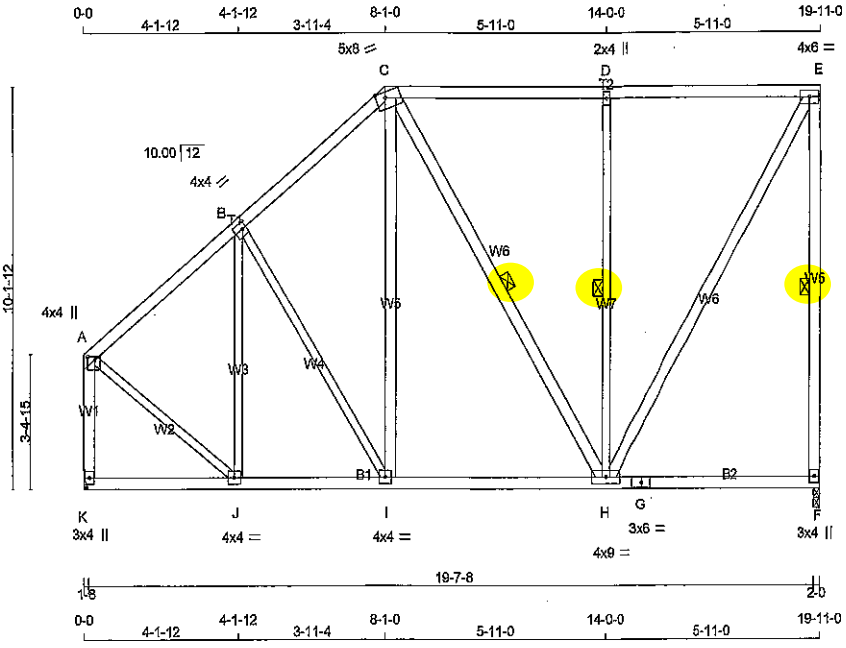

Scale = 1:58.4

TOTAL WEIGHT = 122 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1324	0	1324	0	0	2-0	2-0
K	1324	0	1324	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
K	1055	647 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-H, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-883 / 0	-104.9	-104.9	0.31 (1)	6.12
B- C	-882 / 0	-104.9	-104.9	0.31 (1)	6.12
C- D	-614 / 0	-104.9	-104.9	0.64 (1)	6.18
D- E	-614 / 0	-104.9	-104.9	0.64 (1)	6.18
F- E	-1257 / 0	0.0	0.0	0.63 (1)	5.74
K- A	-1275 / 0	0.0	0.0	0.25 (1)	7.13
K- J	0 / 0	-28.0	-28.0	0.11 (3)	10.00
J- I	0 / 703	-28.0	-28.0	0.20 (2)	10.00
I- H	0 / 653	-28.0	-28.0	0.32 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-399 / 0	0.32 (1)
B- I	-101 / 0	0.12 (1)
I- C	0 / 275	0.04 (2)
C- H	-75 / 0	0.05 (1)
H- D	-770 / 0	0.54 (1)
H- E	0 / 1193	0.19 (1)
A- J	0 / 876	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.32/1.00 (H-I:2) , WB=0.54/1.00 (D-H:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.26 (J) (INPUT = 1.00)

DWG NO. TAM 7277 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T52	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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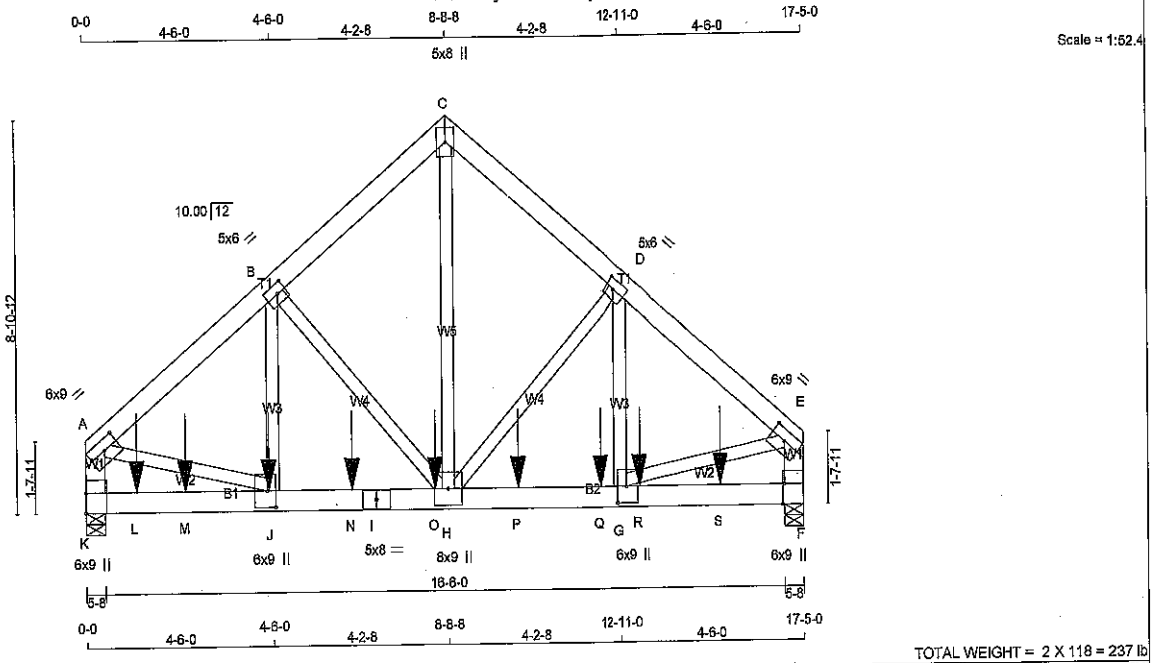


TOTAL WEIGHT = 2 X 118 = 237 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	12	SIDE(183.1)
I- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- J	1	6	SIDE(229.7)
H- C	1	6	SIDE(229.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	2.75	4.25
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TTW+p	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	6.0	9.0	2.75	4.25
F	BMV1+t	MT20	6.0	9.0	Edge	0.50
G	BMWW+t	MT20	6.0	9.0	4.50	2.50
H	BMWWW+t	MT20	8.0	9.0		
I	BS-t	MT20	5.0	8.0		
J	BMWW+t	MT20	6.0	9.0	4.50	2.50
K	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	8580	0	8580	0	0	5-8	5-8
F	7686	0	7686	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	6702	4340 / 0	1177 / 0	0 / 0	0 / 0	1185 / 0	0 / 0
F	6026	3864 / 0	1083 / 0	0 / 0	0 / 0	1079 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-8085 / 0	-104.9	-104.9	0.20 (1)	4.13
B- C	-5911 / 0	-104.9	-104.9	0.14 (1)	4.76
C- D	-5911 / 0	-104.9	-104.9	0.14 (1)	4.76
D- E	-8111 / 0	-104.9	-104.9	0.20 (1)	4.12
K- A	-6949 / 0	0.0	0.0	0.25 (1)	5.66
F- E	-6971 / 0	0.0	0.0	0.25 (1)	5.66
K- L	0 / 0	-28.0	-28.0	0.73 (1)	10.00
L- M	0 / 0	-28.0	-28.0	0.73 (1)	10.00
M- J	0 / 0	-28.0	-28.0	0.73 (1)	10.00
J- N	0 / 6210	-28.0	-28.0	0.94 (1)	10.00
N- I	0 / 6210	-28.0	-28.0	0.94 (1)	10.00
I- O	0 / 6210	-28.0	-28.0	0.94 (1)	10.00
O- H	0 / 6210	-28.0	-28.0	0.94 (1)	10.00
H- P	0 / 6230	-28.0	-28.0	0.89 (1)	10.00
P- Q	0 / 6230	-28.0	-28.0	0.89 (1)	10.00
Q- G	0 / 6230	-28.0	-28.0	0.89 (1)	10.00
G- R	0 / 0	-28.0	-28.0	0.51 (1)	10.00
R- S	0 / 0	-28.0	-28.0	0.51 (1)	10.00
S- F	0 / 0	-28.0	-28.0	0.51 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 7124	0.63 (1)
H- D	-2580 / 0	0.65 (1)
G- D	0 / 2905	0.26 (1)
B- H	-2550 / 0	0.64 (1)
J- B	0 / 2865	0.25 (1)
A- J	0 / 6398	0.57 (1)
G- E	0 / 6418	0.57 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
J	4-4-12	-1582	-1582	—	BACK	VERT	TOTAL
L	1-2-12	-1582	-1582	—	BACK	VERT	TOTAL
M	2-4-12	-1582	-1582	—	BACK	VERT	TOTAL
N	6-4-12	-1582	-1582	—	BACK	VERT	TOTAL
O	8-4-12	-1582	-1582	—	BACK	VERT	TOTAL
P	10-4-12	-1582	-1582	—	BACK	VERT	TOTAL
Q	12-4-12	-1582	-1582	—	BACK	VERT	TOTAL
R	13-4-12	-1437	-1437	—	BACK	VERT	TOTAL
S	15-4-12	-1437	-1437	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.94/1.00 (H-J:1) , WB=0.65/1.00 (D-H:1) , SSI=0.76/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

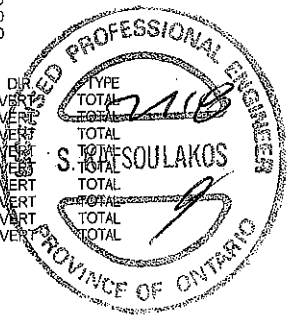
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.82 (I) (INPUT = 1.00)



P6/6

DWG NO. TAM 7259-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T52	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1582.3 lbs FACTORED DOWN AT 1-2-12, 1582.3 lbs FACTORED DOWN AT 2-4-12, 1582.3 lbs FACTORED DOWN AT 4-4-12, 1582.3 lbs FACTORED DOWN AT 6-4-12, 1582.3 lbs FACTORED DOWN AT 8-4-12, 1582.3 lbs FACTORED DOWN AT 10-4-12, 1582.3 lbs FACTORED DOWN AT 12-4-12, AND 1437.0 lbs FACTORED DOWN AT 13-4-12, AND 1437.0 lbs FACTORED DOWN AT 15-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7259-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T53	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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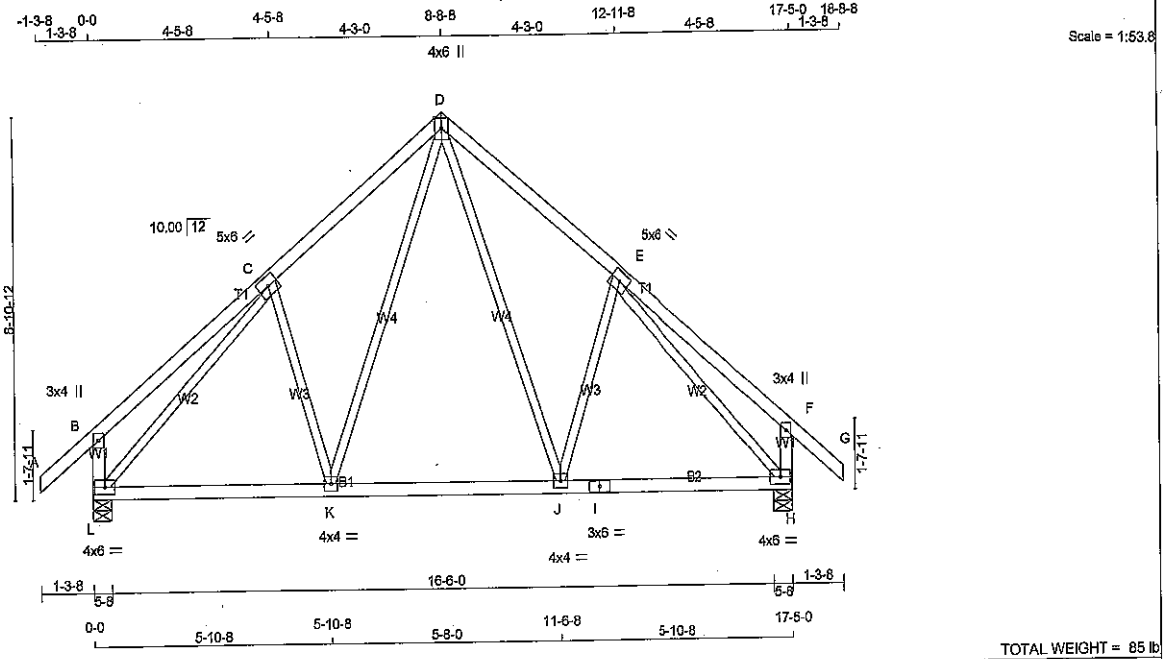

TOTAL WEIGHT = 85 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1303	0	1303	0	0	5-8	5-8
H	1303	0	1303	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1021	656 / 0	183 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	1021	656 / 0	183 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 35	-104.9	-104.9	0.32 (1)	10.00
C- D	-981 / 0	-104.9	-104.9	0.26 (1)	5.98
D- E	-981 / 0	-104.9	-104.9	0.26 (1)	5.98
E- F	0 / 35	-104.9	-104.9	0.32 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- B	-317 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-317 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 808	-28.0	-28.0	0.30 (2)	10.00
K- J	0 / 586	-28.0	-28.0	0.28 (2)	10.00
J- I	0 / 808	-28.0	-28.0	0.30 (2)	10.00
I- H	0 / 808	-28.0	-28.0	0.30 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- J	0 / 451	0.10 (1)
J- E	-265 / 49	0.12 (1)
K- D	0 / 451	0.10 (1)
C- K	-265 / 49	0.12 (1)
L- C	-1223 / 0	0.93 (1)
E- H	-1223 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.30/1.00 (K-L:2) , WB=0.93/1.00 (C-L:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.33 (E) (INPUT = 1.00)

DWG NO. TAM 7260-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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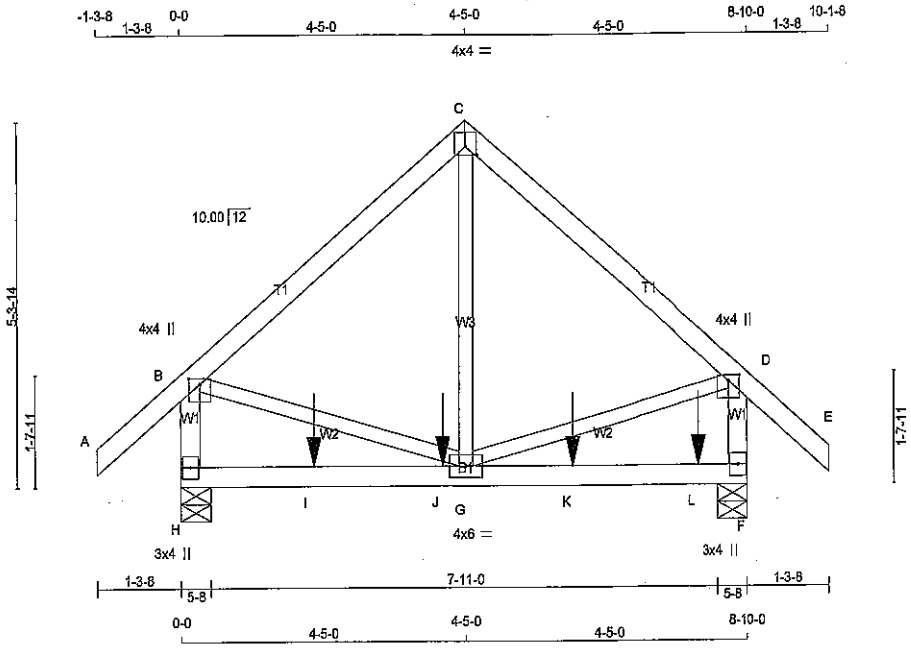

TOTAL WEIGHT = 41 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 163.1 lbs FACTORED DOWN AT 2-0-12, 163.1 lbs FACTORED DOWN AT 4-0-12, AND 19.8 lbs FACTORED DOWN AT 6-0-12, AND 25.4 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	950	0	950	0	0	5-8	5-8
F	867	0	867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	726	499 / 0	108 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	674	442 / 0	115 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00		G- C	0 / 264	0.07 (2)
B- C	-516 / 0	-104.9	-104.9	0.40 (1)	6.25		B- G	0 / 413	0.10 (1)
C- D	-516 / 0	-104.9	-104.9	0.40 (1)	6.25		G- D	0 / 413	0.10 (1)
D- E	0 / 47	-104.9	-104.9	0.16 (1)	10.00				
H- B	-824 / 0	0.0	0.0	0.10 (1)	7.81				
F- D	-824 / 0	0.0	0.0	0.10 (1)	7.81				
H- I	0 / 0	-28.0	-28.0	0.30 (1)	10.00				
I- J	0 / 0	-28.0	-28.0	0.30 (1)	10.00				
J- G	0 / 0	-28.0	-28.0	0.30 (1)	10.00				
G- K	0 / 0	-28.0	-28.0	0.25 (2)	10.00				
K- L	0 / 0	-28.0	-28.0	0.25 (2)	10.00				
L- F	0 / 0	-28.0	-28.0	0.25 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
I	2-0-12	-163	-163	—	BACK	VERT	TOTAL
J	4-0-12	-163	-163	—	BACK	VERT	TOTAL
K	6-0-12	-11	-20	—	BACK	VERT	TOTAL
L	8-0-12	-15	-25	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.30/1.00 (G-H:1) , WB=0.10/1.00 (D-G:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7228-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T55	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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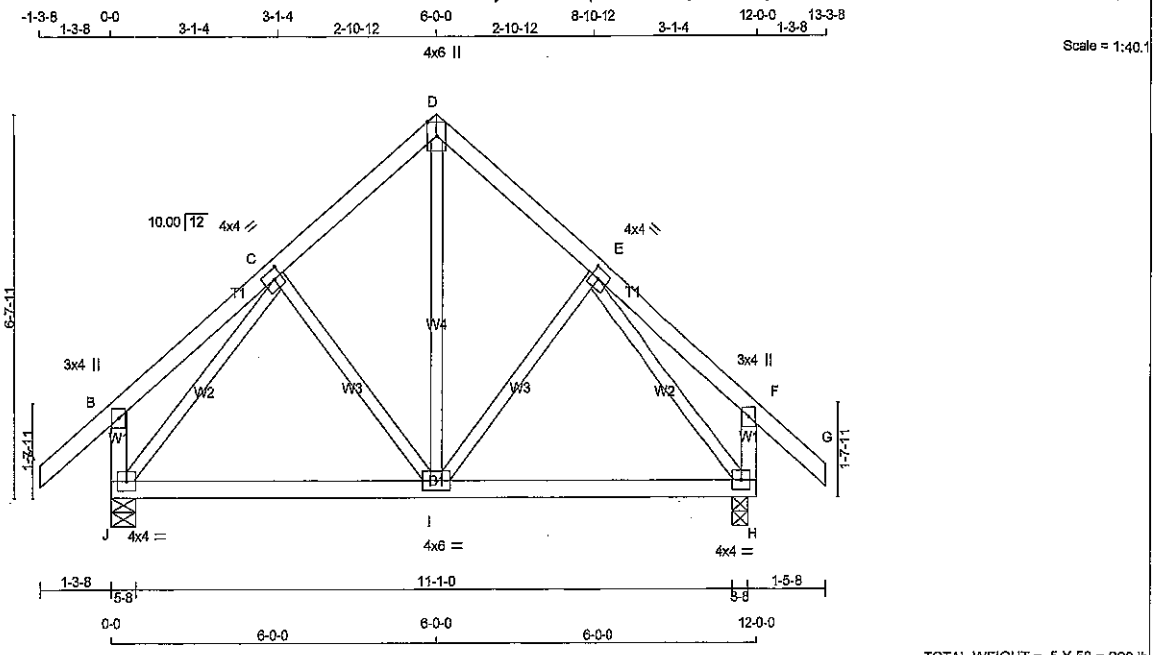

TOTAL WEIGHT = 5 X 58 = 290 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	943	0	943	0	0	5-8	5-8
H	943	0	943	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	I- D	0 / 415	0.09 (1)	
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00	I- E	-133 / 39	0.05 (1)	
C- D	-540 / 0	-104.9	-104.9	0.11 (1)	6.25	C- I	-133 / 39	0.05 (1)	
D- E	-540 / 0	-104.9	-104.9	0.11 (1)	6.25	J- C	-779 / 0	0.29 (1)	
E- F	0 / 23	-104.9	-104.9	0.15 (1)	10.00	E- H	-779 / 0	0.29 (1)	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00				
J- B	-266 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-266 / 0	0.0	0.0	0.03 (1)	7.81				
J- I	0 / 481	-28.0	-28.0	0.34 (2)	10.00				
I- H	0 / 481	-28.0	-28.0	0.34 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.34/1.00 (H-I:2) , WB=0.29/1.00 (E-H:1) , SSI=0.14/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7261-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	T56	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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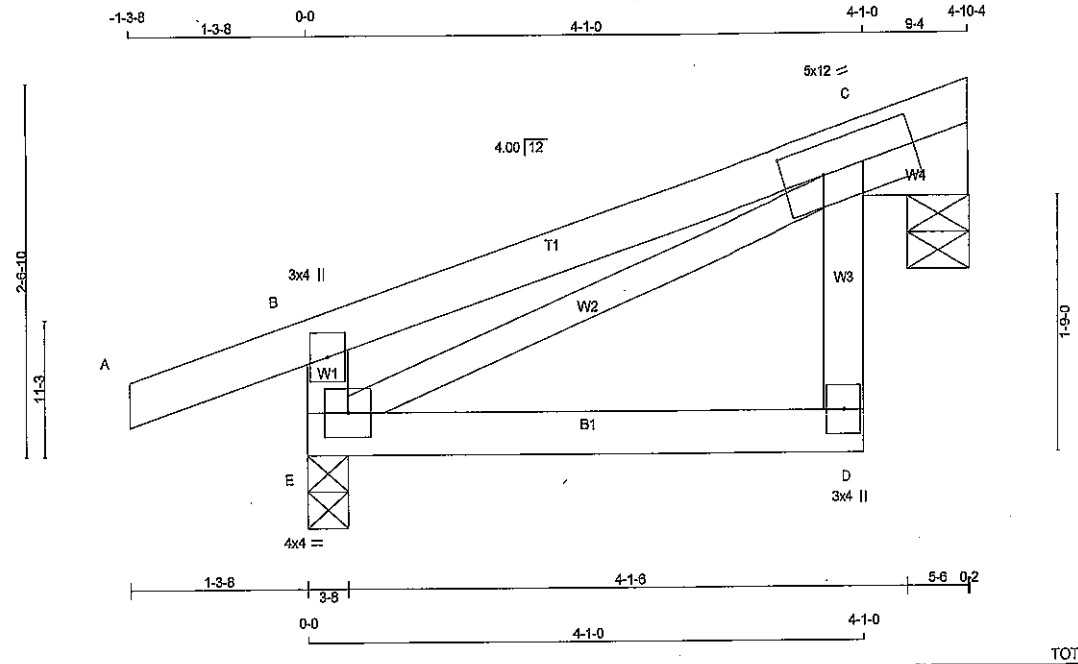

TOTAL WEIGHT = 6 X 19 = 115 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	12.0	2.50	3.75
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	271	0	271	0	0	5-8 (5-6)	5-8
E	412	0	412	0	0	3-8	3-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	216	133 / 0	43 / 0	0 / 0	0 / 0	41 / 0	0 / 0
E	311	220 / 0	43 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-104.9	-104.9	0.13 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-104.9	-104.9	0.30 (1)	10.00			
D- C	0 / 100	0.0	0.0	0.02 (3)	10.00			
E- B	-355 / 0	0.0	0.0	0.04 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	9.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	58.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.14/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7262-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T60	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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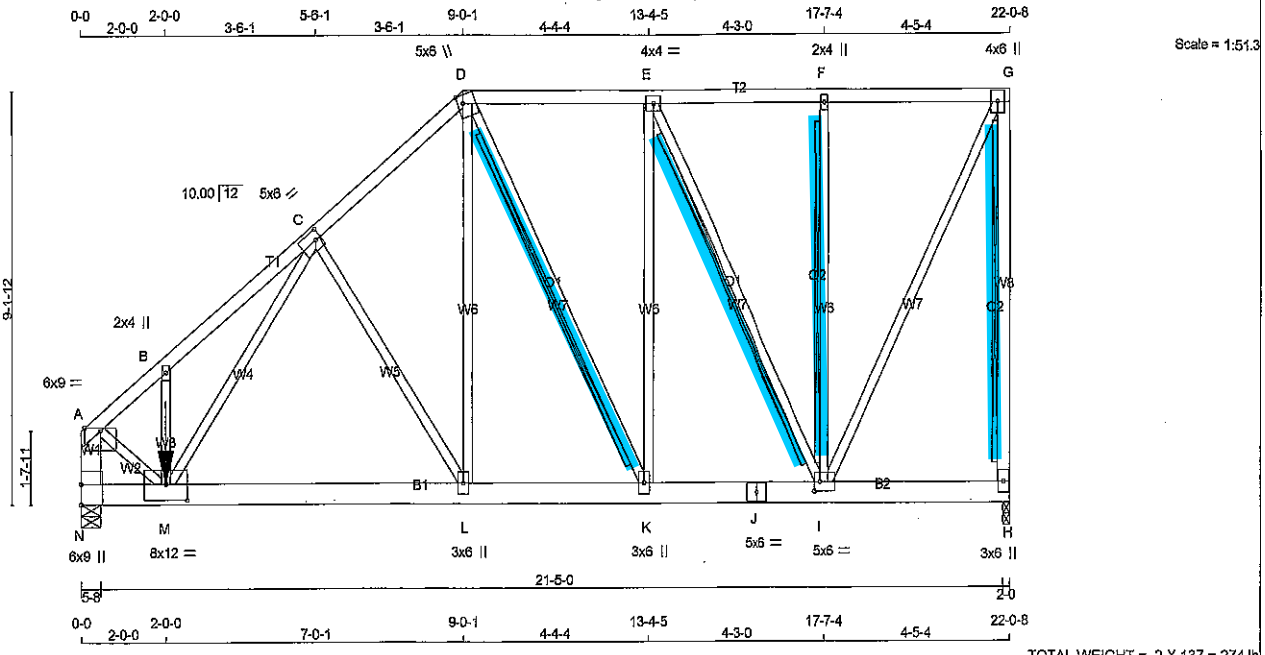

TOTAL WEIGHT = 2 X 137 = 274 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- H	1	12	TOP
N- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	2	SIDE(1404.0)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- M	1	2	SIDE(536.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	6.0	2.50	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	3.0	6.0		
L	BMWW+t	MT20	3.0	6.0		
M	BMWWW-t	MT20	8.0	12.0	4.25	6.00
N	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	2140	0	2140	0	0	2-0	2-0
N	8230	0	8230	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1697	1055 / 0	327 / 0	0 / 0	0 / 0	316 / 0	0 / 0
N	6476	4111 / 0	1190 / 0	0 / 0	0 / 0	1175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-H, D-K, E-I, F-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-7058 / 0	-104.9	-104.9	0.25 (1)		3.50
B- C	-6955 / 0	-104.9	-104.9	0.36 (1)		3.46
C- D	-2523 / 0	-104.9	-104.9	0.18 (1)		5.51
D- E	-1569 / 0	-104.9	-104.9	0.16 (1)		6.25
E- F	-956 / 0	-104.9	-104.9	0.15 (1)		6.25
F- G	-956 / 0	-104.9	-104.9	0.14 (1)		6.25
H- G	-2082 / 0	0.0	0.0	0.70 (1)		7.81
N- A	-7948 / 0	0.0	0.0	0.29 (1)		5.33
N- M	0 / 0	-28.0	-28.0	0.19 (1)		10.00
M- L	0 / 2945	-28.0	-28.0	0.38 (1)		10.00
L- K	0 / 1928	-28.0	-28.0	0.22 (1)		10.00
K- J	0 / 1570	-28.0	-28.0	0.12 (1)		10.00
J- I	0 / 1570	-28.0	-28.0	0.12 (1)		10.00
I- H	0 / 0	-28.0	-28.0	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
M- B	-134 / 47	0.01 (1)
M- C	0 / 4727	0.59 (1)
C- L	-1973 / 0	0.85 (1)
L- D	0 / 2001	0.25 (1)
A- M	0 / 6317	0.78 (1)
D- K	-804 / 0	0.42 (1)
K- E	0 / 775	0.10 (1)
E- I	-1407 / 0	0.73 (1)
I- F	-494 / 0	0.20 (1)
I- G	0 / 2119	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
M	2-0-0	-7440	-7440	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.70/1.00 (G-H:1) , BC=0.36/1.00 (L-M:1) , WB=0.85/1.00 (C-L:1) , SSI=0.12/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.58 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7279-B
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T60	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 7440.0 lbs FACTORED DOWN AT 2-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7279-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T61	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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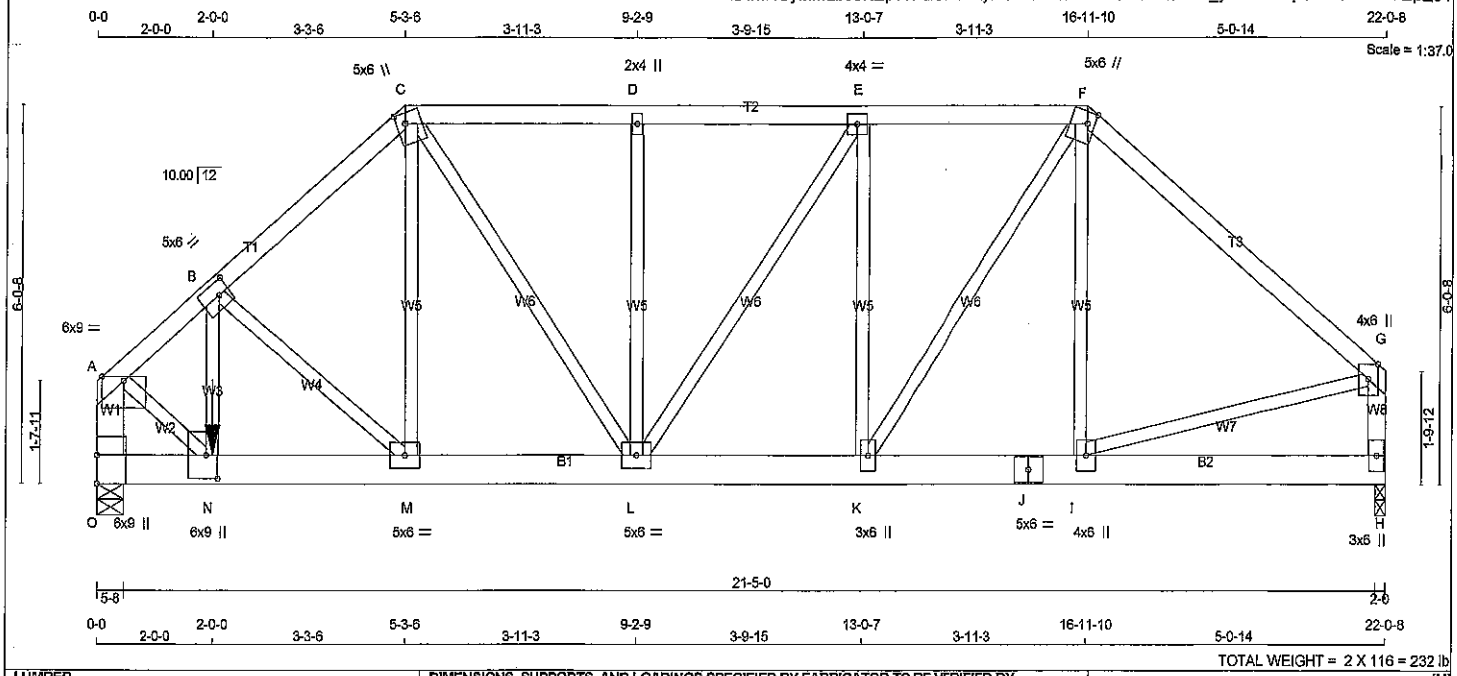

TOTAL WEIGHT = 2 X 116 = 232 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- G	1	12	TOP
H- G	1	12	TOP
O- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- J	2	2	SIDE(1404.0)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- N	1	3	SIDE(402.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TMWW-t	MT20	5.0	6.0	2.50	2.25
C	TTWW+m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	6.0	9.0	4.50	2.25
O	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	7762	0	7762	0	0	5-8	5-8
H	2093	0	2093	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	6120	3864 / 0	1139 / 0	0 / 0	0 / 0	1117 / 0	0 / 0
H	1662	1030 / 0	322 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-6607 / 0	-104.9	-104.9	0.17 (1)	3.70	N- B	0 / 3375		0.42 (1)
B- C	-3783 / 0	-104.9	-104.9	0.16 (1)	4.73	B- M	-2876 / 0		0.48 (1)
C- D	-2830 / 0	-104.9	-104.9	0.14 (1)	5.32	M- C	0 / 2229		0.28 (1)
D- E	-2830 / 0	-104.9	-104.9	0.14 (1)	5.31	C- L	-183 / 0		0.08 (1)
E- F	-2402 / 0	-104.9	-104.9	0.13 (1)	5.67	L- D	-426 / 0		0.12 (1)
F- G	-2081 / 0	-104.9	-104.9	0.30 (1)	5.77	L- E	0 / 764		0.09 (1)
O- A	-7432 / 0	0.0	0.0	0.27 (1)	5.49	K- E	-1066 / 0		0.29 (1)
H- G	-2033 / 0	0.0	0.0	0.12 (1)	7.74	K- F	0 / 1421		0.18 (1)
						I- F	-278 / 80		0.08 (1)
O- N	0 / 0	-28.0	-28.0	0.22 (1)	10.00	A- N	0 / 5954		0.74 (1)
N- M	0 / 5081	-28.0	-28.0	0.55 (1)	10.00	I- G	0 / 1653		0.20 (1)
M- L	0 / 2935	-28.0	-28.0	0.24 (1)	10.00				
L- K	0 / 2402	-28.0	-28.0	0.18 (1)	10.00				
K- J	0 / 1590	-28.0	-28.0	0.13 (1)	10.00				
J- I	0 / 1590	-28.0	-28.0	0.13 (1)	10.00				
I- H	0 / 0	-28.0	-28.0	0.04 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
N	2-0-0	-6925	-6925	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30/1.00 (F-G:1) , BC=0.55/1.00 (M-N:1) , WB=0.74/1.00 (A-N:1) , SSI=0.11/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7280-18 M/L
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T61	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6924.9 lbs FACTORED DOWN AT 2-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7280-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T62	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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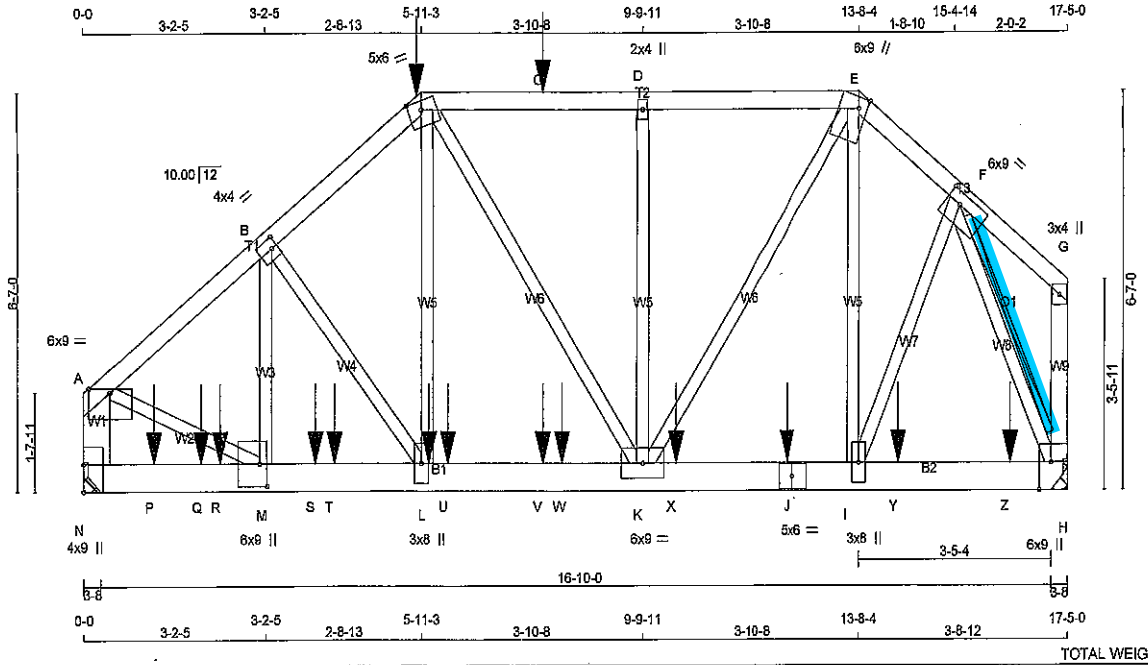

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	TOP
H- G	1	12	TOP
N- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	1.75	2.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMWW-t	MT20	6.0	9.0	2.25	3.00
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+t	MT20	6.0	9.0	Edge	2.50
I	BMWW+t	MT20	3.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW+t	MT20	3.0	8.0		
M	BMWW+t	MT20	6.0	9.0	4.50	1.50
N	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	7467	0	7467	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
H	6953	0	6953	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	5872	3734 / 0	1075 / 0	0 / 0	0 / 0	1063 / 0	0 / 0
H	5482	3462 / 0	1019 / 0	0 / 0	0 / 0	1000 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-6745 / 0	-104.9	-104.9	0.26 (1)		3.60
B- C	-6078 / 0	-104.9	-104.9	0.19 (1)		3.83
C- O	-5033 / 0	-104.9	-104.9	0.22 (1)		4.13
O- D	-5033 / 0	-104.9	-104.9	0.22 (1)		4.13
D- E	-5033 / 0	-104.9	-104.9	0.22 (1)		4.13
E- F	-4585 / 0	-104.9	-104.9	0.12 (1)		4.40
F- G	-14 / 0	-104.9	-104.9	0.05 (1)		6.25
N- A	-6462 / 0	0.0	0.0	0.24 (1)		5.84
H- G	-118 / 0	0.0	0.0	0.01 (1)		7.81
N- P	0 / 0	-28.0	-28.0	0.45 (1)		10.00
P- Q	0 / 0	-28.0	-28.0	0.45 (1)		10.00
Q- R	0 / 0	-28.0	-28.0	0.45 (1)		10.00
R- M	0 / 0	-28.0	-28.0	0.45 (1)		10.00
M- S	0 / 5187	-28.0	-28.0	0.59 (1)		10.00
S- T	0 / 5187	-28.0	-28.0	0.59 (1)		10.00
T- L	0 / 5187	-28.0	-28.0	0.59 (1)		10.00
L- U	0 / 4695	-28.0	-28.0	0.58 (1)		10.00
U- V	0 / 4695	-28.0	-28.0	0.58 (1)		10.00
V- W	0 / 4695	-28.0	-28.0	0.58 (1)		10.00
W- K	0 / 4695	-28.0	-28.0	0.58 (1)		10.00
K- X	0 / 3516	-28.0	-28.0	0.61 (1)		10.00
X- J	0 / 3516	-28.0	-28.0	0.61 (1)		10.00
J- I	0 / 3516	-28.0	-28.0	0.61 (1)		10.00
I- Y	0 / 2344	-28.0	-28.0	0.53 (1)		10.00
Y- Z	0 / 2344	-28.0	-28.0	0.53 (1)		10.00
Z- H	0 / 2344	-28.0	-28.0	0.53 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 2978	0.37 (1)
K- C	0 / 637	0.08 (1)
K- D	-458 / 0	0.16 (1)
K- E	0 / 2956	0.35 (1)
I- E	0 / 238	0.03 (1)
I- F	0 / 2945	0.36 (1)
F- H	-6279 / 0	0.80 (1)
A- M	0 / 5553	0.69 (1)
M- B	0 / 821	0.10 (1)
B- L	-918 / 0	0.18 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.61/1.00 (I-K:1) , WB=0.80/1.00 (F-H:1) , SSI=0.61/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7281 -1R
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T62	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 49.2 lbs FACTORED DOWN AT 5-11-3, AND 45.9 lbs FACTORED DOWN AT 8-0-12 ON TOP CHORD, AND 1295.8 lbs FACTORED DOWN AT 1-2-12, 163.1 lbs FACTORED DOWN AT 2-0-12, 1295.8 lbs FACTORED DOWN AT 2-4-12, 163.1 lbs FACTORED DOWN AT 4-0-12, 1295.8 lbs FACTORED DOWN AT 4-4-12, 19.8 lbs FACTORED DOWN AT 6-0-12, 1295.8 lbs FACTORED DOWN AT 6-4-12, 19.8 lbs FACTORED DOWN AT 8-0-12, 1295.8 lbs FACTORED DOWN AT 8-4-12, 1295.8 lbs FACTORED DOWN AT 10-4-12, 1295.8 lbs FACTORED DOWN AT 12-4-12, AND 1295.8 lbs FACTORED DOWN AT 14-4-12, AND 1295.8 lbs FACTORED DOWN AT 16-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-11-3	-49	-49	—	FRONT	VERT	TOTAL
J	12-4-12	-1296	-1296	—	BACK	VERT	TOTAL
L	6-0-12	-11	-20	—	FRONT	VERT	TOTAL
O	8-0-12	-46	-46	—	FRONT	VERT	TOTAL
P	1-2-12	-1296	-1296	—	BACK	VERT	TOTAL
Q	2-0-12	-163	-163	—	FRONT	VERT	TOTAL
R	2-4-12	-1296	-1296	—	BACK	VERT	TOTAL
S	4-0-12	-163	-163	—	FRONT	VERT	TOTAL
T	4-4-12	-1296	-1296	—	BACK	VERT	TOTAL
U	6-4-12	-1296	-1296	—	BACK	VERT	TOTAL
V	8-0-12	-11	-20	—	FRONT	VERT	TOTAL
W	8-4-12	-1296	-1296	—	BACK	VERT	TOTAL
X	10-4-12	-1296	-1296	—	BACK	VERT	TOTAL
Y	14-4-12	-1296	-1296	—	BACK	VERT	TOTAL
Z	16-4-12	-1296	-1296	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7281-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T66	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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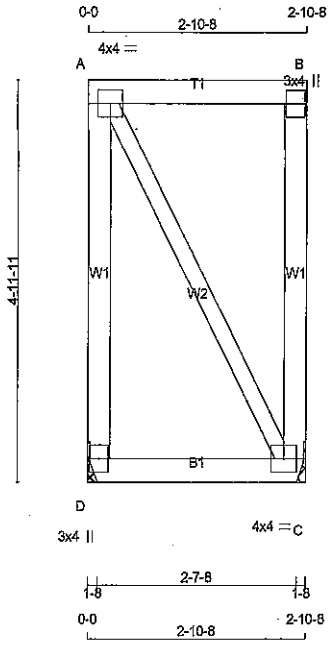

Scale = 1:28.6

TOTAL WEIGHT = 21 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	4.0	
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	191	0	191	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
C	191	0	191	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	152	93 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0
C	152	93 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-151 / 0	0.0	0.0	0.06 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-104.9	-104.9	0.15 (1)	10.00			
C- B	-151 / 0	0.0	0.0	0.06 (1)	7.81			
D- C	0 / 0	-28.0	-28.0	0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF.
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.07/1.00 (C-D:2) , WB=0.00/1.00 (A-C:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (C) (INPUT = 0.90)
JSI METAL= 0.02 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7282-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T67	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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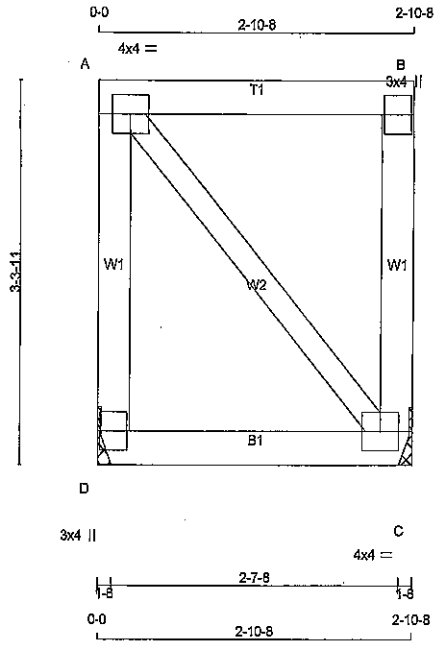

Scale = 1:19.9

TOTAL WEIGHT = 16 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	4.0	
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	191	0	191	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
C	191	0	191	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	152	93 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0
C	152	93 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-151 / 0	0.0	0.0	0.03 (1)	7.81		A- C	0 / 0	0.00 (1)
A- B	0 / 0	-104.9	-104.9	0.15 (1)	10.00				
C- B	-151 / 0	0.0	0.0	0.03 (1)	7.81				
D- C	0 / 0	-28.0	-28.0	0.07 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.07/1.00 (C-D:2) , WB=0.00/1.00 (A-C:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (C) (INPUT = 0.90)
JSI METAL= 0.02 (C) (INPUT = 1.00)

DWG NO. TAM 72838
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T68	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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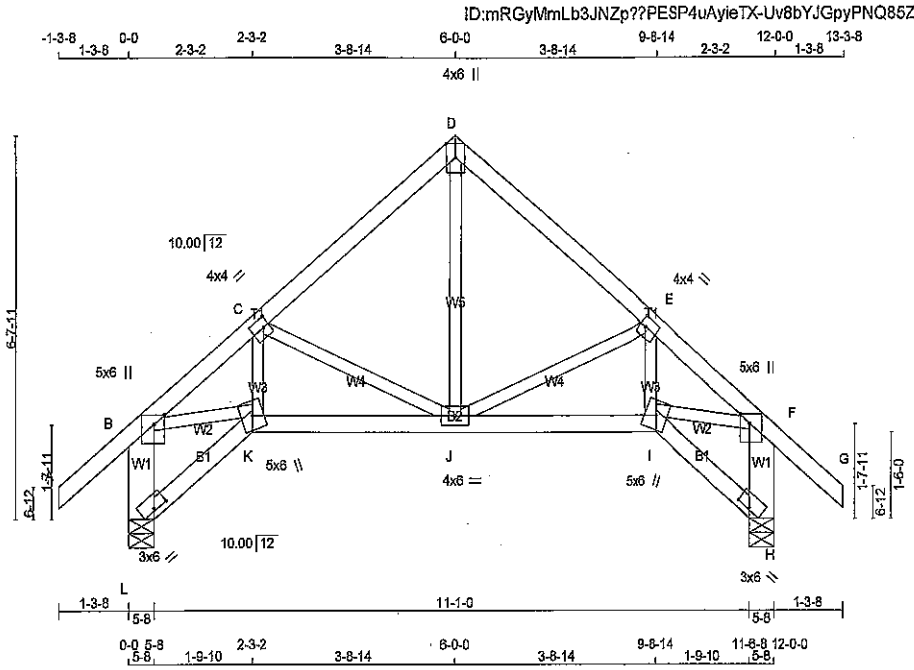

TOTAL WEIGHT = 2 X 58 = 116 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BVM1-I	MT20	3.0	6.0	0.50	3.00
I	BBWW+m	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BBWW+m	MT20	5.0	6.0		
L	BVM1-I	MT20	3.0	6.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	943	0	943	0	0	5-8	5-8
H	943	0	943	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1186 / 0	-104.9	-104.9	0.13 (1)	5.73	
C- D	-680 / 0	-104.9	-104.9	0.17 (1)	6.25	
D- E	-680 / 0	-104.9	-104.9	0.17 (1)	6.25	
E- F	-1186 / 0	-104.9	-104.9	0.13 (1)	5.73	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
L- B	-911 / 0	0.0	0.0	0.07 (1)	7.81	
H- F	-911 / 0	0.0	0.0	0.07 (1)	7.81	
L- K	0 / 0	-28.0	-28.0	0.04 (2)	10.00	
K- J	0 / 924	-28.0	-28.0	0.21 (1)	10.00	
J- I	0 / 924	-28.0	-28.0	0.21 (1)	10.00	
I- H	0 / 0	-28.0	-28.0	0.04 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 524	0.12 (1)
J- E	-464 / 0	0.13 (1)
I- E	0 / 187	0.04 (2)
I- F	0 / 939	0.21 (1)
C- J	-464 / 0	0.13 (1)
K- C	0 / 187	0.04 (2)
B- K	0 / 939	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.21/1.00 (I-J:1) , WB=0.21/1.00 (F-I:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7284-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T69S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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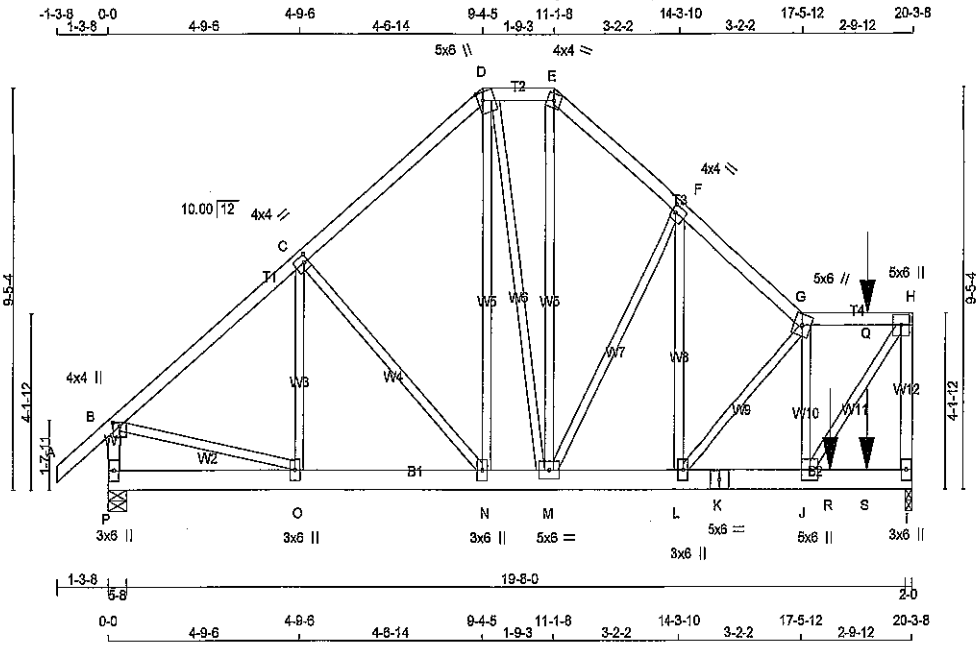

TOTAL WEIGHT = 2 X 127 = 254 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- G	1	12	TOP
G- H	1	12	SIDE(0.0)
H- I	1	12	TOP
P- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- K	2	12	TOP
K- I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TTWW+m	MT20	5.0	6.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW+t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L, N, O						
L	BMWW+t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	3126	0	3126	0	0	2-0	2-0
P	1689	0	1689	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2471	1550 / 0	466 / 0	0 / 0	0 / 0	454 / 0	0 / 0
P	1326	847 / 0	240 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-1504 / 0	-104.9	-104.9	0.23 (1)	6.25
C- D	-1281 / 0	-104.9	-104.9	0.22 (1)	6.25
D- E	-998 / 0	-104.9	-104.9	0.03 (1)	6.25
E- F	-1319 / 0	-104.9	-104.9	0.10 (1)	6.25
F- G	-1702 / 0	-104.9	-104.9	0.10 (1)	6.25
G- Q	-1805 / 0	-104.9	-104.9	0.15 (1)	6.25
Q- H	-1805 / 0	-104.9	-104.9	0.15 (1)	6.25
I- H	-2722 / 0	0.0	0.0	0.35 (1)	6.92
P- B	-1627 / 0	0.0	0.0	0.09 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.04 (3)	10.00
O- N	0 / 1183	-28.0	-28.0	0.10 (1)	10.00
N- M	0 / 955	-28.0	-28.0	0.10 (1)	10.00
M- L	0 / 1330	-28.0	-28.0	0.13 (1)	10.00
L- K	0 / 1829	-28.0	-28.0	0.19 (1)	10.00
K- J	0 / 1829	-28.0	-28.0	0.19 (1)	10.00
J- R	0 / 0	-28.0	-28.0	0.27 (1)	10.00
R- S	0 / 0	-28.0	-28.0	0.27 (1)	10.00
S- I	0 / 0	-28.0	-28.0	0.27 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-163 / 140	0.04 (1)
C- N	-347 / 0	0.16 (1)
N- D	0 / 291	0.04 (1)
D- M	0 / 238	0.03 (1)
M- E	0 / 612	0.08 (1)
M- F	-741 / 0	0.37 (1)
L- F	0 / 517	0.06 (1)
L- G	-771 / 0	0.15 (1)
J- G	-854 / 0	0.11 (1)
J- H	0 / 3093	0.38 (1)
B- O	0 / 1221	0.15 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	19-1-12	-130	-130	—	BACK	VERT	TOTAL
R	18-2-8	-1802	-1802	—	BACK	VERT	TOTAL
S	19-1-12	-41	-71	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35/1.00 (H-I:1) , BC=0.27/1.00 (I-J:1) , WB=0.38/1.00 (H-J:1) , SSI=0.50/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (O) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 7285-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T69S	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 19-1-12 ON TOP CHORD, AND 1801.9 lbs FACTORED DOWN AT 18-2-8, AND 71.5 lbs FACTORED DOWN AT 19-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7285-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T70S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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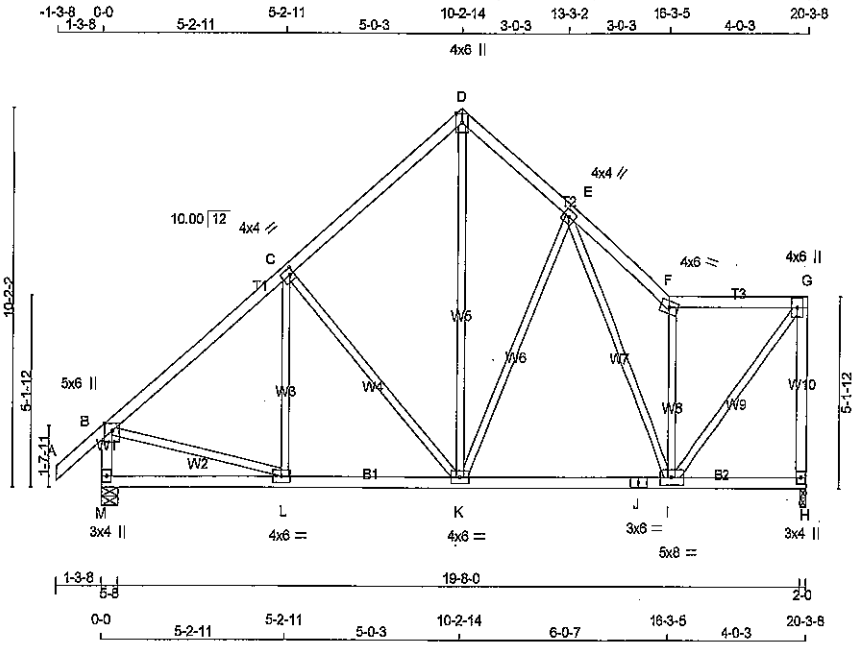

TOTAL WEIGHT = 2 X 104 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW+t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	2-0	2-0
M	1494	0	1494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
M	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- C	-97 / 168	0.05 (1)	
B- C	-1253 / 0	-104.9	-104.9	0.39 (1)	5.29	C- K	-438 / 0	0.50 (1)	
C- D	-956 / 0	-104.9	-104.9	0.37 (1)	5.87	K- D	0 / 854	0.19 (1)	
D- E	-939 / 0	-104.9	-104.9	0.13 (1)	6.25	K- E	-392 / 0	0.51 (1)	
E- F	-1159 / 0	-104.9	-104.9	0.13 (1)	5.78	E- I	0 / 177	0.04 (2)	
F- G	-868 / 0	-104.9	-104.9	0.23 (1)	6.25	I- F	-1090 / 0	0.43 (1)	
H- G	-1308 / 0	0.0	0.0	0.59 (1)	7.06	I- G	0 / 1415	0.32 (1)	
M- B	-1433 / 0	0.0	0.0	0.15 (1)	6.82	B- L	0 / 1025	0.23 (1)	
M- L	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
L- K	0 / 994	-28.0	-28.0	0.29 (2)	10.00				
K- J	0 / 852	-28.0	-28.0	0.27 (2)	10.00				
J- I	0 / 852	-28.0	-28.0	0.27 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.59/1.00 (G-H:1) , BC=0.29/1.00 (K-L:2) , WB=0.51/1.00 (E-K:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7286-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T71A	1	2	TRUSS DESC.	

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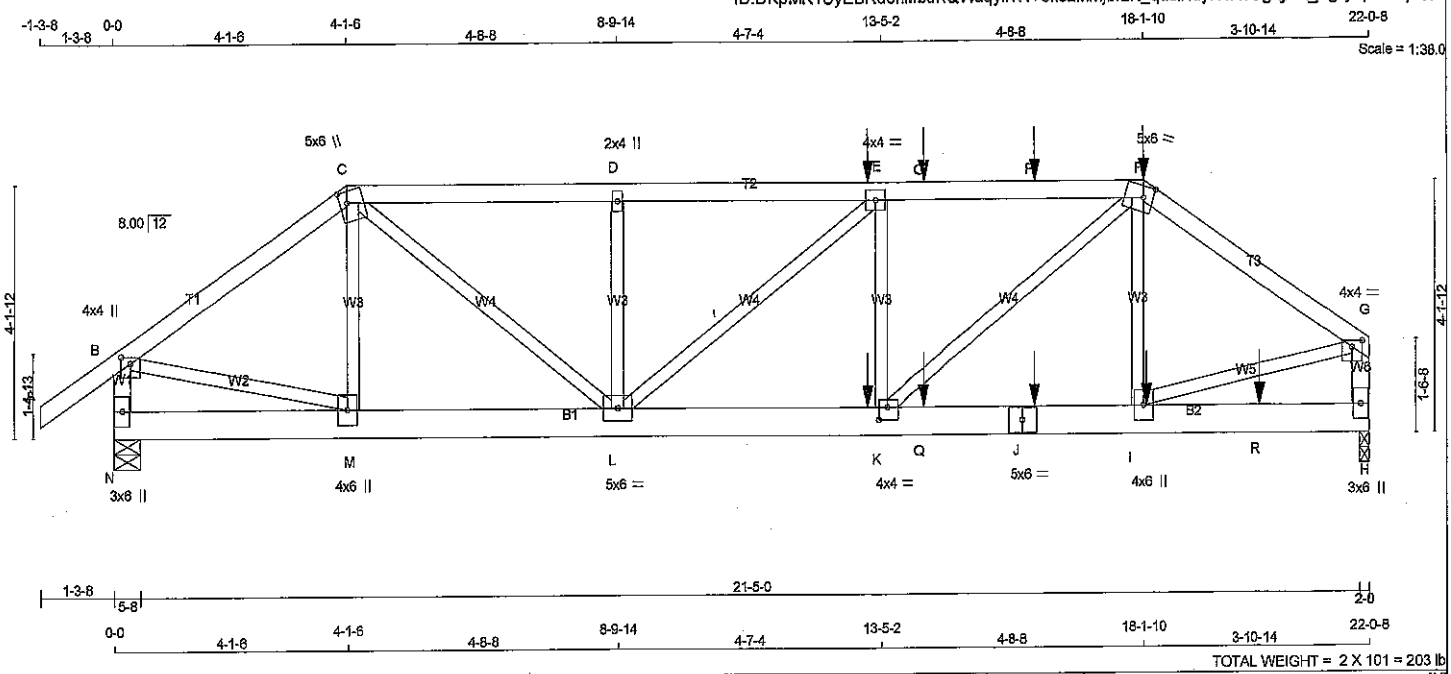


TOTAL WEIGHT = 2 X 101 = 203 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
N- B	1	12	TOP
H- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.50	1.75
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1817	0	1817	0	0	5-8	5-8
H	1984	0	1984	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1436	900 / 0	271 / 0	0 / 0	0 / 0	264 / 0	0 / 0
H	1606	941 / 0	345 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

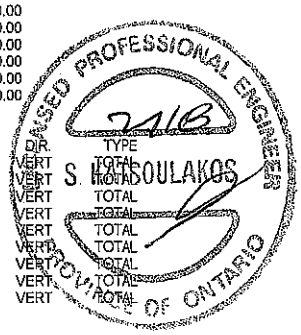
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-1872 / 0	-104.9	-104.9	0.19 (1)	6.15	
C- D	-2583 / 0	-104.9	-104.9	0.19 (1)	5.45	
D- E	-2583 / 0	-104.9	-104.9	0.27 (1)	5.33	
E- O	-2858 / 0	-104.9	-104.9	0.31 (1)	5.08	
O- P	-2858 / 0	-104.9	-104.9	0.31 (1)	5.08	
P- F	-2858 / 0	-104.9	-104.9	0.31 (1)	5.08	
F- G	-2170 / 0	-104.9	-104.9	0.16 (1)	5.83	
N- B	-1769 / 0	0.0	0.0	0.10 (1)	7.81	
H- G	-1924 / 0	0.0	0.0	0.11 (1)	7.81	
N- M	0 / 0	-28.0	-28.0	0.03 (3)	10.00	
M- L	0 / 1549	-28.0	-28.0	0.13 (1)	10.00	
L- K	0 / 2858	-28.0	-28.0	0.22 (1)	10.00	
K- Q	0 / 1795	-28.0	-28.0	0.15 (1)	10.00	
Q- J	0 / 1795	-28.0	-28.0	0.15 (1)	10.00	
J- I	0 / 1795	-28.0	-28.0	0.15 (1)	10.00	
I- R	0 / 0	-28.0	-28.0	0.05 (3)	10.00	
R- H	0 / 0	-28.0	-28.0	0.05 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-215 / 83	0.03 (1)
C- L	0 / 1323	0.16 (1)
L- D	-494 / 0	0.06 (1)
L- E	-356 / 0	0.11 (1)
K- E	-636 / 52	0.06 (1)
K- F	0 / 1360	0.17 (1)
I- F	-266 / 174	0.03 (1)
B- M	0 / 1597	0.20 (1)
I- G	0 / 1873	0.23 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	13-2-4	-130	-130	---	FRONT	VERT	TOTAL
F	18-1-10	-133	-133	---	FRONT	VERT	TOTAL
I	18-2-4	-41	-71	---	FRONT	VERT	TOTAL
J	16-2-4	-41	-71	---	FRONT	VERT	TOTAL
K	13-2-4	-41	-71	---	FRONT	VERT	TOTAL
O	14-2-4	-130	-130	---	FRONT	VERT	TOTAL
P	16-2-4	-130	-130	---	FRONT	VERT	TOTAL
Q	14-2-4	-41	-71	---	FRONT	VERT	TOTAL
R	20-2-4	-41	-71	---	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.31/1.00 (E-F:1) , BC=0.22/1.00 (K-L:1) , WB=0.23/1.00 (G-I:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.30 (G) (INPUT = 1.00)

DWG NO. TAM 7310-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T71A	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 13-2-4, 129.6 lbs FACTORED DOWN AT 14-2-4, AND 129.6 lbs FACTORED DOWN AT 16-2-4, AND 133.1 lbs FACTORED DOWN AT 18-1-10 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 13-2-4, 71.5 lbs FACTORED DOWN AT 14-2-4, 71.5 lbs FACTORED DOWN AT 16-2-4, AND 71.5 lbs FACTORED DOWN AT 18-2-4, AND 71.5 lbs FACTORED DOWN AT 20-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 7 3 1 0 - 1 8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T72A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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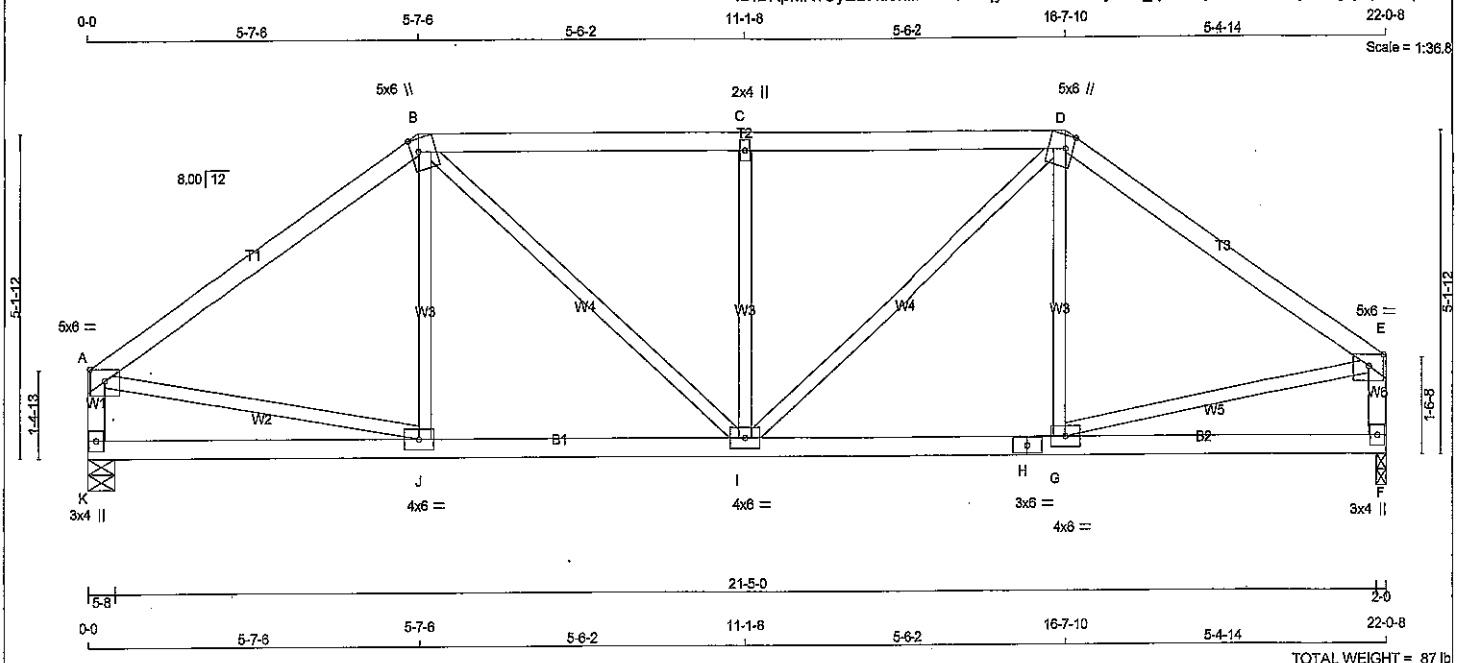

TOTAL WEIGHT = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMVW-p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1465	0	1465	0	0	5-8	5-8
F	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
F	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1551 / 0	-104.9	-104.9	0.70 (1)	4.35	
B- C	-1750 / 0	-104.9	-104.9	0.62 (1)	4.22	
C- D	-1750 / 0	-104.9	-104.9	0.62 (1)	4.22	
D- E	-1513 / 0	-104.9	-104.9	0.64 (1)	4.49	
K- A	-1400 / 0	0.0	0.0	0.14 (1)	6.88	
F- E	-1403 / 0	0.0	0.0	0.15 (1)	6.87	
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00	
J- I	0 / 1287	-28.0	-28.0	0.36 (2)	10.00	
I- H	0 / 1255	-28.0	-28.0	0.35 (2)	10.00	
H- G	0 / 1255	-28.0	-28.0	0.35 (2)	10.00	
G- F	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-63 / 205	0.05 (3)
B- I	0 / 616	0.14 (1)
I- C	-707 / 0	0.28 (1)
I- D	0 / 659	0.15 (1)
G- D	-102 / 183	0.04 (3)
A- J	0 / 1311	0.30 (1)
G- E	0 / 1286	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.70/1.00 (A-B:1) , BC=0.36/1.00 (I-J:2) , WB=0.30/1.00 (A-J:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7311 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T73A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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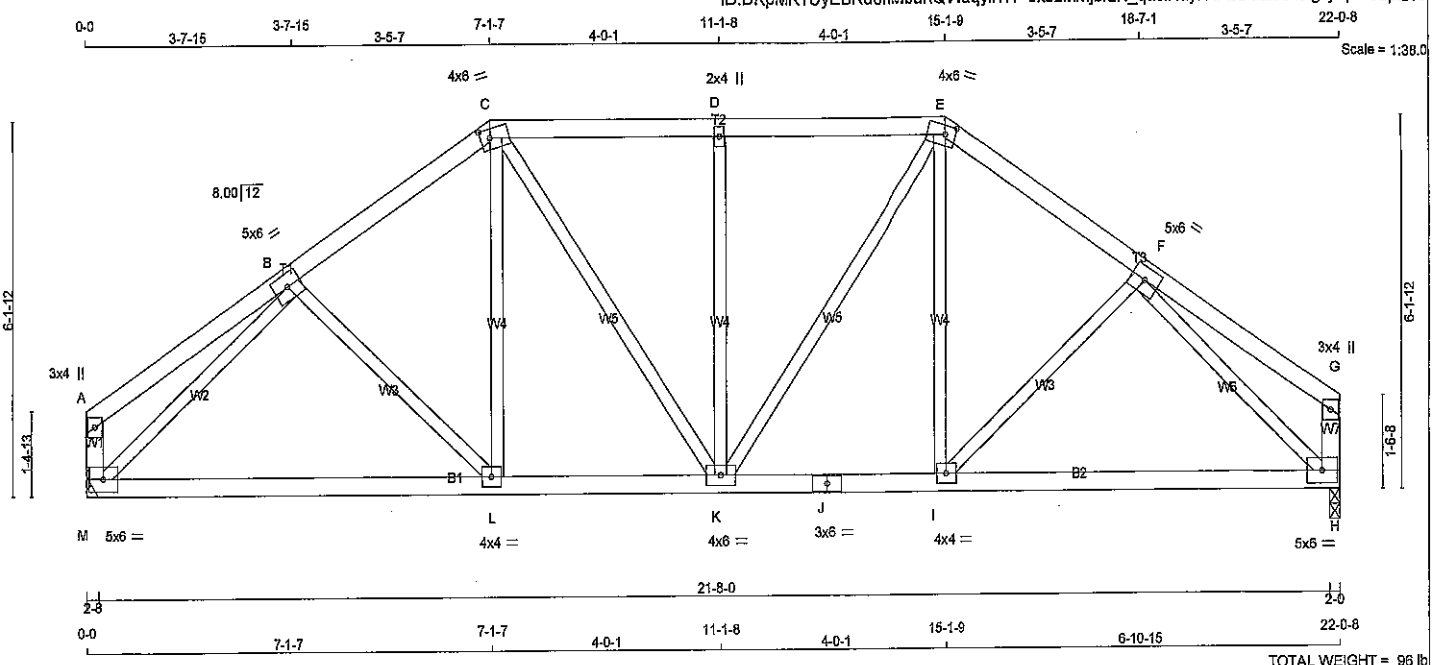

TOTAL WEIGHT = 96 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.00
D	TMV+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.00
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1465	0	1465	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-8	
H	1465	0	1465	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
H	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-104.9	-104.9	0.20 (1)	10.00
B- C	-1501 / 0	-104.9	-104.9	0.18 (1)	5.19
C- D	-1409 / 0	-104.9	-104.9	0.23 (1)	5.26
D- E	-1409 / 0	-104.9	-104.9	0.23 (1)	5.26
E- F	-1477 / 0	-104.9	-104.9	0.17 (1)	5.23
F- G	0 / 22	-104.9	-104.9	0.19 (1)	10.00
M- A	-146 / 0	0.0	0.0	0.02 (1)	7.81
H- G	-135 / 0	0.0	0.0	0.01 (1)	7.81
M- L	0 / 1273	-28.0	-28.0	0.45 (2)	10.00
L- K	0 / 1233	-28.0	-28.0	0.45 (2)	10.00
K- J	0 / 1213	-28.0	-28.0	0.44 (2)	10.00
J- I	0 / 1213	-28.0	-28.0	0.44 (2)	10.00
I- H	0 / 1211	-28.0	-28.0	0.43 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	-61 / 99	0.03 (1)
L- C	0 / 330	0.07 (2)
C- K	0 / 312	0.07 (1)
K- D	-507 / 0	0.30 (1)
K- E	0 / 347	0.08 (1)
I- E	0 / 298	0.07 (2)
I- F	-3 / 117	0.03 (3)
M- B	-1774 / 0	0.69 (1)
F- H	-1740 / 0	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.46/1.00 (K-L:2) , WB=0.69/1.00 (B-M:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7312-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T74	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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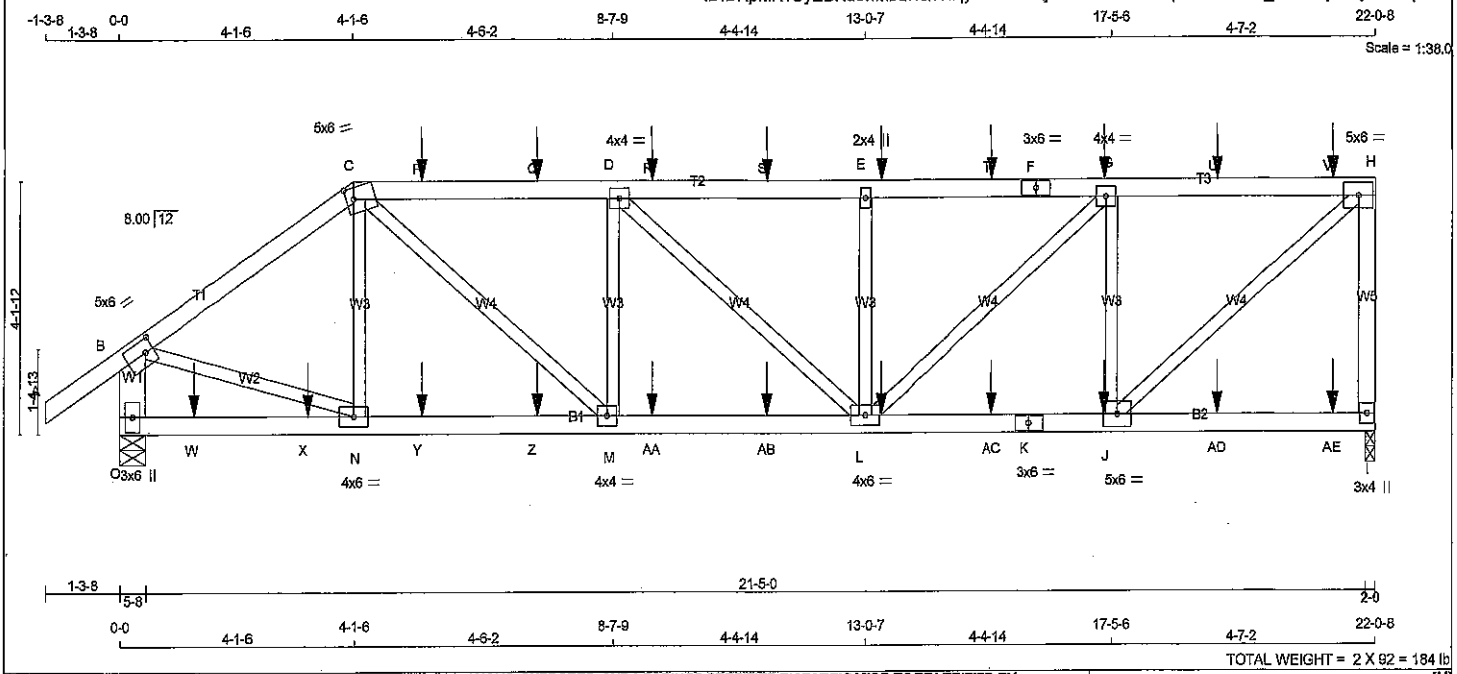


TOTAL WEIGHT = 2 X 92 = 184 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	1	12	SIDE(61.0)
K- I	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	2416	0	2416	0	0	2-0	2-0
O	2291	0	2291	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1945	1160 / 0	407 / 0	0 / 0	0 / 0	378 / 0	0 / 0
O	1852	1092 / 0	396 / 0	0 / 0	0 / 0	365 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40		-104.9	-104.9	0.08 (1)	10.00
B- C	-2414 / 0		-104.9	-104.9	0.20 (1)	5.58
C- P	-3292 / 0		-104.9	-104.9	0.33 (1)	4.77
P- Q	-3292 / 0		-104.9	-104.9	0.33 (1)	4.77
Q- D	-3292 / 0		-104.9	-104.9	0.33 (1)	4.77
D- R	-3326 / 0		-104.9	-104.9	0.33 (1)	4.75
R- S	-3326 / 0		-104.9	-104.9	0.33 (1)	4.75
S- E	-3326 / 0		-104.9	-104.9	0.33 (1)	4.75
E- T	-3326 / 0		-104.9	-104.9	0.31 (1)	4.78
T- F	-3326 / 0		-104.9	-104.9	0.31 (1)	4.78
F- G	-3326 / 0		-104.9	-104.9	0.31 (1)	4.78
G- U	-2352 / 0		-104.9	-104.9	0.26 (1)	5.51
U- V	-2352 / 0		-104.9	-104.9	0.26 (1)	5.51
V- H	-2352 / 0		-104.9	-104.9	0.26 (1)	5.51
I- H	-2315 / 0		0.0	0.0	0.32 (1)	7.37
O- B	-2217 / 0		0.0	0.0	0.08 (1)	7.81
O- W	0 / 0		-28.0	-28.0	0.12 (3)	10.00
W- X	0 / 0		-28.0	-28.0	0.12 (3)	10.00
X- N	0 / 0		-28.0	-28.0	0.12 (3)	10.00
N- Y	0 / 1998		-28.0	-28.0	0.25 (2)	10.00
Y- Z	0 / 1998		-28.0	-28.0	0.25 (2)	10.00
Z- M	0 / 1998		-28.0	-28.0	0.25 (2)	10.00
M-AA	0 / 3293		-28.0	-28.0	0.34 (1)	10.00
AA-AB	0 / 3293		-28.0	-28.0	0.34 (1)	10.00
AB- L	0 / 3293		-28.0	-28.0	0.34 (1)	10.00
L-AC	0 / 2352		-28.0	-28.0	0.27 (1)	10.00
AC- K	0 / 2352		-28.0	-28.0	0.27 (1)	10.00
K- J	0 / 2352		-28.0	-28.0	0.27 (1)	10.00
J-AD	0 / 0		-28.0	-28.0	0.12 (3)	10.00
AD-AE	0 / 0		-28.0	-28.0	0.12 (3)	10.00
AE- I	0 / 0		-28.0	-28.0	0.12 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
N- C	-276 / 164		0.04 (1)
B- N	0 / 2070		0.26 (1)
C- M	0 / 1700		0.21 (1)
M- D	-892 / 0		0.12 (1)
D- L	0 / 44		0.01 (1)
L- E	-680 / 0		0.09 (1)
L- G	0 / 1294		0.16 (1)
J- G	-1726 / 0		0.24 (1)
J- H	0 / 3070		0.38 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
E	13-3-12	-130	-130	—	BACK	VERT	TOTAL
G	17-3-12	-130	-130	—	BACK	VERT	TOTAL
J	17-3-12	-41	-71	—	BACK	VERT	TOTAL
L	13-3-12	-41	-71	—	BACK	VERT	TOTAL
P	5-3-12	-130	-130	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.34/1.00 (L-M:1) , WB=0.38/1.00 (H-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM 7313-18 P6/6

STRUCTURAL COMPONENT ONLY

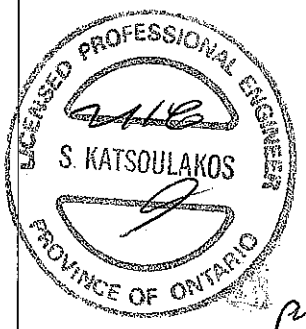
CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.6 lbs FACTORED DOWN AT 5-3-12, 129.6 lbs FACTORED DOWN AT 7-3-12, 129.6 lbs FACTORED DOWN AT 9-3-12, 129.6 lbs FACTORED DOWN AT 11-3-12, 129.6 lbs FACTORED DOWN AT 13-3-12, 129.6 lbs FACTORED DOWN AT 15-3-12, 129.6 lbs FACTORED DOWN AT 17-3-12, AND 129.6 lbs FACTORED DOWN AT 19-3-12, AND 143.8 lbs FACTORED DOWN AT 21-3-12 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 1-3-12, 71.5 lbs FACTORED DOWN AT 3-3-12, 71.5 lbs FACTORED DOWN AT 5-3-12, 71.5 lbs FACTORED DOWN AT 7-3-12, 71.5 lbs FACTORED DOWN AT 9-3-12, 71.5 lbs FACTORED DOWN AT 11-3-12, 71.5 lbs FACTORED DOWN AT 13-3-12, 71.5 lbs FACTORED DOWN AT 15-3-12, 71.5 lbs FACTORED DOWN AT 17-3-12, AND 71.5 lbs FACTORED DOWN AT 19-3-12, AND 78.1 lbs FACTORED DOWN AT 21-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	7-3-12	-130	-130	—	BACK	VERT	TOTAL
R	9-3-12	-130	-130	—	BACK	VERT	TOTAL
S	11-3-12	-130	-130	—	BACK	VERT	TOTAL
T	15-3-12	-130	-130	—	BACK	VERT	TOTAL
U	19-3-12	-130	-130	—	BACK	VERT	TOTAL
V	21-3-12	-144	-144	—	BACK	VERT	TOTAL
W	1-3-12	-41	-71	—	BACK	VERT	TOTAL
X	3-3-12	-41	-71	—	BACK	VERT	TOTAL
Y	5-3-12	-41	-71	—	BACK	VERT	TOTAL
Z	7-3-12	-41	-71	—	BACK	VERT	TOTAL
AA	9-3-12	-41	-71	—	BACK	VERT	TOTAL
AB	11-3-12	-41	-71	—	BACK	VERT	TOTAL
AC	15-3-12	-41	-71	—	BACK	VERT	TOTAL
AD	19-3-12	-41	-71	—	BACK	VERT	TOTAL
AE	21-3-12	-45	-78	—	BACK	VERT	TOTAL



DWG NO. TAM7313-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T75	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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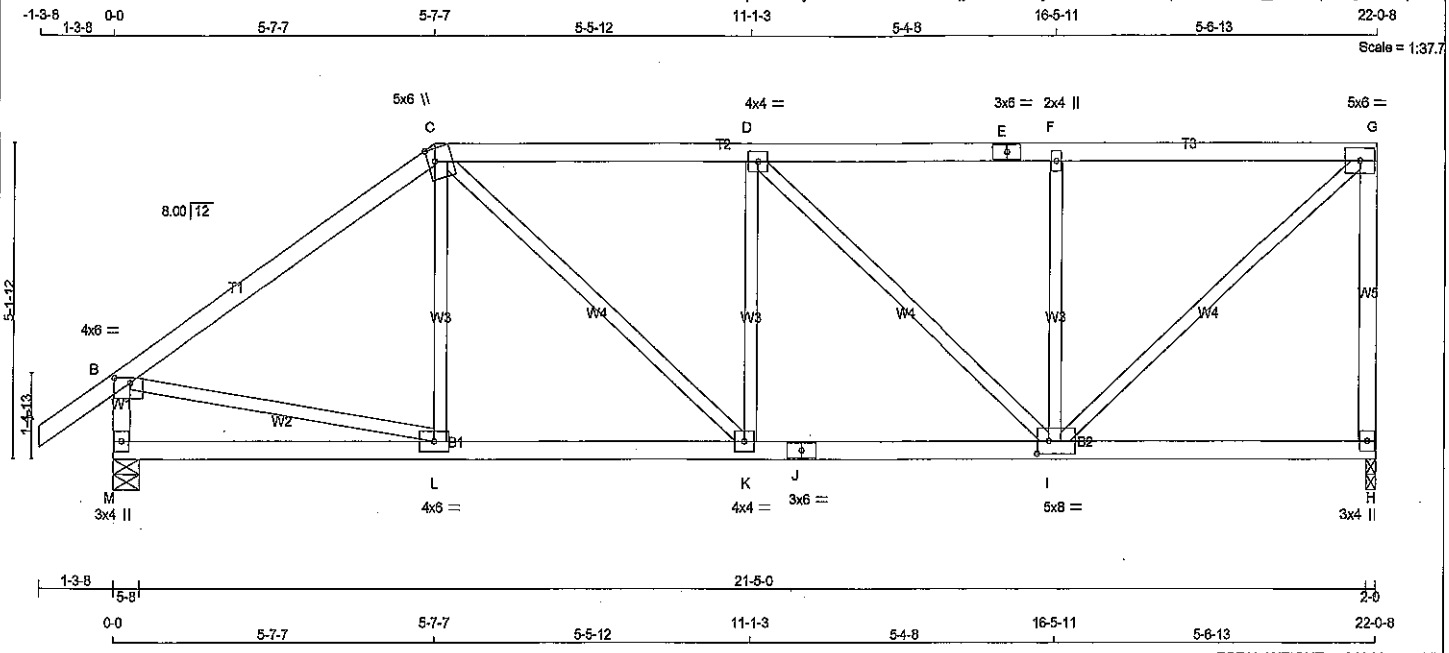

TOTAL WEIGHT = 2 X 92 = 184 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1465	0	1465	0	0	2-0	2-0
M	1609	0	1609	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1166	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
M	1265	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1550 / 0	-104.9	-104.9	0.70 (1)	4.35	
C- D	-1734 / 0	-104.9	-104.9	0.50 (1)	4.47	
D- E	-1334 / 0	-104.9	-104.9	0.47 (1)	4.98	
E- F	-1334 / 0	-104.9	-104.9	0.47 (1)	4.98	
F- G	-1334 / 0	-104.9	-104.9	0.48 (1)	4.98	
H- G	-1401 / 0	0.0	0.0	0.63 (1)	6.88	
M- B	-1544 / 0	0.0	0.0	0.16 (1)	6.62	
M- L	0 / 0	-28.0	-28.0	0.22 (3)	10.00	
L- K	0 / 1287	-28.0	-28.0	0.36 (2)	10.00	
K- J	0 / 1734	-28.0	-28.0	0.39 (2)	10.00	
J- I	0 / 1734	-28.0	-28.0	0.39 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-65 / 205	0.05 (3)
B- L	0 / 1311	0.30 (1)
C- K	0 / 596	0.13 (1)
K- D	-258 / 103	0.10 (1)
D- I	-539 / 0	0.54 (1)
I- F	-630 / 0	0.25 (1)
I- G	0 / 1770	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.39/1.00 (I-K:2) , WB=0.54/1.00 (D-I:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM73/14-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T76	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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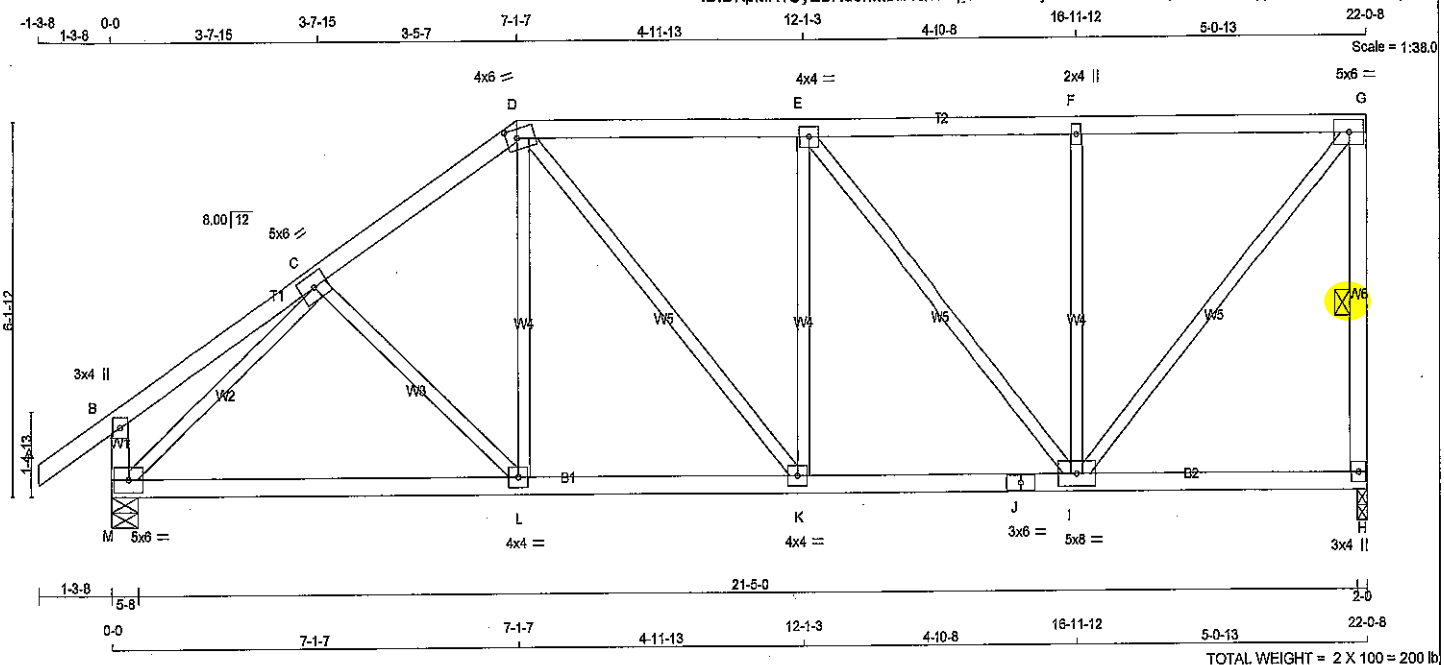

TOTAL WEIGHT = 2 X 100 = 200 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1465	0	1465	0	0	2-0	2-0
M	1609	0	1609	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1166	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
M	1265	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- L	-61 / 99	0.03 (1)
B- C	0 / 22	-104.9	-104.9	0.20 (1)	10.00	L- D	0 / 324	0.07 (2)
C- D	-1501 / 0	-104.9	-104.9	0.23 (1)	5.10	M- C	-1774 / 0	0.69 (1)
D- E	-1412 / 0	-104.9	-104.9	0.39 (1)	5.00	D- K	0 / 278	0.08 (1)
E- F	-1030 / 0	-104.9	-104.9	0.37 (1)	5.66	K- E	-104 / 143	0.06 (1)
F- G	-1030 / 0	-104.9	-104.9	0.37 (1)	5.66	E- I	-597 / 0	0.69 (1)
H- G	-1407 / 0	0.0	0.0	0.25 (1)	5.49	I- F	-571 / 0	0.34 (1)
M- B	-290 / 0	0.0	0.0	0.03 (1)	7.81	I- G	0 / 1575	0.35 (1)
M- L	0 / 1273	-28.0	-28.0	0.44 (2)	10.00			
L- K	0 / 1232	-28.0	-28.0	0.45 (2)	10.00			
K- J	0 / 1413	-28.0	-28.0	0.33 (2)	10.00			
J- I	0 / 1413	-28.0	-28.0	0.33 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.45/1.00 (K-L:2) , WB=0.69/1.00 (C-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM D 315 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T77	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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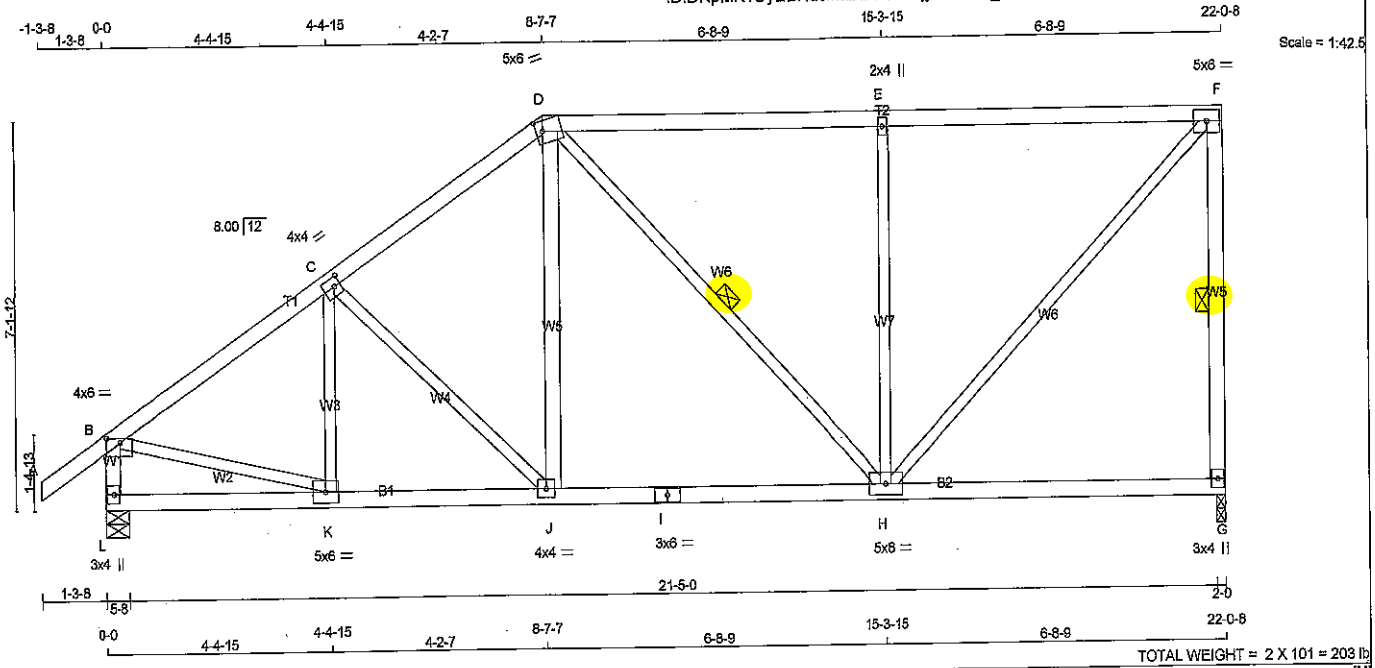

TOTAL WEIGHT = 2 X 101 = 203 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
L	1609	0	1609	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1265	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1595 / 0	-104.9	-104.9	0.38 (1)	4.80	
C- D	-1388 / 0	-104.9	-104.9	0.36 (1)	5.06	
D- E	-1100 / 0	-104.9	-104.9	0.89 (1)	4.22	
E- F	-1100 / 0	-104.9	-104.9	0.89 (1)	4.22	
G- F	-1390 / 0	0.0	0.0	0.32 (1)	5.52	
L- B	-1555 / 0	0.0	0.0	0.16 (1)	6.61	
L- K	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
K- J	0 / 1351	-28.0	-28.0	0.31 (1)	10.00	
J- I	0 / 1136	-28.0	-28.0	0.44 (2)	10.00	
I- H	0 / 1136	-28.0	-28.0	0.44 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.32 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-208 / 86	0.06 (1)
C- J	-303 / 0	0.18 (1)
J- D	0 / 384	0.06 (2)
D- H	-55 / 0	0.03 (2)
H- E	-874 / 0	0.76 (1)
H- F	0 / 1573	0.35 (1)
B- K	0 / 1391	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.89/1.00 (E-F:1) , BC=0.44/1.00 (H-J:2) , WB=0.78/1.00 (E-H:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM 7316-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T78	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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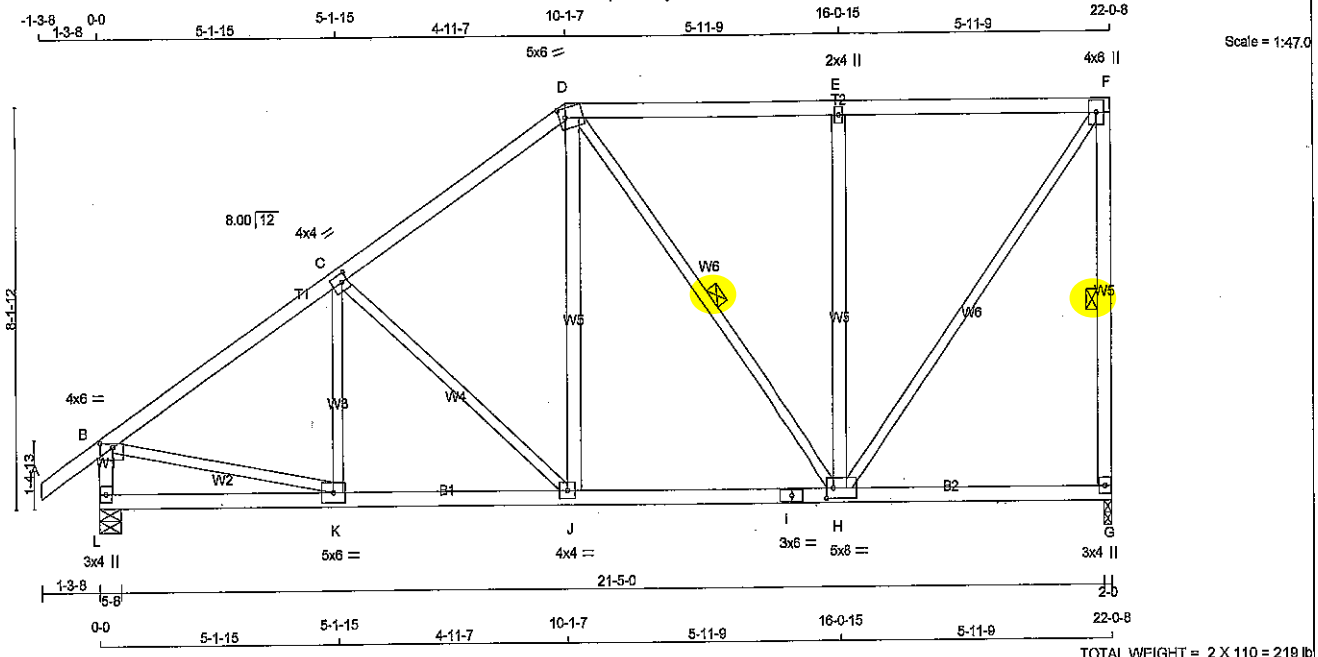

TOTAL WEIGHT = 2 X 110 = 219 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0	2.50	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1465	0	1465	0	0	2-0	2-0
L	1609	0	1609	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1168	716 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1265	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1607 / 0	-104.9	-104.9	0.54 (1)	4.56
C- D	-1268 / 0	-104.9	-104.9	0.50 (1)	5.04
D- E	-881 / 0	-104.9	-104.9	0.67 (1)	5.34
E- F	-881 / 0	-104.9	-104.9	0.67 (1)	5.34
G- F	-1398 / 0	0.0	0.0	0.42 (1)	5.51
L- B	-1549 / 0	0.0	0.0	0.16 (1)	6.81
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00
K- J	0 / 1365	-28.0	-28.0	0.32 (2)	10.00
J- I	0 / 1032	-28.0	-28.0	0.36 (2)	10.00
I- H	0 / 1032	-28.0	-28.0	0.36 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-130 / 160	0.04 (1)
C- J	-457 / 0	0.39 (1)
J- D	0 / 450	0.07 (1)
D- H	-249 / 0	0.17 (1)
H- E	-776 / 0	0.71 (1)
H- F	0 / 1458	0.33 (1)
B- K	0 / 1395	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.67/1.00 (E-F:1) , BC=0.36/1.00 (H-J:2) , WB=0.71/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

DWG NO. TAM 7317-78
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T79	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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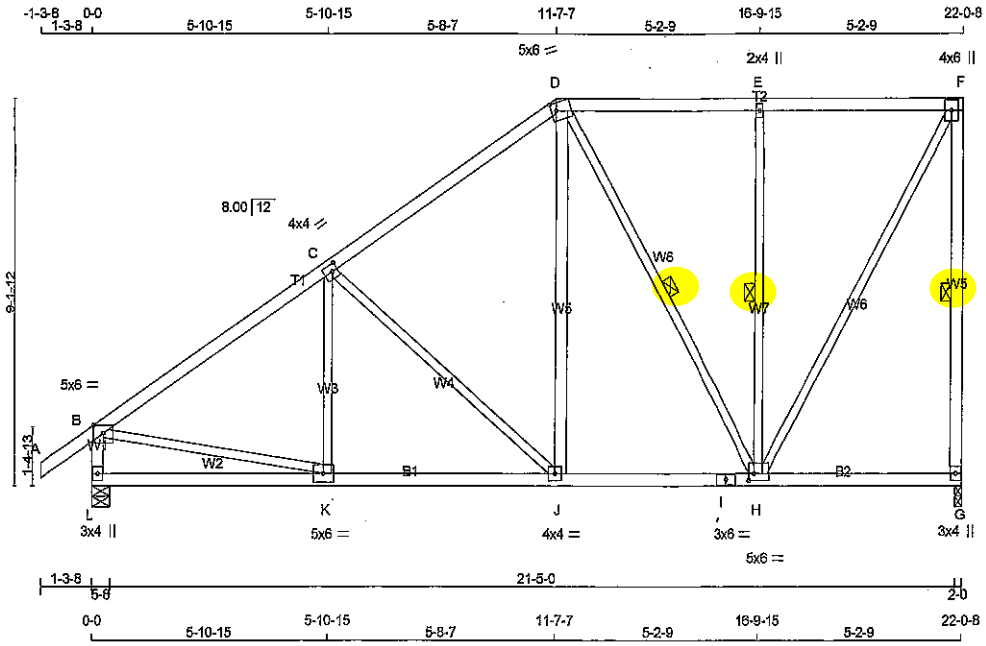

TOTAL WEIGHT = 6 X 113 = 678 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1465	0	1465	0	0	2-0	2-0
L	1609	0	1609	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1165	715 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
L	1265	805 / 0	231 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1601 / 0	-104.9	-104.9	0.52 (1)	4.61	
C- D	-1136 / 0	-104.9	-104.9	0.48 (1)	5.32	
D- E	-704 / 0	-104.9	-104.9	0.36 (1)	6.25	
E- F	-705 / 0	-104.9	-104.9	0.36 (1)	6.25	
G- F	-1405 / 0	0.0	0.0	0.55 (1)	5.50	
L- B	-1542 / 0	0.0	0.0	0.16 (1)	6.63	
L- K	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
K- J	0 / 1366	-28.0	-28.0	0.38 (2)	10.00	
J- I	0 / 918	-28.0	-28.0	0.28 (2)	10.00	
I- H	0 / 918	-28.0	-28.0	0.28 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-64 / 226	0.05 (3)
C- J	-601 / 0	0.72 (1)
J- D	0 / 538	0.09 (1)
D- H	-421 / 0	0.32 (1)
H- E	-676 / 0	0.37 (1)
H- F	0 / 1389	0.31 (1)
B- K	0 / 1388	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.38/1.00 (J-K:2) , WB=0.72/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

DWG NO. TAM7318-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T80	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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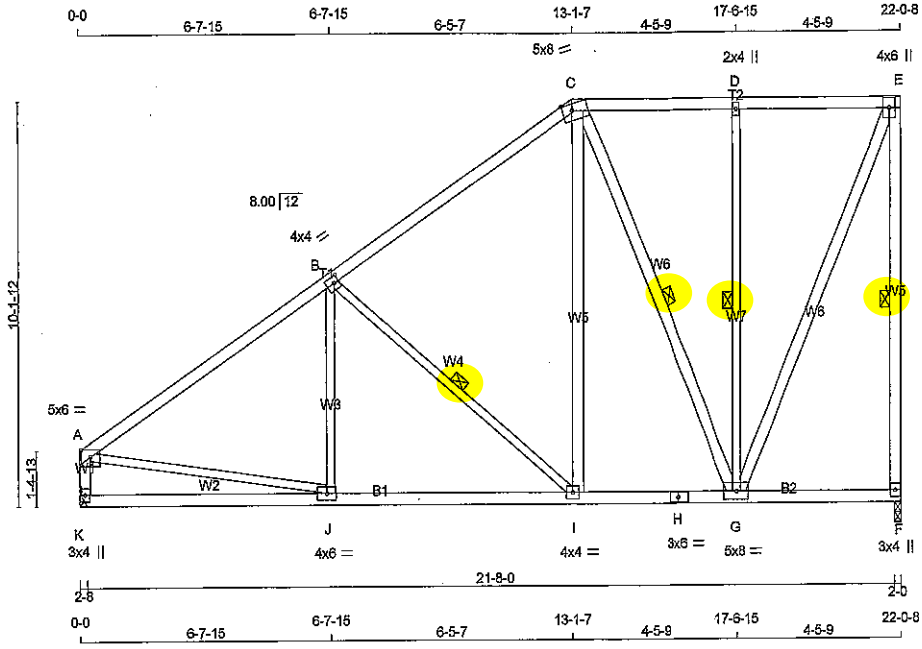


TOTAL WEIGHT = 2 X 124 = 248 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1465	0	1465	0	0	2-0	2-0
K	1465	0	1465	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1168	718 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0
K	1168	718 / 0	231 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1583 / 0	-104.9	-104.9	0.68 (1)		4.36
B- C	-997 / 0	-104.9	-104.9	0.61 (1)		5.31
C- D	-558 / 0	-104.9	-104.9	0.26 (1)		6.25
D- E	-558 / 0	-104.9	-104.9	0.26 (1)		6.25
F- E	-1412 / 0	0.0	0.0	0.71 (1)		5.49
K- A	-1390 / 0	0.0	0.0	0.14 (1)		6.90
K- J	0 / 0	-28.0	-28.0	0.32 (3)		10.00
J- I	0 / 1356	-28.0	-28.0	0.46 (2)		10.00
I- H	0 / 798	-28.0	-28.0	0.24 (2)		10.00
H- G	0 / 798	-28.0	-28.0	0.24 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.12 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-7 / 285	0.06 (3)
B- I	-738 / 0	0.37 (1)
I- C	0 / 624	0.10 (1)
C- G	-581 / 0	0.36 (1)
G- D	-577 / 0	0.41 (1)
G- E	0 / 1353	0.22 (1)
A- J	0 / 1373	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.71/1.00 (E-F:1) , BC=0.46/1.00 (I-J:2) , WB=0.41/1.00 (D-G:1) , SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

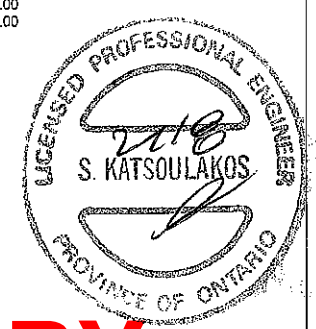
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)



DWG NO. TAM 7319 -S8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T81S	1	2	TRUSS DESC.	

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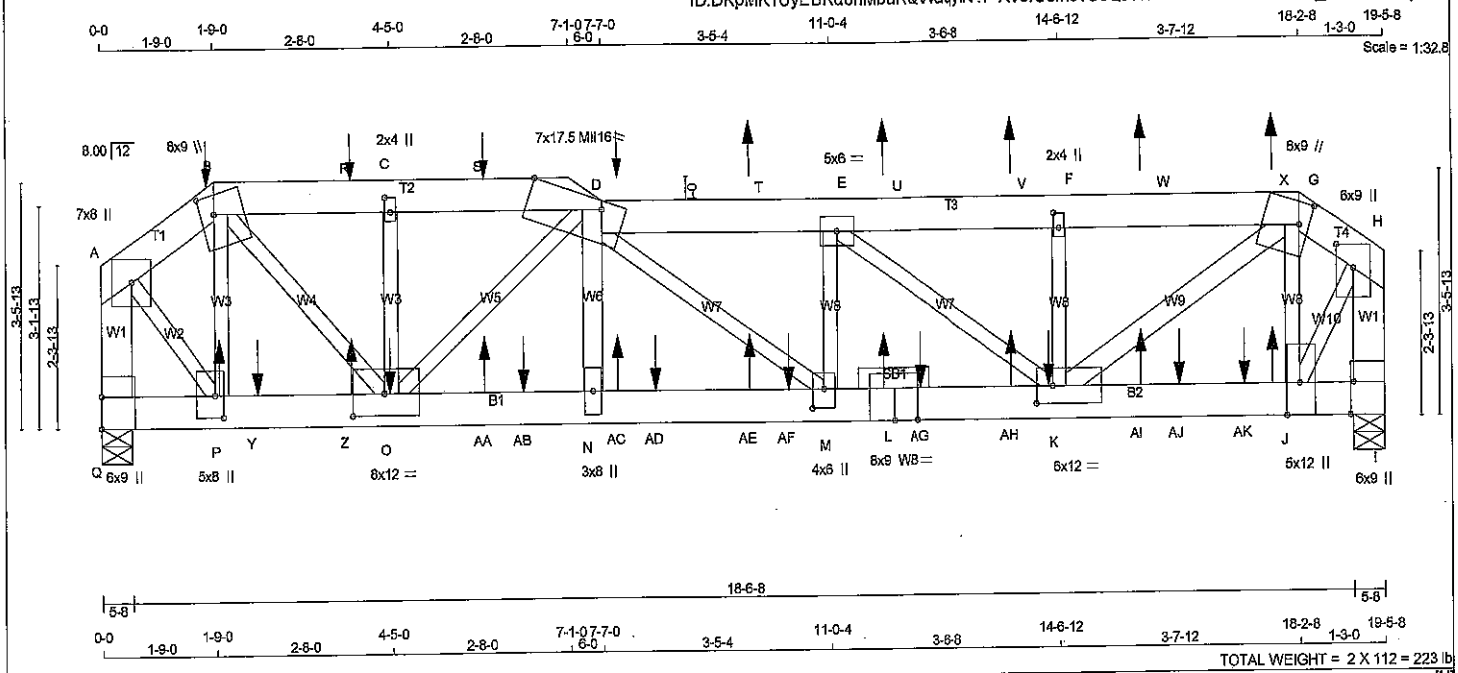

TOTAL WEIGHT = 2 X 112 = 223 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
Q - A	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
Q - L	2x6	DRY	1650F 1.5E	SPF
L - I	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	SIDE(122.0)
B- D	2	12	SIDE(61.0)
D- G	2	12	SIDE(122.0)
G- H	2	12	TOP
Q- A	2	12	TOP
I- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- L	2	12	SIDE(183.1)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
C- O	1	6	SIDE(229.4)
K- F	1	5	SIDE(229.4)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	7.0	8.0		
B	TTWW+m	MT20	8.0	9.0	3.25	2.50
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMTWWW-d	MII16	7.0	17.5	Edge	13.25
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TTWW+m	MT20	8.0	9.0	3.75	1.75
H	TMVW+p	MT20	6.0	9.0	4.00	3.00
I	BMV1+t	MT20	6.0	9.0	Edge	0.50
J	BMWW+t	MT20	5.0	12.0	Edge	2.25
K	BMWWW-t	MT20	6.0	12.0	3.00	3.00
L	BS-t	MT20	8.0	9.0		
M	BMWW+t	MT20	4.0	6.0	3.25	2.00
N	BMW+w	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	7698	0	7698	0	0	5-8	5-8
I	8284	0	8284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	6047	3852 / 0	1098 / 0	0 / 0	0 / 0	1097 / 0	0 / 0
I	8531	4113 / 0	1210 / 0	0 / 0	0 / 0	1208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-5743 / 0	-104.9	-104.9	0.05 (1)		4.90
B- R	-9729 / 0	-104.9	-104.9	0.19 (1)		3.82
R- C	-9729 / 0	-104.9	-104.9	0.19 (1)		3.82
C- S	-9740 / 0	-104.9	-104.9	0.24 (1)		3.76
S- D	-9740 / 0	-104.9	-104.9	0.24 (1)		3.76
D- T	-15396 / 0	-104.9	-104.9	0.48 (1)		2.80
T- E	-15396 / 0	-104.9	-104.9	0.48 (1)		2.80
E- U	-12135 / 0	-104.9	-104.9	0.34 (1)		3.30
U- V	-12135 / 0	-104.9	-104.9	0.34 (1)		3.30
V- F	-12135 / 0	-104.9	-104.9	0.34 (1)		3.30
F- W	-12136 / 0	-104.9	-104.9	0.29 (1)		3.36
W- X	-12136 / 0	-104.9	-104.9	0.29 (1)		3.36
X- G	-12136 / 0	-104.9	-104.9	0.29 (1)		3.36
G- H	-5414 / 0	-104.9	-104.9	0.04 (1)		5.03
Q- A	-7965 / 0	0.0	0.0	0.32 (1)		5.33
I- H	-9105 / 0	0.0	0.0	0.36 (1)		5.00
Q- P	0 / 0	-28.0	-28.0	0.14 (1)		10.00
P- Y	0 / 4651	-28.0	-28.0	0.33 (1)		10.00
Y- Z	0 / 4651	-28.0	-28.0	0.33 (1)		10.00
Z- O	0 / 4651	-28.0	-28.0	0.33 (1)		10.00
O-AA	0 / 14080	-28.0	-28.0	0.74 (1)		10.00
AA-AB	0 / 14080	-28.0	-28.0	0.74 (1)		10.00
AB- N	0 / 14080	-28.0	-28.0	0.74 (1)		10.00
N-AC	0 / 13945	-28.0	-28.0	0.78 (1)		10.00
AC-AD	0 / 13945	-28.0	-28.0	0.78 (1)		10.00
AD-AE	0 / 13945	-28.0	-28.0	0.78 (1)		10.00
AE-AF	0 / 13945	-28.0	-28.0	0.78 (1)		10.00
AF- M	0 / 13945	-28.0	-28.0	0.78 (1)		10.00
M- L	0 / 15381	-28.0	-28.0	0.85 (1)		10.00
L-AG	0 / 15381	-28.0	-28.0	0.85 (1)		10.00
AG-AH	0 / 15381	-28.0	-28.0	0.85 (1)		10.00
AH- K	0 / 15381	-28.0	-28.0	0.85 (1)		10.00
K-AI	0 / 4352	-28.0	-28.0	0.47 (1)		10.00
AI-AJ	0 / 4352	-28.0	-28.0	0.47 (1)		10.00
AJ-AK	0 / 4352	-28.0	-28.0	0.47 (1)		10.00
AK- J	0 / 4352	-28.0	-28.0	0.47 (1)		10.00
J- I	0 / 0	-28.0	-28.0	0.29 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI
FR-TO		
P- B	-2834 / 0	0.28 (1)
B- O	0 / 7647	0.95 (1)
O- C	0 / 278	0.03 (1)
J- G	-2927 / 0	0.27 (1)
A- P	0 / 8642	0.82 (1)
J- H	0 / 7463	0.92 (1)
N- D	0 / 2664	0.24 (1)
O- D	-5863 / 0	0.87 (1)
D- M	0 / 1668	0.23 (1)
M- E	0 / 1587	0.20 (1)
E- K	-4077 / 0	0.65 (1)
K- F	-116 / 14	0.01 (1)
K- G	0 / 9647	0.85 (1)

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 707 (0.33")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.85/1.00 (K-M:1) , WB=0.95/1.00 (B-O:1) , SSI=0.73/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.97 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7320 -B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T81S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWWW-t	MT20	8.0	12.0	3.75	5.75
P	BMWW+t	MT20	5.0	8.0	3.75	1.50
Q	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.2 lbs FACTORED DOWN AND 138.4 lbs FACTORED UP AT 1-9-0, 1.2 lbs FACTORED DOWN AND 138.4 lbs FACTORED UP AT 3-9-12, 1.2 lbs FACTORED DOWN AND 138.4 lbs FACTORED UP AT 5-9-12, 1.2 lbs FACTORED DOWN AND 138.4 lbs FACTORED UP AT 7-9-12, 1.0 lbs FACTORED DOWN AND 157.4 lbs FACTORED UP AT 9-9-12, 1.0 lbs FACTORED DOWN AND 157.4 lbs FACTORED UP AT 11-9-12, 1.0 lbs FACTORED DOWN AND 157.4 lbs FACTORED UP AT 13-9-12, AND 1.0 lbs FACTORED DOWN AND 157.4 lbs FACTORED UP AT 15-9-12, AND 1.0 lbs FACTORED DOWN AND 127.9 lbs FACTORED UP AT 17-9-12 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 11.5 lbs FACTORED UP AT 1-9-12, 1581.0 lbs FACTORED DOWN AT 2-4-12, 1.0 lbs FACTORED DOWN AND 11.5 lbs FACTORED UP AT 3-9-12, 1581.0 lbs FACTORED DOWN AT 4-4-12, 1.0 lbs FACTORED DOWN AND 11.5 lbs FACTORED UP AT 5-9-12, 1437.0 lbs FACTORED DOWN AT 6-4-12, 1.0 lbs FACTORED DOWN AND 11.5 lbs FACTORED UP AT 7-9-12, 1581.0 lbs FACTORED DOWN AT 8-4-12, 1.0 lbs FACTORED DOWN AND 13.3 lbs FACTORED UP AT 9-9-12, 1581.0 lbs FACTORED DOWN AT 10-4-12, 1.0 lbs FACTORED DOWN AND 13.3 lbs FACTORED UP AT 11-9-12, 1581.0 lbs FACTORED DOWN AT 12-4-12, 1.0 lbs FACTORED DOWN AND 13.3 lbs FACTORED UP AT 13-9-12, 1581.0 lbs FACTORED DOWN AT 14-4-12, 1.0 lbs FACTORED DOWN AND 13.3 lbs FACTORED UP AT 15-9-12, 1581.0 lbs FACTORED DOWN AT 16-4-12, AND 1437.0 lbs FACTORED DOWN AT 17-4-12, AND 1.0 lbs FACTORED DOWN AND 13.3 lbs FACTORED UP AT 17-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	1-9-0	-0	-1	138	FRONT	VERT	TOTAL
D	7-9-12	-0	-1	138	FRONT	VERT	TOTAL
J	17-9-12	11	1	13	FRONT	VERT	TOTAL
K	14-4-12	-1581	-1581	—	BACK	VERT	TOTAL
L	11-9-12	11	1	13	FRONT	VERT	TOTAL
O	4-4-12	-1581	-1581	—	BACK	VERT	TOTAL
P	1-9-12	7	1	11	FRONT	VERT	TOTAL
R	3-9-12	-0	-1	138	FRONT	VERT	TOTAL
S	5-9-12	-0	-1	138	FRONT	VERT	TOTAL
T	9-9-12	99	-1	157	FRONT	VERT	TOTAL
U	11-9-12	99	-1	157	FRONT	VERT	TOTAL
V	13-9-12	99	-1	157	FRONT	VERT	TOTAL
W	15-9-12	99	-1	157	FRONT	VERT	TOTAL
X	17-9-12	69	-1	128	FRONT	VERT	TOTAL
Y	2-4-12	-1581	-1581	—	BACK	VERT	TOTAL
Z	3-9-12	7	1	11	FRONT	VERT	TOTAL
AA	5-9-12	7	1	11	FRONT	VERT	TOTAL
AB	6-4-12	-1437	-1437	—	BACK	VERT	TOTAL
AC	7-9-12	7	1	11	FRONT	VERT	TOTAL
AD	8-4-12	-1581	-1581	—	BACK	VERT	TOTAL
AE	9-9-12	11	1	13	FRONT	VERT	TOTAL
AF	10-4-12	-1581	-1581	—	BACK	VERT	TOTAL
AG	12-4-12	-1581	-1581	—	BACK	VERT	TOTAL
AH	13-9-12	11	1	13	FRONT	VERT	TOTAL
AI	15-9-12	11	1	13	FRONT	VERT	TOTAL
AJ	16-4-12	-1581	-1581	—	BACK	VERT	TOTAL
AK	17-4-12	-1437	-1437	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
2/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7320-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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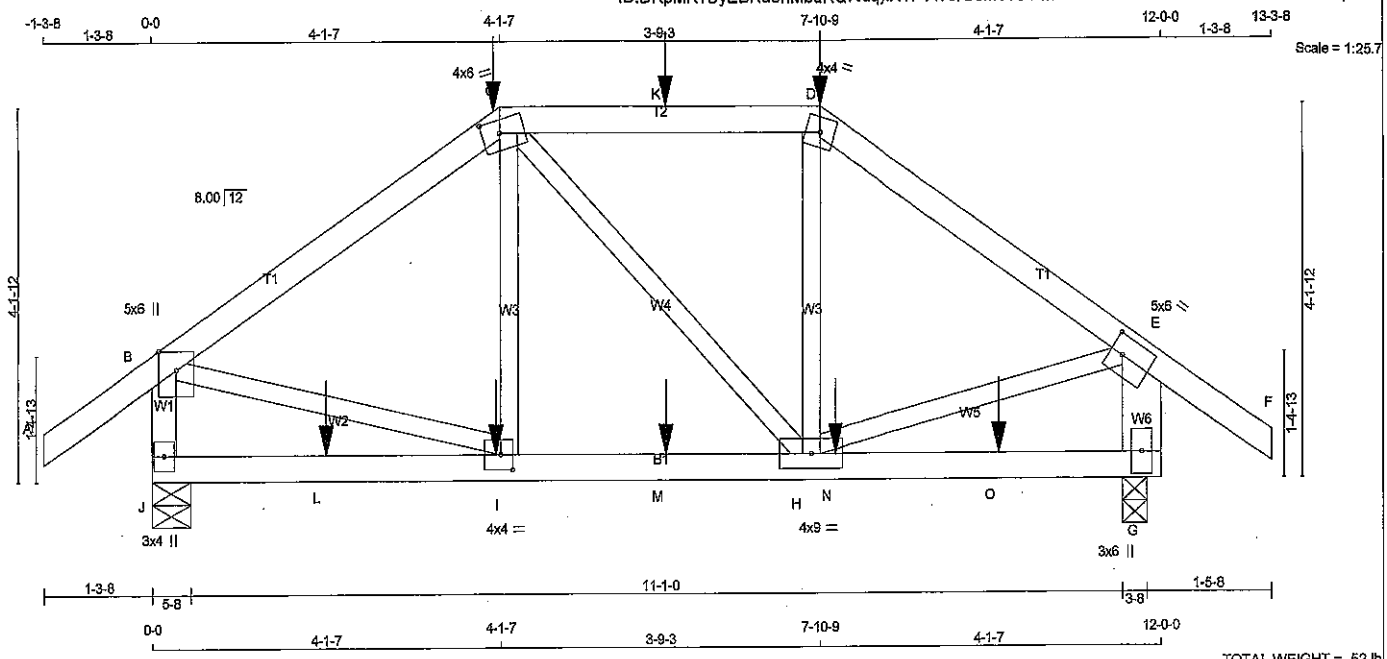

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	5.0	6.0	2.50	1.75
G	BMW1+w	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 133.1 lbs FACTORED DOWN AT 4-1-6, AND 129.6 lbs FACTORED DOWN AT 6-0-12, AND 132.6 lbs FACTORED DOWN AT 7-10-10 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 2-0-12, 71.5 lbs FACTORED DOWN AT 4-0-12, 71.5 lbs FACTORED DOWN AT 6-0-12, AND 71.5 lbs FACTORED DOWN AT 8-0-12, AND 71.5 lbs FACTORED DOWN AT 10-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1218	0	1218	0	0	5-8	5-8
G	1258	0	1258	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	975	589 / 0	197 / 0	0 / 0	0 / 0	189 / 0	0 / 0
G	1007	609 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00
B- C	-1029 / 0	-104.9	-104.9	0.37 (1)	5.65
C- K	-838 / 0	-104.9	-104.9	0.46 (1)	5.85
K- D	-838 / 0	-104.9	-104.9	0.46 (1)	5.85
D- E	-1010 / 0	-104.9	-104.9	0.33 (1)	5.76
E- F	0 / 47	-104.9	-104.9	0.21 (1)	10.00
J- B	-1152 / 0	0.0	0.0	0.13 (1)	7.39
J- L	0 / 0	-28.0	-28.0	0.20 (2)	10.00
L- I	0 / 0	-28.0	-28.0	0.20 (2)	10.00
I- M	0 / 854	-28.0	-28.0	0.29 (2)	10.00
M- H	0 / 854	-28.0	-28.0	0.29 (2)	10.00
H- N	0 / 0	-28.0	-28.0	0.20 (2)	10.00
N- O	0 / 0	-28.0	-28.0	0.20 (2)	10.00
O- G	0 / 0	-28.0	-28.0	0.20 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-18 / 244	0.06 (3)
C- H	-25 / 0	0.01 (1)
H- D	-38 / 232	0.06 (3)
B- I	0 / 882	0.22 (1)
G- E	-1189 / 0	0.08 (1)
H- E	0 / 877	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-6	-133	-133	---	FRONT	VERT	TOTAL
D	7-10-10	-133	-133	---	FRONT	VERT	TOTAL
I	4-0-12	-41	-71	---	FRONT	VERT	TOTAL
K	6-0-12	-130	-130	---	FRONT	VERT	TOTAL
L	2-0-12	-41	-71	---	FRONT	VERT	TOTAL
M	6-0-12	-41	-71	---	FRONT	VERT	TOTAL
N	8-0-12	-41	-71	---	FRONT	VERT	TOTAL
O	10-0-12	-41	-71	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.29/1.00 (H-I:2) , WB=0.22/1.00 (B-I:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

DWG NO. TAM 7321-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T83	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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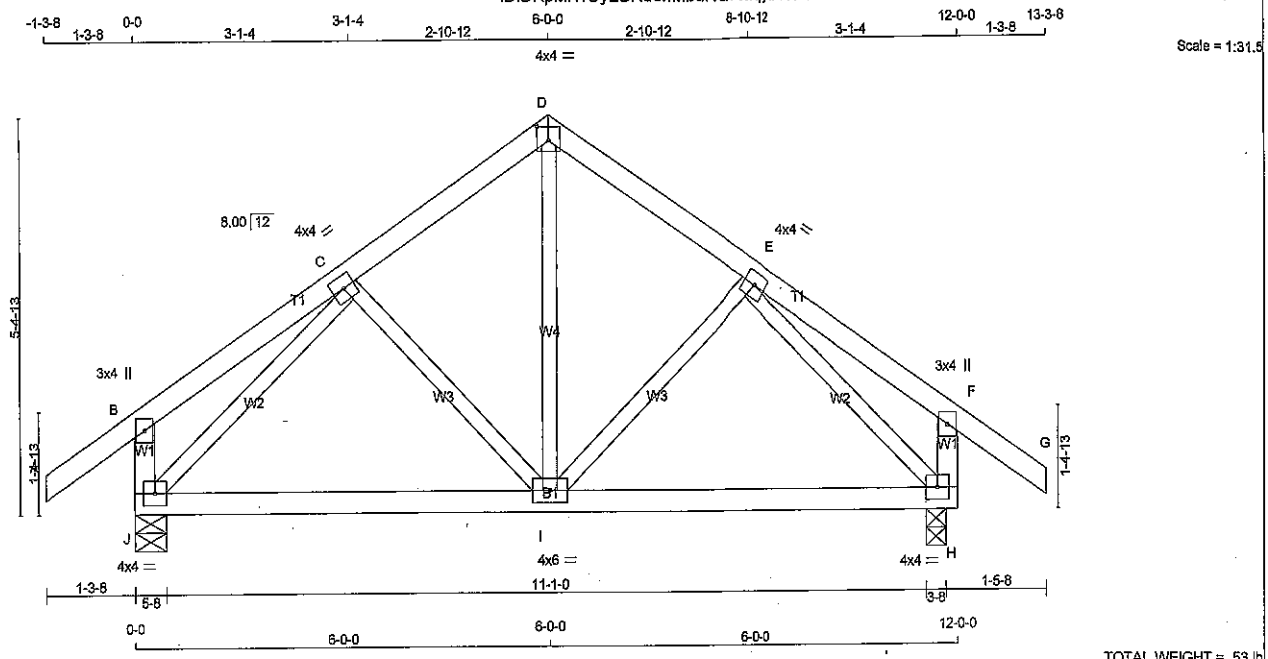

TOTAL WEIGHT = 53 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	942	0	942	0	0	5-8	5-8
H	942	0	942	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	733	479 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	733	479 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 19	-104.9	-104.9	0.14 (1)	10.00	
C- D	-613 / 0	-104.9	-104.9	0.11 (1)	6.25	
D- E	-613 / 0	-104.9	-104.9	0.11 (1)	6.25	
E- F	0 / 19	-104.9	-104.9	0.14 (1)	10.00	
F- G	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
J- B	-267 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-267 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 594	-28.0	-28.0	0.35 (2)	10.00	
I- H	0 / 594	-28.0	-28.0	0.35 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 411	0.09 (1)
I- E	-143 / 44	0.04 (1)
C- I	-143 / 44	0.04 (1)
J- C	-852 / 0	0.25 (1)
E- H	-852 / 0	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.14/1.00 (E-F:1) , BC=0.35/1.00 (H-I:2) , WB=0.25/1.00 (E-H:1) , SSI=0.14/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg,

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7322-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T83C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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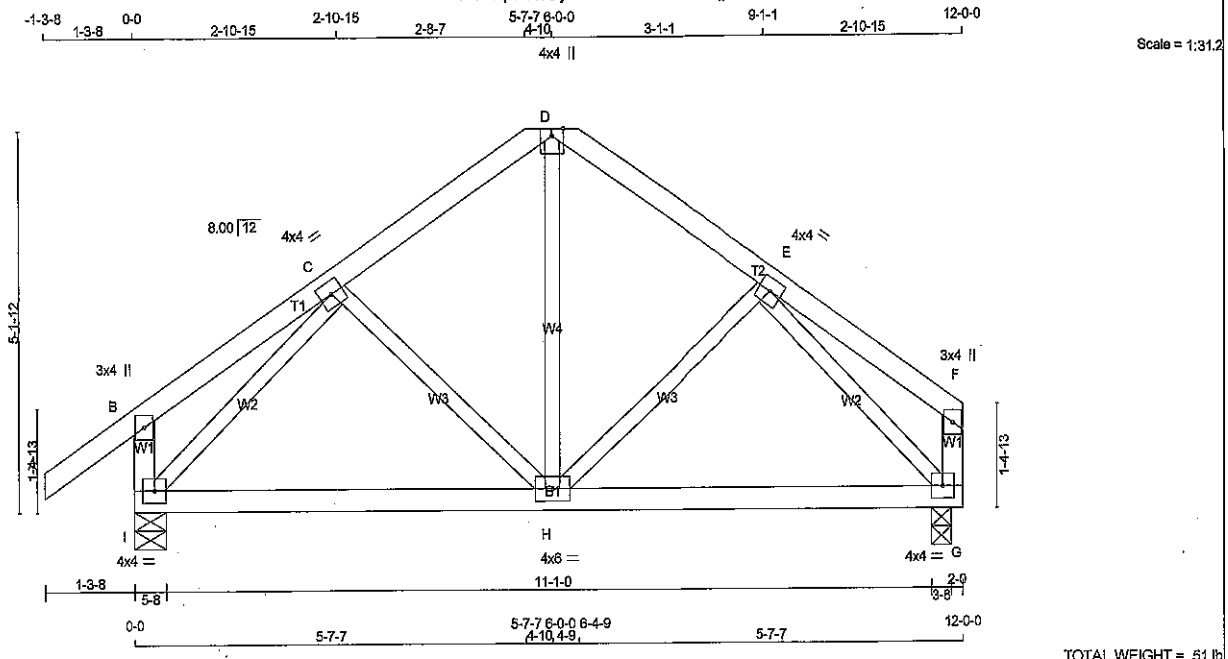

TOTAL WEIGHT = 51 lb

[MiF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMTMW+p	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	942	0	942	0	0	5-8	5-8
G	798	0	798	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	733	479 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	636	390 / 0	126 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00		H- D	0 / 388	0.09 (1)
B- C	0 / 21	-104.9	-104.9	0.15 (1)	10.00		C- H	-134 / 48	0.04 (1)
C- D	-612 / 0	-104.9	-104.9	0.12 (1)	6.25		H- E	-134 / 48	0.04 (1)
D- E	-612 / 0	-104.9	-104.9	0.12 (1)	6.25		I- C	-860 / 0	0.23 (1)
E- F	0 / 21	-104.9	-104.9	0.15 (1)	10.00		E- G	-860 / 0	0.23 (1)
I- B	-254 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	-110 / 0	0.0	0.0	0.01 (1)	7.81				
I- H	0 / 592	-28.0	-28.0	0.35 (2)	10.00				
H- G	0 / 592	-28.0	-28.0	0.35 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.35/1.00 (H-I:2) , WB=0.23/1.00 (C-I:1) , SSI=0.14/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7323-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T84	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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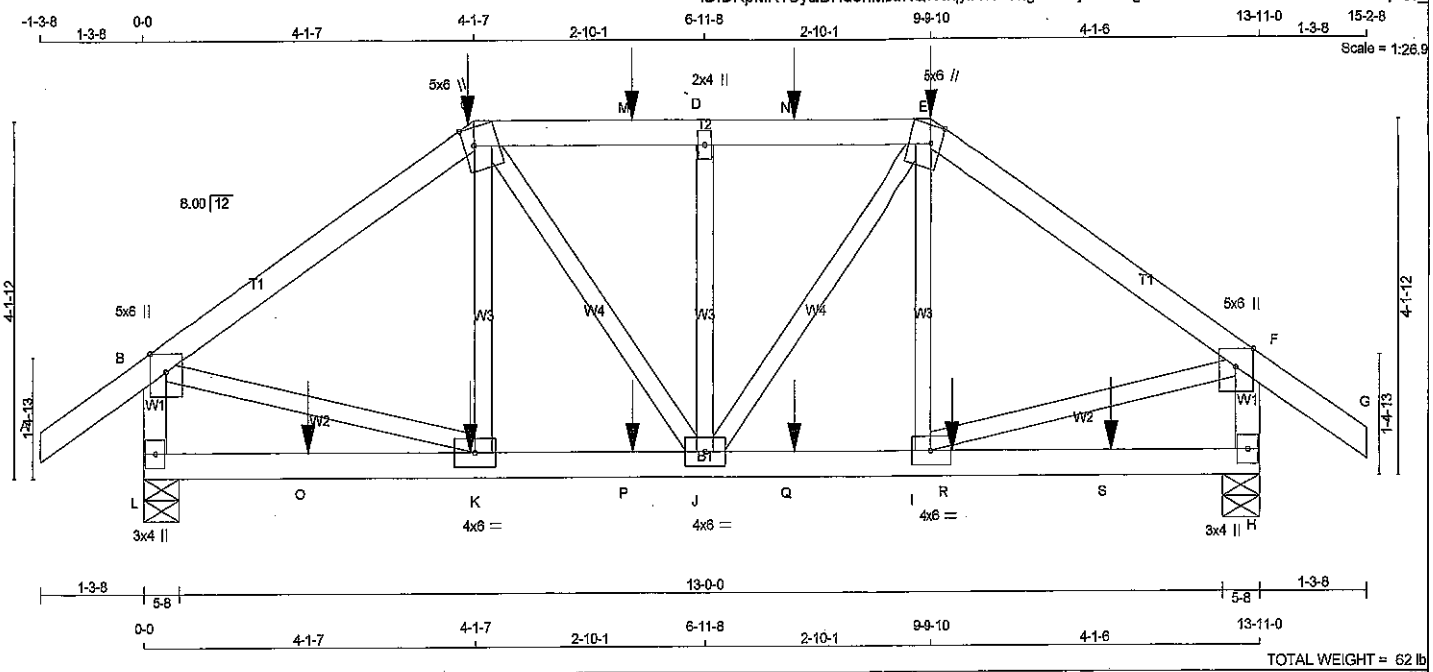


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 133.1 lbs FACTORED DOWN AT 4-1-6, 129.6 lbs FACTORED DOWN AT 6-0-12, AND 129.6 lbs FACTORED DOWN AT 8-0-12, AND 132.3 lbs FACTORED DOWN AT 9-9-9 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 2-0-12, 71.5 lbs FACTORED DOWN AT 4-0-12, 71.5 lbs FACTORED DOWN AT 6-0-12, 71.5 lbs FACTORED DOWN AT 8-0-12, AND 71.5 lbs FACTORED DOWN AT 10-0-12, AND 71.5 lbs FACTORED DOWN AT 12-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1450	0	1450	0	0	5-8	5-8
H	1458	0	1458	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1162	700 / 0	236 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	1170	703 / 0	240 / 0	0 / 0	0 / 0	227 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)		10.00
B- C	-1332 / 0	-104.9	-104.9	0.38 (1)		5.10
C- M	-1341 / 0	-104.9	-104.9	0.25 (1)		5.26
M- D	-1341 / 0	-104.9	-104.9	0.25 (1)		5.26
D- N	-1341 / 0	-104.9	-104.9	0.25 (1)		5.26
N- E	-1341 / 0	-104.9	-104.9	0.25 (1)		5.26
E- F	-1336 / 0	-104.9	-104.9	0.38 (1)		5.09
F- G	0 / 40	-104.9	-104.9	0.15 (1)		10.00
L- B	-1385 / 0	0.0	0.0	0.16 (1)		6.87
H- F	-1389 / 0	0.0	0.0	0.16 (1)		6.87
L- O	0 / 0	-28.0	-28.0	0.19 (2)		10.00
O- K	0 / 0	-28.0	-28.0	0.19 (2)		10.00
K- P	0 / 1105	-28.0	-28.0	0.32 (2)		10.00
P- J	0 / 1105	-28.0	-28.0	0.32 (2)		10.00
J- Q	0 / 1110	-28.0	-28.0	0.34 (2)		10.00
Q- I	0 / 1110	-28.0	-28.0	0.34 (2)		10.00
I- R	0 / 0	-28.0	-28.0	0.20 (2)		10.00
R- S	0 / 0	-28.0	-28.0	0.20 (2)		10.00
S- H	0 / 0	-28.0	-28.0	0.20 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-71 / 227	0.06 (3)
C- J	0 / 398	0.10 (1)
J- D	-558 / 0	0.15 (1)
J- E	0 / 390	0.10 (1)
I- E	-70 / 231	0.06 (3)
B- K	0 / 1142	0.28 (1)
I- F	0 / 1147	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-6	-133	-133	---	FRONT	VERT	TOTAL
E	9-9-9	-132	-132	---	FRONT	VERT	TOTAL
K	4-0-12	-41	-71	---	FRONT	VERT	TOTAL
M	6-0-12	-130	-130	---	FRONT	VERT	TOTAL
N	8-0-12	-130	-130	---	FRONT	VERT	TOTAL
O	2-0-12	-41	-71	---	FRONT	VERT	TOTAL
P	6-0-12	-41	-71	---	FRONT	VERT	TOTAL
Q	8-0-12	-41	-71	---	FRONT	VERT	TOTAL
R	10-0-12	-41	-71	---	FRONT	VERT	TOTAL
S	12-0-12	-41	-71	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.38/1.00 (E-F:1) , BC=0.34/1.00 (I-J:2) , WB=0.28/1.00 (F-I:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

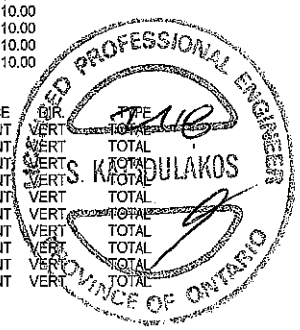
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.28 (K) (INPUT = 1.00)



DWG NO. TAM7324-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T85A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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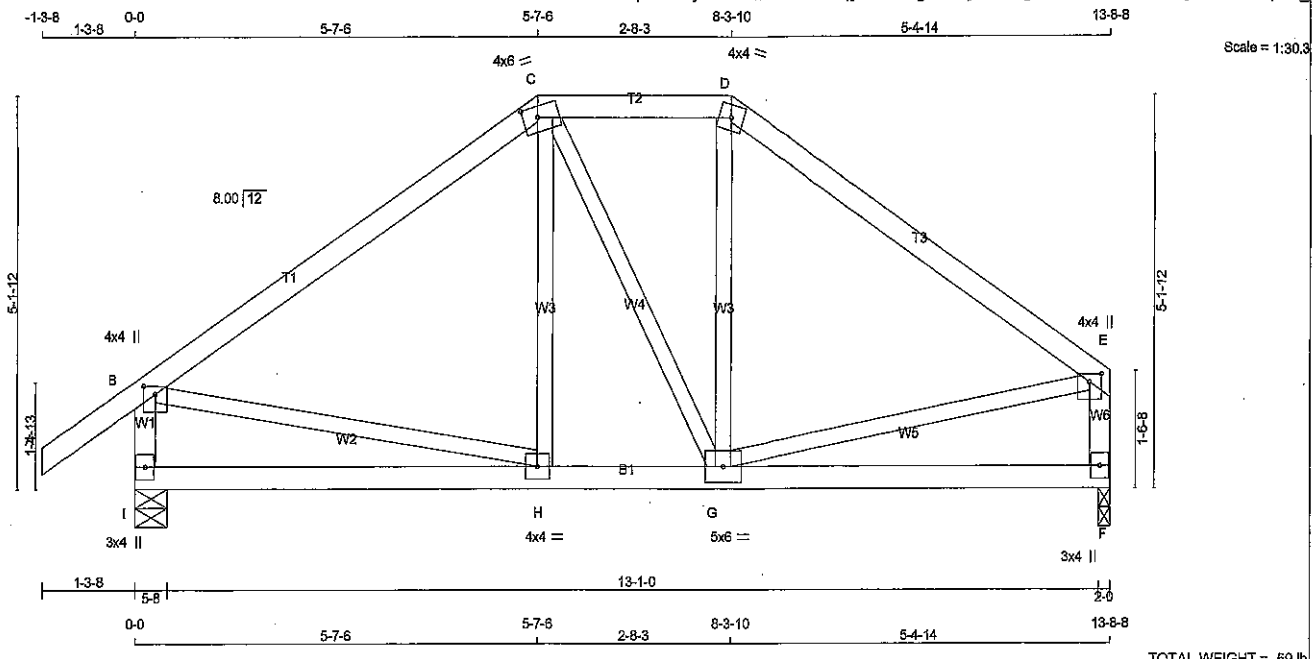

TOTAL WEIGHT = 59 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1055	0	1055	0	0	5-8	5-8
F	911	0	911	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	824	534 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0
F	728	445 / 0	144 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	H- C	0 / 170	0.04 (3)
B- C	-769 / 0	-104.9	-104.9	0.43 (1)	6.23	C- G	-17 / 0	0.01 (1)
C- D	-631 / 0	-104.9	-104.9	0.10 (1)	6.25	G- D	-2 / 160	0.04 (3)
D- E	-760 / 0	-104.9	-104.9	0.40 (1)	6.25	B- H	0 / 650	0.15 (1)
I- B	-987 / 0	0.0	0.0	0.10 (1)	7.81	G- E	0 / 646	0.15 (1)
F- E	-845 / 0	0.0	0.0	0.09 (1)	7.81			
I- H	0 / 0	-28.0	-28.0	0.19 (2)	10.00			
H- G	0 / 839	-28.0	-28.0	0.24 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.18 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.24/1.00 (G-H:2) , WB=0.15/1.00 (B-H:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.24 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7325-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T86A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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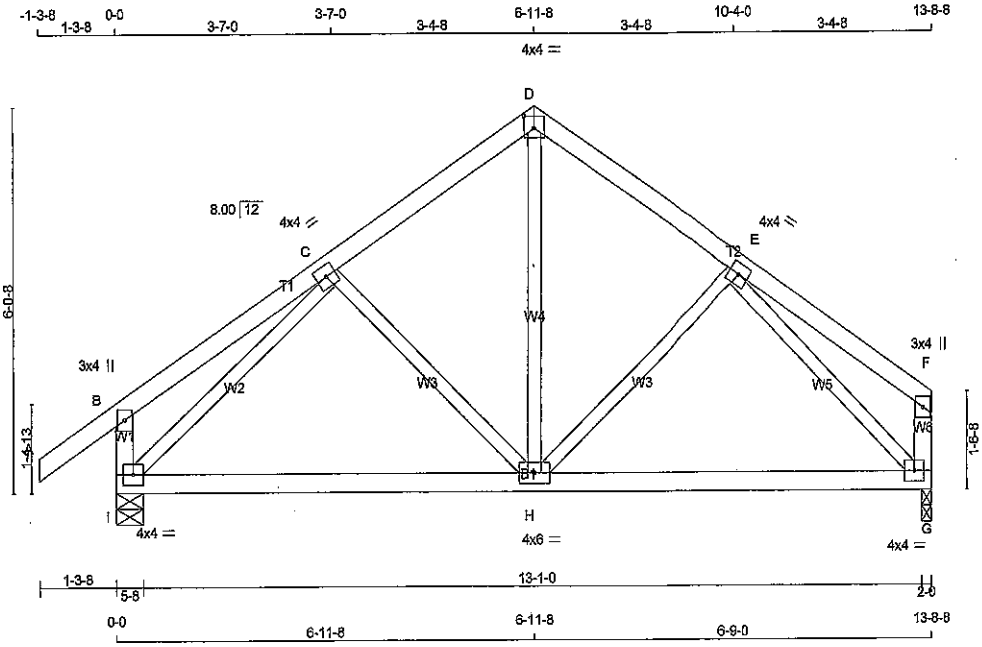

TOTAL WEIGHT = 4 X 58 = 232 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1055	0	1055	0	0	5-8	5-8
G	911	0	911	0	0	2-0	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	824	534 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	726	445 / 0	144 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- H	-192 / 42	0.08 (1)	
B- C	0 / 23	-104.9	-104.9	0.20 (1)	10.00	H- D	0 / 477	0.11 (1)	
C- D	-711 / 0	-104.9	-104.9	0.16 (1)	6.25	H- E	-152 / 54	0.08 (1)	
D- E	-710 / 0	-104.9	-104.9	0.15 (1)	6.25	I- C	-994 / 0	0.37 (1)	
E- F	0 / 23	-104.9	-104.9	0.19 (1)	10.00	E- G	-984 / 0	0.35 (1)	
I- B	-285 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	-130 / 0	0.0	0.0	0.01 (1)	7.81				
I- H	0 / 711	-28.0	-28.0	0.45 (2)	10.00				
H- G	0 / 682	-28.0	-28.0	0.45 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.45/1.00 (H-I:2) , WB=0.37/1.00 (C-I:1) , SSI=0.16/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 732698
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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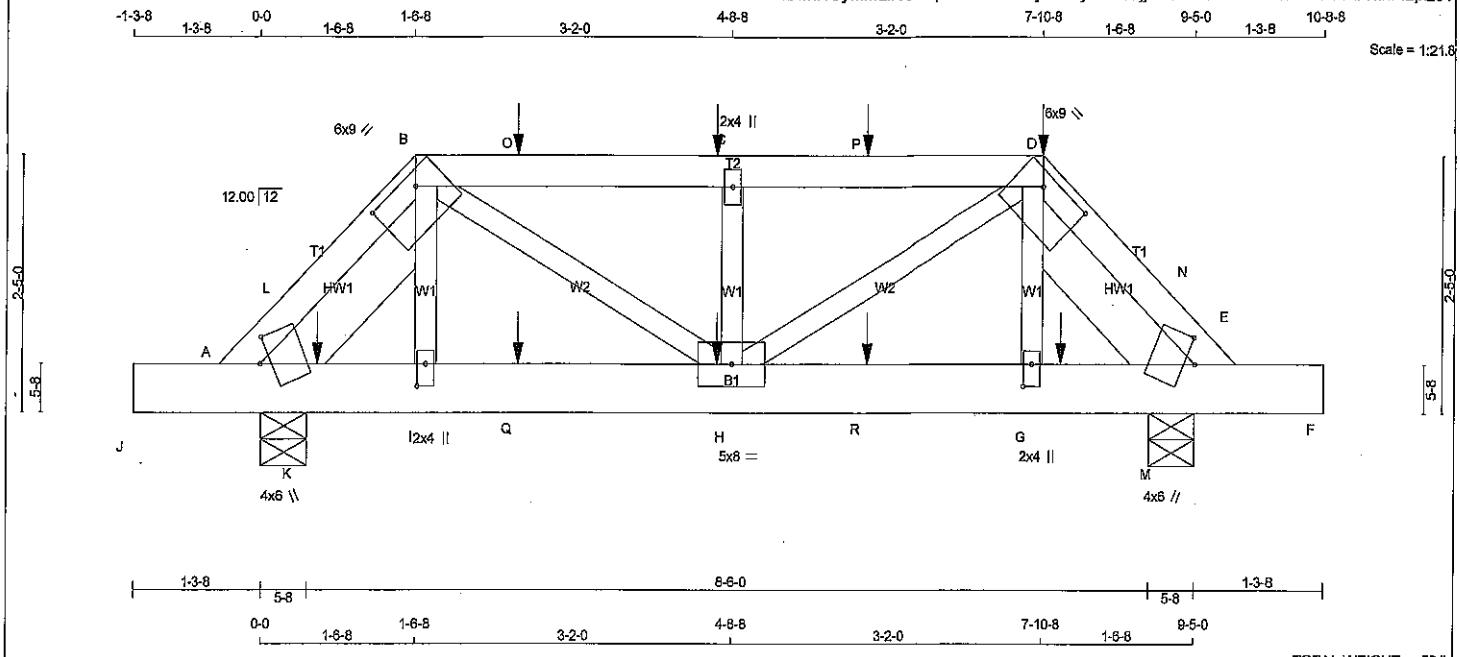

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBMW1+m	MT20	4.0	6.0	2.75	1.25
B	TTWWW-h	MT20	6.0	9.0	1.50	5.75
C	TMW+w	MT20	2.0	4.0		
D	TTWWW-h	MT20	6.0	9.0	1.50	5.75
E	TBMW1+m	MT20	4.0	6.0	2.75	1.25
G	BMW+w	MT20	2.0	4.0	2.50	1.00
H	BMWWW-t	MT20	5.0	8.0		
I	BMW+w	MT20	2.0	4.0	2.50	1.00

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 38.3 lbs FACTORED DOWN AT 2-6-12, 38.3 lbs FACTORED DOWN AT 4-6-12, AND 38.3 lbs FACTORED DOWN AT 6-0-12, AND 41.4 lbs FACTORED DOWN AT 7-10-8 ON TOP CHORD, AND 252.8 lbs FACTORED DOWN AT 6-12, 252.8 lbs FACTORED DOWN AT 2-6-12, 252.8 lbs FACTORED DOWN AT 4-6-12, AND 252.8 lbs FACTORED DOWN AT 6-0-12, AND 252.8 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K(A)	1556	0	1556	0	0	5-8	5-8
M(E)	1459	0	1459	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K(A)	1240	761 / 0	245 / 0	0 / 0	0 / 0	234 / 0	0 / 0
M(E)	1163	712 / 0	230 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K(A), M(E)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-23 / 0	-104.9	-104.9	0.06 (1)		6.25
L- B	-79 / 0	-104.9	-104.9	0.06 (1)		6.25
B- O	-1362 / 0	-104.9	-104.9	0.20 (1)		5.30
O- C	-1362 / 0	-104.9	-104.9	0.20 (1)		5.30
C- P	-1362 / 0	-104.9	-104.9	0.20 (1)		5.30
P- D	-1362 / 0	-104.9	-104.9	0.20 (1)		5.30
D- N	-124 / 0	-104.9	-104.9	0.06 (1)		6.25
N- E	-66 / 0	-104.9	-104.9	0.06 (1)		6.25
J- A	0 / 0	-132.9	-132.9	0.04 (1)		10.00
A- K	0 / 39	-28.0	-28.0	0.10 (1)		10.00
K- I	0 / 567	-28.0	-28.0	0.17 (1)		10.00
I- Q	0 / 580	-28.0	-28.0	0.16 (1)		10.00
Q- H	0 / 580	-28.0	-28.0	0.16 (1)		10.00
H- R	0 / 668	-28.0	-28.0	0.23 (1)		10.00
R- G	0 / 668	-28.0	-28.0	0.23 (1)		10.00
G- M	0 / 647	-28.0	-28.0	0.20 (1)		10.00
M- E	0 / 70	-28.0	-28.0	0.12 (1)		10.00
E- F	0 / 0	-132.9	-132.9	0.04 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 258	0.06 (1)
B- H	0 / 928	0.23 (1)
H- C	-453 / 0	0.08 (1)
H- D	0 / 824	0.20 (1)
G- D	0 / 400	0.10 (1)
K- L	-182 / 0	0.00 (1)
K- B	-1053 / 0	0.09 (1)
D- M	-1153 / 0	0.10 (1)
M- N	-184 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-6-12	-38	-38	---	BACK	VERT	TOTAL
D	7-10-8	-41	-41	---	BACK	VERT	TOTAL
G	8-0-12	-253	-253	---	BACK	VERT	TOTAL
H	4-6-12	-253	-253	---	BACK	VERT	TOTAL
K	6-12	-253	-253	---	BACK	VERT	TOTAL
O	2-6-12	-38	-38	---	BACK	VERT	TOTAL
P	8-0-12	-38	-38	---	BACK	VERT	TOTAL
Q	2-6-12	-253	-253	---	BACK	VERT	TOTAL
R	6-0-12	-253	-253	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.23/1.00 (G-H:1) , WB=0.23/1.00 (B-H:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.39 (D) (INPUT = 1.00)

PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7282-19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	T91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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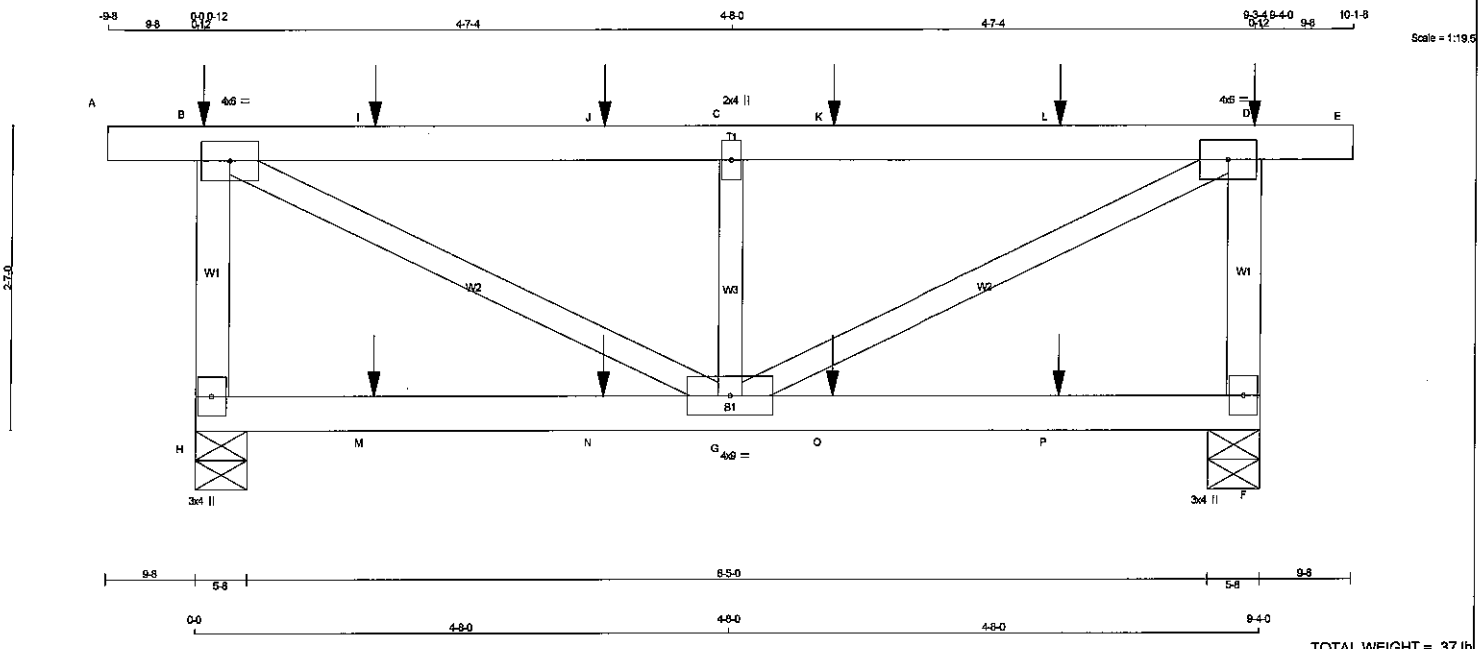

TOTAL WEIGHT = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
C	TMW+w	MT20	2.0	4.0	
D	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWWW-t	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 117.5 lbs FACTORED DOWN AT 0-12, 109.9 lbs FACTORED DOWN AT 1-6-12, 109.9 lbs FACTORED DOWN AT 3-6-12, 109.9 lbs FACTORED DOWN AT 5-6-12, AND 109.9 lbs FACTORED DOWN AT 7-6-12, AND 117.5 lbs FACTORED DOWN AT 9-3-4 ON TOP CHORD, AND 58.1 lbs FACTORED DOWN AT 1-6-12, 58.1 lbs FACTORED DOWN AT 3-6-12, AND 58.1 lbs FACTORED DOWN AT 5-6-12, AND 58.1 lbs FACTORED DOWN AT 7-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1113	0	1113	0	0	5-8	5-8
F	1101	0	1101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	867	566 / 0	149 / 0	0 / 0	0 / 0	152 / 0	0 / 0
F	857	560 / 0	147 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-1037 / 0	0.0	0.0	0.14 (1)	7.68	B- G	0 / 1315	0.33 (1)
A- B	0 / 0	-104.9	-104.9	0.05 (1)	10.00	G- C	-915 / 0	0.16 (1)
B- I	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72	G- D	0 / 1315	0.33 (1)
I- J	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72			
J- C	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72			
C- K	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72			
K- L	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72			
L- D	-1180 / 0	-104.9	-104.9	0.66 (1)	4.72			
D- E	0 / 0	-104.9	-104.9	0.05 (1)	10.00			
F- D	-1028 / 0	0.0	0.0	0.14 (1)	7.71			
H- M	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
M- N	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
N- G	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
G- O	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
O- P	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
P- F	0 / 0	-28.0	-28.0	0.30 (3)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	0-12	-79	-79	—	BACK	VERT	TOTAL
D	9-3-4	-79	-79	—	BACK	VERT	TOTAL
I	1-6-12	-74	-74	—	BACK	VERT	TOTAL
J	3-6-12	-74	-74	—	BACK	VERT	TOTAL
K	5-6-12	-74	-74	—	BACK	VERT	TOTAL
L	7-6-12	-74	-74	—	BACK	VERT	TOTAL
M	1-6-12	-42	-42	—	BACK	VERT	TOTAL
N	3-6-12	-42	-42	—	BACK	VERT	TOTAL
O	5-6-12	-42	-42	—	BACK	VERT	TOTAL
P	7-6-12	-42	-42	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.66/1.00 (C-D:1) , BC=0.30/1.00 (G-H:3) , WB=0.33/1.00 (B-G:1) , SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	P20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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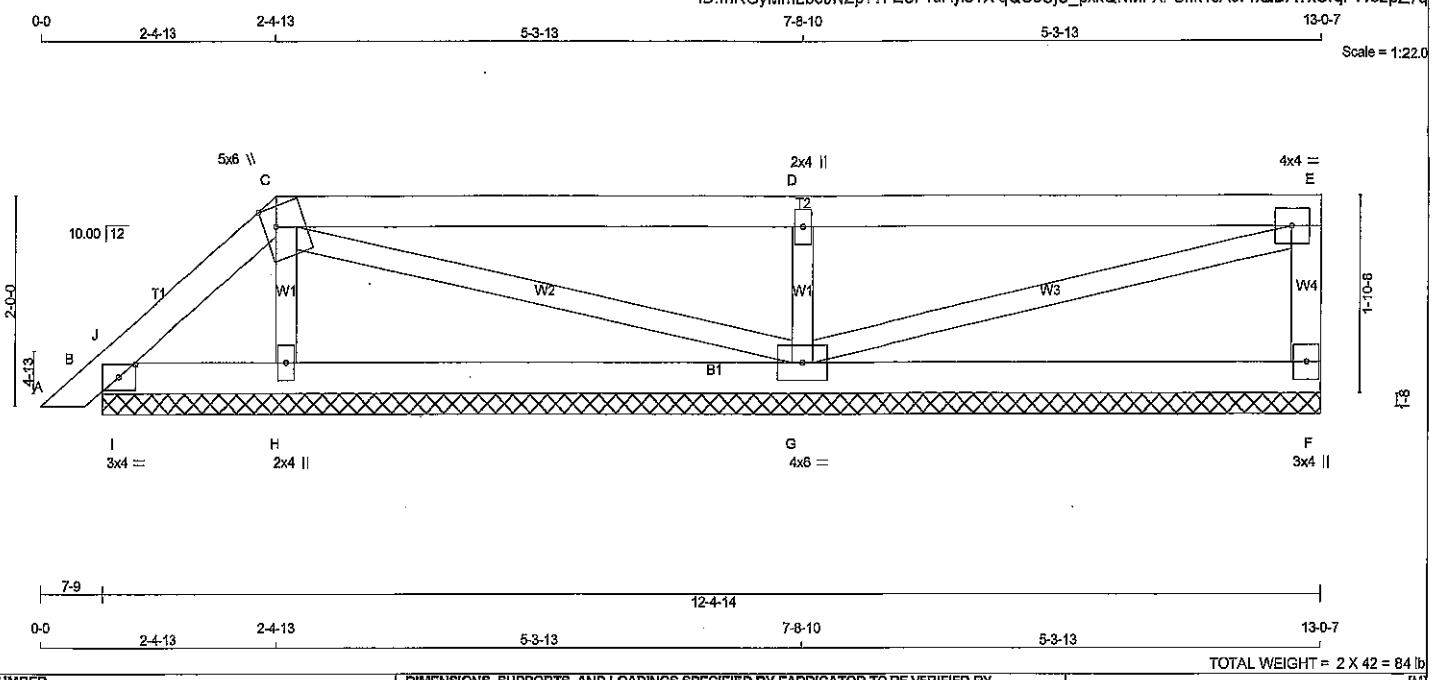

TOTAL WEIGHT = 2 X 42 = 84 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	268	0	268	0	0	12-4-14	12-4-14
B	191	0	191	0	0	12-4-14	12-4-14
H	369	0	369	0	0	12-4-14	12-4-14
G	877	0	877	0	0	12-4-14	12-4-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	214	130 / 0	44 / 0	0 / 0	0 / 0	41 / 0	0 / 0
B	131	117 / 0	2 / 0	0 / 0	0 / 0	12 / 0	0 / 0
H	316	158 / 0	86 / 0	0 / 0	0 / 0	72 / 0	0 / 0
G	692	436 / 0	129 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-28 / 23	-104.9	-104.9	0.02 (1)	6.25
J- C	-64 / 0	-104.9	-104.9	0.03 (1)	6.25
C- D	0 / 0	-104.9	-104.9	0.50 (1)	10.00
D- E	0 / 0	-104.9	-104.9	0.50 (1)	10.00
F- E	-209 / 0	0.0	0.0	0.02 (1)	7.81
B- I	0 / 45	-28.0	-28.0	0.05 (1)	10.00
I- H	0 / 45	-28.0	-28.0	0.10 (2)	10.00
H- G	0 / 29	-28.0	-28.0	0.21 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-235 / 0	0.03 (1)
C- G	-30 / 0	0.02 (1)
G- D	-697 / 0	0.10 (1)
G- E	0 / 0	0.00 (1)
I- J	-143 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50/1.00 (C-D:1) , BC=0.21/1.00 (G-H:2) , WB=0.10/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5267-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	P21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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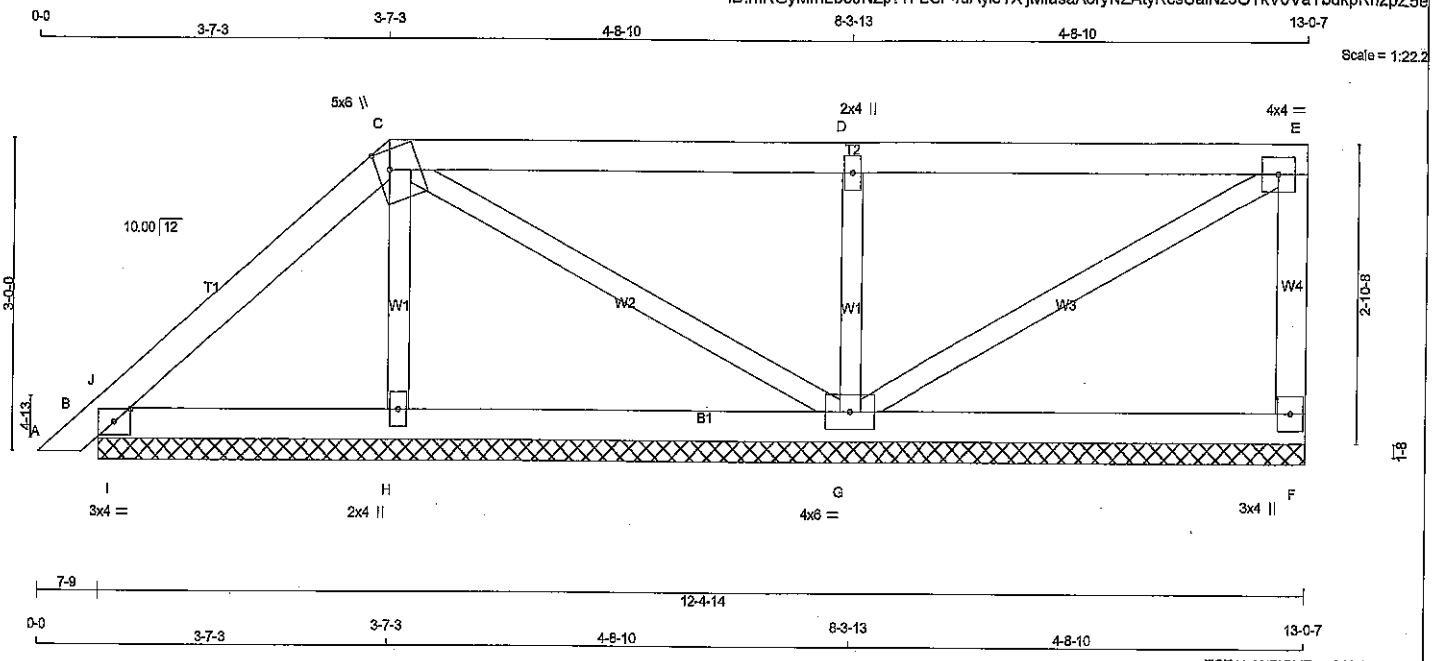

TOTAL WEIGHT = 2 X 45 = 89 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	238	0	238	0	0	12-4-14	12-4-14
B	303	0	303	0	0	12-4-14	12-4-14
H	363	0	363	0	0	12-4-14	12-4-14
G	801	0	801	0	0	12-4-14	12-4-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	191	115 / 0	39 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	224	167 / 0	25 / 0	0 / 0	0 / 0	32 / 0	0 / 0
H	308	157 / 0	82 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	629	402 / 0	114 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-43 / 0	-104.9	-104.9	0.04 (1)	6.25
J- C	-104 / 0	-104.9	-104.9	0.11 (1)	6.25
C- D	-2 / 1	-104.9	-104.9	0.40 (1)	10.00
D- E	-2 / 1	-104.9	-104.9	0.40 (1)	10.00
F- E	-185 / 0	0.0	0.0	0.03 (1)	7.81
B- I	0 / 74	-28.0	-28.0	0.11 (1)	10.00
I- H	0 / 74	-28.0	-28.0	0.11 (1)	10.00
H- G	0 / 65	-28.0	-28.0	0.16 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-219 / 0	0.04 (1)
C- G	-76 / 0	0.04 (1)
G- D	-618 / 0	0.11 (1)
G- E	-1 / 2	0.00 (1)
I- J	-282 / 32	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1) , BC=0.16/1.00 (G-H:2) , WB=0.11/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 7286-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286835	P21Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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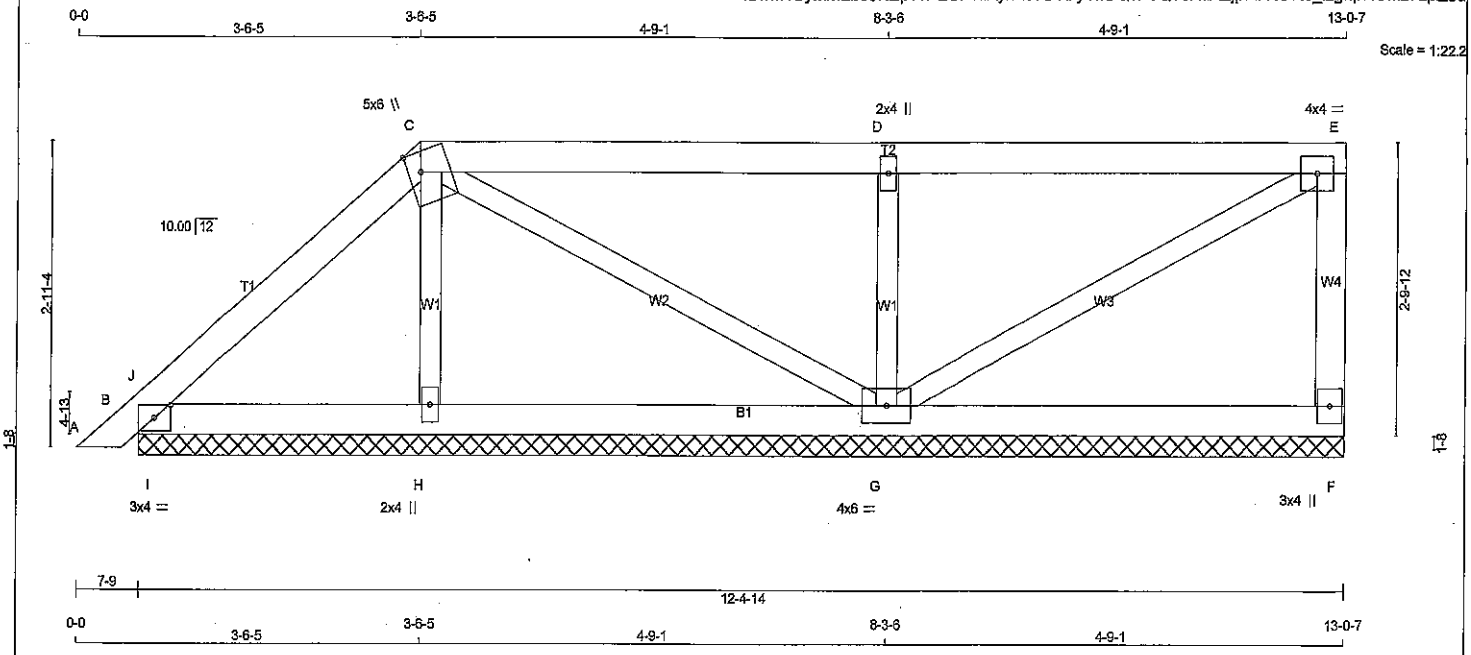

TOTAL WEIGHT = 2 X 44 = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- F	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	240	0	240	0	0	12-4-14	12-4-14
B	296	0	296	0	0	12-4-14	12-4-14
H	364	0	364	0	0	12-4-14	12-4-14
G	806	0	806	0	0	12-4-14	12-4-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	192	116 / 0	39 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	219	164 / 0	24 / 0	0 / 0	0 / 0	31 / 0	0 / 0
H	308	157 / 0	82 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	633	404 / 0	115 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-104.9	-104.9	0.01 (1)		10.00	H- C	-221 / 0	0.02 (1)
B- J	-36 / 0	-104.9	-104.9	0.02 (1)		6.25	C- G	-73 / 0	0.02 (1)
J- C	-102 / 0	-104.9	-104.9	0.05 (1)		6.25	G- D	-623 / 0	0.05 (1)
C- D	-1 / 1	-104.9	-104.9	0.20 (1)		10.00	G- E	-1 / 2	0.00 (1)
D- E	-1 / 1	-104.9	-104.9	0.20 (1)		10.00	I- J	-266 / 27	0.00 (1)
F- E	-187 / 0	0.0	0.0	0.01 (1)		7.81			
B- I	0 / 72	-28.0	-28.0	0.05 (1)		10.00			
I- H	0 / 72	-28.0	-28.0	0.05 (1)		10.00			
H- G	0 / 63	-28.0	-28.0	0.08 (2)		10.00			
G- F	0 / 0	-28.0	-28.0	0.08 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1) , BC=0.08/1.00 (G-H:2) , WB=0.05/1.00 (D-G:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

DWG NO. TAM 7289-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	P22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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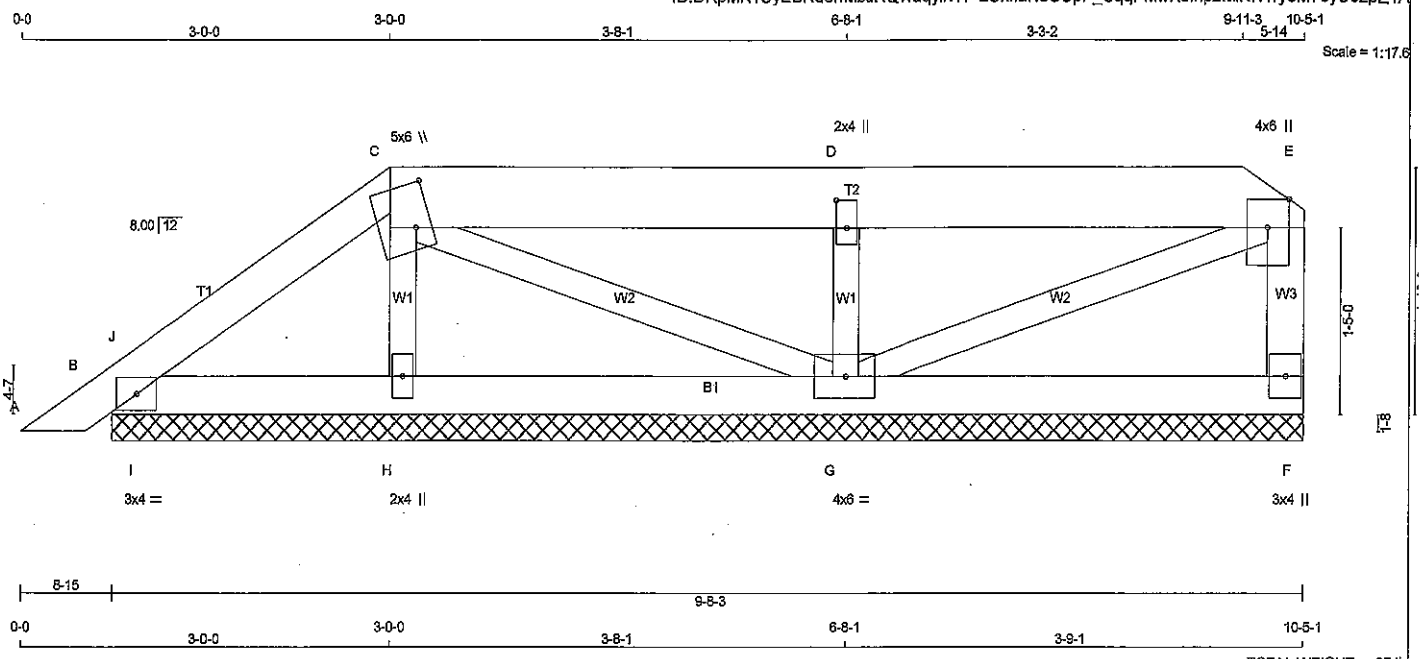

TOTAL WEIGHT = 37 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	4.00	1.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-I	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	191	0	191	0	0	9-8-3	9-8-3
B	241	0	241	0	0	9-8-3	9-8-3
H	308	0	308	0	0	9-8-3	9-8-3
G	616	0	616	0	0	9-8-3	9-8-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	153	92 / 0	31 / 0	0 / 0	0 / 0	29 / 0	0 / 0
B	177	133 / 0	19 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	257	139 / 0	63 / 0	0 / 0	0 / 0	55 / 0	0 / 0
G	485	307 / 0	90 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 18	-104.9	-104.9	0.03 (1)	10.00
B- J	-46 / 0	-104.9	-104.9	0.01 (1)	6.25
J- C	-59 / 0	-104.9	-104.9	0.06 (1)	6.25
C- D	0 / 2	-104.9	-104.9	0.12 (1)	10.00
D- E	0 / 2	-104.9	-104.9	0.12 (1)	10.00
F- E	-149 / 0	0.0	0.0	0.02 (1)	7.81
B- I	0 / 48	-28.0	-28.0	0.06 (1)	10.00
I- H	0 / 48	-28.0	-28.0	0.06 (1)	10.00
H- G	0 / 34	-28.0	-28.0	0.10 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.10 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-197 / 0	0.03 (1)
G- D	-485 / 0	0.07 (1)
C- G	-40 / 0	0.01 (1)
G- E	-3 / 1	0.00 (1)
I- J	-139 / 19	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-E:1) , BC=0.10/1.00 (G-H:2) , WB=0.07/1.00 (D-G:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 7328-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	P23	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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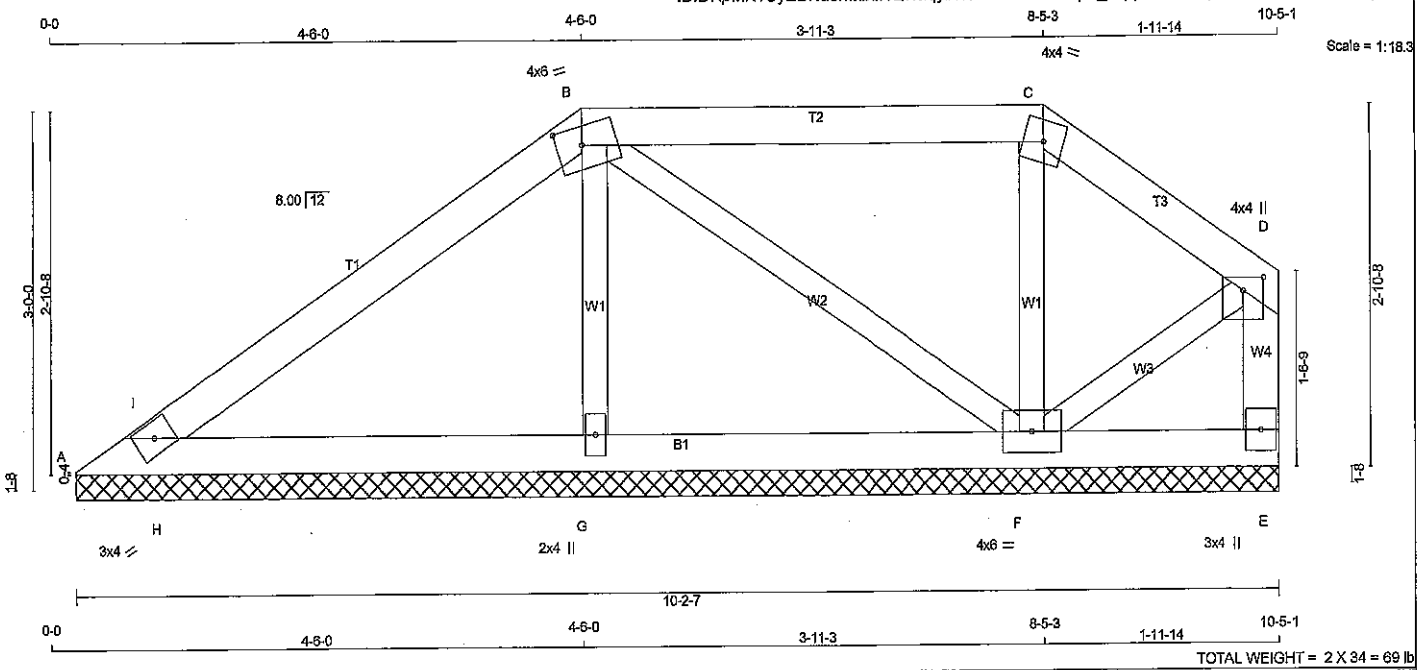

TOTAL WEIGHT = 2 X 34 = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW1-t	MT20	4.0	6.0		
G	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	146	0	146	0	0	10-2-7	10-2-7
E	-12	0	0	0	-16	10-2-7	10-2-7
G	770	0	770	0	0	10-2-7	10-2-7
F	452	0	452	0	0	10-2-7	10-2-7

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	114	74 / 0	19 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	-13	0 / -2	0 / -7	0 / 0	0 / 0	0 / -5	0 / 0
G	618	372 / 0	127 / 0	0 / 0	0 / 0	119 / 0	0 / 0
F	362	218 / 0	74 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC) FR-TO
A- I	0 / 229		-104.9	-104.9	0.06 (1)	10.00	G- B	-581 / 0	0.10 (1)
I- B	0 / 237		-104.9	-104.9	0.24 (1)	10.00	B- F	0 / 113	0.03 (1)
B- C	0 / 129		-104.9	-104.9	0.30 (1)	10.00	F- C	-387 / 0	0.07 (1)
C- D	0 / 136		-104.9	-104.9	0.09 (1)	10.00	F- D	-131 / 0	0.02 (1)
E- D	0 / 52		0.0	0.0	0.01 (2)	10.00	H- I	-211 / 47	0.00 (1)
A- H	-233 / 0		-28.0	-28.0	0.19 (1)	6.25			
H- G	-199 / 0		-28.0	-28.0	0.19 (1)	6.25			
G- F	-222 / 0		-28.0	-28.0	0.13 (2)	6.25			
F- E	0 / 0		-28.0	-28.0	0.05 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (B-C:1) , BC=0.19/1.00 (G-H:1) , WB=0.10/1.00 (B-G:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 7329-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286838	P24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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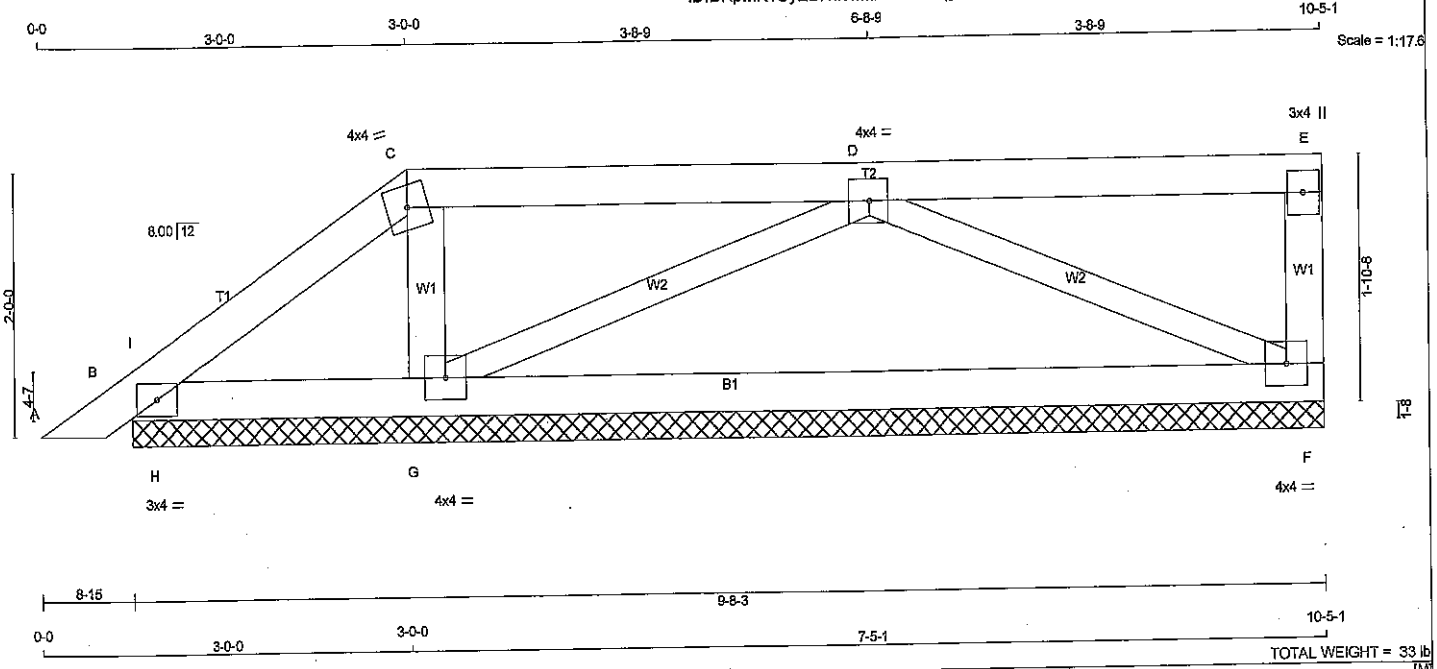

TOTAL WEIGHT = 33 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	436	0	436	0	0	9-8-3	9-8-3
B	90	0	90	0	-51	9-8-3	9-8-3
G	828	0	828	0	0	9-8-3	9-8-3

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	342	222 / 0	60 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	41	77 / 0	0 / -26	0 / 0	0 / 0	0 / -10	0 / 0
G	690	373 / 0	169 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-104.9	-104.9	0.03 (1)	10.00
B- I	0 / 204	-104.9	-104.9	0.10 (2)	10.00
I- C	0 / 110	-104.9	-104.9	0.08 (2)	10.00
C- D	0 / 128	-104.9	-104.9	0.24 (1)	10.00
D- E	0 / 0	-104.9	-104.9	0.22 (1)	10.00
F- E	-151 / 0	0.0	0.0	0.02 (1)	7.81
B- H	-99 / 0	-28.0	-28.0	0.08 (1)	6.25
H- G	-99 / 0	-28.0	-28.0	0.36 (2)	6.25
G- F	0 / 483	-28.0	-28.0	0.39 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-303 / 0	0.03 (1)
G- D	-668 / 0	0.16 (1)
D- F	-526 / 0	0.14 (1)
H- I	-269 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1) , BC=0.39/1.00 (F-G:2) , WB=0.18/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.22 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM7330-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	G14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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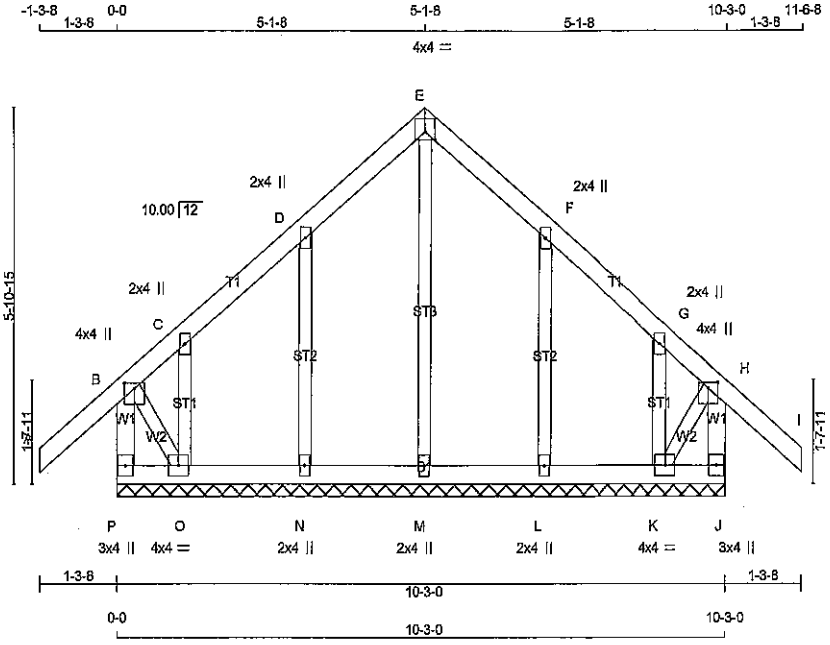

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
P- B	-310 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-71 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	0 / 0	-104.9	-104.9	0.07 (1)	10.00
D- E	-19 / 0	-104.9	-104.9	0.07 (1)	6.25
E- F	-19 / 0	-104.9	-104.9	0.07 (1)	6.25
F- G	0 / 0	-104.9	-104.9	0.07 (1)	10.00
G- H	-71 / 0	-104.9	-104.9	0.14 (1)	6.25
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- H	-310 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.02 (3)	10.00
O- N	0 / 9	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 3	-28.0	-28.0	0.02 (3)	10.00
M- L	0 / 3	-28.0	-28.0	0.02 (3)	10.00
L- K	0 / 9	-28.0	-28.0	0.02 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-167 / 0	0.09 (1)
N- D	-253 / 0	0.07 (1)
O- C	-50 / 0	0.01 (1)
L- F	-253 / 0	0.07 (1)
K- G	-50 / 0	0.01 (1)
B- O	0 / 17	0.00 (1)
K- H	0 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (N-O:2) , WB=0.09/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM 7263-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	G39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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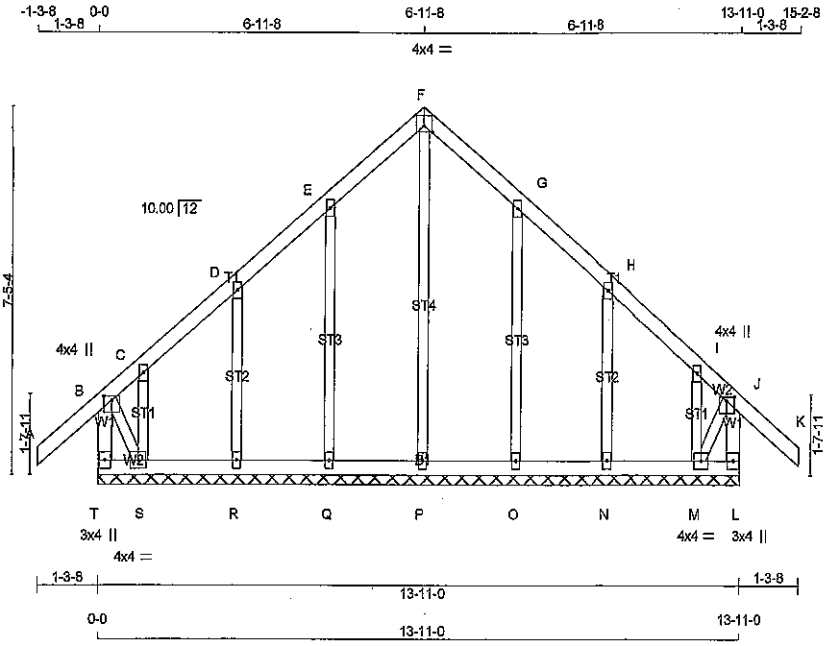

TOTAL WEIGHT = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-326 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-85 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-13 / 0	-104.9	-104.9	0.05 (1)	6.25
D- E	-10 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-21 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-21 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-10 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-13 / 0	-104.9	-104.9	0.05 (1)	6.25
I- J	-85 / 0	-104.9	-104.9	0.14 (1)	6.25
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- J	-326 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 15	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 10	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 6	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 6	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 10	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 15	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-167 / 0	0.16 (1)
Q- E	-236 / 0	0.12 (1)
R- D	-212 / 0	0.05 (1)
S- C	-41 / 0	0.01 (1)
O- G	-236 / 0	0.12 (1)
N- H	-212 / 0	0.05 (1)
M- I	-41 / 0	0.01 (1)
B- S	0 / 29	0.01 (1)
M- J	0 / 29	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (M-N:2) , WB=0.16/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (F) (INPUT = 0.90)
JSI METAL= 0.06 (G) (INPUT = 1.00)

DWG NO. TAM 7264-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	G54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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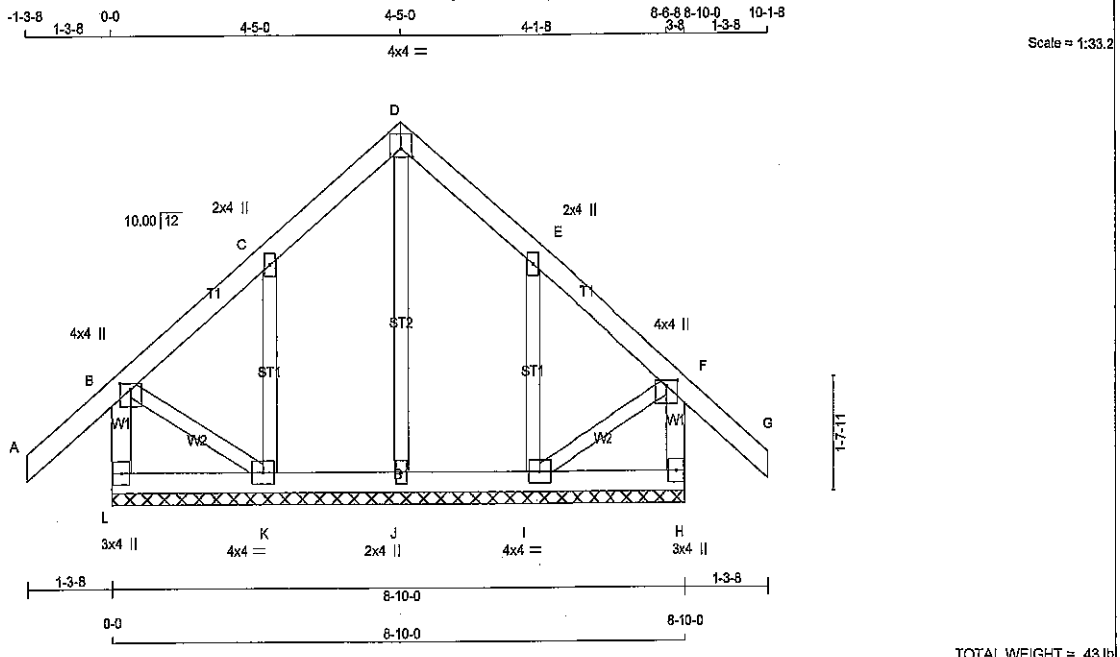

TOTAL WEIGHT = 43 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
L-B	-270 / 0	0.0	0.0	0.03 (1)	7.81	J-D	-137 / 0	0.06 (1)
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K-C	-282 / 0	0.06 (1)
B-C	-11 / 0	-104.9	-104.9	0.08 (1)	6.25	I-E	-282 / 0	0.06 (1)
C-D	-37 / 0	-104.9	-104.9	0.08 (1)	6.25	I-F	0 / 25	0.01 (1)
D-E	-37 / 0	-104.9	-104.9	0.08 (1)	6.25	B-K	0 / 25	0.01 (1)
E-F	-11 / 0	-104.9	-104.9	0.08 (1)	6.25			
F-G	0 / 47	-104.9	-104.9	0.14 (1)	10.00			
H-F	-270 / 0	0.0	0.0	0.03 (1)	7.81			
L-K	0 / 0	-28.0	-28.0	0.04 (3)	10.00			
K-J	0 / 14	-28.0	-28.0	0.04 (2)	10.00			
J-I	0 / 14	-28.0	-28.0	0.04 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.04 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.04/1.00 (J-K:2) , WB=0.06/1.00 (C-K:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

DWG NO. TAM7265-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
286831	G55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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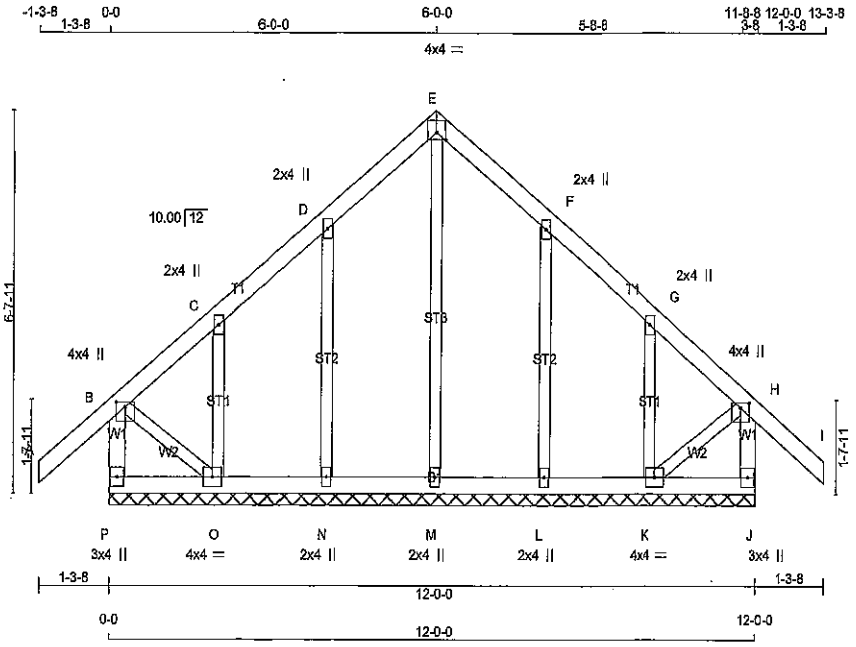


TOTAL WEIGHT = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-323 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-57 / 0	-104.9	-104.9	0.14 (1)		6.25
C- D	-12 / 0	-104.9	-104.9	0.07 (1)		6.25
D- E	-31 / 0	-104.9	-104.9	0.07 (1)		6.25
E- F	-31 / 0	-104.9	-104.9	0.07 (1)		6.25
F- G	-12 / 0	-104.9	-104.9	0.07 (1)		6.25
G- H	-57 / 0	-104.9	-104.9	0.14 (1)		6.25
H- I	0 / 47	-104.9	-104.9	0.14 (1)		10.00
J- H	-323 / 0	0.0	0.0	0.03 (1)		7.81
P- O	0 / 0	-28.0	-28.0	0.03 (2)		10.00
O- N	0 / 17	-28.0	-28.0	0.03 (2)		10.00
N- M	0 / 12	-28.0	-28.0	0.02 (2)		10.00
M- L	0 / 12	-28.0	-28.0	0.02 (2)		10.00
L- K	0 / 17	-28.0	-28.0	0.03 (2)		10.00
K- J	0 / 0	-28.0	-28.0	0.03 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-153 / 0	0.11 (1)
N- D	-251 / 0	0.09 (1)
O- C	-139 / 0	0.03 (1)
L- F	-251 / 0	0.09 (1)
K- G	-139 / 0	0.03 (1)
B- O	0 / 26	0.01 (1)
K- H	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (K-L:2) , WB=0.11/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM7266-18
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

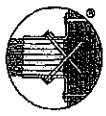
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.

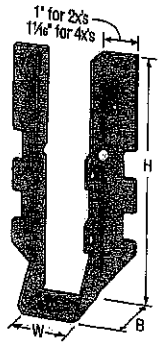


Double-Shear Nailing Top View

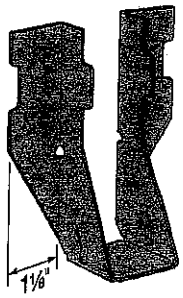
Double-Shear Nailing Side View; Do not bend tab

Dome Double-Shear Nailing Side View *(available on some models)*

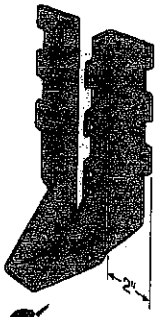
U.S. Patent 5,603,580



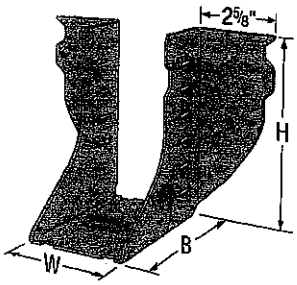
LUS28



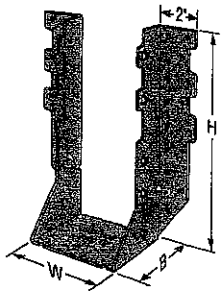
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)

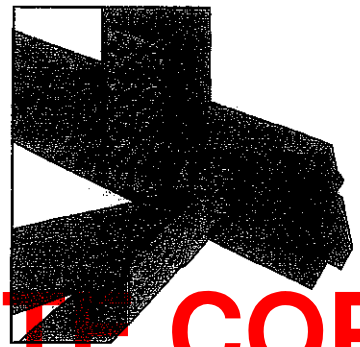


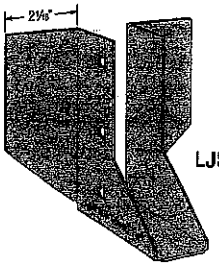
HGUS28-2



HHUS210-2

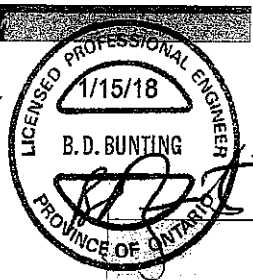
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



SIMPSON Strong-Tie®

HHUS/HGUS

See Hanger Options information on pp. 125–127.

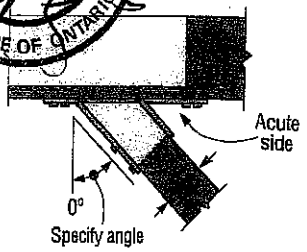
HHUS – Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS – Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

	Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
	Single 2x Sizes											
	LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
	LU24L	22	1⅜	3	1⅝	2¹¹⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
	LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS	LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
	HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
	LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
	HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
	LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS	LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
	HUS28	16	1⅝	7¹⁄₁₆	3	6¹⁄₁₆	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
	HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
	LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS	LUS210	18	1⅜	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

SIMPSON Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3⁹⁄₁₆	7⁹⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⁹⁄₁₆	7¾₁₆	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3⁹⁄₁₆	9⅜₁₆	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3⁹⁄₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6⁹⁄₁₆	7¾₁₆	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6⁹⁄₁₆	9⅜₁₆	4	8⅜	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3⁹⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7¼₁₆	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3⁹⁄₁₆	8¾	2	5¹⁵⁄₁₆	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3⅝	10⅞₁₆	4	10¹⁵⁄₁₆	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80

See footnotes on p. 258.

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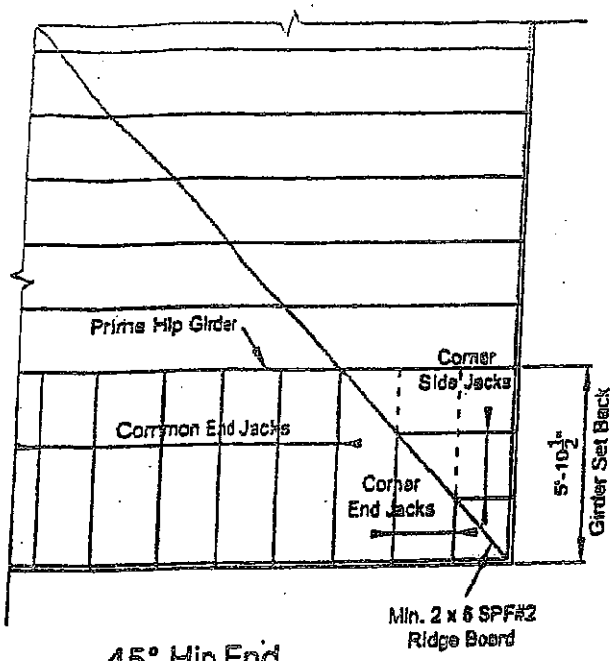

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14R
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
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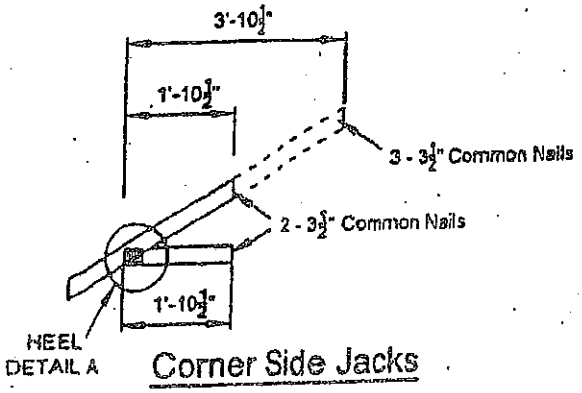
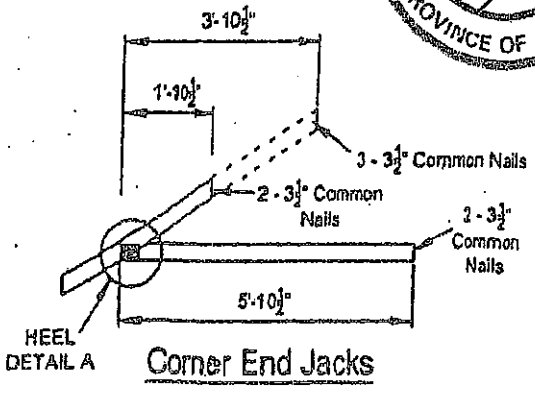
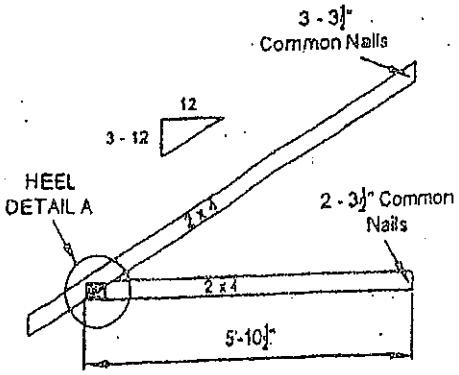


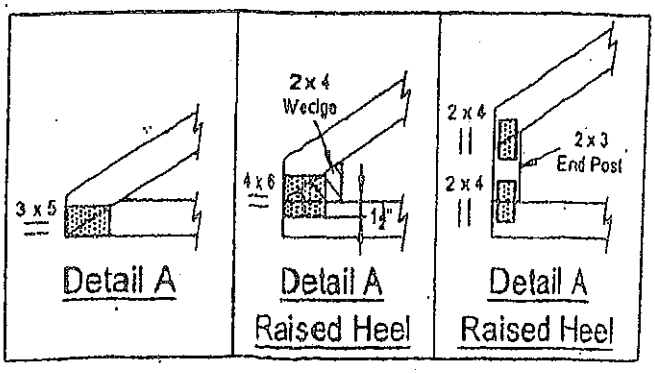
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

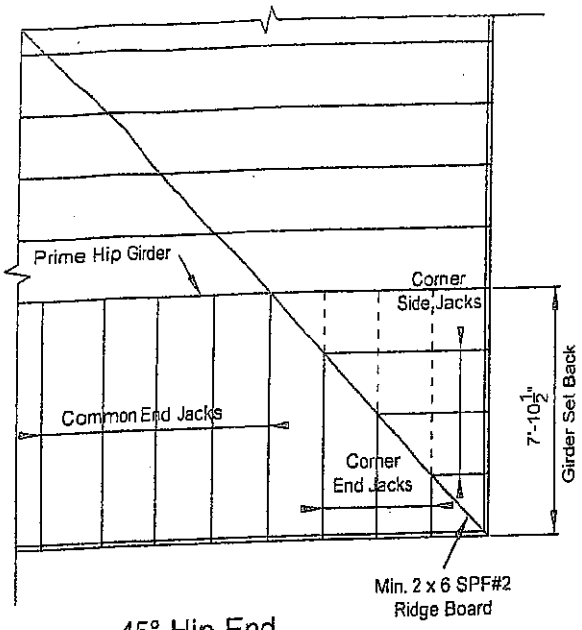
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F.



45° Hip End

DWG NO TAM 3504.1
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
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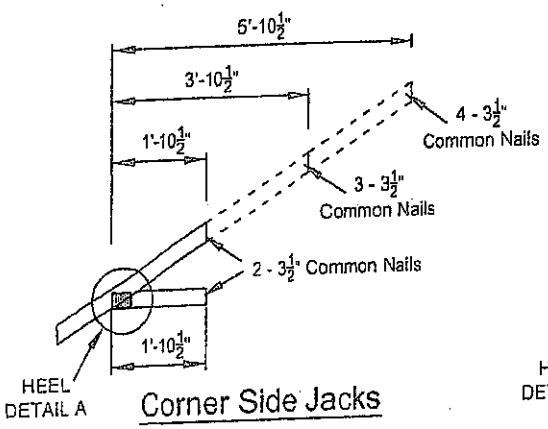
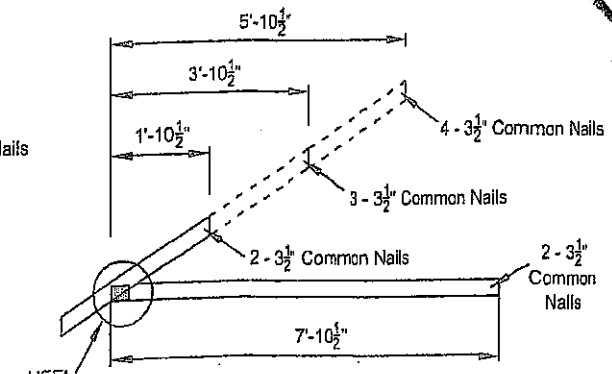
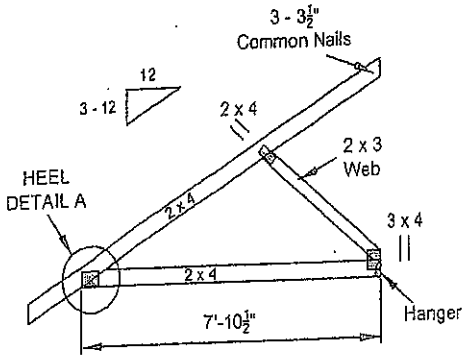


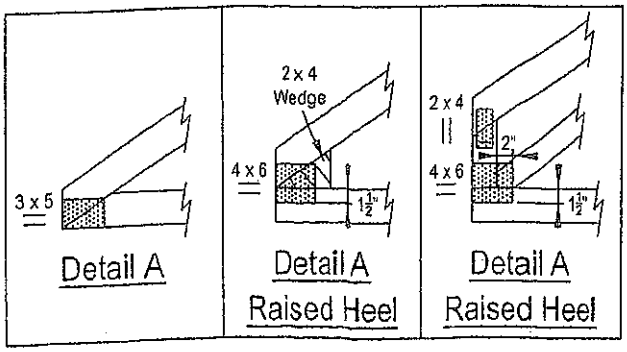
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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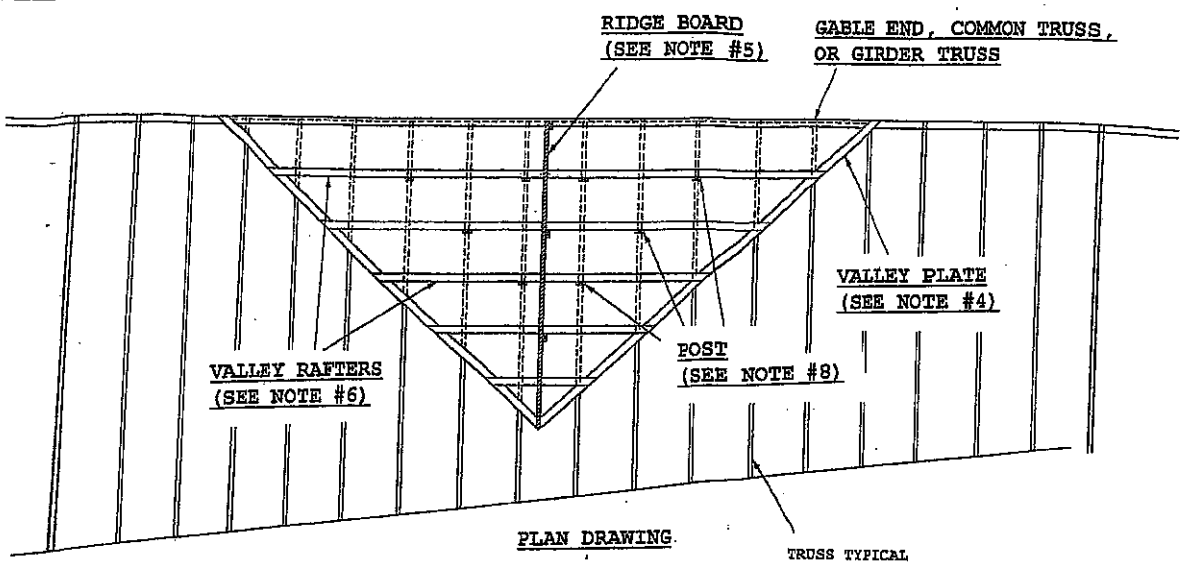
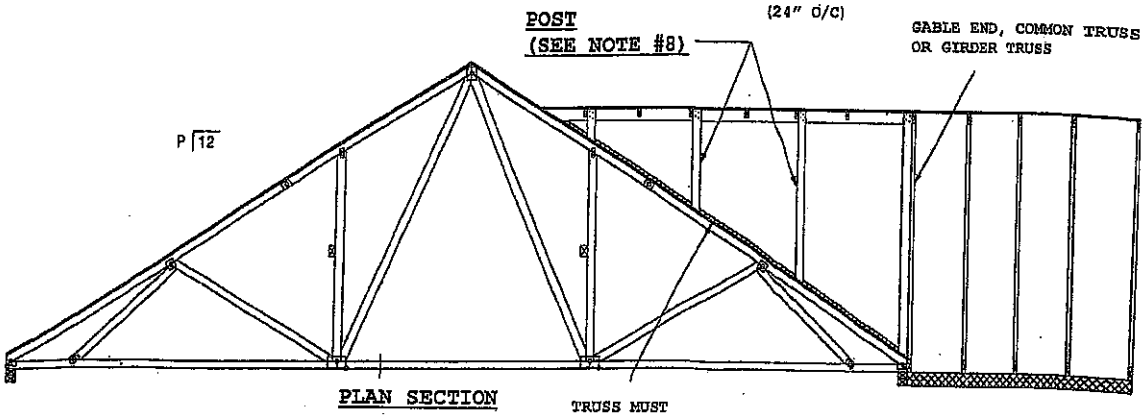
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING

TRUSS TYPICAL
(24" O/C)



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

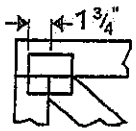
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

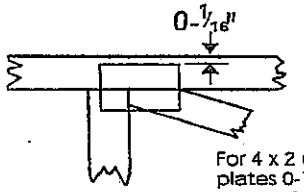
DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

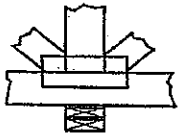
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

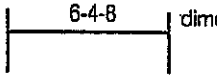
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

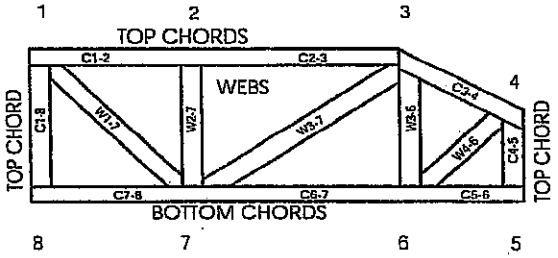
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
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LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO T4794-14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.