

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

NOTE:

H -1'-6"-0 RAISED CEILING

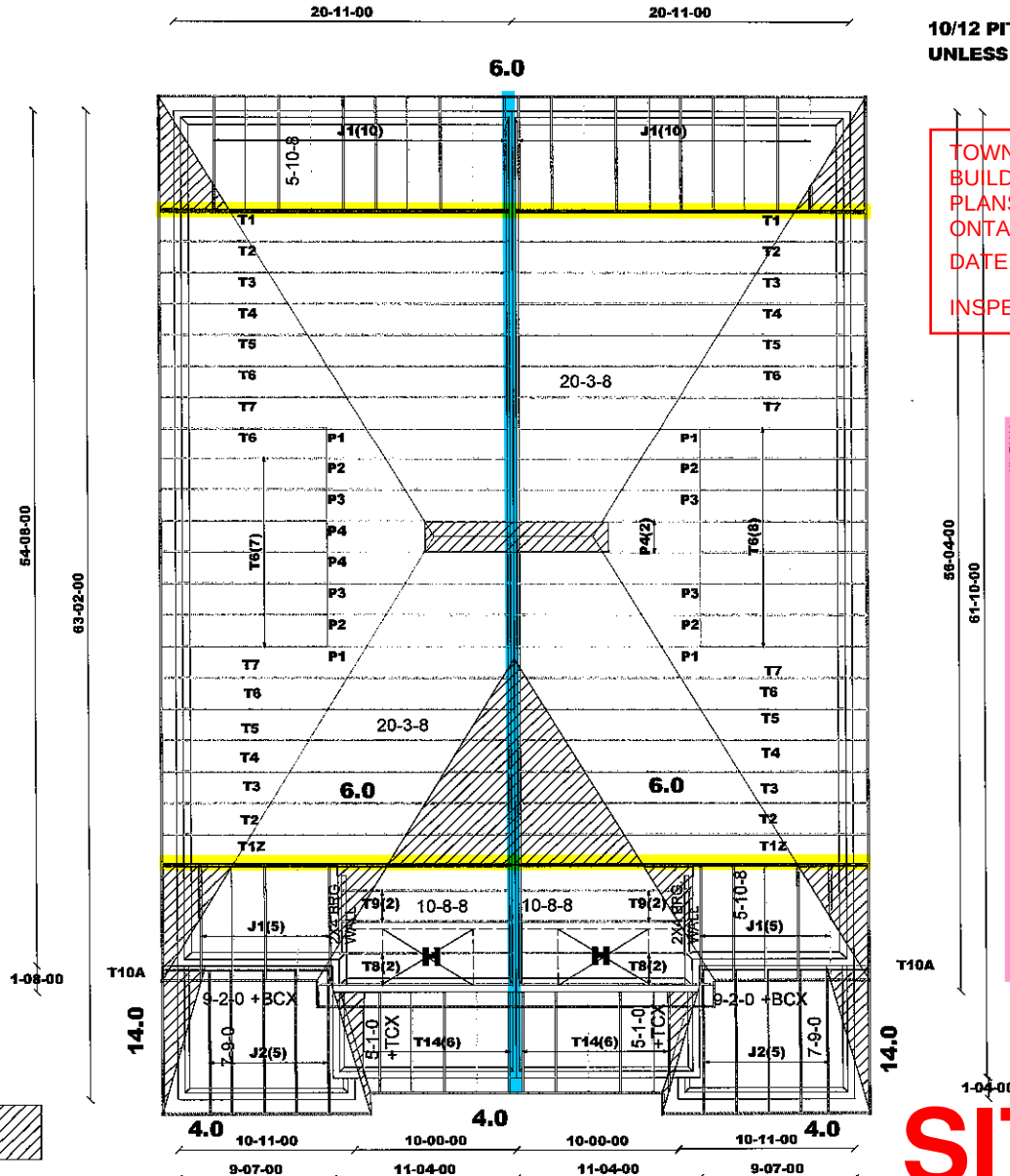
PRELIMINARY
LAYOUT ONLY
T-170587

DENOTES:
CONVENTIONAL
FRAMING



EL:A (STD/OPT)
286695 D

Center line of
wood party wall
EL:A (STD/OPT)(REV)
286695 D



10/12 PITCHES (TYP.)
UNLESS NOTED

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-10-19

INSPECTOR: BG

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBCELEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2X4 SP@24" o.c.
WITH A 2X4 SP@24" VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

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Job Track: 44755

Layout ID: 286703

Plan Log: 93746

Builder / Location:

BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

Date: 05-Feb-18

Designer: sonny

Model / Elevation:

SD25-4 SONOMA4/ EL:A (STD/OPT) PAIRED
WITH EL:A(STD/OPT)(REV.)

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Milek ver 7.5.0

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

EL:B (STD/OPT)
286713 D

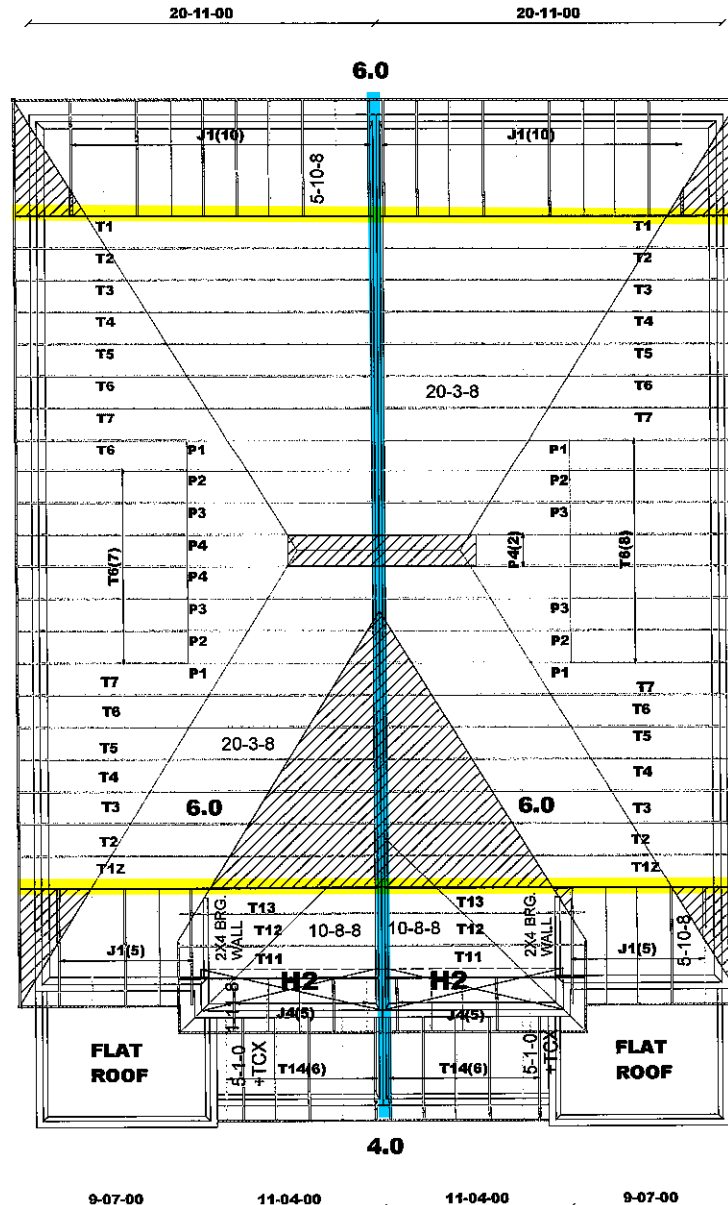
Center line of
wood party wall
EL:B (STD/OPT)(REV)
286713 D

10/12 PITCHES (TYP.)
UNLESS NOTED

NOTE:
H2 -24" RAISED PLATE

PRELIMINARY
LAYOUT ONLY
T-170587

DENOTES:
CONVENTIONAL
FRAMING



SITE COPY

mil. 434



Job Track: **44755**
Layout ID: **286706**
Plan Log: **93746**

Builder / Location:

BAYVIEW WELLINGTON HOMES / BRADFORD

Project:

GREEN VALLEY EAST

Date:

06-Feb-18

Designer:

sonny

Model / Elevation:

**SD25-4 SONOMA4/ EL:B (STD/OPT) PAIRED
WITH EL:B(STD/OPT)(REV.)**

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Mitek ver 7.5.0

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)

NOTE:
ALL B-2-2X10 (FLUSH)

PRELIMINARY
LAYOUT ONLY
T-170587

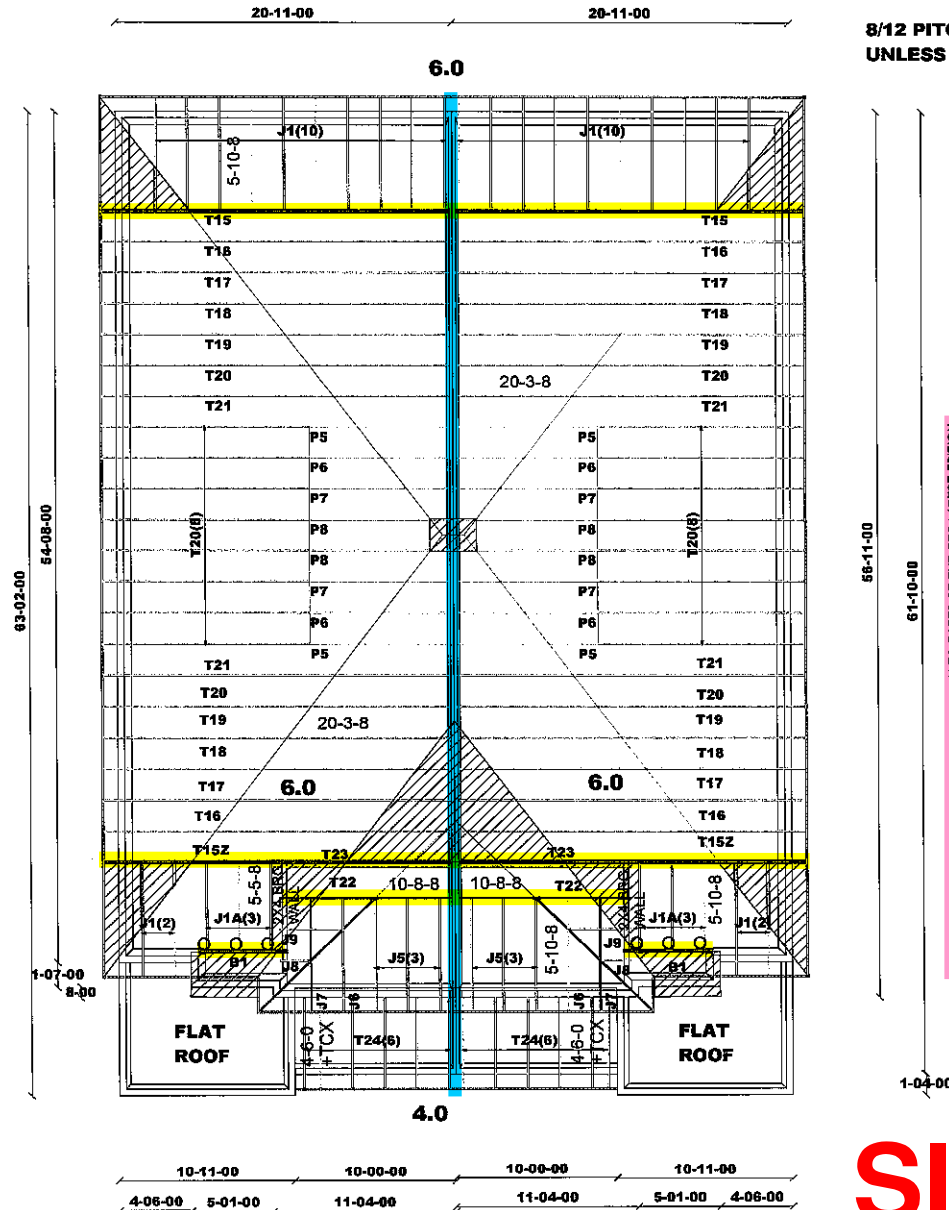
DENOTES:
CONVENTIONAL
FRAMING



EL:C (STD/OPT)
286716 D

Center line of
wood party wall

EL:C (STD/OPT)(REV)
286716 D



8/12 PITCHES (TYP.)
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE CBC, LATEST EDITION.
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2X6 SPP @ 24" O.C.
WITH A 2X4 SPP VERTICAL POST TO THE TRUSS UNDER EACH CROSS JOINT.
DESIGN LOADS: SNOW LOAD 32.5 PSF, IC DEAD 3 PSF, BC DEAD 7 PSF, BC WIND 10.5 PSF, BC WIND 7 PSF.
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 32.5 PSF
IC DEAD 3 PSF
BC DEAD 7 PSF
BC WIND 10.5 PSF
BC WIND 7 PSF

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Job Track: **44755**
Layout ID: **286715**
Plan Log: **93746**

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **05-Feb-18** Designer: **sonny**

Model / Elevation: **SD25-4 SONOMA4/ EL:C(STD/OPT) PAIRED WITH EL:C(STD/OPT)(REV.)**

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Mitek ver 7.5.0



Delivery Shiplist

DATE	02/05/18
SALES REP	Mario

JOB TRACK:44755 LAYOUT ID: 286695 D LOCATION:
 BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
 MODEL: SD25-4 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1 2 Ply	T1 HALF HIP	10.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	163.54 104.00		
	1 2 Ply	T1Z HALF HIP	10.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	163.54 104.00		
	2	T2 HALF HIP	10.00 0.00	20-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	174.78 111.66		
	2	T3 HALF HIP	10.00 0.00	20-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	186.80 118.66		
	2	T4 HALF HIP	10.00 0.00	20-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	190.36 119.66		
	2	T5 HALF HIP	10.00 0.00	20-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	201.04 126.66		
	10	T6 HALF HIP	10.00 0.00	20-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	1149.60 723.30		
	2	T7 HALF HIP	10.00 0.00	20-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-04	242.62 151.66		
	2	T8 MONOPITCH	10.00 8.00	10-08-08	10-06-12	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 10-06-12	126.54 81.66		
	2	T9 MONOPITCH	10.00 0.00	10-08-08	10-06-12	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 10-06-12	111.48 70.66		
	1	T10A HALF HIP	14.00 0.00	09-02-00	02-10-15	2 X 4	2 X 4	01-03-08 00-10-08	01-06-01 02-10-15	54.88 37.00		
	6	T14 MONO TRUSS	4.00 0.00	05-01-00	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-00-04	109.86 72.00		
	2	P1 PIGGYBACK	10.00 0.00	10-08-07	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	73.12 48.66		
	2	P2 PIGGYBACK	10.00 0.00	10-08-07	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	75.64 49.00		
	2	P3 PIGGYBACK	10.00 0.00	10-08-07	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	81.00 52.66		
	2	P4 PIGGYBACK	10.00 0.00	10-08-07	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	81.70 52.00		
	15	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	5	J2 JACK-PARTIAL	4.00 0.00	07-09-00	02-10-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-10-15	117.10 75.00		

TOTAL # TRUSS= 63.00

TOTAL BFT OF ALL TRUSSES=

2258.29 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3555.45 LBS.

SITE COPY



Delivery Shiplist

DATE	02/05/18
SALES REP	Mario

JOB TRACK:44755 LAYOUT ID: 286713 **D** LOCATION:
 BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
 MODEL: SD25-4 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	10.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	163.54 104.00		
	2 Ply											
	1	T1Z HALF HIP	10.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	163.54 104.00		
	2 Ply											
	2	T2 HALF HIP	10.00 0.00	20-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	174.78 111.66		
	2	T3 HALF HIP	10.00 0.00	20-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	186.80 118.66		
	2	T4 HALF HIP	10.00 0.00	20-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	190.36 119.66		
	2	T5 HALF HIP	10.00 0.00	20-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	201.04 126.66		
	10	T6 HALF HIP	10.00 0.00	20-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	1149.60 723.30		
	2	T7 HALF HIP	10.00 0.00	20-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-01-04	242.62 151.66		
	1	T11 HALF HIP	10.00 0.00	10-08-08	03-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-04	45.67 29.50		
	1	T12 HALF HIP	10.00 0.00	10-08-08	06-11-04	2 X 4	2 X 4	01-03-08 00-00-00	03-07-11 06-11-04	59.57 38.67		
	1	T13 HALF HIP	10.00 0.00	10-08-08	08-07-04	2 X 4	2 X 4	01-03-08 00-00-00	03-07-11 08-07-04	65.53 42.50		
	6	T14 MONO TRUSS	4.00 0.00	05-01-00	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-00-04	109.86 72.00		
	2	P1 PIGGYBACK	10.00 0.00	10-08-07	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	73.12 48.66		
	2	P2 PIGGYBACK	10.00 0.00	10-08-07	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	75.64 49.00		
	2	P3 PIGGYBACK	10.00 0.00	10-08-07	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	81.00 52.66		
	2	P4 PIGGYBACK	10.00 0.00	10-08-07	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	81.70 52.00		
	15	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	5	J4 JACK-OPEN	10.00 0.00	01-11-08	03-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-04	43.60 30.00		

TOTAL # TRUSS= 61.00

TOTAL BFT OF ALL TRUSSES=

2134.64 BFT.

TOTAL WEIGHT OF ALL TRUSSES= 3359.82 LBS.

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Delivery Shiplist

DATE	02/05/18
SALES REP	Mario

JOB TRACK: 44755	LAYOUT ID: 286716	LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: SD25-4	ELEVATION: C	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		
	3	J1A JACK-OPEN	6.00 0.00	05-05-08	04-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-08 04-01-04	43.14 27.99		
	3	J5 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	53.49 32.01		
	1	J6 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	14.97 9.33		
	1	J7 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	J8 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	00-00-00 -00-01-01	01-04-13 00-03-08	5.90 4.00		
	1	J9 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	00-00-00 01-10-15	01-04-13 00-05-03	8.64 5.33		
	2	P5 PIGGYBACK	8.00 0.00	07-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	53.38 34.66		
	2	P6 PIGGYBACK	8.00 0.00	07-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	56.34 37.34		
	2	P7 PIGGYBACK	8.00 0.00	07-11-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	60.28 38.66		
	2	P8 PIGGYBACK	8.00 0.00	07-11-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-10-08	65.98 43.34		
	1 2 Ply	T15 HALF HIP	8.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-01-04	162.46 103.68		
	1 2 Ply	T15Z HALF HIP	8.00 0.00	20-03-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-01-04	162.46 103.68		
	2	T16 HALF HIP	8.00 0.00	20-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	173.92 110.00		
	2	T17 HALF HIP	8.00 0.00	20-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	178.26 112.66		
	2	T18 HALF HIP	8.00 0.00	20-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	192.54 121.34		
	2	T19 HALF HIP	8.00 0.00	20-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	204.10 127.00		
	10	T20 HALF HIP	8.00 0.00	20-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	1080.70 673.30		

SITE COPY



Delivery Shiplist

DATE	02/05/18
SALES REP	Mario

JOB TRACK:44755 LAYOUT ID: 286716 D LOCATION:
 BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
 MODEL: SD25-4 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	T21 HALF HIP	8.00 0.00	20-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	220.22 138.66		
	1	T22 HALF HIP	8.00 0.00	10-08-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 05-03-13	50.12 32.33		
	1	T23 HALF HIP	8.00 0.00	10-08-08	06-07-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 06-07-13	53.73 35.17		
	6	T24 MONO TRUSS	4.00 0.00	04-06-00	02-01-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-15	100.32 67.98		

TOTAL # TRUSS= 62.00

TOTAL BFT OF ALL TRUSSES=

1993.83 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3154.66 LBS.

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Tamarack Roof Truss, Burlington



HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 224.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 4-0-12, 126.3 lbs FACTORED DOWN AT 6-0-12, 126.3 lbs FACTORED DOWN AT 8-0-12, 126.3 lbs FACTORED DOWN AT 10-0-12, 126.3 lbs FACTORED DOWN AT 12-0-12, 126.3 lbs FACTORED DOWN AT 14-0-12, 126.3 lbs FACTORED DOWN AT 16-0-12, AND 126.3 lbs FACTORED DOWN AT 18-0-12, AND 166.7 lbs FACTORED DOWN AT 20-3-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 10-0-12, 69.9 lbs FACTORED DOWN AT 12-0-12, 69.9 lbs FACTORED DOWN AT 14-0-12, 69.9 lbs FACTORED DOWN AT 16-0-12, AND 69.9 lbs FACTORED DOWN AT 18-0-12, AND 88.8 lbs FACTORED DOWN AT 20-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	8-0-12	-126	-126	—	BACK	VERT	TOTAL
Q	10-0-12	-126	-126	—	BACK	VERT	TOTAL
R	12-0-12	-126	-126	—	BACK	VERT	TOTAL
S	14-0-12	-126	-126	—	BACK	VERT	TOTAL
T	16-0-12	-126	-126	—	BACK	VERT	TOTAL
U	18-0-12	-126	-126	—	BACK	VERT	TOTAL
V	2-0-12	-40	-70	—	BACK	VERT	TOTAL
W	4-0-12	-40	-70	—	BACK	VERT	TOTAL
X	6-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	8-0-12	-40	-70	—	BACK	VERT	TOTAL
Z	10-0-12	-40	-70	—	BACK	VERT	TOTAL
AA	12-0-12	-40	-70	—	BACK	VERT	TOTAL
AB	14-0-12	-40	-70	—	BACK	VERT	TOTAL
AC	16-0-12	-40	-70	—	BACK	VERT	TOTAL
AD	18-0-12	-40	-70	—	BACK	VERT	TOTAL

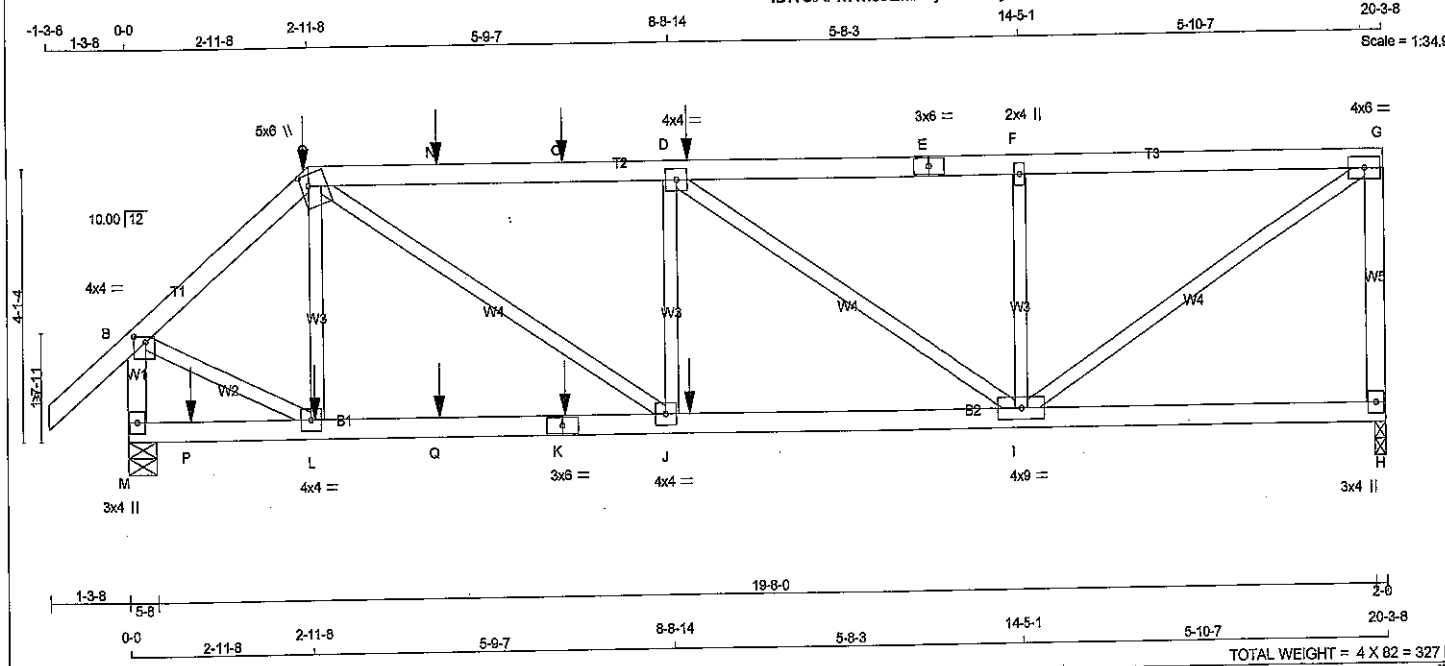


DWG NO. TAM 7394-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

Tamarack Roof Truss, Burlington

Scale = 1:34.9



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - E	1	12
E - G	1	12
G - H	1	12
M - B	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - K	1	12
K - H	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWV-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWV-t	MT20	4.0	8.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWVW-t	MT20	4.0	9.0		
J	BMWVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWVW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	2-0	5-8
H	1579	0	1579	0	0	2-0	2-0		
M	2197	0	2197	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE					
H	1259	771 / 0	250 / 0	0 / 0	0 / 0	238 / 0	0 / 0	0 / 0	0 / 0
M	1736	1089 / 0	326 / 0	0 / 0	0 / 0	321 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO			
A-B	0 / 47	-104.9	-104.9 0.08 (1)	10.00	L-C	-372 / 167	0.05 (1)
B-C	-1920 / 0	-104.9	-104.9 0.10 (1)	6.21	B-L	0 / 1598	0.20 (1)
C-N	-2595 / 0	-104.9	-104.9 0.43 (1)	5.10	C-J	0 / 1358	0.17 (1)
N-O	-2595 / 0	-104.9	-104.9 0.43 (1)	5.10	J-D	-509 / 121	0.07 (1)
O-D	-2595 / 0	-104.9	-104.9 0.43 (1)	5.10	D-I	-803 / 0	0.35 (1)
D-E	-1929 / 0	-104.9	-104.9 0.38 (1)	5.79	I-F	-632 / 0	0.09 (1)
E-F	-1929 / 0	-104.9	-104.9 0.38 (1)	5.79	I-G	0 / 2301	0.28 (1)
F-G	-1929 / 0	-104.9	-104.9 0.27 (1)	5.97			
H-G	-1509 / 0	0.0	0.0 0.20 (1)	7.81			
M-B	-2163 / 0	0.0	0.0 0.13 (1)	7.56			

M-P	0 / 0	-28.0	-28.0 0.14 (2)	10.00
P-L	0 / 0	-28.0	-28.0 0.14 (2)	10.00
L-Q	0 / 1460	-28.0	-28.0 0.23 (2)	10.00
Q-K	0 / 1460	-28.0	-28.0 0.23 (2)	10.00
K-J	0 / 1460	-28.0	-28.0 0.23 (2)	10.00
J-I	0 / 2596	-28.0	-28.0 0.29 (1)	10.00
I-H	0 / 0	-28.0	-28.0 0.12 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
C	2-11-8	-16	-18	---	FRONT	VERT
C	2-11-8	-130	-130	---	BACK	VERT
C	2-11-8	-209	-209	---	FRONT	VERT
D	9-0-4	-126	-126	---	BACK	VERT
J	9-0-4	-40	-70	---	BACK	VERT
K	7-0-4	-40	-70	---	BACK	VERT
L	3-0-4	-40	-70	---	BACK	VERT
N	5-0-4	-126	-126	---	BACK	VERT
O	7-0-4	-126	-126	---	BACK	VERT
P	1-0-4	-40	-70	---	BACK	VERT
Q	5-0-4	-40	-70	---	BACK	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.43/1.00 (C-D-I), BC=0.29/1.00 (J-K-I),
WB=0.35/1.00 (D-I-I), SSI=0.22/1.00 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMPM=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN
MT20	618 354	1667 822
		2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
286703	T1Z	2	2	TRUSS DESC.		
Version 8,200 S Dec 12 2017 MITek Industries, Inc. Thu Feb 1 10:46:32 2018 Page 2 ID:VsIAPxWhs5EmiF5jw80XK2ynJon-Klmo7 My1jMxhyZ7?7CiBlh1HColygYx89fV4ZzpYCb						

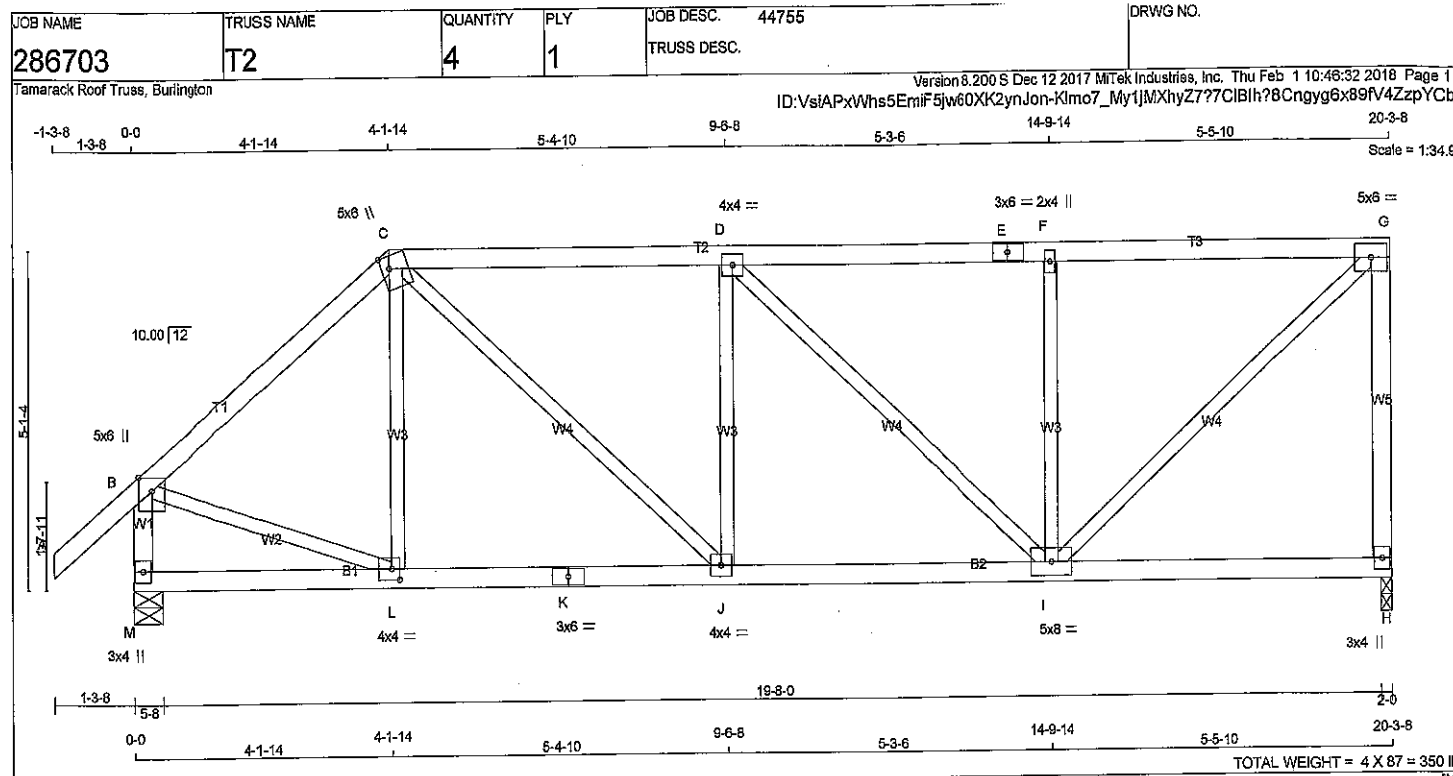
Tamarack Roof Truss, Burlington

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 224.8 lbs FACTORED DOWN AT 2-11-8,
129.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs
FACTORED DOWN AT 5-0-4, AND 126.3 lbs
FACTORED DOWN AT 7-0-4, AND 126.3 lbs
FACTORED DOWN AT 9-0-4 ON TOP CHORD,
AND 89.9 lbs FACTORED DOWN AT 1-0-4, 89.9
lbs FACTORED DOWN AT 3-0-4, 89.9 lbs
FACTORED DOWN AT 5-0-4, AND 89.9 lbs
FACTORED DOWN AT 7-0-4, AND 89.9 lbs
FACTORED DOWN AT 9-0-4 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

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DWG NO. TAM 7395-18
STRUCTURAL
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TTVW+m	MT20	5.0	6.0 2.25 1.50
D	TMVW-t	MT20	4.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMVW+w	MT20	2.0	4.0
G	TMVW-t	MT20	5.0	6.0
H	BMV1+p	MT20	3.0	4.0
I	BMVWV-t	MT20	5.0	8.0
J	BMVW-t	MT20	4.0	4.0
K	BS-t	MT20	3.0	6.0
L	BMVW-t	MT20	4.0	4.0 2.00 1.50
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	1349	0	1349	0	0
M	1494	0	1494	0	0

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	1075	659 / 0	213 / 0
M	1173	749 / 0	213 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 47	-104.9 -104.9	0.14 (1) 10.00
B-C	-1238 / 0	-104.9 -104.9	0.35 (1) 5.33
C-D	-1487 / 0	-104.9 -104.9	0.46 (1) 4.80
D-E	-1196 / 0	-104.9 -104.9	0.45 (1) 5.23
E-F	-1196 / 0	-104.9 -104.9	0.45 (1) 5.23
F-G	-1196 / 0	-104.9 -104.9	0.45 (1) 5.23
H-G	-1286 / 0	0.0 0.0	0.57 (1) 7.11
M-B	-1449 / 0	0.0 0.0	0.15 (1) 6.79
M-L	0 / 0	-28.0 -28.0	0.15 (3) 10.00
L-K	0 / 945	-28.0 -28.0	0.26 (2) 10.00
K-J	0 / 945	-28.0 -28.0	0.26 (2) 10.00
J-I	0 / 1487	-28.0 -28.0	0.35 (2) 10.00
I-H	0 / 0	-28.0 -28.0	0.20 (3) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.68")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.68")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.57/1.00 (G-H:1), BC=0.35/1.00 (I-J:2), WB=0.38/1.00 (D-I:1), SS=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

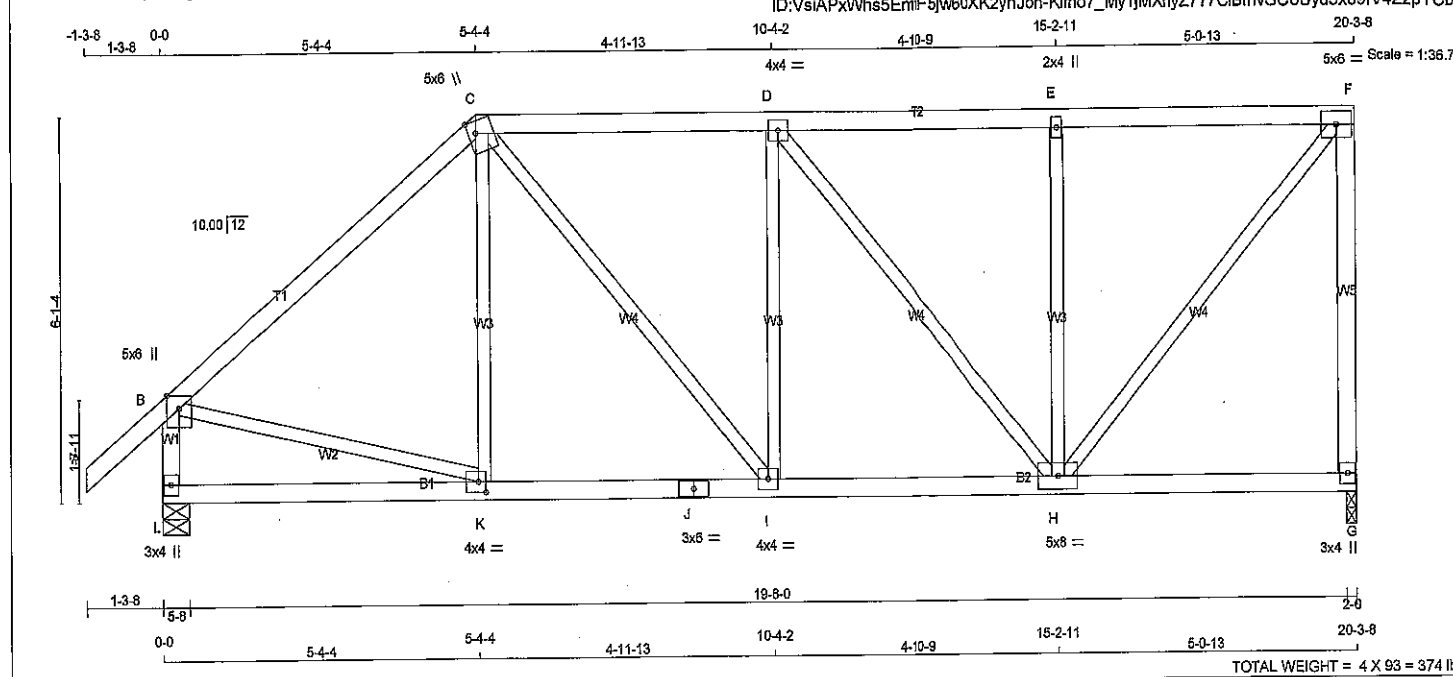
JSI GRIP= 0.85 (L) (INPUT = 0.80)

JSI METAL= 0.35 (L) (INPUT = 1.00)



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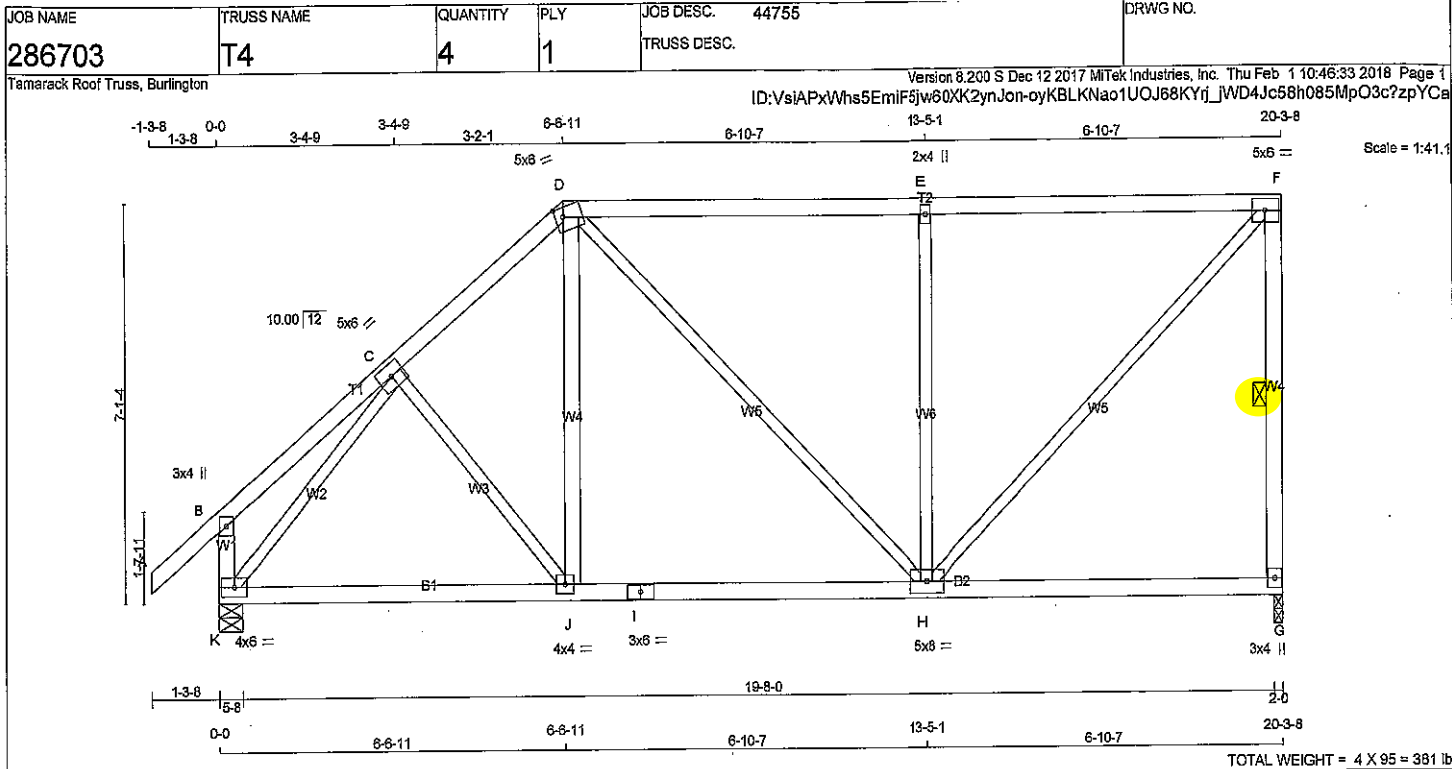
DWG NO. TAM 7396-18
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY				DESIGN CRITERIA			
N. L. G. A. RULES				BUILDING DESIGNER				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				TOP CH. LL = 32.5 PSF			
A - C	2x4	DRY	No.2	FACTORED				DL = 3.0 PSF			
C - F	2x4	DRY	No.2	GROSS REACTION				BOT CH. LL = 10.5 PSF			
G - F	2x4	DRY	No.2	DOWN				DL = 7.0 PSF			
L - B	2x4	DRY	No.2	UP				TOTAL LOAD = 53.0 PSF			
L - J	2x4	DRY	No.2	L				SPACING = 24.0 IN. C/C			
J - G	2x4	DRY	No.2	L				LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
ALL WEBS	2x3	DRY	No.2	UNFACTORED REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010			
EXCEPT				1ST LCASE				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				MAX./MIN. COMPONENT REACTIONS				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				JT COMBINED				- CSA 086-09			
				G				- TPIC 2011			
				L				(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				L				ALLOWABLE DEFL.(LL)= L/360 (0.68")			
				L				CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")			
				L				ALLOWABLE DEFL.(TL)= L/360 (0.68")			
				L				CALCULATED VERT. DEFL.(TL)= L/ 999 (0.07")			
				L				CST TC=0.93/1.00 (F-G:1), BC=0.30/1.00 (H-1:2), WB=0.51/1.00 (D-H:1), SS=0.25/1.00 (E-F:1)			
				L				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
				L				COMPANION LIVE LOAD FACTOR = 0.50			
				L				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				L				NAIL VALUES			
				L				PLATE GRIP(DRY) SHEAR SECTION			
				L				(PSI) (PLI) (PLI)			
				L				MAX MIN MAX MIN MAX MIN			
				L				MT20 618 354 1667 622 2284 1656			
				L				PLATE PLACEMENT TOL. = 0.250 inches			
				L				PLATE ROTATION TOL. = 5.0 Deg.			
				L				JSI GRIP= 0.84 (K) (INPUT = 0.90)			
				L				JSI METAL= 0.35 (K) (INPUT = 1.00)			
				L				DWG NO. TAM 7397-18			
				L				STRUCTURAL COMPONENT ONLY			

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
G - F	2x4	DRY	No.2 SPF
K - B	2x4	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
I - G	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
J - D	2x4	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+I	MT20	5.0	6.0	
D	TTWV+m	MT20	5.0	6.0	2.00 1.75
E	TMVW+w	MT20	2.0	4.0	
F	TMVW+I	MT20	5.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVWVW+I	MT20	5.0	6.0	
I	BS+I	MT20	3.0	6.0	
J	BMVW+I	MT20	4.0	4.0	
K	BMVWV1+I	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
G	1349	0	0	1349	0	0	2-0	2-0	
K	1494	0	0	1494	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOOSE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0		
K	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	C-J	-42 / 85	0.02 (3)
B-C	0 / 24	-104.9 -104.9	0.17 (1)	10.00	J-D	0 / 317	0.05 (2)
C-D	-1194 / 0	-104.9 -104.9	0.19 (1)	5.63	D-H	0 / 147	0.03 (1)
D-E	-1008 / 0	-104.9 -104.9	0.92 (1)	4.20	H-E	-594 / 0	0.78 (1)
E-F	-1006 / 0	-104.9 -104.9	0.92 (1)	4.20	H-F	0 / 1417	0.32 (1)
G-F	-1270 / 0	0.0 0.0	0.29 (1)	5.71	K-C	-1487 / 0	0.63 (1)
K-B	-278 / 0	0.0 0.0	0.03 (1)	7.81			
K-J	0 / 923	-28.0 -28.0	0.39 (2)	10.00			
J-I	0 / 901	-28.0 -28.0	0.40 (2)	10.00			
I-H	0 / 901	-28.0 -28.0	0.40 (2)	10.00			
H-G	0 / 0	-28.0 -28.0	0.31 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA D88-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.09")
ALLOWABLE DEFL. (TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.15")

CSI: TC=0.92/1.00 (E-F:1), BC=0.40/1.00 (H-J:2), WB=0.78/1.00 (E-H:1), SS=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	PLI	MAX MIN	MAX MIN
MT20	618	354	1867	822	2284 1656

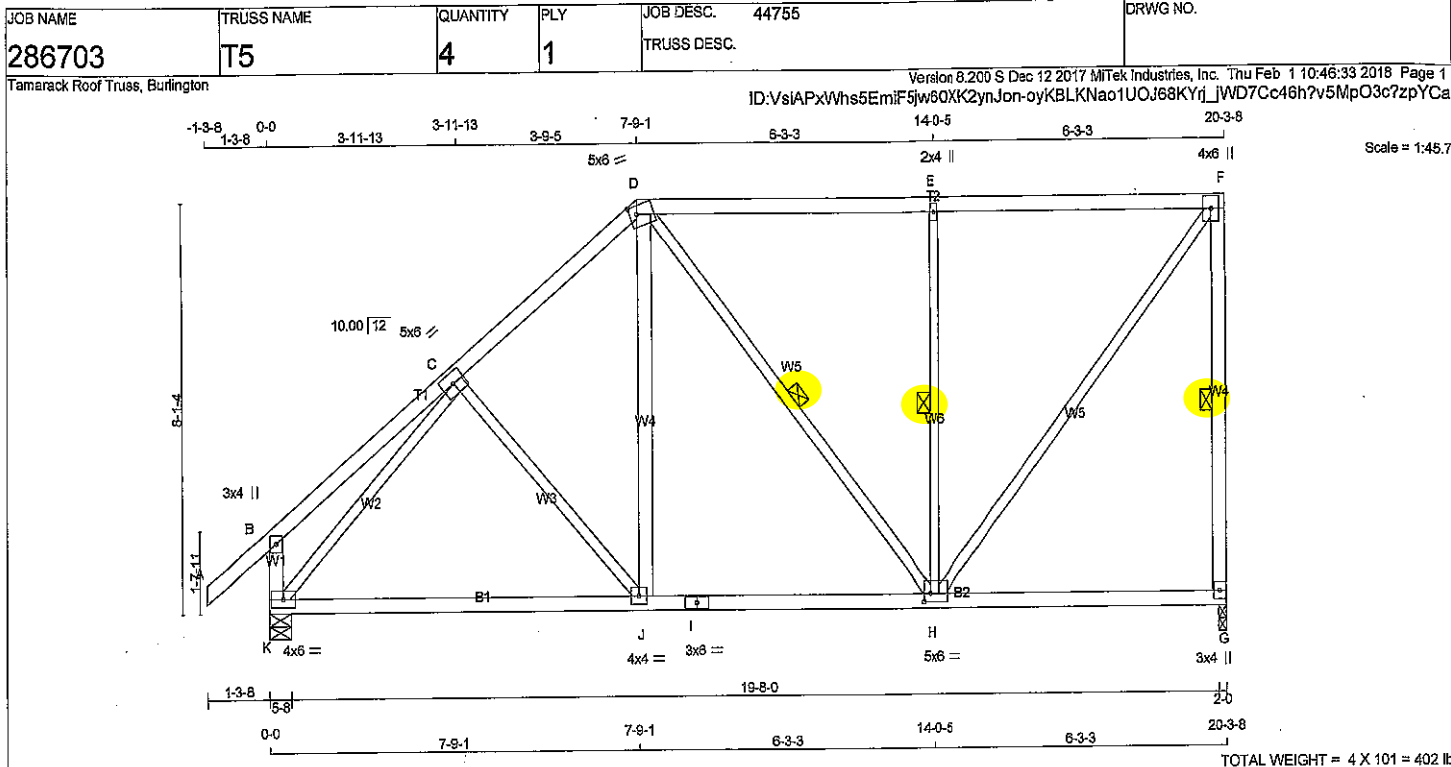
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.38 (C) (INPUT = 1.00)

DWG NO. TAW 7398-18
STRUCTURAL
COMPONENT ONLY

SITE COPY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	5.0	6.0
D	TTVW-m	MT20	5.0	6.0 2.00 1.75
E	TMVW-w	MT20	2.0	4.0
F	TMVW+p	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVWV-t	MT20	5.0	6.0 2.00 1.50
I	BS-t	MT20	3.0	6.0
J	BMVW-t	MT20	4.0	4.0
K	BMVW1-t	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT				
G	1349	0	1349	0
K	1494	0	1494	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
JT								
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0	
K	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	LC1 MAX	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO			FR-TO		FROM TO		
A-B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	C-J	-143 / 62	0.09 (1)		
B-C	0 / 30	-104.9 -104.9	0.25 (1)	10.00	J-D	0 / 394	0.08 (2)		
C-D	-1133 / 0	-104.9 -104.9	0.28 (1)	5.62	D-H	-77 / 0	0.05 (2)		
D-E	-823 / 0	-104.9 -104.9	0.74 (1)	5.26	H-E	-815 / 0	0.34 (1)		
E-F	-823 / 0	-104.9 -104.9	0.74 (1)	5.26	H-F	0 / 1315	0.30 (1)		
F-G	-1275 / 0	0.0	0.0 0.38 (1)	5.71	K-C	-1449 / 0	0.86 (1)		
K-B	-300 / 0	0.0	0.0 0.03 (1)	7.81					
K-J	0 / 941	-28.0	-28.0 0.47 (2)	10.00					
J-I	0 / 851	-28.0	-28.0 0.47 (2)	10.00					
I-H	0 / 851	-28.0	-28.0 0.47 (2)	10.00					
H-G	0 / 0	-28.0	-28.0 0.23 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (TL) = $L/936$ (0.26")

CSI: TC=0.74/1.00 (E-F-1), BC=0.47/1.00 (H-J-2),
WB=0.86/1.00 (C-K-1), SS=0.32/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)

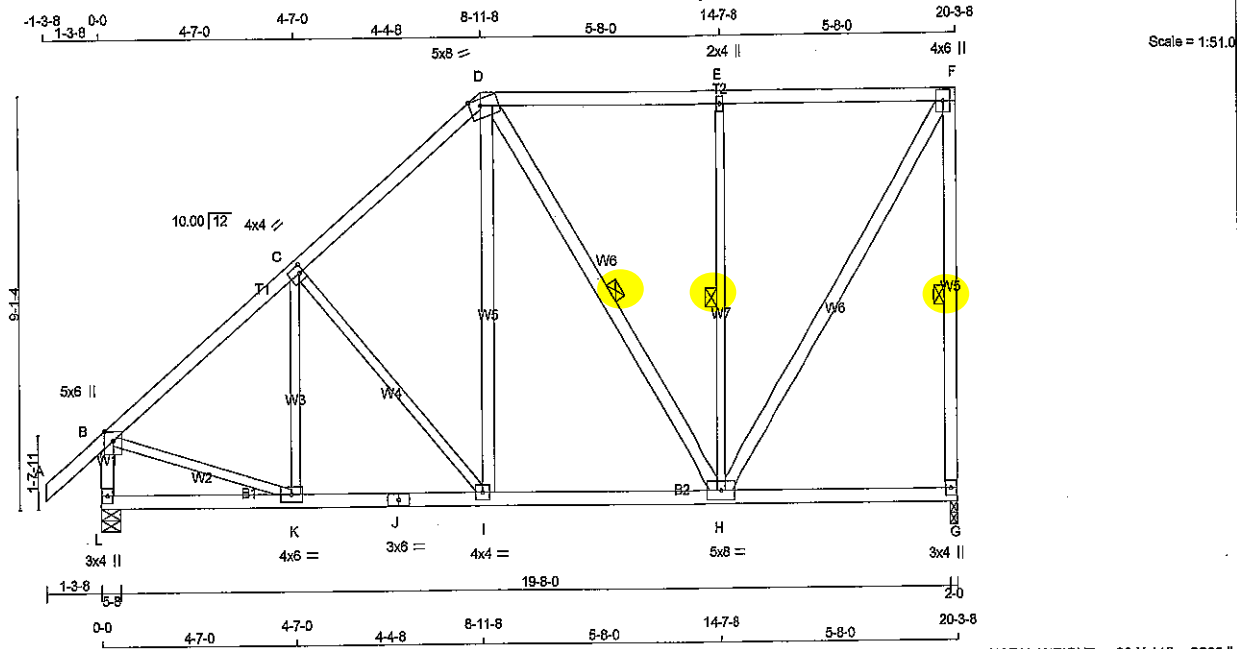


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DWG NO. TAM 7399-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

ID:VslAPxWns5EmIF5Jw60XK2ynJon-oyKBLKNao1UOJ68KYrj_JWD9ac7dh675MpO3c?zpYCa



TOTAL WEIGHT = 20 X 115 = 2299 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+1	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	8.0	Edge	3.00
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW+1	MT20	5.0	8.0		
I	BMVW+1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMVW+1	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	1349	0	1349	0	2-0
L	1494	0	1494	0	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0	0 / 0
L	1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 47	-104.9 -104.9	0.14 (1)	K-C	-145 / 128
B-C	-1257 / 0	-104.9 -104.9	0.40 (1)	C-I	-339 / 0
C-D	-1040 / 0	-104.9 -104.9	0.38 (1)	I-D	0 / 392
D-E	-881 / 0	-104.9 -104.9	0.59 (1)	D-H	-172 / 0
E-F	-882 / 0	-104.9 -104.9	0.59 (1)	H-E	-738 / 0
G-F	-1285 / 0	0.0 0.0	0.50 (1)	H-F	0 / 1260
L-B	-1440 / 0	0.0 0.0	0.15 (1)	B-K	0 / 1032
L-K	0 / 0	-28.0 -28.0	0.13 (3)		
K-J	0 / 993	-28.0 -28.0	0.24 (2)		
J-I	0 / 993	-28.0 -28.0	0.24 (2)		
I-H	0 / 775	-28.0 -28.0	0.31 (2)		
H-G	0 / 0	-28.0 -28.0	0.23 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.59/1.00 (E-F:1), BC=0.31/1.00 (H-I:2), WB=0.40/1.00 (E-H:1), SS=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

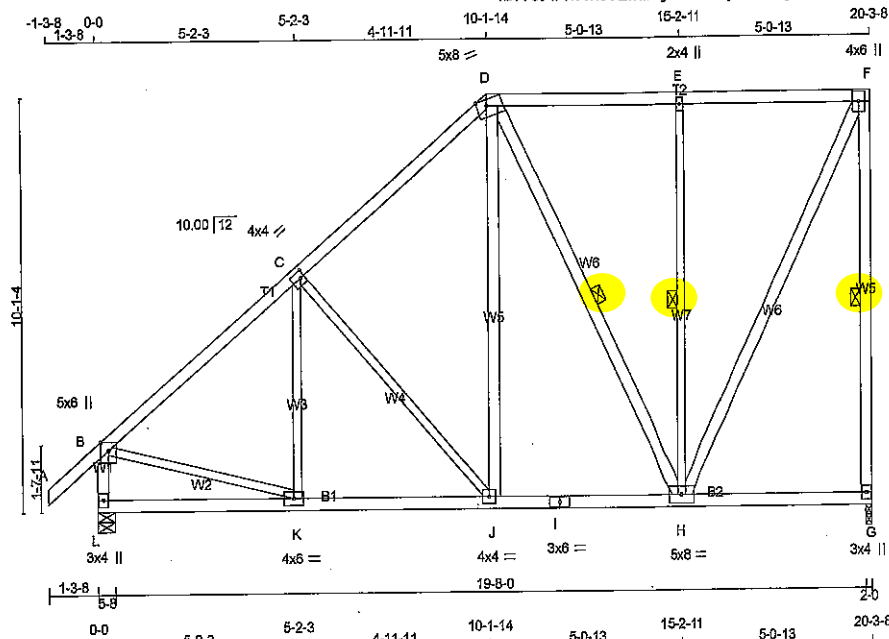
JSI GRIP= 0.70 (F) (INPUT = 0.90)
JSI METAL= 0.30 (J) (INPUT = 1.00)



SITE COPY

DRWG NO. TAM 7400-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



Scale = 1:56.2

TOTAL WEIGHT = 4 X 121 = 485 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
J - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	6.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT VERT	JT VERT	DOWN	UP
G 1349	0	1349	0
L 1494	0	1494	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM/LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD
G 1075	659 / 0	213 / 0	0 / 0	0 / 0	203 / 0
L 1173	749 / 0	213 / 0	0 / 0	0 / 0	211 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 147	-104.9	10.00
B-C	-1256 / 0	-104.9	5.04
C-D	-853 / 0	-104.9	5.82
D-E	-582 / 0	-104.9	6.25
E-F	-562 / 0	-104.9	6.25
G-F	-1291 / 0	0.0	5.68
L-B	-1435 / 0	0.0	6.82
L-K	0 / 0	-28.0	10.00
K-J	0 / 996	-28.0	10.00
J-I	0 / 705	-28.0	10.00
I-H	0 / 705	-28.0	10.00
H-G	0 / 0	-28.0	10.00

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO			
K-C	-89 / 182	0.05 (1)	
C-J	-443 / 0	0.51 (1)	
J-D	0 / 1461	0.07 (1)	
D-H	-310 / 0	0.20 (1)	
H-E	-557 / 0	0.46 (1)	
H-F	0 / 1225	0.20 (1)	
B-K	0 / 1027	0.23 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.84/1.00 (F-G:1), BC=0.29/1.00 (J-K:2), WB=0.51/1.00 (C-J:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

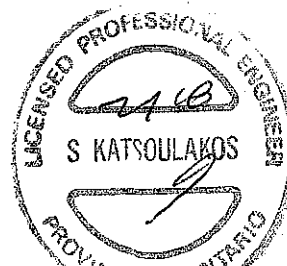
NAIL VALUES

PLATE GRIP(DRY)	SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

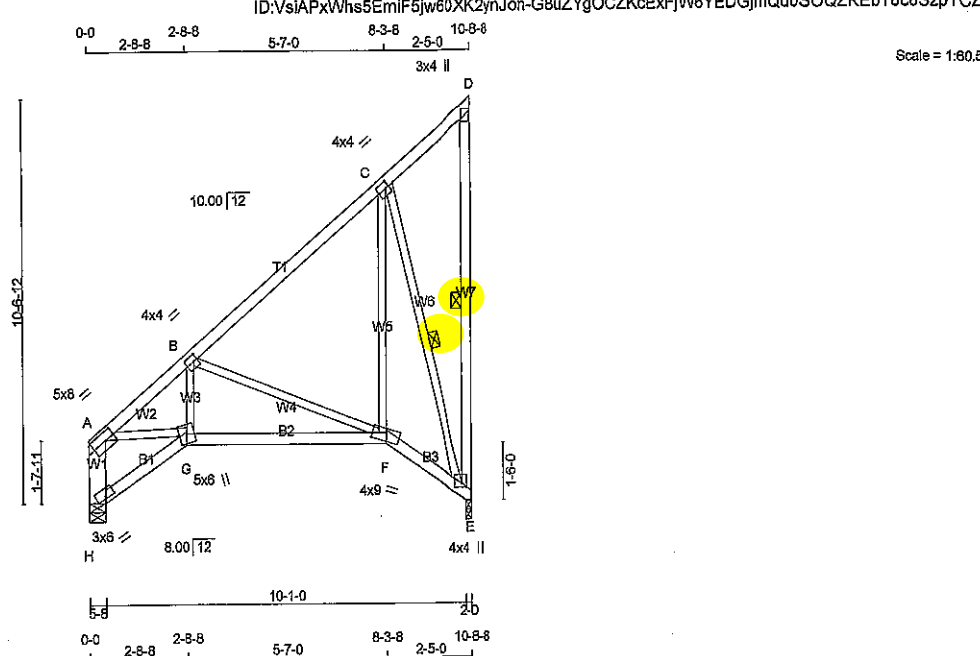
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (K) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



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DRWG NO. TAM 2401 -18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	8.0	2.50	3.25
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TMVW-1	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW+1+p	MT20	4.0	4.0	1.25	1.75
F	BBVW+m	MT20	4.0	8.0	2.75	4.50
G	BBVW+m	MT20	5.0	6.0		
H	BVM-1	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
E	712	0	0	2-0
H	712	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO		FR-TO			
A-B	-983 / 0	-104.9 -104.9	0.22 (1)	G-B	0 / 242	0.05 (2)	
B-C	-304 / 0	-104.9 -104.9	0.25 (1)	B-F	-587 / 0	0.34 (1)	
C-D	-53 / 0	-104.9 -104.9	0.22 (1)	F-C	0 / 485	0.11 (1)	
E-D	-50 / 0	0.0 0.0	0.03 (1)	C-E	-824 / 0	0.40 (1)	
H-A	-674 / 0	0.0 0.0	0.05 (1)	A-G	0 / 793	0.18 (1)	
H-G	0 / 0	-28.0 -28.0	0.06 (2)				
G-F	0 / 782	-28.0 -28.0	0.34 (2)				
F-E	0 / 290	-28.0 -28.0	0.08 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 63.0 PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.38")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")

ALLOWABLE DEFL.(TL)= $L/360$ (0.38")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CSI: TC=0.25/1.00 (B-C:1), BC=0.34/1.00 (F-G:2), WB=0.40/1.00 (C-E:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)

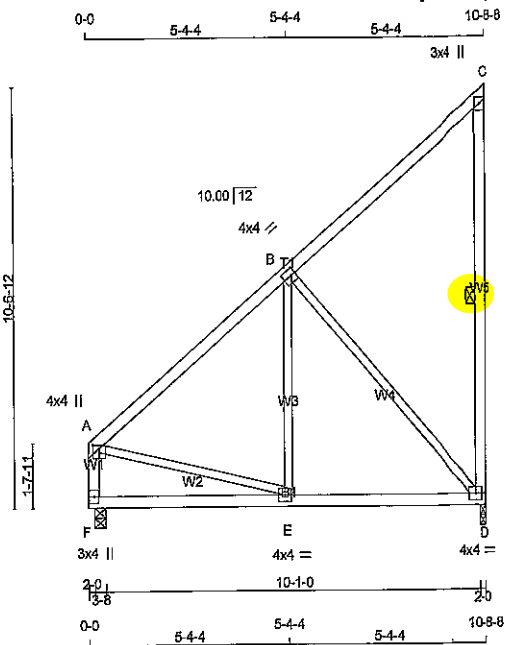
JSI METAL= 0.20 (A) (INPUT = 1.00)



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DRWG NO. TAM 7402-18

STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 4 X 56 = 223 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	BMVW-1	MT20	4.0	4.0	
E	BMVW-1	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT
D	712	0	712	0	0
F	712	0	712	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0
F	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-B	-491 / 0	-104.9 -104.9	0.39 (1) 6.25
B-C	-44 / 0	-104.9 -104.9	0.39 (1) 6.25
D-C	-211 / 0	0.0 0.0	0.12 (1) 6.25
F-A	-654 / 0	0.0 0.0	0.07 (1) 7.81
F-E	0 / 0	-28.0 -28.0	0.23 (3) 10.00
E-D	0 / 411	-28.0 -28.0	0.27 (2) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.39/1.00 (A-B:1), BC=0.27/1.00 (D-E:2), WB=0.74/1.00 (B-D:1), CSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

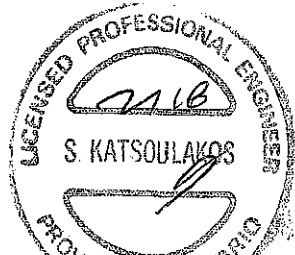
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

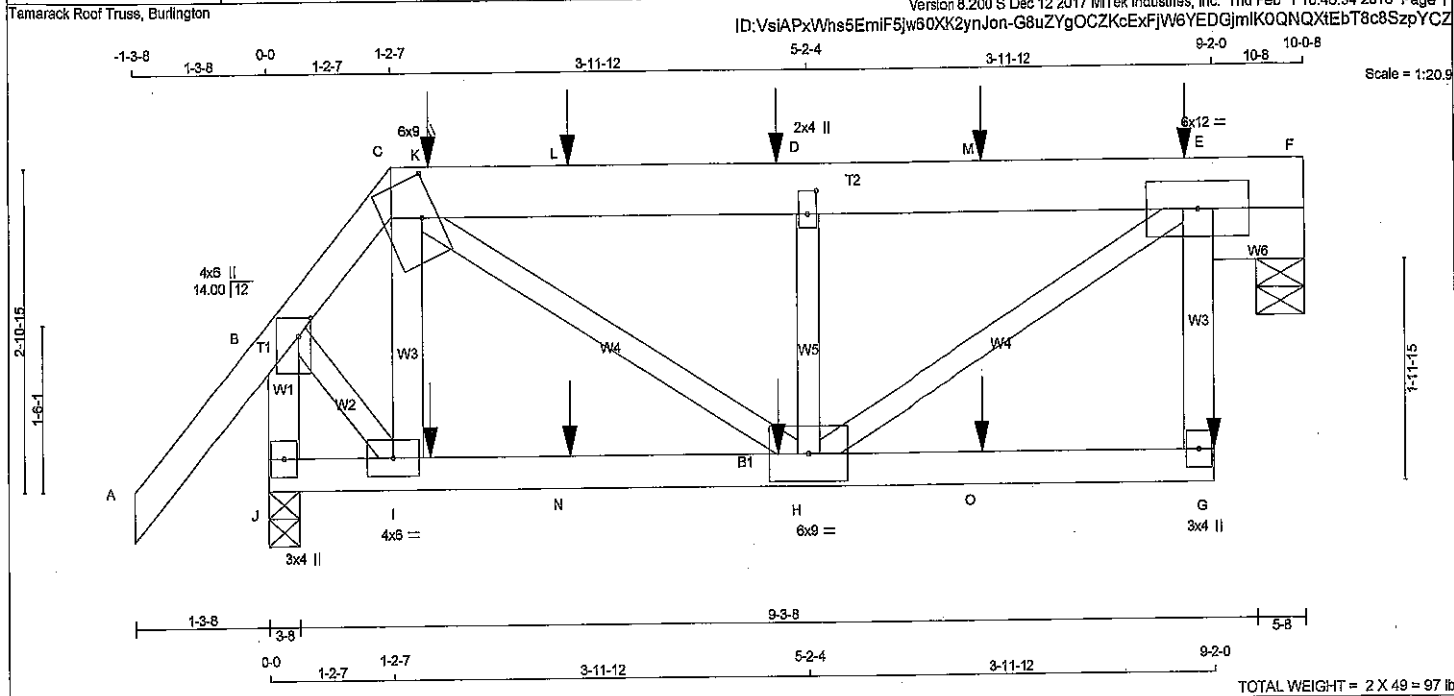
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (A) (INPUT = 0.80)
JSI METAL= 0.18 (A) (INPUT = 1.00)



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DRWG NO. TAM 7403-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x6	DRY 1650F 1.5E	SPF
E - F	2x6	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
J - G	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	1.50
C	TTWW+m	MT20	6.0	9.0	4.50	1.75
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMVWW-t	MT20	6.0	12.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	6.0	9.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 79.2 lbs FACTORED DOWN AT 1-6-12, 45.4 lbs FACTORED DOWN AT 2-10-12, 45.4 lbs FACTORED DOWN AT 4-10-12, AND 45.4 lbs FACTORED DOWN AT 6-10-12, AND 45.4 lbs FACTORED DOWN AT 8-10-12 ON TOP CHORD, AND 336.7 lbs FACTORED DOWN AT 1-6-12, 336.7 lbs FACTORED DOWN AT 2-10-12, 336.7 lbs FACTORED DOWN AT 4-10-12, AND 336.7 lbs FACTORED DOWN AT 6-10-12, AND 336.7 lbs FACTORED DOWN AT 8-10-12, AND 336.7 lbs FACTORED DOWN AT 10-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
F	1620	0	1620	0
J	1793	0	1793	0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	1285	798 / 0	248 / 0	0 / 0	239 / 0	0 / 0	0 / 0
J	1406	900 / 0	254 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/56	-104.9 -104.9	0.16 (1)	I-C	0/128	0.02 (3)	
B-C	-1274 / 0	-104.9 -104.9	0.18 (1)	C-H	0/1316	0.33 (1)	
C-K	-1899 / 0	-104.9 -104.9	0.22 (1)	H-D	-1067 / 0	0.19 (1)	
K-L	-1899 / 0	-104.9 -104.9	0.22 (1)	H-E	0/2252	0.56 (1)	
L-D	-1899 / 0	-104.9 -104.9	0.22 (1)	B-I	0/1029	0.25 (1)	
D-M	-1899 / 0	-104.9 -104.9	0.72 (1)				
M-E	-1899 / 0	-104.9 -104.9	0.72 (1)				
E-F	0 / 0	-104.9 -104.9	0.42 (1)				
G-E	0 / 492	0.0 0.0	0.09 (1)				
J-B	-1875 / 0	0.0 0.0	0.22 (1)				
J-I	0 / 0	-28.0 -28.0	0.21 (1)				
I-N	0 / 789	-28.0 -28.0	0.47 (1)				
N-H	0 / 789	-28.0 -28.0	0.47 (1)				
H-O	0 / 0	-28.0 -28.0	0.39 (1)				
O-G	0 / 0	-28.0 -28.0	0.39 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	TOTAL
D	4-10-12	-45	-45	---	FRONT	VERT	TOTAL	
E	8-10-12	-45	-45	---	FRONT	VERT	TOTAL	
G	9-2-0	-347	-347	---	FRONT	VERT	TOTAL	
H	4-10-12	-337	-337	---	FRONT	VERT	TOTAL	
I	1-6-12	-337	-337	---	FRONT	VERT	TOTAL	
K	1-6-12	-79	-79	---	FRONT	VERT	TOTAL	
L	2-10-12	-45	-45	---	FRONT	VERT	TOTAL	
M	6-10-12	-45	-45	---	FRONT	VERT	TOTAL	
N	2-10-12	-337	-337	---	FRONT	VERT	TOTAL	
O	6-10-12	-337	-337	---	FRONT	VERT	TOTAL	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/768 (0.16")

CSI: TC=0.72/1.00 (D-E:1), BC=0.47/1.00 (H-I:1), WB=0.56/1.00 (E-H:1), SS=0.54/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

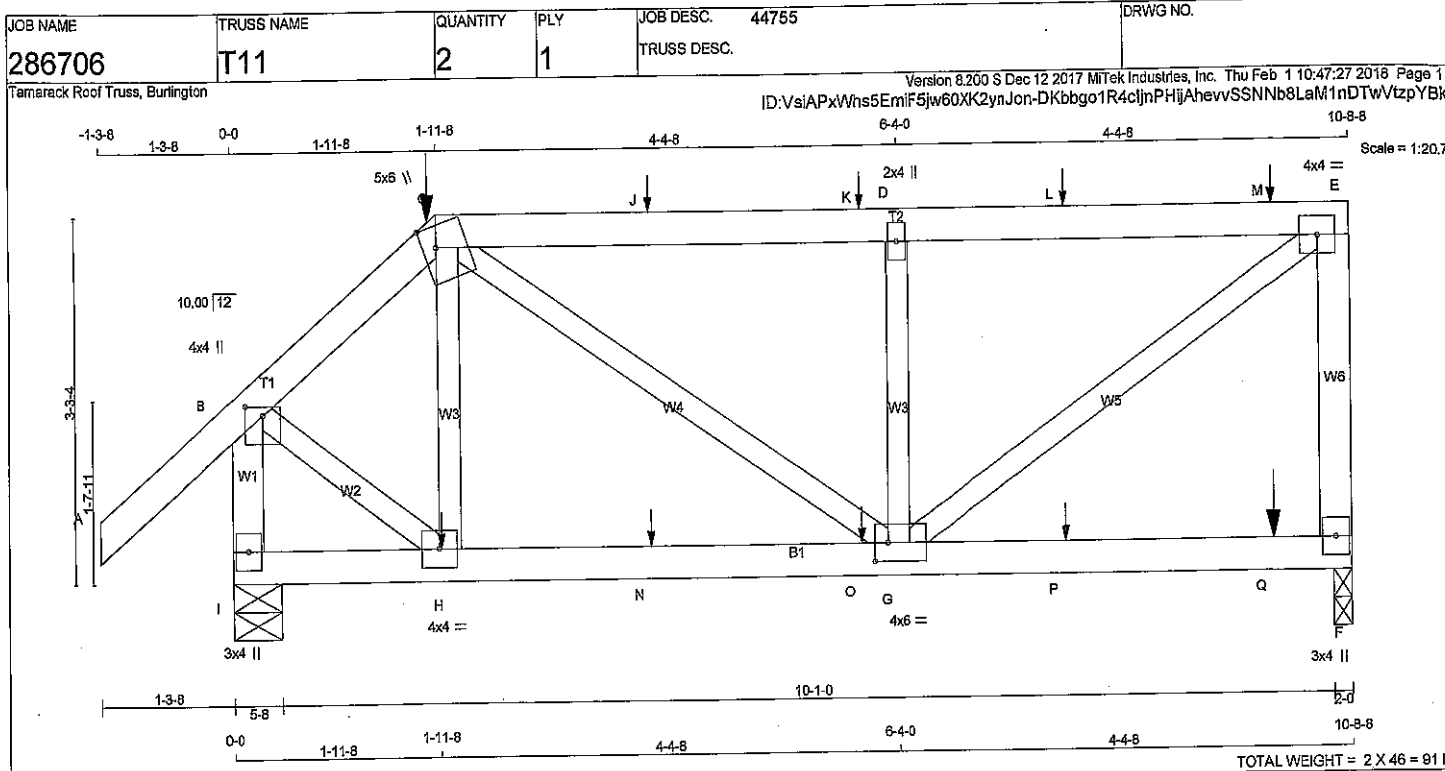
JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.58 (E) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 7404-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

DWG NO. TAM 7404-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
C	TTWW+m	MT20	5.0	6.0 2.25 1.50
D	TMVW+w	MT20	2.0	4.0
E	TMVW-l	MT20	4.0	4.0
F	BMV1+p	MT20	3.0	4.0
G	BMVWV-l	MT20	4.0	6.0 2.00 1.50
H	BMVWV-l	MT20	4.0	4.0
I	BMV1+p	MT20	3.0	4.0

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.9 lbs FACTORED DOWN AT 1-11-8, 1.0 lbs FACTORED DOWN AT 3-11-12, 1.0 lbs FACTORED DOWN AT 5-11-12, AND 1.0 lbs FACTORED DOWN AT 7-11-12, AND 1.0 lbs FACTORED DOWN AT 9-11-12 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AT 1-11-12, 1.0 lbs FACTORED DOWN AT 3-11-12, 1.0 lbs FACTORED DOWN AT 5-11-12, AND 1.0 lbs FACTORED DOWN AT 7-11-12, AND 1.0 lbs FACTORED DOWN AT 9-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
F	713	0	713	0	0	2-0	2-0	2-0	2-0
I	859	0	859	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DEAD	SOIL	DEAD	SOIL
F	559	348 / 0	113 / 0	0 / 0	0 / 0	108 / 0	0 / 0	108 / 0	0 / 0	117 / 0	0 / 0
I	667	438 / 0	112 / 0	0 / 0	0 / 0	117 / 0	0 / 0	117 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.		MAX. FACTORED		FACTORED		WEBS		MEMB.		MAX. FACTORED		FACTORED	
FR-TO	LOC.	LOC.	LOC.	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (PLF)	FR-TO	LOC.	LOC.	LOC.	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (PLF)
A-B	0 / 47	0 / 47	0 / 47	0 / 47	-104.9	-104.9	0.16 (1)	10.00	H-C	-155 / 52	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)
B-C	-529 / 0	-529 / 0	-529 / 0	-529 / 0	-104.9	-104.9	0.08 (1)	6.25	C-G	0 / 378	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)
C-J	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25	G-D	-583 / 0	0.12 (1)	0.12 (1)	0.12 (1)	0.12 (1)	0.12 (1)
J-K	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25	G-E	0 / 861	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)
K-D	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25	B-H	0 / 478	0.12 (1)	0.12 (1)	0.12 (1)	0.12 (1)	0.12 (1)
D-L	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25							
L-M	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25							
M-E	-711 / 0	-711 / 0	-711 / 0	-711 / 0	-104.9	-104.9	0.37 (1)	6.25							
F-E	-661 / 0	-661 / 0	-661 / 0	-661 / 0	0.0	0.0	0.12 (1)	7.81							
I-B	-845 / 0	-845 / 0	-845 / 0	-845 / 0	0.0	0.0	0.10 (1)	7.81							
I-H	0 / 0	0 / 0	0 / 0	0 / 0	-28.0	-28.0	0.07 (2)	10.00							
H-N	0 / 398	0 / 398	0 / 398	0 / 398	-28.0	-28.0	0.19 (2)	10.00							
N-O	0 / 398	0 / 398	0 / 398	0 / 398	-28.0	-28.0	0.19 (2)	10.00							
O-G	0 / 398	0 / 398	0 / 398	0 / 398	-28.0	-28.0	0.19 (2)	10.00							
G-P	0 / 0	0 / 0	0 / 0	0 / 0	-28.0	-28.0	0.14 (3)	10.00							
P-Q	0 / 0	0 / 0	0 / 0	0 / 0	-28.0	-28.0	0.14 (3)	10.00							
Q-F	0 / 0	0 / 0	0 / 0	0 / 0	-28.0	-28.0	0.14 (3)	10.00							

FACTORED CONCENTRATED LOADS (LBS)		MEMB.		MAX. FACTORED		FACTORED		WEBS		MEMB.		MAX. FACTORED		FACTORED	
JT	LOC.	LOC.	LOC.	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (PLF)	FR-TO	LOC.	LOC.	LOC.	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (PLF)
C	1-11-8	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
H	1-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
J	3-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
K	5-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
L	7-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
M	9-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
N	3-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
O	5-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
P	7-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1
Q	9-11-12	1	1	1	1	1	1	FRONT	VERT	TOTAL	TOTAL	1	1	1	1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.37/1.00 (D-E:1), BC=0.19/1.00 (G-H:2), WB=0.21/1.00 (E-G:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

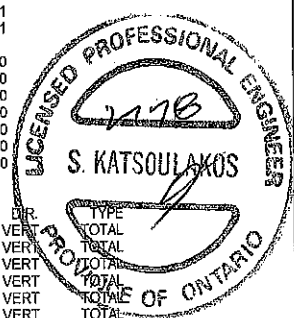
NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

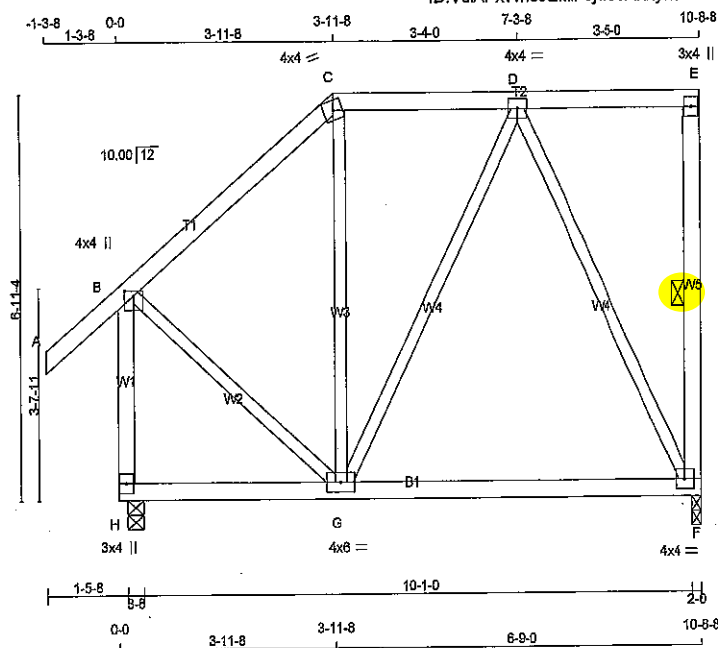
JSI GRIP= 0.87 (G) (INPUT = 0.90)

JSI METAL= 0.27 (E) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 7410 -18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 80 = 119 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
F	712	0	712	0	2-0	2-0
H	857	0	857	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	MAX. LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	666	438 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO								
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G-C	-100 / 95	0.08 (1)
B-C	-375 / 0	-104.9	-104.9	0.29 (1)	6.25	B-G	0 / 370	0.08 (1)
C-D	-285 / 0	-104.9	-104.9	0.20 (1)	6.25	G-D	0 / 145	0.03 (3)
D-E	0 / 0	-104.9	-104.9	0.20 (1)	10.00	D-F	-561 / 0	0.61 (1)
F-E	-137 / 0	0.0	0.0	0.03 (1)	6.25			
H-B	-826 / 0	0.0	0.0	0.18 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
G-F	0 / 256	-28.0	-28.0	0.30 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.29/1.00 (B-C:1), BC=0.30/1.00 (F-G:2), WS=0.61/1.00 (D-F:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



SITE COPY

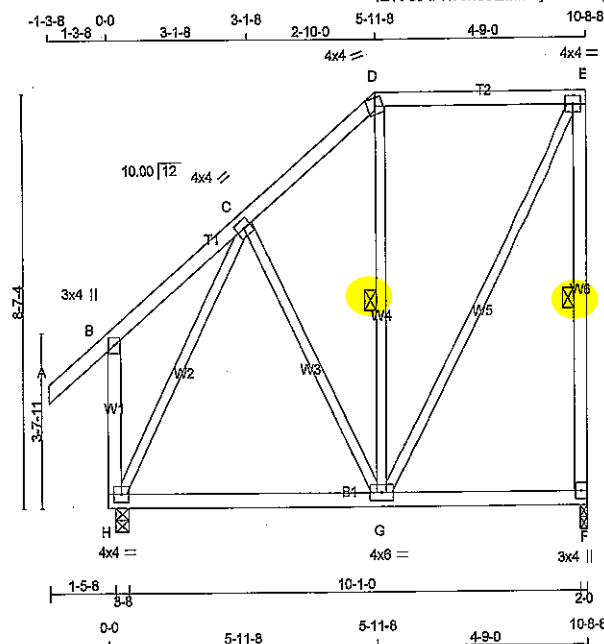
DRWG NO. TAM 7411-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
286706	T13	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Thu Feb 1 10:47:28 2018 Page 1

ID:VsiAPxWhs5Emf5jw60XK2ynJon-hX9zt814nwQaPZruGuCIS6?d8nv54_0B0tDT2JzpYBj



Scale: 1/4\"=1'

TOTAL WEIGHT = 2 X 66 = 131 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2		SPF
D - E	2x4	DRY	No.2		SPF
F - E	2x4	DRY	No.2		SPF
H - B	2x4	DRY	No.2		SPF
H - F	2x4	DRY	No.2		SPF
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	4.0			
D	TTW-m	MT20	4.0	4.0			
E	TMVV-t	MT20	4.0	4.0			
F	BMV1+p	MT20	3.0	4.0			
G	BMWW-t	MT20	4.0	6.0			
H	BMVW1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	712	0	712	0
H	857	0	857	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
F	567	348 / 0	112 / 0	0 / 0	107 / 0
H	666	438 / 0	112 / 0	0 / 0	115 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX.	MAX.	UNBRAC	LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX.	CSI (LC)
FR-TO			(LBS)			(PLF)	CS	(LC)	FR-TO					(LBS)		(LC)
A-B	0 / 47	-104.9	-104.9	0.14	(1)	10.00	G-D	-165 / 71	0.08	(1)						
B-C	0 / 23	-104.9	-104.9	0.18	(1)	10.00	G-E	0 / 481	0.11	(1)						
C-D	-334 / 0	-104.9	-104.9	0.12	(1)	6.25	C-G	-76 / 55	0.08	(1)						
D-E	-234 / 0	-104.9	-104.9	0.30	(1)	6.25	H-C	-566 / 0	0.44	(1)						
F-E	-668 / 0	0.0	0.0	0.23	(1)	6.25										
H-B	-269 / 0	0.0	0.0	0.06	(1)	7.81										
H-G	0 / 271	-28.0	-28.0	0.27	(2)	10.00										
G-F	0 / 0	-28.0	-28.0	0.25	(3)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.30/1.00 (D-E:1), BC=0.27/1.00 (G-H:2),
WB=0.44/1.00 (C-H:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

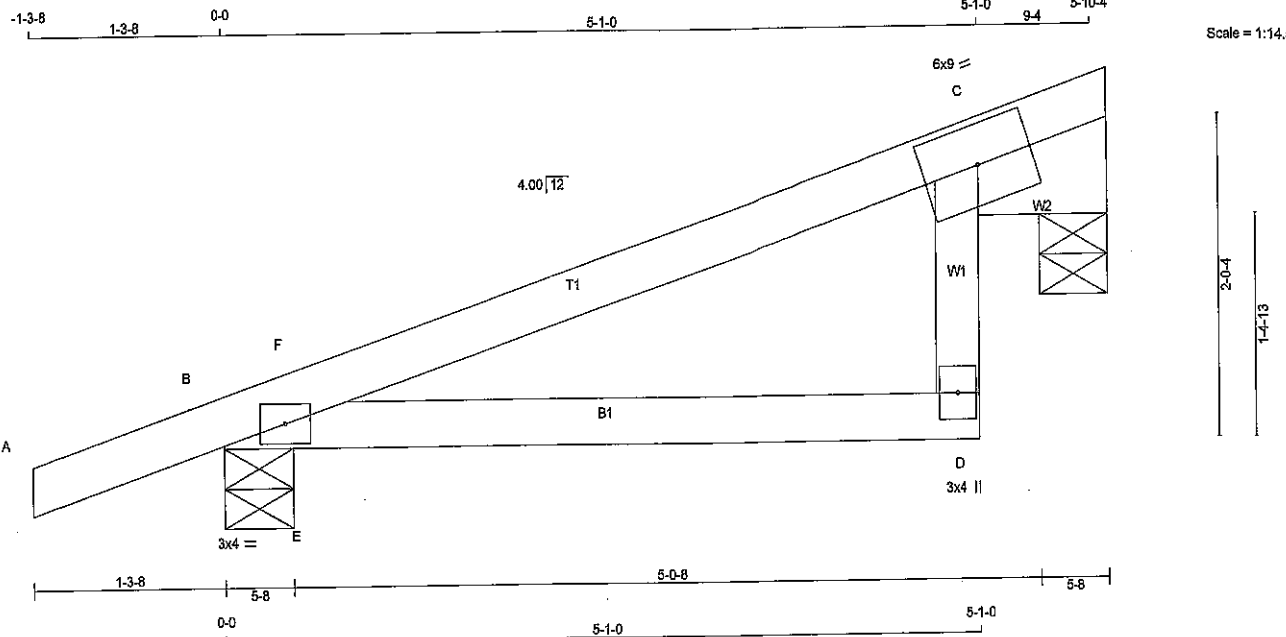
JSI GRIP= 0.67 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)



SITE COPY

DRWG NO. TAM 7412-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 12 X 18 = 220 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		
C	TMVW1-1	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	703	0	703	0	5-8	5-8
B	739	0	739	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
C	513	408 / 0	53 / 0	0 / 0	0 / 0	51 / 0	0 / 0
B	536	426 / 0	53 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 25	-104.9	-145.6	0.16 (1)	10.00	E-F	-653 / 133	0.00 (1)	
B-F	-48 / 43	-145.6	-164.1	0.12 (3)	8.25				
F-C	0 / 11	-164.1	-299.9	0.82 (1)	10.00				
D-C	0 / 149	0.0	0.0	0.03 (1)	10.00				
B-E	0 / 0	-28.0	-28.0	0.58 (1)	10.00				
E-D	0 / 0	-28.0	-28.0	0.58 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	OL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	OL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/492 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/360 (0.18")

CSI: TO=0.82/1.00 (C-F:1), BC=0.58/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.55/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

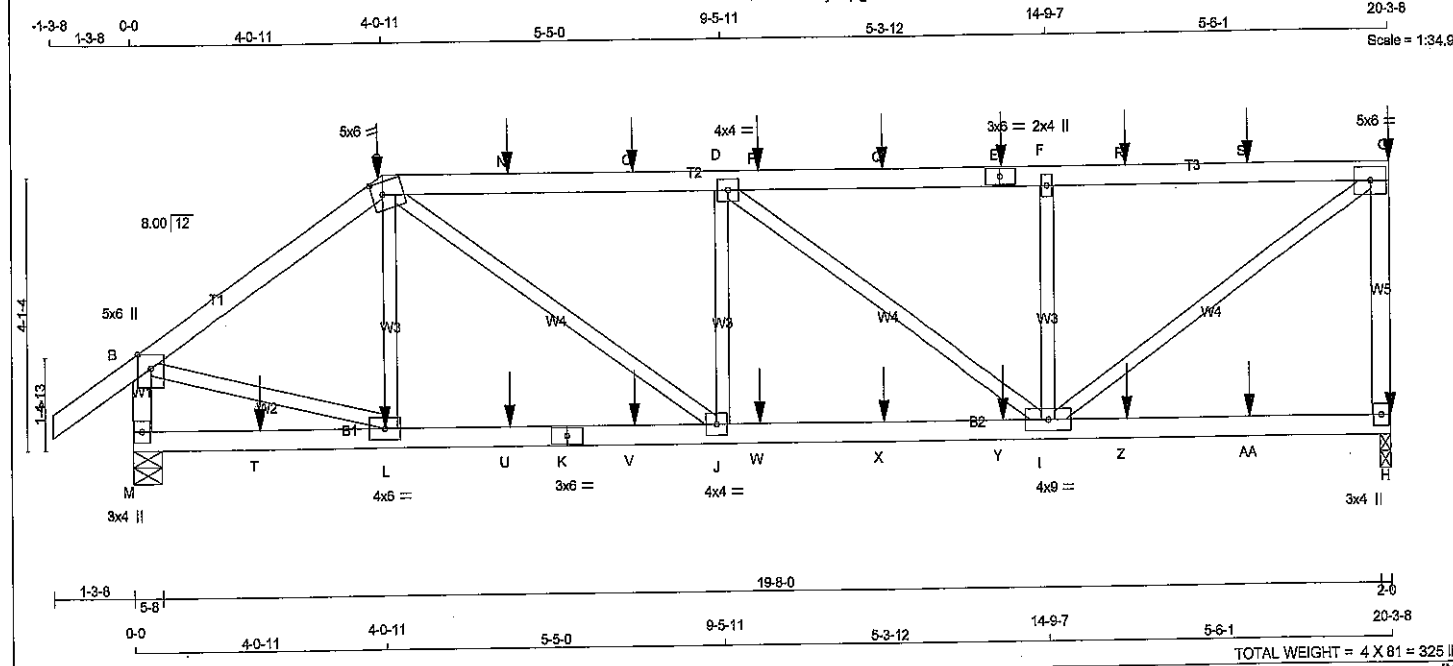
JSI GRIP= 0.48 (B) (INPUT = 0.90)

JSI METAL= 0.14 (B) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 7405-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 4 X 81 = 325 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1 12	SIDE(61.0)
C - E	1 12	SIDE(61.0)
E - G	1 12	SIDE(61.0)
G - H	1 12	TOP
M - B	1 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - K	1 12	SIDE(61.0)
K - H	1 12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+P	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMVW+I	MT20	4.0	4.0	
E	TS-I	MT20	3.0	6.0	
F	TMVW+P	MT20	2.0	4.0	
G	TMVW+I	MT20	5.0	6.0	
H	BMV+P	MT20	3.0	4.0	
I	BMVWW+I	MT20	4.0	9.0	
J	BMVWW-I	MT20	4.0	4.0	
K	BS-I	MT20	3.0	6.0	
L	BMVWW+I	MT20	4.0	6.0	
M	BMV+P	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 2357 0	2357 0	0 0	2-0
M	VERT 2380 0	2380 0	0 0	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW LIVE PERM.LIVE	0/0	363/0	0/0
H	1898	1141/0 385/0	0/0	345/0	0/0
M	1577	1183/0 349/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO	
A-B	0/40	-104.9 -104.9 0.08 (1)	10.00	L-C -297/208 0.04 (1)
B-C	-2547/0	-104.9 -104.9 0.20 (1)	5.47	B-L 0/2188 0.27 (1)
C-N	-3174/0	-104.9 -104.9 0.42 (1)	4.71	C-J 0/1303 0.16 (1)
N-O	-3174/0	-104.9 -104.9 0.42 (1)	4.71	J-D -502/116 0.07 (1)
O-D	-3174/0	-104.9 -104.9 0.42 (1)	4.71	D-I -808/0 0.32 (1)
D-P	-2518/0	-104.9 -104.9 0.41 (1)	5.18	I-F -987/0 0.13 (1)
P-Q	-2518/0	-104.9 -104.9 0.41 (1)	5.18	I-G 0/3083 0.38 (1)
Q-E	-2518/0	-104.9 -104.9 0.41 (1)	5.18	
E-F	-2518/0	-104.9 -104.9 0.41 (1)	5.18	
F-R	-2518/0	-104.9 -104.9 0.41 (1)	5.18	
R-S	-2518/0	-104.9 -104.9 0.41 (1)	5.18	
S-G	-2518/0	-104.9 -104.9 0.41 (1)	5.18	
H-G	-2218/0	0.0 0.0 0.30 (1)	7.49	
M-B	-2324/0	0.0 0.0 0.13 (1)	7.35	

M-T	0/0	-28.0 -28.0 0.13 (3)	10.00
T-L	0/0	-28.0 -28.0 0.13 (3)	10.00
L-U	0/2108	-28.0 -28.0 0.26 (1)	10.00
U-K	0/2108	-28.0 -28.0 0.26 (1)	10.00
K-V	0/2108	-28.0 -28.0 0.26 (1)	10.00
V-J	0/2108	-28.0 -28.0 0.26 (1)	10.00
J-W	0/3175	-28.0 -28.0 0.37 (1)	10.00
W-X	0/3175	-28.0 -28.0 0.37 (1)	10.00
X-Y	0/3175	-28.0 -28.0 0.37 (1)	10.00
Y-I	0/3175	-28.0 -28.0 0.37 (1)	10.00
I-Z	0/0	-28.0 -28.0 0.19 (3)	10.00
Z-AA	0/0	-28.0 -28.0 0.19 (3)	10.00
AA-H	0/0	-28.0 -28.0 0.19 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR	TYPE
C	4-0-11	-23	-25	FRONT	VERT	DEAD
C	4-0-11	-130	-130	FRONT	VERT	TOTAL
C	4-0-11	-282	-282	FRONT	VERT	SNOW
E	14-0-12	-126	-126	FRONT	VERT	TOTAL
G	20-3-8	-167	-167	FRONT	VERT	TOTAL
H	20-3-8	-51	-89	FRONT	VERT	TOTAL
L	4-0-12	-40	-70	FRONT	VERT	TOTAL
N	6-0-12	-126	-126	FRONT	VERT	TOTAL
O	6-0-12	-126	-126	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 53.0	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.42/1.00 (C-D:1), BC=0.37/1.00 (I-J:1), WB=0.38/1.00 (G-I:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

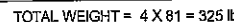
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.35 (K) (INPUT = 1.00)



DRY: SEASONED LUMBER.

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

PLATES (table is in inches)

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

LOADING
TOTAL LOAD CASES: (4)

M-Q	0/0	-28.0	-28.0	0.16 (2)	10.00
Q-R	0/0	-28.0	-28.0	0.16 (2)	10.00
R-L	0/0	-28.0	-28.0	0.16 (2)	10.00
L-S	0/1989	-28.0	-28.0	0.28 (2)	10.00
S-K	0/1989	-28.0	-28.0	0.28 (2)	10.00
K-T	0/1989	-28.0	-28.0	0.28 (2)	10.00
T-J	0/1989	-28.0	-28.0	0.28 (2)	10.00
J-I	0/2800	-28.0	-28.0	0.31 (1)	10.00
I-H	0/0	-28.0	-28.0	0.10 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIP	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	HEAD
C	4-0-11	-282	-282	—	FRONT	VERT	SNOW
K	7-0-12	-71	-84	—	BACK	VERT	TOTAL
N	5-0-12	-159	-159	—	BACK	VERT	TOTAL
O	7-0-12	-159	-159	—	BACK	VERT	TOTAL
P	9-0-12	-159	-159	—	BACK	VERT	TOTAL
Q	1-0-12	-40	-70	—	BACK	VERT	TOTAL
R	3-0-12	-40	-70	—	BACK	VERT	TOTAL
S	5-0-12	-71	-84	—	BACK	VERT	TOTAL
T	9-0-12	-71	-84	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.42/1.00 (C-D:1), BC=0.31/1.00 (I-J:1),
WB=0.41/1.00 (D-I:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1607	822	2284 105

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.39 (K) (INPUT = 1.00)

AM 7414-18
STRUCTURAL
MENT ONLY

HANGERS NOTES

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DWG NO. TAM 7414
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

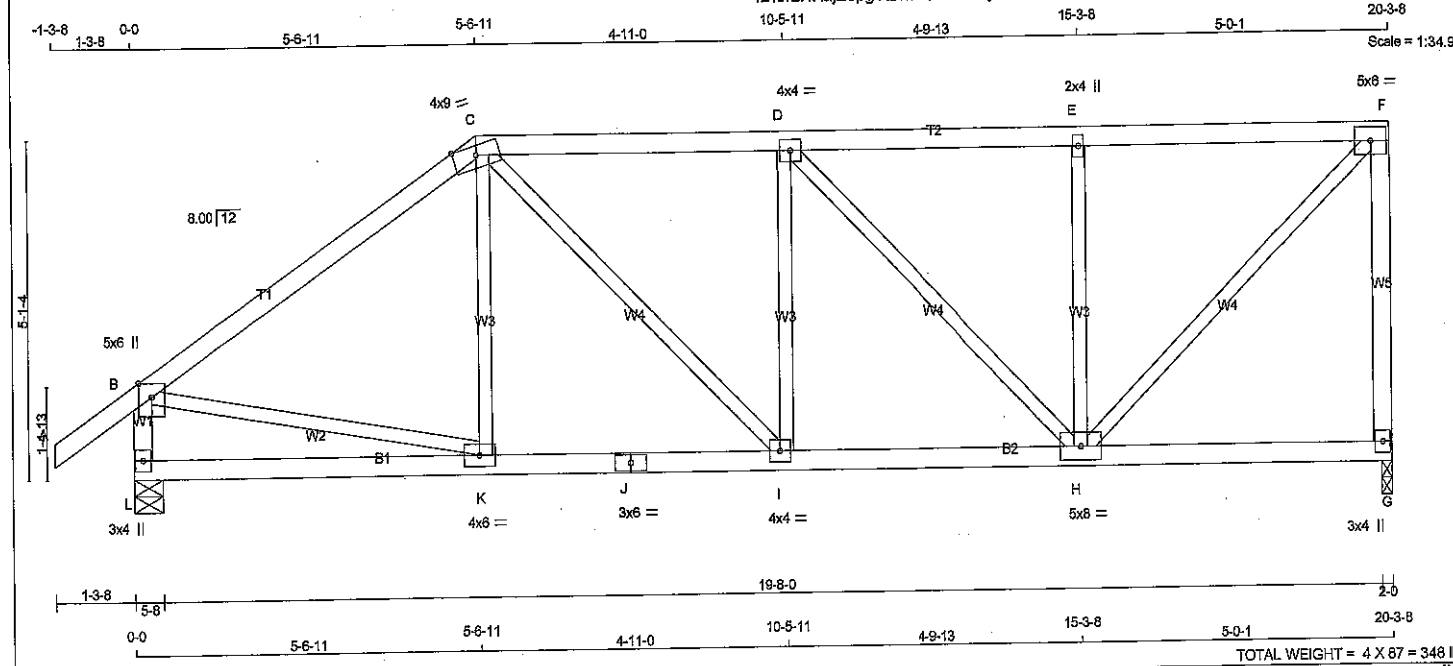
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Tamarack Roof Truss, Burlington				Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Thu Feb 1 10:50:29 2018 Page 2	
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HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 304.5 lbs FACTORED DOWN AT 4-0-11, 158.9 lbs FACTORED DOWN AT 5-0-12, AND 158.9 lbs FACTORED DOWN AT 7-0-12, AND 158.9 lbs FACTORED DOWN AT 9-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 83.6 lbs FACTORED DOWN AT 5-0-12, AND 83.6 lbs FACTORED DOWN AT 7-0-12, AND 83.6 lbs FACTORED DOWN AT 9-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



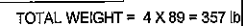
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DWG NO. TAM **2414**
 STRUCTURAL
 COMPONENT ONLY

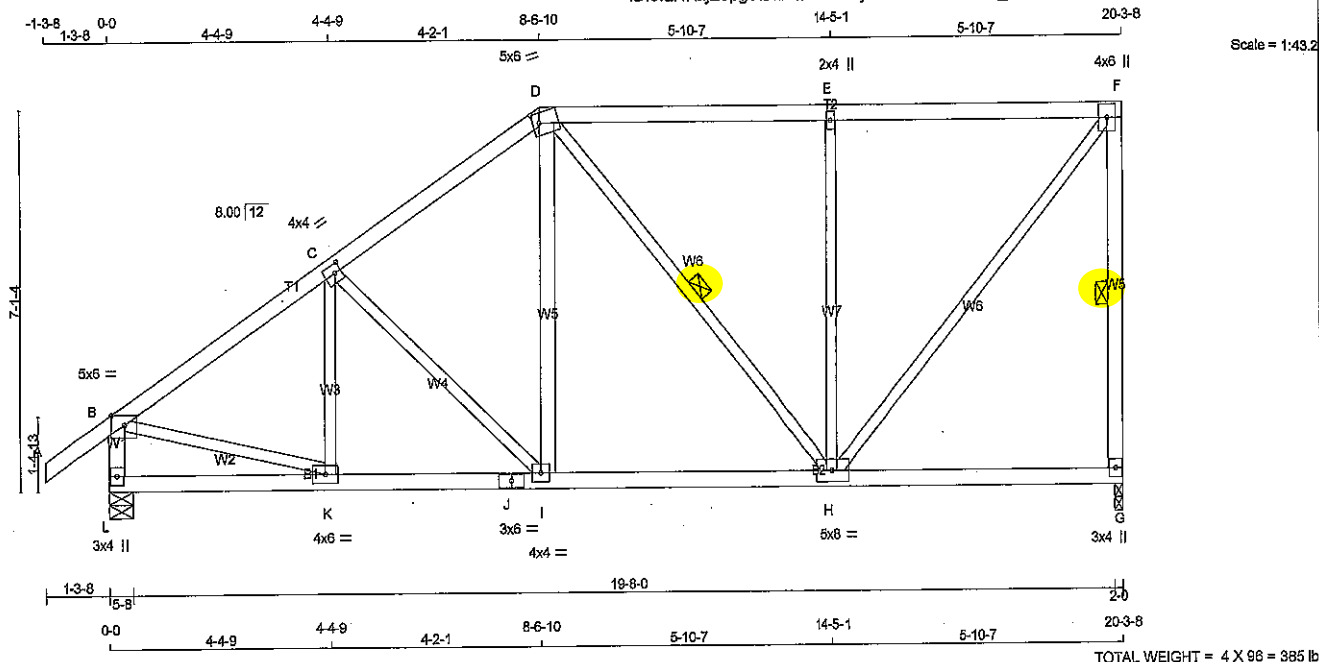


LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
CHORDS				FACTORED				TOP CH. LL = 32.5 PSF			
A - C	2x4	DRY	No.2	GROSS REACTION				DL = 3.0 PSF			
C - F	2x4	DRY	No.2	MAXIMUM FACTORED				BOT CH. LL = 10.5 PSF			
G - F	2x4	DRY	No.2	GROSS REACTION				DL = 7.0 PSF			
L - B	2x4	DRY	No.2	MAXIMUM FACTORED				TOTAL LOAD = 53.0 PSF			
L - J	2x4	DRY	No.2	GROSS REACTION				SPACING = 240 IN. C/C			
J - G	2x4	DRY	No.2	MAXIMUM FACTORED				LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
				GROSS REACTION				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
				MAXIMUM FACTORED				THIS DESIGN COMPLIES WITH:			
				GROSS REACTION				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				MAXIMUM FACTORED				- CSA 086-09			
				GROSS REACTION				- TPIC 2011			
				MAXIMUM FACTORED				(55% OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				GROSS REACTION				ALLOWABLE DEFL.(LL)= L/360 (0.68")			
				MAXIMUM FACTORED				CALCULATED VERT. DEFL.(LL)= L/899 (0.08")			
				GROSS REACTION				ALLOWABLE DEFL.(TL)= L/360 (0.68")			
				MAXIMUM FACTORED				CALCULATED VERT. DEFL.(TL)= L/899 (0.08")			
				GROSS REACTION				CS: TC=0.66/1.00 (B-C-1), BC=0.33/1.00 (I-K-2), WB=0.43/1.00 (D-H-1), SS=0.24/1.00 (E-F-1)			
				MAXIMUM FACTORED				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				GROSS REACTION				COMP=1.10 SHEAR=1.10 TENS= 1.10			
				MAXIMUM FACTORED				COMPANION LIVE LOAD FACTOR = 0.50			
				GROSS REACTION				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				MAXIMUM FACTORED				NAIL VALUES			
				GROSS REACTION				PLATE GRIP(DRY) SHEAR SECTION			
				MAXIMUM FACTORED				(PSI) (PLI) (PLI)			
				GROSS REACTION				MAX MIN MAX MIN MAX MIN			
				MAXIMUM FACTORED				MT20 618 354 1667 822 2284 1650			
				GROSS REACTION				PLATE PLACEMENT TOL. = 0.250 inches			
				MAXIMUM FACTORED				PLATE ROTATION TOL. = 5.0 Deg.			
				GROSS REACTION				CSI GRIP= 0.87 (C) (INPUT = 0.90)			
				MAXIMUM FACTORED				CSI METAL= 0.34 (J) (INPUT = 1.00)			
				GROSS REACTION				DWG NO. TAM 7415-18			
				MAXIMUM FACTORED				STRUCTURAL			
				GROSS REACTION				COMPONENT ONLY			

SITE COPY



DWG NO. TAM 7416-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	5.0	8.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
G	1349	0	0	2-0
L	1493	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
G	1075	659 / 0	213 / 0	0 / 0	203 / 0	0 / 0
L	1172	748 / 0	213 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-104.9 -104.9	0.14 (1)	10.00	K-C	-175 / 104	0.05 (1)
B-C	-1442 / 0	-104.9 -104.9	0.36 (1)	5.01	C-I	-328 / 0	0.19 (1)
C-D	-1214 / 0	-104.9 -104.9	0.35 (1)	5.37	I-D	0 / 373	0.06 (2)
D-E	-804 / 0	-104.9 -104.9	0.65 (1)	5.35	D-H	-134 / 0	0.07 (1)
E-F	-804 / 0	-104.9 -104.9	0.65 (1)	5.35	H-E	-763 / 0	0.67 (1)
G-F	-1282 / 0	0.0	0.29 (1)	5.69	H-F	0 / 1385	0.31 (1)
L-B	-1440 / 0	0.0	0.15 (1)	6.61	B-K	0 / 1280	0.28 (1)
L-K	0 / 0	-28.0	-28.0	0.12 (3)	10.00		
K-J	0 / 1224	-28.0	-28.0	0.27 (1)	10.00		
J-I	0 / 1224	-28.0	-28.0	0.27 (1)	10.00		
I-H	0 / 991	-28.0	-28.0	0.35 (2)	10.00		
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	DL	PSF
	32.5	3.0	PSF
BOT CH.	LL	DL	PSF
	10.5	7.0	PSF
TOTAL LOAD	53.0		PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.65/1.00 (E-F:1), BC=0.35/1.00 (H-I:2), WB=0.67/1.00 (E-H:1), SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
	PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1887	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

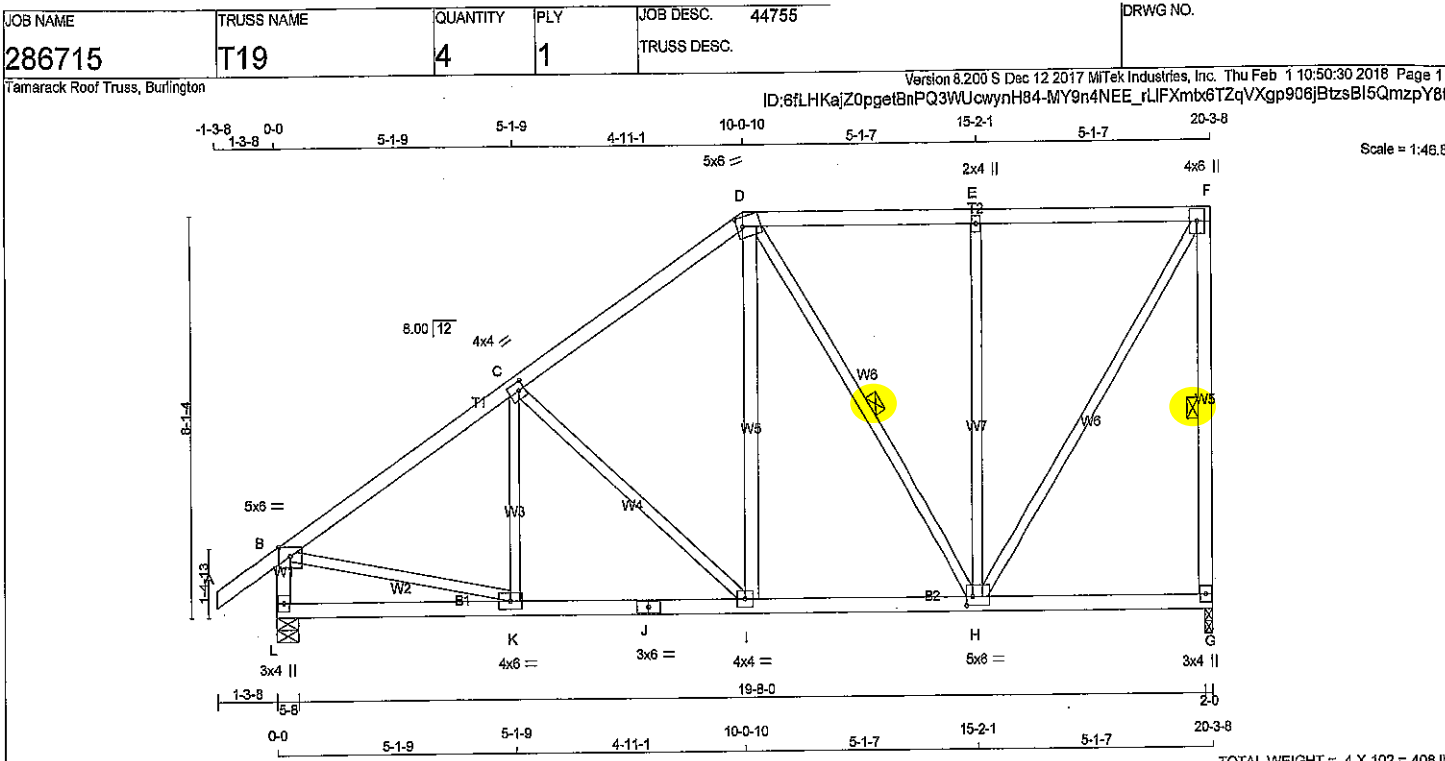
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)



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DRWG NO. TAM 2417-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

I - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMWW-w	MT20	2.0	4.0		
F	TMVW-p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0	2.25	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
VERT	HORZ	UP/LIFT	IN-SX
JT 1349	0	1349	0
G 1493	0	1493	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT 1075	659 / 0 213 / 0 0 / 0 0 / 0 203 / 0 0 / 0
L 1172	748 / 0 213 / 0 0 / 0 0 / 0 211 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO				FR-TO			
A-B	0 / 40	-104.9	-104.9 0.14 (1)	10.00	K-C	-102 / 174	0.04 (3)
B-C	-1448 / 0	-104.9	-104.9 0.52 (1)	4.77	C-I	-478 / 0	0.40 (1)
C-D	-1091 / 0	-104.9	-104.9 0.49 (1)	5.36	I-D	0 / 452	0.07 (1)
D-E	-711 / 0	-104.9	-104.9 0.47 (1)	6.25	D-H	-316 / 0	0.19 (1)
E-F	-711 / 0	-104.9	-104.9 0.47 (1)	6.25	H-E	-684 / 0	0.84 (1)
G-F	-1290 / 0	0.0	0.0 0.38 (1)	5.68	H-F	0 / 1268	0.29 (1)
L-B	-1433 / 0	0.0	0.0 0.15 (1)	6.82	B-K	0 / 1261	0.28 (1)
L-K	0 / 0	-28.0	-28.0 0.18 (3)	10.00			
K-J	0 / 1234	-28.0	-28.0 0.31 (2)	10.00			
J-I	0 / 1234	-28.0	-28.0 0.31 (2)	10.00			
I-H	0 / 885	-28.0	-28.0 0.27 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.52/1.00 (B-C:1), BC=0.31/1.00 (I-K:2), WB=0.84/1.00 (E-H:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

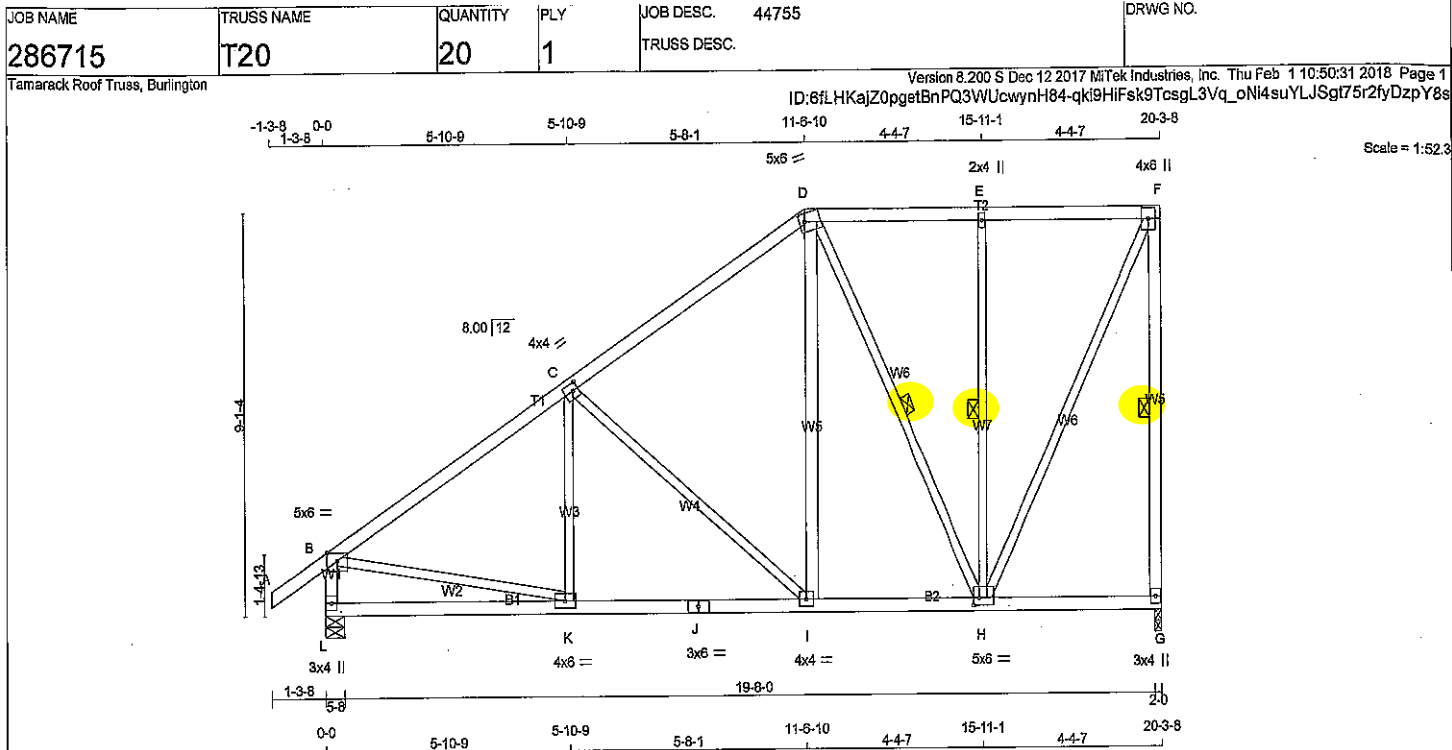
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)



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DWG NO. TAM 7418-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 20 X 108 = 2161 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMVV-w	MT20	2.0	4.0		
F	TMVV+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UP		
G	1349	0	1349	0	2-0	2-0
L	1493	0	1493	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND PERM. LIVE	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
G	1075	659 / 0	213 / 0	0 / 0	203 / 0	0 / 0	0 / 0
L	1172	748 / 0	213 / 0	0 / 0	211 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	K-C	-39 / 237
B-C	-1438 / 0	-104.9	-104.9	0.50 (1)	4.84	C-I	-619 / 0
C-D	-957 / 0	-104.9	-104.9	0.47 (1)	5.70	I-D	0 / 540
D-E	-556 / 0	-104.9	-104.9	0.25 (1)	6.25	D-H	-481 / 0
E-F	-556 / 0	-104.9	-104.9	0.25 (1)	6.25	H-E	-564 / 0
F-G	-1267 / 0	0.0	0.0	0.50 (1)	5.67	H-F	0 / 1251
L-B	-1426 / 0	0.0	0.0	0.15 (1)	6.84	B-K	0 / 1251
L-K	0 / 0	-28.0	-28.0	0.24 (3)	10.00		
K-J	0 / 1230	-28.0	-28.0	0.37 (2)	10.00		
J-I	0 / 1230	-28.0	-28.0	0.37 (2)	10.00		
I-H	0 / 770	-28.0	-28.0	0.21 (2)	10.00		
H-G	0 / 0	-28.0	-28.0	0.12 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD = 53.0 PSF				

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.69")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.69")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.06")

CSI: TC=0.50/1.00 (F-G:1), BC=0.37/1.00 (I-K:2), WB=0.73/1.00 (C-I:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

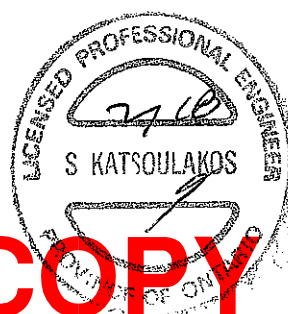
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

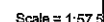
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)



DWG NO. TAM 7419-18
STRUCTURAL
COMPONENT ONLY

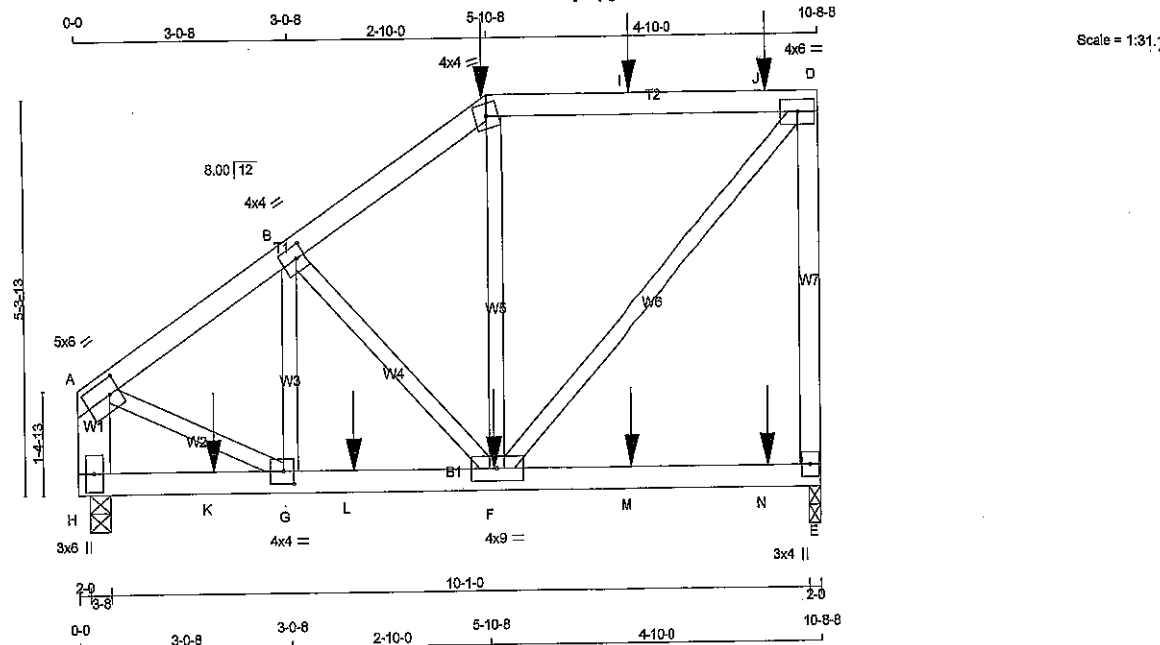
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TOTAL WEIGHT = $4 \times 110 = 440$ lb

DWG NO. TAM 7420-18
STRUCTURAL
COMPONENT ONLY

SITE COPY



TOTAL WEIGHT = 2 X 50 = 100 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - A	2x6	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	6.0	2.50 1.75
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	6.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMVWV-t	MT20	4.0	9.0	
G	BMVWV-t	MT20	4.0	4.0	2.00 1.75
H	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 550.0 lbs FACTORED DOWN AT 5-10-8, AND 126.3 lbs FACTORED DOWN AT 7-11-4, AND 138.3 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 81.5 lbs FACTORED DOWN AT 1-11-4, 71.7 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, AND 71.7 lbs FACTORED DOWN AT 7-11-4, AND 77.4 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	DOWN	DOWN	BRG	BRG
E	1356	0	1356	0	2-0
H	1109	0	1109	0	3-8

UNFACTORED REACTIONS		1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	1075	668 / 0	207 / 0	0 / 0	0 / 0	200 / 0	0 / 0	
H	894	532 / 0	168 / 0	0 / 0	0 / 0	174 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
MEMB.		(LBS)		(PLF)		CS1 (LC)	UNBRAC		(LBS)		CS1 (L.C)
FR-TO							LENGTH FR-TO				
A-B	-1072 / 0		-104.9	-104.9	0.18 (1)	5.85	G-B	-183 / 84	0.04 (1)		
B-C	-974 / 0		-104.9	-104.9	0.17 (1)	6.07	B-F	-169 / 0	0.05 (1)		
C-I	-782 / 0		-104.9	-104.9	0.78 (1)	5.10	F-C	-481 / 119	0.21 (1)		
I-J	-782 / 0		-104.9	-104.9	0.78 (1)	5.10	F-D	0 / 1141	0.28 (1)		
J-D	-782 / 0		-104.9	-104.9	0.78 (1)	5.10	A-G	0 / 984	0.24 (1)		
E-D	-1252 / 0		0.0	0.0	0.63 (1)	7.15					
H-A	-1053 / 0		0.0	0.0	0.08 (1)	7.81					
H-K	0 / 0		-28.0	-28.0	0.12 (2)	10.00					
K-G	0 / 0		-28.0	-28.0	0.12 (2)	10.00					
G-L	0 / 908		-28.0	-28.0	0.34 (2)	10.00					
L-F	0 / 908		-28.0	-28.0	0.34 (2)	10.00					
F-M	0 / 0		-28.0	-28.0	0.24 (2)	10.00					
M-N	0 / 0		-28.0	-28.0	0.24 (2)	10.00					
N-E	0 / 0		-28.0	-28.0	0.24 (2)	10.00					

FACTORED CONCENTRATED LOADS (LBS)		JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-550	-550				FRONT	VERT	TOTAL
F	5-11-4	-41	-72				FRONT	VERT	TOTAL
I	7-11-4	-126	-126				FRONT	VERT	TOTAL
J	9-11-4	-138	-138				FRONT	VERT	TOTAL
K	1-11-4	-59	-81				FRONT	VERT	TOTAL
L	3-11-4	-41	-72				FRONT	VERT	TOTAL
M	7-11-4	-41	-72				FRONT	VERT	TOTAL
N	9-11-4	-44	-77				FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")

CALCULATED VERT. DEFL.(LL) = L/699 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.36")

CALCULATED VERT. DEFL.(TL) = L/699 (0.07")

CSI: TC=0.78/1.00 (C-D:1), BC=0.34/1.00 (F-G:2), WB=0.28/1.00 (D-F:1), SS=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 818 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)

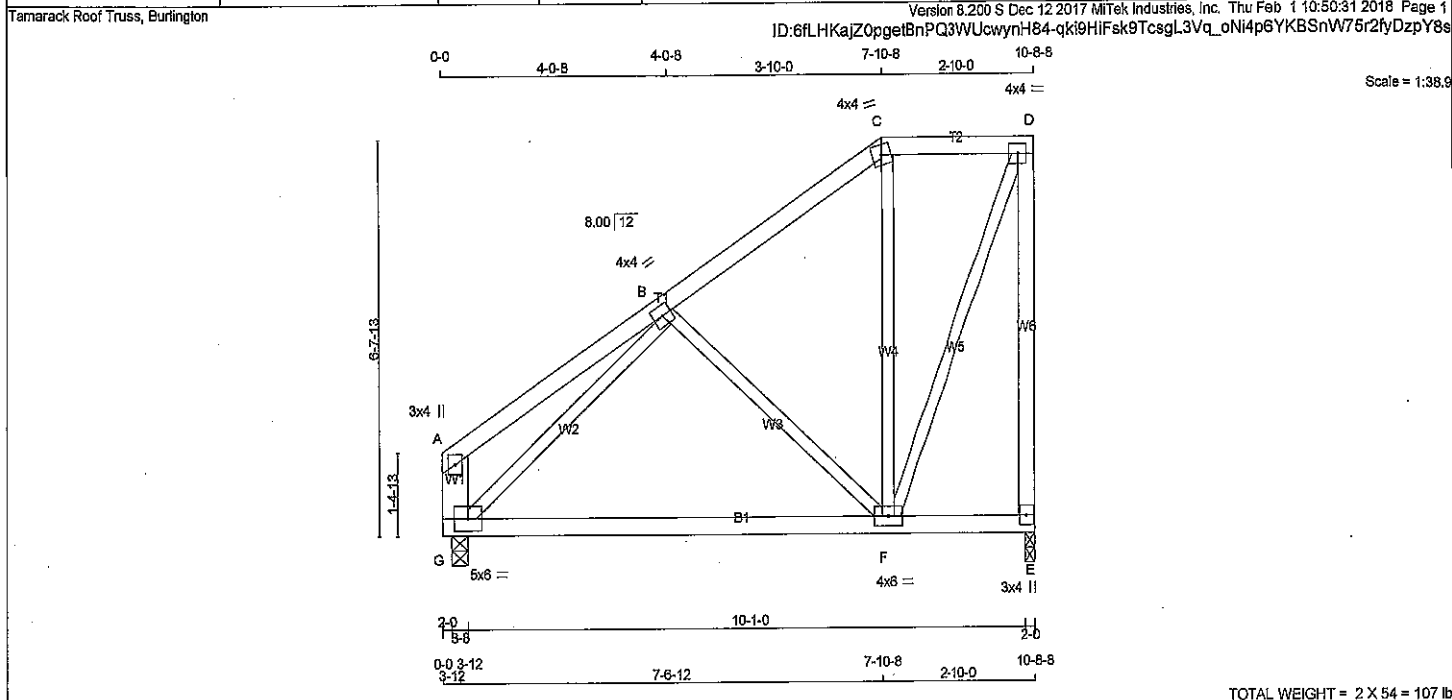
JSI METAL= 0.34 (G) (INPUT = 1.00)



SITE COPY

DRWG NO. TAM 7421-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
286715	T23	2	1	TRUSS DESC.		



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY; SEASONED LUMBER.				

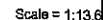
PLATES (table is in inches)	PLATES	W	LEN	Y	X
A - TMV+p	MT20	3.0	4.0		
B - TMVW-t	MT20	4.0	4.0		
C - TTW-m	MT20	4.0	4.0		
D - TMVW-t	MT20	4.0	4.0		
E - BMV1+p	MT20	3.0	4.0		
F - BMVWW-t	MT20	4.0	6.0		
G - BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG					
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
E	712	0	712	0	0	2-0	2-0		
G	712	0	712	0	0	3-8	3-8		
UNFACTORED REACTIONS									
JT	1ST LOASE	MAX./MIN.	COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
E	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0		
G	567	348 / 0	112 / 0	0 / 0	0 / 0	107 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH		
FR-TO					FR-TO				
A-B	0 / 28	-104.9	-104.9	0.27 (1)	10.00	B-F	-324 / 2	0.16 (1)	
B-C	-333 / 0	-104.9	-104.9	0.21 (1)	6.25	F-C	-138 / 72	0.10 (1)	
C-D	-251 / 0	-104.9	-104.9	0.11 (1)	6.25	F-D	0 / 637	0.14 (1)	
E-D	-733 / 0	0.0	0.0	0.68 (1)	7.81	G-B	-676 / 0	0.30 (1)	
G-A	-157 / 0	0.0	0.0	0.01 (1)	7.81				
G-F	0 / 492	-28.0	-28.0	0.44 (2)	10.00				
F-E	0 / 0	-28.0	-28.0	0.40 (3)	10.00				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011
(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL)= L/350 (0.36")
CALCULATED VERT. DEFL.(LL) = L/863 (0.15")
ALLOWABLE DEFL.(TL)= L/350 (0.36")
CALCULATED VERT. DEFL.(TL) = L/519 (0.25")
CSI: TC=0.68/1.00 (D-E:1), BC=0.44/1.00 (F-G:2), WB=0.30/1.00 (B-G:1), SS=0.18/1.00 (F-G:3)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10
COMPANION LIVE LOAD FACTOR = 0.50
AUTOSOLVE RIGHT HEEL ONLY
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1957 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)
DWG NO. TAM 7422-18
STRUCTURAL COMPONENT ONLY



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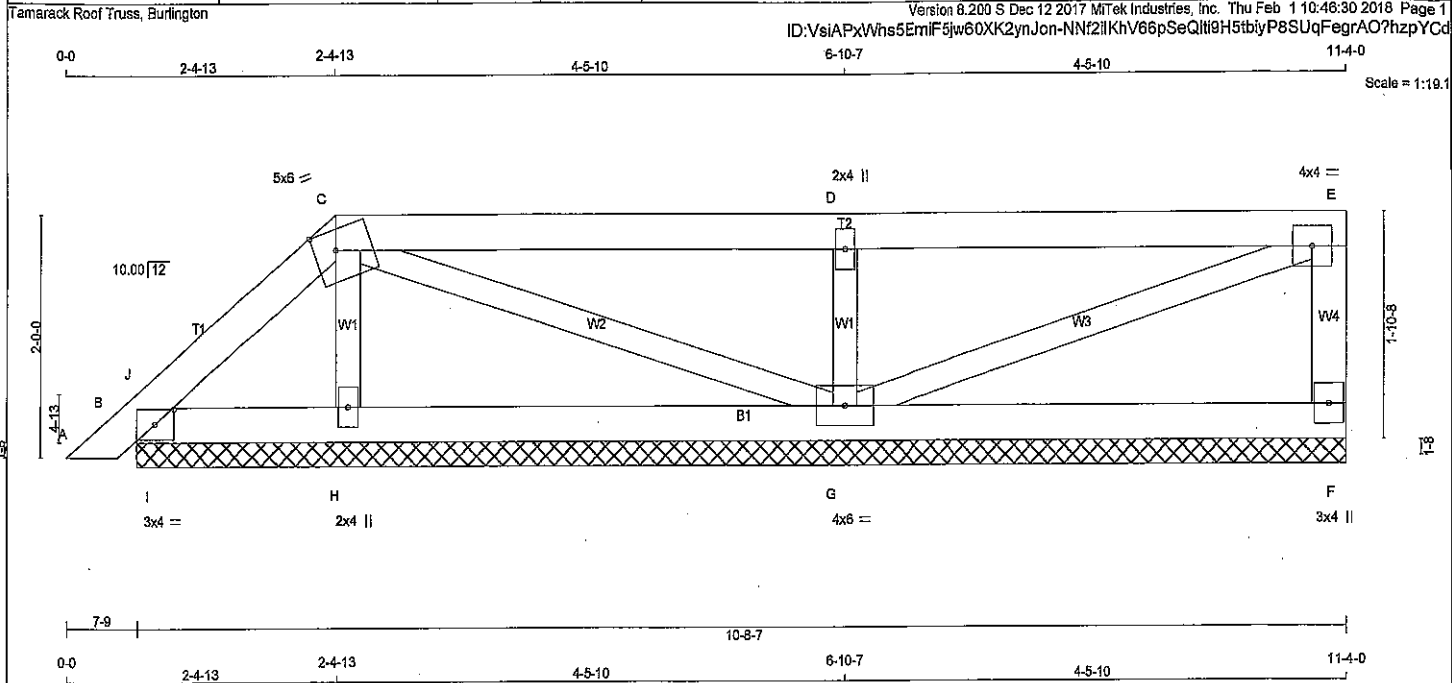


TOTAL WEIGHT = $12 \times 17 = 201$ lb

DWG NO. TAM 7423-18
STRUCTURAL
COMPONENT ONLY



SITE COPY



TOTAL WEIGHT = 4 X 37 = 146 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 F - E 2x4 DRY No.2 SPF
 B - F 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM51-H	MT20	3.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMW-w	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
F	225	0	225	0	10-8-7	10-8-7
B	197	0	197	0	10-8-7	10-8-7
H	318	0	318	0	10-8-7	10-8-7
G	740	0	740	0	10-8-7	10-8-7

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	180	109/0	37/0	0/0	0/0	34/0	0/0
B	139	115/0	8/0	0/0	0/0	16/0	0/0
H	270	137/0	72/0	0/0	0/0	81/0	0/0
G	583	368/0	108/0	0/0	0/0	106/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/16	-104.9	-104.9	0.02 (1)	10.00	H-C	-202/0	0.03 (1)	
B-J	-36/5	-104.9	-104.9	0.01 (1)	8.25	C-G	-33/0	0.01 (1)	
J-C	-61/0	-104.9	-104.9	0.04 (1)	8.25	G-D	-586/0	0.09 (1)	
C-D	0/1	-104.9	-104.9	0.36 (1)	10.00	E-G	0/0	0.00 (1)	
D-E	0/0	-104.9	-104.9	0.35 (1)	10.00	I-J	-128/0	0.00 (1)	
F-E	-176/0	0.0	0.0	0.02 (1)	7.81				
B-I	0/44	-28.0	-28.0	0.05 (1)	10.00				
I-H	0/44	-28.0	-28.0	0.07 (2)	10.00				
H-G	0/30	-28.0	-28.0	0.15 (2)	10.00				
G-F	0/0	-28.0	-28.0	0.15 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 32.5 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.36/1.00 (C-D:1), BC=0.15/1.00 (G-H:2), WB=0.09/1.00 (D-G:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.80)
 JSI METAL= 0.11 (D) (INPUT = 1.00)



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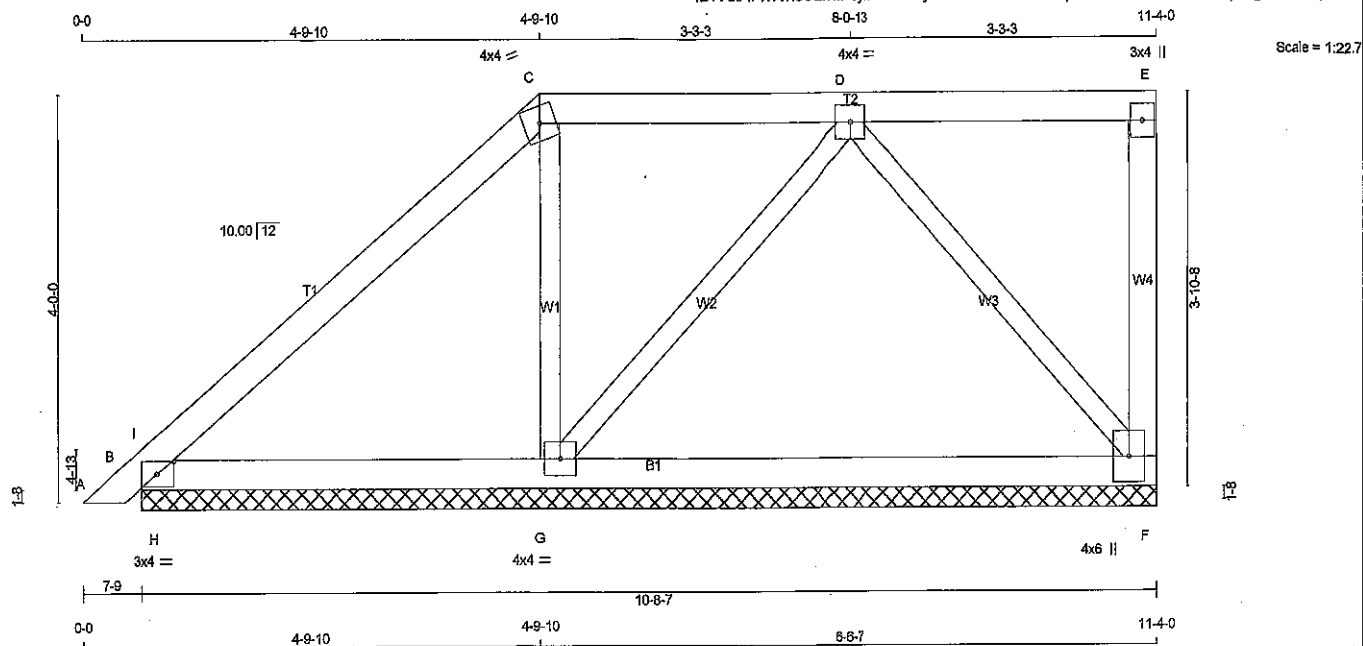
DRWG NO. TAM 7406-8
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 286703	TRUSS NAME P3	QUANTITY 4	PLY 1	JOB DESC. 44755 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Thu Feb 1 10:46:30 2018 Page 1

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TOTAL WEIGHT = 4 X 40 = 162 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TM31-H	MT20	3.0	4.0	1.50	2.00	
C	TTV-m	MT20	4.0	4.0			
D	TMVW-t	MT20	4.0	4.0			
E	TMV+p	MT20	3.0	4.0			
F	BMVW1+p	MT20	4.0	6.0			
G	BMVW1-H	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
F	447	0	447	0	0	10-8-7
B	370	0	370	0	0	10-8-7
G	662	0	662	0	0	10-8-7

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	345	231 / 0	55 / 0	0 / 0	0 / 0	58 / 0	0 / 0
B	271	207 / 0	28 / 0	0 / 0	0 / 0	37 / 0	0 / 0
G	557	292 / 0	143 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO					FR-TO		
A-B	0 / 16	-104.9	-104.9	0.02 (1)	G-C	-248 / 0	0.06 (1)
B-I	-12 / 149	-104.9	-104.9	0.14 (1)	G-D	-235 / 0	0.09 (1)
I-C	-117 / 0	-104.9	-104.9	0.22 (1)	D-F	-337 / 0	0.13 (1)
C-D	-71 / 0	-104.9	-104.9	0.18 (1)	H-I	-580 / 0	0.00 (1)
D-E	0 / 0	-104.9	-104.9	0.18 (1)			
F-E	-130 / 0	0.0	0.0	0.03 (1)			
B-H	0 / 79	-28.0	-28.0	0.20 (1)			
H-G	0 / 79	-28.0	-28.0	0.28 (2)			
G-F	0 / 227	-28.0	-28.0	0.30 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 32.5	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 53.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-I:1), BC=0.30/1.00 (F-G:2), WB=0.13/1.00 (D-F:1), SS=0.45/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354 1667 822 2284 1656	

PLATE PLACEMENT TOL = 0.250 Inches

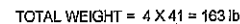
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)



SITE COPY

DRWG NO. TAM7408-18
STRUCTURAL
COMPONENT ONLY



PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	MTB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-I	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW1-I	MT20	4.0	4.0		

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX.	UNBRAC	MEMB.	FORCE (LBS)	LC1 MAX	LC1 MAX.
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	0/16	-104.9	-104.9	0.02 (1)	10.00	F-C	-424/0	0.15 (1)	
B-H	-140/265	-104.9	-104.9	0.24 (1)	6.25	F-D	0/142	0.03 (1)	
H-C	-170/0	-104.9	-104.9	0.38 (1)	6.25	G-H	-871/129	0.00 (1)	
C-D	-107/0	-104.9	-104.9	0.51 (1)	6.25				
E-D	-373/0	0.0	0.0	0.15 (1)	7.81				
B-G	0/116	-28.0	-28.0	0.31 (1)	10.00				
G-F	0/116	-28.0	-28.0	0.31 (1)	10.00				
F-E	0/0	-28.0	-28.0	0.25 (2)	10.00				

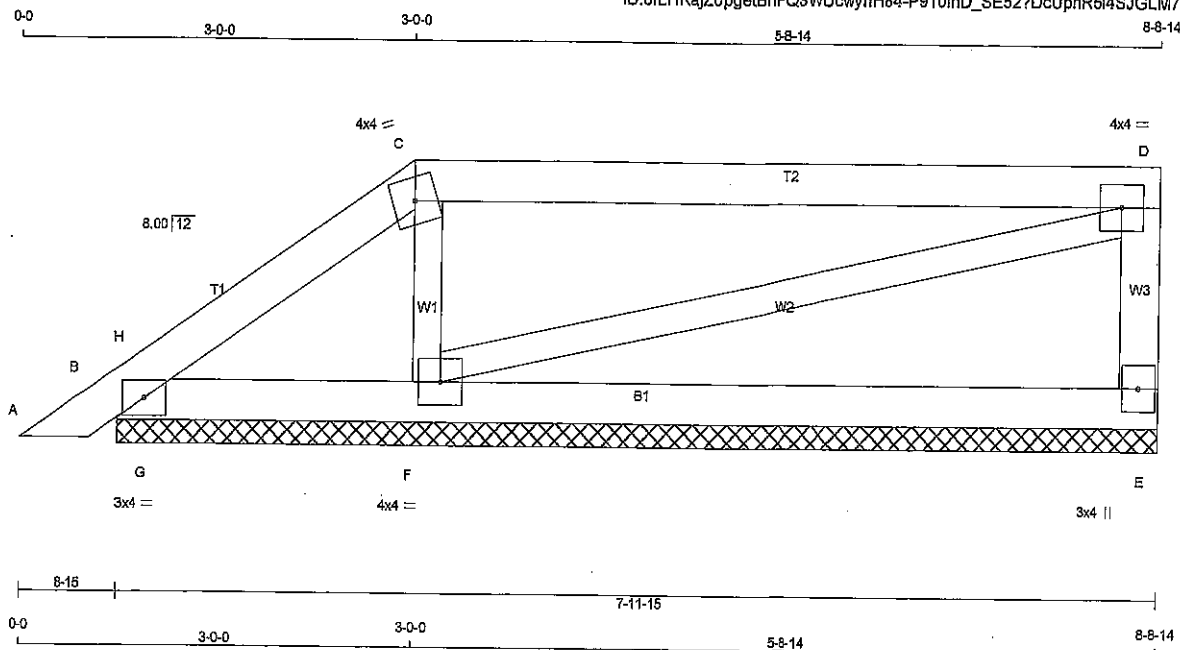
JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 7409-18
STRUCTURAL
COMPONENT ONLY

Scale = 1:16.6



TOTAL WEIGHT = $4 \times 27 = 107 \text{ lb}$

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
B - E	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	VERT	HORZ	DOWN	HORZ	BRG	BRG
JT	368	0	368	0	7-11-15	7-11-15
E	211	0	211	0	7-11-15	7-11-15
B	553	0	553	0	7-11-15	7-11-15

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	285	189 / 0	47 / 0	0 / 0	0 / 0	49 / 0	0 / 0
B	140	134 / 0	0 / 4	0 / 0	0 / 0	10 / 0	0 / 0
F	469	239 / 0	125 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. B. F.

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT' OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/18	-104.9	-104.9	0.03 (1)	10.00	F-C	-364/0	0.05 (1)	
B-H	-3/50	-104.9	-104.9	0.04 (2)	10.00	F-D	-4/17	0.00 (1)	
H-C	-58/0	-104.9	-104.9	0.05 (1)	6.25	G-H	-204/0	0.00 (1)	
C-D	-17/4	-104.9	-104.9	0.59 (1)	6.25				
E-D	-306/0	0.0	0.0	0.03 (1)	7.81				
B-G	-3/41	-28.0	-28.0	0.08 (1)	10.00				
G-F	-3/41	-28.0	-28.0	0.21 (2)	10.00				
F-E	0/0	-28.0	-28.0	0.21 (2)	10.00				

SPECIFIED LOADS:			
TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		=	53.0 PSF

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.59/1.00 (C-D:1), BC=0.21/1.00 (F-G:2),
WB=0.05/1.00 (C-F:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

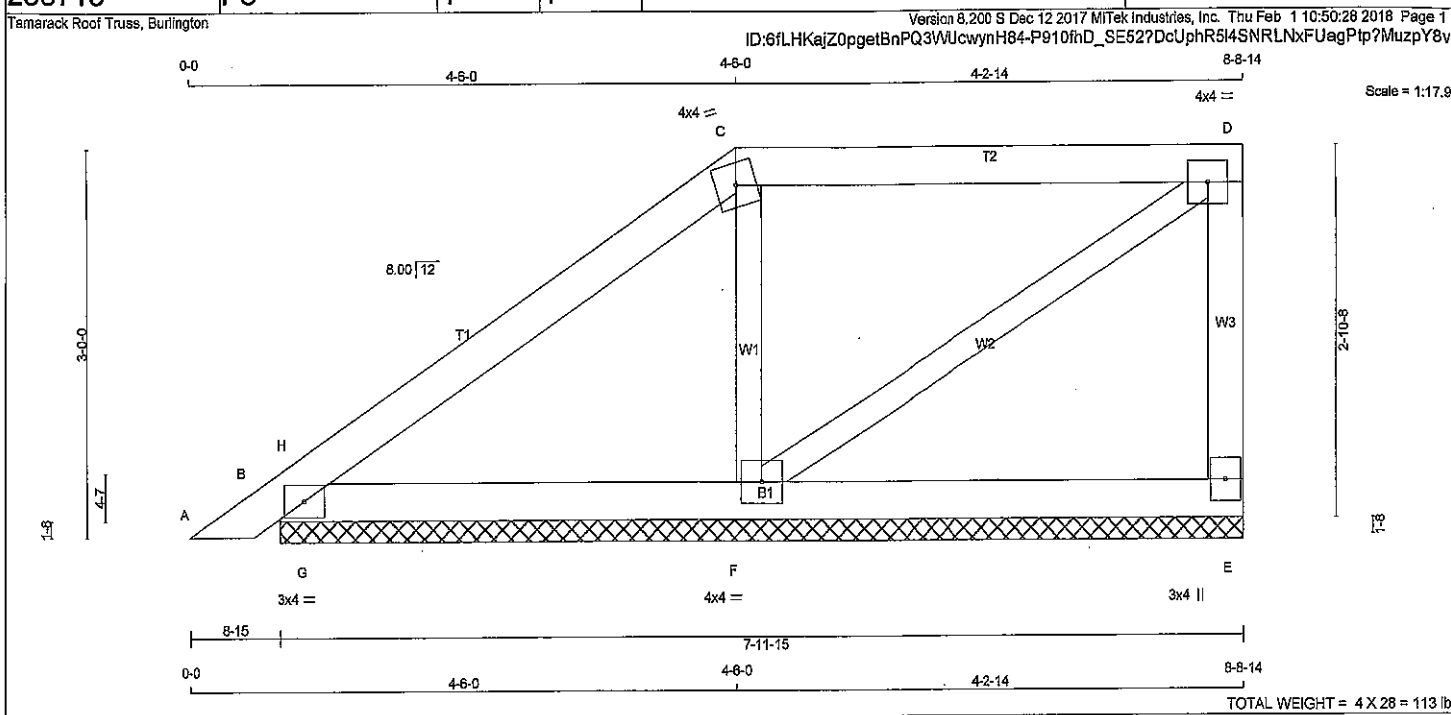
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)



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DWG NO. TAM 7424-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TMVW-t	MT20	4.0	4.0
E	BMV1+p	MT20	3.0	4.0
F	BMWW1-I	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
BEARINGS						
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD		
	GROSS REACTION	GROSS REACTION	BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
E	291	0	291	0	0	7-11-15
B	343	0	343	0	0	7-11-15
F	498	0	498	0	0	7-11-15

UNFACTORED REACTIONS						
1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	224	150 / 0	36 / 0	0 / 0	0 / 0	38 / 0
B	257	186 / 0	33 / 0	0 / 0	0 / 0	39 / 0
F	413	226 / 0	99 / 0	0 / 0	0 / 0	87 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	
FR-TO		FROM	TO				
A-B	0 / 18	-104.9	-104.9	0.03 (1)	10.00	F-C	-348 / 0
B-H	-52 / 18	-104.9	-104.9	0.07 (1)	6.25	F-D	0 / 58
H-C	-84 / 0	-104.9	-104.9	0.18 (1)	6.25	G-H	-357 / 50
C-D	-49 / 0	-104.9	-104.9	0.32 (1)	6.25		
E-D	-253 / 0	0.0	0.0	0.04 (1)	7.81		
B-G	0 / 63	-28.0	-28.0	0.16 (1)	10.00		
G-F	0 / 63	-28.0	-28.0	0.16 (1)	10.00		
F-E	0 / 0	-28.0	-28.0	0.14 (2)	10.00		

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	32.5 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	53.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1), BC=0.16/1.00 (B-G:1), WB=0.08/1.00 (C-F:1), SSI=0.28/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES							
PLATE	GRIP(DRY)	SHEAR	SECTION				
	(PSI)	(PLI)	(PLI)				
	MAX	MIN	MAX	MIN	MAX	MIN	
MT20	618	354	1867	822	2284	1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

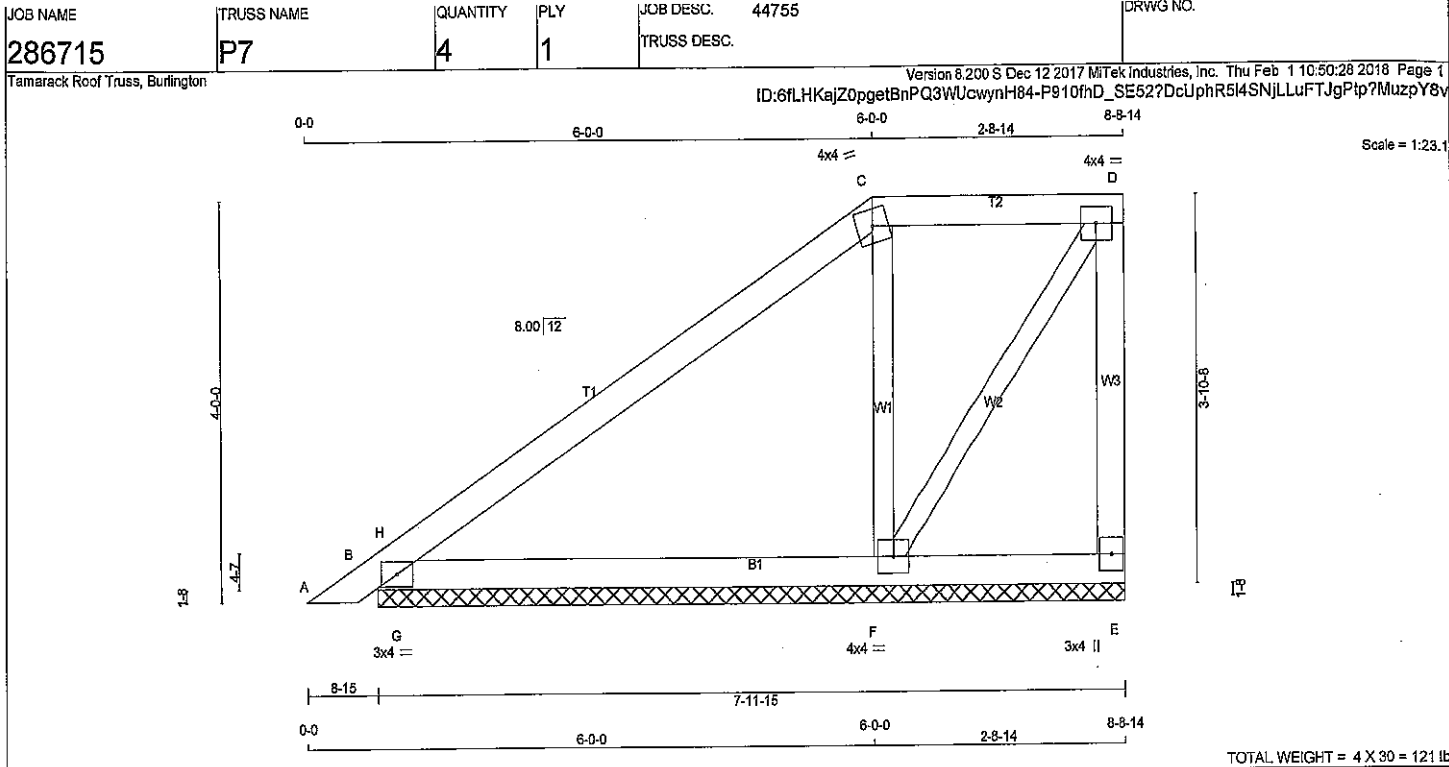
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DWG NO. TAM 7425-18
STRUCTURAL
COMPONENT ONLY



SITE COPY

DWG NO. TAM 7425-08
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMBVW-I	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	219	0	219	0	7-11-15	7-11-15
B	447	0	447	0	7-11-15	7-11-15
F	466	0	466	0	7-11-15	7-11-15

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	163	119 / 0	20 / 0	0 / 0	0 / 0	24 / 0	0 / 0
B	343	233 / 0	53 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	388	210 / 0	95 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 18	-104.9 -104.9	0.03 (1)	10.00	F-C	-329 / 0	0.08 (1)
B-H	-128 / 138	-104.9 -104.9	0.18 (1)	6.25	F-D	0 / 115	0.03 (1)
H-C	-108 / 0	-104.9 -104.9	0.37 (1)	8.25	G-H	-660 / 146	0.00 (1)
C-D	-68 / 0	-104.9 -104.9	0.13 (1)	8.25			
E-D	-236 / 0	0.0 0.0	0.05 (1)	7.81			
B-G	0 / 78	-28.0 -28.0	0.29 (1)	10.00			
G-F	0 / 78	-28.0 -28.0	0.29 (1)	10.00			
F-E	0 / 0	-28.0 -28.0	0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-H:1), BC=0.29/1.00 (F-G:1), WB=0.08/1.00 (C-F:1), SSI=0.52/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

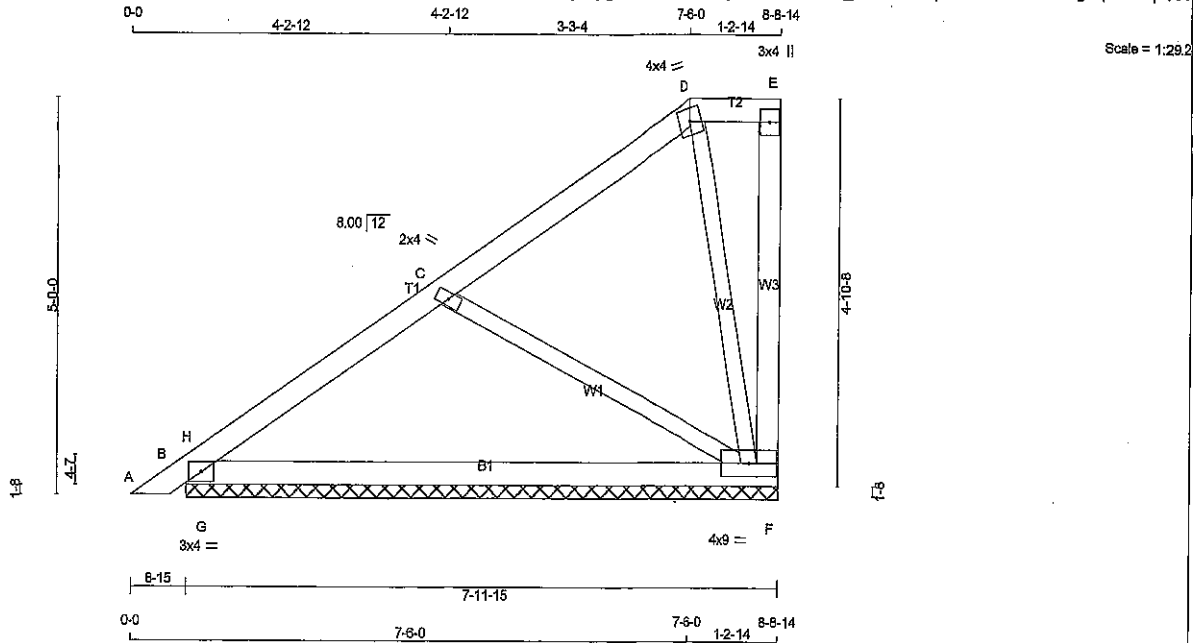


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DWG NO. TAM 7426-08
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Thu Feb 1 10:50:28 2018 Page 1
ID:6fLHKajZ0pgetBnQ3WUcwynH84-P910thD_SE52?DcUphR5I4SNoLJEFSfgPtp?MuzpY8v



TOTAL WEIGHT = 4 X 33 = 132 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS				SPF	
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVWW1-I	MT20	4.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
	VERT	HORZ	DOWN	HORZ		
F	531	0	531	0	7-11-15	7-11-15
B	601	0	601	0	7-11-15	7-11-15

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	423	260 / 0	84 / 0	0 / 0	0 / 0	80 / 0	0 / 0
B	470	303 / 0	84 / 0	0 / 0	0 / 0	84 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 18	-104.9	-104.9 0.03 (1)	10.00	C-F	-489 / 0	0.18 (1)
B-H	-899 / 0	-104.9	-104.9 0.38 (3)	5.98	D-F	-188 / 7	0.06 (1)
H-C	-499 / 0	-104.9	-104.9 0.36 (3)	6.25	G-H	0 / 981	0.00 (1)
C-D	-51 / 0	-104.9	-104.9 0.23 (1)	6.25			
D-E	0 / 0	-104.9	-104.9 0.03 (1)	10.00			
F-E	-65 / 0	0.0	0.0 0.03 (1)	7.81			
B-G	0 / 462	-28.0	-28.0 0.34 (2)	10.00			
G-F	0 / 462	-28.0	-28.0 0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 49.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.36/1.00 (B-H:3), BC=0.40/1.00 (F-G:2), WB=0.18/1.00 (C-F:1), SS1=0.60/1.00 (B-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

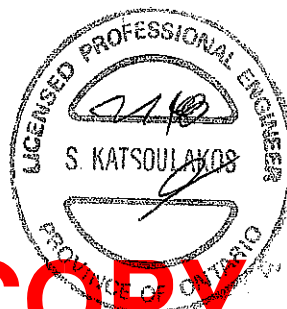
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)



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DWG NO. TAM 742-18
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



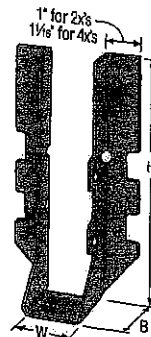
Double-Shear Nailing Top View



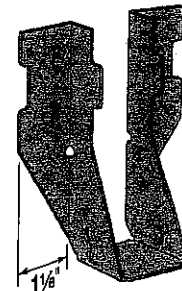
Double-Shear Nailing Side View; Do not bend tab



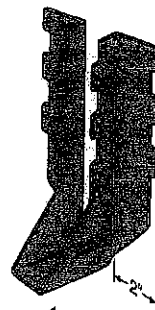
Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580



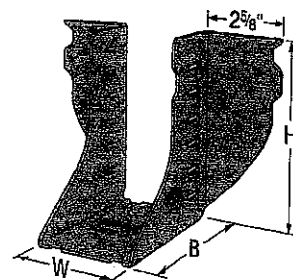
✓ LUS28



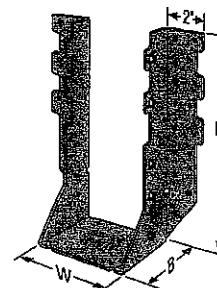
LU26L



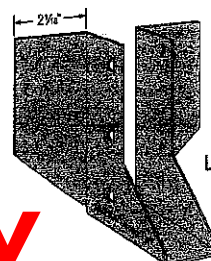
✓ HUS210
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2

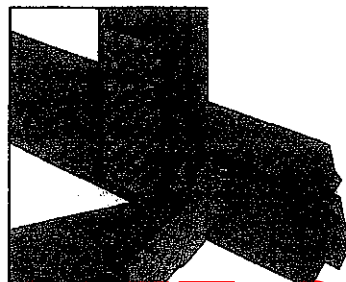


✓ HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



Plated Truss Connectors

EXACT COPY

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

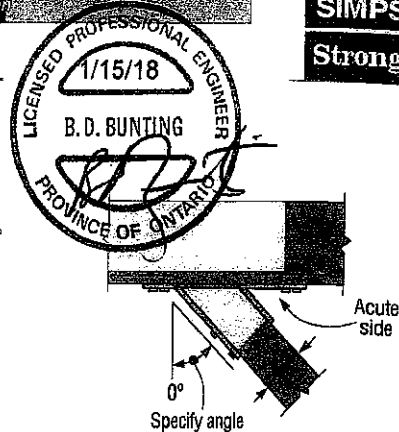
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.86 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _p ²	Header	Joist	D-F-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K ₀ = 1.15)	(K ₀ = 1.00)	(K ₀ = 1.15)	(K ₀ = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								316	723	287	514
LU24L	22	1 1/8	3	1 1/8	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								160	454	142	322
LU26L	22	1 1/8	5	1 1/8	4 1/8	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								320	712	287	507
SS LUS26	18	1 1/8	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								632	935	574	725
HUS26	16	1 1/8	5 3/8	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
								1240	2187	920	1724
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(18) 16d	(6) 16d	2055	4265	1460	4115
								914	1837	649	1831
HGUS26	12	1 1/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685	6625	2685	5700
								1196	2957	1196	2535
LU28L	20	1 1/8	6 1/8	1 1/8	5 1/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								507	977	454	689
SS LUS28	18	1 1/8	6 1/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								632	1127	574	796
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
								1604	2388	1190	1933
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
								1474	3419	1474	3073
LU210L	20	1 1/8	8	1 1/8	7 1/8	(18) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								507	1170	454	787
SS LUS210	18	1 1/8	7 1/8	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2765	1290	2210
								632	1239	574	983

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

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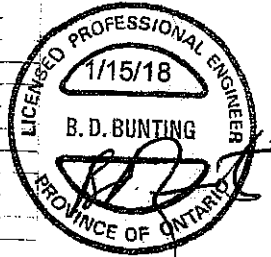
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

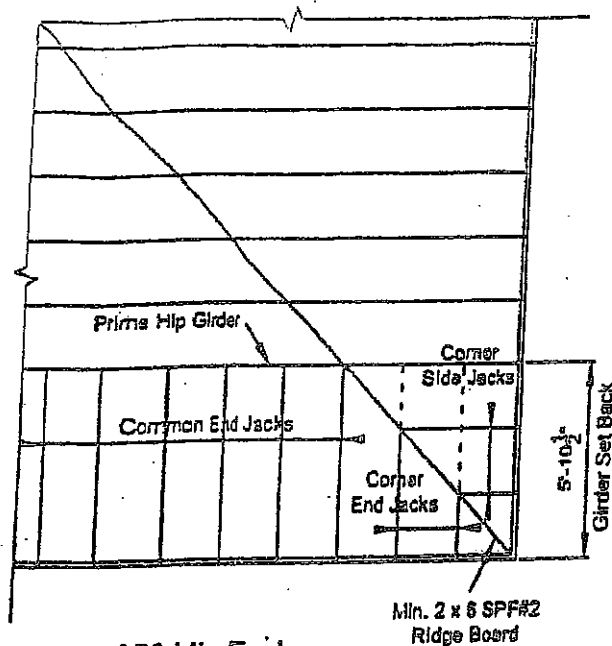
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _s ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
SS LUS28-2	18	3 1/8	7 1/8	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
SS LUS210-2	18	3 1/8	9 1/8	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-3	14	4 1/8	9 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	9660	4235	6865
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10400
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
SS HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
HGUS410	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

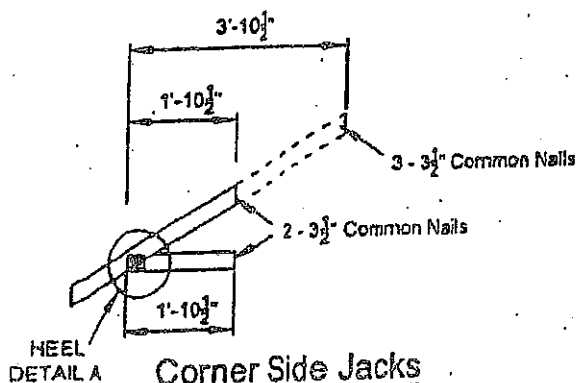
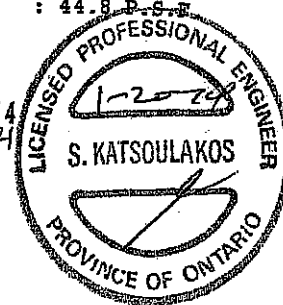
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

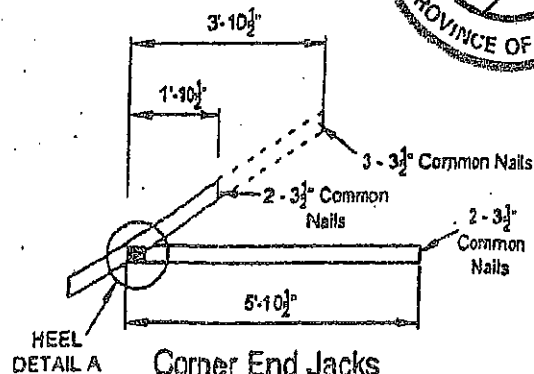
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

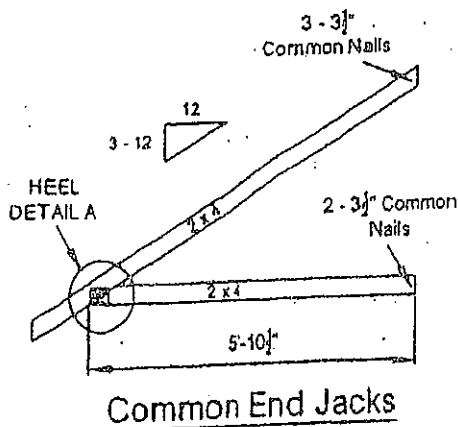
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



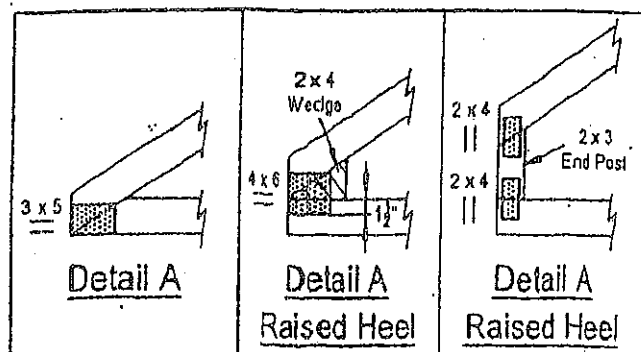
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATE DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

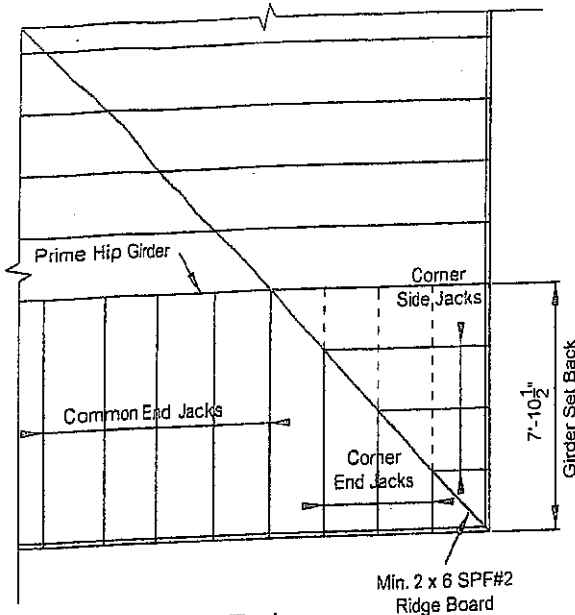
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

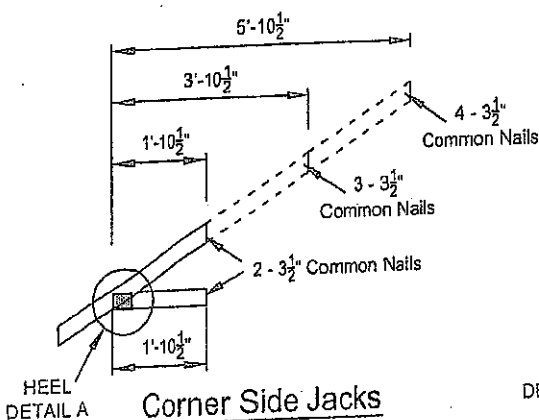
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.

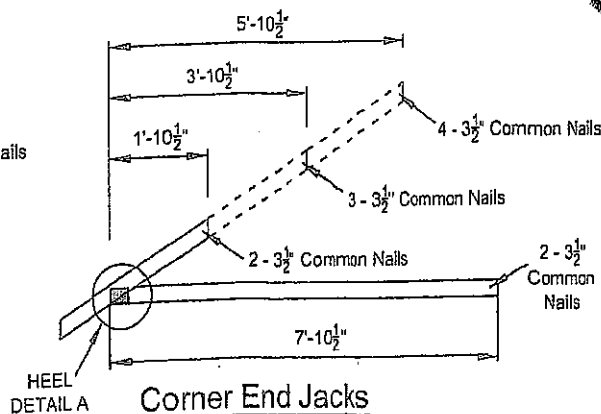


45° Hip End

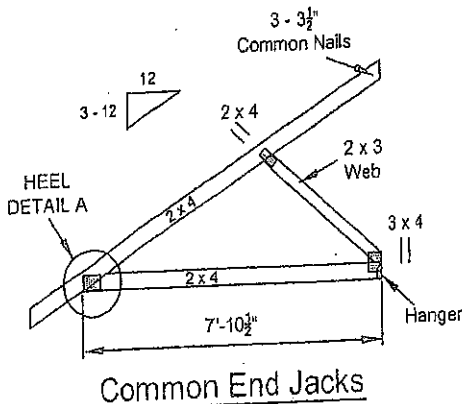
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STRUCTURAL
COMPONENT ONLY



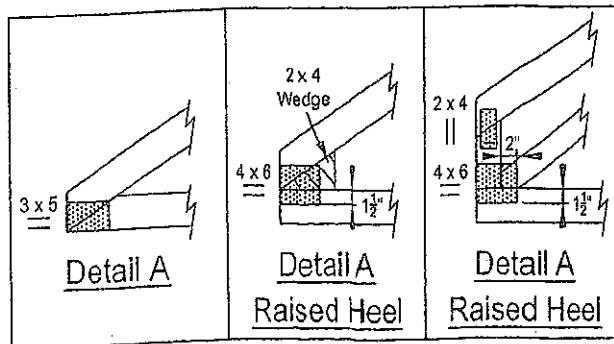
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

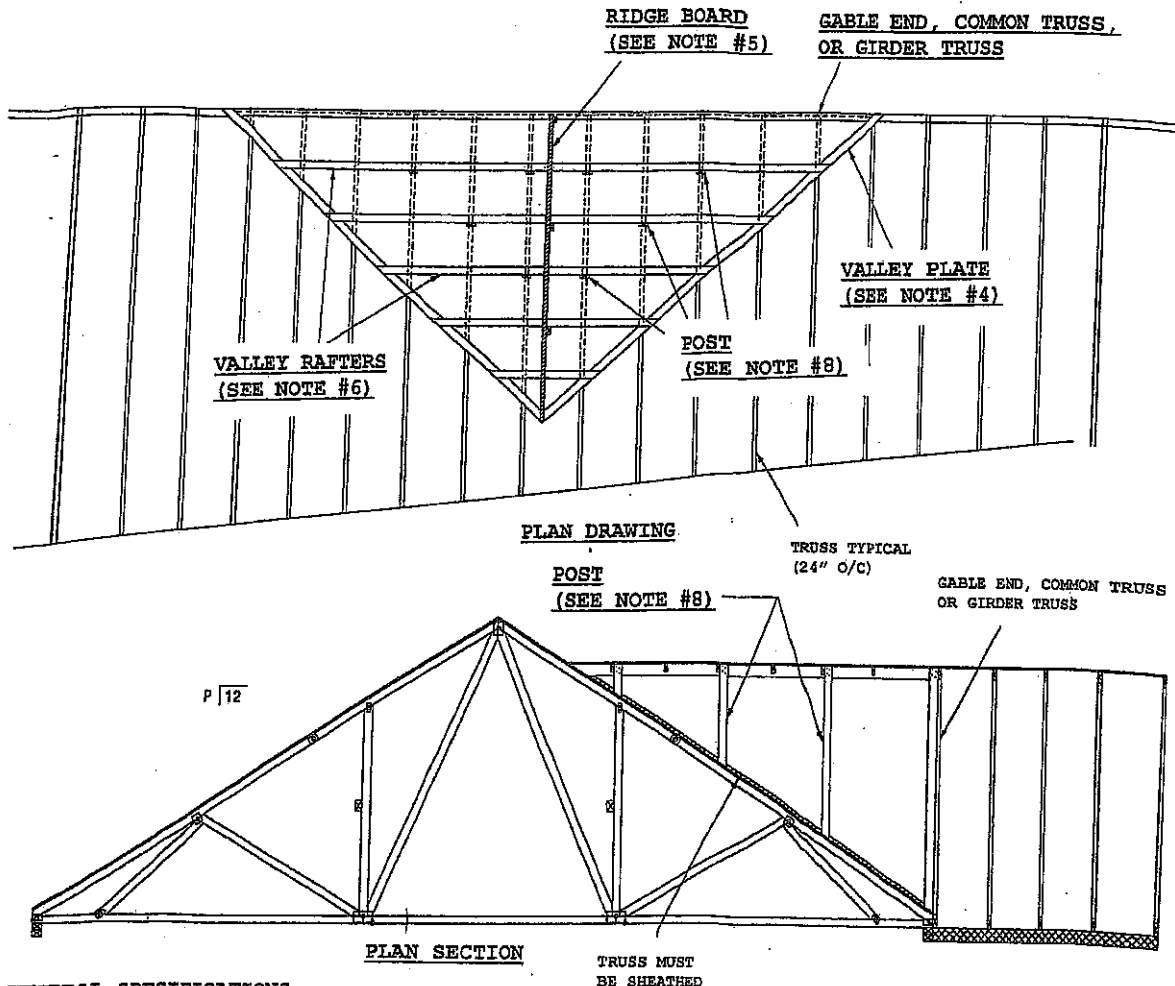
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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

TRUSS MUST
BE SHEATHED

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE), SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

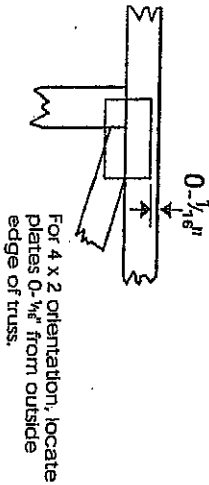
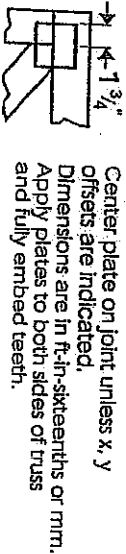


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Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

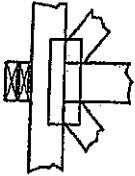
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

BEARING

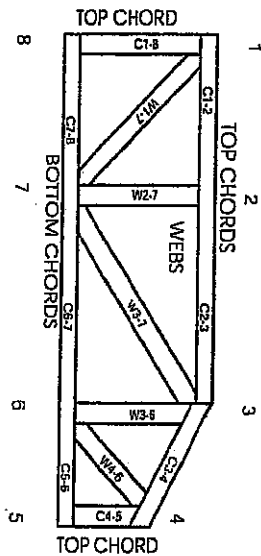


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DS9-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in feet-sixteenths or mm
(Drawings not to scale)



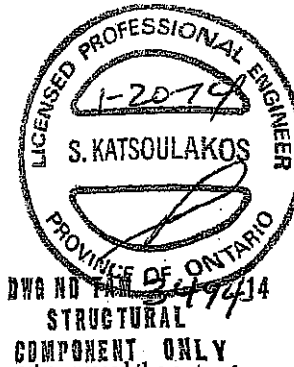
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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