

1 (1			
Length	Product	Plies	Net Qty
16-00-00	9 1/2" NI-40x	1	17
16-00-00	9 1/2" NI-40x	2	4
14-00-00	9 1/2" NI-40x	1	26
12-00-00	9 1/2" NI-40x	1	15
10-00-00	9 1/2" NI-40x	1	5
6-00-00	9 1/2" NI-40x	1	Ż
4-00-00	9 1/2" NI-40x	1	1
2-00-00	9 1/2" NI-40x	-1	4
6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
	16-00-00 16-00-00 14-00-00 12-00-00 10-00-00 6-00-00 6-00-00 6-00-00 6-00-00 6-00-00 6-00-00	16-00-00 9 1/2" NI-40x 16-00-00 9 1/2" NI-40x 14-00-00 9 1/2" NI-40x 12-00-00 9 1/2" NI-40x 10-00-00 9 1/2" NI-40x 6-00-00 9 1/2" NI-40x 4-00-00 9 1/2" NI-40x 2-00-00 9 1/2" NI-40x 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	16-00-00 9 1/2" NI-40x 2 14-00-00 9 1/2" NI-40x 2 14-00-00 9 1/2" NI-40x 1 12-00-00 9 1/2" NI-40x 1 10-00-00 9 1/2" NI-40x 1 6-00-00 9 1/2" NI-40x 1 4-00-00 9 1/2" NI-40x 1 2-00-00 9 1/2" NI-40x 1 2-00-00 9 1/2" NI-40x 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1 6-00-00 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1

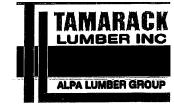
Connector Summary					
Qty	Manuf	Product			
9	H1	IUS2.56/9.5			
2	H1	IUS2.56/9.5			
7	H1	IUS2.56/9.5			

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES

DATE: 2018-10-19

INSPECTOR: BG





FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4

ELEVATION: A,B,C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

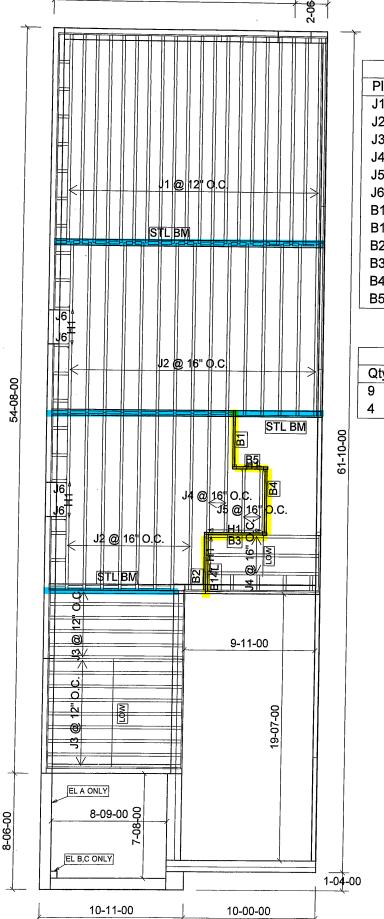
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERÀMIC TILE APPLICATION AS PER O.B.C 9.30.6. LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

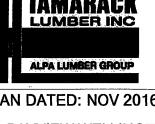
1st FLOOR



18-05-00

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	20
J2	14-00-00	9 1/2" NI-40x	1	23
J3	12-00-00	9 1/2" NI-40x	1	15
J4	10-00-00	9 1/2" NI-40x	1	5
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	4
B1	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B14L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1 .
B5	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary				
Qty	Manuf	Product		
9	H1	IUS2.56/9.5		
4	H1	IUS2.56/9.5		



FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4

ELEVATION: A,B,C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ **REVISION:** 

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

LOADING:

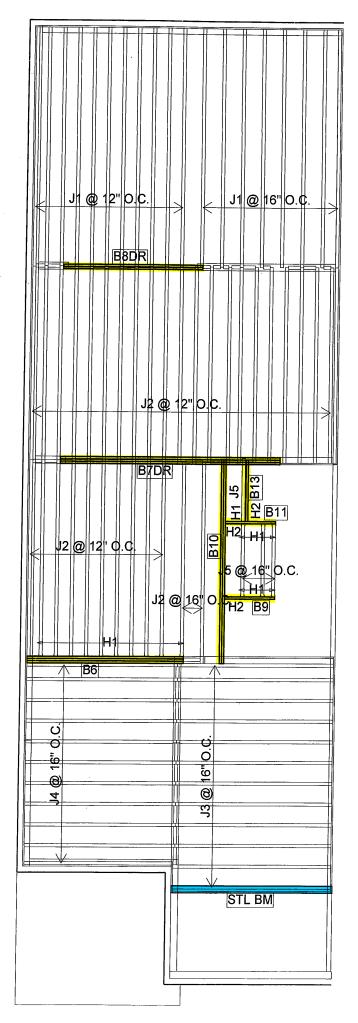
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft2 DEAD LOAD: 15.0 jb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 14/02/2018

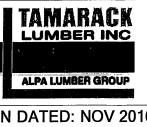
### 1st FLOOR

WOB & WOD CON.



		Products	<del></del>	
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	19
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	12
J4	10-00-00	9 1/2" NI-40x	1	11
J5	6-00-00	9 1/2" NI-40x	1	4
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3 .
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3

(	Connector Summary					
Qty	Manuf	Product				
7	H1	IUS2.56/9.5				
10	10 H1 IUS2.56/9.5					
1	H2	HUS1.81/10				
2	H2	HUS1.81/10				



**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4

ELEVATION: A,B

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ **REVISION:** 

NOTES:

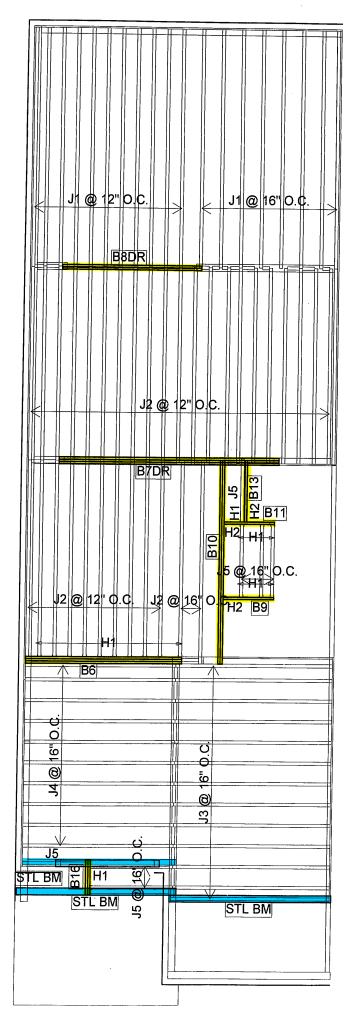
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft2 DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

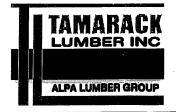
DATE: 29/08/2017

2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	19
J2	14-00-00	9 1/2" NI-40x	1	33
J3	12-00-00	9 1/2" NI-40x	1	13
J4	10-00-00	9 1/2" NI-40x	1	10
J5	6-00-00	9 1/2" NI-40x	1	7
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1 .	1
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B16	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3

Connector Summary					
Qty	Manuf	Product			
7	H1	IUS2.56/9.5			
1	H1	IUS2.56/9.5			
10	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			
2	H2	HUS1.81/10			



**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4

**ELEVATION:** C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

**NOTES:** 

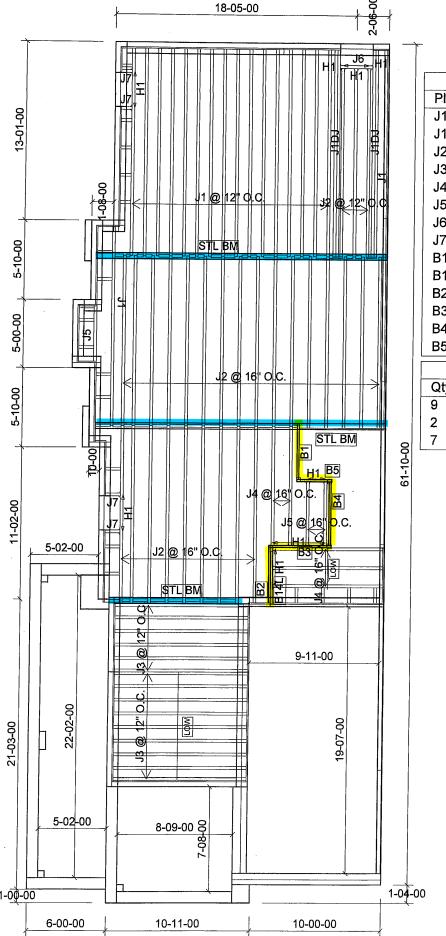
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	18
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	28
J3	12-00-00	9 1/2" NI-40x	1	15
J4	10-00-00	9 1/2" NI-40x	1	5
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	1 .
J7	2-00-00	9 1/2" NI-40x	1	4
B1	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B14L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
Con	nector Sumn	nary		

Manuf

H1

H1

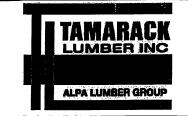
H1

Product

IUS2.56/9.5

IUS2.56/9.5

IUS2.56/9.5



FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4 CORNER

**ELEVATION:** C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

**NOTES:** 

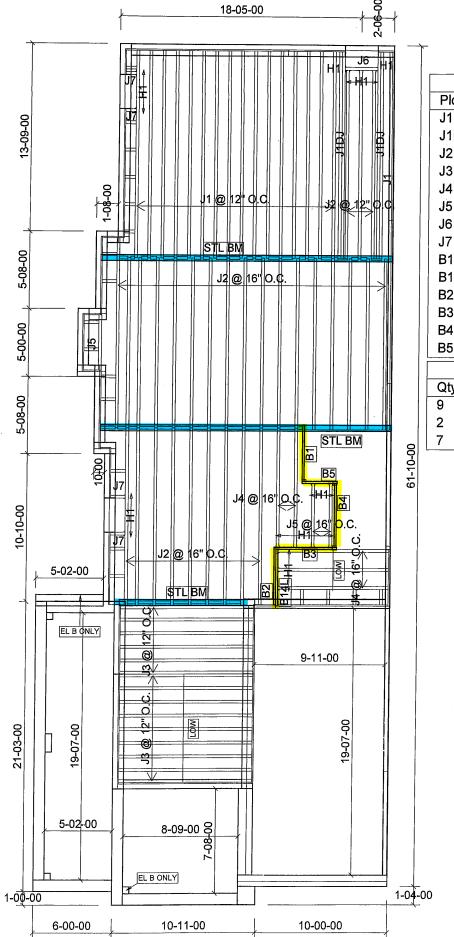
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

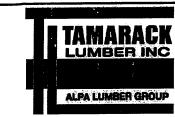
DATE: 14/02/2018

1st FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	17
J1DJ	16-00-00	9 1/2" NI-40x	2	4
J2	14-00-00	9 1/2" NI-40x	1	29
J3	12-00-00	9 1/2" NI-40x	1	15
J4	10-00-00	9 1/2" NI-40x	1	5
J5	6-00-00	9 1/2" NI-40x	1	3
J6	4-00-00	9 1/2" NI-40x	1	1
J7	2-00-00	9 1/2" NI-40x	1	4
B1	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B14L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
0				

Connector Summary					
Qty Manuf Product					
9	H1	IUS2.56/9.5			
2	H1	IUS2.56/9.5			
7	H1	IUS2.56/9.5			



**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4 CORNER

ELEVATION: A,B

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

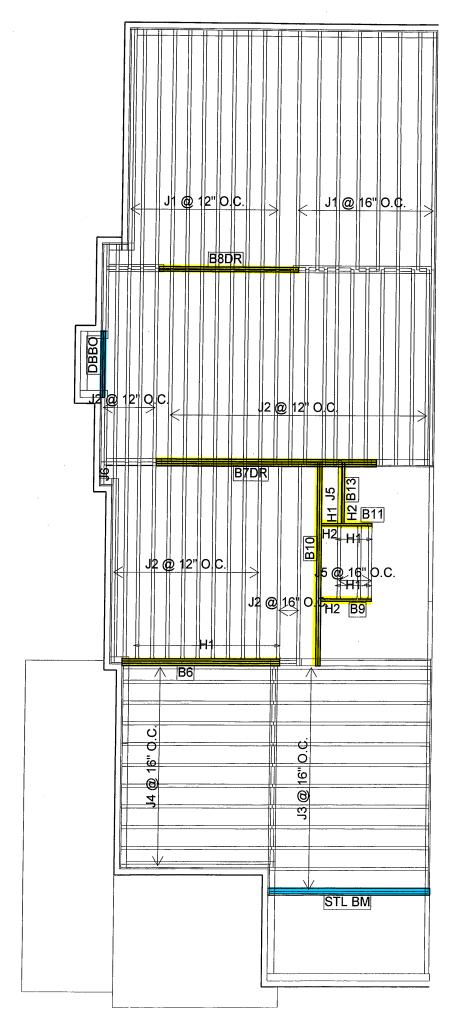
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

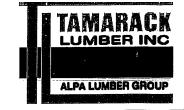
DATE: 14/02/2018

1st FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	19
J2	14-00-00	9 1/2" NI-40x	1	36
J3	12-00-00	9 1/2" NI-40x	1	12
J4	10-00-00	9 1/2" NI-40x	1	11
J5	6-00-00	9 1/2" NI-40x	1	4
J6	2-00-00	9 1/2" NI-40x	1	1
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3 ,
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2 /
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1 ,
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3 ✓

Connector Summary							
Qty	Manuf	Product					
7	H1	IUS2.56/9.5					
10	H1	IUS2.56/9.5					
1	H2	HUS1.81/10					
2	H2	HUS1.81/10					



**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-4 SONOMA 4 CORNER

ELEVATION: A,B

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

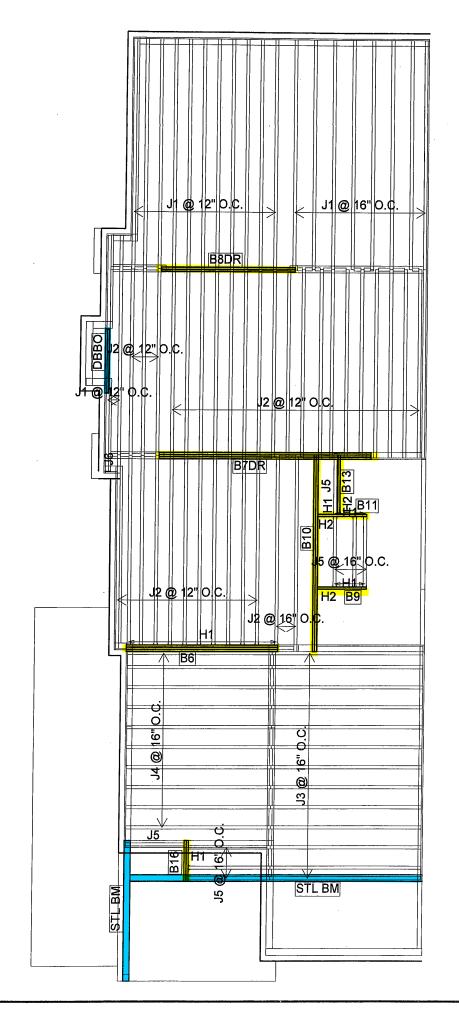
REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

2nd FLOOR



Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	1	21		
J2	14-00-00	9 1/2" NI-40x	1	34		
J3	12-00-00	9 1/2" NI-40x	1	13		
J4	10-00-00	9 1/2" NI-40x	1	10		
J5	6-00-00	9 1/2" NI-40x	1	8		
J6	2-00-00	9 1/2" NI-40x	1	1		
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3		
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B16	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3		

Connector Summary							
Qty Manuf Product							
7	H1	IUS2.56/9.5					
1	H1	IUS2.56/9.5					
11	H1	IUS2.56/9.5					
1	H2	HUS1.81/10					
2	H2	HUS1.81/10					



**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY E AST

MODEL: SD25-4 SONOMA 4 CORNER

**ELEVATION:** C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

**NOTES:** 

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 14/02/2018

2nd FLOOR



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1242)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:22

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1242)

Specifier:

Designer: CZ Company:

Misc:

			7 7 7 7 7
J B0	04-01-1	0	

Total Horizontal Product Length = 04-01-10

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 1-3/4"	485/0	252/0						
B1 5-1/4"	553/0	287/0						

Lood Summon					Live	Dead	Snow Wind	Trib.
	oad Summary g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L 00-00-0	0 04-01-1	0 240	120		n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-0	0 03-08-0	6 12	6		n/a

	Factore d	Factored	Demand/	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	917 ft-lbs	12,704 ft-lbs	7.2%	1	01-11-01
End Shear	534 lbs	5,785 lbs	9.2%	1	00-11-04
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	01-11-01
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	01-11-01
Max Defl.	0.006"	n/a	n/a	4	01-11-01
Span / Depth	4.6	n/a	n/a		00-00-00

Do orio	o er Summ omto	Dim.(L x W)	Demand	De mand/ Resistance Support	Demand/ Resistance Member	Material
B0 B1	n <b>g Supports</b> Post Beam	1-3/4" x 1-3/4" 5-1/4" x 1-3/4"	1,043 lbs 1,189 lbs	41.9% 24.2%	27.9% 10.6%	Un specified Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

000.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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**CONFORMS TO OBC 2012** 

OWG NO. TAM 9259 -18 STRUCTURAL COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1241)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

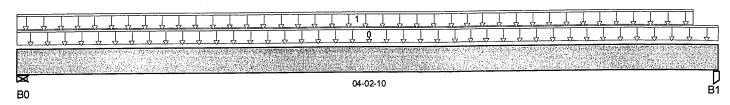
Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1241)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 04-02-10

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 4-3/8"	586/0	304/0						
B1 1-3/4"	525/0	272/0						

Lood Summany			Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 User Load	Unf. Lin. (lb/ft)	L 00-00-00	04-02-10 240	120	·	n/a
1 FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	04-00-14 24	12		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,042 ft-lbs	12,704 ft-lbs	8.2%	1	02-02-10
End Shear	603 lbs	5,785 lbs	10.4%	1	01-01-14
Total Load Defl.	L/999 (0.008")	n/a	n/a	4	02-02-10
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	02-02-10
Max Defl.	0.008"	n/a	n/a	4	02-02-10
Span / Depth	4.8	n/a	n/a		00-00-00

Bearin	ng Supports	Dim.(L x W)	Demand	De mand/ Re sistance Support	De mand/ Resistance Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	1,259 lbs	30.8%	13.5%	Unspecified
B1	Post	1-3/4" x 1-3/4"	1,127 lbs	45.3%	30.2%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. **CONFORMS TO OBC 2012** 

SITE C

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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ON DWG NO. TAM 9260. **STRUCTURAL** COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1269)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

Build 5033 Job Name:

Address:

Customer: Code reports:

City, Province, Postal Code: BRADFORD,

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1269)

Specifier:

Designer: CZ

Company: Misc:

<b>©</b>	$\overline{\mathbb{V}}$	2/	3
ј В0		04-07-00	B1

Total Horizontal Product Length = 04-07-00

Reaction Summary (D	Down / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0, 1-3/4"	442/0	232/0			
B1 3-1/2"	417/0	310/0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J4(i1346)	Conc. Pt. (lbs)	L 00-01-08	00-01-08	238	119		n/a
1 J4(i1334)	Conc. Pt. (lbs)	L 01-05-08	01-05-08	226	113		n/a
2 J5(i1303)	Conc. Pt. (lbs)	L 02-09-08	02-09-08	133	67		n/a
3 -	Conc. Pt. (lbs)	L 04-04-04	04-04-04	262	221		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	630 ft-lbs	12,704 ft-lbs	5%	1	01-05-08
End Shear	442 lbs	5,785 lbs	7.6%	1	00-11-04
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	02-02-00
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	02-02-00
Max Defl.	0.006"	n/a	n/a	4	02-02-00
Span / Depth	5.4	n/a	n/a		00-00-00

<b>-</b>	Dim . (L x W)	De man d	De mand/ Resistance Support	Demand/ Resistance Member	Material	
Bearing Supports B0 Post B1 Post	1-3/4" x 1-3/4"	953 lbs	38.3%	25.5%	Un specified	
	3-1/2" x 1-3/4"	1,013 lbs	20.4%	13.6%	Un specified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. **CONFORMS TO OBC 2012** 

SITE C

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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S. KATSOULAKOS

ON STRUCTURAL -18 COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1580)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

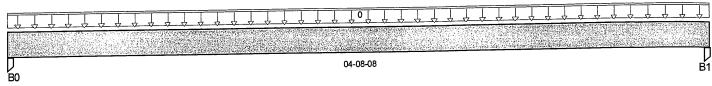
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1580)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 04-08-08

Reaction Summary (	(Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 1-3/4"	26 / 0	25 / 0			
B1, 1-3/4"	26 / 0	25 / 0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	
0 FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	04-08-08	11	6	_	n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	77 ft-lbs	12,704 ft-lbs	0.6%	1	02-04-04
End Shear	42 lbs	5,785 lbs	0.7%	1	00-11-04
Total Load Defl.	L/999 (0.001")	n/a	n/a	4	02-04-04
Live Load Defl.	L/999 (0")	n/a	n/a	5	02-04-04
Max Defl.	0.001"	n/a	n/a	4	02-04-04
Span / Depth	5.7	n/a	n/a		00-00-00

Pooris	ng Supports	Dim . (L x W)	De man d	De mand/ Re sistance Support	De mand/ Resistance Member	Material
B0	Post	1-3/4" x 1-3/4"	70 lbs	2.8%	1.9%	Unspecified
B1	Post	1-3/4" x 1-3/4"	70 lbs	2.8%	1.9%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

**CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1343)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

Address: City, Province, Postal Code:BRADFORD,

Customer:

Build 5033 Job Name:

Code reports:

CCMC 12472-R

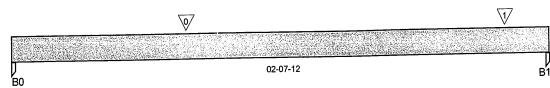
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1343)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 02-07-12

Reaction Summary (Down / L	Jplift) (lbs) Live	De ad	Snow	Wind	
B0, 3-1/2" B1, 3-1/2"	104/0 318/0	59 / 0 25 9 / 0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J5(i1303)	Conc. Pt. (lbs)	L 00-10-04			67 238		n/a n/a
1 -	Conc. Pt. (lbs)	L 02-05-02	02-05-02	289	230		100

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	141 ft-lbs	12,704 ft-lbs	1.1%	1	00-10-04
End Shear	141 lbs	5,785 lbs	2.4%	1	01-01-00
	L/999 (0")	n/a	n/a	4	01-03-03
Total Load Defl.	L/999 (0")	n/a	n/a	5	01-03-03
Live Load Defl.	0"	n/a	n/a	4	01-03-03
Max Defl. Span / Depth	2.8	n/a	n/a		00-00-00

Doorin	a Sunnorte	Dim . (L x W)	De man d	De man d/ Re sistance Support	Demand/ Resistance Member	Material
B0 B1	n <b>g Supports</b> Post Post	3-1/2" x 1-3/4" 3-1/2" x 1-3/4"	229 lbs 801 lbs	4.6% 16.1%	3.1% 10.7%	Un specified Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9263 .70 STRUCTURAL COMPONENT ONLY



### Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i1554)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

Build 5033

Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

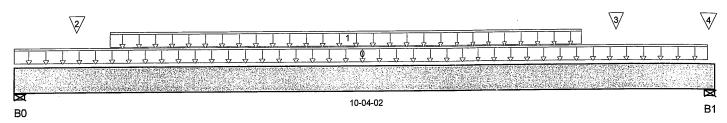
Description: Designs\Flush Beams\1st Floor\Flush Beams\B6(i1554)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 10-04-02

Reaction Summary	(Down / Uplift) (lbs)				
Bearing	Live	De ad	Snow	Wind	
B0, 4-3/8"	1,212/0	679/0			
B1. 7-1/4"	1,540 / 0	847/0			

	ad Cummani					Live	Dead	Snow	Wind	Trib.
	ad Summary g Description	Load Type	Re	Ref. Start		1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-02-14	6	3			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-14	08-04-14	258	130			n/a
2	J2(i1450)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	234	117			n/a
3	J2(i1463)	Conc. Pt. (lbs)	L	08-10-14	08-10-14	301	151			n/a
4	.12(i1618)	Conc. Pt. (lbs)	L	10-02-14	10-02-14	343	172			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,611 ft-lbs	39,636 ft-lbs	16.7%	1	04-10-14
End Shear	2,596 lbs	17,356 lbs	15%	1	08-11-06
Total Load Defl.	L/999 (0.101")	n/a	n/a	4	05-00-06
Live Load Defl.	L/999 (0.065")	n/a	n/a	5	05-00-06
Max Defl.	0.101"	n/a	n/a	4	05-00-06
Span / Depth	12	n/a	n/a		00-00-00

Bearing Supports				Demand/ Resistance	Demand/ Resistance	
		Dim.(LxW)	Demand	Support	Member	Material
BO	Wall/Plate	4-3/8" x 5-1/4"	2,668 lbs	21.7%	9.5%	Unspecified
R1	\\\\all/Plate	7-1/4" x 5-1/4"	3.369 lbs	16.6%	7.3%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

SITE COPY



Po 12

DWG NO. TAM 9/269 STRUCTURAL COMPONENT ONLY



### Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i1554)

BC CALC® Design Report



CCMC 12472-R

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\86(i1554

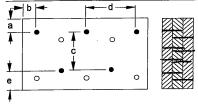
Specifier:

Designer: CZ

Company.

Misc:

Connection Diagram



4 rows

a minimum = 1" b minimum = 3"

e minimum = 2"

Calculated Side Load = 551.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d

Nails

3-1/2" ARDOX SPIRAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9264 .7 STRUCTURAL COMPONENT ONLY





### Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7DR(i1792)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report



BC CALC® Design Repor

Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

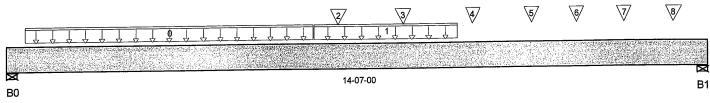
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B7D

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 14-07-00

Reaction Summary (Down I	Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 4"	3,503/0	1,901 / 0			
B1.4"	3,198 / 0	1,794 / 0			

						Live	Dead	Snow	wina	i rib.
	ad Summary g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-04-08	06-04-08	504	252			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	06-04-08	09-04-08	253	126			n/a
2	J2(i1728)	Conc. Pt. (lbs)	L	06-10-08	06-10-08	294	<sup>*</sup> 147			n/a
2	J2(i1720) J2(i1710)	Conc. Pt. (lbs)	L	08-02-08	08-02-08	336	168			n/a
4	-	Conc. Pt. (lbs)	L	09-08-05	09-08-05	618	310			n/a
5	_	Conc. Pt. (lbs)	L	10-11-01	10-11-01	848	498	-		n/a
6	J2(i1436)	Conc. Pt. (lbs)	Ĺ	11-10-08	11-10-08	220	110			n/a
7	J2(i1439)	Conc. Pt. (lbs)	Ī.	12-10-08	12-10-08	251	125			n/a
8	J2(i1423)	Conc. Pt. (lbs)	Ĺ	13-10-08	13-10-08	258	129			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	27,315 ft-lbs	60,415 ft-lbs	45.2%	1	06-10-08
End Shear	7.113 lbs	21,696 lbs	32.8%	1	01-03-14
Total Load Defl.	L/360 (0.468")	0.702"	66.7%	4	07-04-08
Live Load Defl.	L/557 (0.303")	0.468"	64.7%	5	07-04-08
Max Defl.	0.468"	n/a	n/a	4	07-04-08
Span / Depth	14.2	n/a	n/a		00-00-00

Reari	ing Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4" x 5-1/4"	7,630 lbs	44.7%	29.8%	Unspecified
B1	Wall/Plate	4" x 5-1/4"	7,039 lbs	41.3%	27.5%	Unspecified

Notes







### Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7DR(i1792)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B7

Specifier:

Misc:

CZ Designer:

Company.

City, Province, Postal Code: BRADFORD, Customer: Code reports:

Build 5033

Job Name:

Address:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-05-04, Bottom: 00-05-04.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Design based on Dry Service Condition.

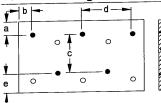
Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Connection Diagram



c = 6-7/8" a minimum = #" d = 🀠 b minimum = 3" e minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nailing schedule applies to both sides of the member.

Member has no side loads.

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL



COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1694)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

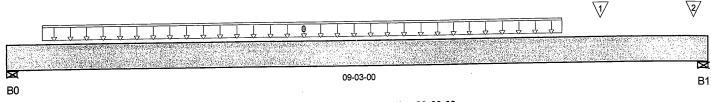
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8D

Specifier:

Designer: CZ Company:

Misc:



Total Horiz	zontal Pro	oduct l	Length =	= 09-0	J3-00

Reaction Summary (Dow	/n / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 3-3/4"	2,329 / 0	1,211/0			
B1, 4-1/4"	2,755/0	1,425 / 0			•

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
<u> </u>	Unf. Lin. (lb/ft)	I 00-05-08	07-04-04	556	278		n/a
0 Smoothed Load	Conc. Pt. (lbs)	L 07-10-04			299		n/a
1 - 2 -	Conc. Pt. (lbs)	L 09-00-12	09-00-12	646	324		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	11,250 ft-lbs	25,408 ft-lbs	44.3%	1	04-10-04
End Shear	4.626 lbs	11,571 lbs	40%	1	01-01-04
Total Load Defl.	L/485 (0.216")	0.435"	49.5%	4	04-09-00
Live Load Defl.	L/736 (0.142")		48.9%	5	04-09-00
	0.216"	n/a	n/a	4	04-09-00
Max Defl. Span / Depth	11	n/a	n/a		00-00-00

Bearing Supports		m: (1 14/)	Domand	Resistance Support	Resistance Member	Material	
		Dim . (L x W)	Demand				
B0	Wall/Plate	3-3/4" x 3-1/2"	5,007 lbs	47%	31.3%	Unspecified	
		4-1/4" x 3-1/2"	5.914 lbs	49%	32.6%	Unspecified	
B1	Wall/Plate	4-1/4 X 3-1/2	0,0 17 103	-10 70	22.070		

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria. Calculations assume unbraced length of Top: 00-03-03, Bottom: 00-03-03.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9









### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1694)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B{

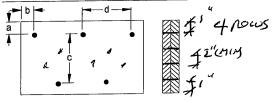
Specifier:

Designer: CZ

Company.

Misc:

### **Connection Diagram**



a minimum = 2" c = 3 - 1/2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Spiker Nails

3-1/2" ARDOX SPIRAL

### **Disclosure**

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 926618 STRUCTURAL COMPONENT ONLY





### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1576)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

t \*\*

Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\89(i1576)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 03-04-00

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind	
BO	106/0	61 / 0			
B1.3-1/2"	175/0	96 / 0			

			Live	Dead	Snow Wind	Trib.	
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	
	Conc. Pt. (lbs)	J 01-02-04	01-02-04	133	67		n/a
0 J5(i1546) 1 J5(i1556)	Conc. Pt. (lbs)	L 02-06-04			54		n/a
2 I5(i1582)	Conc. Pt. (lbs)	L 03-02-12	03-02-12	41	20	Ÿ.	n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	250 ft-lbs	12,704 ft-lbs	2%	1	01-02-04	
End Shear	229 lbs	5.785 lbs	4%	1	00-11-08	
Total Load Defl.	L/999 (0.001")	n/a	n/a	4	01-07-01	
	L/999 (0.001")	n/a	n/a	5	01-07-01	
Live Load Defl.	0.001"	n/a	n/a	4	01-07-01	
Max Defl. Span / Depth	3.8	n/a	n/a		00-00-00	

Poor	ing Supports	Dim . (L x W)	De man d	De man d/ Re s istance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	235 lbs	n/a	5.5%	HUS1.81/10
B1	Post	3-1/2" x 1-3/4"	383 lbs	7.7%	5.1%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9767 - 18
STRUCTURAL
COMPONENT ONLY



**CONFORMS TO OBC 2012** 



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1232)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

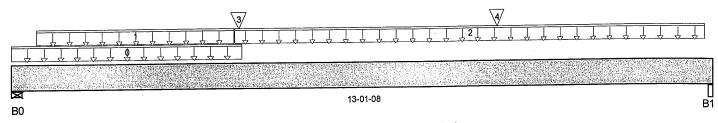
Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1232)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 13-01-08

Reaction Summary (Down /	Uplift) (lbs)	De ad	Snow	Wind	
B0, 5-1/2"	1,260 / 0	703/0			
B1, 3-1/2"	604/0	376/0			

La ad Oamana ma			L	.ive	Dead	Snow W	ind Trib.
Load Summary Tag Description	Load Type	Ref. Start	End 1.	.00	0.65	1.00 1.1	15
0 User Load	Unf. Lin. (lb/ft)	L 00-00-00	04-03-12 2	240	120		n/a
1 FC3 Floor Material	Unf. Lin. (lb/ft)	L 00-05-08	04-02-00 2	29	15		n/a
2 FC3 Floor Material	Unf. Lin. (lb/ft)	L 04-02-00	12-11-12 5	3	26		n/a
3 B9(i1576)	Conc. Pt. (lbs)	L 04-02-14	04-02-14 1	03	59		n/a
/	Conc. Pt. (lbs)	I 09-01-02	09-01-02 1	48	88		n/a
4 B11(i1461)	COTIC. 1 L. (103)	2 00 01 02					

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	5,537 ft-lbs	25,408 ft-lbs	21.8%	1	04-07-06	
End Shear	2,061 lbs	11,571 lbs	17.8%	1	01-03-00	
Total Load Defl.	Ľ675 (0.222'')	0.625"	35.5%	4	06-04-14	
Live Load Defl.	L/1,076 (0.139")	0.417"	33.5%	5	06-04-14	
Max Defl.	0.222"	n/a	n/a	4	06-04-14	
Snan / Denth	15.8	n/a	n/a		00-00-00	

Bearing Supports				Resistance		
		Dim. (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	2,769 lbs	26.9%	11.8%	Unspecified
R1	Beam	3-1/2" x 3-1/2"	1,376 lbs	10.3%	9.2%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9









### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Fioor\...\B10(i1232)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report



**Build 5033** Job Name:

Address: City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Fiush Beams\1st Floor\Flush Beams\B10(i12:

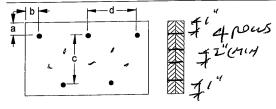
Specifier:

Designer: CZ

Company:

Misc:

### **Connection Diagram**



a minimum = 2" c = 3 - 1/2" d= 🗫 6 b minimum = 3"

Calculated Side Load = 42.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

3-1/2" ARDOX SPIRAL

### Disclosure

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DWG NO. TAM 9266 STRUCTURAL COMPONENT ONLY





### Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i1461)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

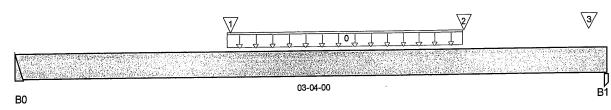
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i1461)

Specifier:

Designer: CZ Company.

Misc:



Total Horizontal Product Length = 03-04-00

Reaction Summary (D	Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0	151/0	89 / 0			
B1, 3-1/2"	202/0	114/0			

1						Live	Dead	Snow	Wind	i rib.
	Summa <i>r</i> y scription	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
	3 Floor Material	Unf. Lin. (lb/ft)	L	01-02-04	02-06-04	3				n/a
4	311001 Material	Conc. Pt. (lbs)	Ī	01-02-08	01-02-08	204	112			n/a
1 -	(AEEO)	Conc. Pt. (lbs)	ī	02-06-04	02-06-04	105	53			n/a
	(i1556)	· · ·	i-	03-02-12	03-02-12		20			n/a
3 .15	<i>(</i> i1582)	Conc. Pt. (lbs)		03-02-12	00-02-12	71	_0			

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	363 ft-lbs	12,704 ft-lbs	2.9%	1	01-02-04
End Shear	333 lbs	5,785 lbs	5.8%	1	00-11-08
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	01-06-09
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-06-09
Max Defl.	0.002"	n/a	n/a	4	01-06-09
Span / Depth	3.8	n/a	n/a		00-00-00

Reari	ng Supports	Dim . (L x W)	De man d	De mand/ Re sistance Support	De mand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	339 lbs	n/a	7.9%	HUS1.81/10
B1	Post	3-1/2" x 1-3/4"	446 lbs	9%	6%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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CONFORMS TO OBC

DWG NO. TAM 9269 STRUCTURAL COMPONENT ONLY



### Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B13(i1675)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

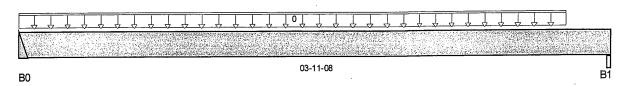
Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i1675)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 03-11-08

Reaction Summary	(Down / Uplift) (lbs)						
Bearing	Live	De ad	Snow	Wind			
B0	12 / 0	15 / 0					
B1, 3-1/2"	11 / 0	15 / 0					
				Live	Dead	Snow Wind	Trib.

1	oad Summary				Live	Dead	Snow Wind	Trib.
	ag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
Ō	FC3 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	03-08-00	6	3		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	32 ft-lbs	12,704 ft-lbs	0.2%	1	01-11-00
End Shear	18 lbs	5,785 lbs	0.3%	1	00-11-08
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-11-00
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-11-00
Max Defl.	0"	n/a	n/a	4	01-11-00
Span / Depth	4.6	n/a	n/a		00-00-00

Bear	ing Supports	Dim.(L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Hanger	2" x 1-3/4"	37 lbs	n/a	0.9%	HUS1.81/10
B1	Beam	3-1/2" x 1-3/4"	36 lbs	0.5%	0.5%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

**CONFORMS TO OBC 2012** 

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9270 STRUCTURAL COMPONENT ONLY





### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B14L(i1364)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

\*

Build 5033 Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B14L(i136

Specifier:

Designer: CZ

Company.

Misc:

<b>o</b> /	<b>V</b>	2/	3/
	menologia Participation		
×		04-02-00	Ų
B0			ы

Total Horizontal Product Length = 04-02-00

Reaction Summary (Down	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2"	255/0	153/0			
B1, 3-1/2"	344/0	181/0			

1 1 0			Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
0 1(i649)	Conc. Pt. (lbs)	L 00-02-12	00-02-12	15		n/a
1 J4(i1390)	Conc. Pt. (lbs)	L 01-04-00	01-04-00 228	114		n/a
2 J4(i1388)	Conc. Pt. (lbs)	L 02-08-00	02-08-00 243	121		n/a
3 J4(i1386)	Conc. Pt. (lbs)	L 04-00-12	04-00-12 128	64		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	592 ft-1bs	12,704 ft-lbs	4.7%	1	02-08-00
End Shear	547 lbs	5,785 lbs	9.5%	1	01-03-00
Total Load Defl.	L/999 (0.004")	n/a	n/a	4	02-02-00
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	02-02-00
Max Defl.	0.004"	n/a	n/a	4	02-02-00
Span / Depth	4.5	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(L x W)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 1-3/4"	573 lbs	11.1%	4.9%	Un specified
B1	Post	3-1/2" x 1-3/4"	743 lbs	14.9%	9.9%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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**CONFORMS TO OBC 2012** 



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16(i2866)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 11:58:59

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 CORNER EL C.mmdl

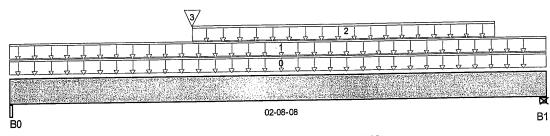
Description: Designs\Flush Beams\1st Floor\Flush Beams\B16(i2866)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 02-08-08

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/4" B1, 5-1/2"	179/0 130/0	273/0 251/0	193/0 196/0		

					Live	Dead	Snow Wind	i rib.
	oad Summary g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
-		Unf. Lin. (lb/ft)	I 00-00-00	02-08-08	33	130	72	n/a
U	User Load LOWROOF	Unf. Lin. (lb/ft)	1 00-00-00	02-08-08		30	72	n/a
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L 00-11-00	02-05-06	6			n/a
2	.15(i2808)	Conc. Pt. (lbs)	L 00-11-00	00-11-00	121	60		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	260 ft-lbs	25,408 ft-lbs	1%	1	01-02-01
End Shear	134 lbs	11,571 lbs	1.2%	1	01-02-12
Total Load Defl.	L/999 (0")	n/a	n/a	35	01-03-13
Live Load Defl.	L/999 (0")	n/a	n/a	51	01-03-13
Max Defl.	0"	n/a	n/a	35	01-03-13
Span / Depth	2.4	n/a	n/a		00-00-00

				De mand/ Resistance		
Beari	ng Supports	Dim . (L x W)	Demand	Support	Member	Material
B0	Beam	5-1/4" x 3-1/2"	720 lbs	7.3%	3.2%	Unspecified
D1	MbII/Plate	5-1/2" x 3-1/2"	674 lbs	6.6%	2.9%	Unspecified

Notes

POVINCE OF DWG NO. TAM 9272-18 **STRUCTURAL** 

COMPONENT ONLY



**Build 5033** 

Job Name:

Address:

### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16(i2866)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 11:58:59

BC CALC® Design Report



File Name: SD25-4 SONOMA 4 CORNER EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B16(i286

Specifier:

Designer:

Company.

Misc:

City, Province, Postal Code: BRADFORD, Customer: Code reports: CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

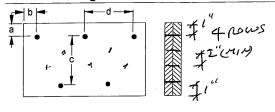
Calculations assume member is fully braced. Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = #" b minimum = 3"

Calculated Side Load = 94.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nails Connectors are:

3-1/2" ARDOX SPIRAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9/272-10 STRUCTURAL COMPONENT ONLY





Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			{	Bare		ļ	1/2" Gyp	sum Ceiling	
Depth	Series		On Cen	tre Spacing				re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-	Span Blocking a	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing				re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11 7/0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22' <del>-</del> 2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24' <b>-</b> 0"	22' <del>-</del> 3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25' <b>-</b> 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23' <b>-</b> 9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28' <b>-</b> 2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27' <b>-</b> 5"	26' <del>-</del> 2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			B	are			1/2" Gy	osum Ceiling	
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	<b>/</b> 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17' <b>-</b> 5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11-7/0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
-	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20' <b>-</b> 2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21' <b>-</b> 7"	20'-7"	19'-5"	24'-0"	22' <del>-</del> 3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	N!-70	25 <b>'-1</b> "	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21' <b>-</b> 6"
10	NI-80	25 <b>'-</b> 6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing		1	On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16' <b>-</b> 2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22 <b>'-</b> 5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/6	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23' <b>-</b> 7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23' <b>-</b> 8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25' <b>-</b> 3"	24' <b>-</b> 1"	22'-9"
	NI-90x	27' <b>-</b> 3"	25' <del>-</del> 4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28' <b>-</b> 8"	26 <b>'-</b> 8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
10	NI-80	29' <b>-</b> 1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25' <b>-</b> 0"	30'-6"	28' <b>-</b> 5"	27'-2"	25 <b>'-</b> 8"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare		İ	1/2" Gyp	sum Ceiling	
Depth	Series	-	On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18' <b>-</b> 1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-//0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17' <b>-</b> 9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18' <b>-</b> 9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20' <b>-</b> 8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19' <b>-</b> 8"	18'-5"	N/A
11-7/0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22' <del>-</del> 2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23' <b>-</b> 2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23' <b>-</b> 9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25' <b>-</b> 8"	24'-6"	N/A	28'-5"	26'-5"	25' <b>-</b> 2"	N/A
LO	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			B	are		l	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/6	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20' <del>-</del> 0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	N!-70	23' <del>-</del> 0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22' <b>-</b> 3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22' <del>-</del> 9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19' <b>-</b> 3"	17'-9"	15'-10"
11-7/8"	NI-60	21' <b>-</b> 9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17' <b>-</b> 5"
	NI-60	24' <b>-</b> 9"	22' <b>-</b> 5"	21'-0"	19'-6"	24'-9"	22' <del>-</del> 5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26' <b>-</b> 6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23' <b>-</b> 3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24' <b>-</b> 3"	22'-4"
	NI-60	27' <b>-</b> 3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28' <b>-</b> 8"	26'-8"	25'-3"	23' <del>-</del> 4"	29' <b>-</b> 3"	26'-11"	25'-3"	23'-4"
10	NI-80	29'-1"	27'-0"	25 <b>'-</b> 9"	23'-10"	29' <b>-</b> 8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
 Minimum bearing length shall be 1-3/4 inches for the end bearings.

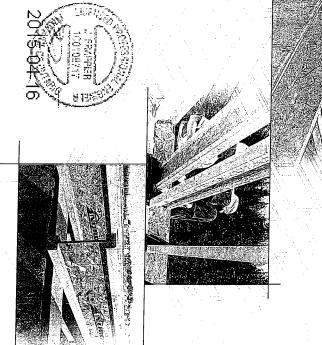
<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

# ENGINEERED WOOD

# NSTALLATION GUIDE

FOR RESIDENTIAL FLOORS



Distributed by:



# SAFETY AND CONSTRUCTION PRECAUTIONS



braced, or serious injuuntil fully fastened and ries can result.



Once sheathed, do not unsheathed I-joists. materials over

### Do not walk on I-joists

over-stress l-joist with concentrated loads from building materials. Never stack building

WARNING

N-C301 / November 2014

braced and sheathed. l-joists are not stable until completely installed, and will not carry any load until fully

# Avoid Accidents by Following these Important Guidelines:

- 1. Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When Lipists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- 2. When the building is completed, the floor sheathing will provide lateral to prevent l-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied support for the top flanges of the I-joists. Until this sheathing is applied,
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long bracing over at least two I-joists. the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each Ljoist. Nail
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered Lioists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

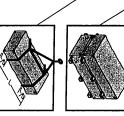
Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully,

# STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- 3. Always stack and handle Lipists in the upright position only. 2. Store, stack, and handle I-joists vertically and level only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise
- Protect I-joists from weather, and use spacers to separate bundles.
- 6. Bundled units should be kept intact until time of installation
- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- to your work crew.

■ Pick I-joists in bundles as shipped by the supplier

- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5<sup>th</sup> points, using a spreader bar if necessary.
- 8. Do not handle I-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST



### MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or or more of the adjacent span. 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of 1/480. For multiple-span applications, the end spans shall be 40% limit states are based on the factored loads of 1.50L + live load of 40 psf and dead load of 15 psf. The ultimate multiple-span residential floor construction with a design
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGBS-71.26 of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used
- 3. Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

### SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

Joist Depth	Joist Series		Simple On centr	e spans e spacing			Multipl On centr	e spans e spacing	
		12"	16"	19.2	24"	12"	16"	19.2"	2
		15:13	14:2		13-5	£ 91	15:4	14-10	1
1		16-35	1514	14-10	14111	1/1/27	16-5	16-0	5.5
			1641	15-6	15.7	87	17.4	16:9*	7
		- N	16.3	15-8	15.9	18210	17.6	16-11	718
			19.0	15'-5'	15.6	184	17:3	16-8	910
		10.4	17.3	16:77	0.0	2 6	9 O		 
		.9-61	18-0	174	17.5	21.6	19:11:	1900	- - -
		14.0	18.3	17.6	77.7	21.9	20-2	19:3	19
		20.4	19191	17-11	18:0	22.5	20-7	19-10	19-91
		20.1	18.7	17-10%	10.74H	22.2	20-6	-8-6E	19
		27-7	20-0	19:1:	1912	23-10	22.7	2141	210
		20.5	2013	1914.	19:5	243	22.5	2] 6	2
	123	2217	20411	19-11	20.0	25.0	23-10	22'-0"	3
		23.6	21.9	19-9	19-10 19-10	24-7	22.9	21:9"	21
63		23-11	22:1	21:1"	21.2	26.5	24-5	23-3	22.
		24-5	22:6	21'-5'	21:-6	26-11-	24-10	23.9	23

### CCMC EVALUATION REPORT 13032-R

**NORDIC I-JOIST SERIES** 

### **I-JOIST HANGERS**

- 1. Hangers shown illustrate the three to support 1-joists. most commonly used metal hangers
- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- 4. Web stiffeners are required when the brace the top flange of the 1-joist. sides of the hangers do not laterally









### Face Mount





### WEB STIFFENERS

### RECOMMENDATIONS:

- the stiffener and the flange is at the top. engineered applications with factored ■ A bearing stiffener is required in all Construction Guide (C101).The gap between reactions greater than shown in the -joist properties table found of the I-joist
- stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and the l-joist is supported in a hanger and the A bearing stiffener is required when
- by the code. The gap between the stiffener than 2,370 lbs is applied to the top flange ■ A load stiffener is required at locations and the flange is at the bottom. adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever between supports, or in the case of a where a factored concentrated load greater
- SI units conversion: 1 inch = 25.4 mm

### FIGURE 2

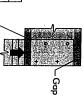
### WEB STIFFENER INSTALLATION DETAILS

CONCENTRATED LOAD



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Tight Joint No Gap

STIFFENER SIZE REQUIREMENTS

Flange Width 3-1/2" 2-1/2

Web Stiffener Size Each Side of Web

1-1/2" x 2-5/16" minimum width 1" x 2-5/16" minimum width

### 33 pieces per unit S-P-F No.2 1950f MSR 33 pieces per unit

33 pieces per unit 2100f MSR

1950f MSR 23 pieces per unit

2100f MSR 23 pieces per unit

2400f MSR 23 pieces per unit

NPG Lumber 23 pieces per unit

finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from to task to the products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables Northic

Nordic Engineered Wood I-joists use only finger-jointed back spruce longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior strength, rung

2019-04-1

OSB 3/8"-

OSB 716'

OSB 71

### **INSTALLING NORDIC I-JOISTS**

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact you
- 2. Except for cutting to length, 1-joist flanges should never be cut, drilled, or notched.
- 3. Install L-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.

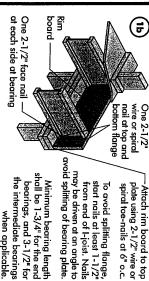
- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple நோர்ல் அமை
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the Concentrated loads greater than those that can normally be expected in residential construction should only be applied to
- 9. Never install Lipists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking I-post-compatible depth selected. panels or other engineered wood products — such as rim board — must be cut to fit between the 1-joists, and an
- 13. Provide permanent lateral support of the bottom flange of all Ljoists at interior supports of multiple-span joists. Similarly structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary support the bottom flange of all cantilevered Ljoists at the end support next to the cantilever extension. In the completed bracing or struts must be used.
- 14. If square-edge panels are used, edges must be supported between 1-joists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

ⓓ

panel NI blocking

One 2-1/2"

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NI Joists	Vertical Load* (pH)	Blocking Panel	Maximum Factored Uniform
NI Joists	3,300	or Rim Joist	Vertical Load* (plf)
*The uniform vertical load	*The uniform vertical load is limited to a joist depth of 16	1-1/8" Rim Board Plus	8,090
inches or less and is base	he hell need to be a standard term load duration.	*The uniform vertical load is limited to a rim board doubt of 17.	ted to a rim board

교등의

It shall not be used in the design of a bending member

top plate per detail 1b Attach I-joist to

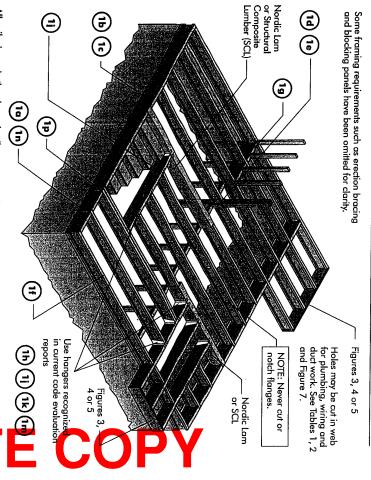
Blocking Panel or Rim Joist

Maximum Factored Uniform Vertical Load\* (pH) 3,300

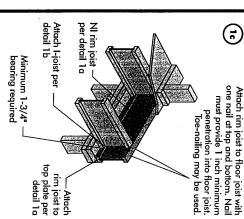
such as joist, header, or raffer. For concentrated vertical

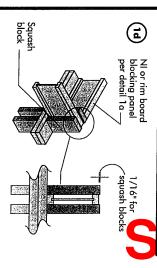
load transfer, see detail

# TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framin lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clart





1-1/8" Rim Board Plus	2x Lumber		Pair of Squash Blocks
4,300	5,500	3-1/2" wide	Maximum Factored Vertical per Pair of Squash Blocks (lbs)
6,600	8,500	5-1/2" wide	h Blocks (lbs)

Provide lateral bracing per detail 1a, 1b, or 1c

8,090	1-1/8" Rim Board Plus
Maximum Factored Uniform Vertical Load* (pH)	Blocking Panel or Rim Joist
Minimum bearing I shall be 1-3/4" for the bearings, and 3-1/ the intermediate bear when applications and shall be shall	One 2-1/2" face nail — at each side at bearing
from end of I-joist. may be driven at an ar avoid splitting of bearing	Rim
To avoid splitting fl	bonorii lidrige
piral / plate using 2-1/2" v p and / spiral toe-nails at a	wire or spiral nail at top and

with same nailing

bearing plate

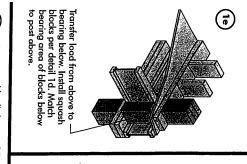
as required for

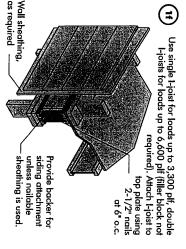
plate (when used for lateral shear transfer, nail to

2-1/2" nails at

6" o.c. to top

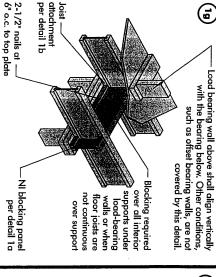
~ ~
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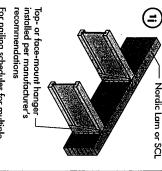




carried to the foundation. required when rim board is used. Bracing per code shall be Rim board may be used in lieu of I-joists. Backer is not

**(1)** 

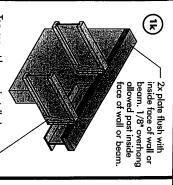




beams, see the manufacturer's recommendations For nailing schedules for multiple

stiffeners shall be used. support the top flange, bearing

Note: Unless hanger sides laterally



detail 1p Filler block per

manufacturer's recommendations Top-mount hanger installed per \_\_\_

> manufacturer's install hanger per

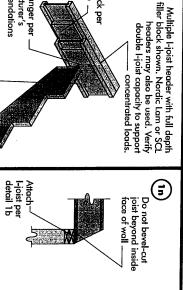
recommendations and a second s

support the top flange, bearing Note: Unless hanger sides laterally stiffeners shall be used

> dinch when possible. Backer block attached per

detail 1h. Nail with twelve 3" nails,

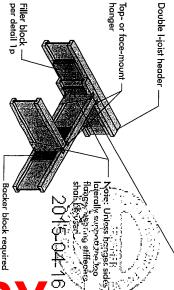
Maximum support capacity = 1,620 lbs



Note: Blocking required for clarity. support, not shown at bearing for lateral

> $\bigcirc$ backer block will fit. Clinch. Install backer tight to top flange.
> Use twelve 3" nails, clinched when possible, Maximum factored Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the Backer block (use if hanger load exceeds 360 lbs)

resistance for hanger for this detail = 1,620 lbs.



Verify double I-joist capacity to support concentrated loads. For hanger capacity see hanger manufacturer's recommendations.

(both sides for face-mou

nangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

3-1/2	اري. الري 1 د	2-1/2"	Flange Width
1-1/2"	1 1 6	-1	Material Thickness Required*
7-1/4"		5-1/2"	Minimum Depth**

- Minimum grade for backer block material shall be S-P-F No. 2 or to CAN/CSA-O325 or CAN/CSA-O437 Standard better for solid sawn lumber and wood structural panels conformir
- For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4"

### Notes:

€

Filler block

- 2. Leave a 1/8 to 1/4-inch gap between to prevent damage to web/flange connection. of filler block and bottom of top I-joist
- Filler block is required between joists for full length of span.
- 4. Nail joists together with two rows of 3" are required. nails at 12 inches o.c. (clinched when can be clinched, only two nails per foot possible) on each side of the double I-jo Total of four nails per foot required. If n

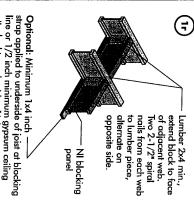
 Offset nails from opposite face by 6"

The maximum factored load that may be applied to one side of the double joist using this detail is 860 lbf/ft. Verify double

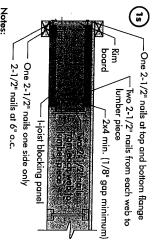
—1/8" to 1/4" gap between top flange and filler block

### 1. Support back of I-joist web during nailing to DOUBLE I-JOIST CONSTRUCTION FILLER BLOCK REQUIREMENTS FOR

8	Flange	Joist	Filler
	O.E.O	Popul	DIOCK SIZE
		9-1/2"	2-1/8" x 6"
٦	2-1/2"×	11-7/8"	2-1/8" x 8"
	1-1/2"	14"	2-1/8" x 10"
		16"	2-1/8" x 12"
		9-1/2"	3" x 6"
oist.	3-1/2"×	11-7/8"	ယူ ×ဇူ
<u>a:</u>	1-1/2"	14"	3" x 10"
		16"	3" x 12"
	3-1/2" ×	11-7/8"	3" x 7"
Ď	2	14"	3º × 9º
1	1	16"	3" x 11"



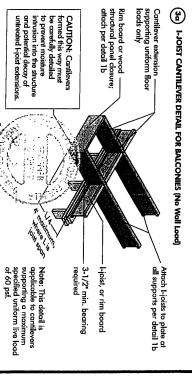
line or 1/2 inch minimum gypsum ceiling attached to underside of joists.



### Notes:

- In some local codes, blocking is prescriptively required in the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to for spacing of the blocking.
- All nails are common spiral in this detail





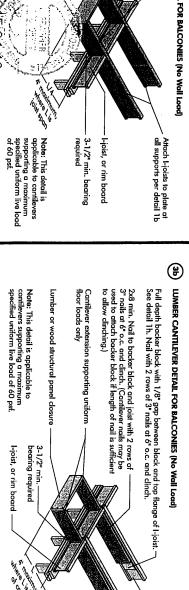
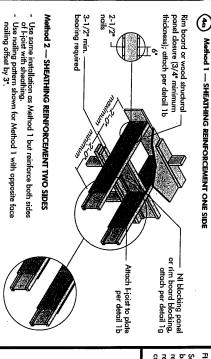


plate at all supports per detail 1b Attach I-joists to

# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

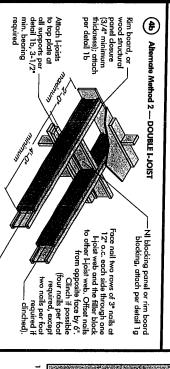
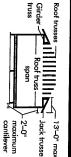


FIGURE 4 (continued) requirements at cantilever. reinforcement See table elow for N Roof truss span 2-0 Girder -



 Jack trusses 13'-0" maximum

For hip roofs with the jack trusses running parallel to the cantileveed floor joists, the l-joist reinforcement requirements for a span of 26 ft. shall be permitted to be used.

## CANTILEVER REINFORCEMENT METHODS ALLOWED

ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ		ZZZZZ ZZ	F
ZZ Z Z Z Z Z Z Z	ZZZZZZZ ZZZZZZZ	Z		LL = 30 psf, DI JOIST SPACII 16
22222222 223222222	ŻŻZZZZZ	N X C L - L	2	psf, DI SPACII S
	zzz	2 2 2		5 pg
	TOTAL STREET,			sf 24
2222222 2222222		ZZZZZZ	_ZZZZZ	ROOF LOAD LL = 40 JOIST 12 1
ZZZZZZZ			xxx ผม	ing (UNFAC psf, DL = 15 SPACING (in 6 19.2
30	V222-1-1	אטטטא××)	×××××	CTORED) 5 psf 1.) 24
ZZZZZZZZ	ZZZZZZZZ	ZZZZZZZ	zż	12 101 = 11
·ZZZZZZZZ	zzzzz		××222	psf, DI SPACII
	××××××××××××××××××××××××××××××××××××××	,	r	15 ps (in.)

- N = No reinforcement required.
  1 = NI reinforced with 3/4" wood structural
- panel on one side only.

  2 = NI reinforced with 3/4" wood structural panel on both sides, or double I-joist.

  X = Try a deeper joist or closer spacing.

  A Mosimum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 pff wall load. Wall load is based on 3".0" naximum width window or door openings

centreline of the reinforcing panel from each side. Clinch when possible.

Block I-joist together with filler blocks for the full length of the reinforcement. \rightarrow for I-joist flange widths greater than 3 inches place an additional row of 3" nails along the

- For larger openings, or multiple 3-0" width openings spaced less than 6-0" o.c., additional joists beneath the opening's cripple studi may be required.

  3. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 paf and dead load of 15 paf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
- 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent of the distance between the supporting wall and the ridge beam.

  When the roof is transed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a trust is used. truss is used.
  Cantilevered joists supporting girder trusses or roof beams may require additional

# RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. hole or duct chase opening shall be in compliance with the requirements of
- 'n I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent Lioist tlange
- Ġ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 6٠ Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the Tables 1 and 2, respectively opening shall be sized and located in compliance with the requirements of longest rectangular hole or duct chase opening) and each hole and duct chase size of the largest square hole (or twice the length of the longest side of the
- 7. A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- <u></u> ٠, All holes and duct chase openings shall be cut in a workman-like manner in A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 11. Limit three maximum size holes per span, of which one may be a duct chase accordance with the restrictions listed above and as illustrated in Figure 7
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Joist Depth				
Joist Series	eleleje;	1-1-2-1-1-2	-}-j-1-j-} <sub>-</sub>	geleti.
2	i e j	iskasi.		
<b>a</b> E				
3 5				
6 G		i de la companya da companya d		
nside fa ound h /4 7	<b>W</b> (1984)		2.00	0.2200
ce of a ble dia: 8	juli		3 00 3 0 5 0 0 0 0	5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6
ny supp neter (i 8-5/8	Lili			1386
oort to (in.)		111111		
_ 0		List I s F		
of hole  0-3/4				100
(ff-in.) 11 12				
2 12-3/		11 15 1		
ិ ខ្		16186		
	42.96	10.900	omers = 6	120

- Hole location distance is measured from inside face of supports to centre of hole. Distances in this chart are based on uniformly loaded joists.

Where: Preduced = <u>Factual</u> x D

actual Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The jet) distance shall not be less than 6 inches from the face of the support to edge of the hole.

The actual measured span distance between the inside faces of supports (fit).

H <u>backed</u> is greater than 1, use 1 in the above calculation for <u>backed</u>.

SAF

015-04-16

### OPTIONAL:

The above table is based on the Lioists used at their maximum span. If the Lioists are placed at less than their full maximum span (see Maximum fice). Spans, the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced =

¥ 11

The minimum distance from the inside face of any support to centre of hole from this table Span Adjustment Factor given in this table.

TABLE 2

## DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only Minimum distance from inside face of any support to centre of opening (fi-in.)

of any support isse length (in.)  16 18  16 18  58 25  610 25

- -444
- Above table may be used for Hoist spacing of 24 inches on centre or less.

  Dud chase opening location distance is measured from inside face of supports to centre of opening.

  The above table is based on simple-span piets only. For other applications, contact your local distributar.

  Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 p dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor. load of 40 psf and

### FIELD-CUT HOLE LOCATOR FIGURE 7

Knockouts	See Table 1 for minimum distance from bearing
See rule 12	2x diameter of larger hole
Maintain minimum 1/8" space between top and bottom flange — all duct chase openings and holes	2x duct chase — Duct chase opening length or hole diameter, minimum distance whichever is from bearing)

A knockout is **NOT** considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes

sharp saw. notch the flange, or over-cut the web. Holes in webs should be cut with a

spaced 15 inches on centre along the length of the Lioist. Where possible, it is preferable to use knockouts instead of

field-cut holes

Never drill, cut or

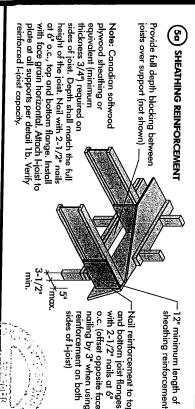
electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are

for the contractor's convenience to install

Knockouts are prescored holes provided

the rectangular hole by drilling a 1-inch diameter hole in each of the four corners the holes is another good method to and then making the cuts between stress concentrations. Slightly rounding the corners, as this can cause unnecessary For rectangular holes, avoid over-cutting the corners is recommended. Starting ninimize damage to the I-joist

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



o.c. (offset opposite face nailing by 3" when using Nail reinforcement to top and bottom joist flanges with 2-1/2" nails at 6" reinforcement on both

**(5**b)

SET-BACK DETAIL

### between joists over support (not shown for clarity) - Attach I-joist to plate at all supports per detail 1b. 3-1/2" minimum I-joist structural panel closure (3/4" minimum thickness), Provide full depth blocking max. Bearing walls Attach joists to

Notes:

attach per detail 1b.

Rim board or wood

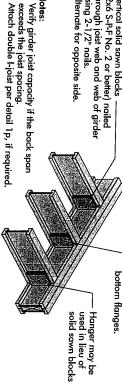
### (F) SET-BACK CONNECTION

nails, toe-nail at top and Nail joist end using 3" bearing required

girder joist per detail 5c.

(2x6 S-P-F No. 2 or better) nailed through joist web and web of girder Alternate for opposite side. using 2-1/2" nails. /ertical solid sawn blocks \_

exceeds the joist spacing.



N = No reinforcement required.
 N reinforced with 3/4° wood structural

panel on one side only.

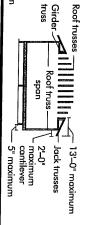
2 = NI reinforced with 3/4" wood structural

panel on both sides, or double I-joist.

X = Try a deeper joist or closer spacing.

2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0"

FIGURE 5 (continued) See table cantilever requirements at reinforcement below for NI Roof truss apan L-maximum cantilever



requirements for a span of 26 ft. shall be permitted to be used the I-joist reinforcement the cantilevered floor joists, trusses running parallel to For hip roofs with the jack

# BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

-5" maximum

JOIST	ROOF		ROOF LOADING	(UNFACTORED)		
OEPTH	SPAN	JOIST SPACING (in.)	LL = 40 psf, JOIST SPA	DL = 15 psf CING (in.)	LL = 50 psf, JOIST SPA	DL = 15 psf CING (in.)
	(11)	2	12 16	19.2 24	12 16	19.2 24
	228 30	X X X X X X X		XXX		X X
	32 34	¥X	(2)	×		×× ××
	36	X		X X		XXX XXX
	28	)				
11-7/8	32 34	, 2 				
	36 % 38 %	×××××××××××××××××××××××××××××××××××××××				
	26 28 30					
Ĭ.	32 34	2 × ×				in in the
	36 38 40		1 X	X	2 X	××× ×××
	26 28 30	1 2 1 2				
16"	32 34	2 1 × 2 ×	- Z Z			4
	36 40	322 X				
Z	42					

For larger openings, or multiple 3-0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple

studs may be required.
Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 2" o.c. requirements for lesser spacing.

- 4. For conventional roof construction using a the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the above is equivalent to the distance between ridge beam, the Roof Truss Span column truss is used. distance between the supporting walls as if a
- Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

# INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.

2-1/2" toe-nails at 6" o.c. (typical) —

Rim board joint -

- 8. Tap the second row of panels into place, using a block to protect groove edges.
- 9. Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and nail to assure accurate and consistent spacing.) 1/8 inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the

## FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24	20	167	Maximum Joist Spacing (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
21	2"	2	Common Wire or Spiral Nails
1-3/4"	1-3/4"	1-3/4"	ail Size and Ty Ring Thread Nails or Screws
2"	2"	2"	pe Staples
6"	6"	6"	Maximun of Fas Edges
12"	12"	12"	1 Spacing teners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.

6

- Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manutacturer.

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

### IMPORTANT NOTE:

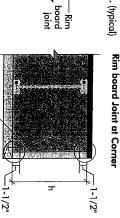
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with

# RIM BOARD INSTALLATION DETAILS

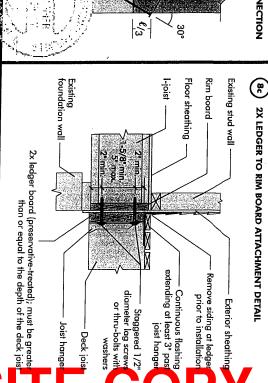
Rim board Joint Between Floor Joists (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

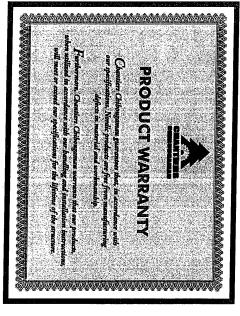


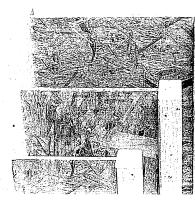
(typical)



(F lop or sole plate Rim board TOE-NAIL CONNECTION AT RIM BOARD







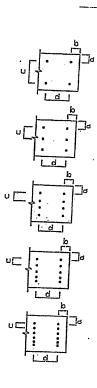
### · MICRO CITY

### Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

	LVL HEA LUMI	LVL HEADER AND CONVENTIONAL LUMBER NAILING DETAILS		
	DETAIL NUMBER	NUMBER OF ROWS	SPACING	
	. A	2	12	
	В	2	8	
	C	2	6	
	D	2	4	
Hammer I.	1A	3	12	
ŀ	1B	3	8	
	1C	3	. 6	
	1D	. 3:	4 .	
	2A	4	. 12 .	
	2B	4	8	
	2C	4	6	
	2D	4	4	
	3A	5	12	
-	3B	5	8	
ŀ	3C	5	. 6	
L	3D	5	4	
1	4A	6 1	12	
L	4B	6	8	
L	4C	6	6	
L	4D	6	4	



### NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
- (2) MINIMUM LUMBER END DISTANCE "b" = 2"
- (3) MINIMUM NAIL ROW SPACING "c" = 2"
- (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
- (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DNG NO TÄNNLOO1. 14

STRUCTURAL

GOMPONENT ONLY

TO BE USED DAVY

WITH BEAM CALES

BEARING THE

STAMP BELOWS

PROVICE NATLING
DETAIL № >/ SEE
DW0 #TAMN1001-14