

20-11-00

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	18
J2	12-00-00	9 1/2" NI-40x	1	34
J3DJ	12-00-00	9 1/2" NI-40x	2	8
J3	10-00-00	9 1/2" NI-40x	1	4
J4	6-00-00	9 1/2" NI-40x	1	8
J5	4-00-00	9 1/2" NI-40x	1	7
J6	2-00-00	9 1/2" NI-40x	1	2
B1	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5A	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B16	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B18L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B17	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

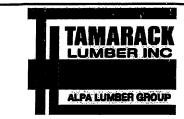
Connector Summary				
Qty	Manuf	Product		
13	H1	IUS2.56/9.5		
10	H1	IUS2.56/9.5		
4	H1	IUS2.56/9.5		
6	H1	IUS2.56/9.5		
2	H2	HGUS410		
1	H3	HUS1.81/10		
4	H3	HUS1.81/10		

TOWN OF BRADFORD WEST GWILLIMBURY BUILDING DEPARTMENT PLANS EXAMINED ONTARIO BUILDING CODE APPLIES

DATE: 2018-10-19

INSPECTOR: BG





FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-3 SONOMA 3

ELEVATION: A,B,C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

LOADING:

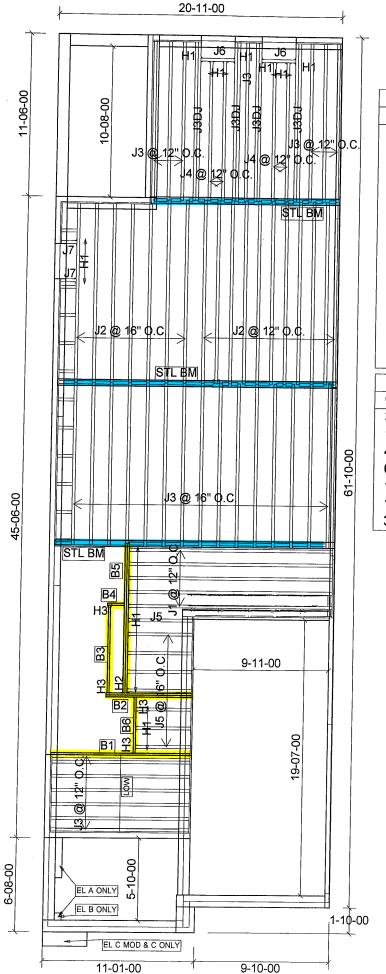
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 14/02/2018

1st FLOOR

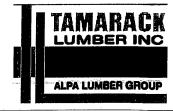
SUNKEN



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	5
J2	14-00-00	9 1/2" NI-40x	1	18
J3	12-00-00	9 1/2" NI-40x	1	29
J3DJ	12-00-00	9 1/2" NI-40x	2	8
J4	10-00-00	9 1/2" NI-40x	1	4
J5	6-00-00	9 1/2" NI-40x	1	8
J6	4-00-00	9 1/2" NI-40x	1	2
J7	2-00-00	9 1/2" NI-40x	1	2
B1	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary				
Qty	Manuf	Product		
3	H1	IUS2.56/9.5		
10	H1	IUS2.56/9.5		
4	H1	IUS2.56/9.5		
6	H1	IUS2.56/9.5		
1	H2	HGUS410		
1	H3	HUS1.81/10		
3	H3	HUS1.81/10		





**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-3 SONOMA 3

ELEVATION: A,B,C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6. LOADING:

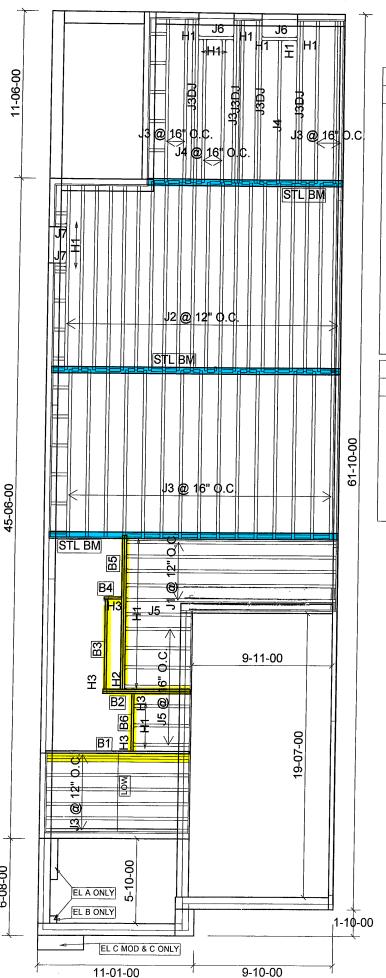
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

# 1st FLOOR

STANDARD



Products				
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	5
J2	14-00-00	9 1/2" NI-40x	1	21
J3	12-00-00	9 1/2" NI-40x	1	28
J3DJ	12-00-00	9 1/2" NI-40x	2	8
J4	10-00-00	9 1/2" NI-40x	1	3
J5	6-00-00	9 1/2" NI-40x	1	8
J6	4-00-00	9 1/2" NI-40x	1	2
J7	2-00-00	9 1/2" NI-40x	1	2
B1	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B6	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary				
Qty	Manuf	Product		
3	H1	IUS2.56/9.5		
10	H1	IUS2.56/9.5		
4	H1	IUS2.56/9.5		
5	H1	IUS2.56/9.5		
1	H2	HGUS410		
1	H3	HUS1.81/10		
3	H3	HUS1.81/10		





**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY E AST

MODEL: SD25-3 SONOMA 3

ELEVATION: A,B,C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2.

O.B.C 9.30.6. LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

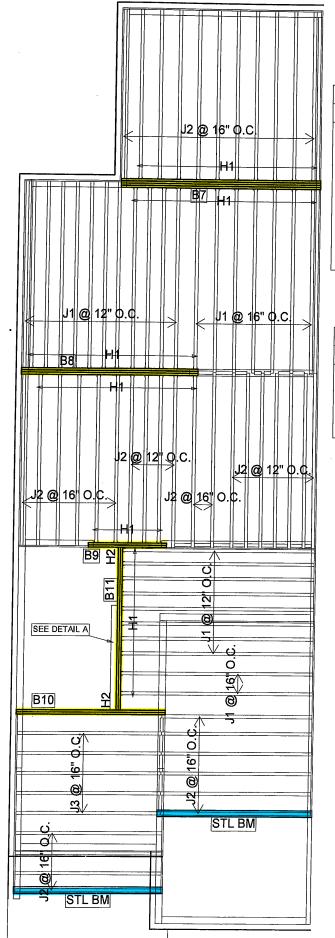
SUBFLOOR: 5/8" GLUED AND NAILED

**CERAMIC TILE APPLICATION AS PER** 

DATE: 01/09/2017

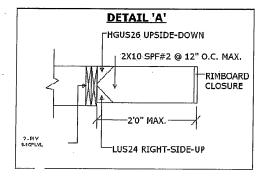
# 1st FLOOR

STANDARD WITH ALT



Products				
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	29
J2	12-00-00	9 1/2" NI-40x	1	40
J3	10-00-00	9 1/2" NI-40x	1	5
B8	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	4	4
B10	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

Connector Summary				
Qty	Manuf	Product		
15	H1	IUS2.56/9.5		
22	H1	IUS2.56/9.5		
21	H1	IUS2.56/9.5		
2	H2	HGUS410		





**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY E AST

MODEL: SD25-3 SONOMA 3

**ELEVATION:** A

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

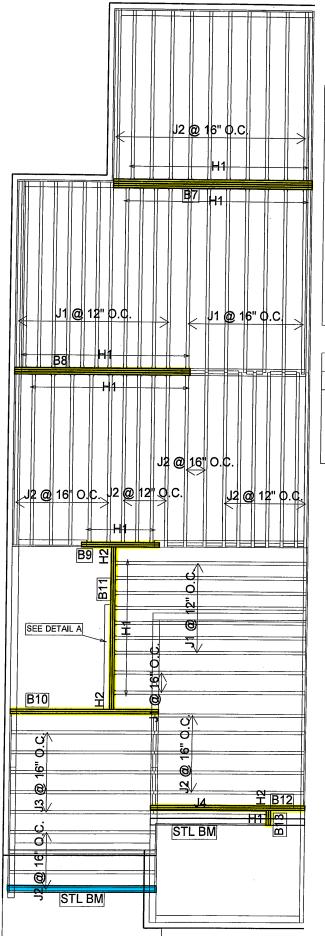
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DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

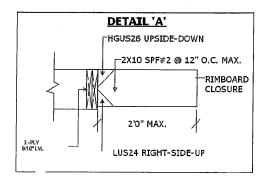
DATE: 28/08/2017

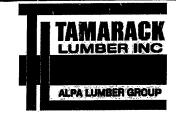
2nd FLOOR



Products				
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	28
J2	12-00-00	9 1/2" NI-40x	1	39
J3	10-00-00	9 1/2" NI-40x	1	5
J4	8-00-00	9 1/2" NI-40x	1	1
B8	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	4	4
B10	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B12	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B13	2-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	Connector Summary				
Qty Manuf		Product			
15	H1	IUS2.56/9.5			
22	H1	IUS2.56/9.5			
21	H1	IUS2.56/9.5			
3	H2	HGUS410			





**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-3 SONOMA 3

ELEVATION: B

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

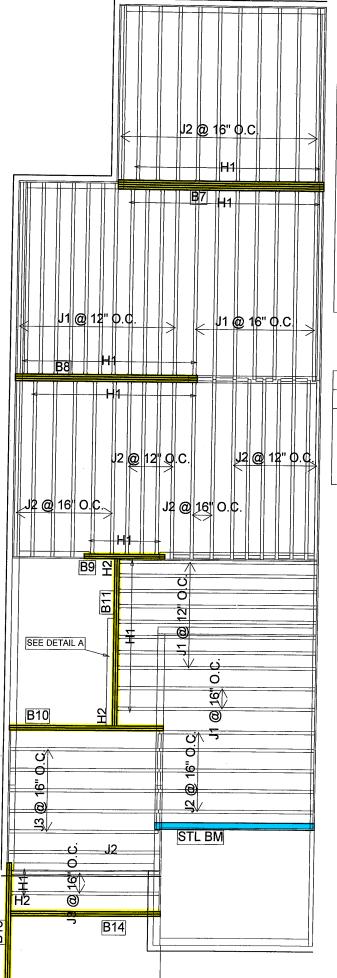
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

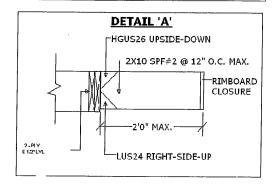
DATE: 28/08/2017

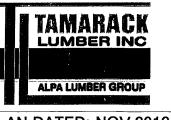
2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	29
J2	12-00-00	9 1/2" NI-40x	1	36
J3	10-00-00	9 1/2" NI-40x	1	7
B8	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	4	4
B10	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B11	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B14	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B15	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	Connector Summary				
Qty	Manuf	Product			
17	H1	IUS2.56/9.5			
22	H1 .	IUS2.56/9.5			
21	H1	IUS2.56/9.5			
3	H2	HGUS410			





**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY EAST

MODEL: SD25-3 SONOMA 3

**ELEVATION:** C

LOT:

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

**NOTES:** 

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DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 28/08/2017

2nd FLOOR



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1556)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report



Diy| i Spair| ivo carrillovoio | 0/12 010 pt

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1556)

Specifier:

Designer: C2

Company:

Misc:

Job Name: Address:

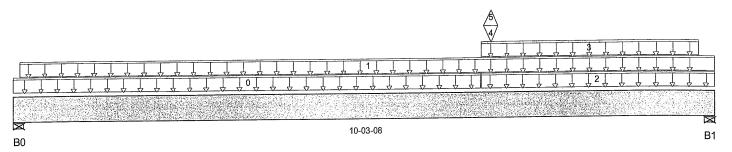
Build 5033

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R



Total Horizontal Product Length = 10-03-08

Reaction Summary (Down / Uplift) (Ibs)								
Bearing	Live	De ad	Snow	Wind				
B0, 5-1/2"	370/174	467/0						
B1, 5-1/2"	680/401	536/0						

						Live	Dead	Snow	Wind	Trib.
	ad Summary g Description	Load Type	, Re	Ref. Start		1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	06-10-08	31	15			n/a
1	UserLoad	Unf. Lin. (lb/ft)	L	00-01-02	10-03-08		60			n/a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	06-10-08	10-03-08	25	13			n/a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	06-10-08	10-00-12	28	14			n/a
4	B11(i1559)	Conc. Pt. (lbs)	L	07-00-04	07-00-04	661	97			n/a
•	B11(i1559)	Conc. Pt. (lbs)	L	07-00-04	07-00-04	-575				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,836 ft-lbs	25,408 ft-lbs	15.1%	1	07-00-04
Neg. Moment	-793 ft-lbs	-25,408 ft-lbs	3.1%	4	07-00-04
End Shear	1.451 lbs	11,571 lbs	12.5%	1	09-00-08
Uplift	119 lbs	n/a	n/a	4	10-03-08
Total Load Defl.	L/999 (0.083")	n/a	n/a	6	05-05-10
Live Load Defl.	L/999 (0.046")	n/a	n/a	8	05-05-10
Max Defl.	0.083"	n/a	n/a	6	05-05-10
Span / Depth	12	n/a	n/a		00-00-00

Reari	ng Supports	Dim . (L x W)	De mand	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	1,138 lbs	11.1%	4.8%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	1,690 lbs	16.4%	7.2%	Unspecified

**Cautions** 

Uplift of 119 lbs found at span 1 - Right. (SIMSON ITSAE O.BI)

Notes

SITE COPY



DWG NO. TAM 92.24.0 STRUCTURAL COMPONENT ONLY

Page 1 of 2



Build 5033

Job Name:

Address:

# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1556)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report



File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i15t

Specifier:

Designer: CZ

Company. Misc:

Customer: CCMC 12472-R Code reports:

City, Province, Postal Code: BRADFORD,

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

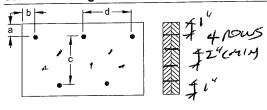
Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Connection Diagram



c = 3 - 1/2" a minimum = # b minimum = 3"

Calculated Side Load = 24.3 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

3-1/2" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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SITE COPY

DWG NO. TAM 9240. STRUCTURAL COMPONENT ONLY



# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1809)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 13:32:38

BC CALC® Design Report

Build 5033 Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1809)

Specifier:

CZ Designer: Company:

Misc:

	\frac{1}{}	
1		
		×
× Company Comp	10-02-06	В1
B0		

Total Horizontal Product Length = 10-02-06

Reaction Summary (Dow	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2" B1, 4-3/8"	245/0 396/0	151/0 229/0			

			1	Live	Dead	Snow Wind	Trib.
Load Summary	Load Type	Ref. Start	End 1	1.00	0.65	1.00 1.15	
Tag Description			10-02-06	12	6		n/a
0 FC2 Floor Material	Unf. Lin. (lb/ft)	L 06-00-08	10-02-00	12	U		!
U FCZ HOOF Waterial	` '	1 06-01-06	06-01-06	592	306		n/a
1 B6(i1843)	Conc. Pt. (lbs)	L 00-01-00	00-01-00	002			

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,070 ft-lbs	7,525 ft-lbs		1	06-01-06 09-00-08
End Shear	844 lbs	5,785 lbs n/a	14.6% n/a	1 4	05-05-05
Total Load Defl. Live Load Defl.	L/999 (0.113") L/999 (0.072")	n/a	n/a	5	05-05-05
Max Defl.	0.113"	n/a	n/a	4	05-05-05
Span / Depth	12	n/a	n/a		00-00-00

a la Companda	Dim . (L × W)	Demand	Demand/ Resistance Support	De mand/ Resistance Member	Material	
Be aring Supports B0 Wall/Plate B1 Wall/Plate	5-1/2" x 1-3/4"	556 lbs	10.8%	4.7%	Un specified	
	4-3/8" x 1-3/4"	880 lbs	21.5%	9.4%	Un specified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 05-07-00, Bottom: 05-07-00.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

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DWG NO. TAM 9U41-18 STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1444)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:05

Build 5033

Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

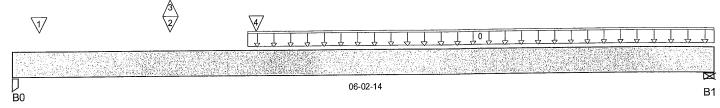
Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1444)

Specifier:

Designer:

Company:

Misc:



## Total Horizontal Product Length = 06-02-14

Reaction Summary (Down / Uplift) (Ibs)							
Be aring	Live	De ad	Snow	Wind			
B0, 3-1/2"	1,347 / 5	770/0					
B1 4-3/8"	489/1	286/0					

ء ا	ad Summary					Live	Dead	Snow	Wind	irib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-01-00	06-02-14	27	13			n/a
1	B3(i1320)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	87	73			n/a
2	B5(i1449)	Conc. Pt. (lbs)	L	01-04-12	01-04-12	1,048	563			n/a
3	B5(i1449)	Conc. Pt. (lbs)	L	01-04-12	01-04-12	-6				n/a
4	B6(i1550)	Conc. Pt. (lbs)	L	02-01-14	02-01-14	584	301			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,558 ft-lbs	25,408 ft-lbs	14%	1	02-01-14
End Shear	2,748 lbs	11,571 lbs	23.7%	1	01-01-00
Total Load Defl.	L/999 (0.025")	n/a	n/a	6	02-09-09
Live Load Defl.	L/999 (0.016")	n/a	n/a	8	02-09-09
Max Defl.	0.025"	n/a	n/a	6	02-09-09
Span / Depth	7.2	n/a	n/a		00-00-00

				De mand/ Resistance	Demand/ Resistance	
Bearing Supports		Dim . (L x W)	Demand	Support	Member	Material
B0	Post	3-1/2" x 3-1/2"	2,983 lbs	30%	20%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	1,092 lbs	13.3%	5.8%	Unspecified

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-01-00, Bottom: 00-01-00.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

SITE COPY



DWG NO. TAM 9242-18 STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1444)

\*

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:05

Build 5033

BC CALC® Design Report

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

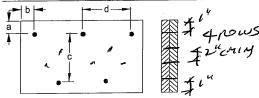
Description: Designs\Flush Beams\Basment\Flush Beams\B2(i144

Specifier:

Designer: Ca Company:

Misc:

## Connection Diagram



Calculated Side Load = 398.8 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d 🔭 Nails

3-1/2" ARDOX SPIRAL

### Disclosure

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DWG NO. TAM 9242 - STRUCTURAL COMPONENT ONLY





# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2098)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 13:32:38

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

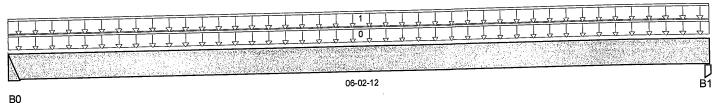
CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl Description: Designs\Flush Beams\Basment\Flush Beams\B3(i2098)

Specifier:

Designer:

Company. Misc:



Total Horizontal Product Length = 06-02-12

Reaction Summary (Down / U	plift) (lbs) Live	Le au	Snow	Wind	
B0 B1, 1-3/4"	207/0 206/0	119/0 118/0			

				Live	Dead	Snow Wind	Trib.
Load Summary	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	<del>_</del>
Tag Description	Unf. Lin. (lb/ft)	1 00-00-00	06-02-12	40	20		n/a
Us er Load     FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	06-02-12	26	13		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	670 ft-lbs	12,704 ft-lbs	5.3%	1	03-01-08
End Shear	318 lbs	5,785 lbs	5.5%	1	00-11-08
Total Load Defl.	L/999 (0.012")	n/a	n/a	4	03-01-08
Live Load Defl.	L/999 (0.008")	n/a	n/a	5	03-01-08
	0.012"	n/a	n/a	4	03-01-08
Max Defl. Span / Depth	7.6	n/a	n/a		00-00-00

<b>D</b>	Cumporto	F		De mand/ Resistance Support	Demand/ Resistance Member	Material	
Bearing Supports B0 Hanger B1 Post		2" x 1-3/4"	459 lbs	n/a	10.7%	Hanger	
		1-3/4" x 1-3/4"	456 lbs	18.3%	12.2%	Unspecified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

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DWG NO. TAM 92 STRUCTÚRAL COMPONENT ONLY





# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i2097)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 13:32:38

BC CALC® Design Report

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i2097)

Specifier:

Designer:

Company.

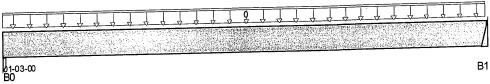
Misc:

Address: City, Province, Postal Code:BRADFORD, Customer:

**Build 5033** Job Name:

Code reports:

CCMC 12472-R



Total Horizontal Product Length = 01-03-00

Reaction Summary (Dowr	ı / Uplift) (lbs) Live	De ad	Snow	Wind
B0, 3-1/2" B1	85 / 0 69 / 0	46 / 0 37 / 0		

				Live	Dead	Snow Wind	Trib.
Load Summary	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
Tag Description  O User Load	Unf. Lin. (lb/ft)	L 00-00-00	01-03-00	124	62		n/a

Controls Summary	ractored ractored -		Demand / Resistance	Load Case	Location
Pos. Moment End Shear Span / Depth	28 ft-lbs 107 lbs 1.2	12,704 ft-lbs 5,785 lbs n/a	0.2% 1.8% n/a	1	00-08-04 01-01-00 00-00-00

Bearing Supports B0 Post B1 Hanger		Dim . (L x W)	Demand	De man d/ Re sistance Support	De mand/ Resistance Member	Materia!	
		3-1/2" x 1-3/4" 2" x 1-3/4"	185 lbs 151 lbs	3.7% n/a	2.5% 3.5%	Unspecified Hanger	

#### Notes

Calculations assume unbraced length of Top: 00-01-00, Bottom: 00-01-00. Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9

## Disclosure

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DWG NO. TAM STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1449)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:05

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

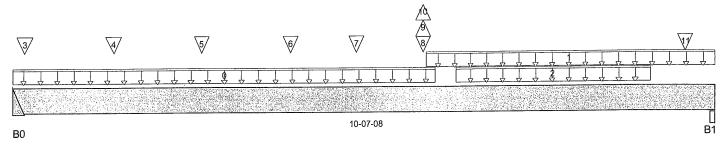
Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1449)

Specifier:

Designer: CZ

Company:

Msc:



Total Horizontal Product Length = 10-07-08

Reaction Summary (Down / Uplift) ( lbs )											
Be aring	Live	De ad	Snow	Wine	d						
BO	1,046 / 6	563/0									
B1, 2-5/8"	2,135 / 8	1,102/0									
Land Commons				Live	Dead	Snow	Wind	Trib.			
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15				
0 FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	06-04-08	26	13			n/a			
1 User Load	Unf. Lin. (lb/ft)	L 06-02 <b>-</b> 12	10-07-08	240	120			n/a			

Tag	Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	06-04-08	26	13			n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	06-02 <del>-</del> 12	10-07-08	240	120			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	06-08-02	09-08-02	306	154			n/a
3	J5(i1553)	Conc. Pt. (lbs)	L	00-02-02	00-02-02	80	40			n/a
4	J5(i1560)	Conc. Pt. (lbs)	L	01-06-02	01-06-02	125	63			n/a
5	J5(i1567)	Conc. Pt. (lbs)	. L	02-10-02	02-10-02	125	63			n/a
6	J5(i1554)	Conc. Pt. (lbs)	L	04-02-02	04-02-02	109	55			n/a
7	J5(i1566)	Conc. Pt. (lbs)	L	05-02-02	05-02-02	94	47			n/a
8	-	Conc. Pt. (lbs)	L	06-02-03	06-02-03	281	117			n/a
9	_	Conc. Pt. (lbs)	L	06-02-03	06-02-03		-7			n/a
10	_	Conc. Pt. (lbs)	L	06-02-03	06-02-03	-14				n/a
11	J1(i1502)	Conc. Pt. (lbs)	L	10-02-02	10-02-02	221	110			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	8,721 ft-lbs	25,408 ft-lbs	34.3%	1	06-06-15
End Shear	3,720 lbs	11,571 lbs	32.1%	1	09-07-06
Total Load Defl.	L/550 (0.226")	0.518"	43.6%	6	05-06-10
Live Load Defl.	L/835 (0.149")	0.345"	43.1%	8	05-06-10
Max Defl.	0.226"	n/a	n/a	6	05-06-10
Span / Depth	13.1	n/a	n/a		00-00-00

				De mand/	Demand/		
				Resistance	Resistance		
Bearing Supports		Dim.(LxW)	Demand	Support	Member	Material	
B0	Hanger	2" x3-1/2"	2,273 lbs	n/a	26.6%	HGUS4 10	
B1	Beam	2-5/8" x 3-1/2"	4,580 lbs	93.3%	40.9%	Unspecified	

**Notes** 

SITE COPY



DWG NO. TAM 9245 of STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1449)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:05

BC CALC® Design Report Build 5033 Job Name:

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i144

Specifier:

Designer: Company:

Misc:

Address: City, Province, Postal Code: BRADFORD,

Customer: Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

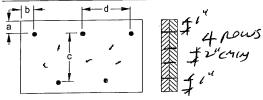
O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

**CONFORMS TO OBC 2012** 

## Connection Diagram



c = 3 - 1/2". a minimum = 2" b minimum = 3"

Calculated Side Load = 390.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d

3-1/2" ARDOX SPIRAL

#### Disclosure

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DWG NO. TAM THE STRUCTURAL COMPONENT ONLY







## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

**PASSED** 

## Basment\Flush Beams\B5A(i1935)

BC CALC® Design Report

Dry | 1 span | No cant.

February 14, 2018 10:56:30

**Build 6215** 

Job name: Address:

City, Province, Postal Code: BRA...RD

Customer: Code reports:

CCMC 12472-R

File name:

SD25-3 SONOMA 3-EL B SUNKEN.mmdl

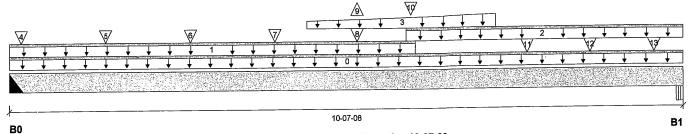
Description: Basment\Flush Beams\B5A(i1935)

Wind

Specifier:

Designer: CZ

Company:



## Total Horizontal Product Length = 10-07-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead		
B0, 2"	839 / 7	477 / 0		
B1. 2-5/8"	1,392 / 8	753 / 0		

10	ad Summary					Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-07-08		10			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	06-04-08	26	13			n\a
2	User Load	Unf. Lin. (lb/ft)	L	06-02-12	10-07-08	240	120			n\a
3	Smoothed Load	Trapezoidal (lb/ft)	L	04-08-02		53	27			n\a
•	Omoonica Load	,			07-08-02	91	45			
4	J4(i1948)	Conc. Pt. (lbs)	L	00-02-02	00-02-02	80	40			n\a
5	J4(i1947)	Conc. Pt. (lbs)	L	01-06-02	01-06-02	125	63			n\a
6	J4(i1952)	Conc. Pt. (lbs)	Ĺ	02-10-02	02-10-02	125	63			n\a
7	J4(i1942)	Conc. Pt. (lbs)	Ĺ	04-02-02	04-02-02	109	55			n\a
8	B17(i1960)	Conc. Pt. (lbs)	L	05-05-08	05-05-08	43	31			n\a
9	B17(i1960)	Conc. Pt. (lbs)	Ĺ	05-05-08	05-05-08	-15				n\a
	B4(i1959)	Conc. Pt. (lbs)	Ĺ	06-03-10	06-03-10	65	35			n\a
10	,	Conc. Pt. (lbs)	Ī	08-02-02	08-02-02	88	44			n\a
11	J5(i1511)	Conc. Pt. (lbs)	ī	09-02-02	09-02-02	88	44			n\a
12	J5(i1511)	Conc. Pt. (lbs)	ı	10-02-02	10-02-02	64	32			n\a
13	J5(i1502)	COHC. Ft. (IDS)	_	10 02 02	, 5 52 62					

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6.166 ft-lbs	23,220 ft-lbs	26.6 %	1	06-03-10
End Shear	2,404 lbs	11,571 lbs	20.8 %	1	09-07-06
Total Load Deflection	L/760 (0.164")	n\a	31.6 %	6	05-05-08
Live Load Deflection	L/999 (0.105")	n\a	n\a	8	05-05-08
Max Defl.	0.164"	n\a	n\a	6	05-05-08
Span / Depth	13.1				

	Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
-	В0	Hanger	2" x 3-1/2"	1,855 lbs	n\a 61.7 %	21.7 % 27.0 %	HGUS410 Unspecified
	B1	Beam	2-5/8" x 3-1/2"	3,029 lbs	01.7 70	27.0 70	Oriopeolitea

**Cautions** 

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity. adequate capacity.





DWG NO. TAM 9246-18 STRUCTURAL COMPONENT ONLY





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

**PASSED** 

Basment\Flush Beams\B5A(i1935)

Dry | 1 span | No cant.

February 14, 2018 10:56:30

BC CALC® Design Report **Build 6215** 

Job name:

Address: City, Province, Postal Code: BRA...RD

Customer: Code reports:

CCMC 12472-R

File name:

SD25-3 SONOMA 3-EL B SUNKEN.mmdl

Description: Basment\Flush Beams\B5A(i1935)

Specifier:

CZ Designer:

Company:

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

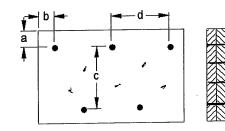
Design based on Dry Service Condition.

**CONFORMS TO OBC 2012** 

Importance Factor: Normal Part code: Part 9

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

**Connection Diagram** 



a minimum = #" b minimum = 3"

Calculated Side Load = 186.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

Nails

3-1/2" ARDOX SPIRAL

### **Disclosure**

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAMB VERSA-RIM PLUS® DWG NO. TAM GALLES STRUCTURAL

COMPONENT ONLY





# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i1550)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:05

BC CALC® Design Report



**Build 5033** Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

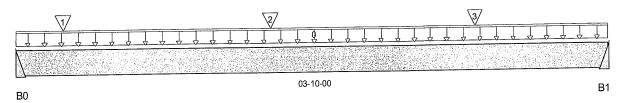
File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i1550)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 03-10-00

Reaction Summary (Down	n / <b>Uplift)</b> ( lbs ) Live	De ad	Snow	Wind	
B0	614/0	317/0			
B1	584/0	301/0			

			L		Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 User Load	Unf. Lin. (lb/ft)	1 00-00-00	03-10-00	240	120		n/a
1 J5(i1565)	Conc. Pt. (lbs)	I 00-03-10	00-03-10	77	39		n/a
2 J5(i1562)	Conc. Pt. (lbs)	I 01-07-10	01-07-10	107	53		n/a
3 J5(i1564)	Conc. Pt. (lbs)	L 02-11-10	02-11-10	94	47		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1.117 ft-lbs	12,704 ft-lbs	8.8%	1	01-09-08
End Shear	733 lbs	5,7851bs	12.7%	1	02-10-08
Total Load Defl.	L/999 (0.007")	n/a	n/a	4	01-10-14
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	01-10-14
Max Defl.	0.007"	n/a	n/a	4	01-10-14
Span / Depth	4.6	n/a	n/a		00-00-00

Bearing Supports		Dim . (L x W)	De man d	De man d/ Re sistance Support	De mand/ Resistance Member	Material	
B0 Hanger		2" x1-3/4"	1,317 lbs	n/a	30.8%	HUS1.81/10	
B1 Hanger		2" x1-3/4"	1,252 lbs	n/a	29.3%	HUS1.81/10	

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL

COMPONENT ONLY



CONFORMS TO OBCZOO



# Quadruple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7(i1508)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

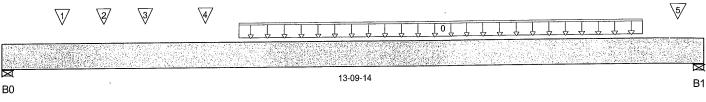
File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i1508)

Specifier:

Designer: CZ Company:

Misc:



### Total Horizontal Product Length = 13-09-14

Reaction Summary (Down / Uplift) (Ibs)									
Be aring	Live	De ad	Snow	Wind					
B0, 4-3/8"	3,093 / 0	1,680 / 0							
B1. 3-1/2"	3,226 / 0	1,743 / 0							

١.	ad Summanı					Live	Dead	Snow	wina	ITID.
	ad Summary  Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
ō	Smoothed Load	Unf. Lin. (lb/ft)	L	04-07-14	12-07-14	486	242			n/a
1	-	Conc. Pt. (lbs)	L	01-02-00	01-02-00	560	281			n/a
2	J1(i1484)	Conc. Pt. (lbs)	L	01-11-14	01-11-14	253	127			n/a
3	-	Conc. Pt. (lbs)	L	02-09-11	02-09-11	562	281			n/a
4		Conc. Pt. (lbs)	L	03-11-14	03-11-14	605	302			n/a
5		Conc. Pt. (lbs)	L	13-03-14	13-03-14	446	223			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	23,372 ft-lbs	52,848 ft-lbs	44.2%	1	06-07-14
End Shear	6,591 lbs	23,142 lbs	28.5%	1	01-01-14
Total Load Defl.	L/305 (0.523")	0.665"	78.8%	4	06-11-14
Live Load Defl.	L/469 (0.34")	0.443"	76.7%	5	06-11-14
Max Defl.	0.523"	n/a	n/a	4	06-11-14
Span / Depth	16.8	n/a	n/a		00-00-00

Bearing Supports				De mand/ Resistance	Demand/ Resistance	
		Dim.(LxW)	De man d	Support	Member	Material
B0	Wall/Plate	4-3/8" x 7"	6,739 lbs	41.2%	18%	Unspecified
B1	Wall/Plate	3-1/2" x 7"	7,018 lbs	53.6%	23.5%	Unspecified

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9





# Quadruple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7(i1508)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report



File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i150\)

Specifier:

Designer: CZ

Company: Misc:

**Build 5033** Job Name:

Address:

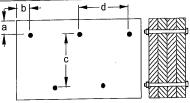
City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

## Connection Diagram



a minimum =  $2\frac{1}{2}^{1}$  c = 41/2" b minimum = 2-1/2"d = 2 1/2"

## Calculated Side Load = 513.4 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Beams 7 inches wide will be assumed to be either top-loaded only, or equally loaded from

Bolts are assumed to be Grade A307 or Grade 2 or higher.

Connectors are: 1/2 in. Staggered Through Bolt

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 92 STRUCTURAL COMPONENT ONLY



# Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B8(i1489)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report

\*

Build 5033 Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

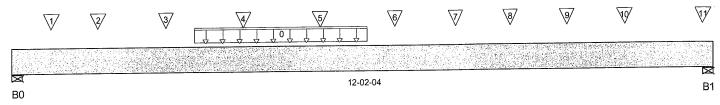
Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i1489)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 12-02-04

Reaction Summary (Down	/ <b>Uplift)</b> ( lbs ) Live	De ad	Snow	Wind	
B0, 5-1/2"	2,641 / 0	1,408 / 0			
B1. 6-3/4"	3,196 / 0	1,688 / 0			

	.1.0					Live	Dead	Snow	Wind	ITID.
	ad Summary  Description	Load Type	Ref	f. Start	End	1.00	0.65	1.00	1.15	<del></del>
0	Smoothed Load	Unf. Lin. (lb/ft)	L	03-02-00	06-02-00	248	124			n/a
1	J1 (i1503)	Conc. Pt. (lbs)	L	00-80-00	00-08-00	185	93			n/a
2	-	Conc. Pt. (lbs)	L	01-05-14	01-05-14	538	269			n/a
2	- -	Conc. Pt. (lbs)	L	02-08-00	02-08-00	561	280			n/a
4	- J2(i1733)	Conc. Pt. (lbs)	Ĺ	04-00-00	04-00-00	309	154			n/a
5	J2(i1738)	Conc. Pt. (lbs)	L	05-04-00	05-04-00	311	156			n/a
6	52(11750)	Conc. Pt. (lbs)	L	06-07-07	06-07-07	524	262			n/a
7	_	Conc. Pt. (lbs)	L	07-08-00	07-08-00	495	248			n/a
8	- -	Conc. Pt. (lbs)	L	08-08-00	08-08-00	483	242			n/a
9	_	Conc. Pt. (lbs)	L	09-08-00	09-08-00	483	242			n/a
10	- -	Conc. Pt. (lbs)	L	10-08-00	10-08-00	562	281			n/a
11	-	Conc. Pt. (lbs)	L	12-00-00	12-00-00	642	321			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	16,636 ft-lbs	39,636 ft-lbs	42%	1	05-08-00
End Shear	5,514 lbs	17,356 lbs	31.8%	1	10-10-00
Total Load Defl.	L/375 (0.361")	0.565"	64%	4	06-01-07
Live Load Defl.	L/574 (0.236")	0.376"	62.7%	5	06-01-07
Max Defl.	0.361"	n/a	n/a	4	06-01-07
Span / Depth	14.3	n/a	n/a		00-00-00

<b>R</b> o ari	ng Supports	Dim . (L x W)	De mand	De man d/ Re sistance Support	Resistance Member	Material
B0	Wall/Plate Wall/Plate	5-1/2" x 5-1/4"	5,722 lbs	37.1%	16.2%	Unspecified
B1		6-3/4" x 5-1/4"	6,903 lbs	36.5%	16%	Unspecified

Notes





DWG NO. TAM 9249-18
STRUCTURAL
COMPONENT ONLY



## Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B8(i1489)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report Build 5033

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B8(i148)

Specifier:

Company.

Designer:

Misc:

Customer:

Code reports:

Job Name:

Address:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

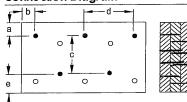
Design based on Dry Service Condition.

City, Province, Postal Code: BRADFORD,

Importance Factor: Normal Part code: Part 9

**CONFORMS TO OBC 2012** 

## Connection Diagram



a minimum = 🗗 b minimum = 3" e minimum = 2"

Calculated Side Load = 535.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d Stoke Nails

3-1/2" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 92 STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1538)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report



**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

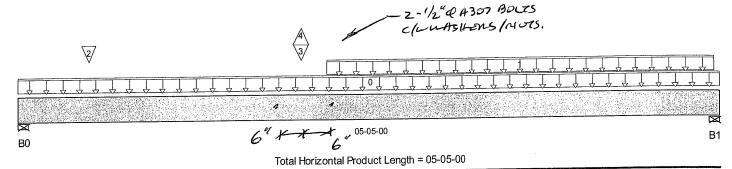
Description: Designs\Flush Beams\1st Floor\Flush Beams\89(i1538)

Specifier:

Designer:

Company:

Misc:



Reaction Summary (Do	wn / Uplift) (lbs)		Sm avv	Wind
Be aring	Live	De ad	Snow	Willu
B0, 4"	1,768 / 154	1,026 / 0		
B1 4"	1,373 / 103	844/0		

_					Live	Dead	Snow	Wind	irib.
_	oad Summary g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15	
<u></u>	User Load	Unf. Lin. (lb/ft)	L 00-00-	00 05-05-0	00	60			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L 02-04-	08 05-04-0	08 230	115			n/a
2	J2(i1738)	Conc. Pt. (lbs)	L 00-06-	08 00-06-0	08 315	157			n/a
2	-	Conc. Pt. (lbs)	L 02-02-	02 02-02-0	2,112	980			n/a
- 4	- -	Conc. Pt. (lbs)	L 02-02-	02 02-02-0	)2 -257		,		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,160 ft-lbs	25,408 ft-lbs	24.2%	1	02-02-12
End Shear	3.344 lbs	11,571 lbs	28.9%	1	01-01-08
Total Load Defl.	L/999 (0.032")	n/a	n/a	6	02-07-09
Live Load Defl.	L/999 (0.021")	n/a	n/a	8	02-07-09
Max Defl.	0.032"	n/a	n/a	6	02-07-09
Span / Depth	6.2	n/a	n/a		00-00-00

Bearing Supports				Demand/ Resistance	Demand/ Resistance	
		Dim . (L x W)	De man d	Support	Member	Material
B0	Wall/Plate	4" x3-1/2"	3,935 lbs	52.6%	23%	Unspecified
B1	Wall/Plate	4" x3-1/2"	3,115 lbs	41.7%	18.2%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

SITE COP'



DWG NO. TAM 9250 STRUCTURAL COMPONENT ONLY



Build 5033

Job Name:

Address:

# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1538)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs \Flush Beams\1st Floor\Flush Beams\B9(i153{

Specifier:

Designer:

Company.

Customer: CCMC 12472-R Code reports:

City, Province, Postal Code: BRADFORD,

Misc:

Connection Diagram

Concentrated side-load exceeds allowable magnitude for connection design. Please consult a technical representative or Professional Engineer for the design of the connection. OK 4714

BOUTING

PROVIDE4 ROWS OF 3-1/2" ARDOX SPIRAL NAILS @ 4 "O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. / "LUMBER EDGE / END DISTANCE. DO NOT USE AIR NAILS.

BOUTS

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9250 -1 STRUCTÚRAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i1559)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

В1

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

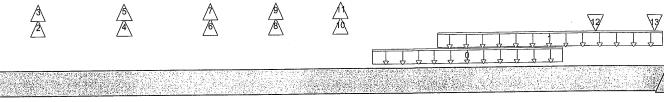
File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i1559)

Specifier:

Designer: CZ Company:

Misc:



B0

10-10-08

Total Horizontal Product Length = 10-10-08

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0	654/578	92 / 0							
B1	1,846 / 254	849/0							

	- d C.,,,,,					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	06-03-08	09-03-08	274	137			n/a
1	UserLoad	Unf. Lin. (lb/ft)	L	07-03-14	10-09-14	240	120			n/a
2	J1 (i1645)	Conc. Pt. (lbs)	L	01-01-08	01-01-08	86	<i>-</i> 55			n/a
3	J1 (i1645)	Conc. Pt. (lbs)	L	01-01-08	01-01-08	-198				n/a
4	J1(i1644)	Conc. Pt. (lbs)	L	02-05-08	02-05-08	89	-50			n/a
5	J1(i1644)	Conc. Pt. (lbs)	L	02-05-08	02-05-08	-188				n/a
6	J1(i1568)	Conc. Pt. (lbs)	L	03-09-08	03-09-08	77	-43			n/a
7	J1(i1568)	Conc. Pt. (lbs)	L	03-09-08	03-09-08	-164				n/a
8	J1(i1561)	Conc. Pt. (lbs)	L	04-09-08	04-09-08	66	-37			n/a
9	J1(i1561)	Conc. Pt. (lbs)	L	04-09-08	04-09-08	-141				n/a
10	J1(i1558)	Conc. Pt. (lbs)	L	05-09-08	05-09-08	66	-37			n/a
11	J1(i1558)	Conc. Pt. (lbs)	L	05-09-08	05-09-08	-141				n/a
12	J1(i1555)	Conc. Pt. (lbs)	L	09-09-08	09-09-08	260	130			n/a
13	J1 (i1549)	Conc. Pt. (lbs)	L	10-08-04	10-08-04	194	97			n/a

	Factored	Factore d	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	6,143 ft-lbs	25,408 ft-lbs	24.2%	1	07-06-11
Neg. Moment	-1,593 ft-lbs	-25,408 ft-lbs	6.3%	4	03-09-08
End Shear	2,955 lbs	11,571 lbs	25.5%	1	09-11-00
Uplift	784 lbs	n/a	n/a	4	00-00-00
Total Load Defl.	L/825 (0.155")	0.533"	29.1%	6	06-00-08
Live Load Defl.	L/999 (0.117")	n/a	n/a	8	05-09-08
Total Neg. Defl.	L/999 (-0.018")	n/a	n/a	7	03-09-08
Max Defl.	0.155"	n/a	n/a	6	06-00-08
Span / Depth	13.5	n/a	n/a		00-00-00

De mand/ De mand/ Resistance Resistance

Dim. (LxW) De mand **Bearing Supports** 

Material Member



DWG NO. TAM 425/-18
STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i1559)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i15t

Specifier:

Designer: CZ

Company.

Misc:

Customer:

B0

B0

В1

Build 5033

Job Name:

Address:

Code reports: CCMC 12472-R

City, Province, Postal Code:BRADFORD,

HGUS410 2" x 3-1/2" 1.096 lbs n/a 12.8% Hanger HGUS410 n/a 0.07 2" x 3-1/2" 784 lbs Hanger Uplift HGUS410 2" x3-1/2" 3.831 lbs n/a 44.9% Hanger

Cautions

Uplift of 784 lbs found at span 1 - Left. Hanger B0 cannot handle uplift of -784 lbs. (SIMSON HOUS410@0.80)

**Notes** 

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

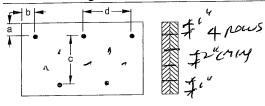
Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = 🛊 " c = 3 - 1/2" b minimum = 3

Calculated Side Load = 162.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

. . . Mails Connectors are: 4

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i1637)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report



Build 5033 Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

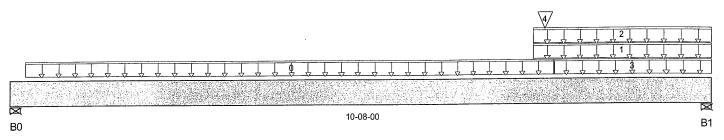
File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i1637)

Specifier:
Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 10-08-00

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 5-1/2"	184/0	191/0	55 / 0					
B1, 3-1/2"	394/0	588/0	408/0					

1.0	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	Ref. Start		1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	08-03-08	27	13			n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	07-11-10	10-08-00	33	128	72		n/a
2	LOWROOF	Unf. Lin. (lb/ft)	L	07-11-10	10-08-00	42	40	90		n/a
3	FC3 Floor Material	Unf. Lin. (lb/ft)	L	08-03-08	10-08-00	23	12			n/a
4	B13(i1652)	Conc. Pt. (lbs)	L	08-01-12	08-01-12	105	86	25		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,037 ft-lbs	25,408 ft-lbs	8%	1	08-00-00
End Shear	1,023 lbs	11,571 lbs	8.8%	1	09-07-00
Total Load Defl.	L/999 (0.053")	n/a	n/a	35	05-09-03
Live Load Defl.	L/999 (0.028")	n/a	n/a	51	05-09-03
Max Defl.	0.053"	n/a	n/a	35	05-09-03
Span / Depth	12.7	n/a	n/a		00-00-00

Bearing Supports				Resistance	Resistance	
		Dim.(LxW)	Dim. (L x W) Demand		Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	543 lbs	5.3%	2.3%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	1,544 lbs	23.6%	10.3%	Unspecified

Notes



DWG NO. TAM 975 2. STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i1637)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:03

BC CALC® Design Report

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i163

Specifier:

CZ Designer: Company:

Misc:

Address: City, Province, Postal Code:BRADFORD.

Customer:

Build 5033

Job Name:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

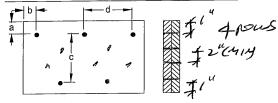
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Connection Diagram



c = 8-1/2" a minimum = 2" d= @ 6 b minimum = 3"

Calculated Side Load = 26.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

3-1/2" ARDOX SPIRAL

#### Disclosure

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# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i1652)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report



File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i1652)

Specifier:

Designer: CZ

Company:

Misc:

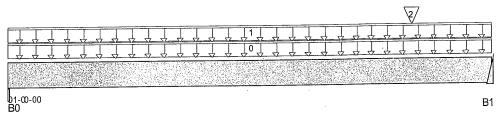
Address: City, Province, Postal Code:BRADFORD, Customer:

Build 5033

Job Name:

Code reports:

CCMC 12472-R



Total Horizontal Product Length = 01-00-00

Reaction Summary (Down / Uplift) (Ibs)								
Be aring	Live	De ad	Snow	vvina				
B0, 5-1/4"	39 / 0	101/0	61 / 0					
B1 .	98 / 0	96 / 0	35 / 0					

1.0				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1.15	
0 User Load	Unf. Lin. (lb/ft)	1. 00-00-00	01-00-00	33	130	72	n/a
1 LOWROOF	Unf. Lin. (lb/ft)	L 00-00-00	01-00-00	11	10	24	n/a
2 J4(i1626)	Conc. Pt. (lbs)	L 00-10-00	00-10-00	93	47		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	17 ft-lbs	n/a	n/a	1	00-08-09
End Shear	172 lbs	11,571 lbs	1.5%	11	80-00-00
Span / Depth	0.7	n/a	n/a		00-00-00

Bearing Supports				Demand/ Resistance	Demand/ Resistance	
		Dim.(LxW)	De m an d	Support	Member	Material
B0	Beam	5-1/4" x 3-1/2"	237 lbs	2.4%	1.1%	Unspecified
B1	Hanger	2" x3-1/2"	285 lbs	n/a	3.3%	HGUS410

#### Notes

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9





4

DWG NO. TAM 9253.78
STRUCTURAL
COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i1652)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report

\*

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i16t

Specifier:

Designer: CZ Company:

Misc:

Job Name: Address:

**Build 5033** 

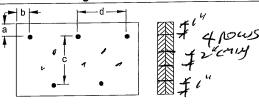
City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

Connection Diagram



Calculated Side Load = 198.3 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 4-7

3-1/2" ARDOX SPIRAL

#### Disclosure

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DWG NO. TAM 9283 - 8
STRUCTURAL
COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i1763)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

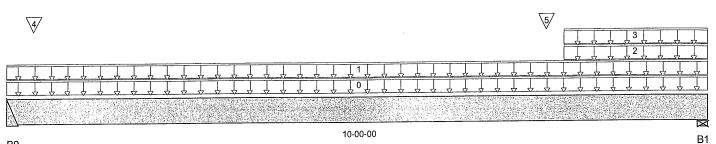
CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i1763)

Specifier:

Designer: CZ Company:

Misc:



В0

Total Horizontal Product Length = 10-00-00

Reaction Summary (Down / Uplift) ( lbs )								
Be aring	Live	De ad	Snow	Wind				
B0	434/0	890/0	639/0					
B1, 5-1/2"	476/0	950/0	711/0					

	ad Cumman					Live	Dead	Snow	Wind	Trib.
	oad Summary g Description	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
0	wall	Unf. Lin. (lb/ft)	L	00-00-00	10-00-00		100			n/a
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-00-00	29	15			n/a
2	low roof	Unf. Lin. (lb/ft)	L	07-11-08	10-00-00	33	30	72		n/a
3	roof	Unf. Lin. (lb/ft)	L	07-11-08	10-00-00	33	30	72		n/a
4	User Load	Conc. Pt. (lbs)	L	00-04-08	00-04-08	242	238	528		n/a
5	User Load	Conc. Pt. (lbs)	L	07-08-08	07-08-08	242	238	528		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,393 ft-lbs	16,515 ft-lbs	14.5%	0	05-03-06
End Shear	1,876 lbs	11,571 lbs	16.2%	13	08-09-00
Total Load Defl.	L/999 (0.093")	n/a	n/a	45	05-00-14
Live Load Defl.	L/999 (0.036")	n/a	n/a	61	05-03-06
Max Defl.	0.093"	n/a	n/a	45	05-00-14
Span / Depth	12	n/a	n/a		00-00-00

Bearing Supports		Dim . (L x W)	De m an d	De mand/ Resistance Support	Demand/ Resistance Member	Material	
B0	Hanger	2" x3-1/2"	2,288 lbs	n/a	26.8%	Hanger	
B1	Wall/Plate	5-1/2" x 3-1/2"	2,493 lbs	24.3%	10.6%	Unspecified	

**Notes** 





DWG NO. TAM 9254 -18 STRUCTURAL COMPONENT ONLY



# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i1763)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i176

Specifier:

Designer:

Company:

Misc:

Customer:

Build 5033

Job Name:

Address:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced. Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

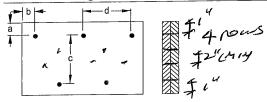
Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

City, Province, Postal Code: BRADFORD,

Importance Factor: Normal Part code: Part 9

## Connection Diagram



a minimum = 1" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Spiker Nails

3-1/2" ARDOX SPIRAL

#### Disclosure

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DWG NO. TAM 9254 . STRUCTURAL COMPONENT ONLY





# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i1762)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 13:44:04

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

, , , , ,

File Name: SD25-3 SONOMA 3-EL C.mmdl

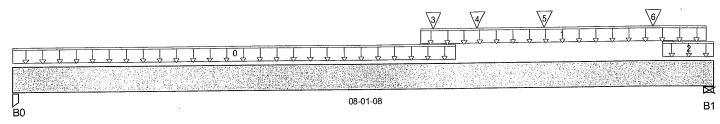
Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i1762)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 08-01-08

Reaction Summary (Do	wn / Uplift) (Ibs) Live	De ad	Snow	Wind	
B0, 3-1/2"	366/0	595/0	527/0		
B1, 5-1/2"	899/0	1,251 / 0	695/0		

irib.
n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,898 ft-lbs	23,481 ft-lbs	25.1%	13	04-10-04
End Shear	2.491 lbs	11,571 lbs	21.5%	1	06-10-08
Total Load Defl.	L/999 (0.078")	n/a	n/a	45	04-03-01
Live Load Defl.	L/999 (0.039")	n/a	n/a	61	04-01-11
Max Defl.	0.078"	n/a	n/a	45	04-03-01
Span / Depth	9.5	n/a	n/a		00-00-00

				De mand/	De mand/		
				Resistance	Resistance		
Beari	ng Supports	Dim . (L x W)	Demand	Support	Member	Material	
B0	Post	3-1/2" x 3-1/2"	1,718 lbs	17.3%	11.5%	Unspecified	
B1	Wall/Plate	5-1/2" x 3-1/2"	3,259 lbs	31.7%	13.9%	Unspecified	











## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i1762)

August 28, 2017 13:44:04

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i176

Specifier:

Designer:

Company. Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 04-08-08, Bottom: 04-08-08. Resistance Factor phi has been applied to all presented results per CSA O86.

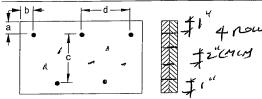
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Connection Diagram



a minimum = 2" b minimum = 3"

Calculated Side Load = 421.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Common Nails

3-1/2" ARDOX SPIRAL

#### Disclosure

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# Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B16(i1761)

BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 14:35:31

**Build 5033** 

Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL B.mmdl

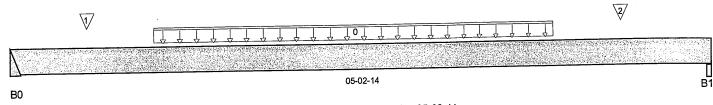
Description: Designs\Flush Beams\Basment\Flush Beams\B16(i1761

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 05-02-14

•		Total Florizontal I	Toddot Longai. To		
Reaction Summary (Dov Bearing	Live	De ad	Snow	Wind	
B0 B1, 5-1/4"	186/0 192/0	105/0 109/0			

					Live	Dead	Snow Wind	Trib.
	ad Summary	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
la	g Description	Unf. Lin. (lb/ft)	L 01-00-14	04-00-14	82	41		n/a
0	Smoothed Load	` · ·	I 00-06-14	00-06-14		35		n/a
1	J5(i1466)	Conc. Pt. (lbs)				30		n/a
2	J5 (i1502)	Conc. Pt. (lbs)	L 04-00-14	04-00-14	00	00		

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	517 ft-lbs	12,704 ft-lbs	4.1%	1	02-06-14
End Shear	331 lbs	5.785 lbs	5.7%	1	00-11-08
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	02-06-02
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	02-06-02
	0.006"	n/a	n/a	4	02-06-02
Max Defl. Span / Depth	6	n/a	n/a		00-00-00

				De man d/ Re sistance	De mand/ Resistance Member	Material	
Bear	ing Supports	Dim.(LxW)	Demand	Support		HUS1.81/10	
B0	Hanger	2" x 1-3/4"	410 lbs	n/a	9.6%		
B1	Beam	5-1/4" x 1-3/4"	425 lbs	8.7%	3.8%	Unspecified	

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Disclosure

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**Build 5033** Job Name:

Address:

# Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B17(i1950)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 14:35:31

BC CALC® Design Report

City, Province, Postal Code: BRADFORD,



File Name: SD25-3 SONOMA 3-EL B.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B17(i1950

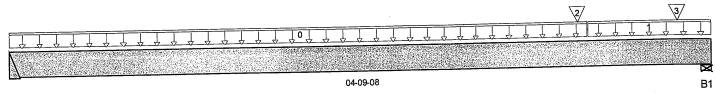
Specifier:

Designer: CZ

Company:

Customer: CCMC 12472-R Code reports:

Misc:



Total Horizontal Product Length = 04-09-08

Reaction Summary (Down I	Uplift) (lbs) Live	De ad	Snow	Wind	
B0 B1, 10"	47 / 0 297 / 0	44 /0 197 / 0			

	1.0					Live	Dead	Snow		Trib.
	ad Summary g Description	Load Type	Ref	. Start	End	1.00	0.65	1.00	1.15	7/0
<u>  a</u>	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-11-08	20	10			n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	03-11-08	04-09-08	9	4			n/a
2	B16(i1761)	Conc. Pt. (lbs)	L	03-10-10	03-10-10	183	103			n/a n/a
3	2(i521)	Conc. Pt. (lbs)	L	04-06-12	04-06-12	75	49			Illa

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	133 ft-lbs	25,408 ft-lbs	0.5%	1	02-03-10
* = = : :	84 lbs	11,571 lbs	0.7%	1	03-02-00
End Shear	L/999 (0.001")	•	n/a	4	02-01-09
Total Load Defl.	L/999 (0")	n/a	n/a	5	02-01-09
Live Load Defl.	0.001"	n/a	n/a	4	02-01-09
Max Defl. Span / Depth	4.9	n/a	n/a		00-00-00

Doorin	a Supports	Dim . (L x W)	De man d	De mand/ Re sistance Support	De mand/ Resistance Member	Material	
В0	<b>g Supports</b> Hanger Wall/Plate	2" x3-1/2" 10" x3-1/2"	126 lbs 692 lbs	n/a 3.7%	1.5% 1.6%	HGUS410 Unspecified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

**CONFORMS TO OBC 2012** 

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9





## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B17(i1950)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 14:35:31

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL B.mmdl

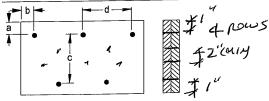
Description: Designs\Flush Beams\Basment\Flush Beams\B17(i19

Specifier:

Designer: CZ Company.

Misc:

## **Connection Diagram**



a minimum = 🛊 " b minimum = 3"

## Calculated Side Load = 84.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

ir regunatio sur Nails Connectors are: 5

3-1/2" ARDOX SPIRAL

## Disclosure

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DWG NO. TAM STRUCTURAL COMPONENT ONLY



## Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B18L(i1924)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 28, 2017 14:35:31

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-3 SONOMA 3-EL B.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B18L(i192

Specifier:

Designer: CZ Company:

Misc:

1/ 0 04-10-10 B<sub>1</sub> B0

	_				
Reaction Summary (Down	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 3-1/2"	534/0	279/0			
B1, 3-1/2"	444/0	234/0			

			Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
	Unf. Lin. (lb/ft)	1 00-00-00	04-00-00 218	109		n/a
<ul><li>0 Smoothed Load</li><li>1 J2(i1928)</li></ul>	Conc. Pt. (lbs)	L 04-04-02	04-04-02 104	52		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,188 ft-lbs	12,704 ft-lbs	9.4%	1	02-06-00
End Shear	801 lbs	5,785 lbs	13.8%	1	03-09-10
Total Load Defl.	L/999 (0.011")	n/a	n/a	4	02-05-04
Live Load Defl.	L/999 (0.008")	n/a	n/a	5	02-05-04
Max Defi.	0.011"	n/a	n/a	4	02-05-04
Span / Depth	5.6	n/a	n/a		00-00-00

<b>-</b> 1 <b>0</b>	Dim . (L x W)	Demand	De mand/ Resistance Support	De mand/ Resistance Member	Material
Bearing Supports B0 Post B1 Post	3-1/2" x 1-3/4" 3-1/2" x 1-3/4"	1,149 lbs	23.1% 19.3%	15.4% 12.8%	Unspecified Unspecified

## Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-05-04, Bottom: 00-05-04. Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

**CONFORMS TO OBC 2012** 

SITE C

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

## Disclosure

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DWG NO. TAM 9258-18 STRUCTURAL COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare		1	1/2" Gyp	sum Ceiling			
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15 <b>'-</b> 5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18' <b>-</b> 11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
	NI-80	21'-11"	20' <b>-</b> 3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spa	n Blocking	Mid-9	Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing			On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15' <b>-</b> 3"	14'-5"	N/A	
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A	
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A	
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A	
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A	
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A	
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A	
44 7/01	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A	
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A	
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A	
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A	
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A	
14"	NI-70	25' <b>-</b> 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A	
	NI-80	25'-7"	23'-8"	22' <del>-</del> 7"	N/A	26'-2"	24'-4"	23' <b>-</b> 2"	N/A	
	NI-90x	26'-4"	24' <del>-</del> 4"	23'-3"	N/A	26'-10"	24'-11"	23' <del>-</del> 9"	N/A	
	Ni-60	26 <b>'-</b> 5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A	
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26' <b>-</b> 5"	25' <b>-</b> 2"	N/A	
10	NI-80	28'-2"	26' <b>-</b> 1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26' <del>.</del> 2"	N/A	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

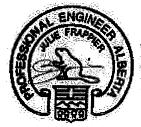
<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







		-		are		l	1/2" Gy	1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing			On Centre Spacing					
		12"	16"	19.2"	24"	12"	16"	<b>/</b> 19.2"	24"			
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"			
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"			
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"			
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"			
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"			
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"			
	NI-40x	19' <b>-</b> 4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"			
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"			
11-7/0	N1-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	<b>19'-9</b> "	18'-10"	17'-10"			
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"			
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"			
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"			
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"			
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19' <b>-</b> 9"			
	NI-80	23' <b>-</b> 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"			
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"			
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"			
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"			
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"			
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22 <b>'-</b> 5"			

			Mid-Spa	n Blocking	Mid-S	Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing		]	On Cent	re Spacing		
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-8"	17'-2"	16'-3"	<b>15'-2</b> "	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	<b>15'-5</b> "	
	NI-70	20'-0"	- 18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/6	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"	
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22' <del>-</del> 6"	21'-5"	20'-0"	
	NI-90x	24'-3"	22' <b>-</b> 6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"	
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"	
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23' <b>-</b> 9"	22'-4"	
	NI-80	26'-6"	24'-7"	23'-5"	22' <b>-</b> 2"	27'-1"	25' <b>-</b> 3"	24'-1"	22'-9"	
	NI-90x	27' <b>-</b> 3"	25'-4"	24'-1"	22' <b>-</b> 9"	27'-9"	25'-11"	24 <b>'-</b> 8"	23'-4"	
	NI-60	27' <b>-</b> 3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"	
16"	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"	
10	NI-80 .	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"	
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25 <b>'-</b> 8"	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare			1/2" Gypsum Ceiling				
Depth	Series		On Cen	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19' <b>-</b> 3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	<b>1</b> 9'-7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20' <b>-</b> 9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spa	n Blocking	Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing		On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15' <b>-</b> 7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	<b>17'-9</b> "	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-//0	N!-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21' <del>-</del> 7"	20' <b>-</b> 5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22' <b>-</b> 5"	21'-0"	N/A
14"	NI-70	25' <b>-</b> 3"	23'-4"	22' <del>-</del> 3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23' <b>-</b> 3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27' <b>-</b> 9"	25' <b>-</b> 8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28' <b>-</b> 2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are		I	1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"		
	N!-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"		
9-1/2"	NI-60	17'-2"	16'-2"	<b>15'-5"</b>	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"		
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"		
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"		
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"		
	NI-40x	19'-4"	17'-11"	17'-3"	<b>15'-10"</b>	19'-11"	18'-6"	17' <b>-</b> 9"	15'-10"		
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"		
11-7/6	NI-70	20'-9"	19'-2"	18'-3"	17' <b>-</b> 5"	21'-4"	19'-9"	18'-10"	17'-10"		
	NI-80	21'-1"	19' <b>-</b> 5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"		
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"		
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"		
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"		
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19' <b>-</b> 9"		
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22' <del>-</del> 3"	21' <del>-</del> 2"	20'-0"		
	Ni-90x	24'-1"	22 <b>'-</b> 3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"		
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22' <b>-</b> 9"	21'-8"	20'-6"		
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"		
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"		
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23 <b>'-</b> 8"	22 <b>'-</b> 5"		

			Mid-Spa	n Blocking		Mid-9	nd 1/2" Gypsum	Ceiling	
Depth	Series	On Centre Spacing				On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
11-7/8"	NI-40x	21'-3"	19'-3"	17' <b>-</b> 9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22' <del>-</del> 6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
14"	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19' <b>-</b> 6"	24'-9"	22' <b>-</b> 5"	21'-0"	19'-6"
	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22' <b>-</b> 9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23' <b>-</b> 3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27' <b>-</b> 9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28' <b>-</b> 8"	26'-8"	25 <b>'-</b> 3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
10	NI-80	29'-1"	27'-0"	25' <b>-</b> 9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



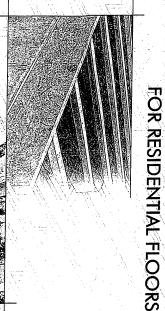
<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

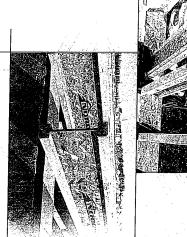
<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

## NSTALLATION GUIDE ENGINEERED WOOD





H 137

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# SAFETY AND CONSTRUCTION PRECAUTIONS



braced, or serious injuuntil fully fastened and Do not walk on I-joists ries can result.



over-stress I-joist with concentrated loads from Once sheathed, do not materials over unsheathed I-joists. building materials. Never stack building

N-C301 / November 2014

WARNING

Lipists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

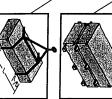
# Avoid Accidents by Following these Important Guidelines:

- 1. Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- 2. When the building is completed, the floor sheathing will provide lateral temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling. support for the top flanges of the I-joists. Until this sheathing is applied
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long bracing over at least two I-joists. the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining minimum of two 2-1/2" nails fastened to the top surface of each Lipist. Nail and spaced no more than 8 feet on centre, and must be secured with a
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

can result in serious accidents. Follow these installation guidelines carefully. Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic Ljoists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required

# STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- 2. Store, stack, and handle Ljoists vertically and level only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise. 3. Always stack and handle L-joists in the upright position only.
- Protect I-joists from weather, and use spacers to separate bundles.
- Bundled units should be kept intact until time of installation.
- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- Pick I-joists in bundles as shipped by the supplier
- ■Orient the bundles so that the webs of the I-joists are vertical
- Pick the bundles at the 5<sup>th</sup> points, using a spreader bar if necessary.
- 8. Do not handle I-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST





## MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span. live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L +
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used shall meet the requirements given in CGBS-71.26
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

## SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

				Joist Depth
				Joist Series
23-3 23-3 23-3 24-5 24-8	2015 2015 2017	20 4 4 5 4 2 4 4 5 4 2 4 4 5	7.3	12"
2218 2218 2211 2211 2236	18.7 20.0 20.3 20.3 20.3	17.00 18.00 18.30 18.37	15:2 15:2 15:4 16:3	Simple On centre 16"
19:9" 20:9" 21:1" 21:5"	19-10 19-1 19-4 19-2 19-9	15.5 16.5 17.4 17.6 17.6 17.10	14:8 14:10 14:10 15:4	e spans re spacing 19.2
20-10 21-21 21-3	18-2* 19-2* 19-5: 19-6: 20-0*	15:60 16:50 17:25 18:04 18:04	3:5     4:0   6:7	24°
24-7 26-0 26-5 26-1	22 2 22 7 23 10 24 3 24 9	2002 2003 2103 2103 2103 2103	101-81 14/81 19/3 19/3 19/3 19/3 19/3 19/3 19/3 19/	12°
22:9" 24:0" 24:5" 24:10"	20-8 20-11 22-11 22-5 22-10 23-1*	17.31 18.5 19.11 20.7 20.7	115/4* 116/5* 116/7/ 17/4*	Multip On cent
211.9 22-11. 23-9	20-0 20-0 21-1 21-5 21-10 22-0		074-107 15-100 16-50 16-51	le spans re spacing 19.2"
21,10 23,0 23,4 23,9	19-4 20-1 21-21 21-6 21-10 22-2	16.7 19.17 19.17 19.17	13:5: 16:11 16:10:	24"

CCMC EVALUATION REPORT 13032-R

## I-JOIST HANGERS

- Hangers shown illustrate the three to support I-joists. most commonly used metal hangers
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based maximum spans. and load capacity based on the on the joist depth, flange width
- 4. Web stiffeners are required when the brace the top flange of the I-joist. sides of the hangers do not laterally







Top Mount



Face Mount

## WEB STIFFENERS

## RECOMMENDATIONS:

- A bearing stiffener is required in all the stiffener and the flange is at the top. Construction Guide (C101).The gap between -joist properties table found of the I-joist reactions greater than shown in the engineered applications with factored
- A bearing stiffener is required when stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and the I-joist is supported in a hanger and the
- adjusted for other load durations as permitted standard term load duration, and may be than 2,370 lbs is applied to the top flange and the flange is at the bottom. by the code. The gap between the stiffener tip and the support. These values are for between supports, or in the case of a where a factored concentrated load greater A load stiffener is required at locations cantilever, anywhere between the cantilever

SI units conversion: 1 inch = 25.4 mm

## WEB STIFFENER INSTALLATION DETAILS FIGURE 2

Flange width 2-1/2" or 3-1/2"

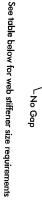
CONCENTRATED LOAD

(Load stiffener)



No Gap Fight Joint





(Bearing stiffener) END BEARING

ga Oga



Flange Width 3-1/2" 2-1/2"

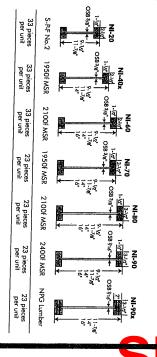
Web Stiffener Size Each Side of Web

1" x 2-5/16" minimum width

1-1/2" x 2-5/16" minimum width

Tight Joint No Gap • • + /

## NORDIC I-JOIST SERIES



finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from forest to the products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables Nortic

Nordic Engineered Wood I-joists use only finger-jointed back spruce longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior streamth zigno

2015-04-1

## INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, லூல்க்குன்
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அளிர்க்கதாயர
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 2045041504246
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Lipists where they will be permanently exposed to weather, or where they will remain in direct contact with
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or 1-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge **may never** be used as blocking or rim boards. Hoist blocking -joist-compatible depth selected panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an
- 13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used
- 14. If square-edge panels are used, edges must be supported between Lipists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or it a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

## One 2-1/2" Attach rim board to top

(<u>a</u>

NI blocking panel \_

F

at each side at bearing One 2-1/2" face nail board wire or spiral nail at top and may be driven at an angle to shall be 1-3/4" for the end splitting of bearing plate. To avoid splitting flange, start nails at least 1-1/2" bearings, and 3-1/2" for Minimum bearing length from end of I-joist. Nails

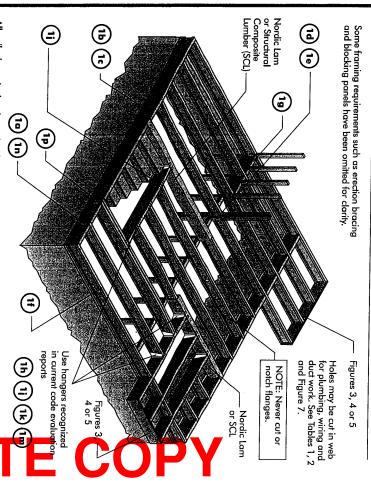
8.090	1-1/8" Rim Board Plus
Maximum Factored Unifor	Blocking Panel
Vertical Load* (ptf)	or Rim Joist

or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or

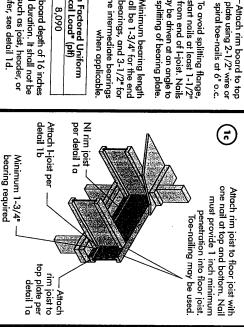
rafter. For concentrated vertical load transfer, see detail 1d.

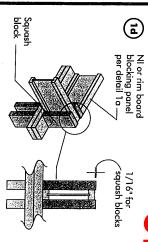
ALE: CAL

# TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framini (umber assumed to be Spruce-Prine-Fir No. 2 or better, Individual components not shown to scale for clarity.



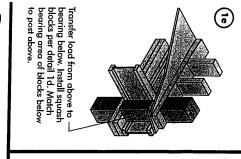


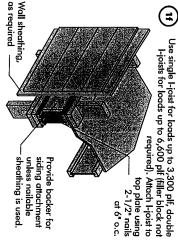
Pair of Squash Blocks	Maximum Factored Vertical per Pair of Squash Blocks (lbs)	red Vertical per h Blocks (lbs)
	3-1/2" wide	5-1/2" wide
2x Lumber	5,500	8,500
1-1/8" Rim Board Plus	4,300	6,600

Provide lateral bracing per detail 1a, 1b, or 1c

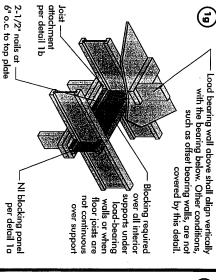
NI Joists	Blocking Panel or Rim Joist	Attach I-joist to
3,300	Maximum Factored Uniform Yertical Load* (plf)	2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)

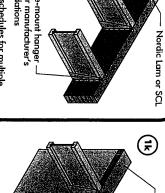






carried to the toundation. required when rim board is used. Bracing per code shall be Rim board may be used in lieu of I-joists. Backer is not





 $\equiv$ 

installed per manufacturer's Top- or face-mount hanger recommendations

recommendations. beams, see the manufacturer's For nailing schedules for multiple

Top-mount hanger installed per ...

Note: Unless hanger sides laterally stiffeners shall be used.

> support the top flange, bearing Note: Unless hanger sides laterally manutacturer's recommendations

stitteners shall be used

**(** 

Notes:

2. Leave a 1/8 to 1/4-inch gap between top 1. Support back of I-joist web during nailing to

Flange Size

**Block Size** 

2-1/2"× 1-1/2"

2-1/8" x 6" 2-1/8" x 8" 2-1/8" x 10" 2-1/8" x 12"

9-1/2 9 9-1/2" 11-7/8" Depth

> nails from each web of adjacent web.

Two 2-1/2" spiral extend block to face

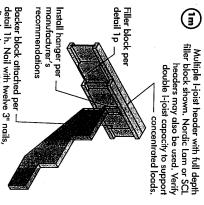
FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

**(**=

Lumber 2x4 min.

prevent damage to web/flange connection.

of filler block and bottom of top I-joist



clinch when possible Maximum support capacity = 1,620 lbs

 $\bigcirc$ 

per detail 1p

Filler block

Do not bevel-cut joist beyond inside

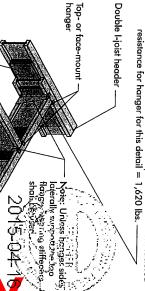
beam. 1/8" overhang inside face of wall or 2x plate flush with

allowed past inside ace of wall or beam.

I-joist per detail 1b Attachtace of wall

at bearing for lateral Note: Blocking required for clarity. support, not shown

> $\bigcirc$ Use twelve 3" nails, clinched when possible. Maximum factored Before installing a backer block to a double I-joist, drive three Backer block (use if hanger load exceeds 360 lbs) backer block will fit. Clinch. Install backer tight to top flange. additional 3" nails through the webs and filler block where the



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

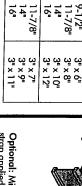
hangers)

(both sides for face-mount Backer block required

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

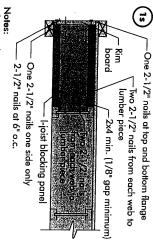
3-1/2"	2-1/2"	Flange Width
1-1/2"	l"	Material Thickness Required*
7-1/4"	5-1/2"	Minimum Depth**

- to CAN/CSA-O325 or CAN/CSA-O437 Standard better for solid sawn lumber and wood structural panels conforming Minimum grade for backer block material shall be S-P-F No. 2 or
- joists with 1-1/2" thick flanges. For 2" thick flanges use net depth For face-mount hangers use net joist depth minus 3-1/4" for

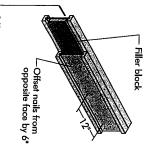


opposite side. to lumber piece, alternate on panel NI blocking

line or 1/2 inch minimum gypsum ceiling Optional: Minimum 1x4 inch attached to underside of joists



- the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements In some local codes, blocking is prescriptively required in for spacing of the blocking
- All nails are common spiral in this detai



Nail joists together with two rows of 3" Filler block is required between joists for

full length of span.

-1/8" to 1/4" gap between top flange and filler block

The maximum factored load that may be

3-1/2" × 2"

are required.

can be clinched, only two nails per foot possible) on each side of the double I-joist. nails at 12 inches o.c. (clinched when

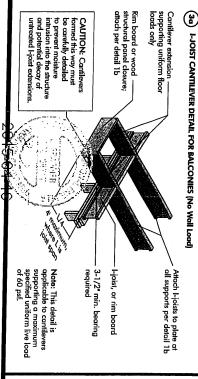
Total of four nails per foot required. If nails

applied to one side of the double joist

using this detail is 860 lbf/ft. Verify double

strap applied to underside of joist at blocking





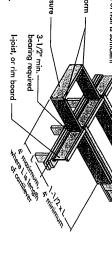


Full depth backer block with 1/8" gap between block and top flange of I-joist. See detail 1 h. Nail with 2 rows of 3" nails at 6" o.c. and clinch.

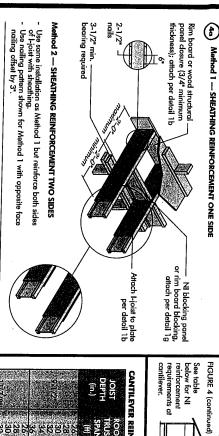
288 min. Nail to backer block and joist with 2 rows of -3" nails at 6" o.c. and clinch. (Carnflever nails may be used to attach backer block if length of nail is sufficient to allow clinching.) Attach I-joists to plate at all supports per detail 1b

floor loads only Cantilever extension supporting uniform

cantilevers supporting a maximum specified uniform live load of 60 psf Note: This detail is applicable to Lumber or wood structural panel closure 3-1/2" min. bearing required

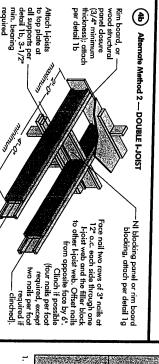


# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" rails at 6" o.c., top and bottom flange. Install with tace grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

€

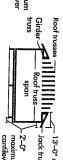


- N = No reinforcement required.
   1 = NI reinforced with 3/4" wood structural
- 2 = Ni reinforced with 3/4" wood structural
  profile on both sides, or double 1-joist,
  profile on both sides, or double 1-joist,
  X = Try a desper joist or closer spacing.
  A Moximum design load shall be: 15 per roof
  dead load, 55 per floor total load, and 80
  pif well load. Well load is based on 3-0° ximum width window or door openings

Block Hoists together with filler blocks for the full length of the reinforcement For Hoist flange widths greater than 3 inches place an additional row of centreline of the reinforcing panel from each side. Clinch when possible.

additional row of 3" nails along the

## reinforcement Roof truss span 2-0 cantilever



 Jack trusses cantilever maximum 13'-0" maximum

For hip roofs with the jack trusses running parallel to the cantilevered floor joists, requirements for a span of 26 ft. shall be permitted to be used. the I-joist reinforcement

## CANTILEVER REINFORCEMENT METHODS ALLOWED

6	¥.	100	2 5	JOIST DEPTH (in.)
25 27 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28 35 28	200 200 200 200 200 200 200 200 200 200	28 30 32 34 36 36	28 30 32 34 36	ROOF TRUSS SPAN (f)
ZZZZZZZ	ZZZZZZZ	ZZZZZZ	ZZZZZ	7. 12 20. = TI
ZZZZZZZ	ZZZZZZZ.	ZZZZZZ	z,z	30 psf, D NST SPAC 16
ZZZZZZZ	ZZZZZZZZ	z zz	ZNVN==	lL = 15 ps ING (in.) 19.2
zzzz	zz z z	2002-13-	×××××	sf R
********			-zzzz	ЭF LO, LL = 4 JOIS
ZZZZZZZZ ZZZZZZ			×××2	DING (UN 0 psf, DL: 17 SPACINI 16 19
32	222-1-1-	××× N N N N	×××××	FACTO 115 ps (in.) 2
ZZZZZZZZ	zzzzzzz	ZZZZZZZ		_ D
·zzzzzzz	i - ezzzzz	VZ	****	LL = 50 p JOIST S 16
zzz	ZZ		××××	ssf, DL = 1 PACING (
	××322211	×××××2	×××××	15 ps (in.)

- For larger openings, or multiple 3-0° width openings spaced less than 6-0° c.c., additional losis beneath the opening's cripple study may be required.

  3. Table applies to joist 17: to 24° c.c. that meet the floor span requirements for a design live load of 40 pet and lead load of 15 pet, and a live load deflection limit of L480. Use 12° o.c. requirements for lesser spacing.
  - For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam.
     When the roof is tramed using a ridge board, the Roof Truss Span is equivalent to the truss is used. distance between the supporting walls as if a
- or root beams vered joists supporting girder trusses beams may require additional

## ITE CO

# **RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS**

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- 'n I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. between the top or bottom of the hole or opening and the adjacent I-joist flange The maximum size hole or the maximum depth of a duct chase opening that can the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained be cut into an I-joist web shall equal the clear distance between the flanges of
- Ċ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 6 Where more than one hole is necessary, the distance between adjacent hole opening shall be sized and located in compliance with the requirements of size of the largest square hole (or twice the length of the longest side of the Tables 1 and 2, respectively. longest rectangular hole or duct chase opening) and each hole and duct chase edges shall exceed twice the diameter of the largest round hole or twice the
- .7 A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a verification. cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 5 All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them

## Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

<ol> <li>Above table</li> </ol>					Joist Depth
may be used	-1-1-1-1-	-1-1-1-1-)	jejelejelejej		Joist Series
for I-joist					
spacing o					<b>5</b>
f 24 inches				196	um dist
s on centre	500 A K	04004N 8505	123 24 0 2 2 2 3	EB\65 NGC 98	ance fro
"	113,750	Mar. 12.53	15252045	<b>大学企业</b>	m inside Rounc 5-1/4
	10.00				face of hole di
		4.00	0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		any sup ameter ( 8-5/8
			111111	38 S. S.	port to c in.)
				<b>计算器的</b> 电电路	entre of ho
	- 000 - 000			100	hole (ff-
	308.2		1111111	A Charles I was a	in.]
200			PREFER I		2-3/4 od
Z = 0			277.00 11	7 ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) (	Span justment Factor

- Aware unue may be used tor Hoist spacing of 24 inches on centre or less.
  Hole location distance is measured from inside face of supports to centre of hole.
  Distances in this chart are based on uniformly loaded joists.

## OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum Frain Spains), the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced = Lactual x D SAF

Where: Dreduced = Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applicase is the distance shall not be less than 6 inches from the face of the support to edge of the hole.

Lactual

SÆ, The actual measured span distance between the inside faces of supports (ft) Span Adjustment Factor given in this table.

U

The minimum distance from the inside face of any support to centre of hole from this table If <u>Lactual</u> is greater than 1, use 1 in the above calculation for <u>Lactual</u>.

SAF

0.504

spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of ñeld-cut holes. notch the flange, or Never drill, cut or

bearing distance from for minimum

of larger hole

whichever is diameter, length or hole 2x duct chase

from bearing) minimum distance (see Table 2 for 2x diameter

Duct chase opening

are 1-1

electrical or small plumbing lines. They for the contractor's convenience to instal Knockouts are prescored holes provided

/2 inches in diameter, and are

See Table 1

FIELD-CUT HOLE LOCATOR

FIGURE 7

should be cut with a Holes in webs over-cut the web

sharp saw.

and then making the cuts between the holes is another good method to the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners the corners, as this can cause unnecessary stress concentrations. Slightly rounding or rectangular holes, avoid over-cutting

and may be ignored for purposes of calculating minimum distances A knockout is **NOT** considered a hole, may be utilized wherever it occurs

Knockouts

See rule 12

between top and bottom flange — all duct chase openings and holes Maintain minimum 1/8" space 

## DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only Minimum distance from inside face of any support to centre of opening (fi-in.)

ᄝ							
Depth		i D					
Se	F.2						
Series							] i i i
		515161	SP(c)		ejejes		
0		1000 1200 1200 1200 1200 1200 1200 1200	-0.00	10	399		oL.
10	24	55550 25550	1440	ķ.	0 0 00	00	E656
•	95	B01-0			3		Nobel S
		606 612	10 40		909	55	-
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cha							
50 le	9 6	6 7	7060 7060	200	000	Fe:	NNEK Nac
ngth							
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	6 .	78 7	0 0 0 0				
	is č	7-8 7-8	يارەرە»،	SE <sup>®</sup>	KI KI	- 00	L-6N
	1	88-3 8-3 1-3	5559	555	=ಜನ	355	2002
	•	3	61.9	6 V.X	730	94	<b>S</b> &&2
		2000 2400 2400	10%	366	222	322 322	
			0.0	# 하두 6	ယူထုံ့ရဲ့	7=6	195

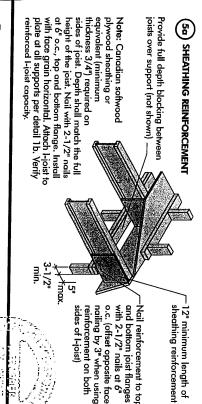
- . Above table may be used for I-joist spacing of 24 inches on centre or less.

  Duct chase opening location distance is measured from inside face of supports to centre of opening.

  The above table is based on simple-span joists only. For other applications, contact your local distributor.

  Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of I/480. For other applications, contact your local distributor.
- filminize damage to the I-joist

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



o.c. (offset opposite face nailing by 3" when using Nail reinforcement to top

ROOF LOADING (UNFACTORED

LL = 40 psf, DL = 15 psf

LL = 50 psf, DL = 15 psf JOIST SPACING (in.)

JOIST SPACING (in.)

19.2

12

19.2

24

7

Jack trusses 13'-0" maximum

For hip roofs with the jack trusses running parallel to the cantilevered floor joists,

Roof truss

span

- maximum cantilever 21-0

requirements for a span of 26 ft. shall be permitted to the I-joist reinforcement

5" maximum

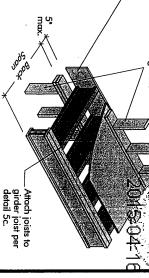
SET-BACK DETAIL

Bearing walls

(<del>5</del>b)

structural panel closure (3/4" minimum thickness), attach per detail 1b. Kim board or wood

- (not shown for clarity) Provide full depth blocking petween joists over support
- Attach I-joist to plate at all
- supports per detail 1b. 3-1/2" minimum 1-joist bearing required



## (F SET-BACK CONNECTION

through joist web and web of girder using 2-1/2" nails. Alternate for opposite side. 2x6 S-P-F No. 2 or better) nailed /ertical solid sawn blocks



Hanger may be used in lieu of solid sawn blocks

bottom flanges. nails, toe-nail at top and Nail joist end using 3"

- N = No reinforcement required.
  1 = NI reinforced with 3/4" wood structural
- panel on one side only.

  2 = NI reinforced with 3/4" wood structural
- X = Try a deeper joist or closer spacing.

  Maximum design load shall be: 15 psf roof dead load, panel on both sides, or double 1-joist.

wall load. Wall load is based on 3'-0" 55 psf floor total load, and 80 plf

FIGURE 5 (continued) reinforcement requirements at below for NI See table BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED DEPTH (in.) Roof truss span ΖZ 12 LL = 30 psf, DL = 15 psfJOIST SPACING (in.) 16 2'-0" ∟ maximum -5" maximum cantilever 19.2 truss Girder L Roof trusses 24

studs may be required.	additional joists	openings spaced	For larger openi
ly be required.	additional joists beneath the opening's cript	openings spaced less than 6'-0" o.c.,	For larger openings, or multiple 3'-0" width

Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of 1/480. Use

- 흫
- above is equivalent to the distance between For conventional roof construction using a distance between the supporting walls as if a the Roof Truss Span is equivalent to the When the roof is framed using a ridge board, the supporting wall and the ridge beam ridge beam, the Roof Truss Span column
- Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

# INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manutacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- . Tap the second row of panels into place, using a block to protect groove edges.
- nail to assure accurate and consistent spacing.) Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The

# FASTENERS FOR SHEATHING AND SUBFLOORING(1)

	5	20	10	(in.)	Spacing	Maximum
4.4		-5/8	0/6	(in.)	ranei Thickness	Minimum
27	2	. 2	2	Spiral Nails	Wire or	
1-3/4"		1-3/4"	1-3/4"	or Screws	Ring Thread	ail Size and Typ
2		2	2	Simples	Stanlar	ē
6"		6.	6.	Edges	of Fas	Moximum
12"		12"	12"	Interm. Supports	leners	Spacing

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing

5

- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

IMPORTANT NOTE:

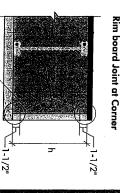
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with

# RIM BOARD INSTALLATION DETAILS

# (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

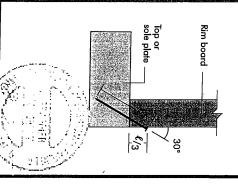






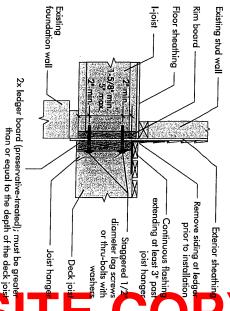
## (F TOE-NAIL CONNECTION AT RIM BOARD

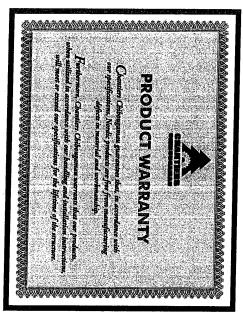
6" o.c. (typical) 2-1/2" toe-nails at

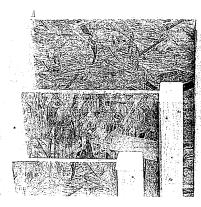


## 8 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint







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	LIVIHEA	LVL HEADER AND CONVENTIONAL			
	LUMBER NAILING DETAILS				
	DETAIL NUMBER				
	. A	2.	12		
	В	2	8		
	С	2	6		
	D	2	4		
- Same	1A	3	12		
	1B	3	8		
	1C	3	. 6		
	1D	3:	4		
	2A	4	. 12		
1	2B	4	8		
L	. 2C	4	6		
Ŀ	2D	4	4		
1	3A	5	12		
1	3B	5	8		
L	3C	5	6		
Ŀ	3D	5	4		
L	4A	6	12		
L	4B	6	8		
Ŀ	4C	6	6		
L	4D	6	4		

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## NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
  - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
  - (3) MINIMUM NAIL ROW SPACING "c" = 2"
  - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
  - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



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GOMPONENT ONLY
TO BE USED ONLY
WITH BEAM CALCS
BEARING THE
STAMP BELOWS

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