

8-11-00

11-01-00

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	39
J1DJ	14-00-00	9 1/2" NI-40x	2	4
J2	12-00-00	9 1/2" NI-40x	1	14
J3	8-00-00	9 1/2" NI-40x	1	4
J4	6-00-00	9 1/2" NI-40x	1	1
J5	4-00-00	9 1/2" NI-40x	1	6
J6	2-00-00	9 1/2" NI-40x	1	4
B2	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B1	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B6L	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
Con	nector Sumr	non		

 Connector Summary

 Qty
 Manuf
 Product

 9
 H1
 IUS2.56/9.5

 4
 H1
 IUS2.56/9.5

 15
 H1
 IUS2.56/9.5

 4
 H1
 IUS2.56/9.5

 2
 H3
 HUS1.81/10

1-00-00

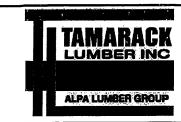
9-10-00

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES

DATE: 2018-10-16

INSPECTOR: BG

SITE COPY



FROM PLAN DATED: DEC 2016

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: SD25-2 SONOMA 2

ELEVATION: A,B,C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

LOADING:

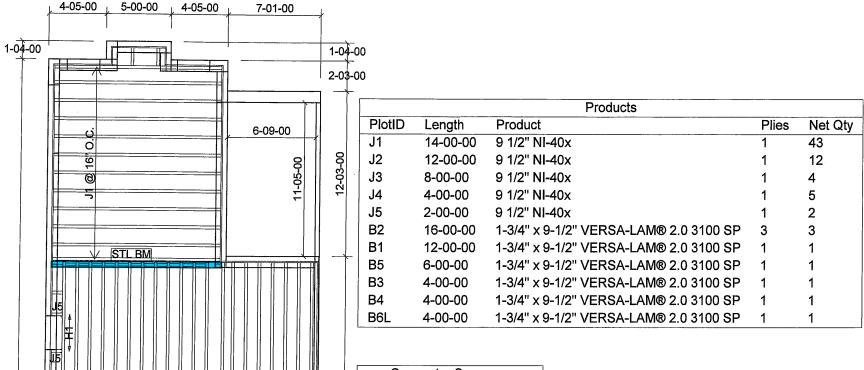
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

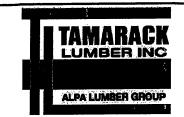
DATE: 14/02/2018

1st FLOOR

STANDARD



	Connector	Summary
Qty	Manuf	Product
9	H1	IUS2.56/9.5
15	H1	IUS2.56/9.5
2	H1	IUS2.56/9.5
2	H3	HUS1.81/10



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: SD25-2 SONOMA 2

ELEVATION: A,B,C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6. LOADING:

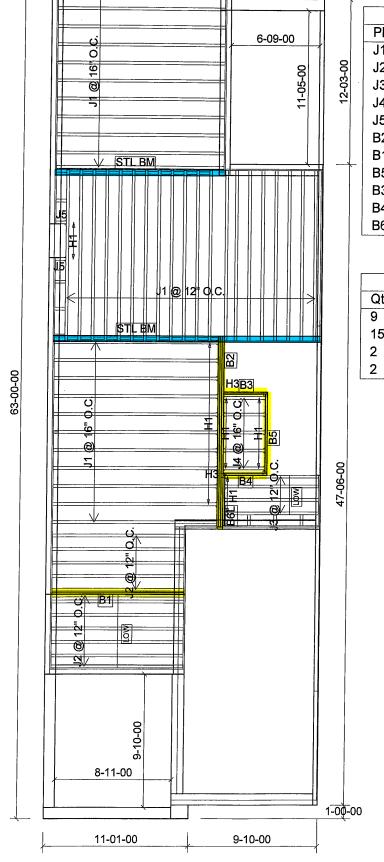
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

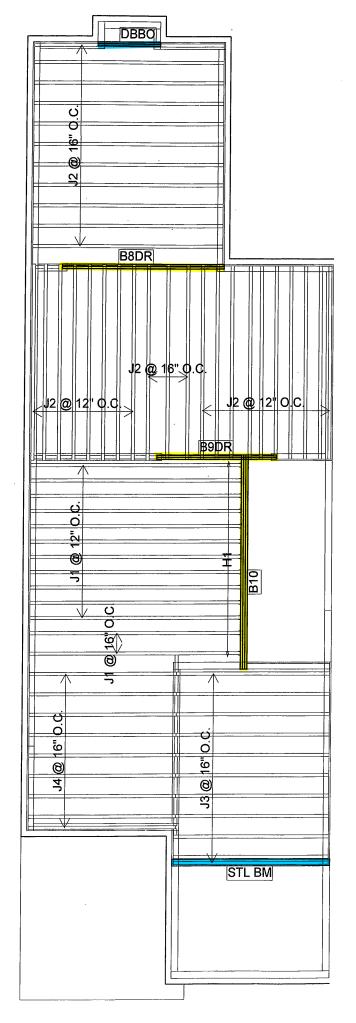
SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 14/02/2018

1st FLOOR

W.O.D. & W.O.B. CON.





		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J2	14-00-00	9 1/2" NI-40x	1	32
J3	12-00-00	9 1/2" NI-40x	1	11
J4	10-00-00	9 1/2" NI-40x	1	9
B10	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B8DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	Connector	Summary			
Qty Manuf Product					
13	H1	IUS2.56/9.5			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY E AST

MODEL: SD25-2 SONOMA 2

ELEVATION: A

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

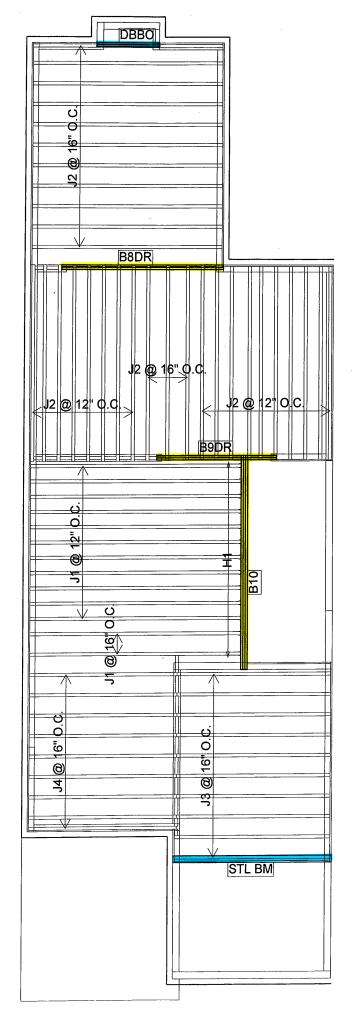
REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS. SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 25/08/2017

2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J2	14-00-00	9 1/2" NI-40x	1	32
J3	12-00-00	9 1/2" NI-40x	1	10
J4	10-00-00	9 1/2" NI-40x	1	9
B10	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B8DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	connector	Summary
Qty	Manuf	Product
13	H1	IUS2.56/9.5



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: SD25-2 SONOMA 2

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

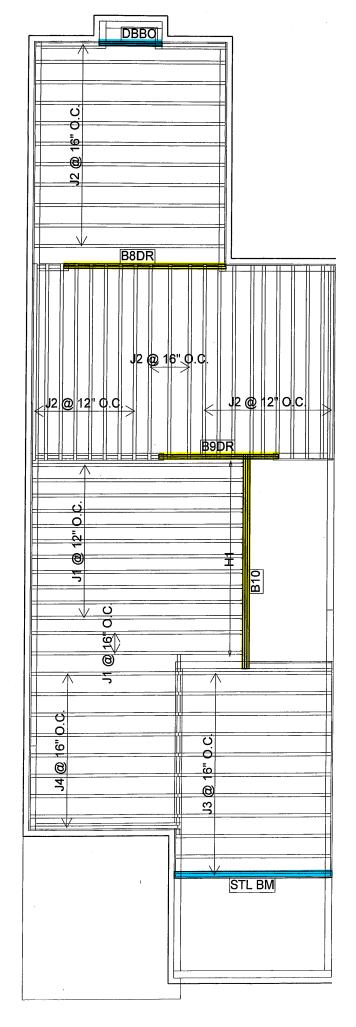
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 25/08/2017

2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J2	14-00-00	9 1/2" NI-40x	1	32
J3	12-00-00	9 1/2" NI-40x	1	11
J4	10-00-00	9 1/2" NI-40x	1	9
B10	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B8DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	connector	Summary			
Qty	Qty Manuf Product				
13	H1	IUS2.56/9.5			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: SD25-2 SONOMA 2

ELEVATION: C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

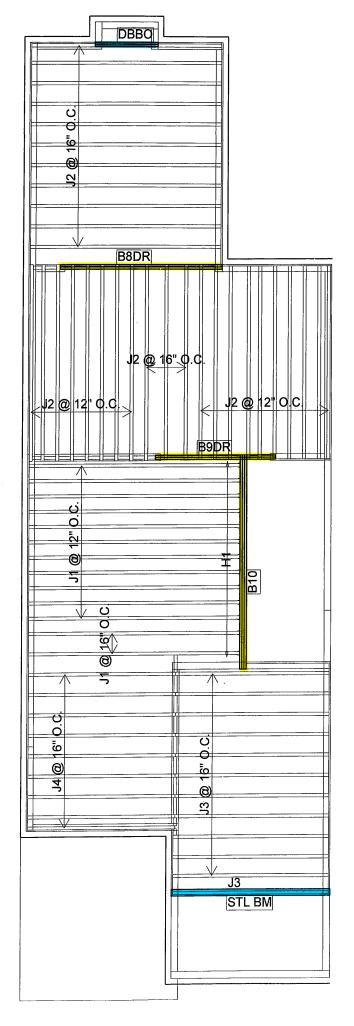
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft

TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 25/08/2017

2nd FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	13
J2	14-00-00	9 1/2" NI-40x	1	32
J3	12-00-00	9 1/2" NI-40x	1	12
J4	10-00-00	9 1/2" NI-40x	1	9
B10	16-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B8DR	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

C	Connector	Summary
Qty	Manuf	Product
13	H1	IUS2.56/9.5



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: SD25-2 SONOMA 2

ELEVATION: C MOD

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 25/08/2017

2nd FLOOR



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1523)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:04

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(j1523)

Specifier:

Designer: CZ

Company: Misc:

	2 17 57 17 17 17	
	, 0	
⊠ B0	10-01-04	B1

Total Horizontal Product Length = 10-01-04

Reaction Summary (Down / Uplift) (lbs)			
Be aring	Live	De ad	Snow	Wind
B0, 4-3/8"	34 / 0	288/0		
B1, 4-3/8"	34 / 0	129/0		

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-01-04	7	3			n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	05-07-00		60			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	735 ft-lbs	8,258 ft-lbs	8.9%	0	04-02-06
End Shear	293 lbs	3,761 lbs	7.8%	0	01-01-14
Total Load Defl.	L/999 (0.037")	n/a	n/a	4	04-09-14
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	05-00-06
Max Defl.	0.037"	n/a	n/a	4	04-09-14
Span / Depth	12	n/a	n/a		00-00-00

Beari	ing Supports	Dim.(L x W)	De man d	De mand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	403 lbs	15.2%	6.6%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	181 lbs	6.8%	3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

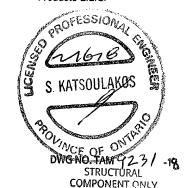
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.







Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1596)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:
City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

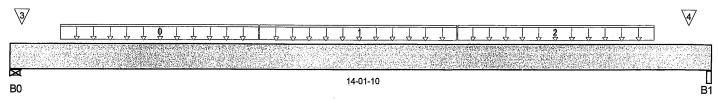
File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1596)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 14-01-10

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 5-15/16"	4,645 / 0	3,128 / 0							
B1, 5-1/4"	2,123/0	1,218/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15		
0	Smoothed Load	Unf. Lin. (lb/ft)	L	01-00-00	05-00-00	259	130			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	05-00-00	09-00-00	326	163			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	09-00-00	13-00-00	259	130			n/a
3	2(i366)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	2,835	2,113			n/a
4	J1 (i1555)	Conc. Pt. (lbs)	L	13-08-00	13-08-00	346	222			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	15,046 ft-lbs	39,636 ft-lbs	38%	1	07-00-00
End Shear	3,960 lbs	17,356 lbs	22.8%	1	01-03-07
Total Load Defl.	L/357 (0.447")	0.666"	67.1%	4	07-00-00
Live Load Defl.	L/555 (0.288")	0.444"	64.9%	5	07-00-00
Max Defl.	0.447"	n/a	n/a	4	07-00-00
Span / Depth	16.8	n/a	n/a		00-00-00

Bearing Supports Dir	Dim.(LxW) Demand		Member	Material	
	5/16" x 5-1/4"10,8 /4" x 5-1/4" 4.7	77 lbs 65.1% 07 lbs 32%	28.5% 14%	Unspecified Unspecified	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 9232. PO STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1596)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports: CCMC 12472-R

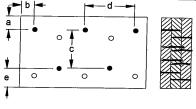
File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i159

Specifier: Designer:

Company: Misc:

Connection Diagram



وسوم 4

a minimum = **2**" b minimum = 3"

c = 61/2" d = 29 4 e minimum = 3"

Calculated Side Load = 521.6 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

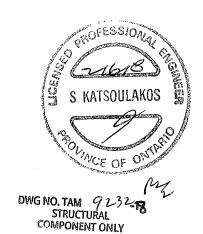
Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood. Products L.L.C.





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1470)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1470)

Specifier:

Designer: CZ

Company:

Misc:

	ring Kapan yang di Salah S
B0 03-04-00] B1

Total Horizontal Product Length = 03-04-00

Be aring	Live	De ad	Snow	Win d	
B0	14 / 0	15 / 0			
B1, 3-1/2"	15 / 0	16 / 0			

- 1	Load Summary					Live	Dead	Snow	Wind	Trib.
	Tag Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
(FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-04-00	9	4			n/a

CONFORMS TO OBC 2012

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	28 ft-lbs	12,704 ft-lbs	0.2%	1	01-07-04
End Shear	16 lbs	5,785 lbs	0.3%	1	00-11-08
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-07-04
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-07-04
Max Defl.	0"	n/a	n/a	4	01-07-04
Span / Depth	3.8	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(LxW) Demand		De mand/ Resistance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	39 lbs	n/a	0.9%	HUS1.81/10
B1	Post	3-1/2" x 1-3/4"	43 lbs	0.9%	0.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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BOISE GLULAM™, SIMPLE FRAMING
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PLUS®, VERSA-RIM®,
VERSA-STRAND®, VERSA-STUD® are
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Products L.L.C.





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1475)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:04

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

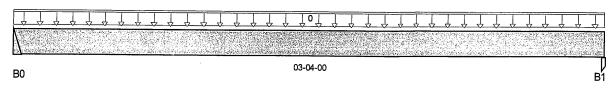
File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1475)

Specifier:

Designer: CZ Company:

Misc:



Total Horizontal Product Length = 03-04-00

			J .				
Reaction Summary (Do	own / Uplift) (lbs) Live	De ad	Snow	Wind			
B0 B1, 3-1/2"	16 / 0 17 / 0	16 / 0 17 / 0					
Load Summary				Live	Dead	Snow Wind	Trib.

Lo	Load Summary					Live	Dead	Snow	/ Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC1 Floor Material	Unf. Lin. (lb/ft)	· L	00-00-00	03-04-00	10	5			n/a

Controls Summary	Factored Factored Demand Resistance		Demand / Resistance	Load Case	Location
Pos. Moment	31 ft-lbs	12,704 ft-lbs	0.2%	1	01-07-04
End Shear	18 lbs	5,785 lbs	0.3%	1	00-11-08
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-07-04
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-07-04
Max Defl.	0"	n/a	n/a	4	01-07-04
Span / Depth	3.8	n/a	n/a		00-00-00

Bearin	ng Supports	Dim.(L x W)	Demand	De man d/ Re s istance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	44 lbs	n/a	1%	HUS1.81/10
B1	Post	3-1/2" x 1-3/4"	47 lbs	0.9%	0.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9234-18
STRUCTURAL
COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1512)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1512)

Specifier:

Designer: CZ

Company:

Misc:

<u></u>	2
05-10-08 B0	D B1

Total Horizontal Product Length = 05-10-08

Reaction Summary (Down / Uplift) (lbs)										
Bearing	Live	De ad	Snow	Wind						
B0, 1-3/4"	198/0	112/0								
B1, 1-3/4"	200/0	114/0								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	01-00-04	05-00-04	69	34			n/a
1	J5(i1563)	Conc. Pt. (lbs)	L	00-04-04	00-04-04	64	32			n/a
2	J5 (i1516)	Conc. Pt. (lbs)	L	05-07-00	05-07-00	58	29			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	630 ft-lbs	12,704 ft-lbs	5%	. 1	03-00-04
End Shear	335 lbs	5,785 lbs	5.8%`	1	04-11-04
Total Load Defl.	L/999 (0.01")	n/a	n/a	4	02-11-04
Live Load Defl.	L/999 (0.006")	n/a	n/a	5	02-11-04
Max Defl.	0.01"	n/a	n/a	4	02-11-04
Span / Depth	7.2	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim.(LxW)	De man d	Support	Member	Material
B0	Post	1-3/4" x 1-3/4"	437 lbs	17.6%	11.7%	Unspecified
B1	Post	1-3/4" x 1-3/4"	442 lbs	17.8%	11.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6L(i1139)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:04

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6L(i1139

Specifier:

Designer: CZ

Company:

Misc:

	\bigcirc	1	\sqrt	3
⊠ B0		03-10-04		D B1

Total Horizontal Product Length = 03-10-04

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 4-3/8"	224/0	121/0							
B1, 3-1/2"	263/0	140/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description		Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
0	J4(i992)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	146	73			n/a
1	J4(i1060)	Conc. Pt. (lbs)	L	01-10-14	01-10-14	146	73			n/a
2	J4(i935)	Conc. Pt. (lbs)	L	02-10-14	02-10-14	134	67			n/a
3	J4(i979)	Conc. Pt. (lbs)	L	03-07-10	03-07-10	61	30			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	462 ft-1bs	12,704 ft-lbs	3.6%	1	01-10-14
End Shear	386 lbs	5,785 lbs	6.7%	1	02-09-04
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	01-11-08
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	01-11-08
MaxDefl.	0.002"	n/a	n/a	4	01-11-08
Span / Depth	4.2	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	De man d/ Re sistance Su pport	De mand/ Resistance Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	488 lbs	11.9%	5.2%	Unspecified
B1	Post	3-1/2" x 1-3/4"	570 lbs	11.5%	7.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-01-06, Bottom: 00-01-06.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

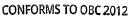
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1234)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

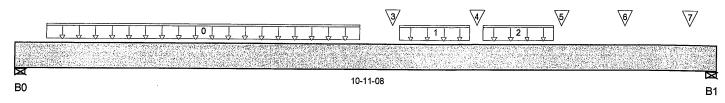
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8D

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 10-11-08

Reaction Summary (Down / Uplift) (lbs)										
Be aring	Live	De ad	Snow	Wind						
B0, 4"	1,401 / 0	749/0								
B1, 5-1/2"	1,466 / 0	783/0								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description *	Load Type		Ref. Start End		1.00	0.65	1.00 1.15		
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-05-12	05-04-08	260	130			n/a
1	Bk1(i1274)	Unf. Lin. (lb/ft)	L	05-11-12	07-01-04	27	14			n/a
2	Bk1(i1174)	Unf. Lin. (lb/ft)	· L	07-03-12	08-05-04	27	14			n/a
3	J2(i1237)	Conc. Pt. (lbs)	L	05-10-08	05-10-08	291	145			n/a
4	J2(i1241)	Conc. Pt. (lbs)	L	07-02-08	07-02-08	332	166			n/a
5	J2(i1287)	Conc. Pt. (lbs)	L	08-06-08	08-06-08	291	145			n/a
6	J2(i1247)	Conc. Pt. (lbs)	L	09-06-08	09-06-08	249	125			n/a
7	J2(i1203)	Conc. Pt. (lbs)	L	10-06-08	10-06-08	171	86			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	7,893 ft-lbs	25,408 ft-lbs	31.1%	1	05-09-04
End Shear	2,835 lbs	11,571 lbs	24.5%	1	01-01-08
Total Load Defl.	L/583 (0.212")	0.515"	41.2%	4	05-09-04
Live Load Defl.	L/894 (0.138")	0.343"	40.3%	5	05-09-04
Max Defl.	0.212"	n/a	n/a	4	05-09-04
Span / Depth	13	n/a	n/a		00-00-00

Do ovin w Commanda		Direc (1 140)	Dominio		Demand/ Resistance	
Bear	ring Supports	Dim. (L x W)	De man d	Support	Member	Material
B0	Wall/Plate	4" x 3-1/2"	3,038 lbs	26.7%	17.8%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	3,178 lbs	20.3%	13.5%	Unspecified

Notes



DWG NO. TAM 9237-18 STRUCTURAL COMPONENT ONLY



BC CALC® Design Report

Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1234)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD.

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B{

Specifier:

Designer: CZ

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-02-10, Bottom: 00-02-10,

Resistance Factor phi has been applied to all presented results per CSA O86.

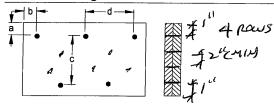
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Connection Diagram



a minimum = #" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Area Nails

3-1/2" ARDOX SPIRAL

Disclosure

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DWG NO. TAM 9237. STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9DR(i1176)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

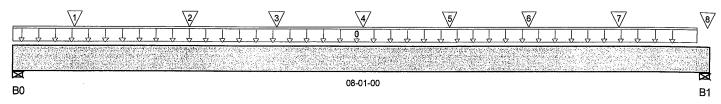
File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B9D

Specifier:

Designer: CZ Company.

Misc:



Total Horizontal Product Length = 08-01-00

Reaction Summary (Down / Uplift) (Ibs)											
Be aring	Live	De ad	Snow	Wind							
B0, 4"	1,543 / 0	1,077 / 0		•••							
B1, 4"	2,711/0	1,702/0									

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	07-11-04		60			n/a
1	J2(i1241)	Conc. Pt. (lbs)	L	00-08-08	00-08-08	332	166			n/a
2	J2(i1287)	Conc. Pt. (lbs)	L	02-00-08	02-00-08	291	145			n/a
3	J2(i1247)	Conc. Pt. (lbs)	L	03-00-08	03-00-08	24 9	125			n/a
4	J2(i1203)	Conc. Pt. (lbs)	L	04-00-08	04-00-08	240	120			n/a
5	J2(i1252)	Conc. Pt. (lbs)	L	05-00-08	05-00-08	294	147			n/a
6	-	Conc. Pt. (lbs)	L	05-11-12	05-11-12	2,306	1,251			n/a
7	J2(i1186)	Conc. Pt. (lbs)	L	07-00-08	07-00-08	217	109			n/a
8	J2(i1188)	Conc. Pt. (lbs)	L	08-00-08	08-00-08	250	125			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	9,677 ft-lbs	25,408 ft-lbs	38.1%	1	05-11-06
End Shear	5,524 lbs	11,571 lbs	47.7%	1	06-11-08
Total Load Defl.	L/672 (0.135")	0.377"	35.7%	4	04-04-02
Live Load Defl.	L/999 (0.082")	n/a	n/a	5	04-04-02
Max Defl.	0.135"	n/a	n/a	4	04-04-02
Span / Depth	9.5	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ing Supports	Dim.(L x W)	Demand	Support	Member	Material
B0	Wall/Plate	4" x 3-1/2"	3,662 lbs	32.2%	21.4%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	6,193 lbs	54.5%	36.3%	Unspecified

Notes



DWG NO. TAM 9238 STRUCTURAL COMPONENT ONLY

Page 1 of 2



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9DR(i1176)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address: City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\BS

Disclosure

Specifier:

Designer:

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-02-01, Bottom: 00-02-01.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. **CONFORMS TO OBC 2012**

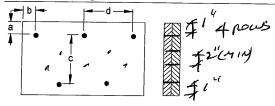
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Connection Diagram



a minimum = 2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Stake: Nails

3-1/2" ARDOX SPIRAL

DWG NO. TAM 92 STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B10(i1175)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

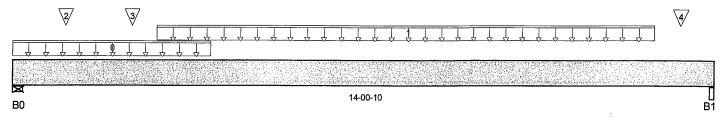
Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1175)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 14-00-10

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 5-1/2"	2,838/0	1,519/0							
B1, 3-1/2"	2,057 / 0	1,127/0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description		Load Type Re		ef. Start End		1.00	0.65	1.00	1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	03-11-08	240	120		_	n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	02-10-08	12-10-08	297	148			n/a
2	J1(i1282)	Conc. Pt. (lbs)	L	01-00-08	01-00-08	390	195			n/a
3	J1 (i1189)	Conc. Pt. (lbs)	L	02-04-08	02-04-08	346	173			n/a
4	J1 (i1289)	Conc. Pt. (lbs)	L	13-04-08	13-04-08	223	112			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	16,317 ft-lbs	39,636 ft-lbs	41.2%	1	06-04-08
End Shear	5,263 lbs	17,356 lbs	30.3%	1	01-03-00
Total Load Defl.	L/319 (0.505")	0.671"	75.2%	4	07-01-08
Live Load Defl.	L/492 (0.328")	0.448"	73.2%	5	07-01-08
Max Defl.	0.505"	n/a	n/a	4	07-01-08
Span / Depth	17	n/a	n/a		00-00-00

Beari	ing Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
В0	Wall/Plate	5-1/2" x 5-1/4"	6,156 lbs	39.9%	17.5%	Unspecified
B1	Beam	3-1/2" x 5-1/4"	4,494 lbs	22.4%	20%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. **CONFORMS TO OBC 2012**

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 9 23 STRUCTURAL COMPONENT ONLY



BC CALC® Design Report

Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B10(i1175)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 25, 2017 11:53:05

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-2 SONOMA 2.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i117

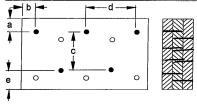
Specifier:

Misc:

Designer: CZ

Company:

Connection Diagram



4 rows

a minimum = 2" b minimum = 3"

e minimum = 2"

Calculated Side Load = 593.8 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d Common Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BCRIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. TAM 923 STRUCTURAL COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







		*****	E	Bare		_ i	1/2" Gyp	sum Ceiling	
Depth	Series		On Cen	tre Spacing			On Cent	re Spacing	
	·	12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16' - 5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19' - 2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-7/0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25'-3"	23'-4"	22 '- 3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25' - 7"	23' - 8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27' - 9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28' - 2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25' - 6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	2 7'- 5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			E	are			1/2" Gyr	osum Ceiling	
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18' - 9"	17'-11"	17'-2"
11-7/0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19' - 9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23' - 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	N1-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-//8	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21' - 5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24'-3"	23' - 2"	21'-10"	26'-8"	24'-11"	23' - 9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24 '- 8"	23'-4"
	NI-60	27'-3"	25 '- 5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28 '-8 "	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24' - 8"
10	NI-80	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15' - 1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-//0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3 "	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20' - 3"	19' - 4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23' - 6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	Ni-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	Ni-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-//6	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22' - 9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22' - 2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23' - 8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25 '- 8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25 '- 6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27 '- 5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are		l	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11 7/0"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18' - 9"	17'-11"	17'-1"
11-7/8"	NI-70	20'-9"	19'-2"	18' - 3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20' - 0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19 '- 9"
	NI-80	23' - 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22' - 3"	21'-2"	20'-0"	24'-8"	22'-10"	21' -9 "	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25 '-1 "	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22 '- 9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15 '- 5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
44 7/01	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	Ni-90x	24'-3"	22' - 6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22' - 5"	21'-0"	19'-6"	24'-9"	22' - 5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
4.611	NI-70	28' - 8"	26'-8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
16"	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28' - 5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

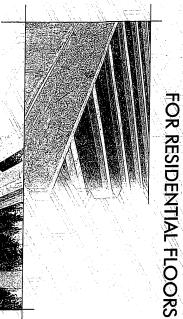
^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

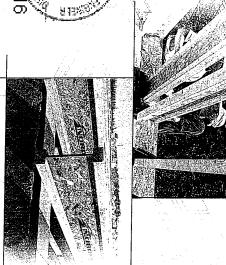
^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



NSTALLATION GUIDE





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SAFETY AND CONSTRUCTION PRECAUTIONS WARNING



Avoid Accidents by Following these Important Guidelines:

Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location,

blocking will be required at the interior support.

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

braced, or serious injuuntil fully fastened and Do not walk on 1-joists ries can result.



over-stress I-joist with concentrated loads from Once sheathed, do not materials over unsheathed I-joists.



Never stack building building materials.

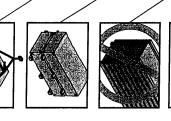
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the Lipists. Until this sheathing is applied.

- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long to prevent I-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied bracing over at least two I-joists. minimum of two 2-1/2" nails fastened to the top surface of each Ljoist. Nail the bracing to a lateral restraint at the end of each boy. Lap ends of adjoining and spaced no more than 8 feet on centre, and must be secured with a
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- closure panels, rim board, or cross-bridging.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

can result in serious accidents. Follow these installation guidelines carefully. Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required

STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- ω Store, stack, and handle I-joists vertically and level only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise Always stack and handle I-joists in the upright position only.
- Ċ Protect I-joists from weather, and use spacers to separate bundles.
- Bundled units should be kept intact until time of installation.
- 7. When handling Ljoists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
- Pick I-joists in bundles as shipped by the supplier
- ■Orient the bundles so that the webs of the I-joists are vertical
- Pick the bundles at the 5th points, using a spreader bar if necessary.
- 8. Do not handle I-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST



MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate or more of the adjacent span. 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. limit states are based on the factored loads of 1.50L + For multiple-span applications, the end spans shall be 40%
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive Standard. No concrete topping or bridging element was of gypsum and/or a row of blocking at mid-span. assumed. Increased spans may be achieved with the used shall meet the requirements given in CGBS-71.26
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- 4. Bearing stiffeners are not required when 1-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI units conversion: 1 inch = 25.4 mm 1 foot = $0.305 \, \text{m}$

SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

	=10.0			Joist Depth
				Joist Series
223.4 23.4 23.11 24.5 24.5	2015 221-15 221-11 221-11 221-11	200 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	16.0 16.0 17.5 17.5	12"
20-8 21-9 22-1 22-6 22-6	20.0 20.0 20.4 20.8	0 0 0 0 7 7 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	15/21 15/21 15/41 16/31	Simple On centre 16"
19'.9" 20'.9" 21'.1" 21'.5"	17:10 18:1 19:1 19:4 19:9 19:51	15.5 16.5 17.4 17.6 17.10	1448 1448 1440 15-6	e spans 'e spacing 19.2
19-10: 20-10: 21-2: 21-6:	18-24 18-25 18-25 18-31 18-310 20-0*	15:60 16:93 17:25:43 17:73 17:73 18:00	3:5! 4! 9! 5:7!	24°
111-92 -111-92 -0-92	2322 23310 23310 2433 250 0	184* 2031 2031 2136 2139 2239	.01-81: 	12"
221.98 241.08 24.10 24.10	20-8 20-11 22-11 22-5 22-10	17.3° 18.5° 18.5° 19-11' 20-2° 20-5°	16:58 16:58 16:78 17:41	Multipl On centr 16"
2119 22411 2313 2319 2410	71 9-181 20-101 211-111 211-101 221-101	19-10 19-0 19-0 19-0 19-10	14:10; 16:31; 16:31; 16:31;	e spans e spacing 19.2"
21:10 23:0 23:4 23:4 23:4	19:4" 20:4" 21:2" 21:46 21:410 22:2"	18-7 18-1 19-1 19-4 19-9	14:7" 15:51 16:11 16:10"	24"

I-JOIST HANGERS

- Hangers shown illustrate the three to support I-joists. most commonly used metal hangers
- 2. All nailing must meet the hanger manutacturer's recommendations
- Hangers should be selected based and load capacity based on the on the joist depth, flange width
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.





Face Mount





CCMC EVALUATION REPORT 13032-R

WEB STIFFENERS

RECOMMENDATIONS:

- Construction Guide (C101).The gap between the stiffener and the flange is at the top. engineered applications with factored A bearing stiffener is required in all reactions greater than shown in the -joist properties table found of the I-joist
- the l-joist is supported in a hanger and the sides of the hanger do not extend up to, and stiffener and flange is at the top. support, the top flange. The gap between the A bearing stiffener is required when
- and the flange is at the bottom. by the code. The gap between the stiffener tip and the support. These values are for than 2,370 lbs is applied to the top flange where a factored concentrated load greater ■ A load stiffener is required at locations adjusted for other load durations as permitted standard term load duration, and may be cantilever, anywhere between the cantilever between supports, or in the case of a

SI units conversion: 1 inch = 25.4 mm

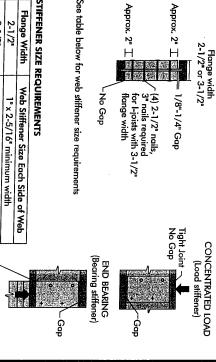
3-1/2

1-1/2" x 2-5/16" minimum width

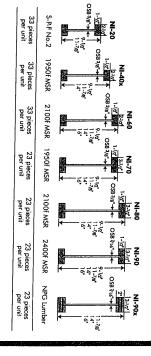
No Gap Tight Join

FIGURE 2

WEB STIFFENER INSTALLATION DETAILS

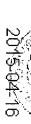


NORDIC I-JOIST SERIES



finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from forest to the products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables Nortic

longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior streamth with Nordic Engineered Wood I-joists use only finger-jointed back spruce 1



INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, coping your
- 2. Except for cutting to length, 1-joist flanges should never be cut, drilled, or notched.
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.

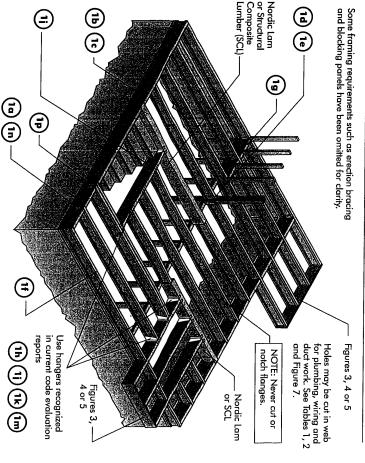
Halle Car

- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple ವರ್ಷ ನಿರುತ್ತು nust
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 2015-04-16
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Hoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Lipists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I joist blocking l-joist-compatible depth selected. panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an
- 13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

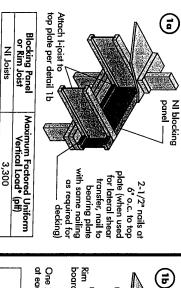
One 2-1/2"

Attach rim board to top

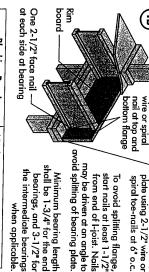
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

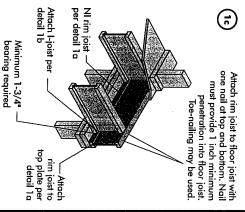


*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see defail 1d.
--



8,090	1-1/8" Rim Board Plus
Maximum Factored Uniform	Blocking Panel
Vertical Load* (plf)	or Rim Joist

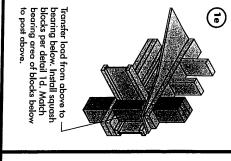
or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or *The uniform vertical load is limited to a rim board depth of 16 inches atter. For concentrated vertical load transfer, see detail 1d

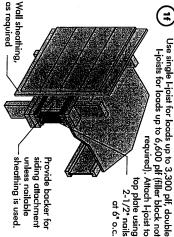


Squash block -	ā
	NI or rim board blocking panel per detail 1a —
	1/16" for squash blocks

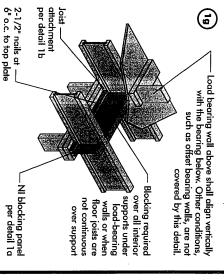
	2x Lumber		Pair of Squash Blocks
) Blocks
4 200	5,500	3-1/2" wide	Maximum Factored Vertical per Pair of Squash Blocks (lbs)
1 100	8,500	5-1/2" wide	red Vertical per h Blocks (lbs)

Provide lateral bracing per detail 1a, 1b, or 1c





required when rim board is used. Bracing per code shall be Rim board may be used in lieu of I-joists. Backer is not

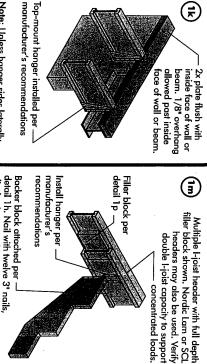


carried to the foundation.

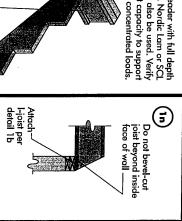
 \equiv

Nordic Lam or SCL

(F)

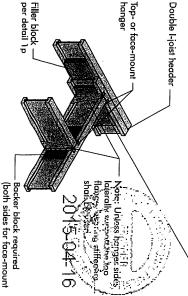


support the top flange, bearing stiffeners shall be used Note: Unless hanger sides laterally Maximum support capacity = 1,620 lbs clinch when possible.



at bearing for lateral Note: Blocking required support, not shown





Verify double I-joist capacity to support concentrated loads. For hanger capacity see hanger manufacturer's recommendations.

nangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

3-1/2"	2-1/2"	Flange Width
1-1/2"	1"	Material Thickness Required*
7-1/4"	5-1/2"	Minimum Depth**

- better for solid sown lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4"



Notes:

support the top flange, bearing stiffeners shall be used. Note: Unless hanger sides laterally beams, see the manutacturer's

For nailing schedules for multiple

recommendations.

installed per manufacturer's

Top- or face-mount hanger

recommendations

(

Filler block

- 2. Leave a 1/8 to 1/4-inch gap between to Support back of I-joist web during nailing to of filler block and bottom of top 1-joist prevent damage to web/flange connection.
- Filler block is required between joists for full length of span.
- 4. Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot possible) on each side of the double I-joi Total of four nails per foot required. If na nails at 12 inches o.c. (clinched when

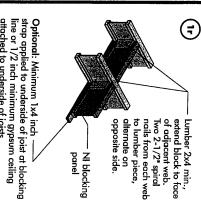
 Offset nails from opposite face by 6"

-1/8" to 1/4" gap between top flange The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

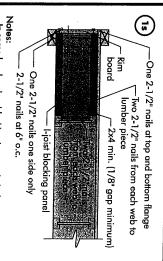
and filler block

DOUBLE I-JOIST CONSTRUCTION FILLER BLOCK REQUIREMENTS FOR

Size	Depth	Block Size
2-1/2"×	9-1/2" 11-7/8"	2-1/8" × 6" 2-1/8" × 8"
1-1/2"	14"	2-1/8" x 10"
	16"	2-1/8" x 12"
	9-1/2"	3" x 6"
3-1/2"×	11-7/8"	သူ × ဇူ
-I/2"	14.	3" × 10"
	16"	3" x 12"
3-1/2"×	11-7/8"	3" x 7"
2 :	14.	3" × 9"
	16"	3" × 11"
	3-1/2" × 1-1	



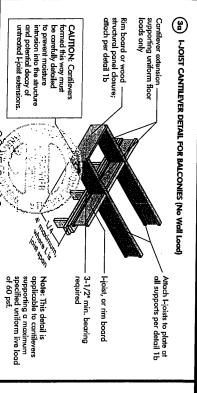
attached to underside of joists.



In some local codes, blocking is prescriptively required in the starter joist. Where required, see local code requirements tor spacing of the blocking the first joist space (or first and second joist space) next to

All nails are common spiral in this detail

CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)





Full depth backer block with 1/8" gap between block and top flange of I-joist. See detail 1 h. Nail with 2 rows of 3" nails at 6" o.c. and clinch.

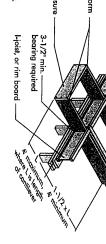
Attach I-joists to plate at all supports per detail 1 b

2x8 min. Nail to backer block and joist with 2 rows of -3" nails at 6" o.c. and clinch. (Canfilever nails may be used to attach backer block if length of nail is sufficient to allow dinching.)

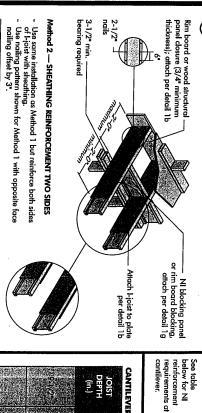
floor loads only Cantilever extension supporting uniform

Note: This detail is applicable to Lumber or wood structural panel closure

specified uniform live load of 60 psf



CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD) (4a) Method 1 — SHEATHING REINFORCEMENT ONE SIDE FIGURE 4 (continued)



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4) required on sides of joist. Depth shall match the full height of the joist. Noil with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced Lioist capacity.

€

thickness); attach per detail 1b to top plate at panel closure (3/4" minimum Rim board, or Attach I-joists wood structural Alternate Method 2 — DOUBLE I-JOIST 'à, Face nail two rows of 3" nails at 12" o.c. each side through one I-joist web and the filler block to other I-joist web. Offset nails NI blocking panel or rim board blocking, attach per detail 1g from opposite face by 6". (four nails per foot required, except Clinch if possible

Block I-joists together with filler blocks for the full length of the reinforcement. The For I-joist flange widths greater than 3 inches place an additional row of 3' nails along the centreline of the reinforcing panel from each side. Clinch when possible.

required

min. bearing

10,

all supports per detail 1b, 3-1/2

Roof truss span

> Girder J Roof trusses

Roof truss span

,– Jack trusses T 21-0" cantilever

13'-0" maximum

For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the I-joist reinforcement requirements for a span of 26 ft. shall be permitted to

cantilever 2<u>-</u>0

JOIST DEPTH (in.)	ROOF TRUSS SPAN (#)	(OOF IL = 30 psf, DL = 15 psf SPAN JOIST SPACING (in.) (ff) 12 16 19.2 24	ipsf 1.) 2.4	ROOF LOADING LL = 40 psf, JOIST SPA 12 16	(UNFACTORED DL = 15 psf CING (in.) 19.2 24		= 50 psf, JOIST SPA	DL = 15 p CING (in.)	94 94
9.11/2.	28 30 4 4 2 3 3 3 3 3		(××××)	ZZZZ:	X 2 2 X	zz	××222	××××	×××××
11.7/8;	26 28 30 32 34		NN 2222	ZZZZZZ =-ZZZZ	N	zzzzz-	2	, קייים אמעט	*****
14	688 ¥8988 888 ¥8988	ZZZZZZZ	11-12-ZZZN		XXXXX	ZZZZZZZ		z×	× ×
	998848888				Z Z Z Z Z Z Z =	ZZZZZZZZ	ZZZZZZZZ	zzzn	×

nails per foot required if

- N = No reinforcement required.
 N reinforced with 3/4 wood structural panel on one side only.
 Ponel on one side only.
 N reinforced with 3/4 wood structural panel on both sides, or double I-joist.
 N reinforced with 3/4 wood structural panel on both sides, or double I-joist.
 N = Try a deeper joist or closer spacing.
 Maximum design load stall les: 15 pet froor that load, and 80 plf wall load. Well load is based on 3-0.0
- For larger openings, or multiple 3-0" width openings spaced less than 6-0" o.c., additional joists beneath the opening's cripple study may be required.

 3. Table applies to joists 17" to 24" o.c. that meet the floor span requirements for a design live load of 40 pst and dead load of 15 pst, and a live load deflection limit of L480, Use 12" o.c. requirements for lesser spacing.
 - 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a

truss is used.

5. Cantilevered joists supporting girder trusses or roof beams may require additional

WEB HOLES

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS.

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. hole or duct chase opening shall be in compliance with the requirements of
- Ņ I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. The maximum size hole or the maximum depth of a duct chase opening that can between the top or bottom of the hole or opening and the adjacent I-joist flange the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained be cut into an I-joist web shall equal the clear distance between the flanges of
- Ņ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥. Where more than one hole is necessary, the distance between adjacent hole size of the largest square hole (or twice the length of the langest side of the langest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively. edges shall exceed twice the diameter of the largest round hole or twice the
- 7. A knockout is **not** considered a hole, may be utilized anywhere it occurs, and and/or duct chase openings. may be ignored for purposes of calculating minimum distances between holes
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a camilevered section of a joist. Holes of greater size may be permitted subject to verification.
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- <u>5</u> All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

AL					Joist Depth
_	100		ol-lekele et		1
					Minimum 4
				100	distance fi 5 6
	16543 16543 16643		54 dt 64 s	22.72.2 41.74.5	rom inside Round 6-1/4
I			300 300 300 300 300 300 300 300 300 300) face of a d hole dia
ľ			0 1 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	42	ny suppor meter (in. 8-5/8
	- 1 m	7 ,000		24.5	† to centro) 9 10
				And the Sec	∋ of hole (f 10-3/4 11
l	100			1111	1-in.)
708.50	100		PK MORIJA		S adju 12-3/4 Fc
China			8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		Span justment factor

- Above table may be used for I-joist spacing of 24 inches on centre or less.

 Hole location distance is measured from inside face of supports to centre of hole.

 Distances in this chart are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the Hoists used at their maximum span. If the Hoists are placed at less than their full maximum span (see Maximum Pacio Spans). The minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced = <u>Lactual</u> x D

Where: Dreduced Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications distance shall not be less than 6 inches from the face of the support to edge of the hole.

The actual measured span distance between the inside faces of supports (ft).

Factual

₽ ₹

Span Adjustment Factor given in this table.

The minimum distance from the inside face of any support to centre of hole from this table

<u>lactual</u> is greater than 1, use 1 in the above calculation for <u>lactual</u> CAF

2019-04-16

(fit). The reduced

TABLE 2

electrical or small plumbing lines. They tor the contractor's convenience to install Knockouts are prescored holes provided DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

Joist Joist	Minimum dis	lance from in	side face o	of any sup	oport to c	entre of o	pening (f	Ė
	e de la composition della comp				18	20		24
	1001 1001 1001	6.0	0000 1004	765 110	776 773	8-08-6 -08-6	7.T. 7 8:2* 8	8:6 8:9
								4.4
								20
72°								61
i i i								ω <u>υ</u>
	150						7	S≧.
								e G E
								i Q

- Above table may be used for Ljoist spacing of 24 inches on centre or less.
 Duct chase opening location distance is measured from inside face of supports to centre of opening.
 The above table is based on simple-span joists only. For other applications, contact your local distributor.
 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor.

and may be ignored for purposes of calculating minimum distances A knockout is NOT considered a hole, may be utilized wherever it occurs

Knockouts

See rule 12

between top and bottom flange — all duct chase openings and holes Maintain minimum 1/8" space

> sharp saw. should be cut with a Holes in webs notch the flange, or over-cut the web Never drill, cut or

bearing -

distance from tor minimum See Table 1

> of larger 2x diameter

diameter, whichever is length or hole 2x duct chase

from bearing) minimum distance see Table 2 for

spaced 15 inches on centre along the length of the I-joist. Where possible, it is

are 1-1/2 inches in diameter, and are

preferable to use knockouts instead of

field-cut holes

Duct chase opening

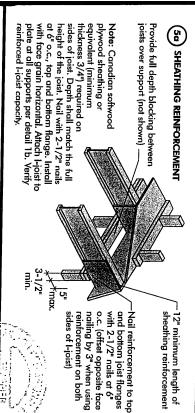
FIELD-CUT HOLE LOCATOR

FIGURE 7

and then making the cuts between the corners, as this can cause unnecessary stress concentrations. Slightly rounding the holes is another good method to diameter hole in each of the four corners the rectangular hole by drilling a 1-inch the corners is recommended. Starting for rectangular holes, avoid over-cutting

minimize damage to the I-joist

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



(5E)

SET-BACK DETAIL

attach per detail 1b. (3/4" minimum thickness),

Provide full depth blocking

between joists over support

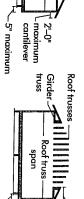
structural panel closure

Rim board or wood

Bearing walls



FIGURE 5 (continued)





trusses running parallel to the cantilevered floor joists, requirements for a span of the I-joist reinforcement For hip roofs with the jack 26 ft. shall be permitted to

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

JOIST DEPTH (in.)	ROOF TRUSS SPAN (H)	LL = JOI 12	30 psf, [ST SPAC)L = 15 ₁ 	osf 24	ROOF L LL = JC 12	OADING = 40 psf, DIST SPA 16	(UNFAC DL = 15 CING (in 19.2	TORED) psf ;) 24	12	_ F	LL = 50 psf, JOIST SPA	LL = 50 psf, DL = 15 JOIST SPACING (ir 2 16 19.2
9.1/ <u>2</u> .4	9 4 2 9 8 6 4 2 9 8 6	いいいーニュ	X	XXXXX	×××××	ממממא	×××××	× × × ×	7.3	(xxxxx		2 2 3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	X X X X X X X X X X X X X X X X X X X
	30 30 32 34 34	zz	, 00000 × × ×	********* ****************************	<×××××	לֶבָּבִיבְּטְמֵנְ	<×××××	EPHENDS		××××××		×2222 - X	×02022->
	28622884 28622884	zzzz	ברטטטטט ברטטטטטט	××××××N	*××××××	3	<×××××××××××××××××××××××××××××××××××××	<××××××	在一种,在1000年,1000年			22	22
O TOTAL	548888888888888888888888888888888888888	-zzzzzz <u>zz</u> -	גבביב מממממ	×××××××××××××××××××××××××××××××××××××	·×××××××××××××××××××××××××××××××××××××	zzz	××××××××××××××××××××××××××××××××××××××	××××××××		*****		××××××××××××××××××××××××××××××××××××××	222

through joist web and web of girder using 2-1/2" nails. Alternate for opposite side.	Vertical solid sawn blocks Vertical solid sawn blocks (2x6 S-P-F No. 2 or better) noticed
Hanger may be used in lieu of solid sawn blocks	— Nail joist end using 3" nails, toe-nail at top and bottom flanges.

Notes:

1

Attach double I-joist per detail 1p, if required Verify girder joist capacity if the back span

exceeds the joist spacing.

50

supports per detail 1b. 3-1/2" minimum I-joist (not shown for clarity)
Attach I-joist to plate at all

max.

bearing required

girder joist per detail 5c. Attach joists to

- N = No reinforcement required.
 N = NI reinforced with 3/4" wood structural
- panel on one side only.
 2 = NI reinforced with 3/4" wood structural
- X = Try a deeper joist or closer spacing.

 2. Maximum design load shall be: 15 psf roof panel on both sides, or double I-joist.
- dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0"
- For larger openings, or multiple 3'.0" width openings spaced less than 6'.0" o.c., additional joists beneath the opening's cripple studs may be required.
- the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use Table applies to joists 12" to 24" o.c. that meet 12" o.c. requirements for lesser spacing.
 - 4. For conventional roof construction using a distance between the supporting walls as if a the supporting wall and the ridge beam. When the roof is framed using a ridge board, ridge beam, the Roof Truss Span column above is equivalent to the distance between the Roof Truss Span is equivalent to the truss is used.
- Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from Ljoist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manutacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single 1-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- 8. Tap the second row of panels into place, using a block to protect groove edges.
- 9. Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and nail to assure accurate and consistent spacing.) 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all railing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24 3/4	20 5/8	116	Maximum Minimum Joist Panel Spacing Thickness (in.) (in.)
2"	2	2"	Common Wire or Spiral Nails
1-3/4"	1-3/4"	1-3/4"	ail Size and Ty Ring Thread Nails or Screws
2	2	2"	pe Staples
6"	6"	6*	Maximun of Fas Edges
12"	12"	12"	n Spacing teners Interm. Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

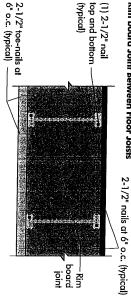
IMPORTANT NOTE:

Floor sheathing must be field glued to the L-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, L-joist spans must be verified with your local distributor.

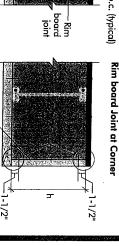
RIM BOARD INSTALLATION DETAILS

(8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim board Joint Between Floor Joists

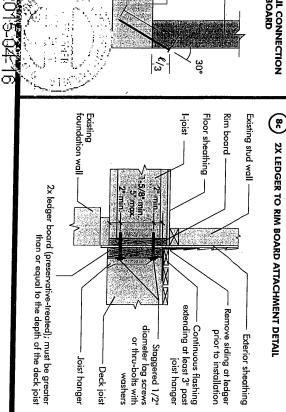


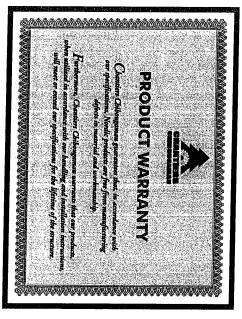
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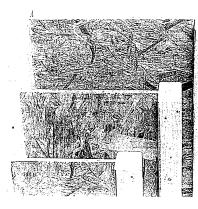


Rim board joint -

(B) Top or sole plate Rim board TOE-NAIL CONNECTION AT RIM BOARD P/3







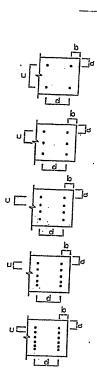
· MICRO CITY

Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

LVL HEADER AND CONVENTIONAL LUMBER NAILING DETAILS DETAIL NUMBER OF ROWS SPACING (INCHES o/o "d" A 2 12 B 2 6 D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6 4D 6 4		TIVI HEAT	DED AND OOK	11 729 1 788 1 2 7
DETAIL NUMBER NUMBER OF ROWS SPACING (INCHES o/o "d" A 2 12 B 2 8 C 2 6 D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6				
A 2 12 B 2 8 C 2 6 D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6		DETAIL	NUMBER	SPACING (INCHES o/c)
B 2 8 C 2 6 D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6		A	1 2	
C 2 6 D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3B 5 8 3C 5 6 3D 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6				
D 2 4 1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3A 5 12 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6				
1A 3 12 1B 3 8 1C 3 6 1D 3 4 2A 4 12 2B 4 8 2C 4 6 2D 4 4 3A 5 12 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6			2	
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2D 4 4 3A 5 12 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6		2C	4	
3A 5 12 3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6		2D	4	
3B 5 8 3C 5 6 3D 5 4 4A 6 12 4B 6 8 4C 6 6			5	
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4A 6 12 4B 6 8 4C 6 6	L		5	
4B 6 8 4C 6 6	L			4
4C 6 6	L		6 .	12
	L			8
4D 6 4	Ŀ			
	L	4D	6	4



NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
- (2) MINIMUM LUMBER END DISTANCE "b" = 2"
- (3) MINIMUM NAIL ROW SPACING "c" = 2"
- (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
- (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DNG NO TANNICOI. 14
STRUCTURAL
GOMPONENT ONLY
TO BE USED ONLY
WITH BEAM CALCS
PSEARING THE
STAMP BELOWS

PROVICE NATLING
DETAIL № × SEE
DNO #TAMN1001-14