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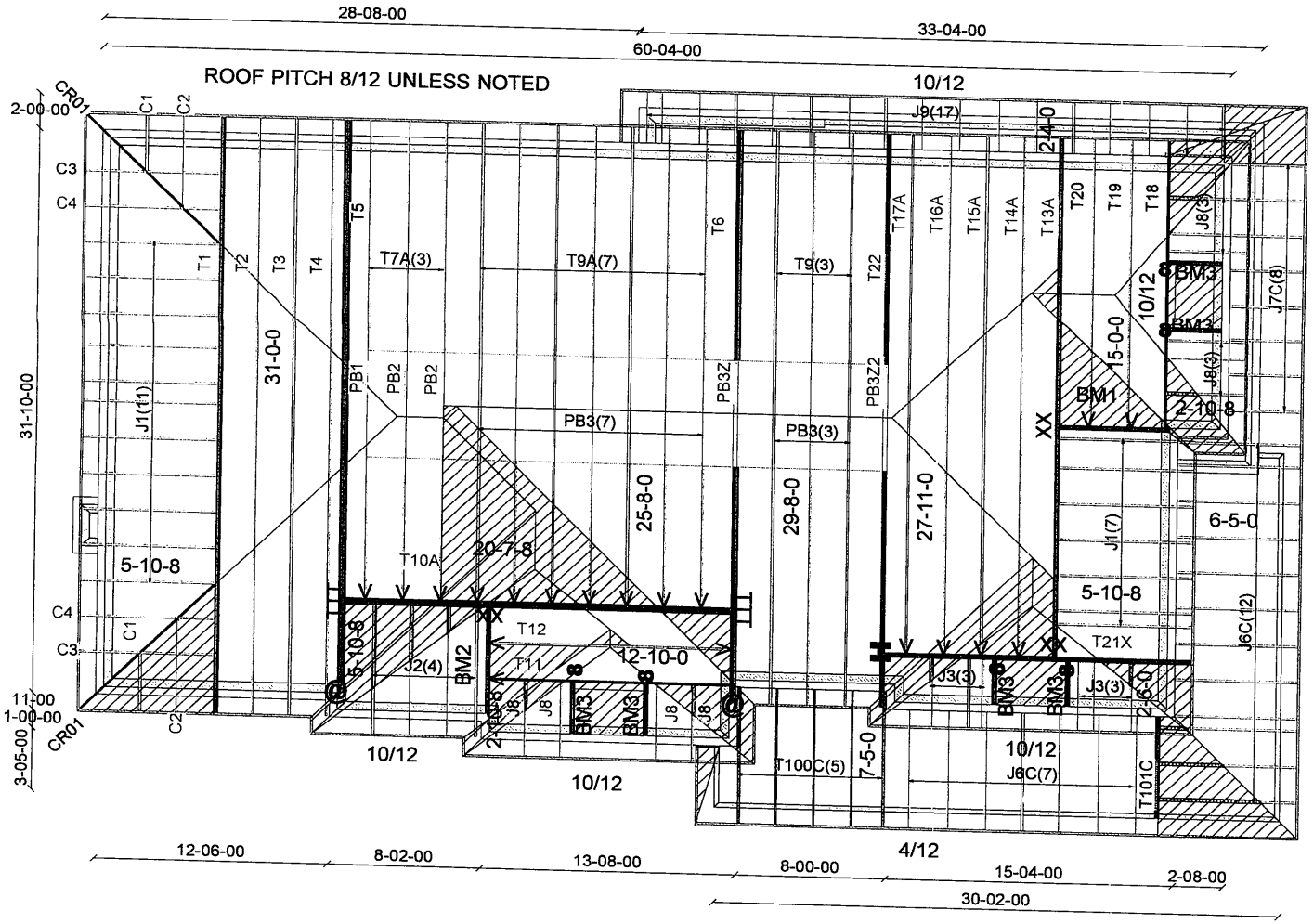

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
LUS24-2 -(8)
HGUS26-2 - (XX)
HGUS28-2 -(H)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(@)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2, BM3 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-8C / A STD.	
Layout ID: 296632	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/4/2018	Designer: AC/S.V.E	

Mitek ver 7.5.0

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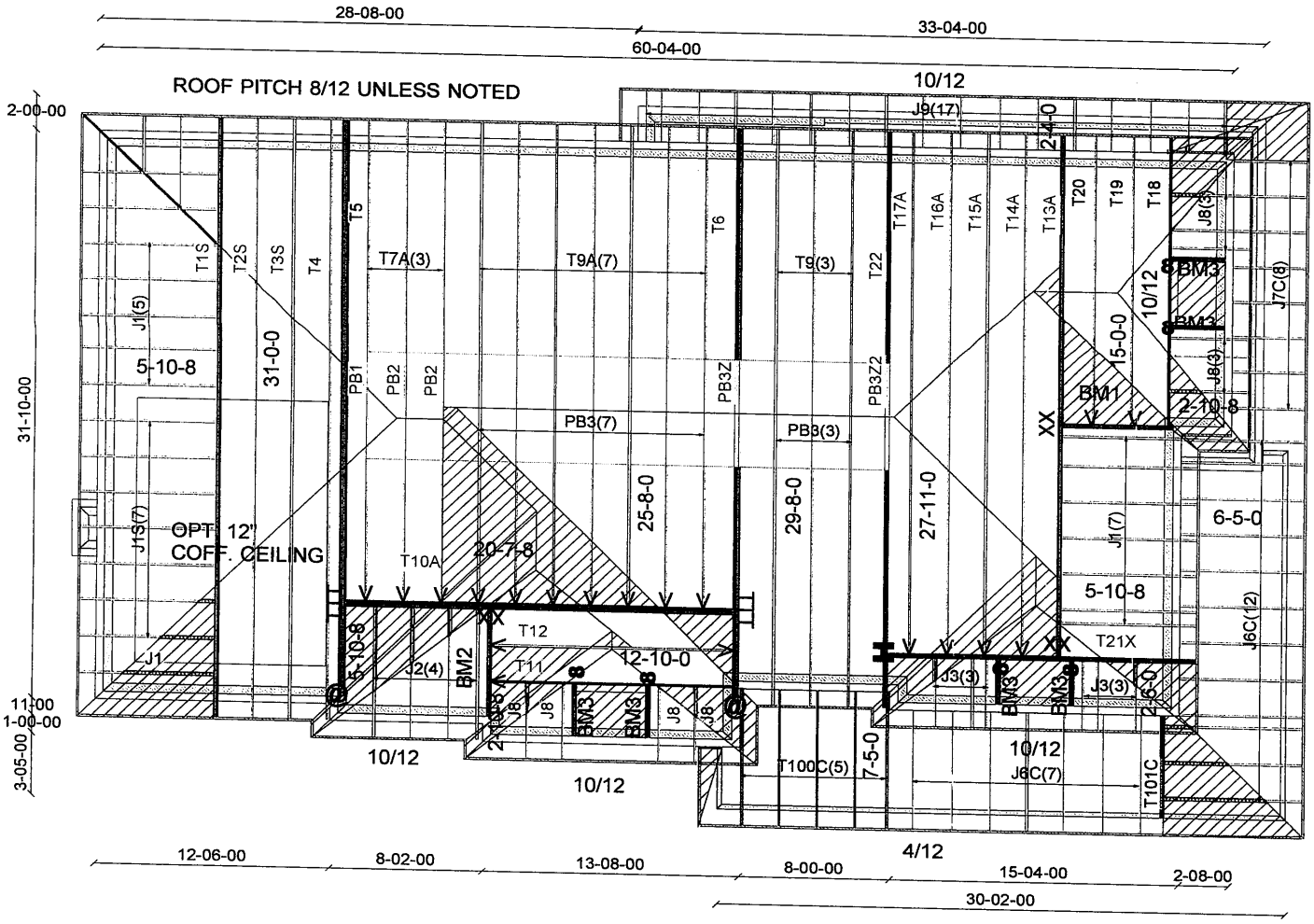

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LJS26DS - (V)
LUS24-2 -(8)
HGUS26-2 - (XX)
HGUS28-2 -(H)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(@)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2, BM3 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-8C / A STD. +OPT. COFF.	
Layout ID: 287687	Project: GREEN VALLEY ESTATES		
Plan Log: 94071	Date: 4/4/2018	Designer: AC/S.V.E	

Mitek ver 7.5.0

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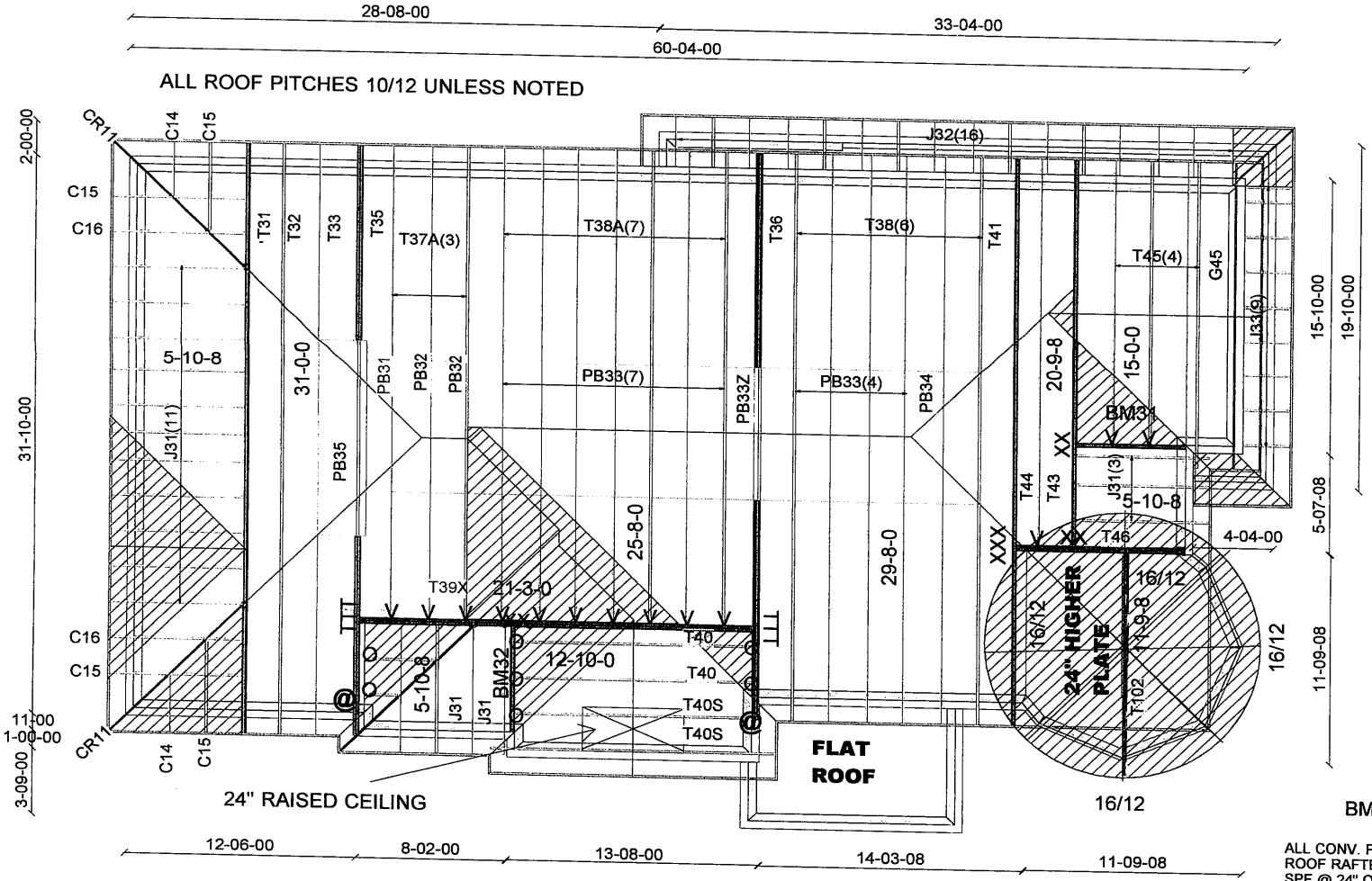

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 -(O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 -(XXX)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(@)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM31,BM32 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-8C / B STD.	
Layout ID: 296633	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/4/2018	Designer: AC/S.V.E	

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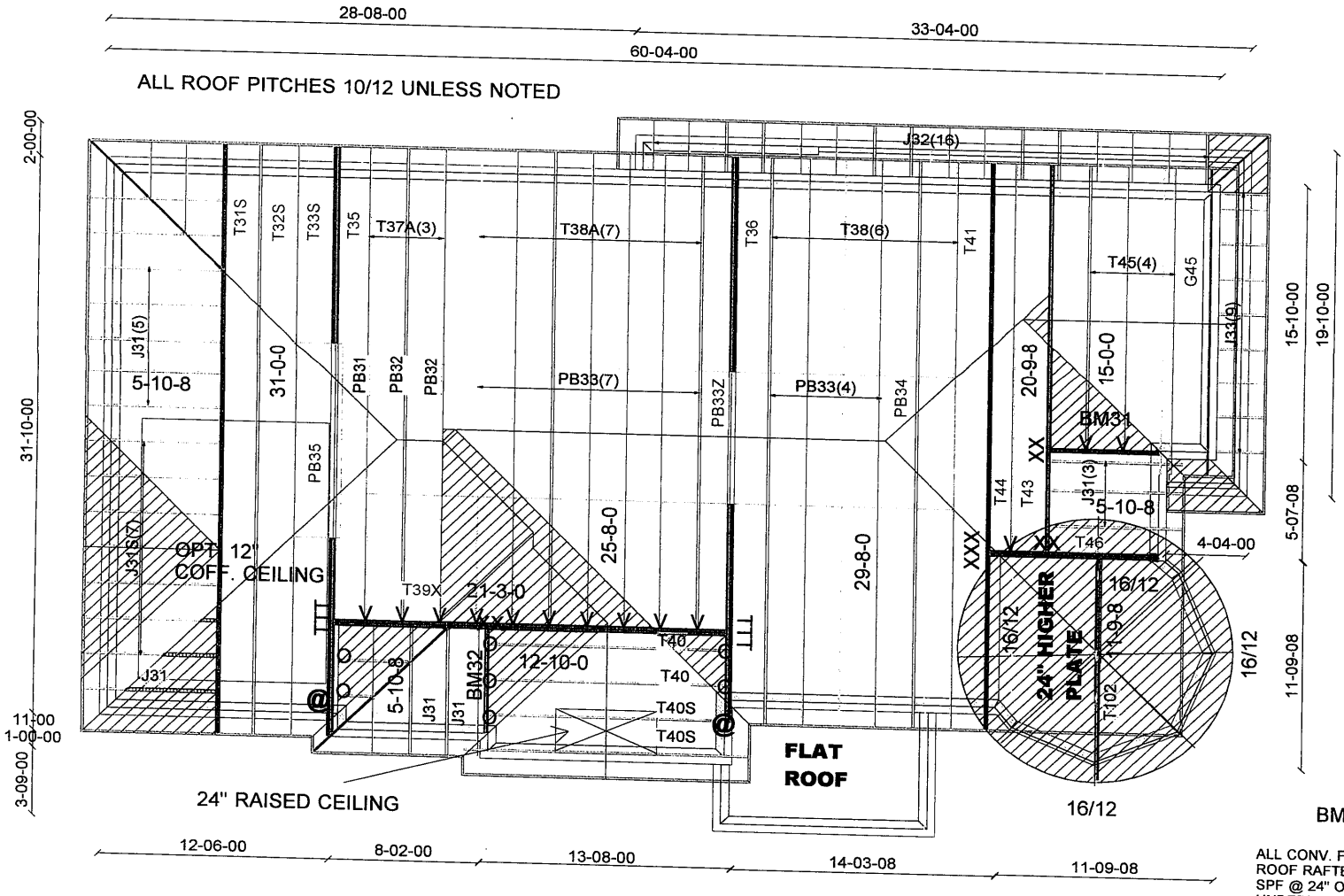

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 -(O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 -(XXX)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(@)

DENOTES CONV. FRAMING

DES'GN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM31,BM32 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

m11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: **44755**
Layout ID: **287688**
Plan Log: **94071**

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**
Project: **GREEN VALLEY ESTATES**
Date: **4/4/2018** Designer: AC/S.V.E

Model / Elevation: **S42-8C / B STD. +OPT. COFF.**

Mitek ver 7.5.0

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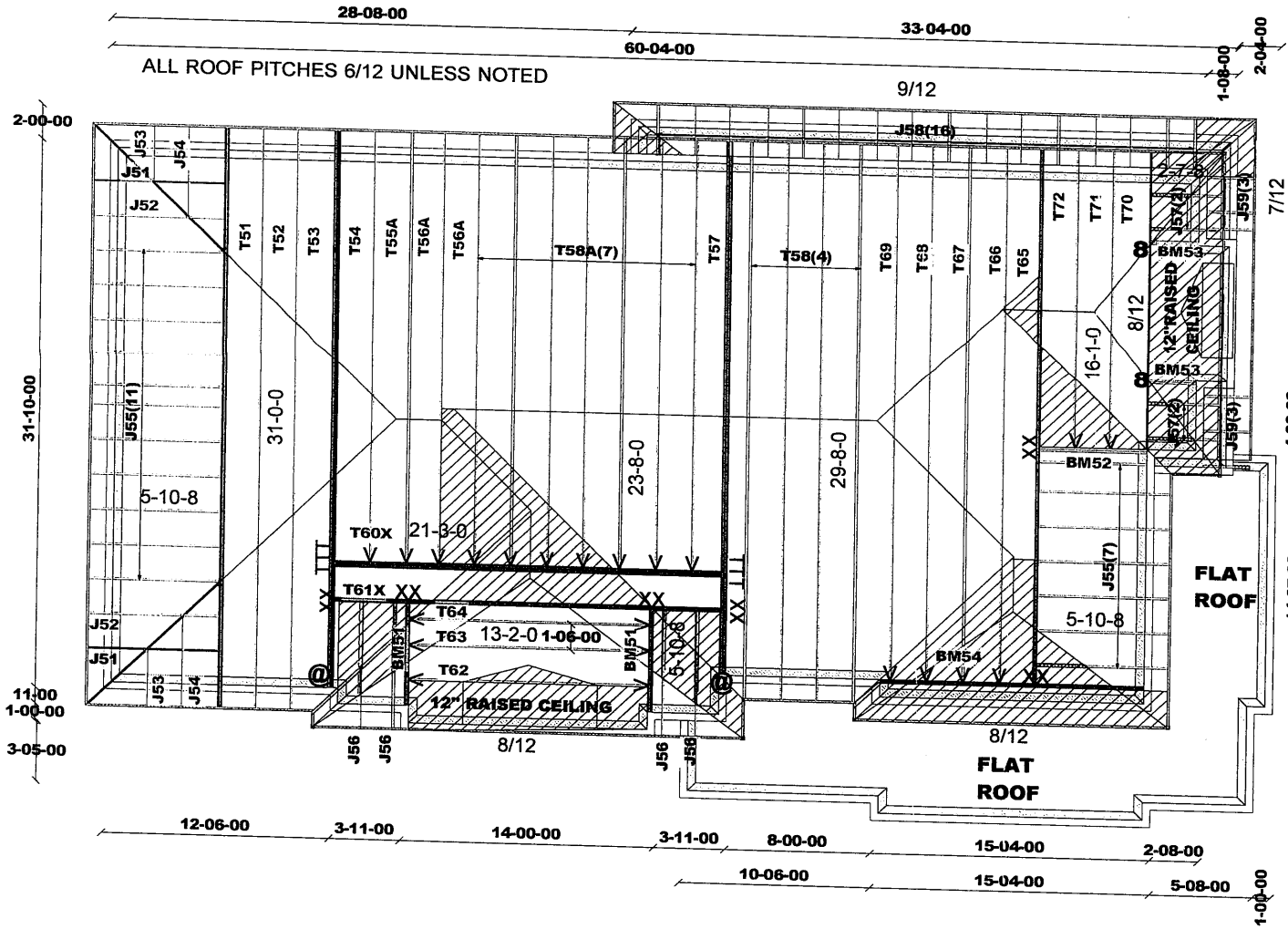

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
LUS24-2 - (8)
HGUS26-2 - (XX)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(2)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM51,BM52,BM53 = 2-2X10
BM54 = 3PLY - 1 3/4" X 11 7/8"LVL2.0E

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-8C / C STD.
	Layout ID: 296634	Project: GREEN VALLEY EAST	
	Plan Log: 94071	Date: 4/5/2018 Designer: AC/S.V.E	

Mitek ver 7.5.0

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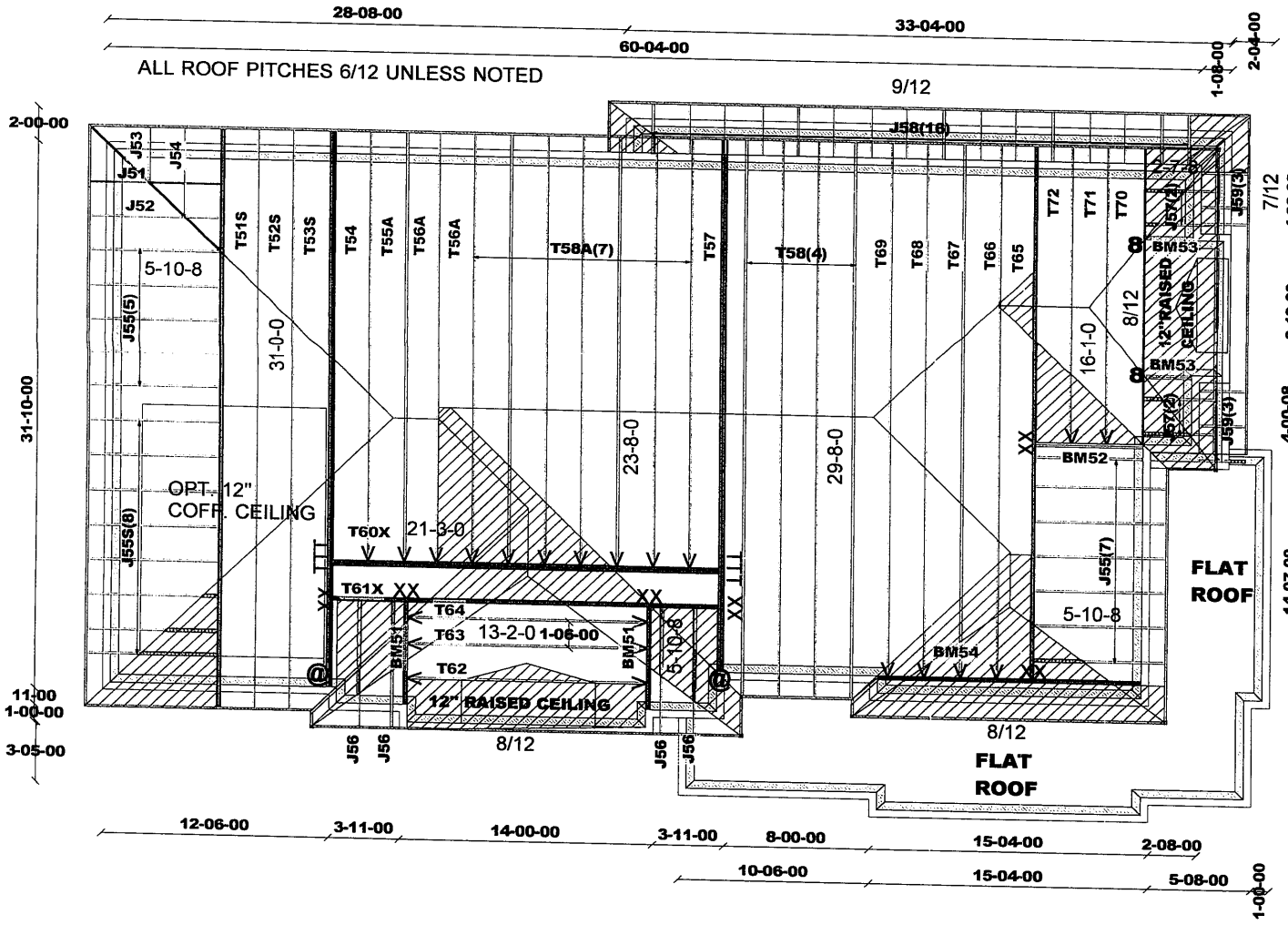


ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
LUS24-2 - (8)
HGUS26-2 - (XX)
THGQ3-SDS4.5 - (TTT)
CP3-6 -(2)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM51,BM52,BM53 = 2-2X10
BM54 = 3PLY - 1 3/4" X 11 7/8"LVL2.0E

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,573

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-8C / C STD. +OPT. COFF.	
Layout ID: 287657	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/5/2018	Designer: AC/S.V.E	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

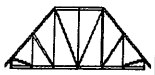
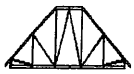
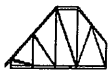
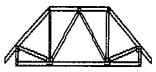
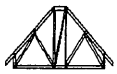
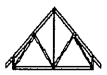
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296632 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: A STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	8.00 0.00	31-00-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	389.22 236.00		
	1	T2 HIP	8.00 0.00	31-00-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.79 87.34		
	1	T3 HIP	8.00 0.00	31-00-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	139.16 88.33		
	1	T4 HIP	8.00 0.00	31-00-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.30 88.67		
	1 3 Ply	T5 HIP GIRDER	8.00 0.00	31-00-00	09-08-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	713.16 432.00		
	1 3 Ply	T6 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	662.94 400.02		
	3	T7A PIGGYBACK	8.00 0.00	25-08-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-11-08	381.72 241.50		
	3	T9 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	404.25 249.99		
	7	T9A PIGGYBACK	8.00 0.00	25-08-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-00-13	860.37 535.50		
	1 2 Ply	T22 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	417.98 256.00		
	1 2 Ply	T13A ROOF	8.00 0.00	27-11-00	05-03-13	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 03-11-02	341.44 206.00		
	1	T14A HIP	8.00 0.00	27-11-00	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	126.99 81.50		
	1	T15A HIP	8.00 0.00	27-11-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	129.35 81.00		
	1	T16A HIP	8.00 0.00	27-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	128.87 80.17		
	1	T17A HIP	8.00 0.00	27-11-00	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	143.82 89.83		
	1	T18 HIP GIRDER	8.00 0.00	15-00-00	04-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	61.77 39.00		
	1	T19 HIP	8.00 0.00	15-00-00	05-08-06	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.89 45.67		
	1	T20 COMMON	8.00 0.00	15-00-00	06-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	64.27 39.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

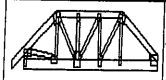
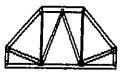
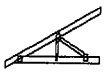
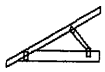
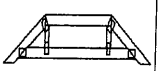
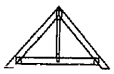
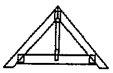
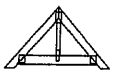
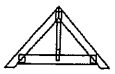
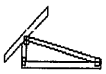
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296632 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: A STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T21X HIP GIRDER	8.00 0.00	15-01-00	03-08-10	2 X 6	2 X 8	01-03-08 00-00-00	01-04-13 01-00-02	197.48 126.00		
	1 3 Ply	T10A HIP GIRDER	8.00 0.00	20-07-08	06-06-06	2 X 6	2 X 6	00-00-00 00-00-00	01-07-13 01-04-13	394.14 242.01		
	1	T11 HIP GIRDER	8.00 0.00	12-10-00	04-00-06	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	52.19 33.00		
	1	T12 COMMON	8.00 0.00	12-10-00	05-08-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	52.58 33.67		
	5	T100C JACK-PARTIAL	4.00 0.00	06-08-00	02-09-10	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 02-09-10	108.10 66.65		
	1 2 Ply	T101C JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 6	00-10-08 00-00-00	00-03-15 02-05-10	43.70 26.66		
	1	PB1 PIGGYBACK	8.00 0.00	05-09-02	01-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	16.78 11.67		
	2	PB2 PIGGYBACK	8.00 0.00	05-09-02	02-03-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	35.68 23.34		
	10	PB3 PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	141.40 96.70		
	1 3 Ply	PB3Z PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	42.42 29.01		
	1 2 Ply	PB3Z2 PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	28.28 19.34		
	18	J1 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	320.94 192.06		
	2	C1 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	18.34 11.34		
	2	C2 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	23.82 15.34		
	2	C3 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	33.68 20.66		
	2	C4 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	39.16 24.66		
	4	J2 JACK-OPEN	10.00 0.00	05-10-08	06-06-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 06-06-06	75.96 48.00		
	6	J3 JACK-OPEN	10.00 0.00	02-06-00	03-08-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 03-08-10	60.78 43.98		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

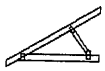
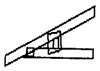
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296632 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: A STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	J8 JACK-OPEN	10.00 0.00	02-10-08	04-00-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 04-00-06	111.20 73.30		
	19	J6C JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 02-05-10	337.06 209.00		
	8	J7C JACK-OPEN	4.00 0.00	03-09-00	01-06-15	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 01-06-15	82.88 53.36		
	17	J9 JACK	10.00 0.00	02-04-00	03-01-07	2 X 4	2 X 4	01-03-08 00-00-00	01-02-02 03-01-07	155.89 102.00		

TOTAL # TRUSS= **157.00** TOTAL BFT OF ALL TRUSSES= **4779.27** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7684.75** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
1	Hangers	HGUS28-2	
3	Hangers	HGUS26-2	
6	Hangers	LUS24-2	
21	Hangers	LJS26DS	

TOTAL # ITEMS= **35.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

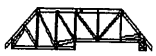
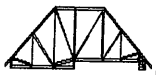
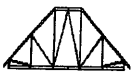
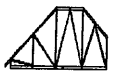
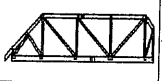
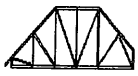
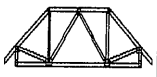
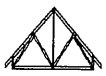
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287687 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-8C **ELEVATION:** A STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S HIP GIRDER	8.00 0.00	31-00-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	395.60 244.34		
	1	T2S HIP	8.00 0.00	31-00-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	144.98 94.33		
	1	T3S HIP	8.00 0.00	31-00-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	147.61 93.67		
	1	T4 HIP	8.00 0.00	31-00-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.30 88.67		
	1 3 Ply	T5 HIP GIRDER	8.00 0.00	31-00-00	09-08-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	713.16 432.00		
	1 3 Ply	T6 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	662.94 400.02		
	3	T7A PIGGYBACK	8.00 0.00	25-08-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-11-08	381.72 241.50		
	3	T9 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	404.25 249.99		
	7	T9A PIGGYBACK	8.00 0.00	25-08-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-00-13	860.37 535.50		
	1 2 Ply	T22 PIGGYBACK	8.00 0.00	29-08-00	09-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	417.98 256.00		
	1 2 Ply	T13A ROOF	8.00 0.00	27-11-00	05-03-13	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 03-11-02	341.44 206.00		
	1	T14A HIP	8.00 0.00	27-11-00	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	126.99 81.50		
	1	T15A HIP	8.00 0.00	27-11-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	129.35 81.00		
	1	T16A HIP	8.00 0.00	27-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	128.87 80.17		
	1	T17A HIP	8.00 0.00	27-11-00	10-07-13	2 X 4	2 X 4	01-03 08 00-00-00	01-04-13 02-06-13	143.82 89.83		
	1	T18 HIP GIRDER	8.00 0.00	15-00-00	04-00-06	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	61.77 39.00		
	1	T19 HIP	8.00 0.00	15-00-00	05-08-06	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.89 45.67		
	1	T20 COMMON	8.00 0.00	15-00-00	06-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	64.27 39.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

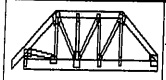
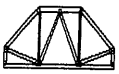
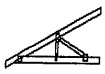
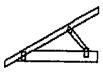
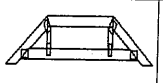
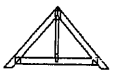
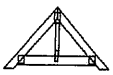
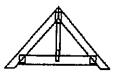
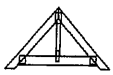
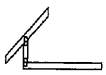
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287687 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: A STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T21X HIP GIRDER	8.00 0.00	15-01-00	03-08-10	2 X 6	2 X 8	01-03-08 00-00-00	01-04-13 01-00-02	197.48 126.00		
	1 3 Ply	T10A HIP GIRDER	8.00 0.00	20-07-08	06-06-06	2 X 6	2 X 6	00-00-00 00-00-00	01-07-13 01-04-13	394.14 242.01		
	1	T11 HIP GIRDER	8.00 0.00	12-10-00	04-00-06	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	52.19 33.00		
	1	T12 COMMON	8.00 0.00	12-10-00	05-08-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	52.58 33.67		
	5	T100C JACK-PARTIAL	4.00 0.00	7-5-0	02-09-10	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 02-09-10	108.10 66.65		
	1 2 Ply	T101C JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 6	00-10-08 00-00-00	00-03-15 02-05-10	43.70 26.66		
	1	PB1 PIGGYBACK	8.00 0.00	05-09-02	01-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	16.78 11.67		
	2	PB2 PIGGYBACK	8.00 0.00	05-09-02	02-03-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	35.68 23.34		
	10	PB3 PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	141.40 96.70		
	1 3 Ply	PB3Z PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	42.42 29.01		
	1 2 Ply	PB3Z2 PIGGYBACK	8.00 0.00	04-05-02	01-10-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	28.28 19.34		
	13	J1 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	231.79 138.71		
	7	J1S JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-03-13	165.83 112.00		
	1	C1 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	7.67 4.67		
	1	C2 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	10.41 6.67		
	1	C3 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	C4 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	14.97 9.33		
	4	J2 JACK-OPEN	10.00 0.00	05-10-08	06-06-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 06-06-06	75.96 48.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

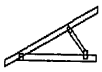
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287687 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: A STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	J3 JACK-OPEN	10.00 0.00	02-06-00	03-08-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 03-08-10	60.78 43.98		
	10	J8 JACK-OPEN	10.00 0.00	02-10-08	04-00-06	2 X 4	2 X 4	01-03-08 00-00-00	01-07-10 04-00-06	111.20 73.30		
	19	J6C JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 02-05-10	337.06 209.00		
	8	J7C JACK-OPEN	4.00 0.00	03-09-00	01-06-15	2 X 4	2 X 4	00-10-08 00-00-00	00-03-15 01-06-15	82.88 53.36		
	17	J9 JACK	10.00 0.00	02-04-00	03-01-07	2 X 4	2 X 4	01-03-08 00-00-00	01-02-02 03-01-07	155.89 102.00		

TOTAL # TRUSS= **155.00** TOTAL BFT OF ALL TRUSSES= **4814.59** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7715.73** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
1	Hangers	HGUS28-2	
6	Hangers	LUS24-2	
21	Hangers	LJS26DS	
3	Hangers	HGUS26-2	

TOTAL # ITEMS= **35.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

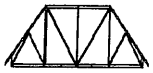
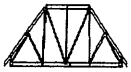
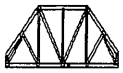
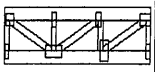
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296633 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: B STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T31 HIP GIRDER	10.00 0.00	31-00-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	00-00-00 00-00-00	399.62 238.66		
	1	T32 HIP	10.00 0.00	31-00-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	144.59 90.67		
	1	T33 HIP	10.00 0.00	31-00-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	159.65 100.67		
	1 3 Ply	T35 PIGGYBACK	10.00 0.00	31-00-00	09-10-07	2 X 6	2 X 6	01-03-08 01-03-08	00-00-00 00-00-00	730.98 440.01		
	1 2 Ply	T41 HIP GIRDER	10.00 0.00	29-08-00	09-02-08	2 X 6	2 X 6	01-03-08 01-03-08	00-00-00 00-00-00	410.28 244.00		
	1 3 Ply	T36 PIGGYBACK	10.00 0.00	29-08-00	10-10-08	2 X 6	2 X 6	01-03-08 01-03-08	00-00-00 00-00-00	677.73 408.00		
	3	T37A PIGGYBACK	10.00 0.00	25-08-00	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-00	457.26 285.00		
	6	T38 PIGGYBACK	10.00 0.00	29-08-00	10-10-08	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	969.72 600.00		
	7	T38A PIGGYBACK	10.00 0.00	25-08-00	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	1051.82 652.19		
	1 2 Ply	T43 HALF HIP	10.00 0.00	20-09-08	06-06-07	2 X 4	2 X 6	01-03-08 00-00-00	00-00-00 00-00-00	248.98 152.00		
	1	T44 HALF HIP	10.00 0.00	20-09-08	08-02-07	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	100.02 62.00		
	4	T45 COMMON	10.00 0.00	15-00-00	07-10-11	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	280.44 178.68		
	1	G45 GABLE	10.00 0.00	15-00-00	07-10-11	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	74.50 47.67		
	1 3 Ply	T39X HIP GIRDER	10.00 0.00	21-03-00	06-06-07	2 X 6	2 X 6	00-00-00 00-00-00	01-05-03 01-07-11	422.43 260.01		
	2	T40 COMMON	10.00 0.00	12-10-00	06-11-14	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	115.20 73.66		
	2	T40S ROOF	10.00 0.00	12-10-00	06-11-14	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	144.44 97.34		
	1 3 Ply	T46 FLAT GIRDER	0.00 0.00	09-01-00	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	00-00-00 00-00-00	141.75 93.99		
	1 2 Ply	T102 COMMON	16.00 0.00	11-09-08	09-07-13	2 X 6	2 X 6	00-10-08 00-10-08	00-00-00 00-00-00	179.96 108.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

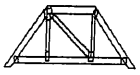
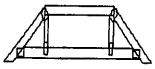
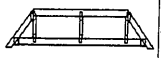
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296633 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: B STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB31 PIGGYBACK	10.00 0.00	07-06-14	02-02-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	25.93 17.33		
	2	PB32 PIGGYBACK	10.00 0.00	07-06-14	03-06-11	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	48.20 29.66		
	11	PB33 PIGGYBACK	10.00 0.00	06-02-14	03-00-00	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	221.65 141.13		
	1 3 Ply	PB33Z PIGGYBACK	10.00 0.00	06-02-14	03-00-00	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	60.45 38.49		
	1	PB34 PIGGYBACK	10.00 0.00	06-02-14	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	18.61 12.33		
	1 3 Ply	PB35 PIGGYBACK	10.00 0.00	09-11-14	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	83.82 52.50		
	16	J31 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	304.00 192.00		
	1	C13 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	00-00-00 00-00-00	6.45 4.67		
	3	C14 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 01-10-15	00-00-00 00-00-00	28.26 20.01		
	5	C15 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 04-01-01	00-00-00 00-00-00	87.90 58.35		
	3	C16 JACK-OPEN	10.00 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 02-01-01	00-00-00 00-00-00	61.65 39.00		
	16	J32 JACK	10.00 0.00	02-04-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	139.52 96.00		
	9	J33 JACK-OPEN	10.00 0.00	01-11-12	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	66.51 42.03		

TOTAL # TRUSS= **122.00** TOTAL BFT OF ALL TRUSSES= **4876.05** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7862.32** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
1	Hangers	HGUS26-3	
3	Hangers	HGUS26-2	
7	Hangers	LUS24	
13	Hangers	LJS26DS	

TOTAL # ITEMS= **28.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

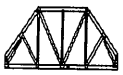
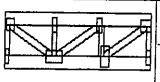
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287688 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: B STD. +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31S HIP GIRDER	10.00 / 0.00	31-00-00	06-06-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	428.48 / 261.66		
	1	T32S HIP	10.00 / 0.00	31-00-00	08-02-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	158.98 / 101.83		
	1	T33S HIP	10.00 / 0.00	31-00-00	09-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	165.81 / 106.17		
	1 / 3 Ply	T35 PIGGYBACK	10.00 / 0.00	31-00-00	09-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	730.98 / 440.01		
	1 / 3 Ply	T36 PIGGYBACK	10.00 / 0.00	29-08-00	10-10-08	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	677.73 / 408.00		
	3	T37A PIGGYBACK	10.00 / 0.00	25-08-00	10-10-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-01-00	457.26 / 285.00		
	6	T38 PIGGYBACK	10.00 / 0.00	29-08-00	10-10-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	969.72 / 600.00		
	7	T38A PIGGYBACK	10.00 / 0.00	25-08-00	10-10-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-11-11	1051.82 / 652.19		
	1 / 2 Ply	T41 HIP GIRDER	10.00 / 0.00	29-08-00	09-02-08	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	410.28 / 244.00		
	1 / 2 Ply	T43 HALF HIP	10.00 / 0.00	20-09-08	06-06-07	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 06-06-07	248.98 / 152.00		
	1	T44 HALF HIP	10.00 / 0.00	20-09-08	08-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-02-07	100.02 / 62.00		
	4	T45 COMMON	10.00 / 0.00	15-00-00	07-10-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.44 / 178.68		
	1	G45 GABLE	10.00 / 0.00	15-00-00	07-10-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	74.50 / 47.67		
	1 / 3 Ply	T39X HIP GIRDER	10.00 / 0.00	21-03-00	06-06-07	2 X 6	2 X 6	00-00-00 / 00-00-00	01-05-03 / 01-07-11	422.43 / 260.01		
	2	T40 COMMON	10.00 / 0.00	12-10-00	06-11-14	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 01-07-11	115.20 / 73.66		
	2	T40S ROOF	10.00 / 0.00	12-10-00	06-11-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.44 / 97.34		
	1 / 3 Ply	T46 FLAT GIRDER	0.00 / 0.00	09-01-00	02-00-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-00-00 / 02-00-00	141.75 / 93.99		
	1 / 2 Ply	T102 COMMON	16.00 / 0.00	11-09-08	09-07-13	2 X 6	2 X 6	00-10-08 / 00-10-08	01-09-08 / 01-09-08	179.96 / 108.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

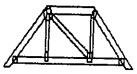
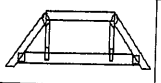
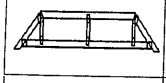
Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287688 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: B STD. +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB31 PIGGYBACK	10.00 0.00	07-06-14	02-02-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	25.93 17.33		
	2	PB32 PIGGYBACK	10.00 0.00	07-06-14	03-06-11	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	48.20 29.66		
	11	PB33 PIGGYBACK	10.00 0.00	06-02-14	03-00-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	221.65 141.13		
	1 3 Ply	PB33Z PIGGYBACK	10.00 0.00	06-02-14	03-00-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	60.45 38.49		
	1	PB34 PIGGYBACK	10.00 0.00	06-02-14	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	18.61 12.33		
	1 3 Ply	PB35 PIGGYBACK	10.00 0.00	09-11-14	01-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	83.82 52.50		
	11	J31 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	209.00 132.00		
	7	J31S JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-05-00 00-00-00	01-07-11 06-06-07	181.16 126.00		
	2	C13 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	16.74 12.00		
	2	C14 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 01-10-15	01-07-11 00-05-13	18.84 13.34		
	2	C15 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	25.86 17.34		
	2	C16 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	31.80 20.00		
	16	J32 JACK	10.00 0.00	02-04-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-04-13 02-04-02	139.52 96.00		
	9	J33 JACK-OPEN	10.00 0.00	01-11-12	02-00-10	2 X 4	2 X 4	01-03-08 00-00-00	00-04-13 02-00-10	66.51 42.03		

TOTAL # TRUSS= **120.00** TOTAL BFT OF ALL TRUSSES= **4922.36** BFT. TOTAL WEIGHT OF ALL TRUSSES= **7906.87** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
1	Hangers	HGUS26-3	
3	Hangers	HGUS26-2	

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/04/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287688 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-8C **ELEVATION:** B STD. +OPT. COFF.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LUS24	
13	Hangers	LJS26DS	

TOTAL # ITEMS= **28.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

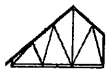
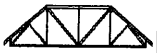
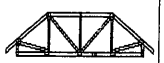
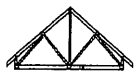
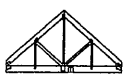
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296634 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-8C **ELEVATION:** C STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T51 HIP GIRDER	6.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	316.08 198.68		
	1	T52 HIP	6.00 0.00	31-00-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	123.85 78.50		
	1	T53 HIP	6.00 0.00	31-00-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	126.50 80.00		
	1 3 Ply	T54 HIP GIRDER	6.00 0.00	31-00-00	07-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	588.15 368.01		
	1	T55A HIP	6.00 0.00	23-08-00	08-02-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-10-00	109.99 68.83		
	2	T56A COMMON	6.00 0.00	23-08-00	08-11-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-10-00	205.06 130.00		
	1 3 Ply	T57 COMMON	6.00 0.00	29-08-00	08-07-00	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	604.92 372.00		
	4	T58 COMMON	6.00 0.00	29-08-00	08-07-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	482.72 302.68		
	7	T58A COMMON	6.00 0.00	23-08-00	08-07-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-02-00	723.38 457.31		
	1 2 Ply	T65 HIP GIRDER	6.00 0.00	29-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	325.14 208.00		
	1	T66 HIP	6.00 0.00	29-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	114.09 71.83		
	1	T67 HIP	6.00 0.00	29-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	122.73 77.34		
	1	T68 HIP	6.00 0.00	29-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	119.83 73.83		
	1	T69 HIP	6.00 0.00	29-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	130.18 81.67		
	1	T70 HIP GIRDER	6.00 0.00	16-01-00	03-01-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	62.73 41.50		
	1	T71 HIP	6.00 0.00	16-01-00	04-05-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	66.47 42.50		
	1	T72 COMMON	6.00 0.00	16-01-00	05-02-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	62.90 39.83		
	1 3 Ply	T60X COMMON	6.00 0.00	21-03-00	06-05-00	2 X 8	2 X 8	00-00-00 00-00-00	01-02-00 01-00-08	400.68 252.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

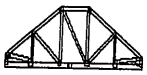
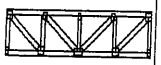
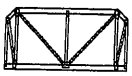
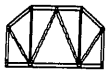
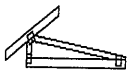
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296634 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: C STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T61X HIP GIRDER	6.00 0.00	21-03-00	05-03-13	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-00-08	218.28 134.66		
	1	T62 FLAT	0.00 0.00	13-02-00	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	02-04-08 02-04-08	55.90 36.00		
	1	T63 HIP	6.00 0.00	13-02-00	03-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-01-08 03-01-08	58.68 38.33		
	1	T64 HIP	6.00 0.00	13-02-00	04-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-01-08 03-01-08	58.87 38.33		
	18	J55 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	302.22 192.06		
	2	J51 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 04-01-01	01-02-00 00-03-08	32.32 20.66		
	2	J52 JACK-OPEN	6.00 0.00	03-09-07	03-00-12	2 X 4	2 X 4	01-03-08 02-01-01	01-02-00 00-03-08	37.42 23.34		
	2	J53 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 00-01-01	01-02-00 00-03-08	16.86 11.34		
	2	J54 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	19.14 12.00		
	4	J56 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	71.32 42.68		
	4	J57 JACK-OPEN	8.00 0.00	02-07-08	03-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	38.68 24.00		
	16	J58 JACK	9.00 0.00	02-04-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-07-02 02-04-02	146.72 101.28		
	6	J59 JACK	7.00 0.00	01-11-12	01-06-03	2 X 4	2 X 4	01-03-08 00-00-00	00-04-05 01-06-03	44.04 28.02		

TOTAL # TRUSS= **97.00** TOTAL BFT OF ALL TRUSSES= **3647.21** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5785.85** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
2	Hangers	LUS24-2	
6	Hangers	HGUS26-2	
22	Hangers	LJS26DS	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **37.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

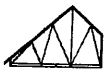
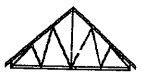
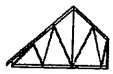
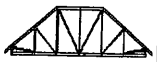
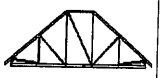
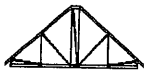
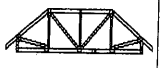
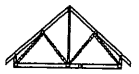
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287657 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: C STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T51S HIP GIRDER	6.00 / 10.28	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	320.54 / 208.00		
	1	T52S HIP	6.00 / 10.28	31-00-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	133.51 / 87.33		
	1	T53S HIP	6.00 / 10.28	31-00-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	132.92 / 85.67		
	1 / 3 Ply	T54 HIP GIRDER	6.00 / 0.00	31-00-00	07-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	588.15 / 368.01		
	1	T55A HIP	6.00 / 0.00	23-08-00	08-02-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-10-00	109.99 / 68.83		
	2	T56A COMMON	6.00 / 0.00	23-08-00	08-11-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-10-00	205.06 / 130.00		
	1 / 3 Ply	T57 COMMON	6.00 / 0.00	29-08-00	08-07-00	2 X 6	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	604.92 / 372.00		
	4	T58 COMMON	6.00 / 0.00	29-08-00	08-07-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	482.72 / 302.68		
	7	T58A COMMON	6.00 / 0.00	23-08-00	08-07-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-02-00	723.38 / 457.31		
	1 / 2 Ply	T65 HIP GIRDER	6.00 / 0.00	29-08-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	325.14 / 208.00		
	1	T66 HIP	6.00 / 0.00	29-08-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	114.09 / 71.83		
	1	T67 HIP	6.00 / 0.00	29-08-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	122.73 / 77.34		
	1	T68 HIP	6.00 / 0.00	29-08-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	119.83 / 73.83		
	1	T69 HIP	6.00 / 0.00	29-08-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	125.55 / 78.67		
	1	T70 HIP GIRDER	6.00 / 0.00	16-01-00	03-01-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	62.73 / 41.50		
	1	T71 HIP	6.00 / 0.00	16-01-00	04-05-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	66.47 / 42.50		
	1	T72 COMMON	6.00 / 0.00	16-01-00	05-02-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	62.90 / 39.83		
	1 / 3 Ply	T60X COMMON	6.00 / 0.00	21-03-00	06-05-00	2 X 8	2 X 8	00-00-00 / 00-00-00	01-02-00 / 01-00-08	400.68 / 252.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

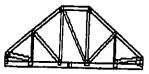
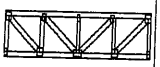
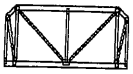
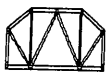
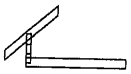
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287657 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-8C ELEVATION: C STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T61X HIP GIRDER	6.00 0.00	21-03-00	05-03-13	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-00-08	218.28 134.66		
	1	T62 FLAT	0.00 0.00	13-02-00	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	02-04-08 02-04-08	55.90 36.00		
	1	T63 HIP	6.00 0.00	13-02-00	03-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-01-08 03-01-08	58.68 38.33		
	1	T64 HIP	6.00 0.00	13-02-00	04-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-01-08 03-01-08	58.87 38.33		
	12	J55 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		
	8	J55S JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	170.64 125.36		
	1	J51 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	11.58 7.33		
	1	J52 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	14.13 8.67		
	1	J53 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	7.02 4.67		
	1	J54 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	9.57 6.00		
	4	J56 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	71.32 42.68		
	4	J57 JACK-OPEN	8.00 0.00	02-07-08	03-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	38.68 24.00		
	16	J58 JACK	9.00 0.00	02-04-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-07-02 02-04-02	146.72 101.28		
	6	J59 JACK	7.00 0.00	01-11-12	01-06-03	2 X 4	2 X 4	01-03-08 00-00-00	00-04-05 01-06-03	44.04 28.02		

TOTAL # TRUSS= **95.00** TOTAL BFT OF ALL TRUSSES= **3688.70** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5808.22** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Crush Plates	CP3-6	
2	Hangers	THGQH3-SDS4.5	
2	Hangers	LUS24-2	
6	Hangers	HGUS26-2	

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287657 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-8C **ELEVATION:** C STD+OPT. COFF.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
22	Hangers	LJS26DS	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **37.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296632	T1	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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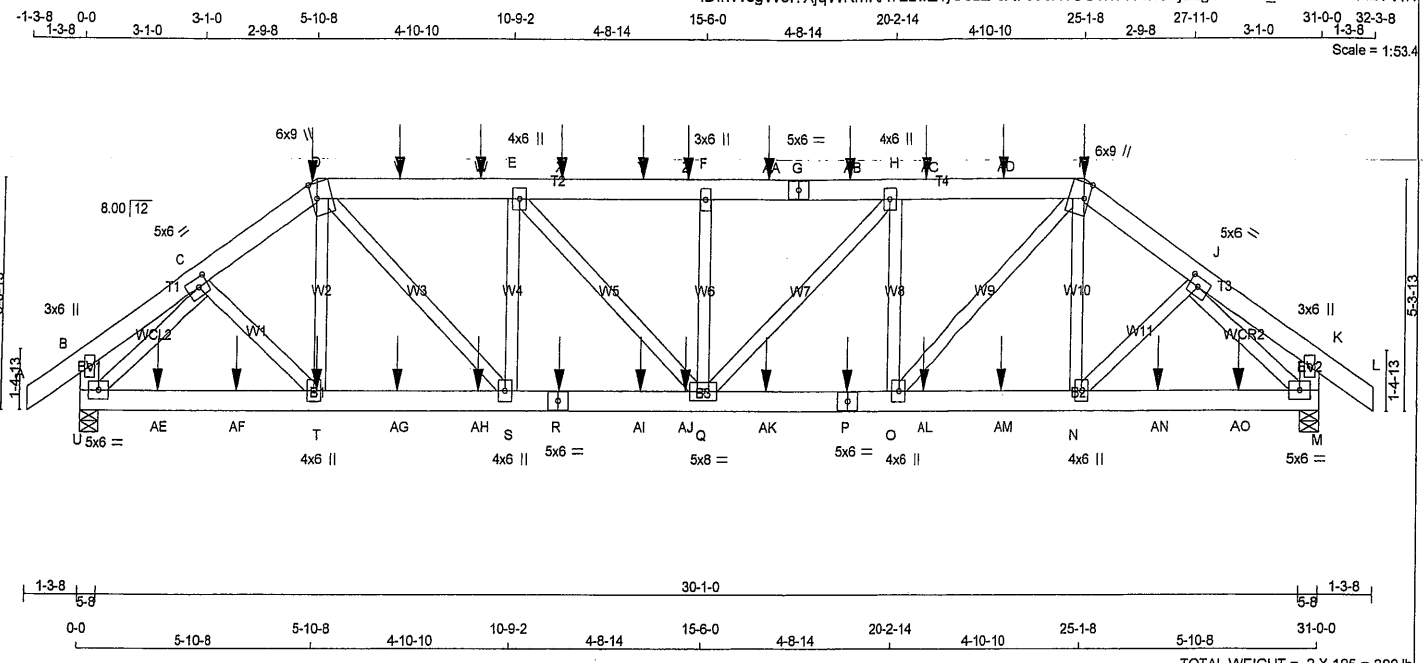


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
D- G	2	12	SIDE(61.0)
G- I	2	12	SIDE(61.0)
I- L	2	12	SIDE(122.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(0.0)
P- M	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	6.0	9.0	4.25	1.50
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	4.25	1.50
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	3.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		
N, O, S, T						
N	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3899	0	3899	0	0	5-8	5-8
M	3869	0	3869	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3095	1814 / 0	564 / 0	0 / 0	0 / 0	717 / 0	0 / 0
M	3059	1812 / 0	544 / 0	0 / 0	0 / 0	703 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

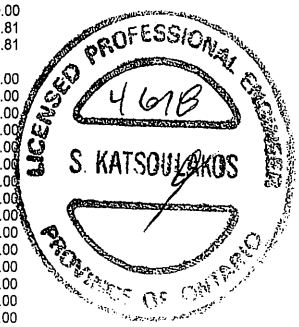
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-13 / 0	-112.4	-112.4	0.05 (1)	6.25
C- D	-4833 / 0	-112.4	-112.4	0.09 (1)	5.21
D- V	-5722 / 0	-112.4	-112.4	0.20 (1)	4.76
V- W	-5722 / 0	-112.4	-112.4	0.20 (1)	4.76
W- E	-5722 / 0	-112.4	-112.4	0.20 (1)	4.76
E- X	-6236 / 0	-112.4	-112.4	0.21 (1)	4.59
X- Y	-6236 / 0	-112.4	-112.4	0.21 (1)	4.59
Y- Z	-6236 / 0	-112.4	-112.4	0.21 (1)	4.59
Z- F	-6236 / 0	-112.4	-112.4	0.21 (1)	4.59
F-AA	-6236 / 0	-112.4	-112.4	0.20 (1)	4.60
AA- G	-6236 / 0	-112.4	-112.4	0.20 (1)	4.60
G-AB	-6236 / 0	-112.4	-112.4	0.20 (1)	4.60
AB- H	-6236 / 0	-112.4	-112.4	0.20 (1)	4.60
H-AC	-5692 / 0	-112.4	-112.4	0.20 (1)	4.77
AC-AD	-5692 / 0	-112.4	-112.4	0.20 (1)	4.77
AD- I	-5692 / 0	-112.4	-112.4	0.20 (1)	4.77
I- J	-4790 / 0	-112.4	-112.4	0.09 (1)	5.23
J- K	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00
U- B	-341 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-341 / 0	0.0	0.0	0.01 (1)	7.81
U-AE	0 / 3533	-28.0	-28.0	0.29 (1)	10.00
AE-AF	0 / 3533	-28.0	-28.0	0.29 (1)	10.00
AF- T	0 / 3533	-28.0	-28.0	0.29 (1)	10.00
T-AG	0 / 4011	-28.0	-28.0	0.31 (1)	10.00
AG-AH	0 / 4011	-28.0	-28.0	0.31 (1)	10.00
AH- S	0 / 4011	-28.0	-28.0	0.31 (1)	10.00
S- R	0 / 5723	-28.0	-28.0	0.42 (1)	10.00
R-AI	0 / 5723	-28.0	-28.0	0.42 (1)	10.00
AI-AJ	0 / 5723	-28.0	-28.0	0.42 (1)	10.00
AJ- Q	0 / 5723	-28.0	-28.0	0.42 (1)	10.00
Q-AK	0 / 5692	-28.0	-28.0	0.41 (1)	10.00
AK- P	0 / 5692	-28.0	-28.0	0.41 (1)	10.00
P- O	0 / 5692	-28.0	-28.0	0.41 (1)	10.00
O-AL	0 / 3974	-28.0	-28.0	0.30 (1)	10.00
AL-AM	0 / 3974	-28.0	-28.0	0.30 (1)	10.00
AM- N	0 / 3974	-28.0	-28.0	0.30 (1)	10.00
N-AN	0 / 3503	-28.0	-28.0	0.29 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- T	0 / 653	0.06 (1)
T- D	-128 / 292	0.03 (3)
D- S	0 / 2408	0.21 (1)
S- E	-1479 / 0	0.20 (1)
E- Q	0 / 734	0.06 (1)
Q- F	-844 / 0	0.11 (1)
Q- H	0 / 778	0.07 (1)
O- H	-1492 / 0	0.20 (1)
O- I	0 / 2418	0.21 (1)
N- I	-162 / 233	0.02 (1)
N- J	0 / 645	0.06 (1)
U- C	-4933 / 0	0.47 (1)
J- M	-4891 / 0	0.46 (1)



[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.42/1.00 (Q-S:1) , WB=0.47/1.00 (C-U:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.63 (C) (INPUT = 1.00)

DWG NO. TAM17777-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 634.1 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 135.3 lbs FACTORED DOWN AT 15-0-12, 135.3 lbs FACTORED DOWN AT 17-0-12, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, AND 135.3 lbs FACTORED DOWN AT 23-0-12, AND 634.1 lbs FACTORED DOWN AT 25-1-8 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, 71.7 lbs FACTORED DOWN AT 11-11-4, 71.7 lbs FACTORED DOWN AT 13-11-4, 71.7 lbs FACTORED DOWN AT 15-0-12, 71.7 lbs FACTORED DOWN AT 17-0-12, 71.7 lbs FACTORED DOWN AT 19-0-12, 71.7 lbs FACTORED DOWN AT 21-0-12, 71.7 lbs FACTORED DOWN AT 23-0-12, AND 87.4 lbs FACTORED DOWN AT 27-0-12, AND 87.4 lbs FACTORED DOWN AT 29-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			LENGTH FR-TO			
AN-AO	0 / 3503	-28.0 -28.0	0.29	(1)	10.00			
AO- M	0 / 3503	-28.0 -28.0	0.29	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-634	-634	—	FRONT	VERT	TOTAL
I	25-1-8	-634	-634	—	FRONT	VERT	TOTAL
P	19-0-12	-41	-72	—	FRONT	VERT	TOTAL
R	11-11-4	-41	-72	—	FRONT	VERT	TOTAL
T	5-11-4	-41	-72	—	FRONT	VERT	TOTAL
V	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
W	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
X	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
Y	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	15-0-12	-135	-135	—	FRONT	VERT	TOTAL
AA	17-0-12	-135	-135	—	FRONT	VERT	TOTAL
AB	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
AC	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
AD	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AE	1-11-4	-50	-87	—	FRONT	VERT	TOTAL
AF	3-11-4	-50	-87	—	FRONT	VERT	TOTAL
AG	7-11-4	-41	-72	—	FRONT	VERT	TOTAL
AH	9-11-4	-41	-72	—	FRONT	VERT	TOTAL
AI	13-11-4	-41	-72	—	FRONT	VERT	TOTAL
AJ	15-0-12	-41	-72	—	FRONT	VERT	TOTAL
AK	17-0-12	-41	-72	—	FRONT	VERT	TOTAL
AL	21-0-12	-41	-72	—	FRONT	VERT	TOTAL
AM	23-0-12	-41	-72	—	FRONT	VERT	TOTAL
AN	27-0-12	-50	-87	—	FRONT	VERT	TOTAL
AO	29-0-12	-50	-87	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17777-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T1S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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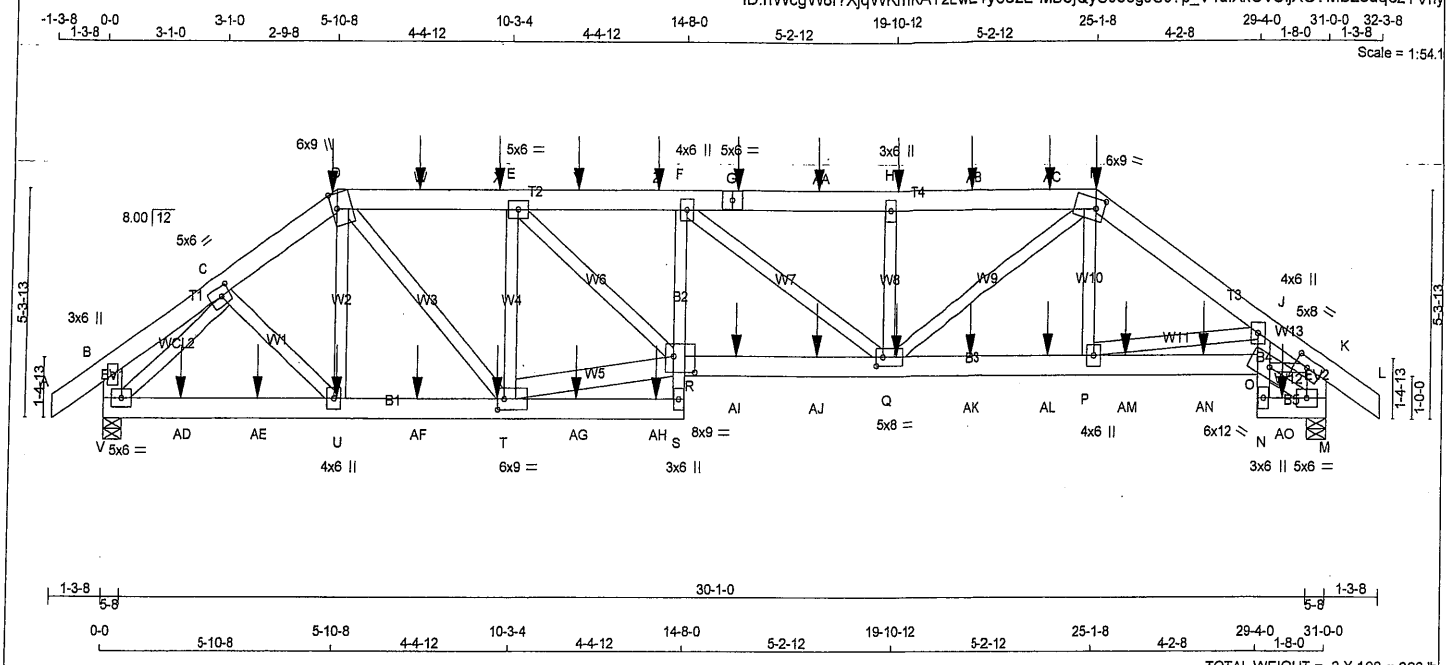


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
D- G	2	12	SIDE(183.1)
G- I	2	12	SIDE(183.1)
I- L	2	12	SIDE(122.0)
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
R- O	2	12	SIDE(183.1)
N- M	2	12	SIDE(0.0)
S- F	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	6.0	9.0	4.25	1.50
E	TMWW-t	MT20	5.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMV+w	MT20	3.0	6.0		
I	TTWW-m	MT20	6.0	9.0	2.75	2.50
J	TMVW+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
V	3895	0	3895	0	0	5-8	5-8
M	4048	0	4048	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3114	1789 / 0	593 / 0	0 / 0	0 / 0	732 / 0	0 / 0
M	3263	1829 / 0	652 / 0	0 / 0	0 / 0	783 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

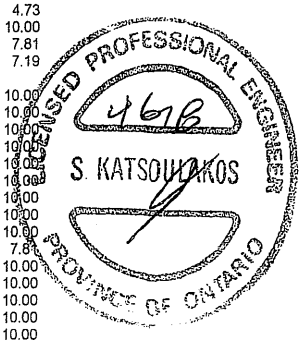
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-13 / 0	-112.4	-112.4	0.05 (1)	6.25
C- D	-4836 / 0	-112.4	-112.4	0.09 (1)	5.21
D- W	-5639 / 0	-112.4	-112.4	0.17 (1)	4.82
W- X	-5639 / 0	-112.4	-112.4	0.17 (1)	4.82
X- E	-5639 / 0	-112.4	-112.4	0.17 (1)	4.82
E- Y	-8017 / 0	-112.4	-112.4	0.21 (1)	4.14
Y- Z	-8017 / 0	-112.4	-112.4	0.21 (1)	4.14
Z- F	-8017 / 0	-112.4	-112.4	0.21 (1)	4.14
F- G	-7646 / 0	-112.4	-112.4	0.25 (1)	4.18
G-AA	-7646 / 0	-112.4	-112.4	0.25 (1)	4.18
AA- H	-7646 / 0	-112.4	-112.4	0.25 (1)	4.18
H-AB	-7646 / 0	-112.4	-112.4	0.26 (1)	4.16
AB-AC	-7646 / 0	-112.4	-112.4	0.26 (1)	4.16
AC- I	-7646 / 0	-112.4	-112.4	0.26 (1)	4.16
I- J	-6190 / 0	-112.4	-112.4	0.20 (1)	4.61
J- K	-6068 / 0	-112.4	-112.4	0.11 (1)	4.73
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00
V- B	-341 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-3877 / 0	0.0	0.0	0.14 (1)	7.19
V-AD	0 / 3535	-28.0	-28.0	0.29 (1)	10.00
AD-AE	0 / 3535	-28.0	-28.0	0.29 (1)	10.00
AE- U	0 / 3535	-28.0	-28.0	0.29 (1)	10.00
U-AF	0 / 4013	-28.0	-28.0	0.30 (1)	10.00
AF- T	0 / 4013	-28.0	-28.0	0.30 (1)	10.00
T-AG	0 / 134	-28.0	-28.0	0.06 (2)	10.00
AG-AH	0 / 134	-28.0	-28.0	0.06 (2)	10.00
AH- S	0 / 134	-28.0	-28.0	0.06 (2)	10.00
S- R	0 / 184	0.0	0.0	0.11 (1)	10.00
R- F	-475 / 0	0.0	0.0	0.11 (1)	7.81
R-AI	0 / 8053	-28.0	-28.0	0.61 (1)	10.00
AI-AJ	0 / 8053	-28.0	-28.0	0.61 (1)	10.00
AJ- Q	0 / 8053	-28.0	-28.0	0.61 (1)	10.00
Q-AK	0 / 5182	-28.0	-28.0	0.41 (1)	10.00
AK-AL	0 / 5182	-28.0	-28.0	0.41 (1)	10.00
AL- P	0 / 5182	-28.0	-28.0	0.41 (1)	10.00
P-AM	0 / 5157	-28.0	-28.0	0.41 (1)	10.00
AM-AN	0 / 5157	-28.0	-28.0	0.41 (1)	10.00
AN- O	0 / 5157	-28.0	-28.0	0.41 (1)	10.00
N- O	0 / 97	0.0	0.0	0.16 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- U	0 / 654	0.06 (1)
U- D	-165 / 225	0.02 (1)
D- T	0 / 2449	0.22 (1)
T- E	-2930 / 0	0.40 (1)
T- R	0 / 5715	0.37 (1)
E- R	0 / 3126	0.28 (1)
F- Q	-506 / 0	0.13 (1)
Q- H	-1030 / 0	0.09 (1)
Q- I	0 / 3027	0.27 (1)
P- I	0 / 657	0.06 (2)
P- J	0 / 8	0.00 (3)
V- C	-4936 / 0	0.47 (1)
O- M	-275 / 0	0.02 (1)
O- K	0 / 4959	0.61 (1)



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.26/1.00 (H-I:1) , BC=0.61/1.00 (Q-R:1) , WB=0.61/1.00 (K-O:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17297-18
STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T1S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TMVW-t	MT20	5.0	8.0	2.50	3.75
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW+w	MT20	6.0	12.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	2.00
R	BVMWW-l	MT20	8.0	9.0	4.50	6.50
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	9.0	3.00	2.00
U	BMWW+t	MT20	4.0	6.0		
V	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 372.5 lbs FACTORED DOWN AT 25-1-8, 563.4 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 128.6 lbs FACTORED DOWN AT 15-11-4, 128.6 lbs FACTORED DOWN AT 17-11-4, 128.6 lbs FACTORED DOWN AT 19-11-4, AND 128.6 lbs FACTORED DOWN AT 21-11-4, AND 128.6 lbs FACTORED DOWN AT 23-11-4 ON TOP CHORD, AND 75.9 lbs FACTORED DOWN AT 1-11-4, 71.7 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, 71.7 lbs FACTORED DOWN AT 11-11-4, 82.0 lbs FACTORED DOWN AT 13-11-4, 115.8 lbs FACTORED DOWN AT 15-11-4, 115.8 lbs FACTORED DOWN AT 17-11-4, 115.8 lbs FACTORED DOWN AT 19-11-4, 115.8 lbs FACTORED DOWN AT 21-11-4, 115.8 lbs FACTORED DOWN AT 23-11-4, 115.8 lbs FACTORED DOWN AT 25-11-4, AND 115.8 lbs FACTORED DOWN AT 27-11-4, AND 85.0 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- J	-251 / 26	0.0	0.0	0.15 (1)		7.81		
N-AO	0 / 240	-28.0	-28.0	0.03 (2)		10.00		
AO- M	0 / 240	-28.0	-28.0	0.03 (2)		10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-563	-563	—	FRONT	VERT	TOTAL
G	15-11-4	-129	-129	—	FRONT	VERT	TOTAL
H	19-11-4	-129	-129	—	FRONT	VERT	TOTAL
I	25-1-8	-373	-373	—	FRONT	VERT	TOTAL
Q	19-11-4	-116	-116	—	FRONT	VERT	TOTAL
T	9-11-4	-41	-72	—	FRONT	VERT	TOTAL
U	5-11-4	-41	-72	—	FRONT	VERT	TOTAL
W	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
X	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
Y	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
AA	17-11-4	-129	-129	—	FRONT	VERT	TOTAL
AB	21-11-4	-129	-129	—	FRONT	VERT	TOTAL
AC	23-11-4	-129	-129	—	FRONT	VERT	TOTAL
AD	1-11-4	-39	-76	—	FRONT	VERT	TOTAL
AE	3-11-4	-41	-72	—	FRONT	VERT	TOTAL
AF	7-11-4	-41	-72	—	FRONT	VERT	TOTAL
AG	11-11-4	-41	-72	—	FRONT	VERT	TOTAL
AH	13-11-4	-47	-82	—	FRONT	VERT	TOTAL
AI	15-11-4	-116	-116	—	FRONT	VERT	TOTAL
AJ	17-11-4	-116	-116	—	FRONT	VERT	TOTAL
AK	21-11-4	-116	-116	—	FRONT	VERT	TOTAL
AL	23-11-4	-116	-116	—	FRONT	VERT	TOTAL
AM	25-11-4	-116	-116	—	FRONT	VERT	TOTAL
AN	27-11-4	-116	-116	—	FRONT	VERT	TOTAL
AO	29-11-4	-49	-85	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.63 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
461B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17749-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296632	T2	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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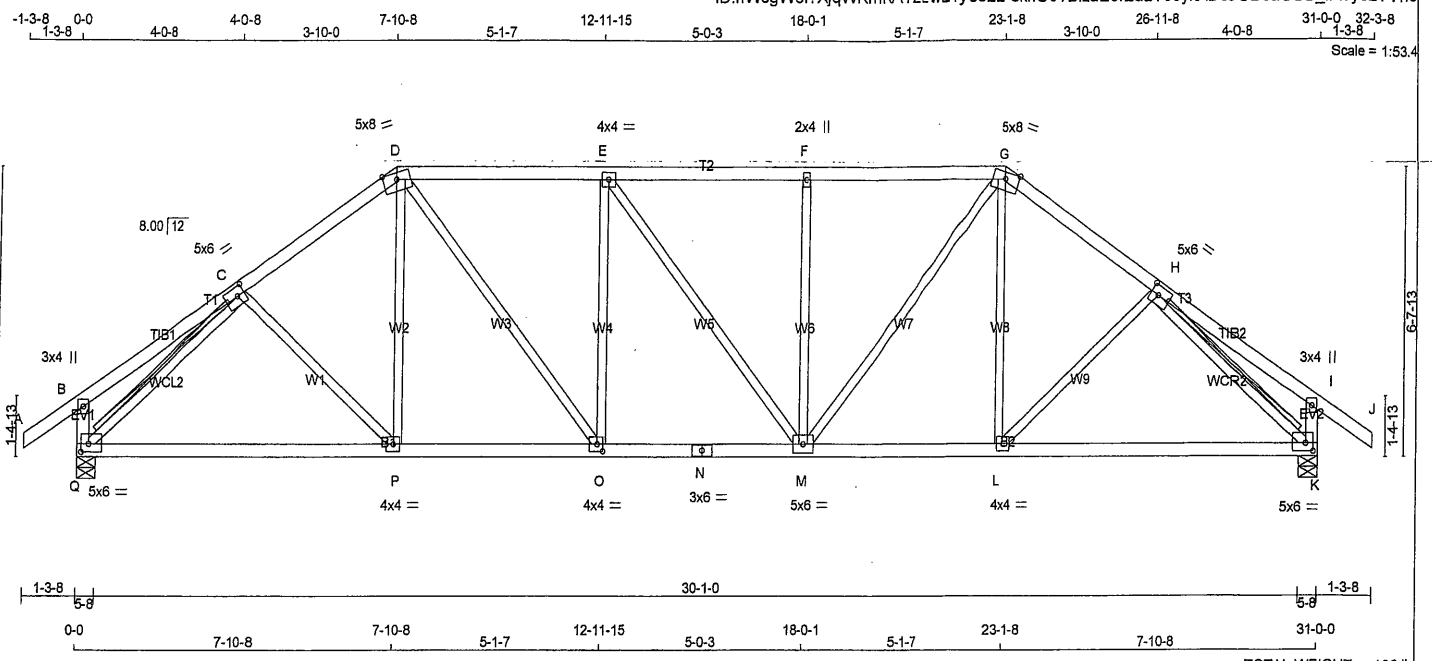


TOTAL WEIGHT = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TMWW-t	MT20	5.0	6.0	2.50	2.25
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.25	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2331	0	2331	0	0	5-8	5-8
K	2331	0	2331	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
K	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-Q, H-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

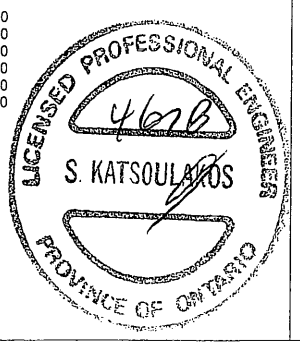
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
B- C	0 / 26	-112.4	-112.4	0.26 (1)	10.00	
C- D	-2479 / 0	-112.4	-112.4	0.28 (1)	4.15	
D- E	-2632 / 0	-112.4	-112.4	0.42 (1)	3.88	
E- F	-2630 / 0	-112.4	-112.4	0.39 (1)	3.91	
F- G	-2630 / 0	-112.4	-112.4	0.42 (1)	3.88	
G- H	-2479 / 0	-112.4	-112.4	0.28 (1)	4.14	
H- I	0 / 26	-112.4	-112.4	0.26 (1)	10.00	
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
Q- B	-328 / 0	0.0	0.0	0.03 (1)	7.81	
K- I	-328 / 0	0.0	0.0	0.03 (1)	7.81	
Q- P	0 / 2029	-28.0	-28.0	0.59 (2)	10.00	
P- O	0 / 2041	-28.0	-28.0	0.61 (2)	10.00	
O- N	0 / 2632	-28.0	-28.0	0.50 (1)	10.00	
N- M	0 / 2632	-28.0	-28.0	0.50 (1)	10.00	
M- L	0 / 2041	-28.0	-28.0	0.61 (2)	10.00	
L- K	0 / 2029	-28.0	-28.0	0.59 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- P	0 / 141	0.03 (3)
P- D	0 / 315	0.07 (3)
D- O	0 / 942	0.21 (1)
O- E	-615 / 0	0.45 (1)
E- M	-3 / 0	0.00 (1)
M- F	-614 / 0	0.45 (1)
M- G	0 / 938	0.21 (1)
L- G	0 / 316	0.07 (3)
L- H	0 / 141	0.03 (3)
Q- C	-2787 / 0	0.66 (1)
H- K	-2787 / 0	0.66 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.42/1.00 (F-G:1) , BC=0.61/1.00 (L-M:2) , WB=0.66/1.00 (H-K:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.79 (N) (INPUT = 1.00)

DWG NO. TAM 17778-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T2S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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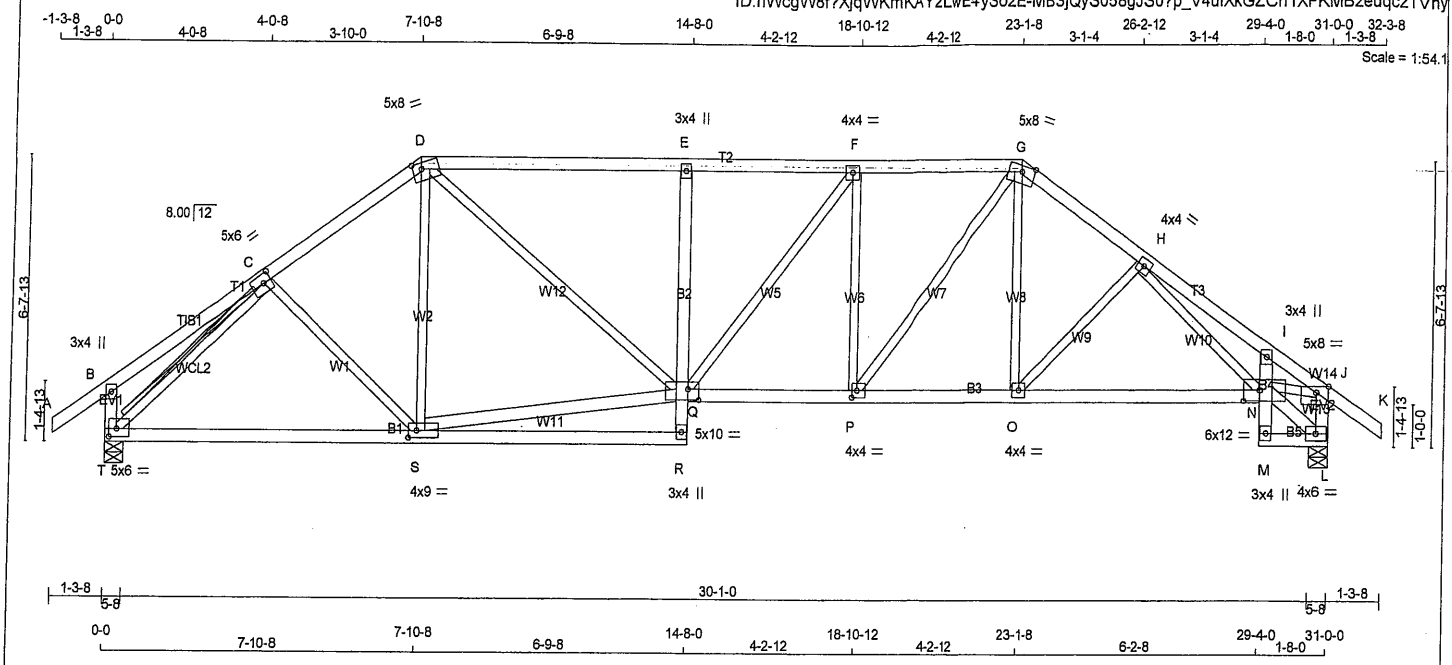


TOTAL WEIGHT = 145 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, E, I						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	8.0	1.75	2.75
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	4.00
H	TMWW-t	MT20	4.0	4.0		
J	TMVW-p	MT20	5.0	8.0	1.75	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	6.0	12.0	3.00	4.75
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BVMWWW-I	MT20	5.0	10.0	3.00	3.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	4.0	9.0	2.00	2.50
T	BMVW1-t	MT20	5.0	6.0	2.25	2.25

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2328	0	2328	0	0	5-8	5-8
L	2334	0	2334	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1839	1094 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
L	1843	1098 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

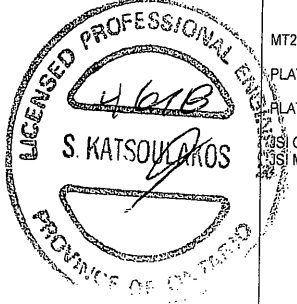
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 26	-112.4	-112.4	0.26 (1)	10.00
C- D	-2481 / 0	-112.4	-112.4	0.28 (1)	4.14
D- E	-3192 / 0	-112.4	-112.4	0.80 (1)	3.04
E- F	-3205 / 0	-112.4	-112.4	0.58 (1)	3.31
F- G	-3024 / 0	-112.4	-112.4	0.37 (1)	3.70
G- H	-2900 / 0	-112.4	-112.4	0.24 (1)	3.91
H- I	-3247 / 0	-112.4	-112.4	0.27 (1)	3.69
I- J	-3209 / 0	-112.4	-112.4	0.22 (1)	3.75
J- K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
T- B	-327 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-2230 / 0	0.0	0.0	0.23 (1)	5.70
T- S	0 / 2031	-28.0	-28.0	0.66 (2)	10.00
S- R	0 / 71	-28.0	-28.0	0.48 (3)	10.00
R- Q	0 / 113	0.0	0.0	0.11 (1)	10.00
Q- E	-738 / 0	0.0	0.0	0.33 (1)	7.81
Q- P	0 / 3024	-28.0	-28.0	0.56 (1)	10.00
P- O	0 / 2397	-28.0	-28.0	0.52 (1)	10.00
O- N	0 / 2689	-28.0	-28.0	0.57 (1)	10.00
M- N	0 / 37	0.0	0.0	0.18 (1)	10.00
N- I	-156 / 0	0.0	0.0	0.13 (1)	7.81
M- L	0 / 127	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 143	0.03 (3)
S- D	-52 / 207	0.05 (3)
Q- F	0 / 290	0.07 (1)
P- F	-665 / 0	0.32 (1)
P- G	0 / 995	0.22 (1)
O- G	0 / 470	0.11 (1)
O- H	-417 / 0	0.15 (1)
H- N	0 / 110	0.03 (3)
T- C	-2789 / 0	0.66 (1)
S- Q	0 / 1990	0.32 (1)
D- Q	0 / 1480	0.33 (1)
N- L	-152 / 0	0.02 (1)
N- J	0 / 2623	0.59 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.66/1.00 (S-T:2) , WB=0.66/1.00 (C-T:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

TPI GRIP= 0.90 (N) (INPUT = 0.90)
TPI METAL= 0.69 (C) (INPUT = 1.00)

DWG NO. TAM 177488
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296632	T3	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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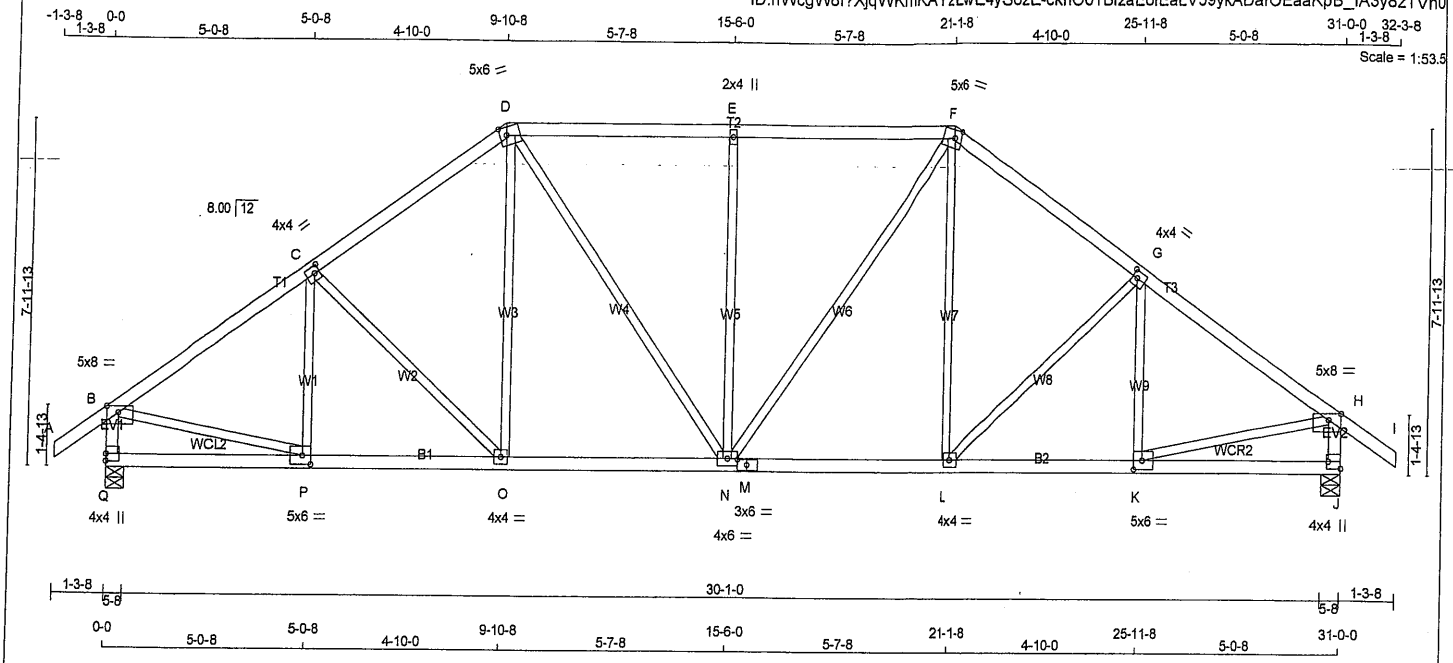


TOTAL WEIGHT = 139 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.75	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0	1.50	2.75
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2331	0	2331	0	0	5-8	5-8
J	2331	0	2331	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

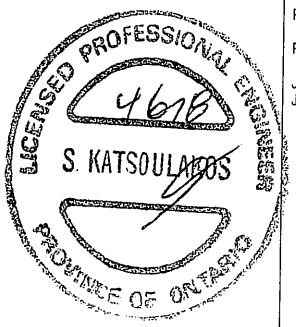
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2552 / 0	-112.4	-112.4	0.45 (1)	3.91
C- D	-2338 / 0	-112.4	-112.4	0.43 (1)	4.08
D- E	-2257 / 0	-112.4	-112.4	0.53 (1)	3.97
E- F	-2257 / 0	-112.4	-112.4	0.53 (1)	3.97
F- G	-2338 / 0	-112.4	-112.4	0.43 (1)	4.08
G- H	-2552 / 0	-112.4	-112.4	0.45 (1)	3.91
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00
Q- B	-2270 / 0	0.0	0.0	0.23 (1)	5.65
J- H	-2270 / 0	0.0	0.0	0.23 (1)	5.65
Q- P	0 / 0	-28.0	-28.0	0.17 (3)	10.00
P- O	0 / 2153	-28.0	-28.0	0.43 (1)	10.00
O- N	0 / 1917	-28.0	-28.0	0.40 (1)	10.00
N- M	0 / 1917	-28.0	-28.0	0.40 (1)	10.00
M- L	0 / 1917	-28.0	-28.0	0.40 (1)	10.00
L- K	0 / 2153	-28.0	-28.0	0.43 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-315 / 60	0.11 (1)
C- O	-324 / 0	0.26 (1)
O- D	0 / 390	0.09 (2)
D- N	0 / 575	0.13 (1)
N- E	-772 / 0	0.94 (1)
N- F	0 / 575	0.13 (1)
L- F	0 / 390	0.09 (2)
L- G	-324 / 0	0.26 (1)
K- G	-315 / 60	0.11 (1)
B- P	0 / 2201	0.50 (1)
K- H	0 / 2201	0.50 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.43/1.00 (K-L:1) , WB=0.94/1.00 (E-N:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.54 (M) (INPUT = 1.00)

DWG NO. TAM 1777-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T3S	1	1	OBC2012_6TCDL TRUSS DESC.	

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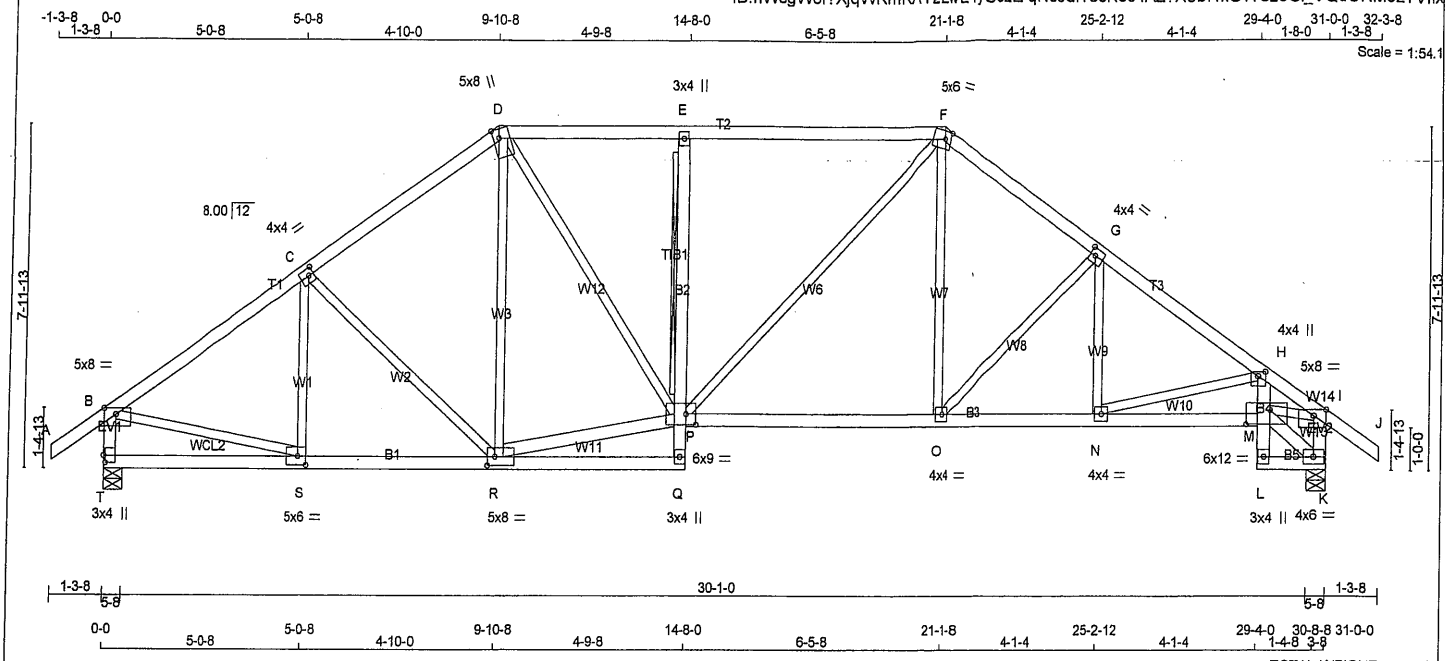


TOTAL WEIGHT = 148 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	8.0	Edge	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	TMVW-p	MT20	5.0	8.0	1.75	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-I	MT20	6.0	12.0	4.00	7.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-I	MT20	6.0	9.0	3.00	3.00
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.25
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	4.0	2.00	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2328	0	2328	0	0	5-8	5-8
K	2334	0	2334	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1839	1094 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
K	1843	1098 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

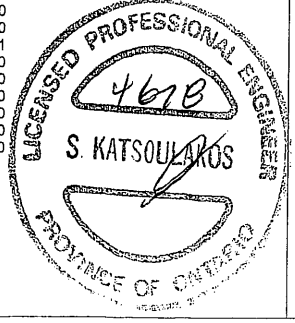
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
B- C	-2547 / 0	-112.4	-112.4	0.45 (1)	3.91	
C- D	-2337 / 0	-112.4	-112.4	0.43 (1)	4.08	
D- E	-2566 / 0	-112.4	-112.4	0.57 (1)	3.66	
E- F	-2572 / 0	-112.4	-112.4	0.69 (1)	3.52	
F- G	-2668 / 0	-112.4	-112.4	0.34 (1)	3.96	
G- H	-3145 / 0	-112.4	-112.4	0.38 (1)	3.64	
H- I	-3745 / 0	-112.4	-112.4	0.25 (1)	3.46	
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
T- B	-2266 / 0	0.0	0.0	0.23 (1)	5.66	
K- I	-2199 / 0	0.0	0.0	0.23 (1)	5.73	
T- S	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
S- R	0 / 2148	-28.0	-28.0	0.44 (1)	10.00	
R- Q	0 / 40	-28.0	-28.0	0.16 (3)	10.00	
Q- P	0 / 90	0.0	0.0	0.06 (1)	10.00	
P- E	-789 / 0	0.0	0.0	0.26 (1)	7.81	
P- O	0 / 2200	-28.0	-28.0	0.50 (2)	10.00	
O- N	0 / 2643	-28.0	-28.0	0.57 (1)	10.00	
N- M	0 / 3270	-28.0	-28.0	0.60 (1)	10.00	
L- M	0 / 37	0.0	0.0	0.24 (1)	10.00	
M- H	0 / 261	0.0	0.0	0.28 (1)	10.00	
L- K	0 / 149	-28.0	-28.0	0.04 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-317 / 57	0.11 (1)
C- R	-322 / 0	0.26 (1)
R- D	-37 / 153	0.04 (1)
P- F	0 / 530	0.12 (1)
O- F	0 / 610	0.14 (1)
O- G	-619 / 0	0.35 (1)
N- G	0 / 210	0.05 (3)
N- H	-645 / 0	0.21 (1)
B- S	0 / 2197	0.49 (1)
R- P	0 / 1915	0.31 (1)
D- P	0 / 1148	0.26 (1)
M- K	-188 / 0	0.02 (1)
M- I	0 / 3086	0.69 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.60/1.00 (M-N:1) , WB=0.69/1.00 (I-M:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

DWG NO. TAM 177498
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T4	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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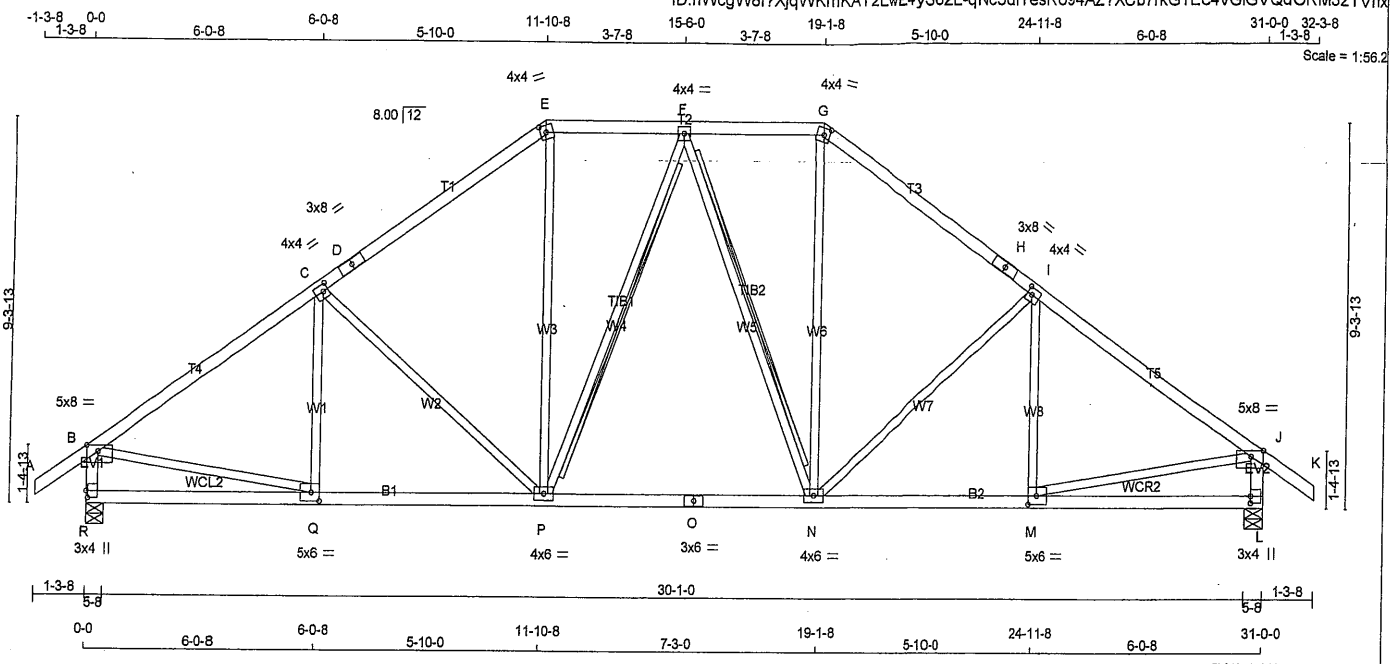

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0	2.00	1.75
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.75	3.50
L	BMV1+p	MT20	3.0	4.0	2.00	
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2331	0	2331	0	0	5-8	5-8
L	2331	0	2331	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
L	1841	1096 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-P, F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	-2578 / 0	-112.4	-112.4	0.68 (1)		3.61
C- D	-2182 / 0	-112.4	-112.4	0.61 (1)		3.94
D- E	-2182 / 0	-112.4	-112.4	0.61 (1)		3.94
E- F	-1784 / 0	-112.4	-112.4	0.20 (1)		4.82
F- G	-1784 / 0	-112.4	-112.4	0.20 (1)		4.82
G- H	-2182 / 0	-112.4	-112.4	0.61 (1)		3.94
H- I	-2182 / 0	-112.4	-112.4	0.61 (1)		3.94
I- J	-2578 / 0	-112.4	-112.4	0.68 (1)		3.61
J- K	0 / 43	-112.4	-112.4	0.15 (1)		10.00
R- B	-2260 / 0	0.0	0.0	0.23 (1)		5.66
L- J	-2260 / 0	0.0	0.0	0.23 (1)		5.66
R- Q	0 / 0	-28.0	-28.0	0.24 (3)		10.00
Q- P	0 / 2182	-28.0	-28.0	0.49 (1)		10.00
P- O	0 / 1878	-28.0	-28.0	0.44 (2)		10.00
O- N	0 / 1878	-28.0	-28.0	0.44 (2)		10.00
N- M	0 / 2182	-28.0	-28.0	0.49 (1)		10.00
M- L	0 / 0	-28.0	-28.0	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-213 / 143	0.09 (1)
C- P	-536 / 0	0.68 (1)
P- E	0 / 775	0.17 (1)
P- F	-258 / 0	0.20 (1)
F- N	-258 / 0	0.20 (1)
N- G	0 / 775	0.17 (1)
N- I	-536 / 0	0.68 (1)
M- I	-213 / 143	0.09 (1)
B- Q	0 / 2216	0.50 (1)
M- J	0 / 2216	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.68/1.00 (I-J:1) , BC=0.49/1.00 (M-N:1) , WB=0.68/1.00 (I-N:1) , SSI=0.27/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.63 (O) (INPUT = 1.00)

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4618
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PROVINCE OF ONTARIO

DWG NO. TAM 17750-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T5	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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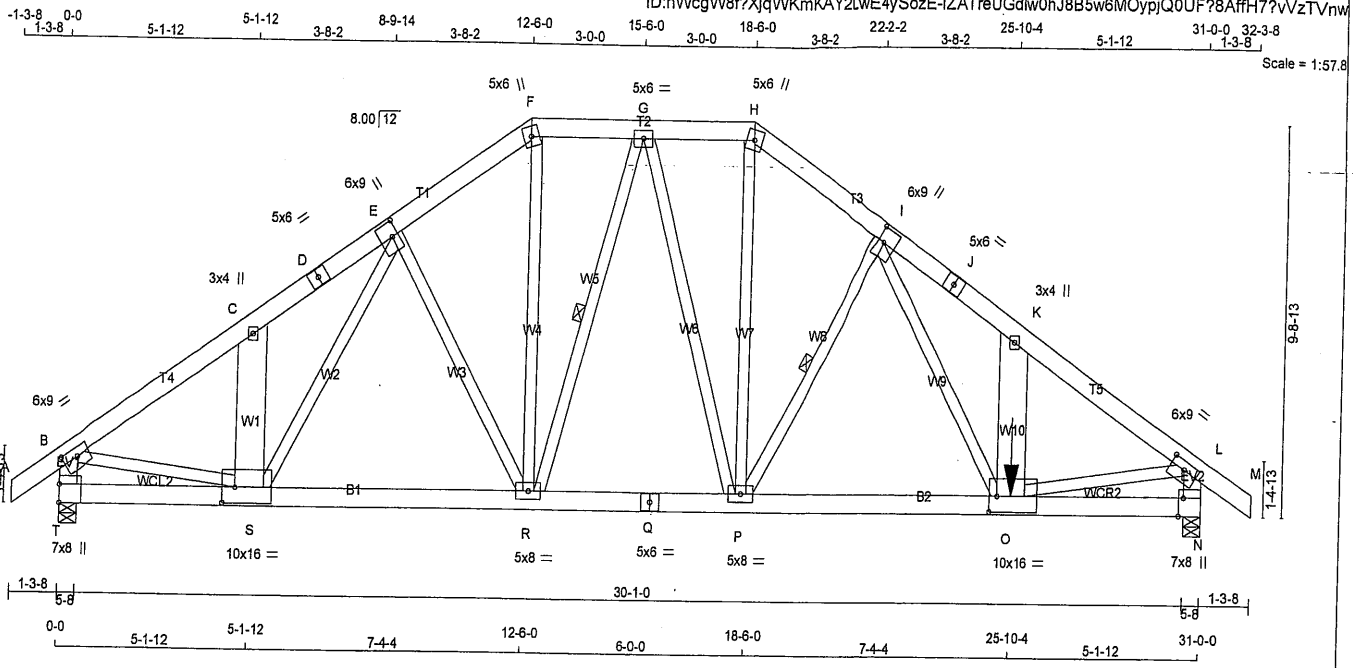

TOTAL WEIGHT = 3 X 238 = 713 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x10	DRY	No.2	SPF
O - K	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
J- M	2	12	TOP
T- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	3	5	TOP
Q- N	3	5	SIDE(1285.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
K- O	5	4	SIDE(2535.3)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	4.50
C	TMW+w	MT20	3.0	4.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	6.0	9.0	4.50	2.00
F	TTW+m	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTW+m	MT20	5.0	6.0		
I	TMWW+t	MT20	6.0	9.0	4.50	2.00
J	TS-t	MT20	5.0	6.0		
K	TMW+w	MT20	3.0	4.0		
L	TMVW-t	MT20	6.0	9.0	2.50	4.50
N	BMV1+p	MT20	7.0	8.0	Edge	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4397	0	4397	0	0	5-8	5-8
N	12493	0	12493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	3472	2068 / 0	613 / 0	0 / 0	0 / 0	791 / 0	0 / 0
N	9865	5874 / 0	1741 / 0	0 / 0	0 / 0	2250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.03 (1)	10.00
B- C	-5584 / 0	-112.4	-112.4	0.08 (1)	5.79
C- D	-5580 / 0	-112.4	-112.4	0.05 (1)	5.83
D- E	-5580 / 0	-112.4	-112.4	0.05 (1)	5.83
E- F	-5488 / 0	-112.4	-112.4	0.05 (1)	5.87
F- G	-4586 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-5881 / 0	-112.4	-112.4	0.07 (1)	5.70
H- I	-7068 / 0	-112.4	-112.4	0.10 (1)	5.28
I- J	-16724 / 0	-112.4	-112.4	0.27 (1)	3.56
J- K	-16724 / 0	-112.4	-112.4	0.27 (1)	3.56
K- L	-16906 / 0	-112.4	-112.4	0.33 (1)	3.50
L- M	0 / 44	-112.4	-112.4	0.03 (1)	10.00
T- B	-4318 / 0	0.0	0.0	0.09 (1)	7.81
N- L	-12289 / 0	0.0	0.0	0.26 (1)	5.32
T- S	0 / 0	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 4698	-28.0	-28.0	0.09 (1)	10.00
R- Q	0 / 5255	-28.0	-28.0	0.12 (1)	10.00
Q- P	0 / 5255	-28.0	-28.0	0.12 (1)	10.00
P- O	0 / 8836	-28.0	-28.0	0.22 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-524 / 0	0.02 (1)
S- E	-139 / 37	0.04 (1)
E- R	-306 / 0	0.09 (1)
R- F	0 / 2607	0.14 (1)
R- G	-2278 / 0	0.35 (1)
G- P	0 / 2124	0.11 (1)
P- H	0 / 3526	0.19 (1)
P- I	-6007 / 0	0.56 (1)
I- O	0 / 11174	0.60 (1)
O- K	-177 / 93	0.01 (1)
B- S	0 / 4727	0.25 (1)
O- L	0 / 14227	0.76 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR TYPE
O	25-10-4	-12218	-12218	---	BACK	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCRC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.33/1.00 (K-L:1) , BC=0.22/1.00 (O-P:1) , WB=0.76/1.00 (L-O:1) , SSI=0.07/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.95 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
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PROVINCE OF ONTARIO

DWG NO. TAM 1775/-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T5	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWWW-t	MT20	10.0	16.0	4.75	2.75
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWWW-t	MT20	10.0	16.0	4.75	4.25
T	BMV1+p	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 12217.8 lbs FACTORED DOWN AT 25-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

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4618
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PROVINCE OF ONTARIO

DWG NO. TAM 17251-R8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T6	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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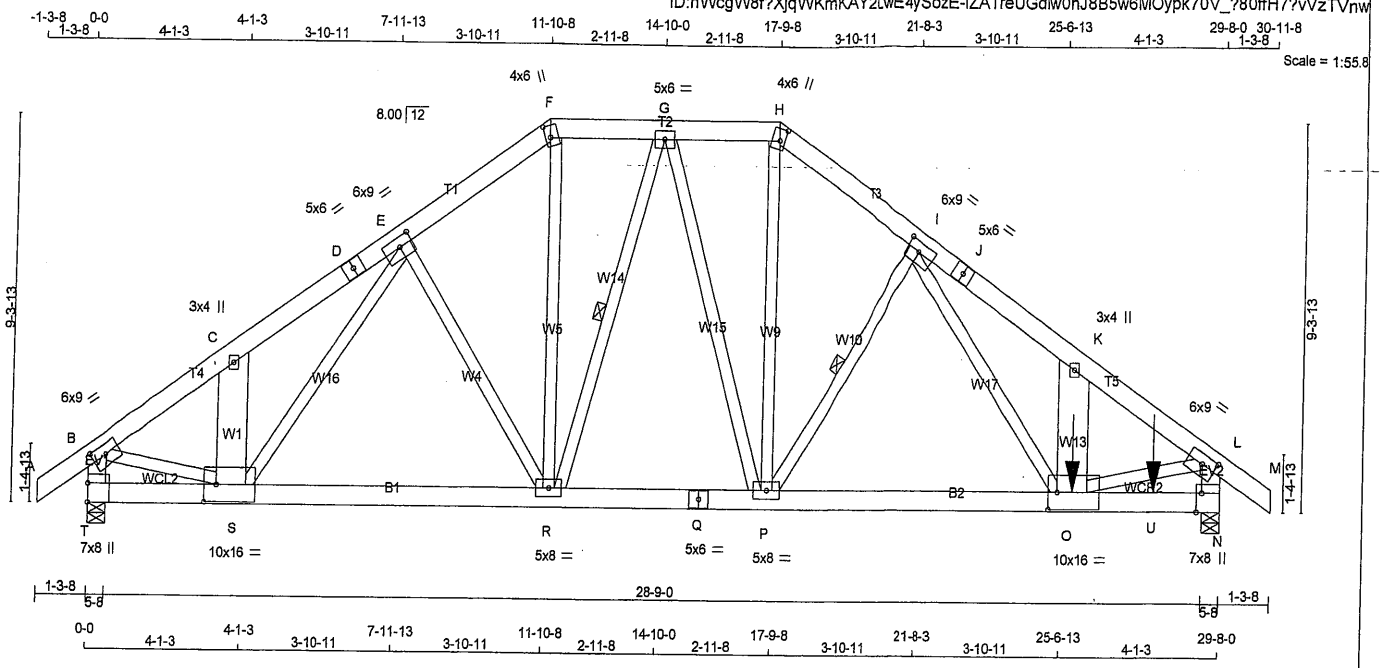


TOTAL WEIGHT = 3 X 221 = 663 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x10	DRY	No.2	SPF
O - K	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
J- M	2	12	TOP
T- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	3	4	TOP
Q- N	3	4	SIDE(1681.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
K- O	5	4	SIDE(1643.5)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TMW+w	MT20	3.0	4.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	6.0	9.0	2.75	4.00
F	TTW+m	MT20	4.0	6.0	3.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TTW+m	MT20	4.0	6.0	3.50	1.50
I	TMWW-t	MT20	6.0	9.0	2.75	4.00
J	TS-t	MT20	5.0	6.0		
K	TMW+w	MT20	3.0	4.0		
L	TMVW-t	MT20	6.0	9.0	2.75	4.25
N	BMV1+p	MT20	7.0	8.0	Edge	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3761	0	3761	0	0	5-8	5-8
N	12768	0	12768	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2967	1772 / 0	520 / 0	0 / 0	0 / 0	675 / 0	0 / 0
N	10062	6023 / 0	1755 / 0	0 / 0	0 / 0	2285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) T, N (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-P, G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

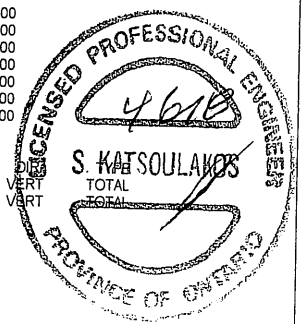
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX.. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.03 (1)	10.00
B- C	-4493 / 0	-112.4	-112.4	0.05 (1)	6.25
C- D	-4461 / 0	-112.4	-112.4	0.04 (1)	6.25
D- E	-4461 / 0	-112.4	-112.4	0.04 (1)	6.25
E- F	-4512 / 0	-112.4	-112.4	0.05 (1)	6.25
F- G	-3761 / 0	-112.4	-112.4	0.05 (1)	6.25
G- H	-4749 / 0	-112.4	-112.4	0.05 (1)	6.19
H- I	-5726 / 0	-112.4	-112.4	0.11 (1)	5.70
I- J	-15348 / 0	-112.4	-112.4	0.28 (1)	3.68
J- K	-15348 / 0	-112.4	-112.4	0.28 (1)	3.68
K- L	-15582 / 0	-112.4	-112.4	0.28 (1)	3.66
L- M	0 / 44	-112.4	-112.4	0.03 (1)	10.00
T- B	-3706 / 0	0.0	0.0	0.08 (1)	7.81
N- L	-12090 / 0	0.0	0.0	0.26 (1)	5.36
T- S	0 / 0	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 3900	-28.0	-28.0	0.08 (1)	10.00
R- Q	0 / 4285	-28.0	-28.0	0.10 (1)	10.00
Q- P	0 / 4285	-28.0	-28.0	0.10 (1)	10.00
P- O	0 / 7298	-28.0	-28.0	0.18 (1)	10.00
O- U	0 / 0	-28.0	-28.0	0.13 (1)	10.00
U- N	0 / 0	-28.0	-28.0	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-431 / 0	0.01 (1)
E- R	-316 / 0	0.08 (1)
R- F	0 / 2090	0.11 (1)
P- H	0 / 2808	0.15 (1)
P- I	-4721 / 0	0.40 (1)
O- K	-27 / 132	0.00 (3)
B- S	0 / 3845	0.21 (1)
O- L	0 / 13222	0.71 (1)
R- G	-1732 / 0	0.24 (1)
G- P	0 / 1531	0.08 (1)
S- E	-331 / 0	0.08 (1)
I- O	0 / 10219	0.55 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
O	25-10-4	-11171	-11171	—	FRONT	VERT	TOTAL
U	27-11-12	-873	-873	—	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.28/1.00 (I-K:1) , BC=0.18/1.00 (O-P:1) , WB=0.71/1.00 (L-O:1) , SSI=0.14/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PL.) MAX	MIN	SECTION (PL.) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.88 (B) (INPUT = 1.00)

DWG NO. TAM VT752-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T6	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWWW-t	MT20	10.0	16.0	5.00	3.00
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWWW-t	MT20	10.0	16.0	5.00	4.00
T	BMV1+p	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 11170.6 lbs FACTORED DOWN AT 25-10-4, AND 873.1 lbs FACTORED DOWN AT 27-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

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461B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17752-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T7A	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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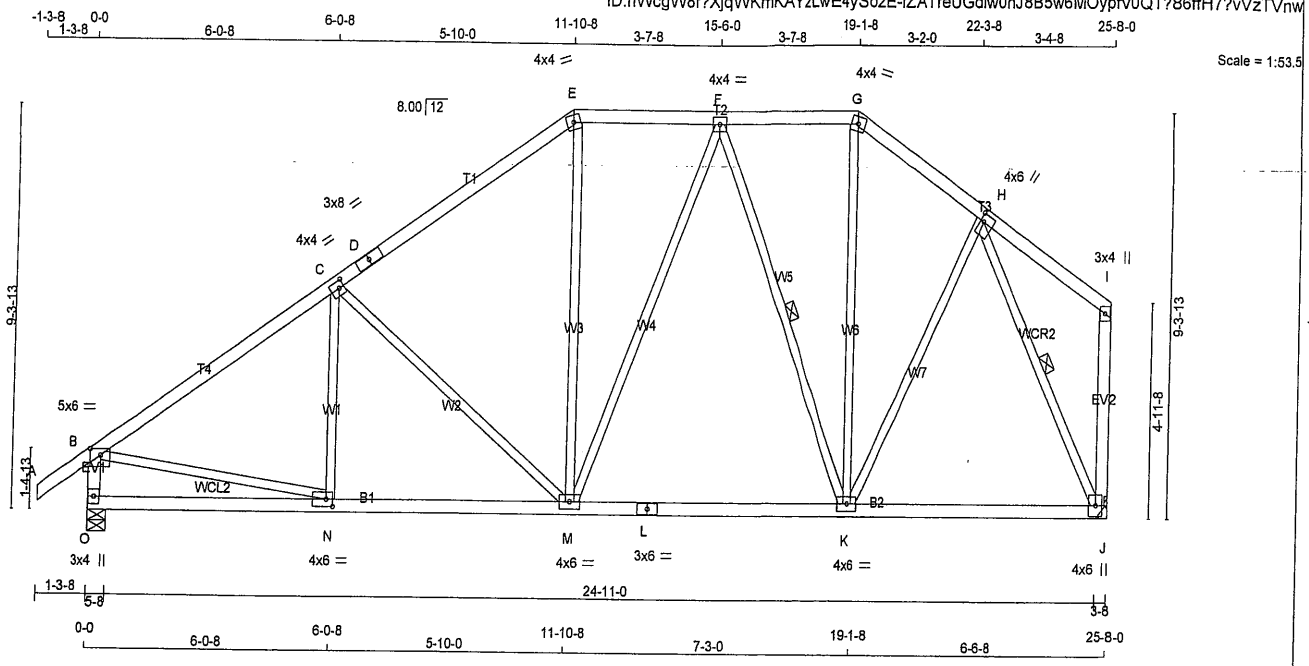


TOTAL WEIGHT = 3 X 127 = 382 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW+t	MT20	4.0	6.0	2.25	1.00
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.00
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1956	0	1956	0	0	5-8	5-8
J	1802	0	1802	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1542	923 / 0	269 / 0	0 / 0	0 / 0	350 / 0	0 / 0
J	1437	834 / 0	269 / 0	0 / 0	0 / 0	334 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-K, H-J.

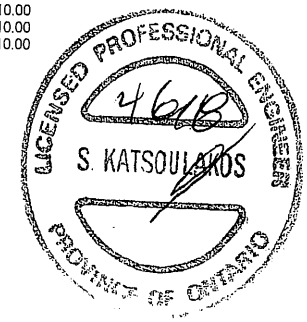
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	-2048 / 0	-112.4	-112.4	0.62 (1)		4.04
C- D	-1586 / 0	-112.4	-112.4	0.56 (1)		4.55
D- E	-1586 / 0	-112.4	-112.4	0.56 (1)		4.55
E- F	-1285 / 0	-112.4	-112.4	0.19 (1)		5.49
F- G	-998 / 0	-112.4	-112.4	0.19 (1)		6.04
G- H	-1219 / 0	-112.4	-112.4	0.15 (1)		5.65
H- I	0 / 22	-112.4	-112.4	0.18 (1)		10.00
O- B	-1886 / 0	0.0	0.0	0.19 (1)		6.11
J- I	-142 / 0	0.0	0.0	0.06 (1)		7.81
O- N	0 / 0	-28.0	-28.0	0.25 (3)		10.00
N- M	0 / 1741	-28.0	-28.0	0.42 (2)		10.00
M- L	0 / 1238	-28.0	-28.0	0.47 (2)		10.00
L- K	0 / 1238	-28.0	-28.0	0.47 (2)		10.00
K- J	0 / 757	-28.0	-28.0	0.41 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-130 / 184	0.08 (1)
C- M	-607 / 0	0.77 (1)
M- E	0 / 449	0.10 (1)
M- F	0 / 155	0.03 (2)
F- K	-659 / 0	0.44 (1)
K- G	0 / 359	0.08 (1)
K- H	0 / 529	0.12 (1)
B- N	0 / 1768	0.40 (1)
H- J	-1759 / 0	0.66 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.47/1.00 (K-M:2) , WB=0.77/1.00 (C-M:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (N) (INPUT = 0.90)
JSI METAL= 0.50 (H) (INPUT = 1.00)

DWG NO. TAM V7753-K
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T9	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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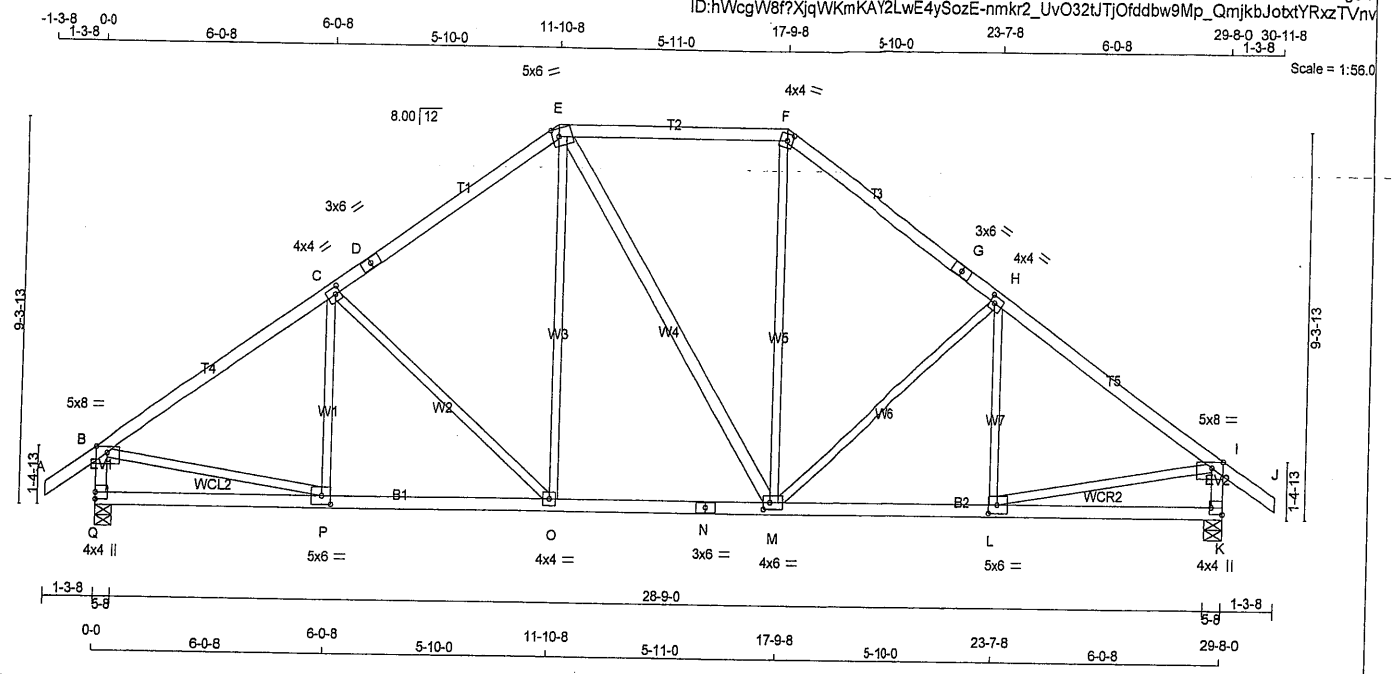

TOTAL WEIGHT = 3 X 135 = 404 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.75	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	4.0	6.0	2.00	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2237	0	2237	0	0	5-8	5-8
K	2237	0	2237	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1766	1053 / 0	311 / 0	0 / 0	0 / 0	402 / 0	0 / 0
K	1766	1053 / 0	311 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2447 / 0	-112.4	-112.4	0.66 (1)	3.70
C- D	-2029 / 0	-112.4	-112.4	0.60 (1)	4.08
D- E	-2029 / 0	-112.4	-112.4	0.60 (1)	4.08
E- F	-1656 / 0	-112.4	-112.4	0.55 (1)	4.47
F- G	-2030 / 0	-112.4	-112.4	0.60 (1)	4.08
G- H	-2030 / 0	-112.4	-112.4	0.60 (1)	4.08
H- I	-2447 / 0	-112.4	-112.4	0.66 (1)	3.70
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00
Q- B	-2167 / 0	0.0	0.0	0.22 (1)	5.77
K- I	-2167 / 0	0.0	0.0	0.22 (1)	5.77
Q- P	0 / 0	-28.0	-28.0	0.26 (3)	10.00
P- O	0 / 2073	-28.0	-28.0	0.46 (2)	10.00
O- N	0 / 1655	-28.0	-28.0	0.37 (1)	10.00
N- M	0 / 1655	-28.0	-28.0	0.37 (1)	10.00
M- L	0 / 2073	-28.0	-28.0	0.46 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-186 / 164	0.08 (1)
C- O	-559 / 0	0.71 (1)
O- E	0 / 524	0.12 (1)
E- M	0 / 1	0.00 (1)
M- F	0 / 526	0.12 (1)
M- H	-558 / 0	0.70 (1)
L- H	-187 / 162	0.08 (1)
B- P	0 / 2106	0.47 (1)
L- I	0 / 2106	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.46/1.00 (O-P:2) , WB=0.71/1.00 (C-O:1) , SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17254 -R8
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T9A	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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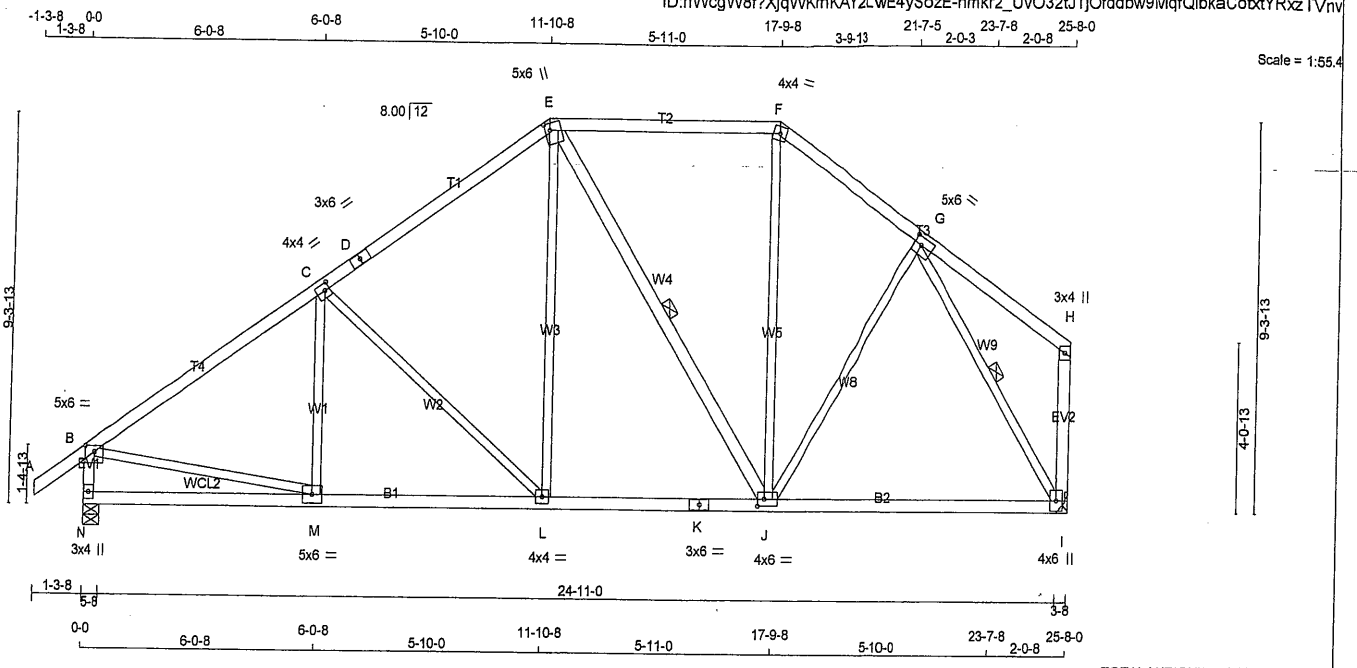


TOTAL WEIGHT = 7 X 123 = 860 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.25	2.25
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	2.00
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1956	0	1956	0	0	5-8	5-8
I	1802	0	1802	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1542	923 / 0	269 / 0	0 / 0	0 / 0	350 / 0	0 / 0
I	1437	834 / 0	269 / 0	0 / 0	0 / 0	334 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	-2050 / 0	-112.4	-112.4	0.62 (1)		4.04
C- D	-1581 / 0	-112.4	-112.4	0.56 (1)		4.55
D- E	-1581 / 0	-112.4	-112.4	0.56 (1)		4.55
E- F	-1114 / 0	-112.4	-112.4	0.52 (1)		5.26
F- G	-1366 / 0	-112.4	-112.4	0.24 (1)		5.31
G- H	0 / 28	-112.4	-112.4	0.28 (1)		10.00
N- B	-1888 / 0	0.0	0.0	0.19 (1)		6.11
I- H	-170 / 0	0.0	0.0	0.05 (1)		7.81
N- M	0 / 0	-28.0	-28.0	0.27 (3)		10.00
M- L	0 / 1743	-28.0	-28.0	0.44 (2)		10.00
L- K	0 / 1282	-28.0	-28.0	0.54 (2)		10.00
K- J	0 / 1282	-28.0	-28.0	0.54 (2)		10.00
J- I	0 / 951	-28.0	-28.0	0.49 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
M- C	-120 / 201	0.05 (1)
L- E	-616 / 0	0.78 (1)
E- J	0 / 537	0.12 (1)
J- F	-310 / 0	0.19 (1)
B- M	0 / 311	0.07 (2)
J- G	0 / 1770	0.40 (1)
G- I	0 / 301	0.07 (2)
	-1811 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.54/1.00 (J-L:2) , WB=0.78/1.00 (C-L:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

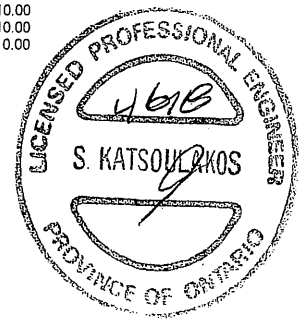
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.44 (M) (INPUT = 1.00)



DWG NO. TAM 17755-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T10A	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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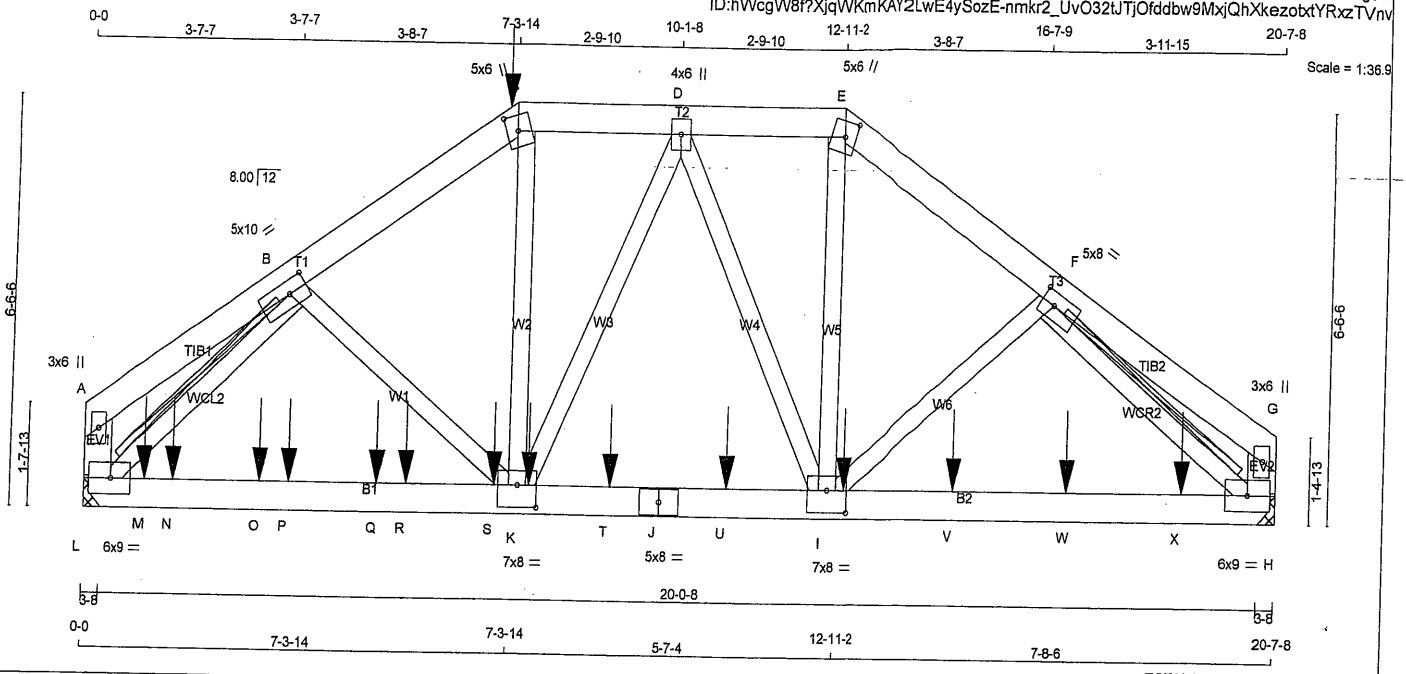


TOTAL WEIGHT = 3 X 131 = 394 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
L - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(183.1)
C- E	2	12	SIDE(91.5)
E- G	2	12	TOP
L- A	2	12	TOP
H- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	2	8	SIDE(925.7)
J- H	2	9	SIDE(586.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW-t	MT20	5.0	10.0	2.50	3.75
C	TTW+m	MT20	5.0	6.0	3.00	2.25
D	TMWW+t	MT20	4.0	6.0		
E	TTW+m	MT20	5.0	6.0	3.00	2.25
F	TMWW-t	MT20	5.0	8.0	2.50	2.75
G	TMV+p	MT20	3.0	6.0		
H	BMVW1-t	MT20	6.0	9.0		
I	BMWWW-t	MT20	7.0	8.0	4.25	4.00
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	7.0	8.0	4.25	4.00
L	BMVW1-t	MT20	6.0	9.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	12246	0	12246	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-10	
H	11199	0	11199	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	9682	5743 / 0	1724 / 0	0 / 0	0 / 0	2215 / 0	0 / 0
H	8841	5267 / 0	1559 / 0	0 / 0	0 / 0	2015 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-L, F-H

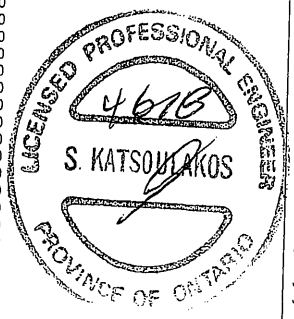
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-37 / 0	-112.4	-112.4	0.07 (1)	6.25	
B- C	-13086 / 0	-112.4	-112.4	0.16 (1)	4.06	
C- D	-11030 / 0	-112.4	-112.4	0.14 (1)	4.38	
D- E	-10582 / 0	-112.4	-112.4	0.13 (1)	4.46	
E- F	-12536 / 0	-112.4	-112.4	0.15 (1)	4.14	
F- G	-31 / 0	-112.4	-112.4	0.07 (1)	6.25	
L- A	-261 / 0	0.0	0.0	0.01 (1)	7.81	
H- G	-270 / 0	0.0	0.0	0.01 (1)	7.81	
L- M	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
M- N	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
N- O	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
O- P	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
P- Q	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
Q- R	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
R- S	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
S- K	0 / 9125	-28.0	-28.0	0.76 (1)	10.00	
K- T	0 / 10780	-28.0	-28.0	0.69 (1)	10.00	
T- J	0 / 10780	-28.0	-28.0	0.69 (1)	10.00	
J- U	0 / 10780	-28.0	-28.0	0.69 (1)	10.00	
U- I	0 / 10780	-28.0	-28.0	0.69 (1)	10.00	
I- V	0 / 9256	-28.0	-28.0	0.80 (1)	10.00	
V- W	0 / 9256	-28.0	-28.0	0.80 (1)	10.00	
W- X	0 / 9256	-28.0	-28.0	0.80 (1)	10.00	
X- H	0 / 9256	-28.0	-28.0	0.80 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	0 / 2319	0.12 (1)
K- C	0 / 5944	0.32 (1)
K- D	0 / 621	0.03 (1)
D- I	-499 / 0	0.09 (1)
I- E	0 / 6374	0.34 (1)
I- F	0 / 1542	0.08 (1)
L- B	-12899 / 0	0.53 (1)
F- H	-12459 / 0	0.54 (1)



DWG NO. TAM 17256-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA [M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 912 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 557 (0.44")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.80/1.00 (H-I:1) , WB=0.54/1.00 (F-H:1) , SSI=0.94/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.81 (J) (INPUT = 1.00)

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T10A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 729.6 lbs FACTORED DOWN AT 7-3-14 ON TOP CHORD, AND 1774.2 lbs FACTORED DOWN AT 1-0-12, 73.3 lbs FACTORED DOWN AT 1-6-12, 1774.2 lbs FACTORED DOWN AT 3-0-12, 73.3 lbs FACTORED DOWN AT 3-6-12, 1774.2 lbs FACTORED DOWN AT 5-0-12, 73.3 lbs FACTORED DOWN AT 5-6-12, 1844.4 lbs FACTORED DOWN AT 7-0-12, 73.3 lbs FACTORED DOWN AT 7-0-12, 1417.5 lbs FACTORED DOWN AT 7-8-0, 1844.4 lbs FACTORED DOWN AT 9-0-12, 1844.4 lbs FACTORED DOWN AT 11-0-12, 1844.4 lbs FACTORED DOWN AT 13-0-12, 1844.4 lbs FACTORED DOWN AT 15-0-12, AND 1844.4 lbs FACTORED DOWN AT 17-0-12, AND 1844.4 lbs FACTORED DOWN AT 19-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	7-3-14	-79	-88	—	FRONT	VERT	DEAD
C	7-3-14	-141	-141	—	FRONT	VERT	TOTAL
C	7-3-14	-510	-510	—	FRONT	VERT	SNOW
I	13-0-12	-1844	-1844	—	BACK	VERT	TOTAL
K	7-8-0	-1418	-1418	—	FRONT	VERT	TOTAL
M	1-0-12	-1774	-1774	—	BACK	VERT	TOTAL
N	1-6-12	-42	-73	—	FRONT	VERT	TOTAL
O	3-0-12	-1774	-1774	—	BACK	VERT	TOTAL
P	3-6-12	-42	-73	—	FRONT	VERT	TOTAL
Q	5-0-12	-1774	-1774	—	BACK	VERT	TOTAL
R	5-6-12	-42	-73	—	FRONT	VERT	TOTAL
S	7-0-12	-1844	-1844	—	BACK	VERT	TOTAL
S	7-0-12	-42	-73	—	FRONT	VERT	TOTAL
T	9-0-12	-1844	-1844	—	BACK	VERT	TOTAL
U	11-0-12	-1844	-1844	—	BACK	VERT	TOTAL
V	15-0-12	-1844	-1844	—	BACK	VERT	TOTAL
W	17-0-12	-1844	-1844	—	BACK	VERT	TOTAL
X	19-0-12	-1844	-1844	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4616
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17756-1B
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T11	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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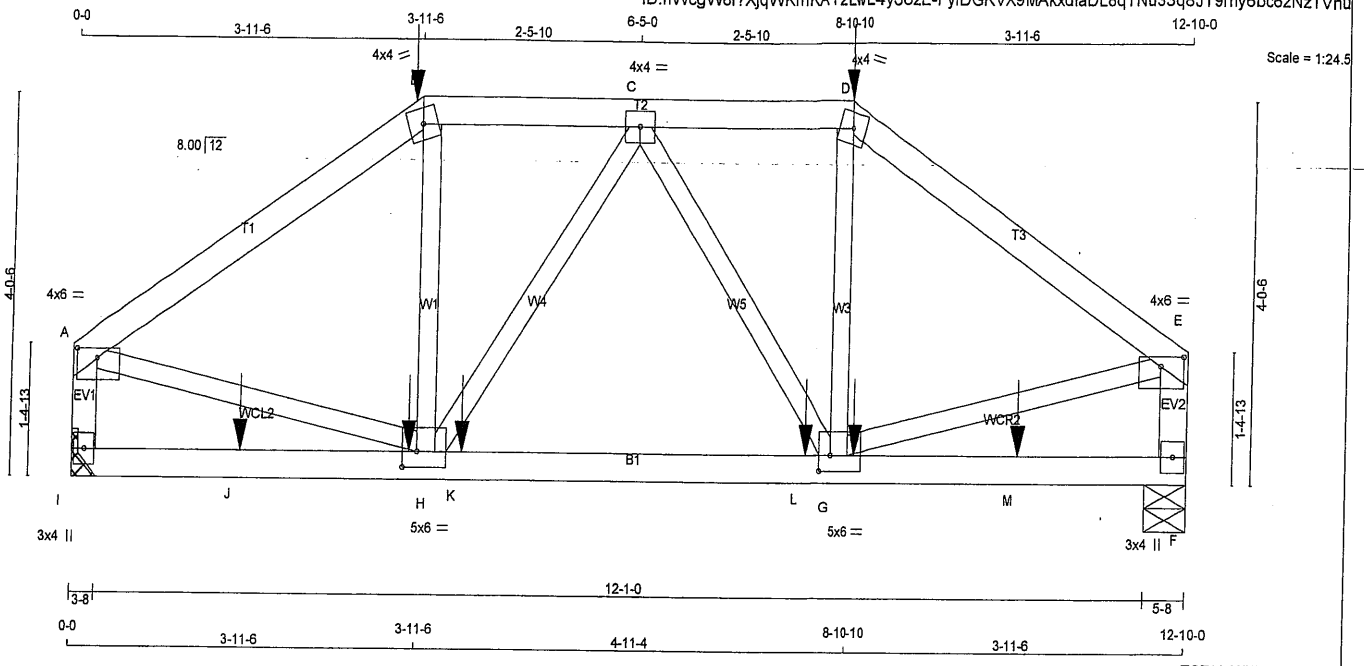

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.25	3.00
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	6.0	1.25	3.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.00	1.50
H	BMWWW-t	MT20	5.0	3.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 199.9 lbs FACTORED DOWN AT 3-11-6, AND 199.9 lbs FACTORED DOWN AT 8-10-10 ON TOP CHORD, AND 14.0 lbs FACTORED DOWN AT 1-11-4, 14.0 lbs FACTORED DOWN AT 3-10-4, 268.5 lbs FACTORED DOWN AT 4-5-8, 268.5 lbs FACTORED DOWN AT 8-4-8, AND 14.0 lbs FACTORED DOWN AT 8-11-12, AND 14.0 lbs FACTORED DOWN AT 10-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1385	0	1385	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
F	1385	0	1385	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1077	665 / 0	170 / 0	0 / 0	0 / 0	242 / 0	0 / 0
F	1077	665 / 0	170 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1452 / 0	-112.4	-112.4	0.38 (1)	4.93	H- B	0 / 280	0.07 (2)
B- C	-1218 / 0	-112.4	-112.4	0.12 (1)	5.65	G- D	0 / 280	0.07 (2)
C- D	-1218 / 0	-112.4	-112.4	0.12 (1)	5.65	A- H	0 / 1247	0.31 (1)
D- E	-1452 / 0	-112.4	-112.4	0.38 (1)	4.93	G- E	0 / 1247	0.31 (1)
I- A	-1347 / 0	0.0	0.0	0.15 (1)	6.95	H- C	-187 / 0	0.06 (1)
F- E	-1347 / 0	0.0	0.0	0.15 (1)	6.95	C- G	-187 / 0	0.06 (1)
I- J	0 / 0	-28.0	-28.0	0.18 (2)	10.00			
J- H	0 / 0	-28.0	-28.0	0.18 (2)	10.00			
H- K	0 / 1316	-28.0	-28.0	0.38 (1)	10.00			
K- L	0 / 1316	-28.0	-28.0	0.38 (1)	10.00			
L- G	0 / 1316	-28.0	-28.0	0.38 (1)	10.00			
G- M	0 / 0	-28.0	-28.0	0.18 (2)	10.00			
M- F	0 / 0	-28.0	-28.0	0.18 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-11-6	-25	-28	—	FRONT	VERT	DEAD
B	3-11-6	-16	-16	—	FRONT	VERT	TOTAL
B	3-11-6	-159	-159	—	FRONT	VERT	SNOW
D	8-10-10	-25	-28	—	FRONT	VERT	DEAD
D	8-10-10	-16	-16	—	FRONT	VERT	TOTAL
D	8-10-10	-159	-159	—	FRONT	VERT	SNOW
G	8-11-12	-8	-14	—	FRONT	VERT	TOTAL
H	3-10-4	-8	-14	—	FRONT	VERT	TOTAL
J	1-11-4	-8	-14	—	FRONT	VERT	TOTAL
K	4-5-8	-268	-268	—	FRONT	VERT	TOTAL
L	8-4-8	-268	-268	—	FRONT	VERT	TOTAL
M	10-10-12	-8	-14	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38/1.00 (A-B:1) , BC=0.38/1.00 (G-H:1) , WB=0.31/1.00 (A-H:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

DWG NO. TAM 17 757-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T12	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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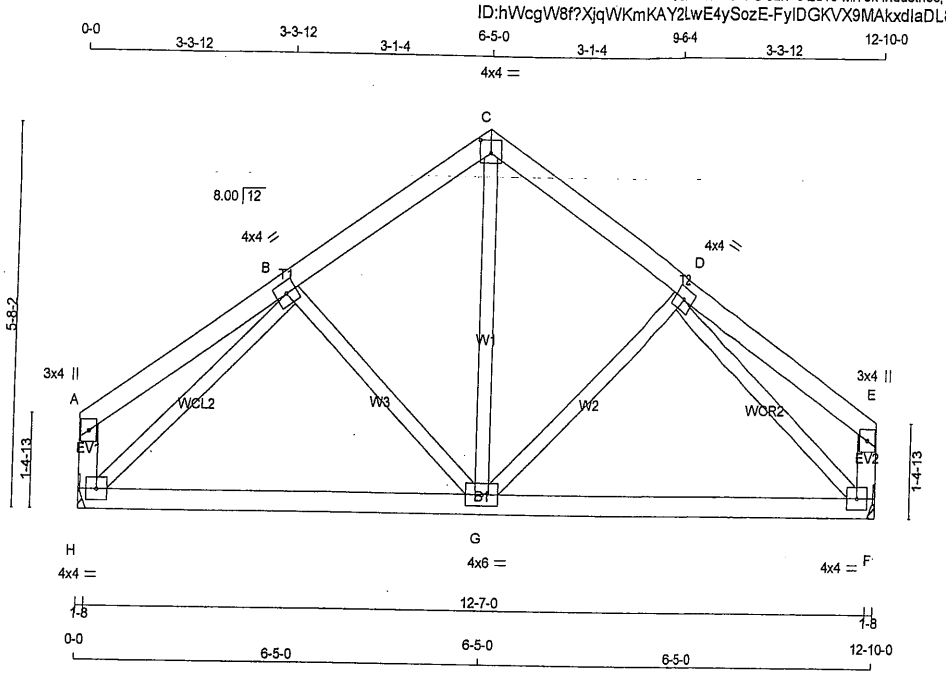


TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	901	0	901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	901	0	901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	718	417 / 0	135 / 0	0 / 0	0 / 0	167 / 0	0 / 0
F	718	417 / 0	135 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.18 (1)	10.00
B- C	-701 / 0	-112.4	-112.4	0.14 (1)	6.25
C- D	-701 / 0	-112.4	-112.4	0.14 (1)	6.25
D- E	0 / 22	-112.4	-112.4	0.18 (1)	10.00
H- A	-140 / 0	0.0	0.0	0.01 (1)	7.81
F- E	-140 / 0	0.0	0.0	0.01 (1)	7.81
H- G	0 / 689	-28.0	-28.0	0.40 (2)	10.00
G- F	0 / 689	-28.0	-28.0	0.40 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 470	0.11 (1)
G- D	-176 / 30	0.06 (1)
B- G	-176 / 30	0.06 (1)
H- B	-977 / 0	0.32 (1)
D- F	-977 / 0	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.40/1.00 (F-G:2) , WB=0.32/1.00 (D-F:1) , SSI=0.16/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

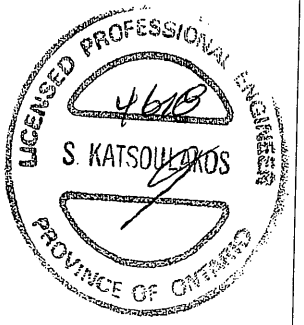
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.35 (D) (INPUT = 1.00)



DWG NO. TAM 17758-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T13A	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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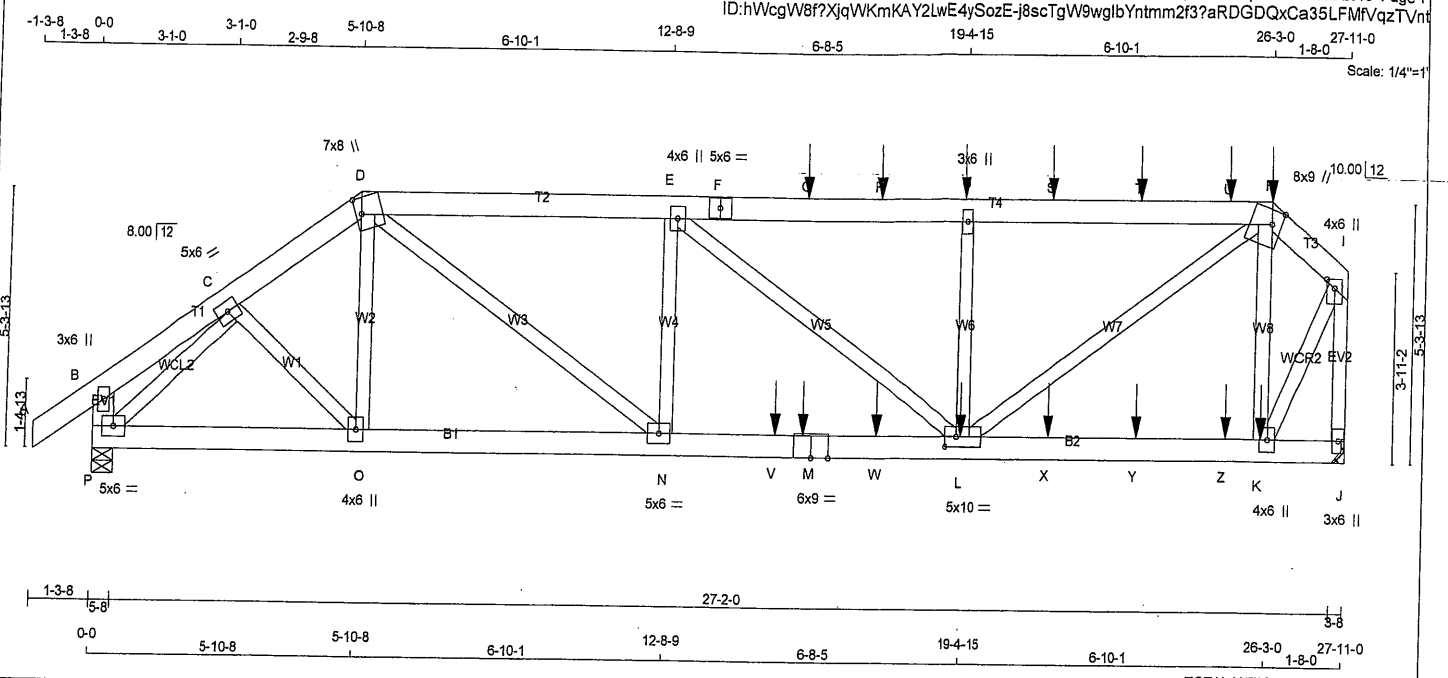

TOTAL WEIGHT = 2 X 171 = 341 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	SIDE(183.1)
H- I	2	12	SIDE(122.0)
P- B	2	12	TOP
J- I	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	12	SIDE(0.0)
M- J	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	7.0	8.0	4.00	1.50
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.50	2.50
I	TMVW+p	MT20	4.0	6.0	2.25	2.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	10.0	2.50	3.00
M	BS-t	MT20	6.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	3193	0	3193	0	0	IN-SX	IN-SX
J	3831	0	3831	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2512	1511 / 0	433 / 0	0 / 0	0 / 0	568 / 0	0 / 0
J	3047	1778 / 0	561 / 0	0 / 0	0 / 0	708 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P
BEARING SIZE FACTOR = 1.15 AT JNT(S) P (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-8 / 0	-112.4	-112.4	0.04 (1)	10.00
C- D	-3816 / 0	-112.4	-112.4	0.07 (1)	5.73
D- E	-5904 / 0	-112.4	-112.4	0.26 (1)	4.64
E- F	-5281 / 0	-112.4	-112.4	0.34 (1)	4.74
F- Q	-5281 / 0	-112.4	-112.4	0.34 (1)	4.74
Q- R	-5281 / 0	-112.4	-112.4	0.34 (1)	4.74
R- G	-5281 / 0	-112.4	-112.4	0.34 (1)	4.74
G- S	-5281 / 0	-112.4	-112.4	0.35 (1)	4.74
S- T	-5281 / 0	-112.4	-112.4	0.35 (1)	4.74
T- U	-5281 / 0	-112.4	-112.4	0.35 (1)	4.74
U- H	-5281 / 0	-112.4	-112.4	0.35 (1)	4.74
H- I	-1769 / 0	-112.4	-112.4	0.02 (1)	6.25
P- B	-325 / 0	0.0	0.0	0.01 (1)	7.81
J- I	-3768 / 0	0.0	0.0	0.42 (1)	6.07
P- O	0 / 2834	-28.0	-28.0	0.11 (1)	10.00
O- N	0 / 3155	-28.0	-28.0	0.28 (1)	10.00
N- V	0 / 5904	-28.0	-28.0	0.54 (1)	10.00
V- M	0 / 5904	-28.0	-28.0	0.54 (1)	10.00
M- W	0 / 5904	-28.0	-28.0	0.54 (1)	10.00
W- L	0 / 5904	-28.0	-28.0	0.54 (1)	10.00
L- X	0 / 1271	-28.0	-28.0	0.19 (1)	10.00
X- Y	0 / 1271	-28.0	-28.0	0.19 (1)	10.00
Y- Z	0 / 1271	-28.0	-28.0	0.19 (1)	10.00
Z- K	0 / 1271	-28.0	-28.0	0.19 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	0 / 447	0.04 (1)
O- D	-310 / 65	0.04 (1)
D- N	0 / 3367	0.30 (1)
N- E	-440 / 59	0.06 (1)
E- L	-770 / 0	0.38 (1)
L- G	-1345 / 0	0.18 (1)
L- H	0 / 4907	0.43 (1)
K- H	-2456 / 0	0.33 (1)
P- C	-3957 / 0	0.37 (1)
K- I	0 / 2861	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	19-4-4	-135	-135	---	BACK	VERT	TOTAL
H	26-3-0	-142	-142	---	BACK	VERT	TOTAL
K	26-1-4	-41	-72	---	BACK	VERT	TOTAL
L	19-4-4	-41	-72	---	BACK	VERT	TOTAL
M	15-8-12	-41	-72	---	BACK	VERT	TOTAL
Q	15-8-12	-135	-135	---	BACK	VERT	TOTAL
R	17-4-4	-135	-135	---	BACK	VERT	TOTAL
S	21-4-4	-135	-135	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCRC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.42/1.00 (I-J:1) , BC=0.54/1.00 (L-N:1) , WB=0.43/1.00 (H-L:1) , SSI=0.41/1.00 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.84 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17759-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T13A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 135.3 lbs FACTORED DOWN AT 15-8-12, 135.3 lbs FACTORED DOWN AT 17-4-4, 135.3 lbs FACTORED DOWN AT 19-4-4, 135.3 lbs FACTORED DOWN AT 21-4-4, 135.3 lbs FACTORED DOWN AT 23-4-4, AND 141.2 lbs FACTORED DOWN AT 25-4-4, AND 141.7 lbs FACTORED DOWN AT 26-3-0 ON TOP CHORD, AND 1698.0 lbs FACTORED DOWN AT 15-1-8, 71.7 lbs FACTORED DOWN AT 15-8-12, 71.7 lbs FACTORED DOWN AT 17-4-4, 71.7 lbs FACTORED DOWN AT 19-4-4, 71.7 lbs FACTORED DOWN AT 21-4-4, 71.7 lbs FACTORED DOWN AT 23-4-4, AND 71.7 lbs FACTORED DOWN AT 25-4-4, AND 71.7 lbs FACTORED DOWN AT 26-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	23-4-4	-135	-135	—	BACK	VERT	TOTAL
U	25-4-4	-141	-141	—	BACK	VERT	TOTAL
V	15-1-8	-1698	-1698	—	BACK	VERT	TOTAL
W	17-4-4	-41	-72	—	BACK	VERT	TOTAL
X	21-4-4	-41	-72	—	BACK	VERT	TOTAL
Y	23-4-4	-41	-72	—	BACK	VERT	TOTAL
Z	25-4-4	-41	-72	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4616
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17259-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T14A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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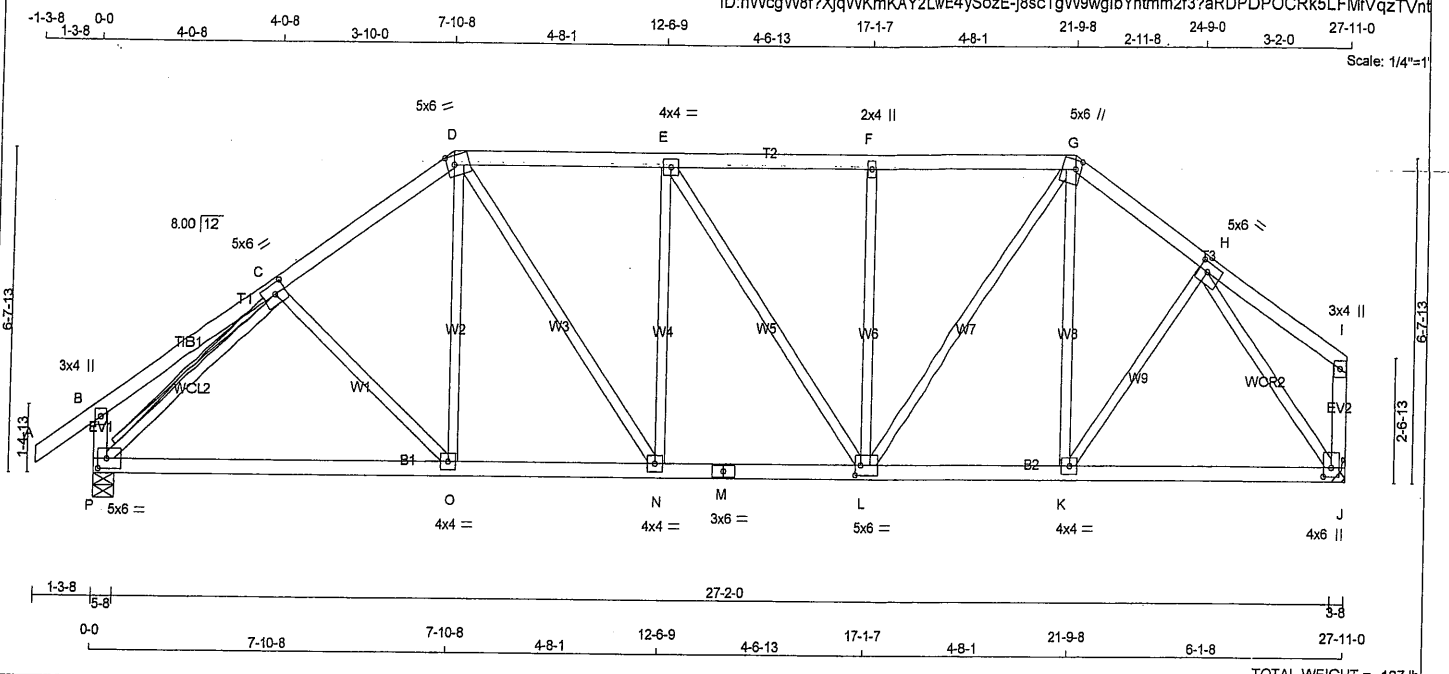


TOTAL WEIGHT = 127 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.25	2.25
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0	2.25	2.00
K, N, O						
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BS-t	MT20	3.0	6.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.25

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2114	0	2114	0	0	5-8	5-8
J	1960	0	1960	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1668	996 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
J	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

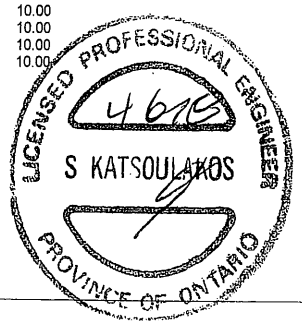
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 27	-112.4	-112.4	0.26 (1)	10.00
C- D	-2155 / 0	-112.4	-112.4	0.34 (1)	4.30
D- E	-2168 / 0	-112.4	-112.4	0.41 (1)	4.19
E- F	-2081 / 0	-112.4	-112.4	0.40 (1)	4.27
F- G	-2081 / 0	-112.4	-112.4	0.39 (1)	4.29
G- H	-1822 / 0	-112.4	-112.4	0.20 (1)	4.77
H- I	0 / 20	-112.4	-112.4	0.15 (1)	10.00
P- B	-327 / 0	0.0	0.0	0.03 (1)	7.81
J- I	-135 / 0	0.0	0.0	0.02 (1)	7.81
P- O	0 / 1800	-28.0	-28.0	0.57 (2)	10.00
O- N	0 / 1772	-28.0	-28.0	0.58 (2)	10.00
N- M	0 / 2169	-28.0	-28.0	0.42 (1)	10.00
M- L	0 / 2169	-28.0	-28.0	0.42 (1)	10.00
L- K	0 / 1494	-28.0	-28.0	0.40 (2)	10.00
K- J	0 / 1248	-28.0	-28.0	0.37 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-43 / 117	0.03 (3)
O- D	0 / 340	0.08 (2)
D- N	0 / 668	0.15 (1)
N- E	-442 / 0	0.32 (1)
E- L	-150 / 0	0.19 (1)
L- F	-560 / 0	0.41 (1)
L- G	0 / 990	0.22 (1)
K- G	-151 / 88	0.11 (1)
K- H	0 / 415	0.09 (1)
P- C	-2472 / 0	0.97 (1)
H- J	-2152 / 0	0.95 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.58/1.00 (N-O:2) , WB=0.97/1.00 (C-P:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.63 (M) (INPUT = 1.00)

DWG NO. TAM 17760-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T15A	1	1	OBC2012_6TCDL TRUSS DESC.	-

Tamarack Roof Truss, Burlington

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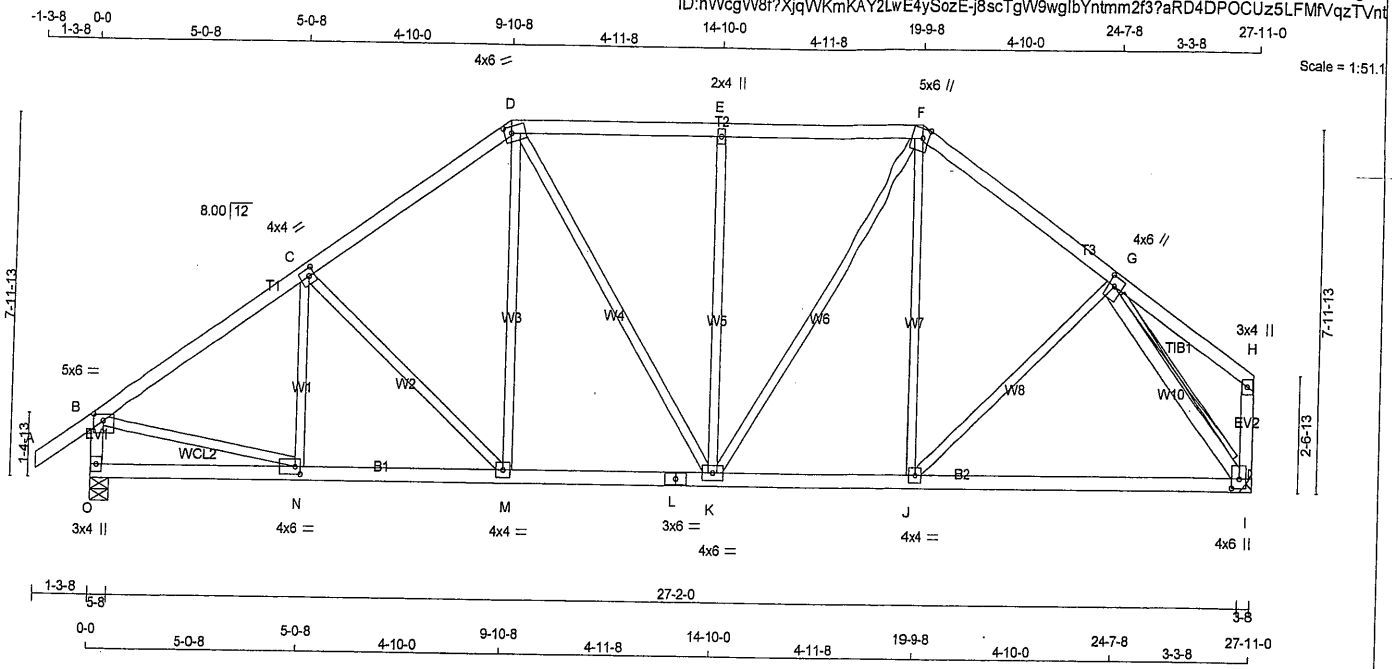


TOTAL WEIGHT = 129 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW+t	MT20	4.0	6.0	2.50	1.75
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0	2.50	2.00
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2114	0	2114	0	0	5-8	5-8
I	1960	0	1960	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1668	996 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT G-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

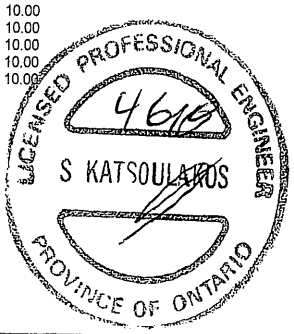
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2257 / 0	-112.4	-112.4	0.43 (1)	4.14
C- D	-2003 / 0	-112.4	-112.4	0.41 (1)	4.37
D- E	-1816 / 0	-112.4	-112.4	0.39 (1)	4.55
E- F	-1816 / 0	-112.4	-112.4	0.39 (1)	4.55
F- G	-1825 / 0	-112.4	-112.4	0.34 (1)	4.63
G- H	0 / 39	-112.4	-112.4	0.33 (1)	10.00
O- B	-2054 / 0	0.0	0.0	0.21 (1)	5.90
I- H	-120 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 0	-28.0	-28.0	0.17 (3)	10.00
N- M	0 / 1908	-28.0	-28.0	0.39 (1)	10.00
M- L	0 / 1638	-28.0	-28.0	0.35 (1)	10.00
L- K	0 / 1638	-28.0	-28.0	0.35 (1)	10.00
K- J	0 / 1494	-28.0	-28.0	0.58 (2)	10.00
J- I	0 / 1310	-28.0	-28.0	0.55 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-261 / 85	0.09 (1)
C- M	-370 / 0	0.30 (1)
M- D	0 / 412	0.09 (2)
D- K	0 / 328	0.07 (1)
K- E	-679 / 0	0.82 (1)
K- F	0 / 593	0.13 (1)
J- F	0 / 249	0.06 (3)
J- G	0 / 274	0.06 (2)
B- N	0 / 1951	0.44 (1)
G- I	-2183 / 0	0.72 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.58/1.00 (J-K:2) , WB=0.82/1.00 (E-K:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.72 (G) (INPUT = 1.00)

DWG NO. TAM1776/-B
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T16A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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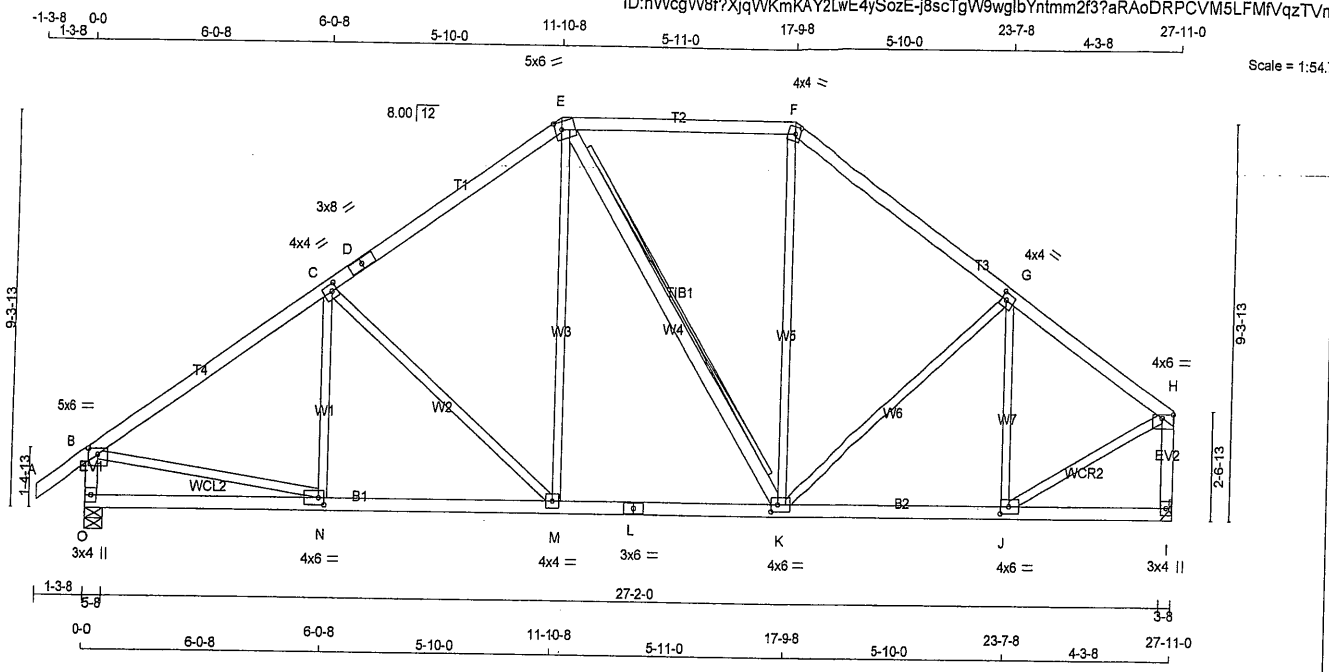

TOTAL WEIGHT = 129 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	4.0	6.0	1.00	3.25
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2114	0	2114	0	0	5-8	5-8
I	1960	0	1960	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1668	996 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2273 / 0	-112.4	-112.4	0.64 (1)	3.85
C- D	-1834 / 0	-112.4	-112.4	0.58 (1)	4.27
D- E	-1834 / 0	-112.4	-112.4	0.58 (1)	4.27
E- F	-1415 / 0	-112.4	-112.4	0.54 (1)	4.77
F- G	-1733 / 0	-112.4	-112.4	0.51 (1)	4.49
G- H	-1729 / 0	-112.4	-112.4	0.43 (1)	4.58
O- B	-2045 / 0	0.0	0.0	0.21 (1)	5.91
I- H	-1914 / 0	0.0	0.0	0.26 (1)	6.07
O- N	0 / 0	-28.0	-28.0	0.26 (3)	10.00
N- M	0 / 1928	-28.0	-28.0	0.45 (2)	10.00
M- L	0 / 1493	-28.0	-28.0	0.36 (2)	10.00
L- K	0 / 1493	-28.0	-28.0	0.36 (2)	10.00
K- J	0 / 1479	-28.0	-28.0	0.36 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.18 (3)	0.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-159 / 176	0.07 (1)
C- M	-582 / 0	0.73 (1)
M- E	0 / 537	0.12 (1)
E- K	-143 / 0	0.12 (1)
K- F	0 / 402	0.09 (2)
K- G	-89 / 0	0.11 (1)
J- G	-620 / 0	0.27 (1)
B- N	0 / 1959	0.44 (1)
J- H	0 / 1669	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.45/1.00 (M-N:2) , WB=0.73/1.00 (C-M:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)

DWG NO. TAM 17762-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T17A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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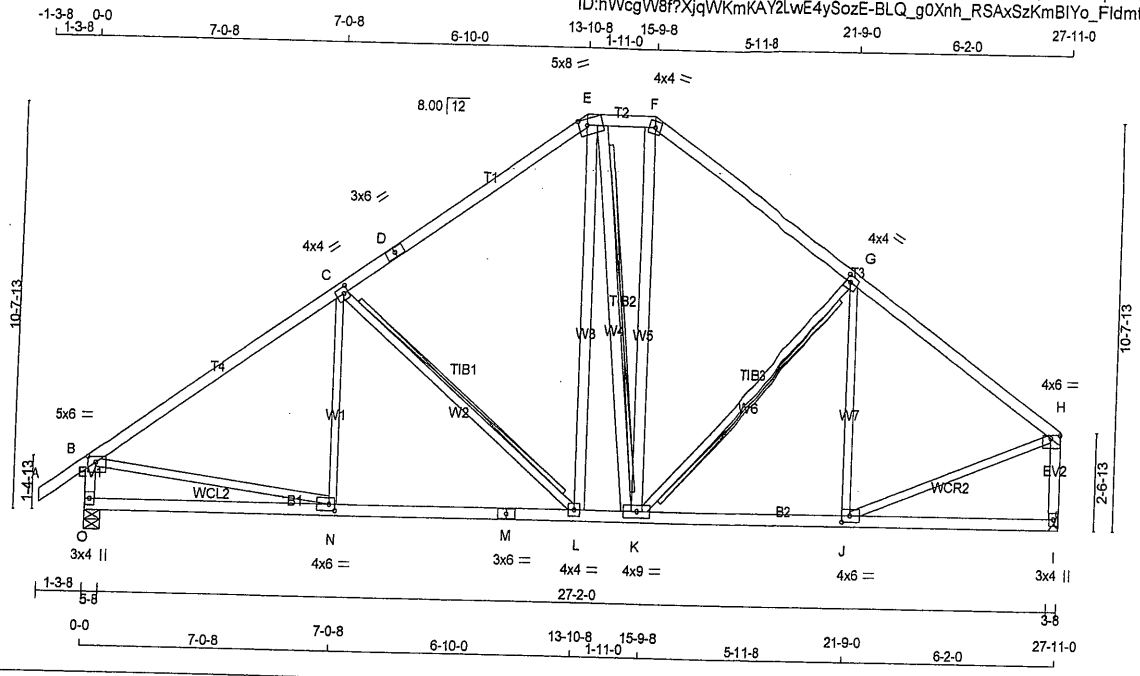

TOTAL WEIGHT = 144 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	4.0	6.0	1.00	3.25
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.00
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2114	0	2114	0	0	IN-SX	IN-SX
I	1960	0	1960	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1668	996 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
I	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-L, E-K
2x3 DRY SPF No.2 T-BRACE AT G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	N- C	-69 / 267	0.06 (3)
B- C	-2263 / 0	-112.4	-112.4	0.91 (1)	3.33	C- L	-786 / 0	0.50 (1)
C- D	-1642 / 0	-112.4	-112.4	0.80 (1)	3.98	L- E	0 / 610	0.10 (1)
D- E	-1642 / 0	-112.4	-112.4	0.80 (1)	3.98	E- K	-123 / 0	0.09 (1)
E- F	-1308 / 0	-112.4	-112.4	0.07 (1)	5.59	K- F	0 / 512	0.08 (1)
F- G	-1612 / 0	-112.4	-112.4	0.59 (1)	4.47	K- G	-398 / 0	0.54 (1)
G- H	-1850 / 0	-112.4	-112.4	0.63 (1)	4.18	J- G	-380 / 88	0.28 (1)
O- B	-2035 / 0	0.0	0.0	0.21 (1)	5.92	B- N	0 / 1949	0.44 (1)
I- H	-1890 / 0	0.0	0.0	0.26 (1)	6.10	J- H	0 / 1678	0.38 (1)
O- N	0 / 0	-28.0	-28.0	0.37 (3)	10.00			
N- M	0 / 1926	-28.0	-28.0	0.56 (2)	10.00			
M- L	0 / 1926	-28.0	-28.0	0.56 (2)	10.00			
L- K	0 / 1329	-28.0	-28.0	0.31 (2)	10.00			
K- J	0 / 1577	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.91/1.00 (B-C:1) , BC=0.56/1.00 (L-N:2) , WB=0.54/1.00 (G-K:1) , SSI=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17763-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T18	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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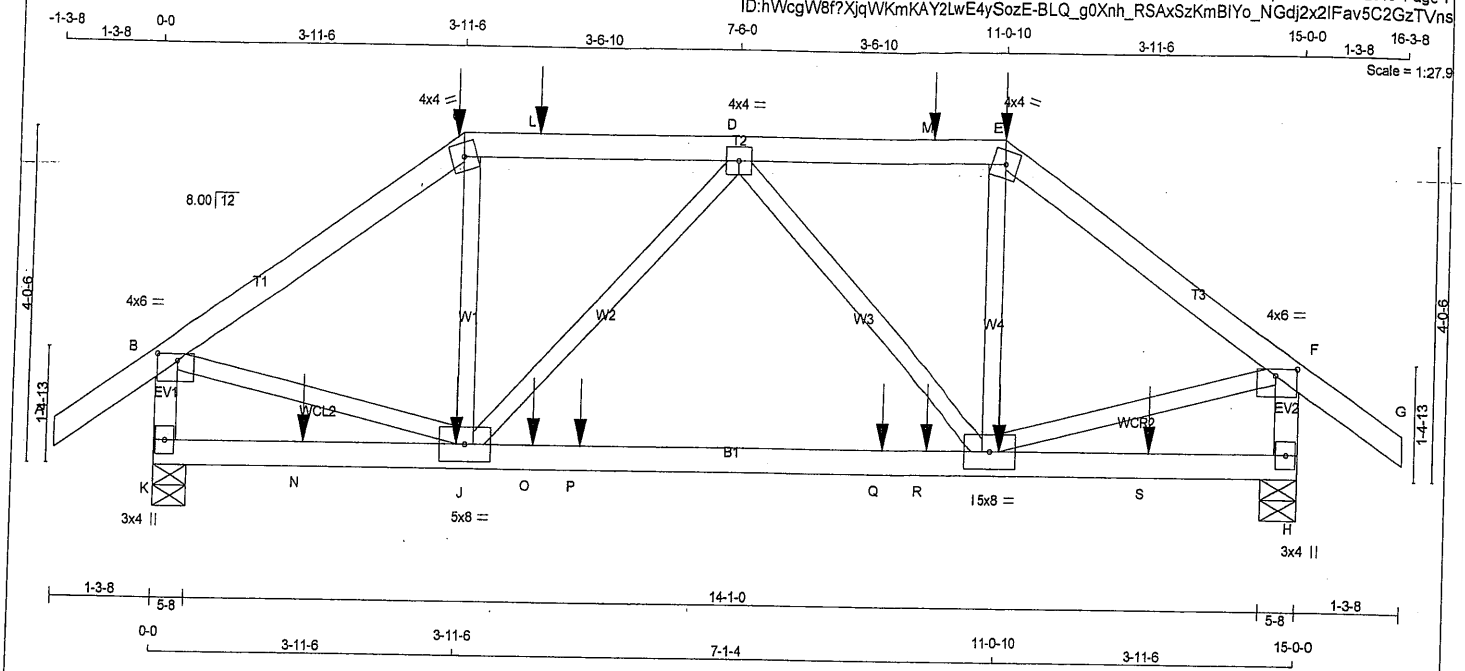

TOTAL WEIGHT = 62 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	6.0	1.00	3.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 184.0 lbs FACTORED DOWN AT 3-11-6, 184.0 lbs FACTORED DOWN AT 11-0-10, 16.6 lbs FACTORED DOWN AT 3-11-6, 9.7 lbs FACTORED DOWN AT 4-11-4, AND 9.7 lbs FACTORED DOWN AT 10-0-12, AND 16.6 lbs FACTORED DOWN AT 11-0-10 ON TOP CHORD, AND 14.0 lbs FACTORED DOWN AT 1-11-4, 14.0 lbs FACTORED DOWN AT 3-11-4, 14.0 lbs FACTORED DOWN AT 4-11-4, 268.5 lbs FACTORED DOWN AT 5-6-8, 268.5 lbs FACTORED DOWN AT 9-5-8, 14.0 lbs FACTORED DOWN AT 10-0-12, AND 14.0 lbs FACTORED DOWN AT 11-0-12, AND 14.0 lbs FACTORED DOWN AT 13-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1710	0	1710	0	0	5-8	5-8
H	1710	0	1710	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1321	830 / 0	199 / 0	0 / 0	0 / 0	292 / 0	0 / 0
H	1321	830 / 0	199 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00
B- C	-1737 / 0	-112.4	-112.4	0.40 (1)	4.57
C- L	-1460 / 0	-112.4	-112.4	0.28 (1)	5.05
L- D	-1460 / 0	-112.4	-112.4	0.28 (1)	5.05
D- M	-1460 / 0	-112.4	-112.4	0.28 (1)	5.05
M- E	-1460 / 0	-112.4	-112.4	0.28 (1)	5.05
E- F	-1737 / 0	-112.4	-112.4	0.40 (1)	4.57
F- G	0 / 43	-112.4	-112.4	0.17 (1)	10.00
K- B	-1721 / 0	0.0	0.0	0.19 (1)	6.30
H- F	-1721 / 0	0.0	0.0	0.19 (1)	6.30
K- N	0 / 0	-28.0	-28.0	0.45 (1)	10.00
N- J	0 / 0	-28.0	-28.0	0.45 (1)	10.00
J- O	0 / 1679	-28.0	-28.0	0.74 (1)	10.00
O- P	0 / 1679	-28.0	-28.0	0.74 (1)	10.00
P- Q	0 / 1679	-28.0	-28.0	0.74 (1)	10.00
Q- R	0 / 1679	-28.0	-28.0	0.74 (1)	10.00
R- I	0 / 1679	-28.0	-28.0	0.74 (1)	10.00
I- S	0 / 0	-28.0	-28.0	0.45 (1)	10.00
S- H	0 / 0	-28.0	-28.0	0.45 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 378	0.09 (1)
J- D	-327 / 0	0.15 (1)
D- I	-327 / 0	0.15 (1)
I- E	0 / 378	0.09 (1)
B- J	0 / 1491	0.37 (1)
I- F	0 / 1491	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-11-6	-25	-28	---	FRONT	VERT	DEAD
C	3-11-6	-17	-17	---	BACK	VERT	TOTAL
C	3-11-6	-159	-159	---	FRONT	VERT	SNOW
E	11-0-10	-25	-28	---	FRONT	VERT	DEAD
E	11-0-10	-17	-17	---	BACK	VERT	TOTAL
E	11-0-10	-159	-159	---	FRONT	VERT	SNOW
I	11-0-12	-8	-14	---	BACK	VERT	TOTAL
J	3-11-4	-8	-14	---	BACK	VERT	TOTAL
L	4-11-4	-10	-10	---	BACK	VERT	TOTAL
M	10-0-12	-10	-10	---	BACK	VERT	TOTAL
N	1-11-4	-8	-14	---	BACK	VERT	TOTAL
O	4-11-4	-8	-14	---	BACK	VERT	TOTAL
P	5-6-8	-268	-268	---	BACK	VERT	TOTAL
Q	9-5-8	-268	-268	---	BACK	VERT	TOTAL
R	10-0-12	-8	-14	---	BACK	VERT	TOTAL
S	13-0-12	-8	-14	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 635 (0.28")

CSI: TC=0.40/1.00 (E-F:1) , BC=0.74/1.00 (I-J:1) , WB=0.37/1.00 (B-J:1) , SSI=0.32/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)

DWG NO. TAM 17764-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T19	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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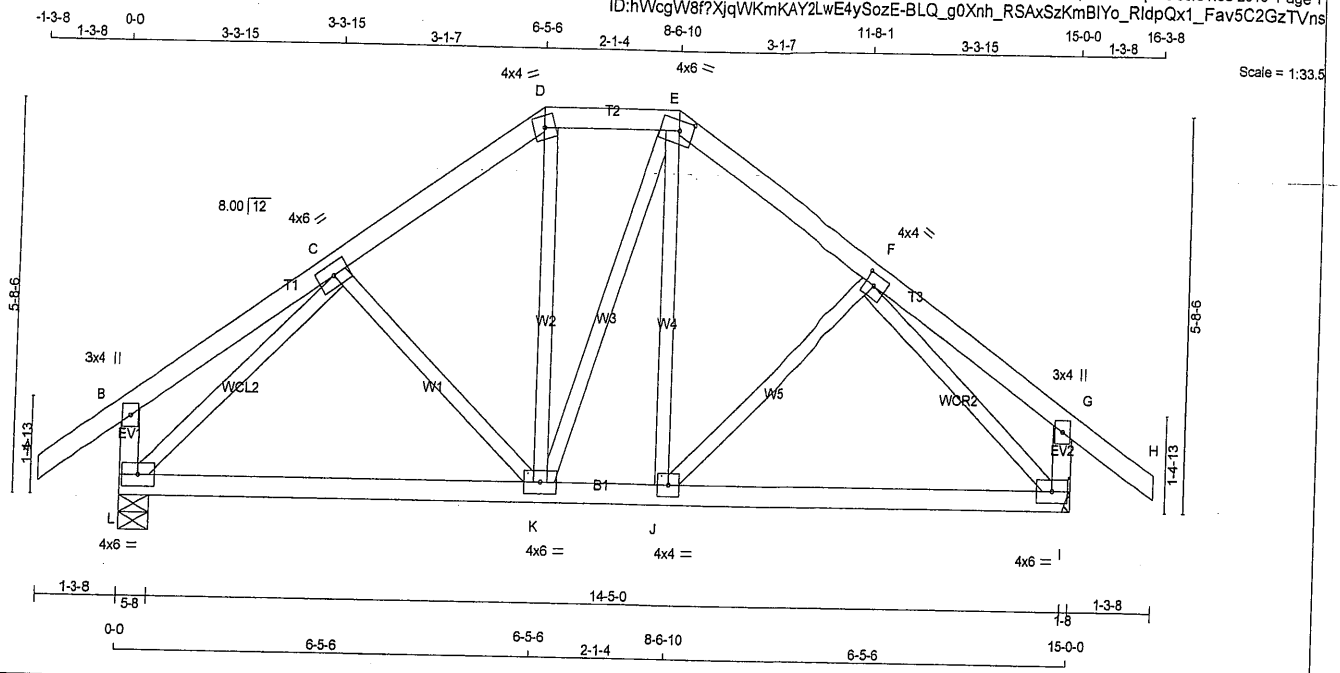

TOTAL WEIGHT = 70 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1208	0	1208	0	0	IN-SX	IN-SX
I	1208	0	1208	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	945	576 / 0	158 / 0	0 / 0	0 / 0	211 / 0	0 / 0
I	945	576 / 0	158 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
B- C	0 / 22	-112.4	-112.4	0.18 (1)	10.00	
C- D	-905 / 0	-112.4	-112.4	0.14 (1)	6.25	
D- E	-737 / 0	-112.4	-112.4	0.07 (1)	6.25	
E- F	-904 / 0	-112.4	-112.4	0.14 (1)	6.25	
F- G	0 / 22	-112.4	-112.4	0.18 (1)	10.00	
G- H	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
L- B	-295 / 0	0.0	0.0	0.03 (1)	7.81	
I- G	-295 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 834	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 736	-28.0	-28.0	0.34 (2)	10.00	
J- I	0 / 833	-28.0	-28.0	0.33 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-141 / 44	0.05 (1)
K- D	0 / 240	0.05 (2)
K- E	0 / 3	0.00 (2)
J- E	0 / 237	0.05 (2)
J- F	-141 / 44	0.05 (1)
L- C	-1181 / 0	0.39 (1)
F- I	-1180 / 0	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.18/1.00 (F-G:1) , BC=0.34/1.00 (K-L:2) , WB=0.39/1.00 (C-L:1) , SSI=0.16/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.42 (F) (INPUT = 1.00)

DWG NO. TAM17765-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T20	1	1	OBC2012_6TCDL TRUSS DESC.	

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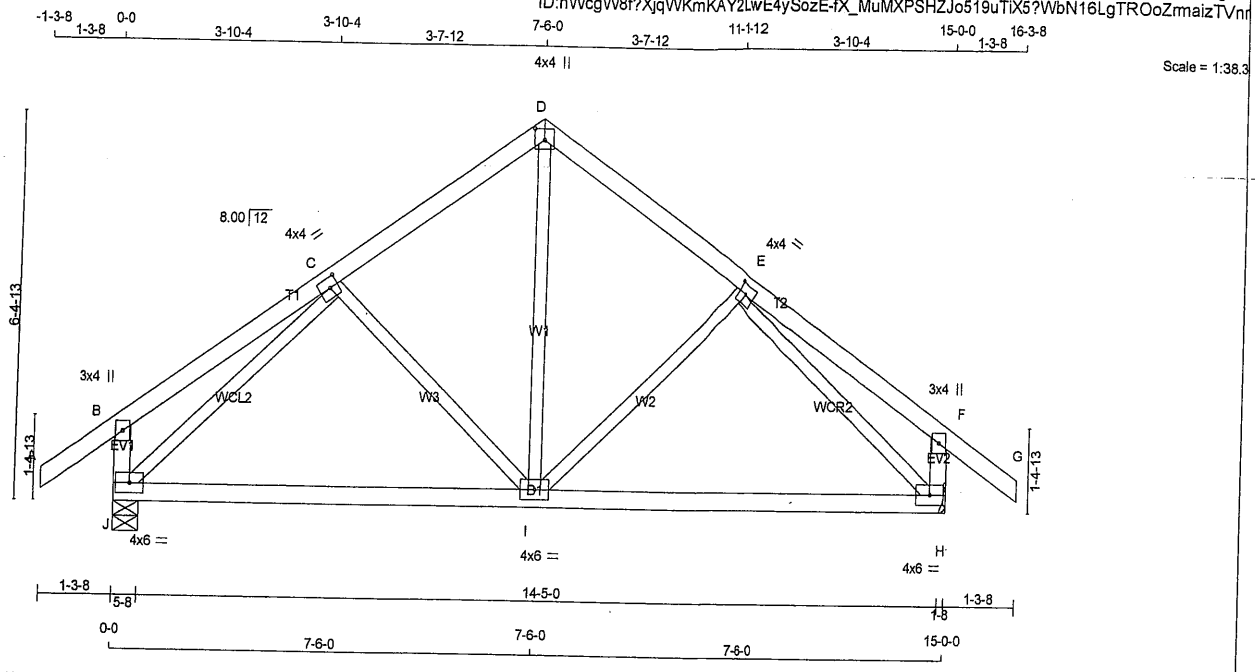

TOTAL WEIGHT = 64 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1208	0	1208	0	0	5-8	5-8
H	1208	0	1208	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	945	576 / 0	158 / 0	0 / 0	0 / 0	211 / 0	0 / 0
H	945	576 / 0	158 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 27	-112.4	-112.4	0.25 (1)		10.00
C- D	-844 / 0	-112.4	-112.4	0.20 (1)		6.25
D- E	-844 / 0	-112.4	-112.4	0.20 (1)		6.25
E- F	0 / 27	-112.4	-112.4	0.25 (1)		10.00
F- G	0 / 43	-112.4	-112.4	0.15 (1)		10.00
J- B	-316 / 0	0.0	0.0	0.03 (1)		7.81
H- F	-316 / 0	0.0	0.0	0.03 (1)		7.81
J- I	0 / 849	-28.0	-28.0	0.55 (3)		10.00
I- H	0 / 849	-28.0	-28.0	0.55 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 580	0.13 (1)
I- E	-232 / 26	0.10 (1)
C- I	-232 / 26	0.10 (1)
J- C	-1174 / 0	0.50 (1)
E- H	-1174 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.55/1.00 (I-J:3) , WB=0.50/1.00 (E-H:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.41 (E) (INPUT = 1.00)

DWG NO. TAM 177662B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T21X	1	2	OBC2012_6TCDL TRUSS DESC.	

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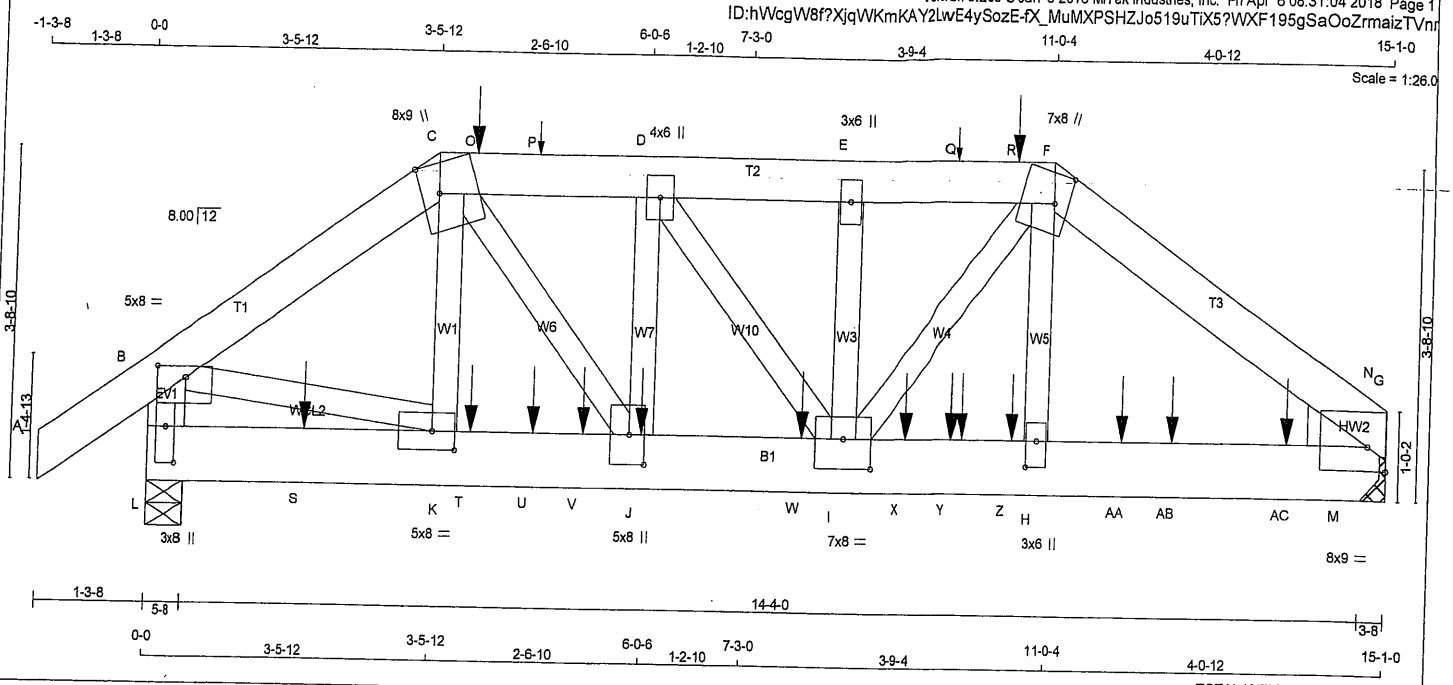


TOTAL WEIGHT = 2 X 99 = 197 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - G	2x8	DRY	1950F 1.7E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	SIDE(122.0)
F- G	2	12	TOP
L- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
J- D	2	3	SIDE(815.4)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	4.25
C	TTWW+m	MT20	8.0	9.0	Edge	2.50
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TTWW+m	MT20	7.0	8.0	4.00	2.00
G	TMBH1-l	MT20	8.0	9.0	3.25	2.50
H	BMW+w	MT20	3.0	6.0	3.50	1.50
I	BMWWW-t	MT20	7.0	8.0	4.00	4.00
J	BMWW+t	MT20	5.0	8.0	4.00	2.25
K	BMWW-t	MT20	5.0	8.0	2.50	3.25
L	BMV1+p	MT20	3.0	8.0	4.75	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
L	5939	0	5939	0	0	5-8	5-8	
G	8428	0	8428	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-9		2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	4579	2802 / 0	814 / 0	0 / 0	0 / 0	1063 / 0	0 / 0
G	6664	3953 / 0	1187 / 0	0 / 0	0 / 0	1524 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

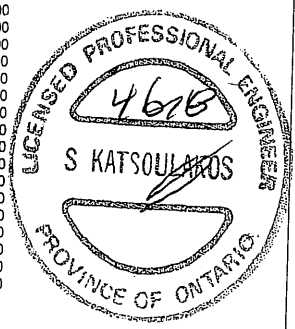
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00	
B- C	-7436 / 0	-112.4	-112.4	0.14 (1)	4.33	
C- O	-9741 / 0	-112.4	-112.4	0.19 (1)	3.79	
O- P	-9741 / 0	-112.4	-112.4	0.19 (1)	3.79	
P- D	-9741 / 0	-112.4	-112.4	0.19 (1)	3.79	
D- E	-10227 / 0	-112.4	-112.4	0.23 (1)	3.67	
E- Q	-10228 / 0	-112.4	-112.4	0.20 (1)	3.69	
Q- R	-10228 / 0	-112.4	-112.4	0.20 (1)	3.69	
R- F	-10228 / 0	-112.4	-112.4	0.20 (1)	3.69	
F- N	-9878 / 0	-112.4	-112.4	0.35 (1)	3.60	
N- G	-14048 / 0	-112.4	-112.4	0.45 (1)	2.89	
L- B	-5909 / 0	0.0	0.0	0.21 (1)	6.07	
L- S	0 / 0	-28.0	-28.0	0.01 (1)	10.00	
S- K	0 / 0	-28.0	-28.0	0.01 (1)	10.00	
K- T	0 / 6126	-28.0	-28.0	0.22 (1)	10.00	
T- U	0 / 6126	-28.0	-28.0	0.22 (1)	10.00	
U- V	0 / 6126	-28.0	-28.0	0.22 (1)	10.00	
V- J	0 / 6126	-35.3	-35.3	0.22 (1)	10.00	
J- W	0 / 9739	-35.3	-35.3	0.32 (1)	10.00	
W- I	0 / 9739	-35.3	-35.3	0.32 (1)	10.00	
I- X	0 / 8245	-35.3	-35.3	0.32 (1)	10.00	
X- Y	0 / 8245	-28.0	-28.0	0.32 (1)	10.00	
Y- Z	0 / 8245	-28.0	-28.0	0.32 (1)	10.00	
Z- H	0 / 8245	-28.0	-28.0	0.32 (1)	10.00	
H-AA	0 / 8122	-28.0	-28.0	0.37 (1)	10.00	
AA-AB	0 / 8122	-28.0	-28.0	0.37 (1)	10.00	
AB-AC	0 / 8122	-28.0	-28.0	0.37 (1)	10.00	
AC- M	0 / 8122	-28.0	-28.0	0.37 (1)	10.00	
M- G	0 / 8122	-28.0	-28.0	0.37 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-904 / 0	0.07 (1)
I- E	-134 / 34	0.01 (1)
I- F	0 / 3136	0.28 (1)
H- F	0 / 2675	0.24 (1)
B- K	0 / 6325	0.56 (1)
C- J	0 / 5801	0.51 (1)
J- D	-756 / 0	0.05 (1)
D- I	0 / 803	0.07 (1)
M- N	0 / 5626	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	6-0-0	-3803	-3803	---	FRONT	VERT	TOTAL
O	3-11-4	-24	-24	---	BACK	VERT	TOTAL
P	4-8-4	1	1	---	BACK	VERT	TOTAL
Q	9-9-12	1	1	---	BACK	VERT	TOTAL



DESIGN CRITERIA

[M]

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 5-3-8
START SPAN CARRIED = 2-6-0
END DISTANCE = 9-2-8
END SPAN CARRIED = 2-6-0
END WALL WIDTH = 3-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.45/1.00 (G-N:1) , BC=0.37/1.00 (G-M:1) , WB=0.56/1.00 (B-K:1) , SSI=0.91/1.00 (G-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 1.00)

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DWG NO. TAM 17767 -18

STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T21X	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 23.9 lbs FACTORED DOWN AT 3-11-4, 1.0 lbs FACTORED DOWN AT 4-8-4, AND 1.0 lbs FACTORED DOWN AT 9-9-12, AND 23.9 lbs FACTORED DOWN AT 10-6-12 ON TOP CHORD, AND 6.3 lbs FACTORED DOWN AT 1-11-4, 6.3 lbs FACTORED DOWN AT 3-11-4, 6.3 lbs FACTORED DOWN AT 4-8-4, 229.8 lbs FACTORED DOWN AT 5-3-8, 3803.4 lbs FACTORED DOWN AT 6-0-0, 1932.2 lbs FACTORED DOWN AT 7-11-4, 229.8 lbs FACTORED DOWN AT 9-2-8, 6.3 lbs FACTORED DOWN AT 9-9-12, 1932.2 lbs FACTORED DOWN AT 9-11-4, 6.3 lbs FACTORED DOWN AT 10-6-12, 1932.2 lbs FACTORED DOWN AT 11-11-4, AND 6.3 lbs FACTORED DOWN AT 12-6-12, AND 1932.2 lbs FACTORED DOWN AT 13-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	10-6-12	-24	-24	—	BACK	VERT	TOTAL
S	1-11-4	-4	-6	—	BACK	VERT	TOTAL
T	3-11-4	-4	-6	—	BACK	VERT	TOTAL
U	4-8-4	-4	-6	—	BACK	VERT	TOTAL
V	5-3-8	-230	-230	—	BACK	VERT	TOTAL
W	7-11-4	-1932	-1932	—	FRONT	VERT	TOTAL
X	9-2-8	-230	-230	—	BACK	VERT	TOTAL
Y	9-9-12	-4	-6	—	BACK	VERT	TOTAL
Y	9-11-4	-1932	-1932	—	FRONT	VERT	TOTAL
Z	10-6-12	-4	-6	—	BACK	VERT	TOTAL
AA	11-11-4	-1932	-1932	—	FRONT	VERT	TOTAL
AB	12-6-12	-4	-6	—	BACK	VERT	TOTAL
AC	13-11-4	-1932	-1932	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
466
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17767-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T22	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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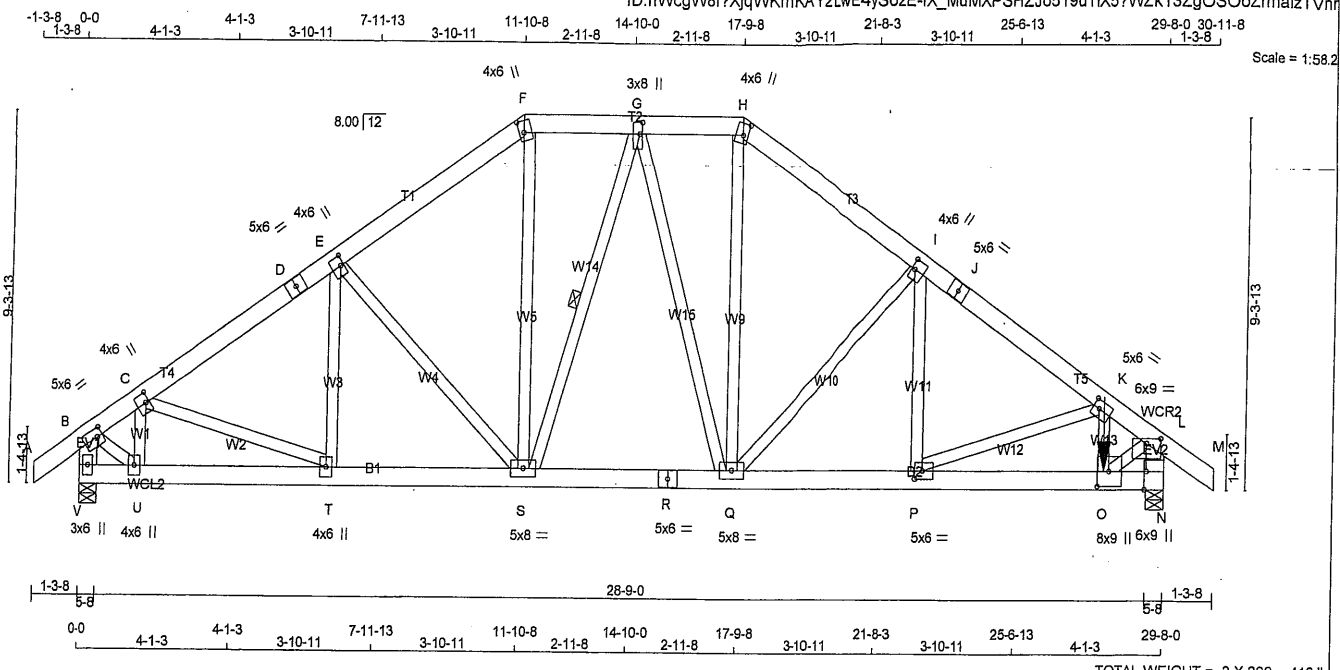


TOTAL WEIGHT = 2 X 209 = 418 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	TOP
H- J	2	12	TOP
J- M	2	12	TOP
V- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- R	2	12	TOP
R- N	3	2	SIDE(2075.4)
WEBS : (0.122"X3") SPIRAL NAILS			
K- O	1	2	SIDE(183.3)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C, E, I						
C	TMWW+t	MT20	4.0	6.0	3.00	1.00
D	TS-t	MT20	5.0	6.0		
F	TTW+m	MT20	4.0	6.0	3.50	1.50
G	TMWW+t	MT20	3.0	8.0	3.50	0.75
H	TTW+m	MT20	4.0	6.0	3.50	1.50
J	TS-t	MT20	5.0	6.0		
K	TMWW-t	MT20	5.0	6.0	2.50	2.00
L	TMVW-p	MT20	6.0	9.0	1.50	4.50
N	BMV1+t	MT20	6.0	9.0	Edge	0.50
O	BMWW+t	MT20	8.0	9.0	4.50	3.75
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2717	0	2717	0	0	5-8	5-8
N	10427	0	10427	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2144	1279 / 0	378 / 0	0 / 0	0 / 0	487 / 0	0 / 0
N	8230	4912 / 0	1452 / 0	0 / 0	0 / 0	1866 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) N (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

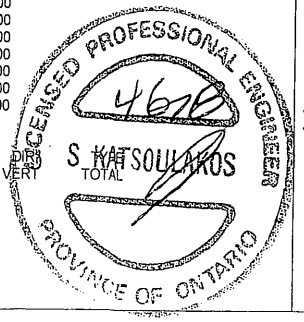
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)		10.00
B- C	-2409 / 0	-112.4	-112.4	0.05 (1)		6.25
C- D	-3155 / 0	-112.4	-112.4	0.13 (1)		6.09
D- E	-3155 / 0	-112.4	-112.4	0.13 (1)		6.09
E- F	-2808 / 0	-112.4	-112.4	0.12 (1)		6.25
F- G	-2324 / 0	-112.4	-112.4	0.06 (1)		6.25
G- H	-2621 / 0	-112.4	-112.4	0.05 (1)		6.25
H- I	-3172 / 0	-112.4	-112.4	0.16 (1)		6.02
I- J	-4835 / 0	-112.4	-112.4	0.19 (1)		5.11
J- K	-4835 / 0	-112.4	-112.4	0.19 (1)		5.11
K- L	-9558 / 0	-112.4	-112.4	0.22 (1)		3.90
L- M	0 / 44	-112.4	-112.4	0.04 (1)		10.00
V- B	-2693 / 0	0.0	0.0	0.10 (1)		7.81
N- L	-10041 / 0	0.0	0.0	0.35 (1)		4.76
V- U	0 / 0	-28.0	-28.0	0.02 (3)		10.00
U- T	0 / 2004	-28.0	-28.0	0.16 (1)		10.00
T- S	0 / 2641	-28.0	-28.0	0.20 (1)		10.00
S- R	0 / 2522	-28.0	-28.0	0.19 (1)		10.00
R- Q	0 / 2522	-28.0	-28.0	0.19 (1)		10.00
Q- P	0 / 4080	-28.0	-28.0	0.33 (1)		10.00
P- O	0 / 7846	-28.0	-28.0	0.73 (1)		10.00
O- N	0 / 0	-28.0	-28.0	0.22 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-1016 / 0	0.06 (1)
C- T	0 / 682	0.06 (1)
T- E	-103 / 153	0.02 (1)
E- S	-486 / 0	0.19 (1)
S- F	0 / 1130	0.10 (1)
Q- H	0 / 1350	0.12 (1)
Q- I	-2146 / 0	0.82 (1)
P- I	0 / 1752	0.15 (1)
P- K	-4030 / 0	0.74 (1)
O- K	0 / 3916	0.35 (1)
B- U	0 / 2263	0.20 (1)
O- L	0 / 8961	0.79 (1)
S- G	-640 / 0	0.14 (1)
G- Q	0 / 335	0.03 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TOTAL
O	28-0-8	-8659	-8659	---	BACK	VERT	



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35/1.00 (L-N:1) , BC=0.73/1.00 (O-P:1) , WB=0.82/1.00 (I-Q:1) , SSI=0.13/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.70 (L) (INPUT = 1.00)

P6 4

DWG NO. TAM17768-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	T22	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW+t	MT20	4.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 8658.6 lbs FACTORED DOWN AT 28-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17768-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296633	T31	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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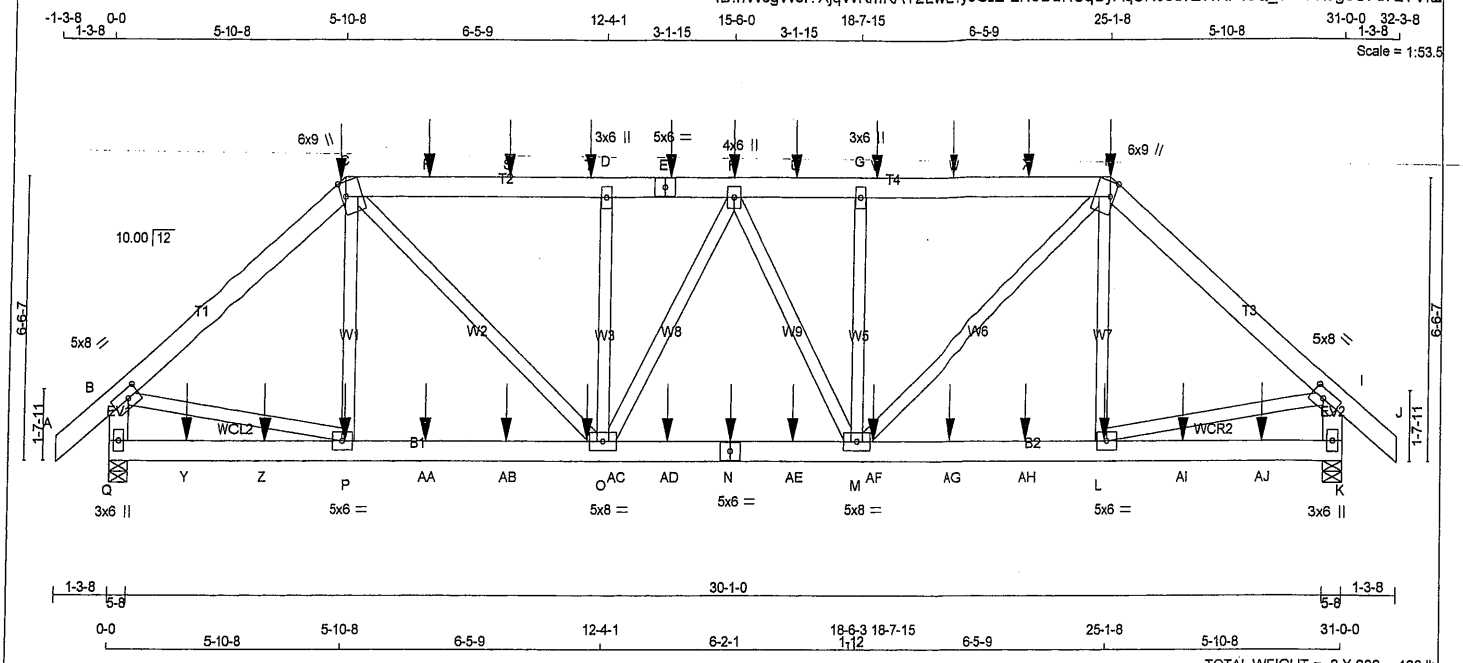

TOTAL WEIGHT = 2 X 200 = 400 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- H	2	12	SIDE(61.0)
H- J	2	12	SIDE(122.0)
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	3935	0	3935	0	0	5-8	5-8
K	3934	0	3934	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	3128	1827 / 0	575 / 0	0 / 0	0 / 0	725 / 0	0 / 0
K	3127	1826 / 0	575 / 0	0 / 0	0 / 0	725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)		10.00
B- C	-4224 / 0	-112.4	-112.4	0.22 (1)		5.35
C- R	-4887 / 0	-112.4	-112.4	0.29 (1)		4.96
R- S	-4887 / 0	-112.4	-112.4	0.29 (1)		4.96
S- T	-4887 / 0	-112.4	-112.4	0.29 (1)		4.96
T- D	-4887 / 0	-112.4	-112.4	0.29 (1)		4.96
D- E	-4888 / 0	-112.4	-112.4	0.26 (1)		4.98
E- F	-4888 / 0	-112.4	-112.4	0.26 (1)		4.98
F- U	-4885 / 0	-112.4	-112.4	0.26 (1)		4.98
U- G	-4885 / 0	-112.4	-112.4	0.26 (1)		4.98
G- V	-4885 / 0	-112.4	-112.4	0.29 (1)		4.96
V- W	-4885 / 0	-112.4	-112.4	0.29 (1)		4.96
W- X	-4885 / 0	-112.4	-112.4	0.29 (1)		4.96
X- H	-4885 / 0	-112.4	-112.4	0.29 (1)		4.96
H- I	-4223 / 0	-112.4	-112.4	0.22 (1)		5.35
I- J	0 / 52	-112.4	-112.4	0.05 (1)		10.00
Q- B	-3806 / 0	0.0	0.0	0.14 (1)		7.24
K- I	-3804 / 0	0.0	0.0	0.14 (1)		7.25
Q- Y	0 / 0	-28.0	-28.0	0.12 (3)		10.00
Y- Z	0 / 0	-28.0	-28.0	0.12 (3)		10.00
Z- P	0 / 0	-28.0	-28.0	0.12 (3)		10.00
P-AA	0 / 3230	-28.0	-28.0	0.28 (1)		10.00
AA-AB	0 / 3230	-28.0	-28.0	0.28 (1)		10.00
AB-AC	0 / 3230	-28.0	-28.0	0.28 (1)		10.00
AC- O	0 / 3230	-28.0	-28.0	0.28 (1)		10.00
O-AD	0 / 4937	-28.0	-28.0	0.38 (1)		10.00
AD- N	0 / 4937	-28.0	-28.0	0.38 (1)		10.00
N-AE	0 / 4937	-28.0	-28.0	0.38 (1)		10.00
AE- M	0 / 4937	-28.0	-28.0	0.38 (1)		10.00
M-AF	0 / 3229	-28.0	-28.0	0.28 (1)		10.00
AF-AG	0 / 3229	-28.0	-28.0	0.28 (1)		10.00
AG-AH	0 / 3229	-28.0	-28.0	0.28 (1)		10.00
AH- L	0 / 3229	-28.0	-28.0	0.28 (1)		10.00
L-AI	0 / 0	-28.0	-28.0	0.12 (3)		10.00
AI-AJ	0 / 0	-28.0	-28.0	0.12 (3)		10.00
AJ- K	0 / 0	-28.0	-28.0	0.12 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
P- C	-231 / 336	0.05 (1)
C- O	0 / 2269	0.20 (1)
O- D	-1150 / 0	0.26 (1)
M- G	-1144 / 0	0.25 (1)
M- H	0 / 2267	0.20 (1)
L- H	-231 / 336	0.05 (1)
B- P	0 / 3291	0.29 (1)
L- I	0 / 3289	0.29 (1)
O- F	-106 / 0	0.03 (1)
F- M	-112 / 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.38/1.00 (M-O:1) , WB=0.29/1.00 (B-P:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.76 (P) (INPUT = 0.90)
SI METAL= 0.43 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4676
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17805-R

STRUCTURAL COMPONENT ONLY.

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296633	T31	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 619.6 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 135.3 lbs FACTORED DOWN AT 15-6-0, 135.3 lbs FACTORED DOWN AT 17-0-12, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, AND 135.3 lbs FACTORED DOWN AT 23-0-12, AND 619.6 lbs FACTORED DOWN AT 25-1-8 ON TOP CHORD, AND 102.9 lbs FACTORED DOWN AT 1-11-4, 88.9 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, 73.3 lbs FACTORED DOWN AT 7-11-4, 73.3 lbs FACTORED DOWN AT 9-11-4, 73.3 lbs FACTORED DOWN AT 11-11-4, 73.3 lbs FACTORED DOWN AT 13-11-4, 73.3 lbs FACTORED DOWN AT 15-6-0, 73.3 lbs FACTORED DOWN AT 17-0-12, 73.3 lbs FACTORED DOWN AT 19-0-12, 73.3 lbs FACTORED DOWN AT 21-0-12, 73.3 lbs FACTORED DOWN AT 23-0-12, 73.3 lbs FACTORED DOWN AT 25-0-12, AND 88.9 lbs FACTORED DOWN AT 27-0-12, AND 102.9 lbs FACTORED DOWN AT 29-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-620	-620	—	FRONT	VERT	TOTAL
E	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
F	15-6-0	-135	-135	—	FRONT	VERT	TOTAL
H	25-1-8	-620	-620	—	FRONT	VERT	TOTAL
L	25-0-12	-42	-73	—	FRONT	VERT	TOTAL
N	15-6-0	-42	-73	—	FRONT	VERT	TOTAL
P	5-11-4	-42	-73	—	FRONT	VERT	TOTAL
R	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
S	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
T	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
U	17-0-12	-135	-135	—	FRONT	VERT	TOTAL
V	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
W	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
X	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
Y	1-11-4	-86	-103	—	FRONT	VERT	TOTAL
Z	3-11-4	-51	-89	—	FRONT	VERT	TOTAL
AA	7-11-4	-42	-73	—	FRONT	VERT	TOTAL
AB	9-11-4	-42	-73	—	FRONT	VERT	TOTAL
AC	11-11-4	-42	-73	—	FRONT	VERT	TOTAL
AD	13-11-4	-42	-73	—	FRONT	VERT	TOTAL
AE	17-0-12	-42	-73	—	FRONT	VERT	TOTAL
AF	19-0-12	-42	-73	—	FRONT	VERT	TOTAL
AG	21-0-12	-42	-73	—	FRONT	VERT	TOTAL
AH	23-0-12	-42	-73	—	FRONT	VERT	TOTAL
AI	27-0-12	-51	-89	—	FRONT	VERT	TOTAL
AJ	29-0-12	-86	-103	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17805-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T31S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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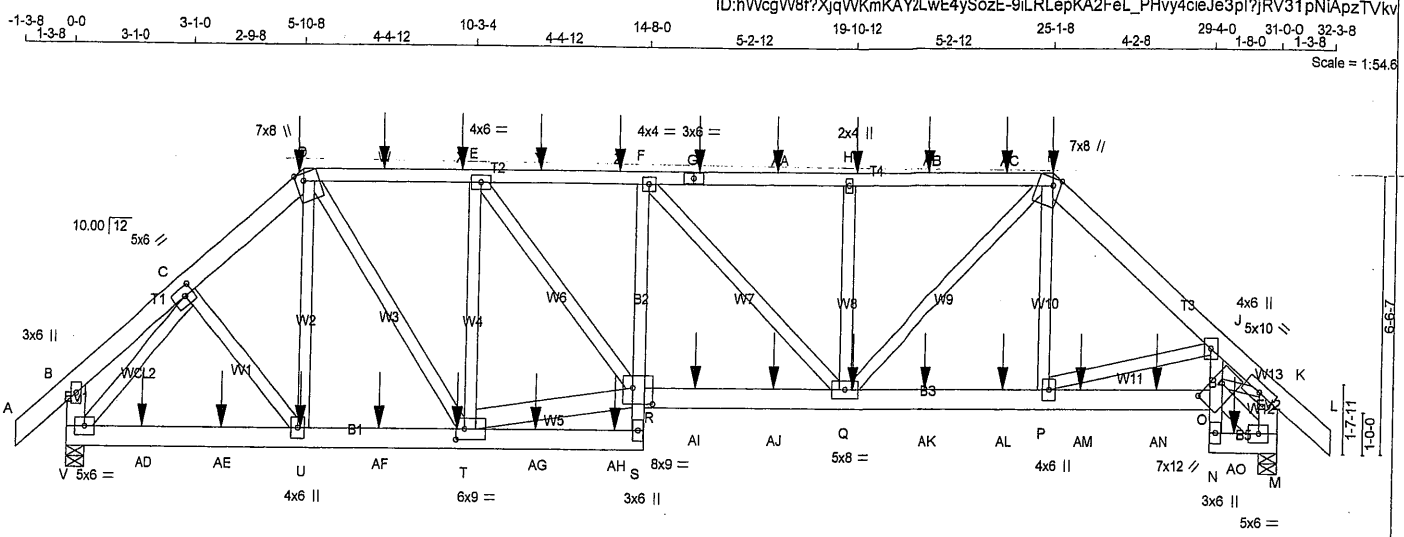
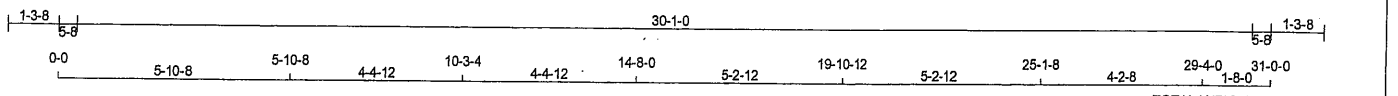




TOTAL WEIGHT = 2 X 204 = 409 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
I- L	2	12	SIDE(122.0)
V- B	2	12	TOP
M- K	2	12	TOP
D- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
R- O	2	12	SIDE(183.1)
N- M	2	12	SIDE(0.0)
S- F	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
P- I	1	6	SIDE(62.9)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4070	0	4070	0	0	5-8	5-8
M	4552	0	4552	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3236	1887 / 0	596 / 0	0 / 0	0 / 0	753 / 0	0 / 0
M	3615	2103 / 0	657 / 0	0 / 0	0 / 0	856 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

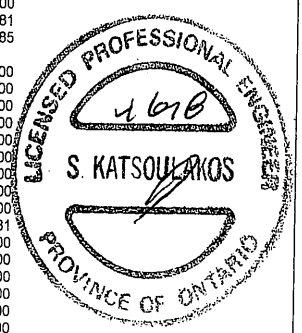
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
C- D	-4405 / 0	-112.4	-112.4	0.08 (1)	5.41
D- W	-4684 / 0	-112.4	-112.4	0.35 (1)	4.11
W- X	-4684 / 0	-112.4	-112.4	0.35 (1)	4.11
X- E	-4684 / 0	-112.4	-112.4	0.35 (1)	4.11
E- Y	-6382 / 0	-112.4	-112.4	0.43 (1)	3.50
Y- Z	-6382 / 0	-112.4	-112.4	0.43 (1)	3.50
Z- F	-6382 / 0	-112.4	-112.4	0.43 (1)	3.50
F- G	-6211 / 0	-112.4	-112.4	0.62 (1)	3.35
G-AA	-6211 / 0	-112.4	-112.4	0.62 (1)	3.35
AA- H	-6211 / 0	-112.4	-112.4	0.62 (1)	3.35
H-AB	-6210 / 0	-112.4	-112.4	0.63 (1)	3.35
AB-AC	-6210 / 0	-112.4	-112.4	0.63 (1)	3.35
AC- I	-6210 / 0	-112.4	-112.4	0.63 (1)	3.35
I- J	-5845 / 0	-112.4	-112.4	0.19 (1)	4.72
J- K	-6849 / 0	-112.4	-112.4	0.13 (1)	4.48
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
V- B	-332 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-4390 / 0	0.0	0.0	0.16 (1)	6.85
V-AD	0 / 2964	-28.0	-28.0	0.26 (1)	10.00
AD-AE	0 / 2964	-28.0	-28.0	0.26 (1)	10.00
AE- U	0 / 2964	-28.0	-28.0	0.26 (1)	10.00
U-AF	0 / 3342	-28.0	-28.0	0.26 (1)	10.00
AF- T	0 / 3342	-28.0	-28.0	0.26 (1)	10.00
T-AG	0 / 94	-28.0	-28.0	0.06 (2)	10.00
AG-AH	0 / 94	-28.0	-28.0	0.06 (2)	10.00
AH- S	0 / 94	-28.0	-28.0	0.06 (2)	10.00
S- R	0 / 184	0.0	0.0	0.08 (1)	10.00
R- F	-601 / 0	0.0	0.0	0.11 (1)	7.81
R-AI	0 / 6401	-28.0	-28.0	0.51 (1)	10.00
AI-AJ	0 / 6401	-28.0	-28.0	0.51 (1)	10.00
AJ- Q	0 / 6401	-28.0	-28.0	0.51 (1)	10.00
Q-AK	0 / 4489	-28.0	-28.0	0.37 (1)	10.00
AK-AL	0 / 4489	-28.0	-28.0	0.37 (1)	10.00
AL- P	0 / 4489	-28.0	-28.0	0.37 (1)	10.00
P-AM	0 / 5329	-28.0	-28.0	0.43 (1)	10.00
AM-AN	0 / 5329	-28.0	-28.0	0.43 (1)	10.00
AN- O	0 / 5329	-28.0	-28.0	0.43 (1)	10.00
N- O	0 / 98	0.0	0.0	0.15 (1)	10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- U	0 / 573	0.05 (1)
U- D	-128 / 248	0.03 (1)
D- T	0 / 2325	0.21 (1)
T- E	-2784 / 0	0.66 (1)
T- R	0 / 4757	0.31 (1)
E- R	0 / 2600	0.23 (1)
F- Q	-267 / 0	0.10 (1)
Q- H	-988 / 0	0.16 (1)
Q- I	0 / 2371	0.21 (1)
P- I	0 / 562	0.05 (2)
P- J	-891 / 0	0.12 (1)
V- C	-4674 / 0	0.53 (1)
O- M	-201 / 0	0.01 (1)
O- K	0 / 5173	0.64 (1)



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.63/1.00 (H-I:1) , BC=0.51/1.00 (Q-R:1) , WB=0.66/1.00 (E-T:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17780 -18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T31S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	10.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-w	MT20	7.0	12.0	2.75	7.50
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BVMWW-l	MT20	8.0	9.0	4.50	6.00
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	9.0	3.00	2.50
U	BMWW+t	MT20	4.0	6.0		
V	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 589.9 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 127.6 lbs FACTORED DOWN AT 15-11-4, 127.6 lbs FACTORED DOWN AT 17-11-4, 127.6 lbs FACTORED DOWN AT 19-11-4, 127.6 lbs FACTORED DOWN AT 21-11-4, AND 127.6 lbs FACTORED DOWN AT 23-11-4, AND 942.6 lbs FACTORED DOWN AT 25-1-8 ON TOP CHORD, AND 87.3 lbs FACTORED DOWN AT 1-11-4, 73.3 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, 73.3 lbs FACTORED DOWN AT 7-11-4, 73.3 lbs FACTORED DOWN AT 9-11-4, 73.3 lbs FACTORED DOWN AT 11-11-4, 83.6 lbs FACTORED DOWN AT 13-11-4, 122.7 lbs FACTORED DOWN AT 15-11-4, 122.7 lbs FACTORED DOWN AT 17-11-4, 122.7 lbs FACTORED DOWN AT 19-11-4, 122.7 lbs FACTORED DOWN AT 21-11-4, 122.7 lbs FACTORED DOWN AT 23-11-4, 122.7 lbs FACTORED DOWN AT 25-11-4, AND 122.7 lbs FACTORED DOWN AT 27-11-4, AND 86.6 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- J	0 / 676	0.0	0.0	0.20 (1)	UNBRAC LENGTH	10.00	
N-AO	0 / 157	-28.0	-28.0	0.02 (2)		10.00	
AO- M	0 / 157	-28.0	-28.0	0.02 (2)		10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-590	-590	—	FRONT	VERT	TOTAL
G	15-11-4	-128	-128	—	FRONT	VERT	TOTAL
H	19-11-4	-128	-128	—	FRONT	VERT	TOTAL
I	25-1-8	-113	-126	—	FRONT	VERT	DEAD
I	25-1-8	-395	-395	—	FRONT	VERT	TOTAL
I	25-1-8	-435	-435	—	FRONT	VERT	SNOW
Q	19-11-4	-123	-123	—	FRONT	VERT	TOTAL
T	9-11-4	-42	-73	—	FRONT	VERT	TOTAL
U	5-11-4	-42	-73	—	FRONT	VERT	TOTAL
W	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
X	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
Y	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
AA	17-11-4	-128	-128	—	FRONT	VERT	TOTAL
AB	21-11-4	-128	-128	—	FRONT	VERT	TOTAL
AC	23-11-4	-128	-128	—	FRONT	VERT	TOTAL
AD	1-11-4	-69	-87	—	FRONT	VERT	TOTAL
AE	3-11-4	-42	-73	—	FRONT	VERT	TOTAL
AF	7-11-4	-42	-73	—	FRONT	VERT	TOTAL
AG	11-11-4	-42	-73	—	FRONT	VERT	TOTAL
AH	13-11-4	-48	-84	—	FRONT	VERT	TOTAL
AI	15-11-4	-123	-123	—	FRONT	VERT	TOTAL
AJ	17-11-4	-123	-123	—	FRONT	VERT	TOTAL
AK	21-11-4	-123	-123	—	FRONT	VERT	TOTAL
AL	23-11-4	-123	-123	—	FRONT	VERT	TOTAL
AM	25-11-4	-123	-123	—	FRONT	VERT	TOTAL
AN	27-11-4	-123	-123	—	FRONT	VERT	TOTAL
AO	29-11-4	-49	-87	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 177890-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296633	T32	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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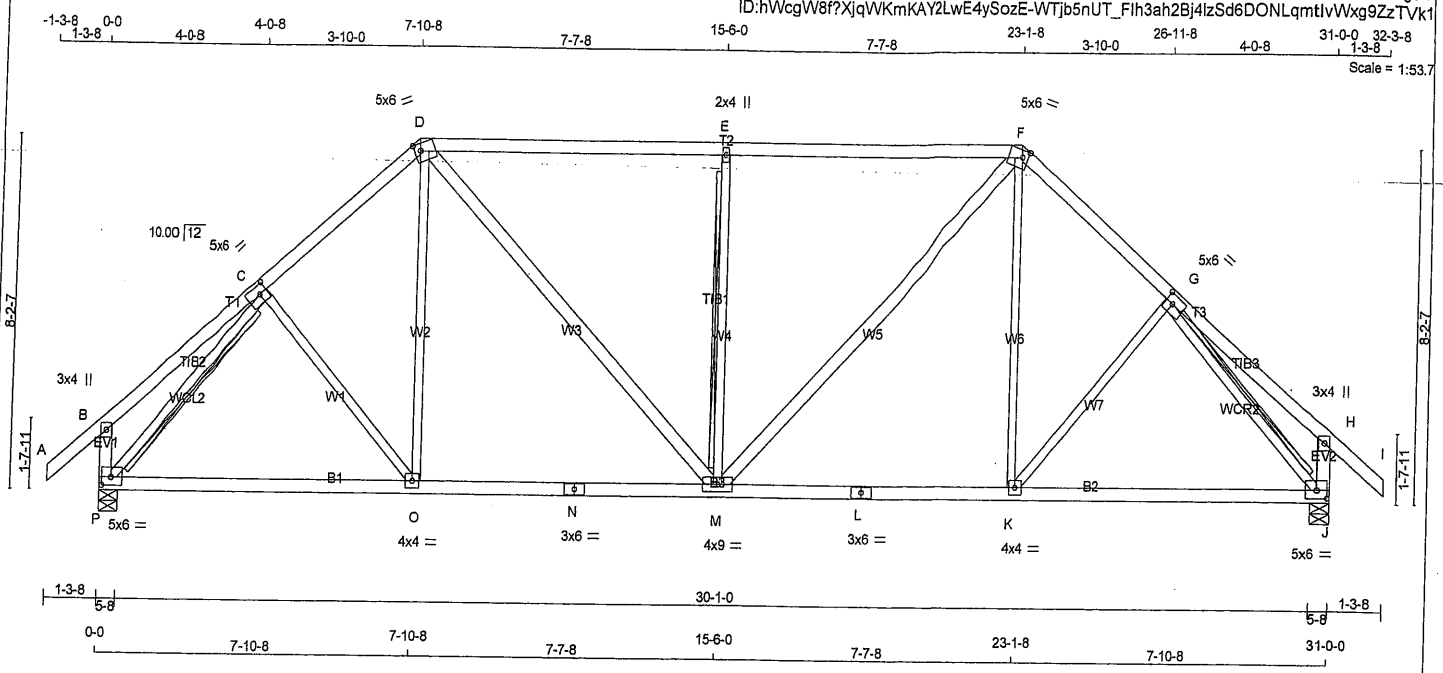


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.25	2.75
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2332	0	2332	0	0	5-8	5-8
J	2332	0	2332	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1842	1097 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0
J	1842	1097 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-M, C-P, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

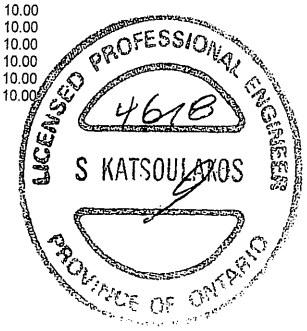
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 31	-112.4	-112.4	0.27 (1)		10.00
C- D	-2174 / 0	-112.4	-112.4	0.27 (1)		4.39
D- E	-2246 / 0	-112.4	-112.4	0.99 (1)		2.73
E- F	-2246 / 0	-112.4	-112.4	0.99 (1)		2.73
F- G	-2174 / 0	-112.4	-112.4	0.27 (1)		4.39
G- H	0 / 31	-112.4	-112.4	0.27 (1)		10.00
H- I	0 / 50	-112.4	-112.4	0.15 (1)		10.00
P- B	-326 / 0	0.0	0.0	0.03 (1)		7.81
J- H	-326 / 0	0.0	0.0	0.03 (1)		7.81
P- O	0 / 1642	-28.0	-28.0	0.61 (2)		10.00
O- N	0 / 1645	-28.0	-28.0	0.62 (2)		10.00
N- M	0 / 1645	-28.0	-28.0	0.62 (2)		10.00
M- L	0 / 1645	-28.0	-28.0	0.62 (2)		10.00
L- K	0 / 1645	-28.0	-28.0	0.62 (2)		10.00
K- J	0 / 1642	-28.0	-28.0	0.61 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-1 / 123	0.03 (3)
O- D	0 / 362	0.08 (3)
D- M	0 / 866	0.14 (1)
M- E	-1055 / 0	0.50 (1)
M- F	0 / 866	0.14 (1)
K- F	0 / 362	0.08 (3)
K- G	-1 / 123	0.03 (3)
P- C	-2524 / 0	0.69 (1)
G- J	-2524 / 0	0.69 (1)



TOTAL WEIGHT = 145 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.99/1.00 (D-E:1) , BC=0.62/1.00 (M-O:2) , WB=0.69/1.00 (G-J:1) , SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 1.00)

DWG NO. TAM 17806-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T32S	1	1	OBC2012_6TCDL TRUSS DESC.	

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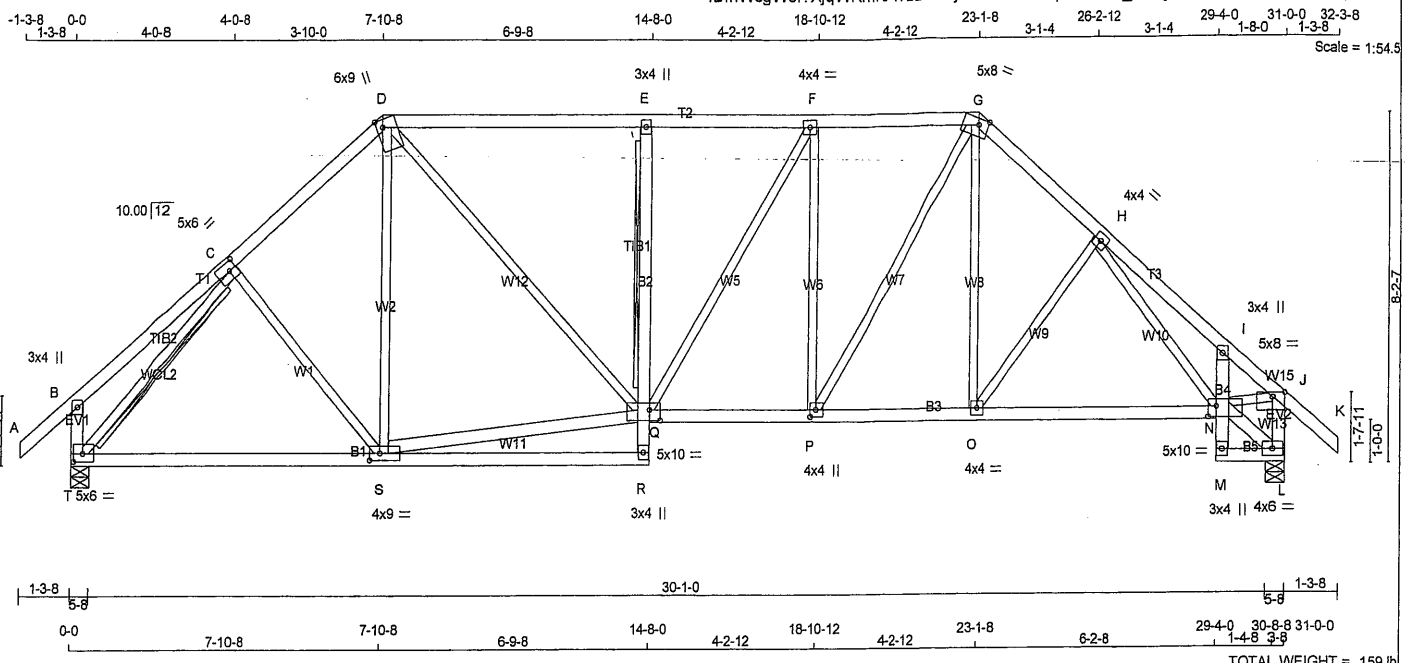


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, E, I						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TMWW-t	MT20	4.0	4.0		
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	5.0	10.0	3.00	2.50
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	4.0	2.00	1.75
Q	BVMWWW-I	MT20	5.0	10.0	3.00	3.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	4.0	9.0	2.00	3.00
T	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2329	0	2329	0	0	5-8	5-8
L	2336	0	2336	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1839	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
L	1844	1099 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-Q
2x4 DRY SPF No.2 T-BRACE AT C-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

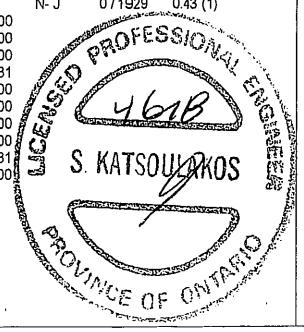
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 32	-112.4	-112.4	0.27 (1)	10.00
C- D	-2172 / 0	-112.4	-112.4	0.27 (1)	4.39
D- E	-2479 / 0	-112.4	-112.4	0.71 (1)	3.55
E- F	-2486 / 0	-112.4	-112.4	0.53 (1)	3.78
F- G	-2346 / 0	-112.4	-112.4	0.32 (1)	4.18
G- H	-2447 / 0	-112.4	-112.4	0.21 (1)	4.24
H- I	-2482 / 0	-112.4	-112.4	0.21 (1)	4.21
I- J	-2512 / 0	-112.4	-112.4	0.18 (1)	4.21
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-326 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-2269 / 0	0.0	0.0	0.24 (1)	5.65
T- S	0 / 1641	-28.0	-28.0	0.62 (2)	10.00
S- R	0 / 44	-28.0	-28.0	0.48 (3)	10.00
R- Q	0 / 113	0.0	0.0	0.07 (1)	10.00
Q- E	-738 / 0	0.0	0.0	0.27 (1)	7.81
Q- P	0 / 2346	-28.0	-28.0	0.44 (1)	10.00
P- O	0 / 1860	-28.0	-28.0	0.45 (2)	10.00
O- N	0 / 2045	-28.0	-28.0	0.47 (2)	10.00
M- N	0 / 37	0.0	0.0	0.10 (1)	10.00
N- I	-168 / 0	0.0	0.0	0.07 (1)	7.81
M- L	0 / 69	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-2 / 123	0.03 (3)
S- D	0 / 239	0.06 (3)
Q- F	0 / 266	0.06 (1)
P- F	-662 / 0	0.60 (1)
P- G	0 / 913	0.21 (1)
O- G	0 / 425	0.10 (2)
O- H	-306 / 0	0.15 (1)
H- N	-197 / 0	0.06 (1)
T- C	-2522 / 0	0.69 (1)
S- Q	0 / 1614	0.26 (1)
D- Q	0 / 1210	0.27 (1)
N- L	-83 / 0	0.01 (1)
N- J	0 / 1929	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.62/1.00 (S-T:2) , WB=0.69/1.00 (C-T:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)

DWG NO. TAM V7281-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296633	T33	1	1	OBC2012_6TCDL TRUSS DESC.	

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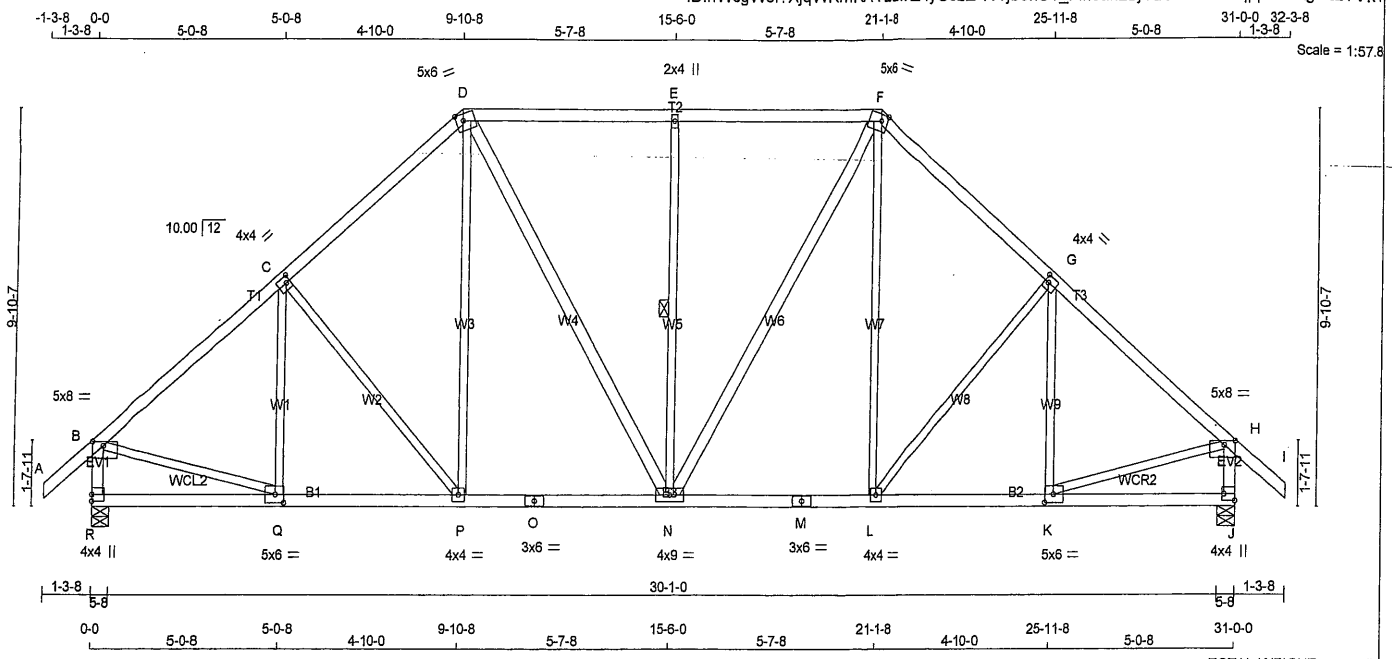


TOTAL WEIGHT = 160 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	1.25	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2332	0	2332	0	0	5-8	5-8
J	2332	0	2332	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1842	1097 / 0	325 / 0	0 / 0	0 / 0	420 / 0	0 / 0
J	1842	1097 / 0	325 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

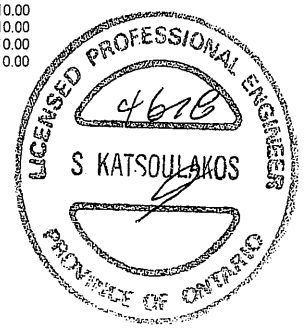
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2221 / 0	-112.4	-112.4	0.45 (1)	4.16
C- D	-2047 / 0	-112.4	-112.4	0.42 (1)	4.32
D- E	-1813 / 0	-112.4	-112.4	0.50 (1)	4.39
E- F	-1813 / 0	-112.4	-112.4	0.50 (1)	4.39
F- G	-2047 / 0	-112.4	-112.4	0.42 (1)	4.32
G- H	-2221 / 0	-112.4	-112.4	0.45 (1)	4.16
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
R- B	-2273 / 0	0.0	0.0	0.24 (1)	5.65
J- H	-2273 / 0	0.0	0.0	0.24 (1)	5.65
R- Q	0 / 0	-28.0	-28.0	0.17 (3)	10.00
Q- P	0 / 1738	-28.0	-28.0	0.36 (1)	10.00
P- O	0 / 1540	-28.0	-28.0	0.35 (2)	10.00
O- N	0 / 1540	-28.0	-28.0	0.35 (2)	10.00
N- M	0 / 1540	-28.0	-28.0	0.35 (2)	10.00
M- L	0 / 1540	-28.0	-28.0	0.35 (2)	10.00
L- K	0 / 1738	-28.0	-28.0	0.36 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-300 / 67	0.16 (1)
C- P	-304 / 0	0.33 (1)
P- D	0 / 394	0.09 (2)
D- N	0 / 540	0.09 (1)
N- E	-771 / 0	0.51 (1)
N- F	0 / 540	0.09 (1)
L- F	0 / 394	0.09 (2)
L- G	-304 / 0	0.33 (1)
K- G	-300 / 67	0.16 (1)
B- Q	0 / 1796	0.40 (1)
K- H	0 / 1796	0.40 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.36/1.00 (P-Q:1) , WB=0.51/1.00 (E-N:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (Q) (INPUT = 0.90)
JSI METAL= 0.46 (O) (INPUT = 1.00)

DWG NO. TAM 17807-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T33S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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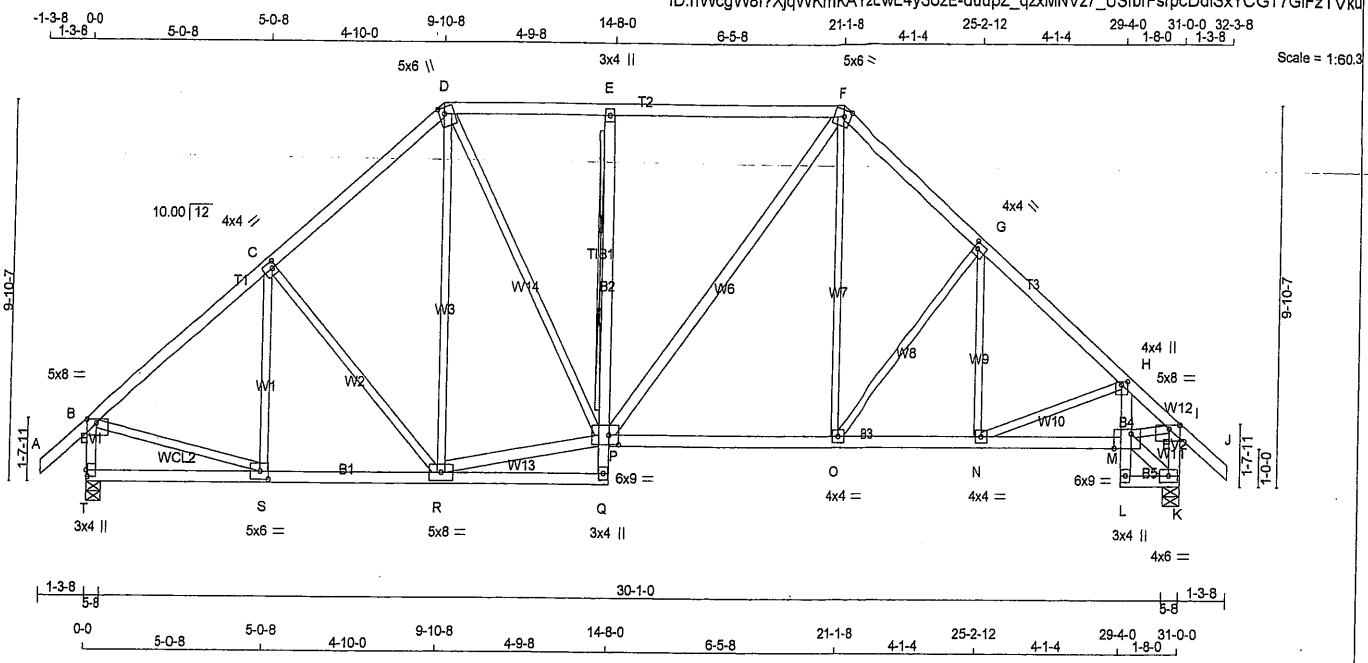


TOTAL WEIGHT = 166 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	TMVW-p	MT20	5.0	8.0	1.25	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	9.0	4.50	5.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	9.0	3.00	3.25
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2329	0	2329	0	0	5-8	5-8
K	2336	0	2336	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1839	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
K	1844	1099 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P

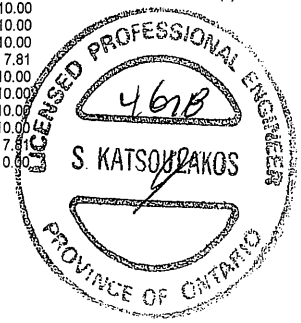
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2217 / 0	-112.4	-112.4	0.45 (1)	4.16
C- D	-2046 / 0	-112.4	-112.4	0.42 (1)	4.32
D- E	-2004 / 0	-112.4	-112.4	0.54 (1)	4.13
E- F	-2007 / 0	-112.4	-112.4	0.64 (1)	3.99
F- G	-2265 / 0	-112.4	-112.4	0.30 (1)	4.28
G- H	-2616 / 0	-112.4	-112.4	0.33 (1)	4.01
H- I	-2813 / 0	-112.4	-112.4	0.19 (1)	4.00
I- J	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-2269 / 0	0.0	0.0	0.24 (1)	5.65
K- I	-2260 / 0	0.0	0.0	0.24 (1)	5.66
T- S	0 / 0	-28.0	-28.0	0.17 (3)	10.00
S- R	0 / 1735	-28.0	-28.0	0.37 (1)	10.00
R- Q	0 / 26	-28.0	-28.0	0.16 (3)	10.00
Q- P	0 / 90	0.0	0.0	0.04 (1)	10.00
P- E	-788 / 0	0.0	0.0	0.19 (1)	7.81
P- O	0 / 1717	-28.0	-28.0	0.44 (2)	10.00
O- N	0 / 2028	-28.0	-28.0	0.48 (2)	10.00
N- M	0 / 2205	-28.0	-28.0	0.43 (1)	10.00
L- M	0 / 37	0.0	0.0	0.12 (1)	10.00
M- H	-91 / 52	0.0	0.0	0.09 (1)	7.81
L- K	0 / 71	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-303 / 63	0.16 (1)
C- R	-302 / 0	0.32 (1)
R- D	0 / 192	0.05 (3)
P- F	0 / 475	0.08 (1)
O- F	0 / 573	0.13 (1)
O- G	-500 / 0	0.40 (1)
N- G	0 / 180	0.04 (3)
N- H	-187 / 0	0.06 (1)
B- S	0 / 1792	0.40 (1)
M- K	-90 / 0	0.01 (1)
M- I	0 / 2154	0.48 (1)
R- P	0 / 1545	0.25 (1)
D- P	0 / 983	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.64/1.00 (E-F:1) , BC=0.48/1.00 (N-O:2) , WB=0.48/1.00 (I-M:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.53 (I) (INPUT = 1.00)

DWG NO. TAM 177782-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T35	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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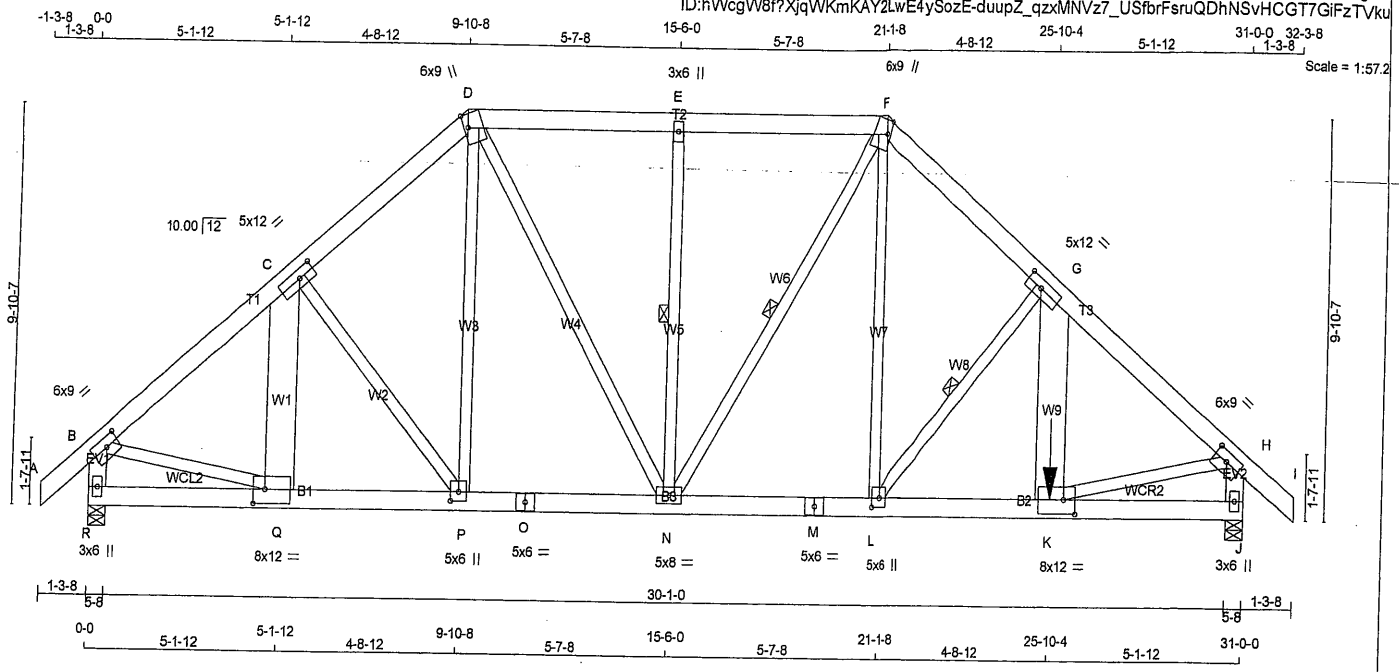


TOTAL WEIGHT = 3 X 244 = 731 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
Q - C	2x10	DRY	No.2	SPF
K - G	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- I	2	12	TOP
R- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	11	TOP
O- M	2	12	TOP
M- J	2	11	SIDE(395.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
G- K	5	4	SIDE(3644.7)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TMWW-t	MT20	5.0	12.0	2.50	5.00
D	TTWW+m	MT20	6.0	9.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TTWW+m	MT20	6.0	9.0	4.00	1.25
G	TMWW-t	MT20	5.0	12.0	2.50	5.00
H	TMVW-t	MT20	6.0	9.0	2.75	4.25
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	8.0	12.0	4.25	3.75
L	BMWW+t	MT20	5.0	6.0	2.75	2.50
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	5.0	6.0	2.75	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	4329	0	4329	0	0	5-8	5-8
J	12342	0	12342	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3418	2037 / 0	603 / 0	0 / 0	0 / 0	778 / 0	0 / 0
J	9745	5805 / 0	1721 / 0	0 / 0	0 / 0	2219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, F-N, G-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

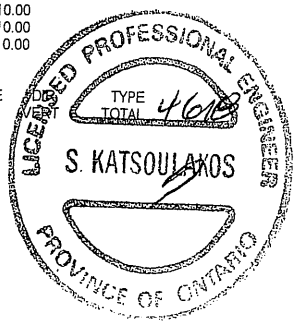
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.03 (1)	10.00
B- C	-4741 / 0	-112.4	-112.4	0.08 (1)	6.16
C- D	-4830 / 0	-112.4	-112.4	0.08 (1)	6.12
D- E	-5124 / 0	-112.4	-112.4	0.10 (1)	5.96
E- F	-5124 / 0	-112.4	-112.4	0.10 (1)	5.96
F- G	-7787 / 0	-112.4	-112.4	0.11 (1)	5.08
G- H	-14531 / 0	-112.4	-112.4	0.22 (1)	3.85
H- I	0 / 52	-112.4	-112.4	0.03 (1)	10.00
R- B	-4248 / 0	0.0	0.0	0.09 (1)	7.81
J- H	-12169 / 0	0.0	0.0	0.27 (1)	5.34
R- Q	0 / 0	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 3669	-28.0	-28.0	0.07 (1)	10.00
P- O	0 / 3671	-28.0	-28.0	0.07 (1)	10.00
O- N	0 / 3671	-28.0	-28.0	0.07 (1)	10.00
N- M	0 / 6055	-28.0	-28.0	0.11 (1)	10.00
M- L	0 / 6055	-28.0	-28.0	0.11 (1)	10.00
L- K	0 / 11163	-28.0	-28.0	0.24 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-692 / 0	0.03 (1)
C- P	-1 / 6	0.00 (3)
P- D	0 / 258	0.01 (3)
D- N	0 / 2825	0.15 (1)
N- E	-705 / 0	0.10 (1)
N- F	-1810 / 0	0.37 (1)
L- F	0 / 6018	0.32 (1)
L- G	-7763 / 0	0.63 (1)
K- G	0 / 9500	0.26 (1)
B- Q	0 / 3754	0.20 (1)
K- H	0 / 11421	0.61 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
K	25-10-4	-11995	-11995	—	BACK	BT	TOTAL 4618



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.27/1.00 (H-J:1) , BC=0.24/1.00 (K-L:1) , WB=0.63/1.00 (G-L:1) , SSI=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.83 (Q) (INPUT = 1.00)

DWG NO. TAM 17703-98
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287688	T35	1	3	TRUSS DESC.	

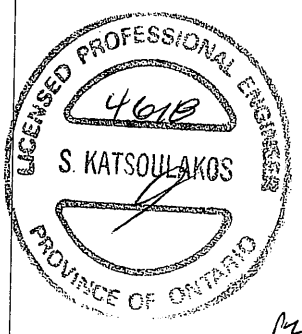
Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	8.0	12.0	4.25	3.75
R	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 11994.9 lbs FACTORED DOWN AT 25-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 17783-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T36	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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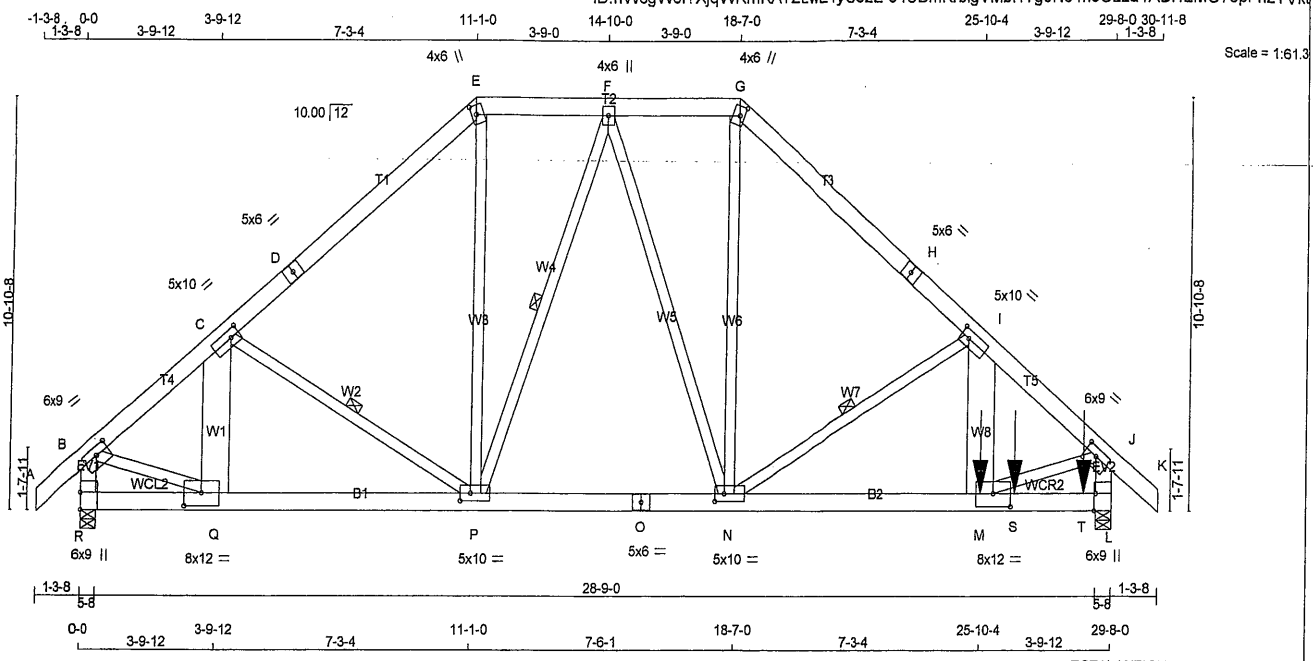

TOTAL WEIGHT = 3 X 226 = 678 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
Q - C	2x10	DRY	No.2	SPF
M - I	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- E	2	12	TOP
E- G	2	12	TOP
G- H	2	12	TOP
H- K	2	12	TOP
R- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	4	TOP
O- L	2	4	SIDE(1087.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
I- M	5	4	SIDE(2499.4)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	4.25
C	TMWW-t	MT20	5.0	10.0	2.50	3.00
D	TS-t	MT20	5.0	6.0		
E	TTW+m	MT20	4.0	6.0	3.00	1.50
F	TMWW+t	MT20	4.0	6.0		
G	TTW+m	MT20	4.0	6.0	3.00	1.50
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	10.0	2.50	3.00
J	TMVW-t	MT20	6.0	9.0	2.50	4.25
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW-t	MT20	8.0	12.0	4.25	6.00
N	BMWWW-t	MT20	5.0	10.0	2.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	3811	0	3811	0	0	5-8	5-8
L	13784	0	13784	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3006	1795 / 0	528 / 0	0 / 0	0 / 0	683 / 0	0 / 0
L	10866	6501 / 0	1898 / 0	0 / 0	0 / 0	2466 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L
BEARING SIZE FACTOR = 1.15 AT JNT(S) R, L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.03 (1)	10.00
B- C	-3981 / 0	-112.4	-112.4	0.12 (1)	6.25
C- D	-4057 / 0	-112.4	-112.4	0.14 (1)	6.25
D- E	-4057 / 0	-112.4	-112.4	0.14 (1)	6.25
E- F	-3104 / 0	-112.4	-112.4	0.05 (1)	6.25
F- G	-4195 / 0	-112.4	-112.4	0.05 (1)	6.25
G- H	-5445 / 0	-112.4	-112.4	0.18 (1)	5.75
H- I	-5445 / 0	-112.4	-112.4	0.18 (1)	5.75
I- J	-14370 / 0	-112.4	-112.4	0.18 (1)	3.89
J- K	0 / 52	-112.4	-112.4	0.03 (1)	10.00
R- B	-3758 / 0	0.0	0.0	0.08 (1)	7.81
L- J	-12765 / 0	0.0	0.0	0.28 (1)	5.23
R- Q	0 / 0	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 3123	-28.0	-28.0	0.07 (1)	10.00
P- O	0 / 3713	-28.0	-28.0	0.08 (1)	10.00
O- N	0 / 3713	-28.0	-28.0	0.08 (1)	10.00
N- M	0 / 11077	-28.0	-28.0	0.20 (1)	10.00
M- S	0 / 0	-28.0	-28.0	0.12 (1)	10.00
S- T	0 / 0	-28.0	-28.0	0.12 (1)	10.00
T- L	0 / 0	-28.0	-28.0	0.12 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-772 / 0	0.02 (1)
C- P	-54 / 0	0.01 (1)
P- E	0 / 2005	0.11 (1)
P- F	-1861 / 0	0.40 (1)
F- N	0 / 1473	0.08 (1)
N- G	0 / 2869	0.15 (1)
N- I	-7998 / 0	0.90 (1)
M- I	0 / 9016	0.25 (1)
B- Q	0 / 3253	0.17 (1)
M- J	0 / 11539	0.62 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR
M	25-10-4	-11356	-11356	—	FRONT	VERT TOTAL
S	26-10-4	-873	-873	—	FRONT	VERT TOTAL
T	28-10-4	-876	-876	—	FRONT	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.28/1.00 (J-L:1) , BC=0.20/1.00 (M-N:1) , WB=0.90/1.00 (I-N:1) , SSI=0.20/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.81 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4616 S. KAFESOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17784-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287688	T36	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	10.0	2.50	3.50
Q	BMWW-t	MT20	8.0	12.0	4.25	6.00
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 11356.4 lbs FACTORED DOWN AT 25-10-4, AND 873.1 lbs FACTORED DOWN AT 26-10-4, AND 875.7 lbs FACTORED DOWN AT 28-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17784-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T37A	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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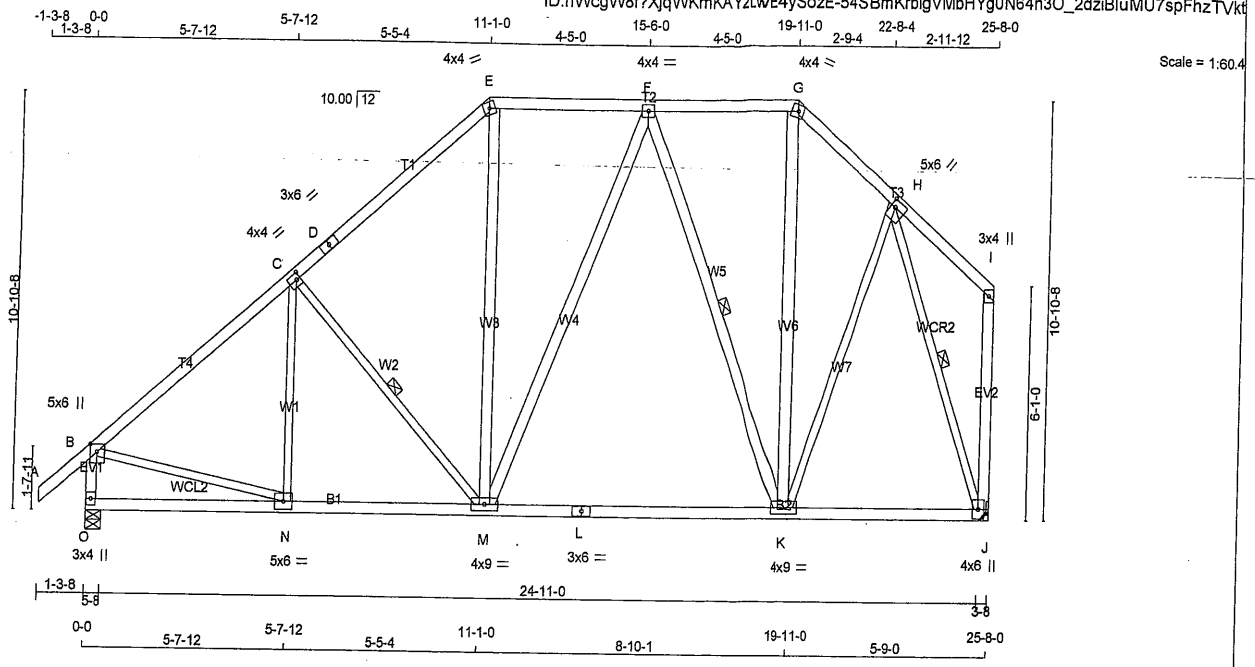


TOTAL WEIGHT = 3 X 152 = 457 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
N - C	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF
B - N	2x3	DRY	No.2	SPF
H - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW+t	MT20	5.0	6.0	2.25	1.50
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1958	0	1958	0	0	5-8	5-8
J	1802	0	1802	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1543	924 / 0	270 / 0	0 / 0	0 / 0	350 / 0	0 / 0
J	1437	834 / 0	270 / 0	0 / 0	0 / 0	334 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-K, H-J.

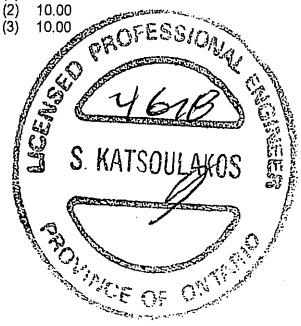
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	-1776 / 0	-112.4	-112.4	0.53 (1)		4.43
C- D	-1460 / 0	-112.4	-112.4	0.49 (1)		4.83
D- E	-1460 / 0	-112.4	-112.4	0.49 (1)		4.83
E- F	-1088 / 0	-112.4	-112.4	0.29 (1)		5.71
F- G	-787 / 0	-112.4	-112.4	0.28 (1)		6.25
G- H	-1043 / 0	-112.4	-112.4	0.11 (1)		6.04
H- I	0 / 22	-112.4	-112.4	0.13 (1)		10.00
O- B	-1890 / 0	0.0	0.0	0.20 (1)		6.10
J- I	-124 / 0	0.0	0.0	0.10 (1)		7.81
O- N	0 / 0	-28.0	-28.0	0.20 (3)		10.00
N- M	0 / 1401	-28.0	-28.0	0.47 (2)		10.00
M- L	0 / 1060	-28.0	-28.0	0.49 (2)		10.00
L- K	0 / 1060	-28.0	-28.0	0.49 (2)		10.00
K- J	0 / 563	-28.0	-28.0	0.44 (3)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI
N- C	-168 / 132	0.11 (1)
C- M	-470 / 0	0.21 (1)
M- E	0 / 483	0.08 (1)
M- F	0 / 138	0.02 (3)
F- K	-729 / 0	0.53 (1)
K- G	0 / 332	0.05 (1)
K- H	0 / 615	0.14 (1)
B- N	0 / 1438	0.32 (1)
H- J	-1717 / 0	0.86 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.49/1.00 (K-M:2) , WB=0.86/1.00 (H-J:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.50 (L) (INPUT = 1.00)

DWG NO. TAM 17785-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T38	6	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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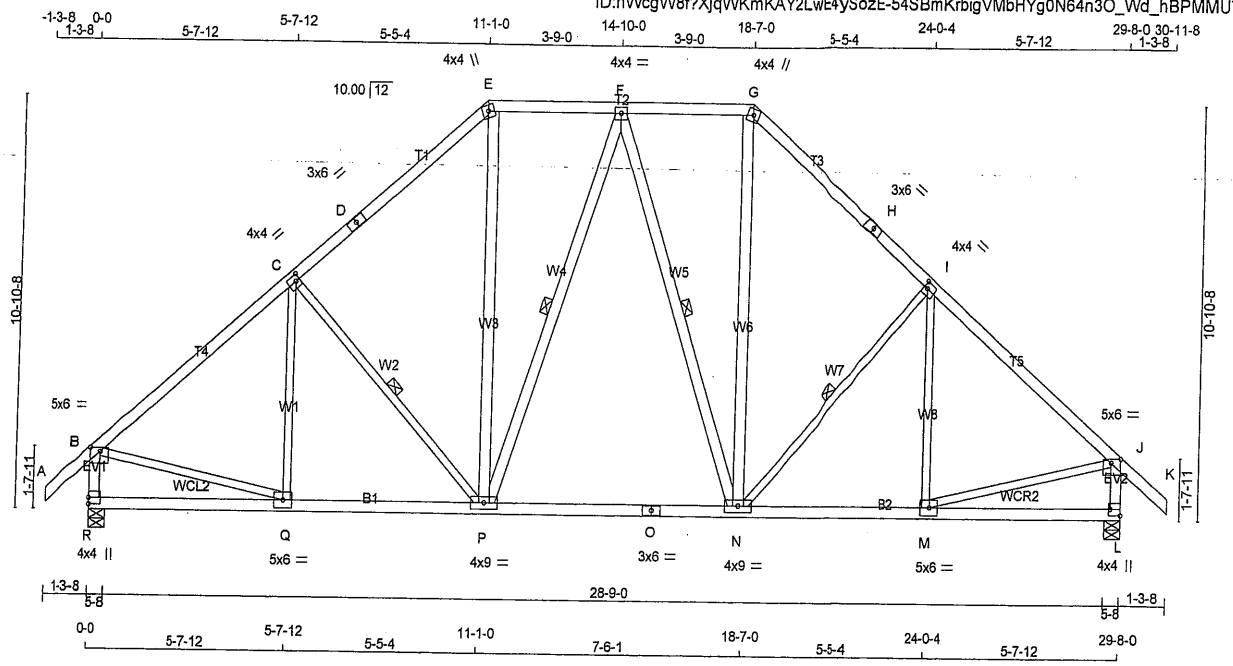

TOTAL WEIGHT = 6 X 162 = 970 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW+m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW+m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.25	3.25
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2239	0	2239	0	0	5-8	5-8
L	2239	0	2239	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1767	1053 / 0	311 / 0	0 / 0	0 / 0	402 / 0	0 / 0
L	1767	1053 / 0	311 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
B- C	-2121 / 0	-112.4	-112.4	0.56 (1)	4.09	
C- D	-1841 / 0	-112.4	-112.4	0.52 (1)	4.37	
D- E	-1841 / 0	-112.4	-112.4	0.52 (1)	4.37	
E- F	-1383 / 0	-112.4	-112.4	0.21 (1)	5.32	
F- G	-1383 / 0	-112.4	-112.4	0.21 (1)	5.32	
G- H	-1841 / 0	-112.4	-112.4	0.52 (1)	4.37	
H- I	-1841 / 0	-112.4	-112.4	0.52 (1)	4.37	
I- J	-2121 / 0	-112.4	-112.4	0.56 (1)	4.09	
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
R- B	-2172 / 0	0.0	0.0	0.23 (1)	5.76	
L- J	-2172 / 0	0.0	0.0	0.23 (1)	5.76	
R- Q	0 / 0	-28.0	-28.0	0.20 (3)	10.00	
Q- P	0 / 1666	-28.0	-28.0	0.43 (2)	10.00	
P- O	0 / 1469	-28.0	-28.0	0.40 (2)	10.00	
O- N	0 / 1469	-28.0	-28.0	0.40 (2)	10.00	
N- M	0 / 1666	-28.0	-28.0	0.43 (2)	10.00	
M- L	0 / 0	-28.0	-28.0	0.20 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-222 / 118	0.14 (1)
C- P	-430 / 0	0.19 (1)
P- E	0 / 750	0.12 (1)
P- F	-264 / 0	0.18 (1)
F- N	-264 / 0	0.18 (1)
N- G	0 / 750	0.12 (1)
N- I	-430 / 0	0.19 (1)
M- I	-222 / 118	0.14 (1)
B- Q	0 / 1710	0.38 (1)
M- J	0 / 1710	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.43/1.00 (P-Q:2) , WB=0.38/1.00 (B-Q:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.51 (O) (INPUT = 1.00)

DWG NO. TAM17286-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T38A	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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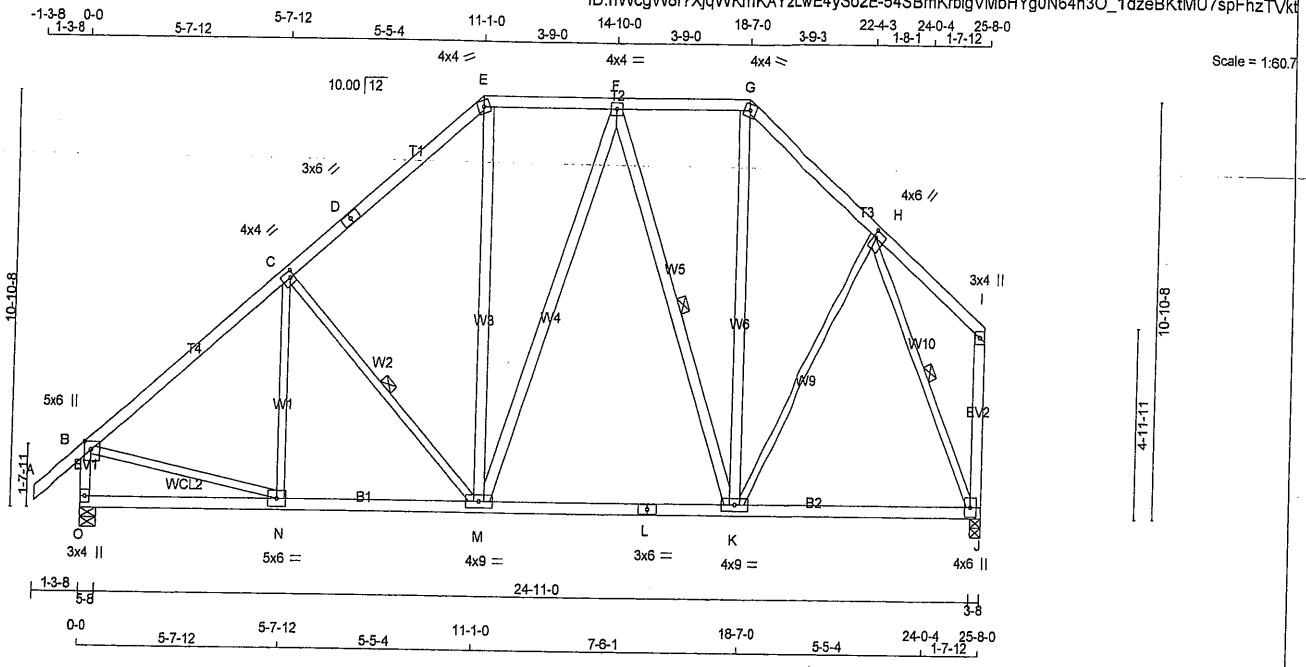


TOTAL WEIGHT = 7 X 150 = 1052 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
N - C	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
B - N	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF
H - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW+t	MT20	4.0	6.0	2.25	1.00
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1958	0	1958	0	0	5-8 IN-SX	5-8 IN-SX
J	1802	0	1802	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1543	924 / 0	269 / 0	0 / 0	0 / 0	350 / 0	0 / 0
J	1437	834 / 0	269 / 0	0 / 0	0 / 0	334 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-K, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

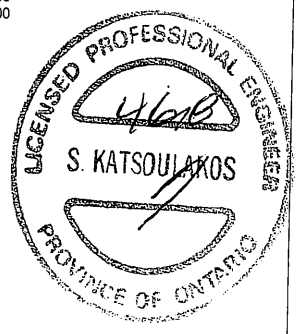
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1779 / 0	-112.4	-112.4	0.53 (1)	4.42
C- D	-1454 / 0	-112.4	-112.4	0.49 (1)	4.83
D- E	-1454 / 0	-112.4	-112.4	0.49 (1)	4.83
E- F	-1083 / 0	-112.4	-112.4	0.21 (1)	5.83
F- G	-898 / 0	-112.4	-112.4	0.20 (1)	6.25
G- H	-1192 / 0	-112.4	-112.4	0.21 (1)	5.63
H- I	0 / 33	-112.4	-112.4	0.23 (1)	10.00
O- B	-1892 / 0	0.0	0.0	0.20 (1)	6.10
J- I	-126 / 0	0.0	0.0	0.06 (1)	7.81
O- N	0 / 0	-28.0	-28.0	0.21 (2)	10.00
N- M	0 / 1403	-28.0	-28.0	0.36 (2)	10.00
M- L	0 / 1078	-28.0	-28.0	0.49 (2)	10.00
L- K	0 / 1078	-28.0	-28.0	0.49 (2)	10.00
K- J	0 / 694	-28.0	-28.0	0.45 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-156 / 154	0.10 (1)
C- M	-480 / 0	0.22 (1)
M- E	0 / 506	0.08 (1)
M- F	0 / 84	0.01 (3)
F- K	-555 / 0	0.38 (1)
K- G	0 / 404	0.06 (1)
B- N	0 / 1441	0.32 (1)
K- H	0 / 407	0.09 (1)
H- J	-1741 / 0	0.74 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.49/1.00 (K-M:2) , WB=0.74/1.00 (H-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.51 (H) (INPUT = 1.00)

DWG NO. TAM 172878
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T39X	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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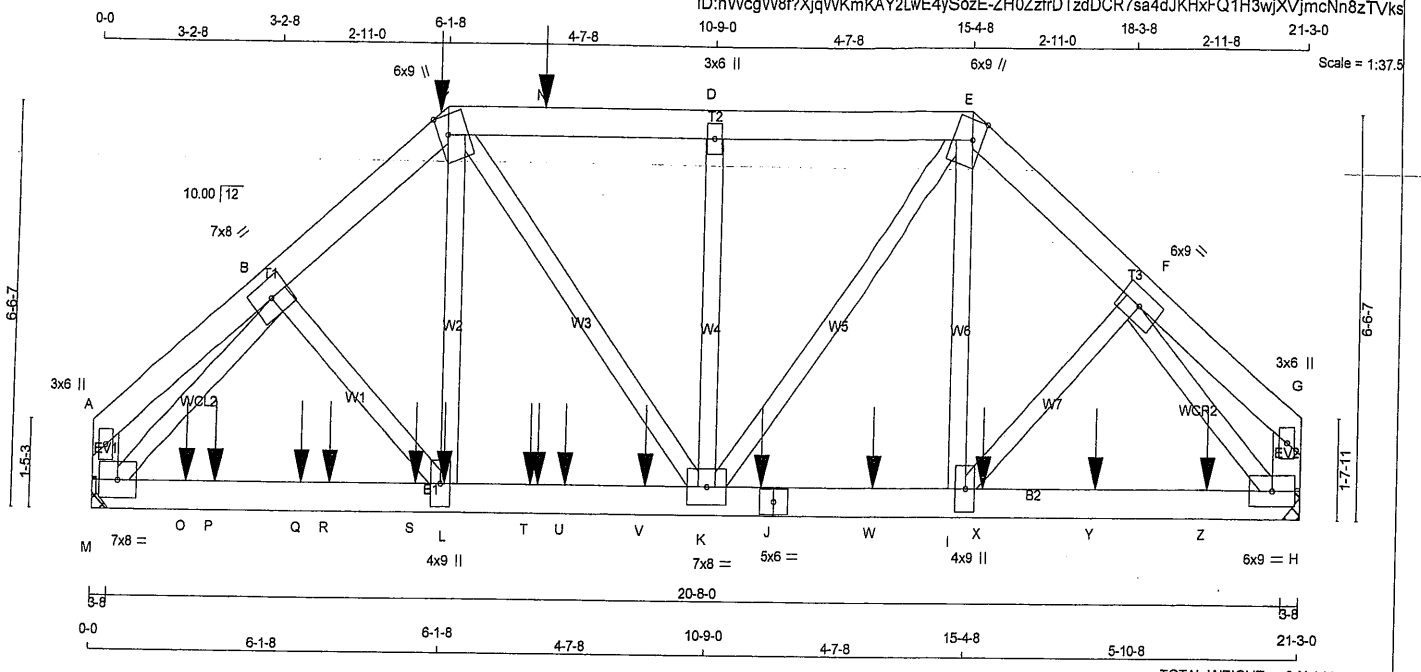

TOTAL WEIGHT = 3 X 141 = 422 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(183.1)
C- E	2	12	SIDE(91.5)
E- G	2	12	TOP
M- A	2	12	TOP
H- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	7	SIDE(925.7)
J- H	2	7	SIDE(694.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW-t	MT20	7.0	8.0		
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	9.0	Edge	2.00
F	TMWW-t	MT20	6.0	9.0		
G	TMV+p	MT20	3.0	6.0		
H	BMVW1-t	MT20	6.0	9.0		
I	BMWW+t	MT20	4.0	9.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	7.0	8.0		
L	BMWW+t	MT20	4.0	9.0		
M	BMVW1-t	MT20	7.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	12023	0	12023	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-9	
H	11384	0	11384	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	9507	5641 / 0	1694 / 0	0 / 0	0 / 0	2171 / 0	0 / 0
H	8994	5349 / 0	1594 / 0	0 / 0	0 / 0	2051 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-78 / 0	-112.4	-112.4	0.08 (1)	6.25	B- L	0 / 1871	0.10 (1)
B- C	-13138 / 0	-112.4	-112.4	0.17 (1)	4.04	L- C	0 / 5733	0.31 (1)
C- N	-11390 / 0	-112.4	-112.4	0.15 (1)	4.32	C- K	0 / 1901	0.10 (1)
N- D	-11390 / 0	-112.4	-112.4	0.15 (1)	4.32	K- D	-459 / 13	0.07 (1)
D- E	-11390 / 0	-112.4	-112.4	0.12 (1)	4.35	K- E	0 / 3674	0.20 (1)
E- F	-11790 / 0	-112.4	-112.4	0.14 (1)	4.26	I- E	0 / 4142	0.22 (1)
F- G	-72 / 0	-112.4	-112.4	0.07 (1)	6.25	I- F	0 / 2178	0.12 (1)
M- A	-291 / 0	0.0	0.0	0.01 (1)	7.81	M- B	-13309 / 0	0.96 (1)
H- G	-268 / 0	0.0	0.0	0.01 (1)	7.81	F- H	-12011 / 0	0.81 (1)
M- O	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
O- P	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
P- Q	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
Q- R	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
R- S	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
S- L	0 / 8803	-28.0	-28.0	0.59 (1)	10.00			
L- T	0 / 10234	-28.0	-28.0	0.61 (1)	10.00			
T- U	0 / 10234	-28.0	-28.0	0.61 (1)	10.00			
U- V	0 / 10234	-28.0	-28.0	0.61 (1)	10.00			
V- K	0 / 10234	-28.0	-28.0	0.61 (1)	10.00			
K- J	0 / 9157	-28.0	-28.0	0.50 (1)	10.00			
J- W	0 / 9157	-28.0	-28.0	0.50 (1)	10.00			
W- I	0 / 9157	-28.0	-28.0	0.50 (1)	10.00			
I- X	0 / 7552	-28.0	-28.0	0.47 (1)	10.00			
X- Y	0 / 7552	-28.0	-28.0	0.47 (1)	10.00			
Y- Z	0 / 7552	-28.0	-28.0	0.47 (1)	10.00			
Z- H	0 / 7552	-28.0	-28.0	0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-1-8	-590	-590	---	FRONT	VERT	TOTAL
J	11-8-4	-1774	-1774	---	BACK	VERT	TOTAL
L	6-2-4	-42	-73	---	FRONT	VERT	TOTAL
N	7-9-12	-135	-135	---	FRONT	VERT	TOTAL
O	1-8-4	-1774	-1774	---	BACK	VERT	TOTAL
P	2-2-4	-69	-87	---	FRONT	VERT	TOTAL
Q	3-8-4	-1774	-1774	---	BACK	VERT	TOTAL
R	4-2-4	-42	-73	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.61/1.00 (K-L:1) , WB=0.96/1.00 (B-M:1) , SSI=0.78/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (F) (INPUT = 0.90)
MSI METAL= 0.83 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4618 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17788-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 589.9 lbs FACTORED DOWN AT 6-1-8, AND 135.3 lbs FACTORED DOWN AT 7-9-12 ON TOP CHORD, AND 1774.2 lbs FACTORED DOWN AT 1-8-4, 87.3 lbs FACTORED DOWN AT 2-2-4, 1774.2 lbs FACTORED DOWN AT 3-8-4, 73.3 lbs FACTORED DOWN AT 4-2-4, 1774.2 lbs FACTORED DOWN AT 5-8-4, 73.3 lbs FACTORED DOWN AT 6-2-4, 1774.2 lbs FACTORED DOWN AT 7-8-4, 73.3 lbs FACTORED DOWN AT 7-9-12, 1761.9 lbs FACTORED DOWN AT 8-3-8, 1774.2 lbs FACTORED DOWN AT 9-8-4, 1774.2 lbs FACTORED DOWN AT 11-8-4, 1774.2 lbs FACTORED DOWN AT 13-8-4, 1774.2 lbs FACTORED DOWN AT 15-8-4, AND 1774.2 lbs FACTORED DOWN AT 17-8-4, AND 1774.2 lbs FACTORED DOWN AT 19-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	5-8-4	-1774	-1774	—	BACK	VERT	TOTAL
T	7-8-4	-1774	-1774	—	BACK	VERT	TOTAL
T	7-9-12	-42	-73	—	FRONT	VERT	TOTAL
U	8-3-8	-1762	-1762	—	FRONT	VERT	TOTAL
V	9-8-4	-1774	-1774	—	BACK	VERT	TOTAL
W	13-8-4	-1774	-1774	—	BACK	VERT	TOTAL
X	15-8-4	-1774	-1774	—	BACK	VERT	TOTAL
Y	17-8-4	-1774	-1774	—	BACK	VERT	TOTAL
Z	19-8-4	-1774	-1774	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17788-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T40	2	1	OBC2012_6TCDL TRUSS DESC.	

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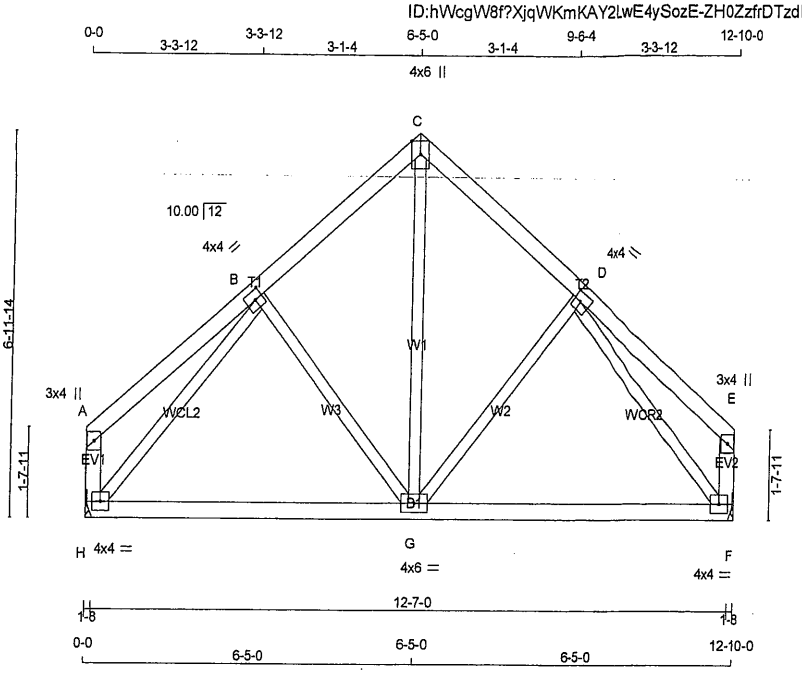

TOTAL WEIGHT = 2 X 58 = 115 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	901	0	901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	901	0	901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	718	417 / 0	135 / 0	0 / 0	0 / 0	167 / 0	0 / 0
F	718	417 / 0	135 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-112.4	-112.4	0.18 (1)	10.00	G- C	0 / 474	0.11 (1)
B- C	-618 / 0	-112.4	-112.4	0.14 (1)	6.25	G- D	-163 / 26	0.07 (1)
C- D	-618 / 0	-112.4	-112.4	0.14 (1)	6.25	B- G	-163 / 26	0.07 (1)
D- E	0 / 26	-112.4	-112.4	0.18 (1)	10.00	H- B	-891 / 0	0.37 (1)
H- A	-138 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-891 / 0	0.37 (1)
F- E	-138 / 0	0.0	0.0	0.01 (1)	7.81			
H- G	0 / 558	-28.0	-28.0	0.39 (3)	10.00			
G- F	0 / 558	-28.0	-28.0	0.39 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD .

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.39/1.00 (G-H:3) , WB=0.37/1.00 (B-H:1) , SSI=0.16/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.33 (D) (INPUT = 1.00)

DWG NO. TAM 17789-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T40S	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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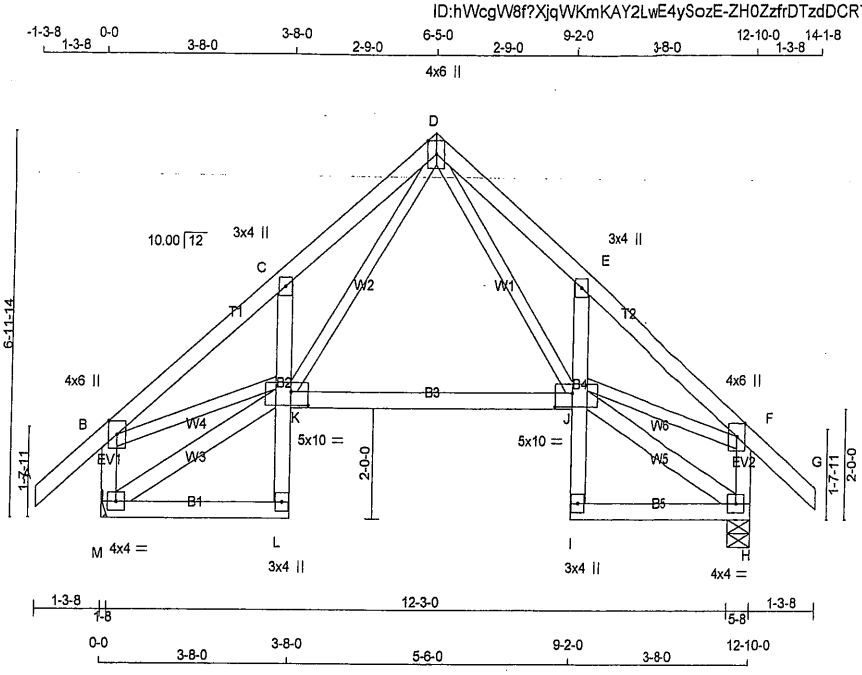

TOTAL WEIGHT = 2 X 72 = 144 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	10.0	3.00	4.00
K	BVMWWW-I	MT20	5.0	10.0	3.00	4.00
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1057	0	1057	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
H	1057	0	1057	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	825	507 / 0	135 / 0	0 / 0	0 / 0	183 / 0	0 / 0
H	825	507 / 0	135 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	D- J	0 / 892	0.20 (1)
B- C	-1375 / 0	-112.4	-112.4	0.17 (1)	5.38	K- D	0 / 892	0.20 (1)
C- D	-1413 / 0	-112.4	-112.4	0.14 (1)	5.36	M- K	-18 / 0	0.00 (1)
D- E	-1413 / 0	-112.4	-112.4	0.14 (1)	5.36	B- K	0 / 1093	0.25 (1)
E- F	-1375 / 0	-112.4	-112.4	0.17 (1)	5.38	J- H	-18 / 0	0.00 (1)
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00	J- F	0 / 1093	0.25 (1)
M- B	-999 / 0	0.0	0.0	0.11 (1)	7.81			
H- F	-999 / 0	0.0	0.0	0.11 (1)	7.81			
M- L	0 / 15	-28.0	-28.0	0.11 (3)	10.00			
L- K	0 / 86	0.0	0.0	0.05 (1)	10.00			
K- C	-424 / 0	0.0	0.0	0.04 (1)	7.81			
K- J	0 / 596	-28.0	-28.0	0.34 (3)	10.00			
I- J	0 / 86	0.0	0.0	0.05 (1)	10.00			
J- E	-424 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	0 / 15	-28.0	-28.0	0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL (TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 915 (0.17")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.34/1.00 (J-K:3) , WB=0.25/1.00 (F-J:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.34 (F) (INPUT = 1.00)

DWG NO. TAM 17790-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T41	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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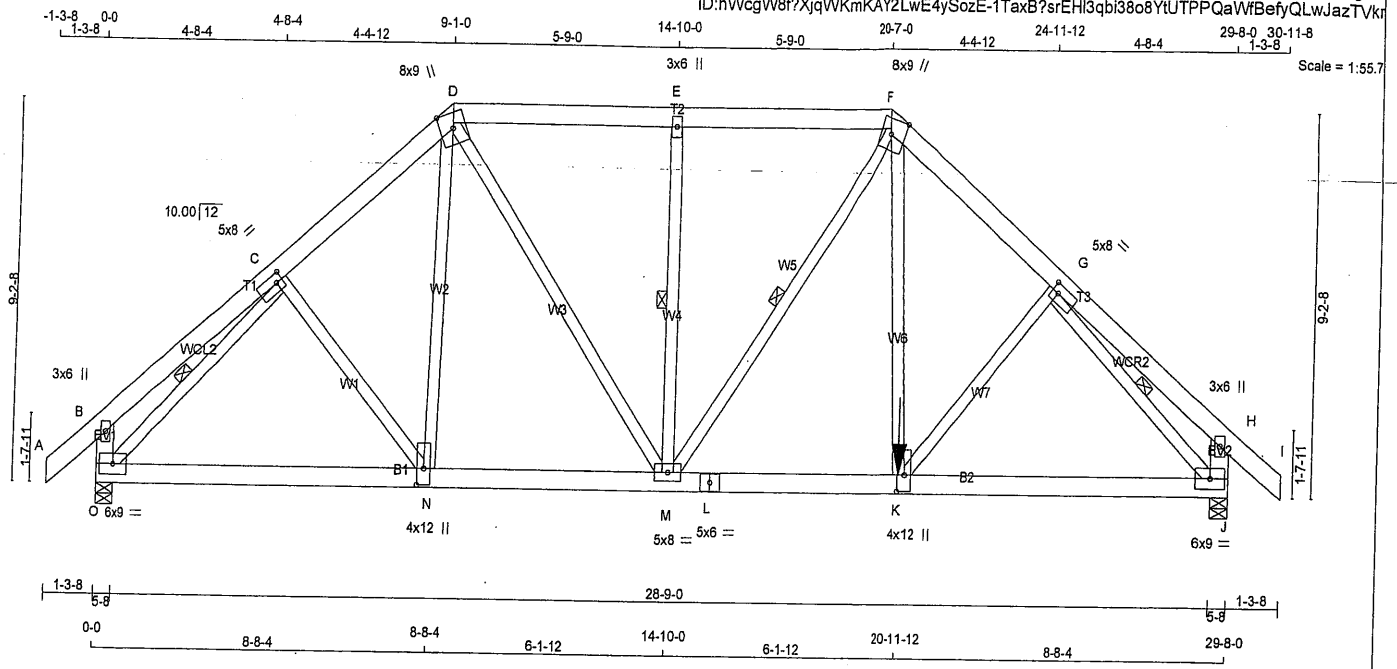


TOTAL WEIGHT = 2 X 205 = 410 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- I	2	12	TOP
O- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- F	1	3	SIDE(1354.0)
2x4	1	6	
N- D	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.00
D	TTWW+m	MT20	8.0	9.0	Edge	
E	TMW+w	MT20	3.0	6.0		
F	TTWW+m	MT20	8.0	9.0	Edge	
G	TMWW-t	MT20	5.0	8.0	2.50	2.00
H	TMV+p	MT20	3.0	6.0		
J	BMVW1-t	MT20	6.0	9.0		
K	BMWW+t	MT20	4.0	12.0	4.75	2.00
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW+t	MT20	4.0	12.0	4.75	2.00
O	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	4127	0	4127	0	0	5-8	5-8
J	7334	0	7334	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	3259	1940 / 0	576 / 0	0 / 0	0 / 0	743 / 0	0 / 0
J	5798	3442 / 0	1032 / 0	0 / 0	0 / 0	1324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, F-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

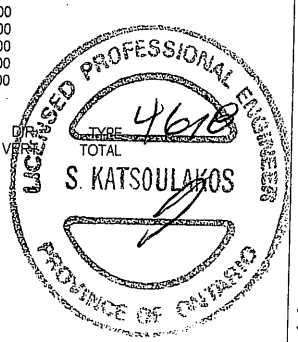
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-2 / 25	-112.4	-112.4	0.08 (1)	10.00
C- D	-4584 / 0	-112.4	-112.4	0.11 (1)	5.30
D- E	-4994 / 0	-112.4	-112.4	0.19 (1)	5.03
E- F	-4994 / 0	-112.4	-112.4	0.19 (1)	5.03
F- G	-8249 / 0	-112.4	-112.4	0.22 (1)	4.08
G- H	-12 / 2	-112.4	-112.4	0.10 (1)	6.25
H- I	0 / 52	-112.4	-112.4	0.05 (1)	10.00
O- B	-375 / 0	0.0	0.0	0.01 (1)	7.81
J- H	-411 / 0	0.0	0.0	0.01 (1)	7.81
O- N	0 / 3377	-28.0	-28.0	0.29 (1)	10.00
N- M	0 / 3483	-28.0	-28.0	0.30 (1)	10.00
M- L	0 / 6021	-28.0	-28.0	0.65 (1)	10.00
L- K	0 / 6021	-28.0	-28.0	0.65 (1)	10.00
K- J	0 / 5864	-152.3	-152.3	0.79 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 233	0.02 (2)
N- D	0 / 292	0.03 (3)
D- M	0 / 2744	0.24 (1)
M- E	-728 / 0	0.14 (1)
M- F	-1865 / 0	0.53 (1)
F- K	0 / 6332	0.56 (1)
K- G	0 / 686	0.06 (1)
O- C	-4974 / 0	0.52 (1)
G- J	-8638 / 0	0.91 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
K	20-11-12	-5892	-5892	---	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 20-11-12
START SPAN CARRIED = 6-0-0
END DISTANCE = 29-8-0
END SPAN CARRIED = 6-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.79/1.00 (J-K:1) , WB=0.91/1.00 (G-J:1) , SSI=0.25/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.87 (K) (INPUT = 1.00)

DWG NO. TAM 1779/18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T41	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5892.3 lbs FACTORED DOWN AT 20-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17291 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T43	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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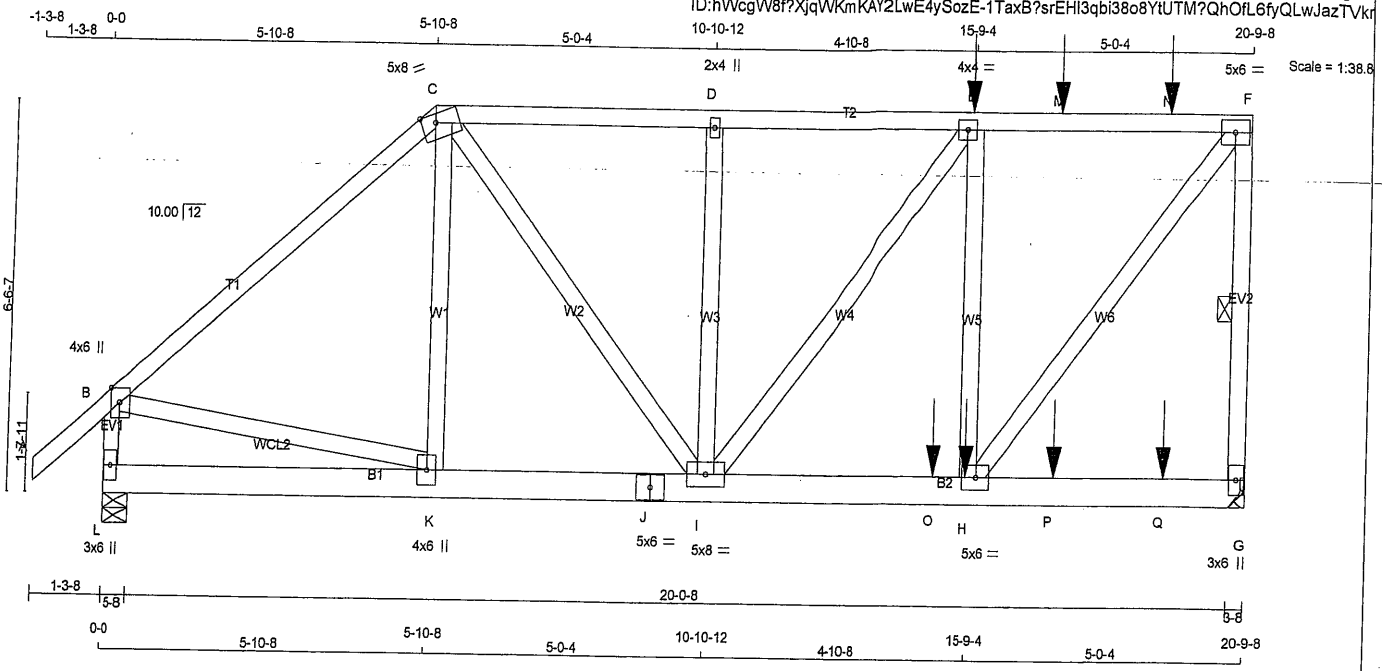

TOTAL WEIGHT = 2 X 124 = 249 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	2	12	TOP
J- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	3046	0	3046	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
L	2127	0	2127	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2407	1431 / 0	427 / 0	0 / 0	0 / 0	549 / 0	0 / 0
L	1668	1012 / 0	282 / 0	0 / 0	0 / 0	375 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.09 (1)	10.00	
B- C	-1961 / 0	-112.4	-112.4	0.44 (1)	5.70	
C- D	-2197 / 0	-112.4	-112.4	0.21 (1)	5.76	
D- E	-2197 / 0	-112.4	-112.4	0.31 (1)	5.60	
E- M	-2168 / 0	-112.4	-112.4	0.33 (1)	5.60	
M- N	-2168 / 0	-112.4	-112.4	0.33 (1)	5.60	
N- F	-2168 / 0	-112.4	-112.4	0.33 (1)	5.60	
G- F	-3009 / 0	0.0	0.0	0.30 (1)	5.32	
L- B	-2050 / 0	0.0	0.0	0.12 (1)	7.71	
L- K	0 / 0	-28.0	-28.0	0.06 (3)	10.00	
K- J	0 / 1500	-28.0	-28.0	0.16 (1)	10.00	
J- I	0 / 1500	-28.0	-28.0	0.16 (1)	10.00	
I- O	0 / 2168	-28.0	-28.0	0.35 (1)	10.00	
O- H	0 / 2168	-28.0	-28.0	0.35 (1)	10.00	
H- P	0 / 0	-28.0	-28.0	0.14 (1)	10.00	
P- Q	0 / 0	-28.0	-28.0	0.14 (1)	10.00	
Q- G	0 / 0	-28.0	-28.0	0.14 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-169 / 162	0.04 (1)
I- C	0 / 1102	0.10 (1)
I- D	-566 / 0	0.13 (1)
I- E	0 / 47	0.00 (1)
H- E	-962 / 0	0.23 (1)
H- F	0 / 3433	0.30 (1)
B- K	0 / 1535	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	TYPE
E	15-8-12	-135	-135	---	BACK	VERT TOTAL
H	15-8-12	-42	-73	---	BACK	VERT TOTAL
M	17-4-4	-135	-135	---	BACK	VERT TOTAL
N	19-4-4	-135	-135	---	BACK	VERT TOTAL
O	15-1-8	-1566	-1566	---	BACK	VERT TOTAL
P	17-4-4	-42	-73	---	BACK	VERT TOTAL
Q	19-4-4	-42	-73	---	BACK	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.35/1.00 (H-I:1) , WB=0.30/1.00 (F-H:1) , SSI=0.49/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S TSATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17792-R8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287688	T43	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 135.3 lbs FACTORED DOWN AT 15-8-12, AND 135.3 lbs FACTORED DOWN AT 17-4-4, AND 135.3 lbs FACTORED DOWN AT 19-4-4 ON TOP CHORD, AND 1565.7 lbs FACTORED DOWN AT 15-1-8, 73.3 lbs FACTORED DOWN AT 15-8-12, AND 73.3 lbs FACTORED DOWN AT 17-4-4, AND 73.3 lbs FACTORED DOWN AT 19-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17792-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T44	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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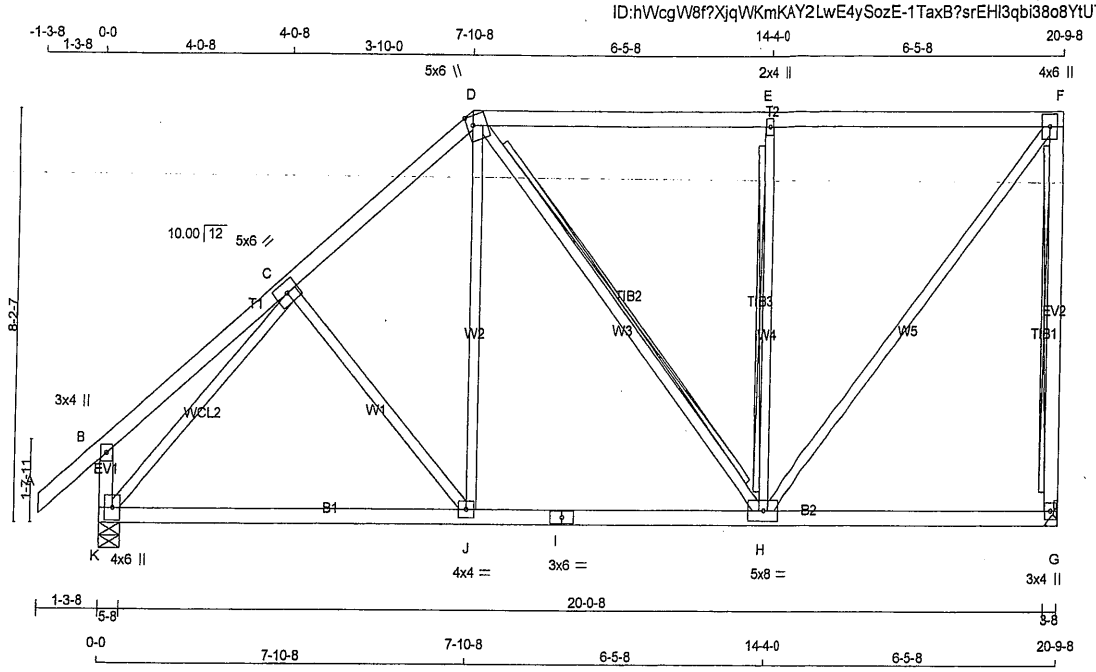

TOTAL WEIGHT = 100 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1460	0	1460	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
K	1616	0	1616	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1164	675 / 0	218 / 0	0 / 0	0 / 0	270 / 0	0 / 0
K	1270	765 / 0	218 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-G, D-H
2x3 DRY SPF No.2 T-BRACE AT E-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 33	-112.4	-112.4	0.28 (1)	10.00
C- D	-1236 / 0	-112.4	-112.4	0.31 (1)	5.39
D- E	-908 / 0	-112.4	-112.4	0.85 (1)	4.64
E- F	-909 / 0	-112.4	-112.4	0.85 (1)	4.64
G- F	-1363 / 0	0.0	0.0	0.53 (1)	7.81
K- B	-324 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1027	-28.0	-28.0	0.49 (2)	10.00
J- I	0 / 926	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 926	-28.0	-28.0	0.49 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-159 / 50	0.10 (1)
J- D	0 / 408	0.09 (2)
D- H	-68 / 0	0.06 (3)
H- E	-900 / 0	0.92 (1)
H- F	0 / 1431	0.32 (1)
K- C	1579 / 0	0.97 (1)

LICENSED PROFESSIONAL ENGINEER 4618 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 914 (0.27")

CSI: TC=0.85/1.00 (E-F:1) , BC=0.49/1.00 (J-K:2) , WB=0.97/1.00 (C-K:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM 17793-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T45	4	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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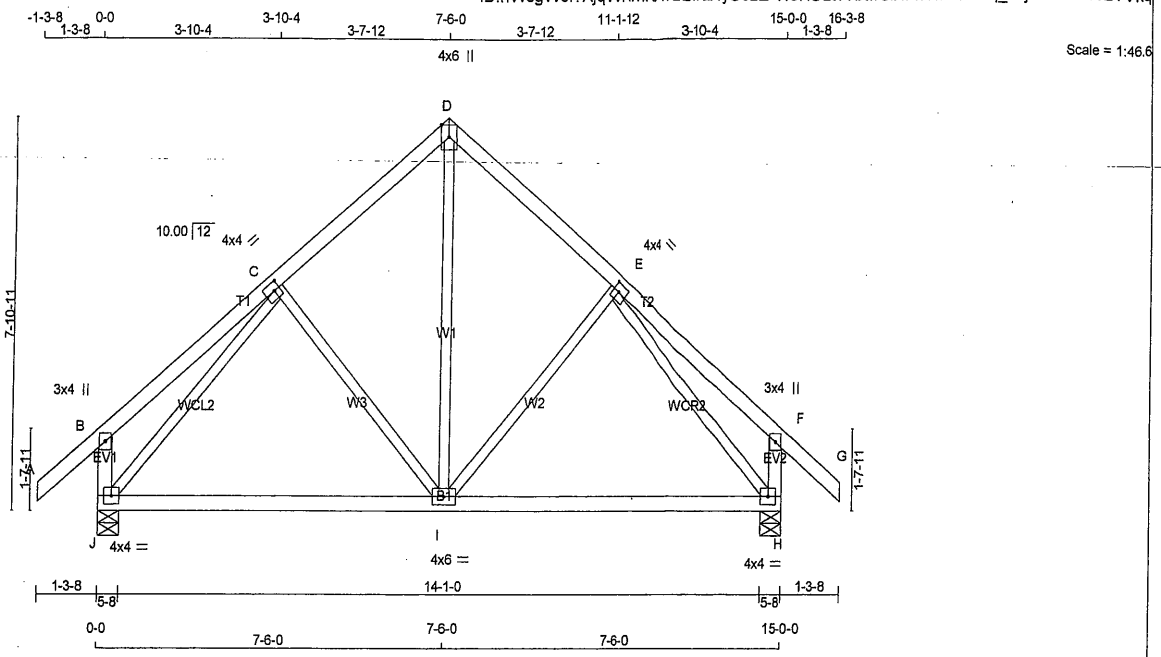

TOTAL WEIGHT = 4 X 70 = 280 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1209	0	1209	0	0	5-8	5-8
H	1209	0	1209	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	946	577 / 0	157 / 0	0 / 0	0 / 0	212 / 0	0 / 0
H	946	577 / 0	157 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	I- D	0 / 584	0.13 (1)
B- C	0 / 31	-112.4	-112.4	0.25 (1)	10.00	I- E	-214 / 21	0.12 (1)
C- D	-744 / 0	-112.4	-112.4	0.20 (1)	6.25	C- I	-214 / 21	0.12 (1)
D- E	-744 / 0	-112.4	-112.4	0.20 (1)	6.25	J- C	-1064 / 0	0.59 (1)
E- F	0 / 31	-112.4	-112.4	0.25 (1)	10.00	E- H	-1064 / 0	0.59 (1)
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00			
J- B	-316 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-316 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 687	-28.0	-28.0	0.54 (3)	10.00			
I- H	0 / 687	-28.0	-28.0	0.54 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.54/1.00 (I-J:3) , WB=0.59/1.00 (E-H:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

DWG NO. TAM 17794-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	G45	1	1	OBC2012_6TCDL TRUSS DESC.	

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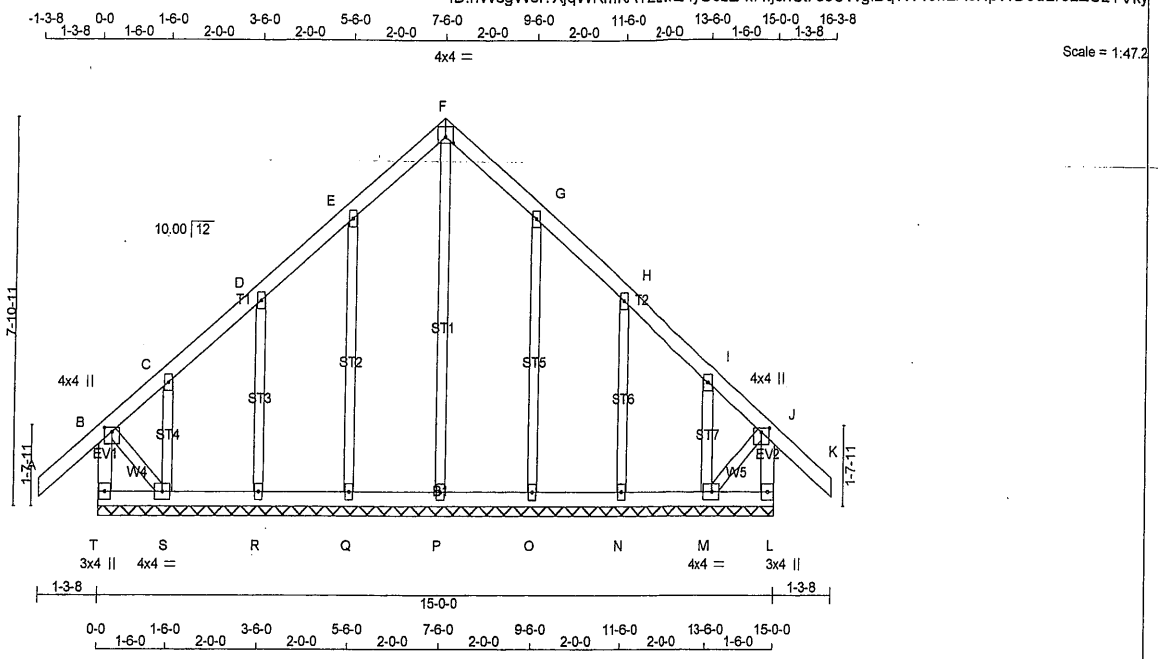

TOTAL WEIGHT = 74 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-79 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-33 / 0	-112.4	-112.4	0.06 (1)	6.25
F- G	-33 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
H- I	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-79 / 0	-112.4	-112.4	0.15 (1)	6.25
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-349 / 0	0.0	0.0	0.04 (1)	7.81
L- J	-349 / 0	0.0	0.0	0.04 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 23	-28.0	-28.0	0.02 (3)	10.00
R- Q	0 / 18	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 14	-28.0	-28.0	0.02 (3)	10.00
P- O	0 / 14	-28.0	-28.0	0.02 (3)	10.00
O- N	0 / 18	-28.0	-28.0	0.02 (3)	10.00
N- M	0 / 23	-28.0	-28.0	0.02 (3)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
P- F	-167 / 0	0.19 (1)
Q- E	-252 / 0	0.16 (1)
R- D	-231 / 0	0.07 (1)
S- C	-107 / 0	0.02 (1)
O- G	-252 / 0	0.16 (1)
N- H	-231 / 0	0.07 (1)
M- I	-107 / 0	0.02 (1)
B- S	0 / 36	0.01 (1)
M- J	0 / 36	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (R-S:3) , WB=0.19/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (F) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 1.00)

DWG NO. TAM 17797-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T46	1	3	OBC2012_6TCDL TRUSS DESC.	

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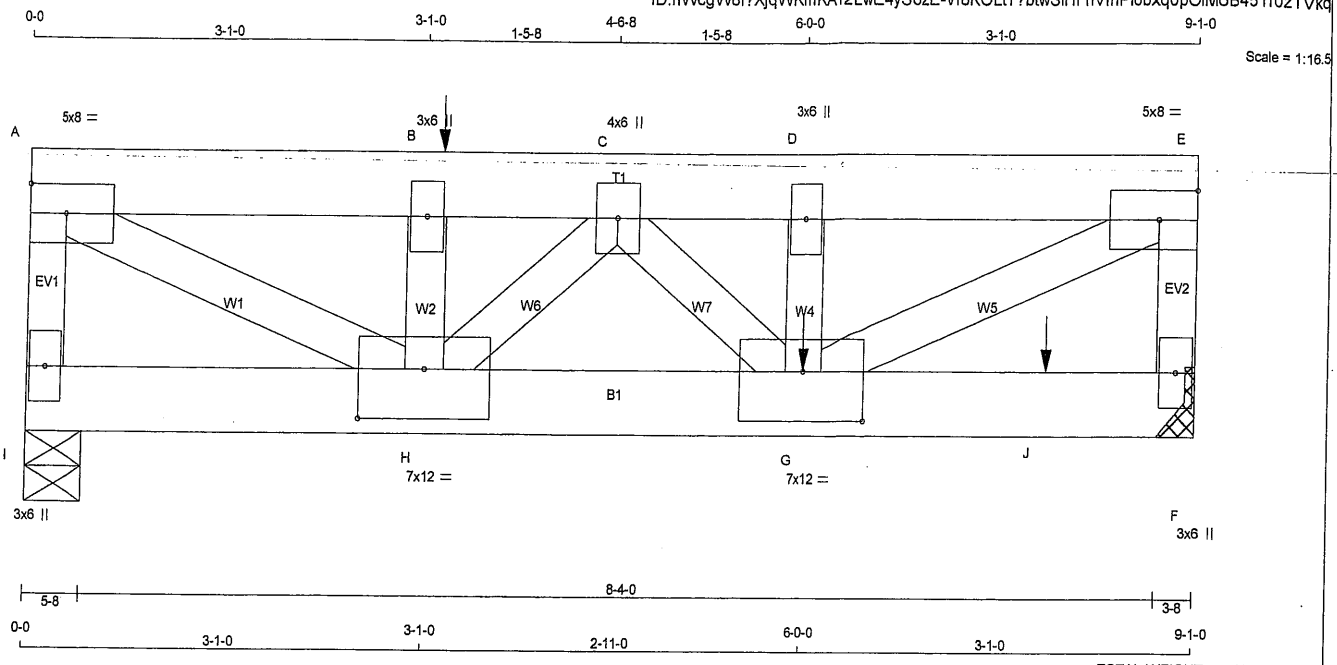


TOTAL WEIGHT = 3 X 48 = 144 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
I- A	1	12	TOP
E- F	1	12	TOP
A- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- F	3	5	SIDE(1274.9)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	2	4	SIDE(141.4)
2x4	1	6	
B- H	2	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		Edge
B	TMW+w	MT20	3.0	6.0		
C	TMWW+t	MT20	4.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMVW-t	MT20	5.0	8.0		Edge
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	7.0	12.0	4.25	6.00
H	BMWWW-t	MT20	7.0	12.0	4.25	6.00
I	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1462.0 lbs FACTORED DOWN AT 3-2-8 ON TOP CHORD, AND 3018.2 lbs FACTORED DOWN AT 6-0-0, AND 1432.0 lbs FACTORED DOWN AT 7-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	4350	0	4350	0	0	I-SX	I-SX
F	5920	0	5920	0	0	F-SX	F-SX

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3460	2020 / 0	641 / 0	0 / 0	0 / 0	800 / 0	0 / 0
F	4689	2770 / 0	844 / 0	0 / 0	0 / 0	1075 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

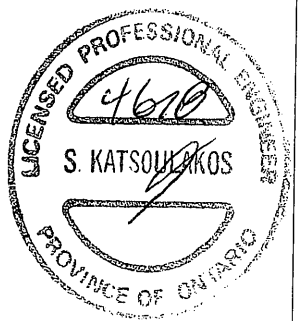
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- A	-3743 / 0	0.0	0.0	0.13 (1)	7.19	A- H	0 / 7651	0.41 (1)
A- B	-6843 / 0	-112.4	-112.4	0.12 (1)	5.31	H- B	-1270 / 0	0.04 (1)
B- C	-6843 / 0	-112.4	-112.4	0.12 (1)	5.31	G- D	0 / 344	0.02 (1)
C- D	-8267 / 0	-112.4	-112.4	0.17 (1)	4.88	G- E	0 / 9243	0.50 (1)
D- E	-8267 / 0	-112.4	-112.4	0.17 (1)	4.88	H- C	-1633 / 0	0.06 (1)
F- E	-4503 / 0	0.0	0.0	0.16 (1)	6.70	C- G	0 / 439	0.02 (1)
I- H	0 / 0	-367.4	-367.4	0.11 (1)	10.00			
H- G	0 / 7965	-367.4	-367.4	0.40 (1)	10.00			
G- J	0 / 0	-367.4	-367.4	0.31 (1)	10.00			
J- F	0 / 0	-367.4	-367.4	0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-2-8	-1462	-1462	---	TOP	VERT	TOTAL
G	6-0-0	-3018	-3018	---	FRONT	VERT	TOTAL
J	7-11-4	-1432	-1432	---	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-9-8
END DISTANCE = 9-1-0
END SPAN CARRIED = 11-9-8
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (D-E:1) , BC=0.40/1.00 (G-H:1) , WB=0.50/1.00 (E-G:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.49 (A) (INPUT = 1.00)

DWG NO. TAM 17795-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296634	T51	1	2	OBC2012_6TCDL TRUSS DESC.	

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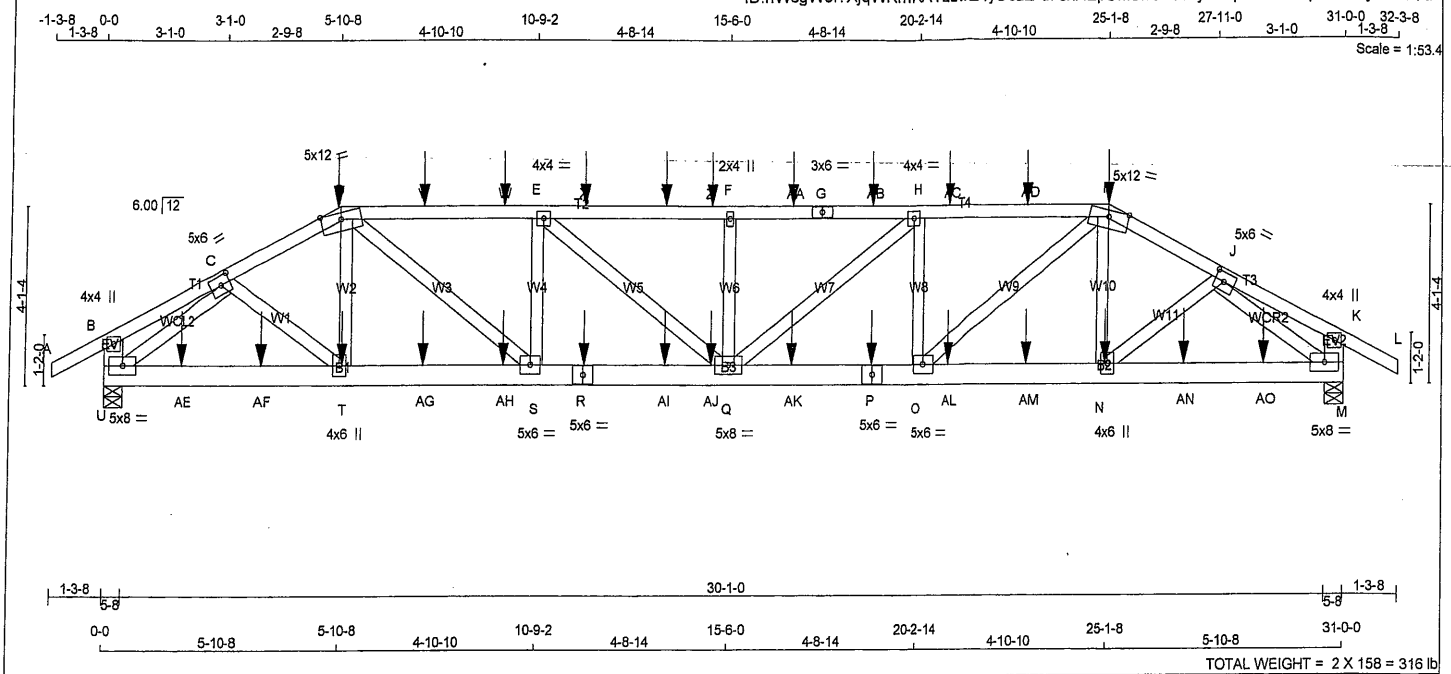


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(0.0)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
T- D	1	6	SIDE(32.9)
N- I	1	6	SIDE(32.9)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	12.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	12.0	Edge	
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	3851	0	3851	0	0	5-8	5-8
M	3846	0	3846	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3061	1787 / 0	563 / 0	0 / 0	0 / 0	711 / 0	0 / 0
M	3057	1785 / 0	562 / 0	0 / 0	0 / 0	710 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

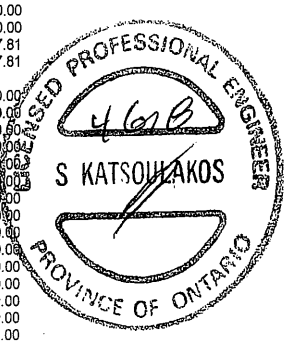
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.08 (1)		10.00
B- C	-2 / 8	-112.4	-112.4	0.07 (1)		10.00
C- D	-5772 / 0	-112.4	-112.4	0.18 (1)		3.93
D- V	-7396 / 0	-112.4	-112.4	0.62 (1)		3.08
V- W	-7396 / 0	-112.4	-112.4	0.62 (1)		3.08
W- E	-7396 / 0	-112.4	-112.4	0.62 (1)		3.08
E- X	-8062 / 0	-112.4	-112.4	0.66 (1)		2.91
X- Y	-8062 / 0	-112.4	-112.4	0.66 (1)		2.91
Y- Z	-8062 / 0	-112.4	-112.4	0.66 (1)		2.91
Z- F	-8062 / 0	-112.4	-112.4	0.66 (1)		2.91
F-AA	-8062 / 0	-112.4	-112.4	0.65 (1)		2.92
AA- G	-8062 / 0	-112.4	-112.4	0.65 (1)		2.92
G-AB	-8062 / 0	-112.4	-112.4	0.65 (1)		2.92
AB- H	-8062 / 0	-112.4	-112.4	0.65 (1)		2.92
H-AC	-7377 / 0	-112.4	-112.4	0.61 (1)		3.11
AC-AD	-7377 / 0	-112.4	-112.4	0.61 (1)		3.11
AD- I	-7377 / 0	-112.4	-112.4	0.61 (1)		3.11
I- J	-5763 / 0	-112.4	-112.4	0.18 (1)		3.93
J- K	-2 / 8	-112.4	-112.4	0.07 (1)		10.00
K- L	0 / 34	-112.4	-112.4	0.08 (1)		10.00
U- B	-301 / 0	0.0	0.0	0.01 (1)		7.81
M- K	-301 / 0	0.0	0.0	0.01 (1)		7.81
U-AE	0 / 4515	-28.0	-28.0	0.36 (1)		10.00
AE-AF	0 / 4515	-28.0	-28.0	0.36 (1)		10.00
AF- T	0 / 4515	-28.0	-28.0	0.36 (1)		10.00
T-AG	0 / 5142	-28.0	-28.0	0.37 (1)		10.00
AG-AH	0 / 5142	-28.0	-28.0	0.37 (1)		10.00
AH- S	0 / 5142	-28.0	-28.0	0.37 (1)		10.00
S- R	0 / 7396	-28.0	-28.0	0.55 (1)		10.00
R-AI	0 / 7396	-28.0	-28.0	0.55 (1)		10.00
AI-AJ	0 / 7396	-28.0	-28.0	0.55 (1)		10.00
AJ- Q	0 / 7396	-28.0	-28.0	0.55 (1)		10.00
Q-AK	0 / 7378	-28.0	-28.0	0.55 (1)		10.00
AK- P	0 / 7378	-28.0	-28.0	0.55 (1)		10.00
P- O	0 / 7378	-28.0	-28.0	0.55 (1)		10.00
O-AL	0 / 5134	-28.0	-28.0	0.37 (1)		10.00
AL-AM	0 / 5134	-28.0	-28.0	0.37 (1)		10.00
AM- N	0 / 5134	-28.0	-28.0	0.37 (1)		10.00
N-AN	0 / 4509	-28.0	-28.0	0.36 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 790	0.07 (1)
T- D	-166 / 267	0.02 (3)
D- S	0 / 2834	0.25 (1)
S- E	-1502 / 0	0.14 (1)
E- Q	0 / 847	0.07 (1)
Q- F	-839 / 0	0.08 (1)
Q- H	0 / 871	0.08 (1)
O- H	-1499 / 0	0.14 (1)
O- I	0 / 2820	0.25 (1)
N- I	-164 / 269	0.02 (3)
N- J	0 / 788	0.07 (1)
U- C	-5670 / 0	0.49 (1)
J- M	-5663 / 0	0.49 (1)



[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.66/1.00 (E-F:1) , BC=0.55/1.00 (Q-S:1) , WB=0.49/1.00 (C-U:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.74 (J) (INPUT = 1.00)

DWG NO. TAM 17832-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMVW1-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 593.9 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 135.3 lbs FACTORED DOWN AT 15-0-12, 135.3 lbs FACTORED DOWN AT 17-0-12, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, AND 135.3 lbs FACTORED DOWN AT 23-0-12, AND 593.9 lbs FACTORED DOWN AT 25-1-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-0-12, 69.9 lbs FACTORED DOWN AT 17-0-12, 69.9 lbs FACTORED DOWN AT 19-0-12, 69.9 lbs FACTORED DOWN AT 21-0-12, 69.9 lbs FACTORED DOWN AT 23-0-12, 69.9 lbs FACTORED DOWN AT 25-0-12, AND 84.6 lbs FACTORED DOWN AT 27-0-12, AND 84.6 lbs FACTORED DOWN AT 29-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					UNBRAC LENGTH FR-TO		
AN-AO	0 / 4509	-28.0	-28.0	0.36 (1)	10.00		
AO- M	0 / 4509	-28.0	-28.0	0.36 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-594	-594	—	FRONT	VERT	TOTAL
I	25-1-8	-594	-594	—	FRONT	VERT	TOTAL
N	25-0-12	-40	-70	—	FRONT	VERT	TOTAL
P	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
R	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
T	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
V	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
W	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
X	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
Y	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	15-0-12	-135	-135	—	FRONT	VERT	TOTAL
AA	17-0-12	-135	-135	—	FRONT	VERT	TOTAL
AB	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
AC	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
AD	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AE	1-11-4	-48	-85	—	FRONT	VERT	TOTAL
AF	3-11-4	-48	-85	—	FRONT	VERT	TOTAL
AG	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AJ	15-0-12	-40	-70	—	FRONT	VERT	TOTAL
AK	17-0-12	-40	-70	—	FRONT	VERT	TOTAL
AL	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
AM	23-0-12	-40	-70	—	FRONT	VERT	TOTAL
AN	27-0-12	-48	-85	—	FRONT	VERT	TOTAL
AO	29-0-12	-48	-85	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4670
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM N83248
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T51S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Fri Apr 6 08:41:39 2018 Page 1

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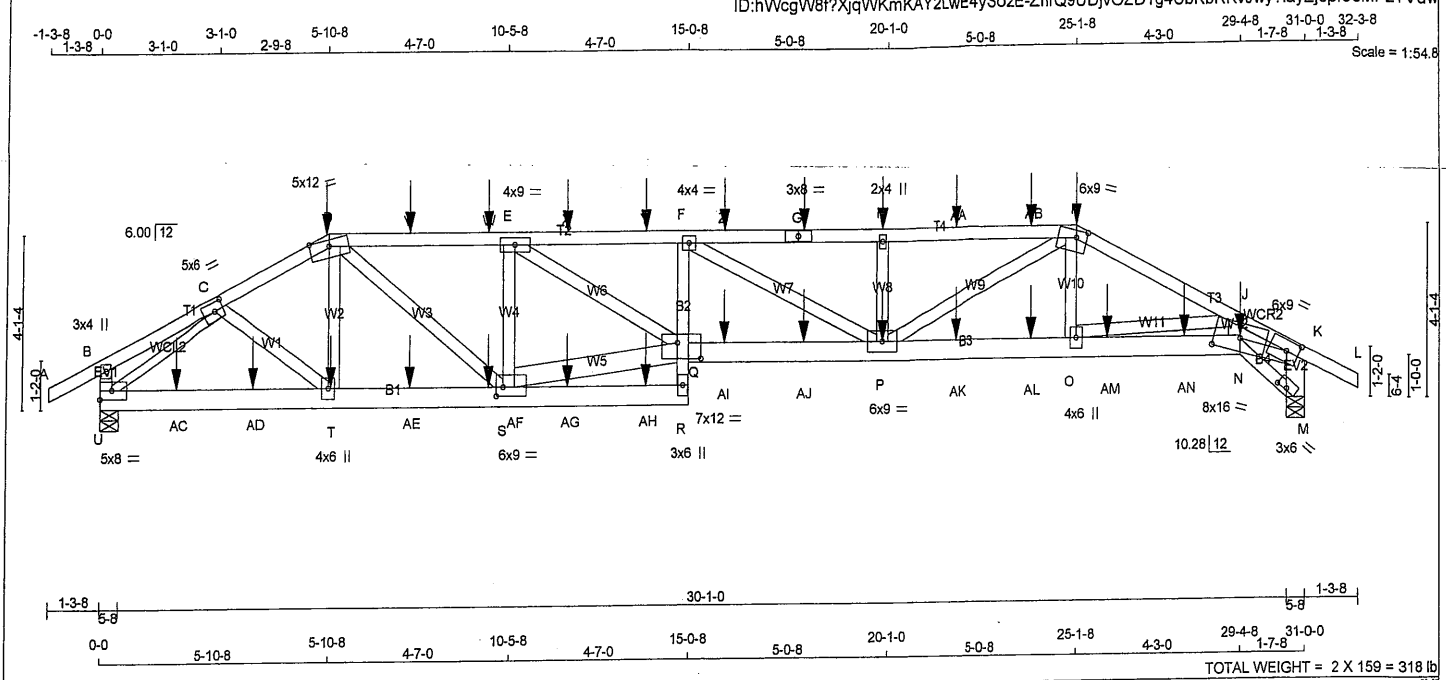

TOTAL WEIGHT = 2 X 159 = 318 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
U - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
U- B	1	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
Q- N	2	12	SIDE(183.1)
R- F	1	12	TOP
N- M	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
T- D	1	6	SIDE(25.8)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	12.0	Edge	
E	TMWW-t	MT20	4.0	9.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	2.00	3.25
J						
K	TMVW-t	MT20	6.0	9.0	3.00	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	4148	0	4148	0	0	5-8	5-8
U	3905	0	3905	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	3330	1895 / 0	651 / 0	0 / 0	0 / 0	784 / 0	0 / 0
U	3116	1803 / 0	588 / 0	0 / 0	0 / 0	725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.08 (1)	10.00
B- C	-3 / 6	-112.4	-112.4	0.08 (1)	10.00
C- D	-5870 / 0	-112.4	-112.4	0.19 (1)	3.89
D- V	-7462 / 0	-112.4	-112.4	0.55 (1)	3.14
V- W	-7462 / 0	-112.4	-112.4	0.55 (1)	3.14
W- E	-7462 / 0	-112.4	-112.4	0.55 (1)	3.14
E- X	-11455 / 0	-112.4	-112.4	0.88 (1)	2.17
X- Y	-11455 / 0	-112.4	-112.4	0.88 (1)	2.17
Y- F	-11455 / 0	-112.4	-112.4	0.88 (1)	2.17
F- Z	-10789 / 0	-112.4	-112.4	0.94 (1)	2.24
Z- G	-10789 / 0	-112.4	-112.4	0.94 (1)	2.24
G- H	-10789 / 0	-112.4	-112.4	0.94 (1)	2.24
H-AA	-10789 / 0	-112.4	-112.4	0.96 (1)	2.24
AA-AB	-10789 / 0	-112.4	-112.4	0.96 (1)	2.24
AB- I	-10789 / 0	-112.4	-112.4	0.96 (1)	2.24
I- J	-8231 / 0	-112.4	-112.4	0.58 (1)	2.95
J- K	-11168 / 0	-112.4	-112.4	0.55 (1)	2.44
K- L	0 / 34	-112.4	-112.4	0.08 (1)	10.00
M- K	-4126 / 0	0.0	0.0	0.15 (1)	7.02
U- B	-307 / 0	0.0	0.0	0.02 (1)	7.81
U-AC	0 / 4587	-28.0	-28.0	0.37 (1)	10.00
AC-AD	0 / 4587	-28.0	-28.0	0.37 (1)	10.00
AD- T	0 / 4587	-28.0	-28.0	0.37 (1)	10.00
T-AE	0 / 5229	-28.0	-28.0	0.37 (1)	10.00
AE-AF	0 / 5229	-28.0	-28.0	0.37 (1)	10.00
AF- S	0 / 5229	-28.0	-28.0	0.37 (1)	10.00
S-AG	0 / 298	-28.0	-28.0	0.08 (2)	10.00
AG-AH	0 / 298	-28.0	-28.0	0.08 (2)	10.00
AH- R	0 / 298	-28.0	-28.0	0.08 (2)	10.00
R- Q	0 / 173	0.0	0.0	0.24 (1)	10.00
Q- F	-363 / 37	0.0	0.0	0.23 (1)	7.81
Q-AI	0 / 11567	-28.0	-28.0	0.89 (1)	10.00
AI-AJ	0 / 11567	-28.0	-28.0	0.89 (1)	10.00
AJ- P	0 / 11567	-28.0	-28.0	0.89 (1)	10.00
P-AK	0 / 7424	-28.0	-28.0	0.60 (1)	10.00
AK-AL	0 / 7424	-28.0	-28.0	0.60 (1)	10.00
AL- O	0 / 7424	-28.0	-28.0	0.60 (1)	10.00
O-AM	0 / 9597	-28.0	-28.0	0.69 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 814	0.07 (1)
T- D	-222 / 202	0.02 (1)
D- S	0 / 2906	0.26 (1)
S- E	-3283 / 0	0.30 (1)
S- Q	0 / 7446	0.48 (1)
E- Q	0 / 4559	0.40 (1)
F- P	-884 / 0	0.17 (1)
P- H	-1041 / 0	0.07 (1)
P- I	0 / 3799	0.34 (1)
O- I	0 / 893	0.08 (1)
O- J	-2247 / 0	0.26 (1)
N- J	0 / 1533	0.14 (1)
N- K	0 / 9998	0.88 (1)
U- C	-5758 / 0	0.72 (1)

LICENSED PROFESSIONAL ENGINEER
468
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 940 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 574 (0.65")

CSI: TC=0.96/1.00 (H-I:1) , BC=0.89/1.00 (P-Q:1) , WB=0.88/1.00 (K-N:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 1780618
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T51S	1	2	OBC2012_6TCDL TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BVM1-I	MT20	3.0	6.0	0.50	3.00
N						
N	TMBBWWW*+	MT20	8.0	16.0	8.00	3.75
O	BMWW+t	MT20	4.0	6.0		
P	BMWWW-t	MT20	6.0	9.0		
Q	BVMWW-I	MT20	7.0	12.0	4.50	7.25
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0	2.50	2.00
T	BMWW+t	MT20	4.0	6.0		
U	BMVW1-t	MT20	5.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 376.3 lbs FACTORED DOWN AT 25-1-8, 566.4 lbs FACTORED DOWN AT 5-10-8, 135.3 lbs FACTORED DOWN AT 7-11-4, 135.3 lbs FACTORED DOWN AT 9-11-4, 135.3 lbs FACTORED DOWN AT 11-11-4, 135.3 lbs FACTORED DOWN AT 13-11-4, 138.8 lbs FACTORED DOWN AT 15-11-4, 138.8 lbs FACTORED DOWN AT 17-11-4, 138.8 lbs FACTORED DOWN AT 19-11-4, AND 138.8 lbs FACTORED DOWN AT 21-11-4, AND 138.8 lbs FACTORED DOWN AT 23-11-4 ON TOP CHORD, AND 74.5 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 70.9 lbs FACTORED DOWN AT 13-11-4, 115.6 lbs FACTORED DOWN AT 15-11-4, 115.6 lbs FACTORED DOWN AT 17-11-4, 115.6 lbs FACTORED DOWN AT 19-11-4, 115.6 lbs FACTORED DOWN AT 21-11-4, 115.6 lbs FACTORED DOWN AT 23-11-4, 115.6 lbs FACTORED DOWN AT 25-11-4, AND 115.6 lbs FACTORED DOWN AT 27-11-4, AND 128.8 lbs FACTORED DOWN AT 29-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	WEBS MAX. MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		LENGTH FR-TO		
AM-AN	0 / 9597	-28.0	-28.0	0.69 (1)	10.00		
AN- N	0 / 9597	-28.0	-28.0	0.69 (1)	10.00		
N- M	0 / 0	-28.0	-28.0	0.01 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-566	-566	—	FRONT	VERT	TOTAL
G	17-11-4	-139	-139	—	FRONT	VERT	TOTAL
H	19-11-4	-139	-139	—	FRONT	VERT	TOTAL
I	25-1-8	-376	-376	—	FRONT	VERT	TOTAL
N	29-4-8	-129	-129	—	FRONT	VERT	TOTAL
P	19-11-4	-116	-116	—	FRONT	VERT	TOTAL
T	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
V	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
W	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
X	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
Y	13-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	15-11-4	-139	-139	—	FRONT	VERT	TOTAL
AA	21-11-4	-139	-139	—	FRONT	VERT	TOTAL
AB	23-11-4	-139	-139	—	FRONT	VERT	TOTAL
AC	1-11-4	-39	-74	—	FRONT	VERT	TOTAL
AD	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	13-11-4	-41	-71	—	FRONT	VERT	TOTAL
AI	15-11-4	-116	-116	—	FRONT	VERT	TOTAL
AJ	17-11-4	-116	-116	—	FRONT	VERT	TOTAL
AK	21-11-4	-116	-116	—	FRONT	VERT	TOTAL
AL	23-11-4	-116	-116	—	FRONT	VERT	TOTAL
AM	25-11-4	-116	-116	—	FRONT	VERT	TOTAL
AN	27-11-4	-116	-116	—	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.94 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
46B
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17808
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296634	T52	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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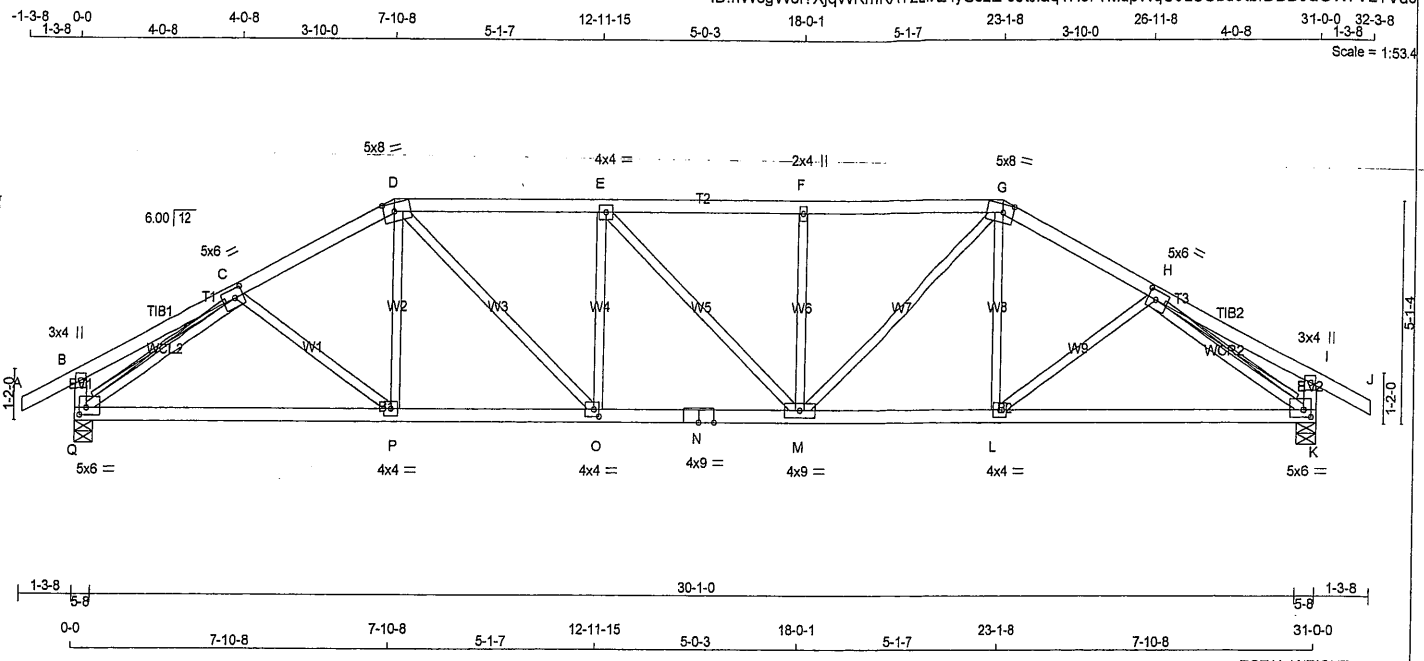


TOTAL WEIGHT = 124 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	2.25	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	3.25
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.00	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2329	0	2329	0	0	5-8	5-8
K	2329	0	2329	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
Q	1840	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
K	1840	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

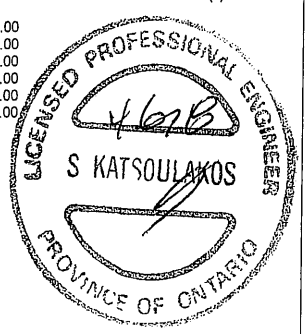
2x4 DRY SPF No.2 T-BRACE AT C-Q, H-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)			FR-TO			
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C- P	0 / 179	0.04 (3)
B- C	0 / 20	-112.4	-112.4	0.24 (1)	10.00	P- D	0 / 304	0.07 (3)
C- D	-3030 / 0	-112.4	-112.4	0.30 (1)	3.78	D- O	0 / 1072	0.24 (1)
D- E	-3476 / 0	-112.4	-112.4	0.50 (1)	3.34	O- E	-616 / 0	0.24 (1)
E- F	-3474 / 0	-112.4	-112.4	0.46 (1)	3.39	E- M	-3 / 0	0.00 (1)
F- G	-3474 / 0	-112.4	-112.4	0.50 (1)	3.34	M- F	-615 / 0	0.24 (1)
G- H	-3030 / 0	-112.4	-112.4	0.30 (1)	3.78	M- G	0 / 1069	0.24 (1)
H- I	0 / 20	-112.4	-112.4	0.24 (1)	10.00	L- G	0 / 304	0.07 (3)
I- J	0 / 34	-112.4	-112.4	0.15 (1)	10.00	L- H	0 / 179	0.04 (3)
Q- B	-331 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-3269 / 0	0.69 (1)
K- I	-331 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-3269 / 0	0.69 (1)
Q- P	0 / 2656	-28.0	-28.0	0.65 (2)	10.00			
P- O	0 / 2694	-28.0	-28.0	0.68 (2)	10.00			
O- N	0 / 3476	-28.0	-28.0	0.65 (1)	10.00			
N- M	0 / 3476	-28.0	-28.0	0.65 (1)	10.00			
M- L	0 / 2695	-28.0	-28.0	0.68 (2)	10.00			
L- K	0 / 2657	-28.0	-28.0	0.65 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.50/1.00 (F-G:1) , BC=0.68/1.00 (L-M:2) , WB=0.69/1.00 (H-K:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.81 (H) (INPUT = 1.00)

DWG NO. TAM 17833-17
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T52S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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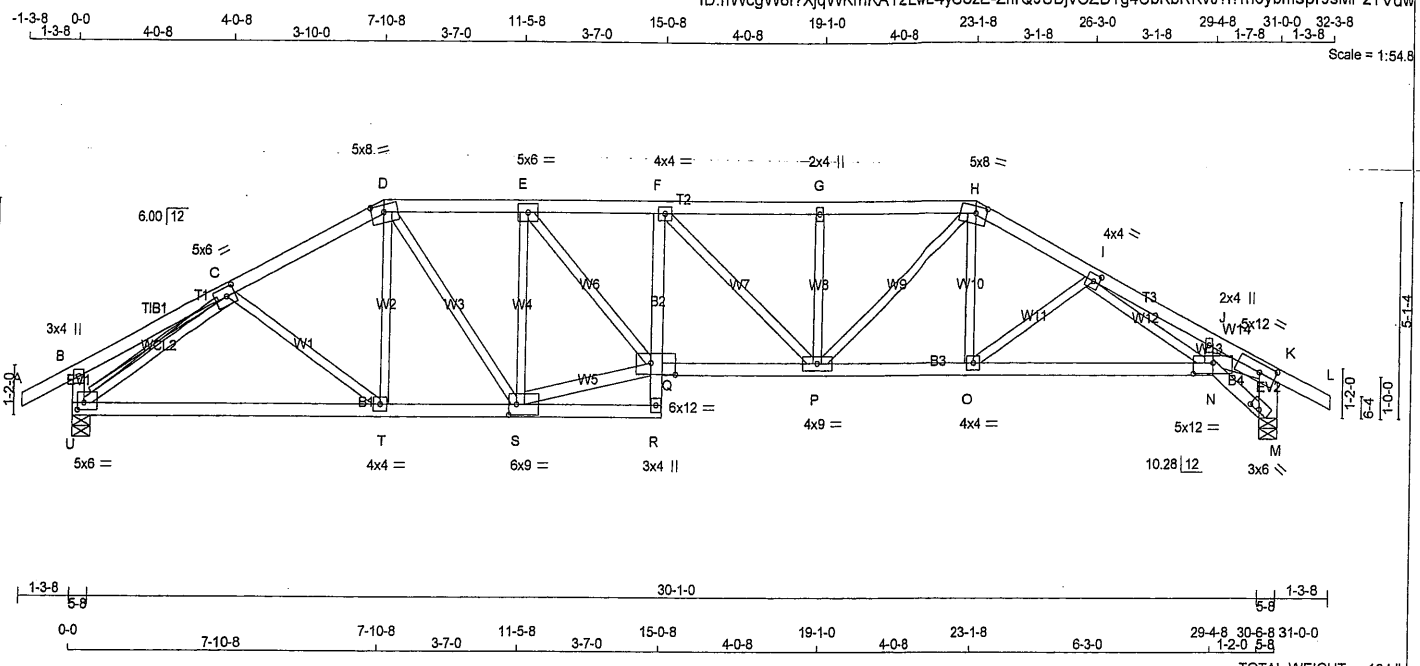

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.75
E	TMWW-t	MT20	5.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMW+w	MT20	2.0	4.0		
K	TMVW-t	MT20	5.0	12.0	2.50	5.00
M	BVM1-l	MT20	3.0	6.0	0.50	3.00
N	BBWWW-p	MT20	5.0	12.0	3.00	6.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BVMWW-l	MT20	6.0	12.0	3.25	7.50
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	9.0	3.00	2.25
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2333	0	2333	0	0	5-8	5-8
U	2326	0	2326	0	0	5-8	5-8

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD. 6.31.24

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1842	1097 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0
U	1837	1093 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 20	-112.4	-112.4	0.24 (1)	10.00
C- D	-3024 / 0	-112.4	-112.4	0.30 (1)	3.78
D- E	-3275 / 0	-112.4	-112.4	0.27 (1)	3.67
E- F	-4432 / 0	-112.4	-112.4	0.36 (1)	3.11
F- G	-4235 / 0	-112.4	-112.4	0.41 (1)	3.13
G- H	-4235 / 0	-112.4	-112.4	0.43 (1)	3.12
H- I	-3774 / 0	-112.4	-112.4	0.33 (1)	3.38
I- J	-5145 / 0	-112.4	-112.4	0.66 (1)	2.48
J- K	-5301 / 0	-112.4	-112.4	0.54 (1)	2.59
K- L	0 / 34	-112.4	-112.4	0.15 (1)	10.00
U- B	-331 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2310 / 0	0.0	0.0	0.15 (1)	6.75
U- T	0 / 2652	-28.0	-28.0	0.66 (2)	10.00
T- S	0 / 2689	-28.0	-28.0	0.68 (2)	10.00
S- R	0 / 103	-28.0	-28.0	0.11 (3)	10.00
R- Q	0 / 85	0.0	0.0	0.15 (1)	10.00
Q- F	-175 / 42	0.0	0.0	0.12 (1)	7.81
Q- P	0 / 4461	-28.0	-28.0	0.79 (1)	10.00
P- O	0 / 3367	-28.0	-28.0	0.66 (1)	10.00
O- N	0 / 3929	-28.0	-28.0	0.77 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 179	0.04 (3)
T- D	0 / 303	0.07 (3)
D- S	0 / 993	0.22 (1)
S- E	-1685 / 0	0.65 (1)
S- Q	0 / 3330	0.54 (1)
E- Q	0 / 1687	0.38 (1)
F- P	-308 / 0	0.16 (1)
P- G	-522 / 0	0.13 (1)
P- H	0 / 1179	0.27 (1)
O- H	0 / 569	0.13 (1)
O- I	-704 / 0	0.19 (1)
I- N	0 / 888	0.20 (1)
N- J	0 / 112	0.03 (3)
N- K	0 / 4682	0.75 (1)
U- C	-3263 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 714 (0.52")

CSI: TC=0.66/1.00 (I-J:1) , BC=0.79/1.00 (P-Q:1) , WB=0.75/1.00 (K-N:1) , SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.81 (C) (INPUT = 1.00)

DWG NO. TAM 17809-RB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296634	T53	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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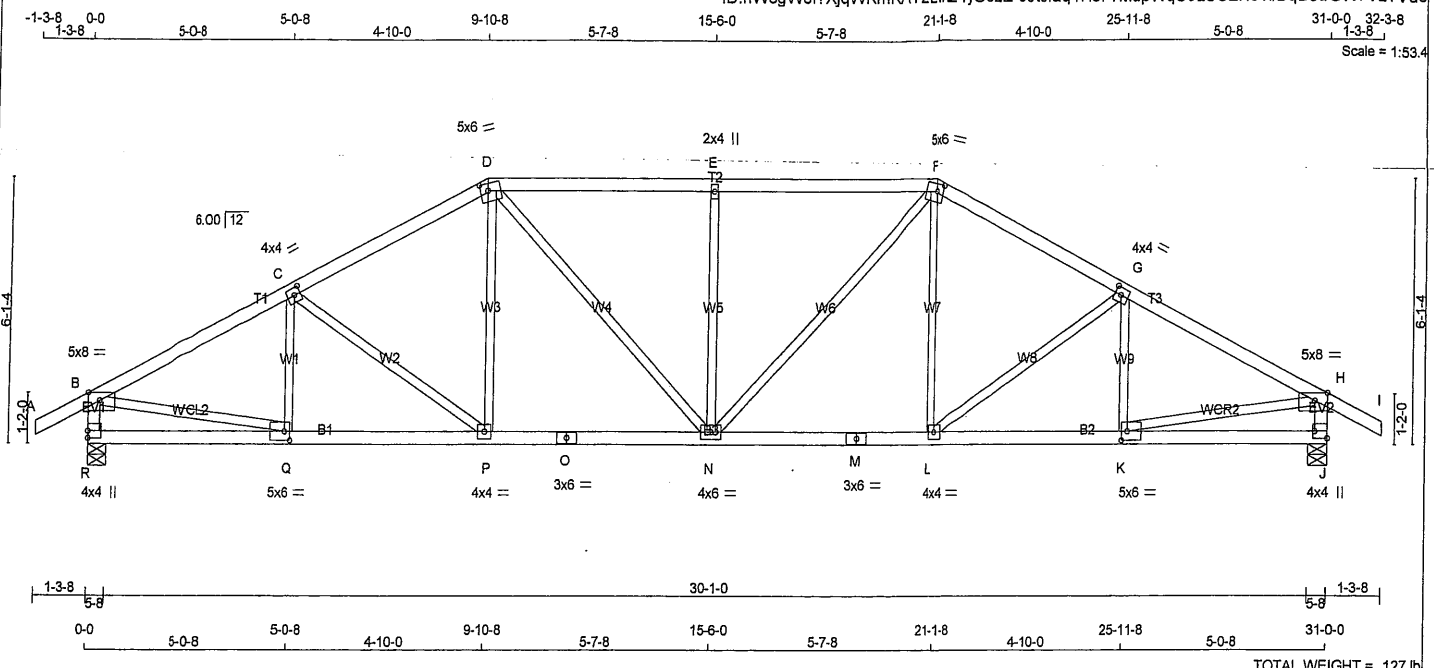


TOTAL WEIGHT = 127 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	1.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	1.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2329	0	2329	0	0	5-8	5-8
J	2329	0	2329	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1840	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1840	1095 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

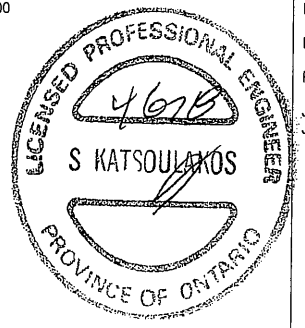
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-3127 / 0	-112.4	-112.4	0.48 (1)	3.54
C- D	-2859 / 0	-112.4	-112.4	0.45 (1)	3.71
D- E	-2985 / 0	-112.4	-112.4	0.58 (1)	3.45
E- F	-2985 / 0	-112.4	-112.4	0.58 (1)	3.45
F- G	-2859 / 0	-112.4	-112.4	0.45 (1)	3.71
G- H	-3127 / 0	-112.4	-112.4	0.48 (1)	3.54
H- I	0 / 34	-112.4	-112.4	0.15 (1)	10.00
R- B	-2266 / 0	0.0	0.0	0.23 (1)	5.66
J- H	-2266 / 0	0.0	0.0	0.23 (1)	5.66
R- Q	0 / 0	-28.0	-28.0	0.16 (3)	10.00
Q- P	0 / 2822	-28.0	-28.0	0.54 (1)	10.00
P- O	0 / 2536	-28.0	-28.0	0.50 (1)	10.00
O- N	0 / 2536	-28.0	-28.0	0.50 (1)	10.00
N- M	0 / 2536	-28.0	-28.0	0.50 (1)	10.00
M- L	0 / 2536	-28.0	-28.0	0.50 (1)	10.00
L- K	0 / 2822	-28.0	-28.0	0.54 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-339 / 49	0.07 (1)
C- P	-353 / 0	0.22 (1)
P- D	0 / 382	0.09 (2)
D- N	0 / 645	0.15 (1)
N- E	-774 / 0	0.45 (1)
N- F	0 / 645	0.15 (1)
L- F	0 / 382	0.09 (2)
L- G	-353 / 0	0.22 (1)
K- G	-339 / 49	0.07 (1)
B- Q	0 / 2862	0.64 (1)
K- H	0 / 2862	0.64 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.54/1.00 (P-Q:1) , WB=0.64/1.00 (B-Q:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.71 (O) (INPUT = 1.00)

DWG NO. TAM17034-19
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T53S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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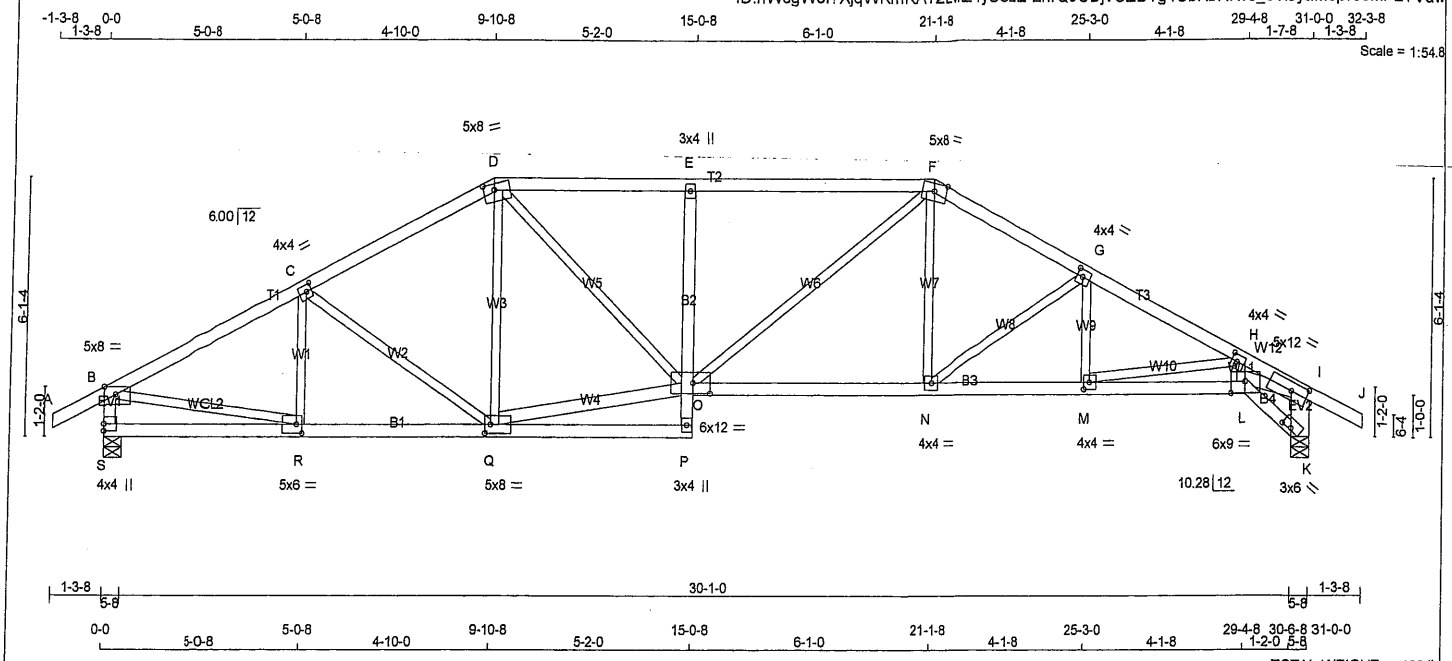

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C, G, H						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
I	TMVW-t	MT20	5.0	12.0	2.50	5.00
K	BVM1-I	MT20	3.0	6.0	0.50	3.00
L	BBWW-p	MT20	6.0	9.0	3.25	4.25
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BMWW-t	MT20	4.0	4.0		
O	BVMWWW-I	MT20	6.0	12.0	3.00	5.25
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	1.75
R	BMWW-t	MT20	5.0	6.0	2.50	1.75
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	2326	0	2326	0	0	5-8	5-8
K	2333	0	2333	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1837	1093 / 0	326 / 0	0 / 0	0 / 0	419 / 0	0 / 0
K	1842	1097 / 0	326 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
B- C	-3121 / 0	-112.4	-112.4	0.48 (1)	3.54	
C- D	-2860 / 0	-112.4	-112.4	0.45 (1)	3.71	
D- E	-3563 / 0	-112.4	-112.4	0.63 (1)	3.09	
E- F	-3579 / 0	-112.4	-112.4	0.73 (1)	3.01	
F- G	-3429 / 0	-112.4	-112.4	0.40 (1)	3.47	
G- H	-4189 / 0	-112.4	-112.4	0.50 (1)	3.04	
H- I	-5453 / 0	-112.4	-112.4	0.45 (1)	2.67	
I- J	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
S- B	-2261 / 0	0.0	0.0	0.23 (1)	5.66	
K- I	-2310 / 0	0.0	0.0	0.15 (1)	6.75	
S- R	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
R- Q	0 / 2816	-28.0	-28.0	0.55 (1)	10.00	
Q- P	0 / 75	-28.0	-28.0	0.18 (3)	10.00	
P- O	0 / 99	0.0	0.0	0.11 (1)	10.00	
O- E	-776 / 0	0.0	0.0	0.25 (1)	7.81	
O- N	0 / 3056	-28.0	-28.0	0.62 (1)	10.00	
N- M	0 / 3784	-28.0	-28.0	0.73 (1)	10.00	
M- L	0 / 4733	-28.0	-28.0	0.84 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.02 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-344 / 41	0.08 (1)
C- Q	-348 / 0	0.22 (1)
Q- D	-129 / 118	0.08 (1)
Q- O	0 / 2503	0.40 (1)
D- O	0 / 1430	0.32 (1)
O- F	0 / 661	0.15 (1)
N- F	0 / 662	0.15 (1)
N- G	-884 / 0	0.37 (1)
M- G	0 / 282	0.06 (2)
M- H	-964 / 0	0.28 (1)
L- H	0 / 604	0.14 (1)
L- I	0 / 4846	0.78 (1)
B- R	0 / 2856	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 815 (0.46")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.84/1.00 (L-M:1) , WB=0.78/1.00 (I-L:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.70 (R) (INPUT = 1.00)

DWG NO. TAM 17810-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T54	1	3	OBC2012_6TCDL TRUSS DESC.	

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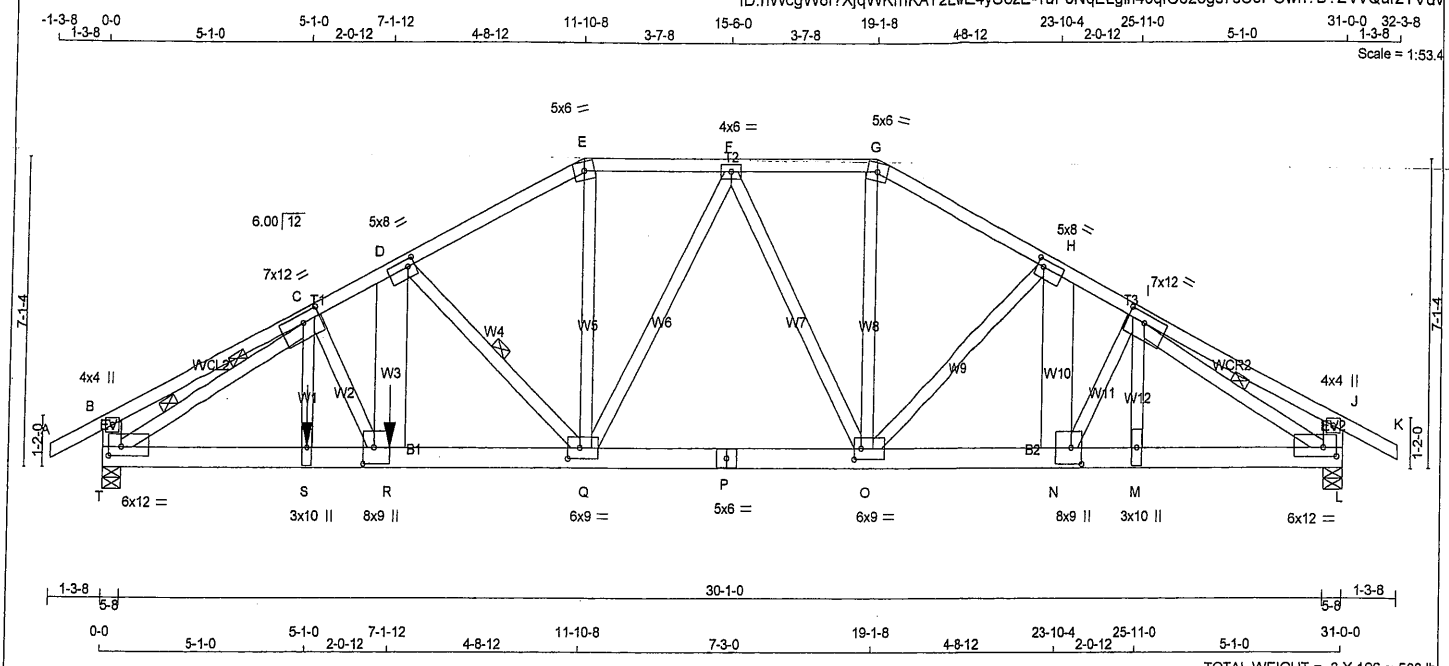

TOTAL WEIGHT = 3 X 196 = 588 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E,	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - D	2x10	DRY	No.2	SPF
N - H	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- G	1	12	TOP
G- K	1	12	TOP
T- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- P	2	9	SIDE(494.5)
P- L	2	9	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
C- S	2	4	SIDE(792.7)
2x4	1	6	
I- M	2	4	
D- R	5	4	SIDE(2661.1)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWWW-t	MT20	7.0	12.0	2.50	5.25
D	TMWW-t	MT20	5.0	8.0	2.00	2.00
E	TTW-m	MT20	5.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	8.0	2.00	2.00
I	TMWWW-t	MT20	7.0	12.0	2.50	5.25
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	6.0	12.0	2.50	3.75
M	BMW+w	MT20	3.0	10.0		
N	BMWW+t	MT20	8.0	9.0	4.50	3.25
O	BMWWW-t	MT20	6.0	9.0	3.00	2.00
P	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	13165	0	13165	0	0	5-8	5-8
L	5214	0	5214	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	10349	6239 / 0	1774 / 0	0 / 0	0 / 0	2336 / 0	0 / 0
L	4108	2461 / 0	715 / 0	0 / 0	0 / 0	932 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-Q, I-L.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.05 (1)	10.00
B- C	-6 / 1	-112.4	-112.4	0.16 (1)	10.00
C- D	-20665 / 0	-112.4	-112.4	0.55 (1)	2.23
D- E	-11693 / 0	-112.4	-112.4	0.37 (1)	3.36
E- F	-10544 / 0	-112.4	-112.4	0.20 (1)	3.63
F- G	-7547 / 0	-112.4	-112.4	0.13 (1)	4.26
G- H	-8379 / 0	-112.4	-112.4	0.24 (1)	4.00
H- I	-8407 / 0	-112.4	-112.4	0.16 (1)	4.03
I- J	0 / 15	-112.4	-112.4	0.13 (1)	10.00
J- K	0 / 34	-112.4	-112.4	0.05 (1)	10.00
T- B	-435 / 0	0.0	0.0	0.01 (1)	7.81
L- J	-404 / 0	0.0	0.0	0.01 (1)	7.81
T- S	0 / 19078	-28.0	-28.0	0.43 (1)	10.00
S- R	0 / 19078	-28.0	-28.0	0.43 (1)	10.00
R- Q	0 / 18505	-28.0	-28.0	0.35 (1)	10.00
Q- P	0 / 9157	-28.0	-28.0	0.16 (1)	10.00
P- O	0 / 9157	-28.0	-28.0	0.16 (1)	10.00
O- N	0 / 7518	-28.0	-28.0	0.13 (1)	10.00
N- M	0 / 7176	-28.0	-28.0	0.15 (1)	10.00
M- L	0 / 7176	-28.0	-28.0	0.15 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	0 / 3764	0.20 (1)
C- R	-1085 / 0	0.06 (1)
R- D	0 / 10742	0.30 (1)
D- Q	-10806 / 0	0.72 (1)
Q- E	0 / 4806	0.26 (1)
Q- F	0 / 3020	0.16 (1)
F- O	-3506 / 0	0.93 (1)
O- G	0 / 3334	0.18 (1)
O- H	-58 / 0	0.01 (1)
N- H	-367 / 0	0.01 (1)
N- I	0 / 647	0.03 (1)
M- I	-67 / 142	0.01 (3)
T- C	-22779 / 0	0.93 (1)
I- L	-8568 / 0	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	7-1-12	-9546	-9546	—	FRONT	VERT	TOTAL
S	5-1-0	-4174	-4174	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.43/1.00 (S-T:1) , WB=0.93/1.00 (F-O:1) , SSI=0.07/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.99 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4618 STEVE SOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM V7811-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T54	1	3	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWWW-t	MT20	6.0	9.0	3.00	3.50
R	BMWW+t	MT20	8.0	9.0	4.50	3.25
S	BMW+w	MT20	3.0	10.0		
T	BMVW1-t	MT20	6.0	12.0	2.50	3.75

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4174.5 lbs FACTORED DOWN AT 5-1-0, AND 9546.3 lbs FACTORED DOWN AT 7-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17011-R8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC, OBC2012_6TCDL	DRWG NO.
287657	T55A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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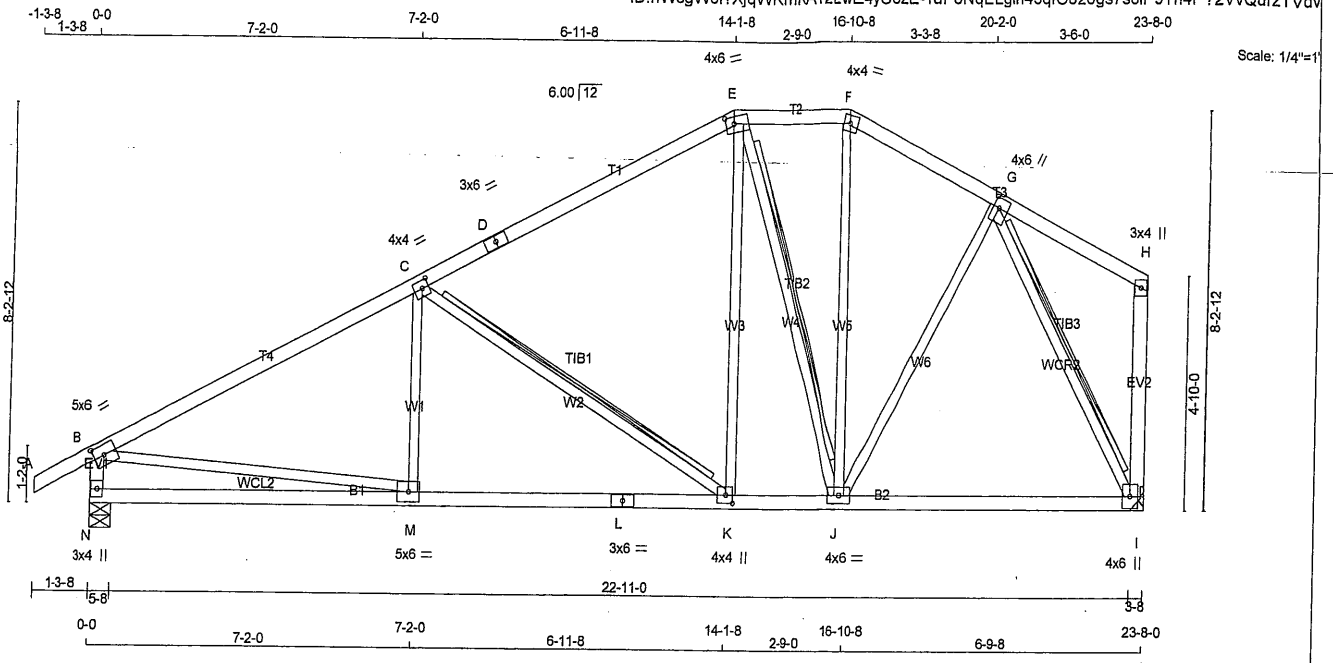

TOTAL WEIGHT = 110 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0	2.50	1.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW+t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1814	0	1814	0	0	5-8	5-8
I	1662	0	1662	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	857 / 0	249 / 0	0 / 0	0 / 0	324 / 0	0 / 0
I	1325	769 / 0	249 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-K, G-I
2x3 DRY SPF No.2 T-BRACE AT E-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-2254 / 0	-112.4	-112.4	0.90 (1)	3.28
C- D	-1377 / 0	-112.4	-112.4	0.78 (1)	4.27
D- E	-1377 / 0	-112.4	-112.4	0.78 (1)	4.27
E- F	-1038 / 0	-112.4	-112.4	0.12 (1)	6.04
F- G	-1177 / 0	-112.4	-112.4	0.17 (1)	5.70
G- H	0 / 19	-112.4	-112.4	0.20 (1)	10.00
N- B	-1734 / 0	0.0	0.0	0.18 (1)	6.32
I- H	-148 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 0	-28.0	-28.0	0.39 (3)	10.00
M- L	0 / 2054	-28.0	-28.0	0.59 (2)	10.00
L- K	0 / 2054	-28.0	-28.0	0.59 (2)	10.00
K- J	0 / 1200	-28.0	-28.0	0.42 (2)	10.00
J- I	0 / 783	-28.0	-28.0	0.37 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-15 / 300	0.07 (3)
C- K	-1019 / 0	0.54 (1)
K- E	0 / 638	0.14 (1)
E- J	-515 / 0	0.60 (1)
J- F	0 / 236	0.05 (2)
J- G	0 / 514	0.12 (1)
B- M	0 / 2069	0.47 (1)
G- I	-1635 / 0	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.90/1.00 (B-C:1) , BC=0.59/1.00 (K-M:2) , WB=0.60/1.00 (G-I:1) , SSI=0.35/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.68 (L) (INPUT = 1.00)

DWG NO. TAM M B12-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T56A	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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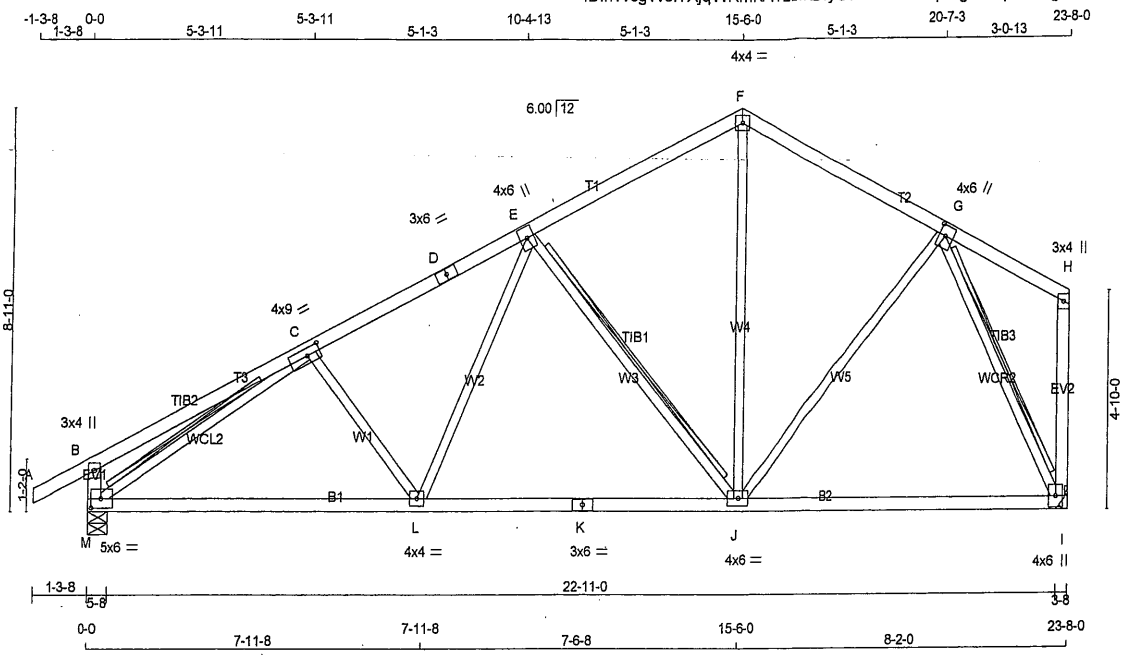

TOTAL WEIGHT = 2 X 103 = 205 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0	2.00	3.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-p	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0	2.75	1.75
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	1814	0	1814	0	0	5-8	5-8
I	1662	0	1662	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1429	857 / 0	249 / 0	0 / 0	0 / 0	324 / 0	0 / 0
I	1325	769 / 0	249 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-J
2x4 DRY SPF No.2 T-BRACE AT C-M, G-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C- L	-279 / 53	0.09 (1)
B- C	0 / 24	-112.4	-112.4	0.38 (1)	10.00	L- E	0 / 490	0.11 (1)
C- D	-2046 / 0	-112.4	-112.4	0.38 (1)	4.38	E- J	-917 / 0	0.93 (1)
D- E	-2046 / 0	-112.4	-112.4	0.38 (1)	4.38	J- F	0 / 588	0.13 (1)
E- F	-1203 / 0	-112.4	-112.4	0.35 (1)	5.41	J- G	0 / 471	0.11 (1)
F- G	-1200 / 0	-112.4	-112.4	0.35 (1)	5.41	M- C	-2403 / 0	0.76 (1)
G- H	0 / 38	-112.4	-112.4	0.36 (1)	10.00	G- I	-1661 / 0	0.56 (1)
M- B	-392 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	-87 / 0	0.0	0.0	0.04 (1)	7.81			
M- L	0 / 2002	-28.0	-28.0	0.59 (2)	10.00			
L- K	0 / 1644	-28.0	-28.0	0.60 (2)	10.00			
K- J	0 / 1644	-28.0	-28.0	0.60 (2)	10.00			
J- I	0 / 750	-28.0	-28.0	0.50 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 966 (0.29")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.60/1.00 (J-L:2) , WB=0.93/1.00 (E-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.50 (M) (INPUT = 1.00)

DWG NO. TAM17813-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T57	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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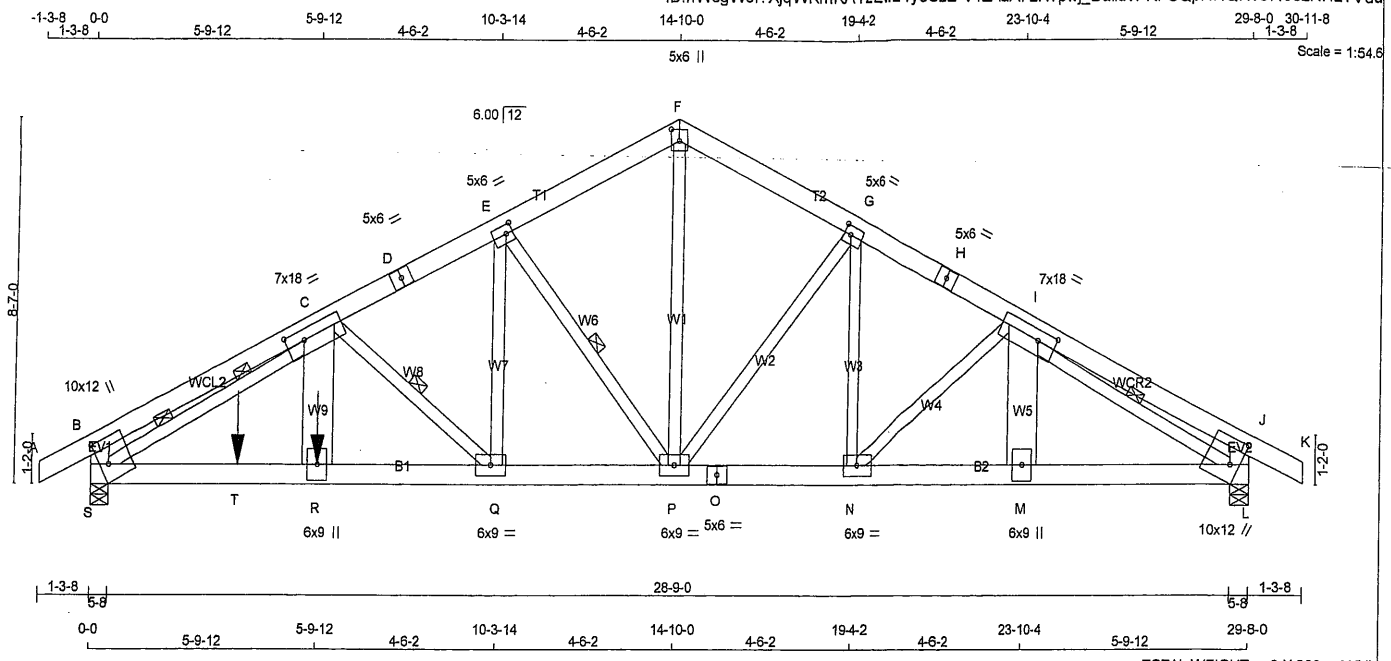


TOTAL WEIGHT = 3 X 202 = 605 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
M - I	2x10	DRY	No.2	SPF
R - C	2x10	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- F	2	12	TOP
F- H	2	12	TOP
H- K	2	12	TOP
S- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	3	5	SIDE(1285.8)
O- L	3	5	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
C- R	5	4	SIDE(1666.3)
2x10	5	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, J, L, S						
B	TMBMVW1*+h	MT20	10.0	12.0	7.25	2.50
C	TMWWW-t	MT20	7.0	18.0	3.00	5.50
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.00
F	TTW+p	MT20	5.0	6.0	3.25	2.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.00
H	TS-t	MT20	5.0	6.0		
I	TMWWW-t	MT20	7.0	18.0	3.00	5.50
L	TMBMVW1*+h	MT20	10.0	12.0	7.25	2.50
M	BMW+w	MT20	6.0	9.0		
N	BMWW-t	MT20	6.0	9.0		
O	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	13758	0	13758	0	0	5-8	5-8
L	4680	0	4680	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	10812	6522 / 0	1851 / 0	0 / 0	0 / 0	2440 / 0	0 / 0
L	3687	2209 / 0	642 / 0	0 / 0	0 / 0	836 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, C-Q, I-L.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

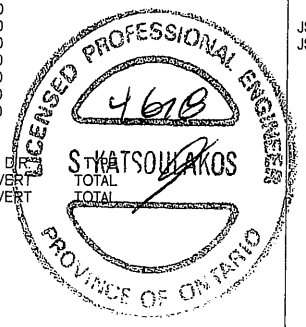
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-112.4	-112.4	0.03 (1)	10.00	P- F	0 / 5996	0.32 (1)
B- C	-37 / 0	-112.4	-112.4	0.17 (1)	6.25	P- G	-486 / 0	0.12 (1)
C- D	-11570 / 0	-112.4	-112.4	0.20 (1)	4.25	N- G	0 / 167	0.01 (3)
D- E	-11570 / 0	-112.4	-112.4	0.20 (1)	4.25	N- I	0 / 67	0.00 (1)
E- F	-7204 / 0	-112.4	-112.4	0.10 (1)	5.25	M- I	0 / 266	0.01 (3)
F- G	-7185 / 0	-112.4	-112.4	0.09 (1)	5.27	E- P	-6544 / 0	0.55 (1)
G- H	-7515 / 0	-112.4	-112.4	0.07 (1)	5.19	Q- E	0 / 6436	0.34 (1)
H- I	-7515 / 0	-112.4	-112.4	0.07 (1)	5.19	C- Q	-11408 / 0	0.62 (1)
I- J	-3 / 13	-112.4	-112.4	0.09 (1)	10.00	R- C	0 / 13421	0.37 (1)
J- K	0 / 35	-112.4	-112.4	0.03 (1)	10.00	S- C	-22647 / 0	0.97 (1)
S- B	-565 / 0	0.0	0.0	0.01 (1)	7.81	I- L	-7831 / 0	0.49 (1)
L- J	-454 / 0	0.0	0.0	0.01 (1)	7.81			
S- T	0 / 19266	-28.0	-28.0	0.87 (1)	10.00			
T- R	0 / 19266	-28.0	-28.0	0.87 (1)	10.00			
R- Q	0 / 19383	-28.0	-28.0	0.52 (1)	10.00			
Q- P	0 / 10417	-28.0	-28.0	0.24 (1)	10.00			
P- O	0 / 6715	-28.0	-28.0	0.15 (1)	10.00			
O- N	0 / 6715	-28.0	-28.0	0.15 (1)	10.00			
N- M	0 / 6663	-28.0	-28.0	0.13 (1)	10.00			
M- L	0 / 6662	-28.0	-28.0	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYP
R	5-9-12	-9719	-9719	---	BACK	VERT	TOTAL
T	3-9-0	-4239	-4239	---	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.20/1.00 (C-E:1) , BC=0.87/1.00 (R-S:1) , WB=0.97/1.00 (C-S:1) , SSI=0.63/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.98 (C) (INPUT = 1.00)

DWG NO. TAM 17014-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T57	1	3	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMWWW-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	6.0	9.0		
R	BMW+w	MT20	6.0	9.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4239.0 lbs FACTORED DOWN AT 3-9-0, AND 9719.3 lbs FACTORED DOWN AT 5-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17814-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T58	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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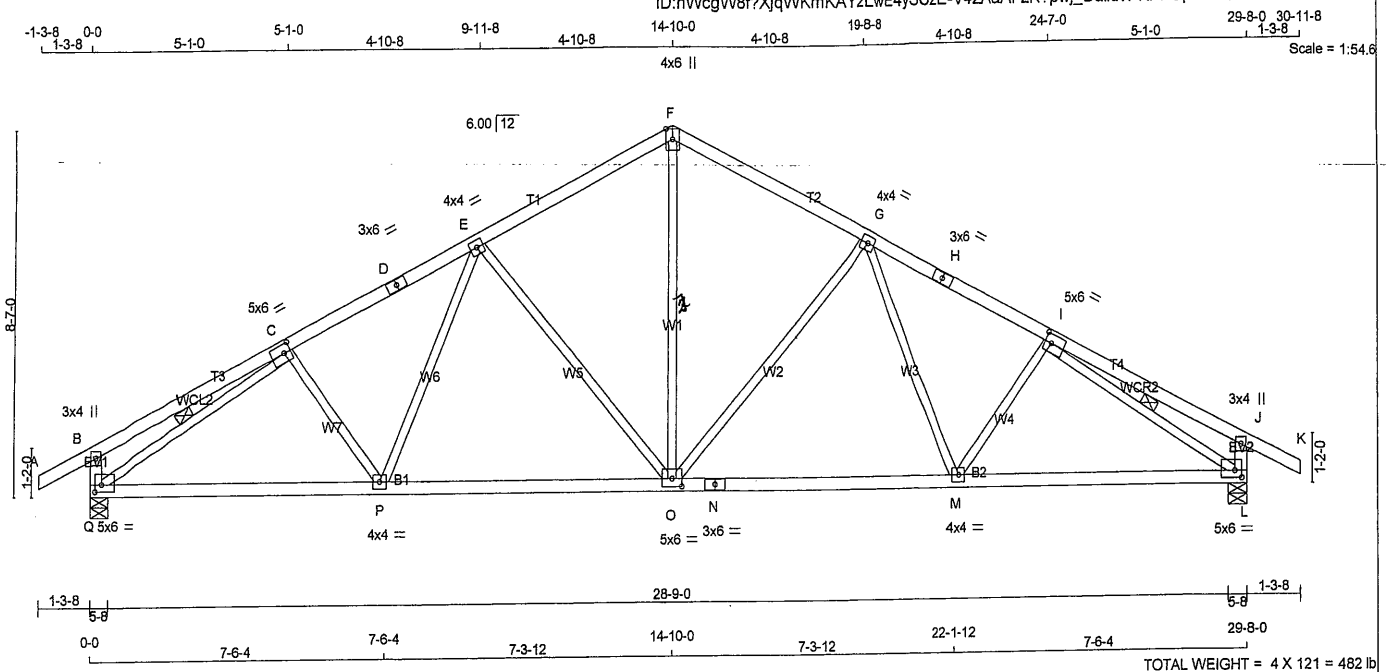

TOTAL WEIGHT = 4 X 121 = 482 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.00	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.25	3.00
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2236	0	2236	0	0	5-8	5-8
L	2236	0	2236	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0
L	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, I-L. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 21	-112.4	-112.4	0.33 (1)	10.00
C- D	-2846 / 0	-112.4	-112.4	0.40 (1)	3.78
D- E	-2846 / 0	-112.4	-112.4	0.40 (1)	3.78
E- F	-2127 / 0	-112.4	-112.4	0.38 (1)	4.30
F- G	-2127 / 0	-112.4	-112.4	0.38 (1)	4.30
G- H	-2846 / 0	-112.4	-112.4	0.40 (1)	3.78
H- I	-2846 / 0	-112.4	-112.4	0.40 (1)	3.78
I- J	0 / 21	-112.4	-112.4	0.33 (1)	10.00
J- K	0 / 34	-112.4	-112.4	0.15 (1)	10.00
Q- B	-385 / 0	0.0	0.0	0.04 (1)	7.81
L- J	-385 / 0	0.0	0.0	0.04 (1)	7.81
Q- P	0 / 2626	-28.0	-28.0	0.65 (2)	10.00
P- O	0 / 2408	-28.0	-28.0	0.63 (2)	10.00
O- N	0 / 2408	-28.0	-28.0	0.63 (2)	10.00
N- M	0 / 2408	-28.0	-28.0	0.63 (2)	10.00
M- L	0 / 2626	-28.0	-28.0	0.65 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1428	0.32 (1)
O- G	-809 / 0	0.96 (1)
G- M	0 / 401	0.09 (2)
M- I	-132 / 110	0.04 (1)
E- O	-809 / 0	0.96 (1)
P- E	0 / 401	0.09 (2)
C- P	-132 / 110	0.04 (1)
Q- C	-3164 / 0	0.80 (1)
I- L	-3164 / 0	0.80 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.40/1.00 (C-E:1) , BC=0.65/1.00 (P-Q:2) , WB=0.96/1.00 (E-O:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 1.00)

DWG NO. TAM 17815-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T58A	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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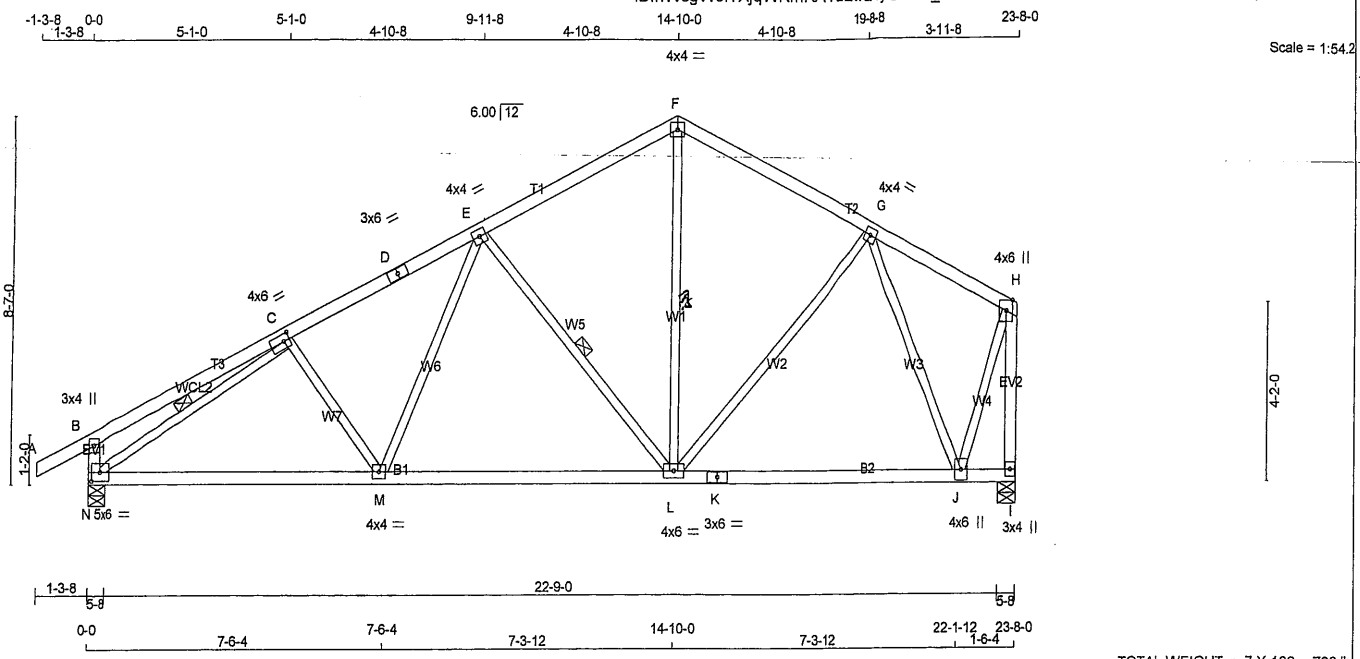

TOTAL WEIGHT = 7 X 103 = 723 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-p	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1814	0	1814	0	0	5-8	5-8
I	1662	0	1662	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	857 / 0	249 / 0	0 / 0	0 / 0	324 / 0	0 / 0
I	1325	769 / 0	249 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.34 (1)	10.00
C- D	-2082 / 0	-112.4	-112.4	0.35 (1)	4.38
D- E	-2082 / 0	-112.4	-112.4	0.35 (1)	4.38
E- F	-1284 / 0	-112.4	-112.4	0.33 (1)	5.30
F- G	-1285 / 0	-112.4	-112.4	0.33 (1)	5.30
G- H	-522 / 0	-112.4	-112.4	0.29 (1)	6.25
N- B	-384 / 0	0.0	0.0	0.04 (1)	7.81
I- H	-1702 / 0	0.0	0.0	0.49 (1)	6.37
N- M	0 / 2001	-28.0	-28.0	0.58 (2)	10.00
M- L	0 / 1692	-28.0	-28.0	0.55 (2)	10.00
L- K	0 / 963	-28.0	-28.0	0.40 (2)	10.00
K- J	0 / 963	-28.0	-28.0	0.40 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- F	0 / 677	0.15 (1)
L- G	0 / 252	0.06 (1)
G- J	-1222 / 0	0.87 (1)
J- H	0 / 1399	0.31 (1)
E- L	-867 / 0	0.34 (1)
M- E	0 / 456	0.10 (2)
C- M	-231 / 65	0.07 (1)
N- C	-2411 / 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.49/1.00 (H-I:1) , BC=0.58/1.00 (M-N:2) , WB=0.87/1.00 (G-J:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.59 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17816-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T60X	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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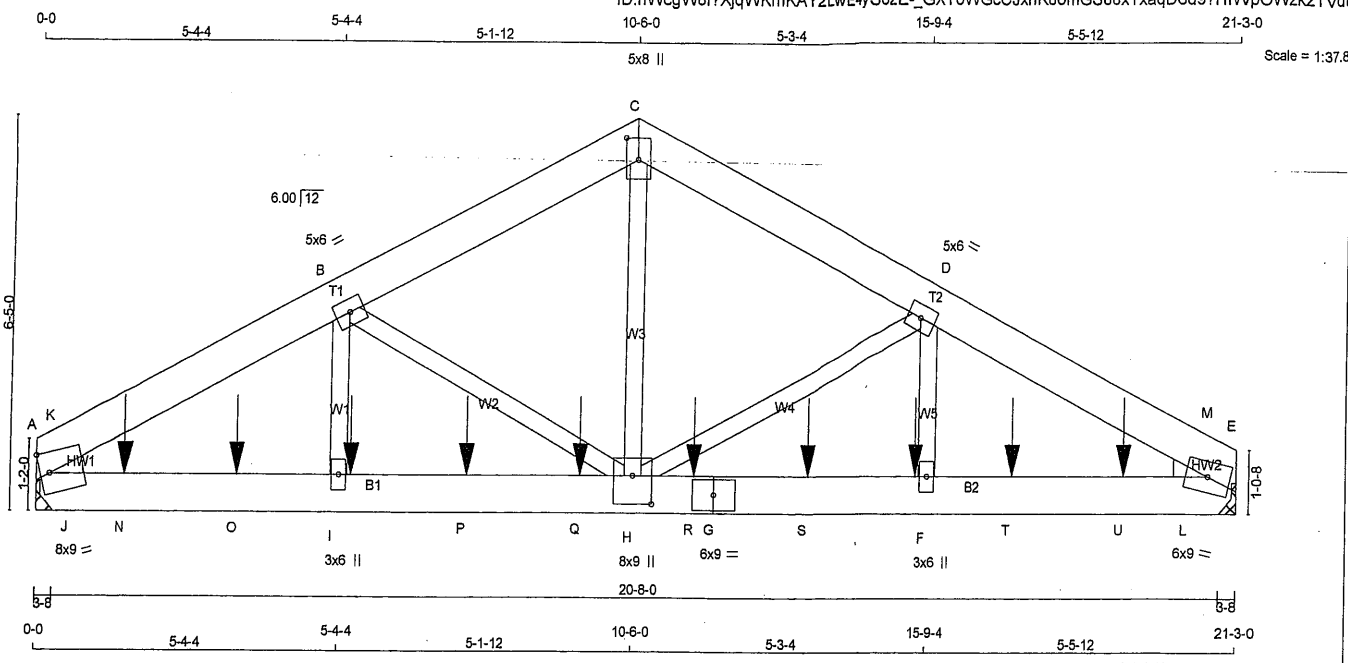

TOTAL WEIGHT = 3 X 134 = 401 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - E	2x8	DRY	No.2	SPF
A - G	2x8	DRY	No.2	SPF
G - E	2x8	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 2	12	TOP
C- E 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
A- G 2	7	SIDE(635.2)
G- E 2	9	SIDE(638.6)
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	8.0	9.0		Edge
B	TMWW-t	MT20	5.0	6.0		
C	TTW+p	MT20	5.0	8.0	4.25	2.50
D	TMWW-t	MT20	5.0	6.0		
E	TMBH1-m	MT20	6.0	9.0		
F	BMW+w	MT20	3.0	6.0		
G	BS-t	MT20	6.0	9.0		
H	BMWWW+t	MT20	8.0	9.0	5.50	4.00
I	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
A	9748	0	9748	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 L
E	9574	0	9574	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	7707	4574 / 0	1373 / 0	0 / 0	0 / 0	1760 / 0	0 / 0
E	7570	4492 / 0	1349 / 0	0 / 0	0 / 0	1729 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- K	-18371 / 0	-112.4	-112.4	0.41 (1)	3.66
K- B	-14251 / 0	-112.4	-112.4	0.38 (1)	4.14
B- C	-10819 / 0	-112.4	-112.4	0.23 (1)	4.84
C- D	-10846 / 0	-112.4	-112.4	0.19 (1)	4.88
D- M	-14953 / 0	-112.4	-112.4	0.24 (1)	4.23
M- E	-17719 / 0	-112.4	-112.4	0.26 (1)	3.91
A- J	0 / 12469	-28.0	-28.0	0.76 (1)	10.00
J- N	0 / 12469	-28.0	-28.0	0.76 (1)	10.00
N- O	0 / 12469	-28.0	-28.0	0.76 (1)	10.00
O- I	0 / 12469	-28.0	-28.0	0.76 (1)	10.00
I- P	0 / 12469	-28.0	-28.0	0.78 (1)	10.00
P- Q	0 / 12469	-28.0	-28.0	0.78 (1)	10.00
Q- H	0 / 12469	-28.0	-28.0	0.78 (1)	10.00
H- R	0 / 13199	-28.0	-28.0	0.80 (1)	10.00
R- G	0 / 13199	-28.0	-28.0	0.80 (1)	10.00
G- S	0 / 13199	-28.0	-28.0	0.80 (1)	10.00
S- F	0 / 13199	-28.0	-28.0	0.80 (1)	10.00
F- T	0 / 13199	-28.0	-28.0	0.71 (1)	10.00
T- U	0 / 13199	-28.0	-28.0	0.71 (1)	10.00
U- L	0 / 13199	-28.0	-28.0	0.71 (1)	10.00
L- E	0 / 13199	-28.0	-28.0	0.71 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 3402	0.18 (1)
B- H	-3177 / 0	0.44 (1)
H- C	0 / 8772	0.47 (1)
H- D	-3990 / 0	0.58 (1)
F- D	0 / 3813	0.20 (1)
J- K	0 / 6413	0.00 (1)
L- M	0 / 4517	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
F	15-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
I	5-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
N	1-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
O	3-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
P	7-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
Q	9-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
R	11-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
S	13-6-12	-1634	-1634	---	FRONT	VERT	TOTAL
T	17-3-12	-1634	-1634	---	FRONT	VERT	TOTAL
U	19-3-12	-1634	-1634	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41/1.00 (A-K:1) , BC=0.80/1.00 (F-H:1) , WB=0.58/1.00 (D-H:1) , SSI=0.70/1.00 (A-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.75 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4618 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17817-A
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T60X	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1633.7 lbs FACTORED DOWN AT 1-6-12, 1633.7 lbs FACTORED DOWN AT 3-6-12, 1633.7 lbs FACTORED DOWN AT 5-6-12, 1633.7 lbs FACTORED DOWN AT 7-6-12, 1633.7 lbs FACTORED DOWN AT 9-6-12, 1633.7 lbs FACTORED DOWN AT 11-6-12, 1633.7 lbs FACTORED DOWN AT 13-6-12, 1633.7 lbs FACTORED DOWN AT 15-6-12, AND 1633.7 lbs FACTORED DOWN AT 17-3-12, AND 1633.7 lbs FACTORED DOWN AT 19-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17817-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T61X	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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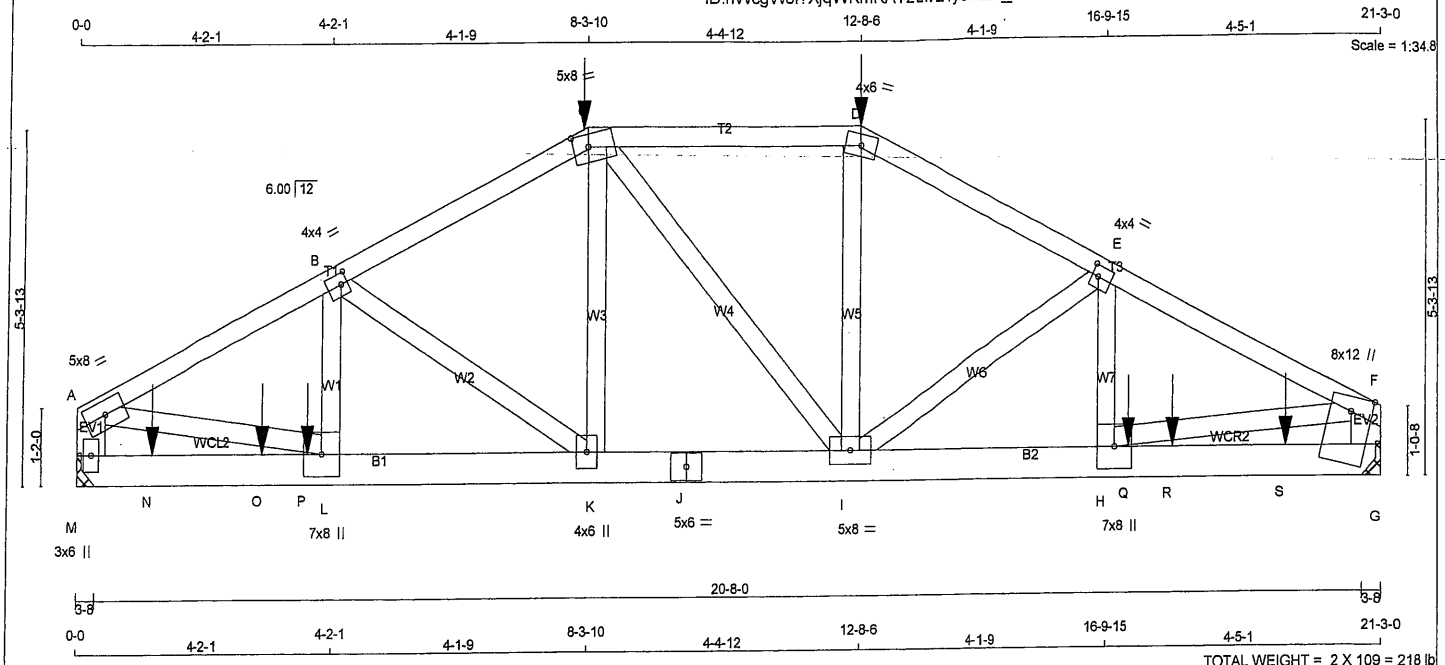

TOTAL WEIGHT = 2 X 109 = 218 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
M- A	2	12	TOP
G- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(183.1)
J- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- C	1	6	SIDE(57.1)
I- D	1	6	SIDE(57.1)
B- L	1	3	SIDE(331.9)
E- H	1	3	SIDE(331.9)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	2.25	3.00
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F						
F	TMBMVW1*+mMT20		8.0	12.0	Edge	
G						
H	BMWW+t	MT20	7.0	8.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	7.0	8.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	4267	0	4267	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
G	4202	0	4202	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	3327	2049 / 0	539 / 0	0 / 0	0 / 0	739 / 0	0 / 0
G	3277	2018 / 0	532 / 0	0 / 0	0 / 0	728 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	-6219 / 0	-112.4	-112.4	0.30 (1)	3.70		L- B	0 / 982	0.09 (1)
B- C	-4786 / 0	-112.4	-112.4	0.23 (1)	4.22		B- K	-1588 / 0	0.25 (1)
C- D	-4334 / 0	-112.4	-112.4	0.29 (1)	4.32		K- C	0 / 963	0.09 (1)
D- E	-4835 / 0	-112.4	-112.4	0.25 (1)	4.19		C- I	0 / 70	0.01 (1)
E- F	-6470 / 0	-112.4	-112.4	0.34 (1)	3.61		I- D	0 / 1041	0.09 (1)
M- A	-4018 / 0	0.0	0.0	0.14 (1)	7.10		I- E	-1806 / 0	0.28 (1)
G- F	-3960 / 0	0.0	0.0	0.14 (1)	7.13		H- E	0 / 1177	0.10 (1)
							A- L	0 / 5675	0.50 (1)
M- N	0 / 0	-28.0	-28.0	0.22 (1)	10.00		H- F	0 / 5866	0.52 (1)
N- O	0 / 0	-28.0	-28.0	0.22 (1)	10.00				
O- P	0 / 0	-28.0	-28.0	0.22 (1)	10.00				
P- L	0 / 0	-28.0	-28.0	0.22 (1)	10.00				
L- K	0 / 5580	-28.0	-28.0	0.41 (1)	10.00				
K- J	0 / 4288	-28.0	-28.0	0.31 (1)	10.00				
J- I	0 / 4288	-28.0	-28.0	0.31 (1)	10.00				
I- H	0 / 5804	-28.0	-28.0	0.42 (1)	10.00				
H- Q	0 / 0	-28.0	-28.0	0.23 (1)	10.00				
Q- R	0 / 0	-28.0	-28.0	0.23 (1)	10.00				
R- S	0 / 0	-28.0	-28.0	0.23 (1)	10.00				
S- G	0 / 0	-28.0	-28.0	0.23 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	8-3-10	-93	-104	---	FRONT	VERT	DEAD
C	8-3-10	-594	-594	---	FRONT	VERT	SNOW
D	12-8-6	-93	-104	---	FRONT	VERT	DEAD
D	12-8-6	-594	-594	---	FRONT	VERT	SNOW
N	1-2-12	-41	-72	---	BACK	VERT	TOTAL
O	3-0-12	-41	-72	---	BACK	VERT	TOTAL
P	3-9-8	-1974	-1974	---	BACK	VERT	TOTAL
Q	17-2-8	-1974	-1974	---	BACK	VERT	TOTAL
R	17-11-4	-41	-72	---	BACK	VERT	TOTAL
S	19-9-4	-41	-72	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.42/1.00 (H-I:1) , WB=0.52/1.00 (F-H:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.61 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4616 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM17018-T61X
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T61X	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 686.5 lbs FACTORED DOWN AT 8-3-10, AND 686.5 lbs FACTORED DOWN AT 12-8-6 ON TOP CHORD, AND 71.7 lbs FACTORED DOWN AT 1-2-12, 71.7 lbs FACTORED DOWN AT 3-0-12, 1974.2 lbs FACTORED DOWN AT 3-9-8, 1974.2 lbs FACTORED DOWN AT 17-2-8, AND 71.7 lbs FACTORED DOWN AT 17-11-4, AND 71.7 lbs FACTORED DOWN AT 19-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM N618-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T62	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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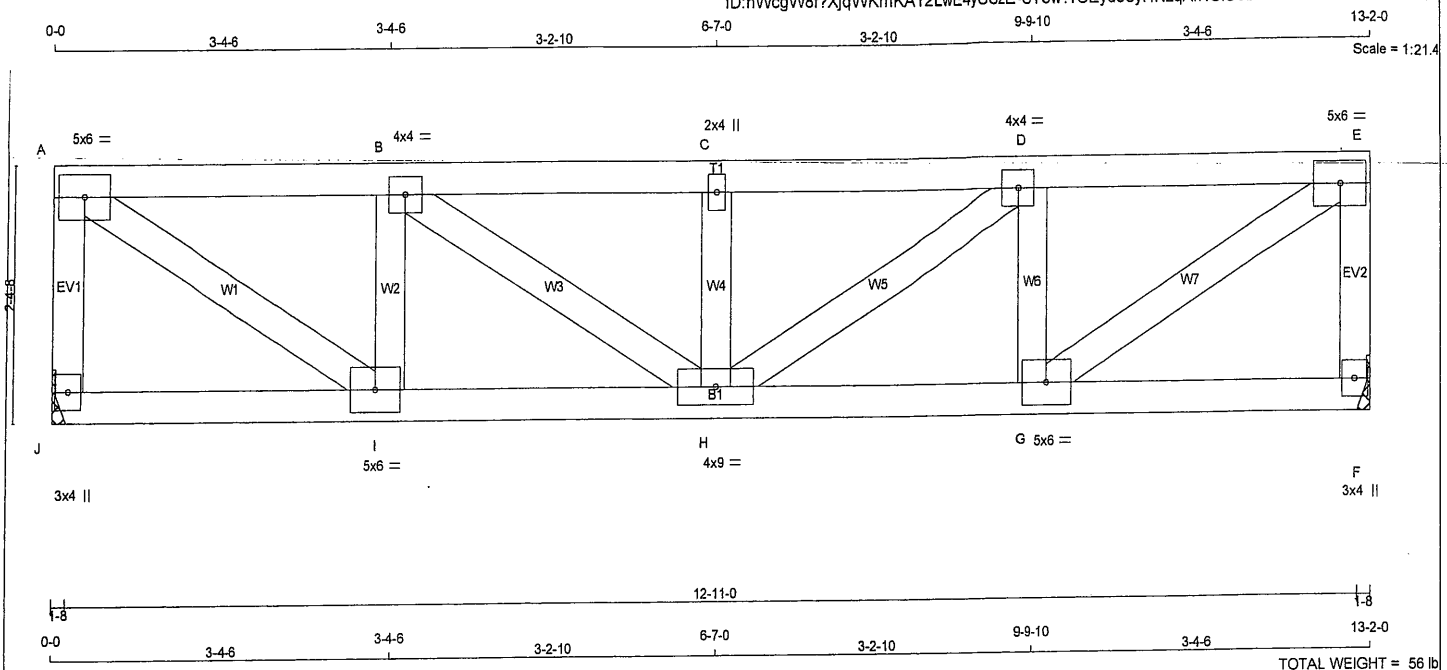

TOTAL WEIGHT = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	5.0	6.0	
B	TMWW-t	MT20	4.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	5.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW-t	MT20	5.0	6.0	
H	BMWWW-t	MT20	4.0	9.0	
I	BMWW-t	MT20	5.0	6.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1127	0	1127	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	1127	0	1127	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	898	521 / 0	168 / 0	0 / 0	0 / 0	209 / 0	0 / 0
F	898	521 / 0	168 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1043 / 0	0.0	0.0	0.14 (1)	7.66
A- B	-1434 / 0	-112.4	-112.4	0.20 (1)	5.20
B- C	-1823 / 0	-112.4	-112.4	0.22 (1)	4.71
C- D	-1823 / 0	-112.4	-112.4	0.22 (1)	4.71
D- E	-1434 / 0	-112.4	-112.4	0.20 (1)	5.20
F- E	-1043 / 0	0.0	0.0	0.14 (1)	7.66
J- I	0 / 0	-58.7	-58.7	0.10 (2)	10.00
I- H	0 / 1434	-58.7	-58.7	0.33 (1)	10.00
H- G	0 / 1434	-58.7	-58.7	0.33 (1)	10.00
G- F	0 / 0	-58.7	-58.7	0.10 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 1686	0.30 (1)
I- B	-671 / 0	0.08 (1)
B- H	0 / 463	0.08 (1)
H- C	-330 / 0	0.04 (1)
H- D	0 / 463	0.08 (1)
G- D	-671 / 0	0.08 (1)
G- E	0 / 1686	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-0-0
END DISTANCE = 13-2-0
END SPAN CARRIED = 3-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.33/1.00 (H-I:1) , WB=0.30/1.00 (A-I:1) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

DWG NO. TAM 17819-19
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T63	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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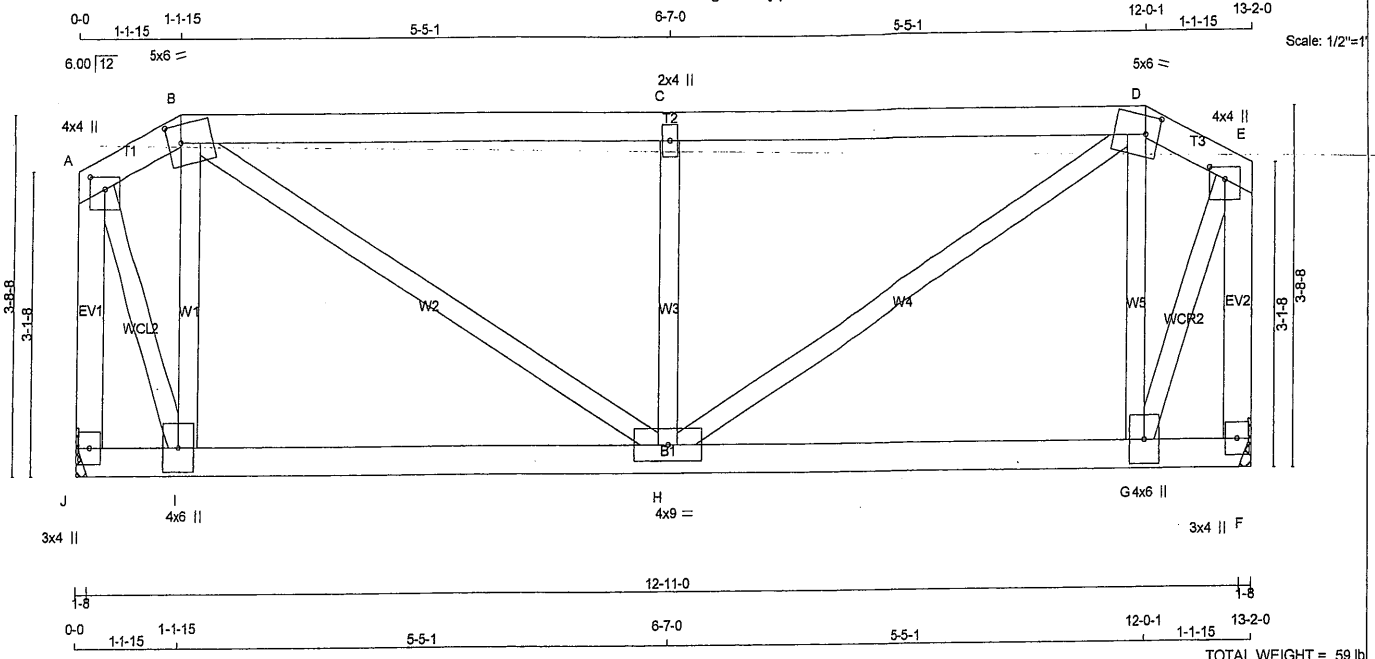


TOTAL WEIGHT = 59 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	924	0	924	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	924	0	924	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	737	428 / 0	138 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	737	428 / 0	138 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

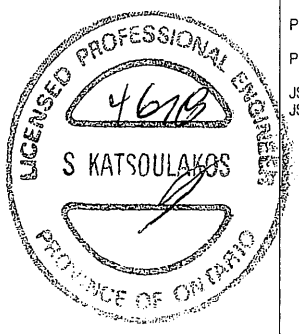
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-363 / 0	-112.4	-112.4	0.03 (1)		6.25
B- C	-1022 / 0	-112.4	-112.4	0.58 (1)		5.26
C- D	-1022 / 0	-112.4	-112.4	0.58 (1)		5.26
D- E	-363 / 0	-112.4	-112.4	0.03 (1)		6.25
J- A	-950 / 0	0.0	0.0	0.16 (1)		7.81
F- E	-950 / 0	0.0	0.0	0.16 (1)		7.81
J- I	0 / 0	-28.0	-28.0	0.13 (3)		10.00
I- H	0 / 306	-28.0	-28.0	0.20 (3)		10.00
H- G	0 / 306	-28.0	-28.0	0.20 (3)		10.00
G- F	0 / 0	-28.0	-28.0	0.13 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-585 / 0	0.13 (1)
B- H	0 / 845	0.19 (1)
H- C	-751 / 0	0.17 (1)
H- D	0 / 845	0.19 (1)
G- D	-585 / 0	0.13 (1)
A- I	0 / 790	0.18 (1)
G- E	0 / 790	0.18 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.20/1.00 (H-I:3) , WB=0.19/1.00 (D-H:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (FSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

DWG NO. TAM 17820-B7
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T64	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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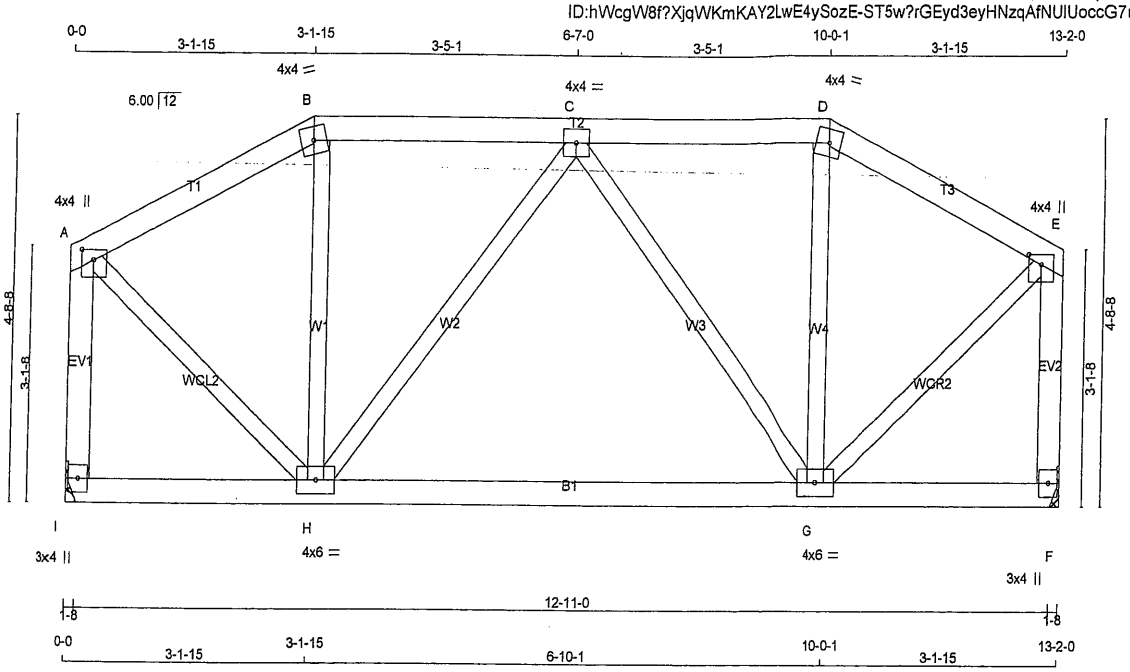


TOTAL WEIGHT = 59 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	924	0	924	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	924	0	924	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	737	428 / 0	138 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	737	428 / 0	138 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-595 / 0	-112.4	-112.4	0.20	(1)	6.25	H- B	-58 / 89	0.02 (3)
B- C	-530 / 0	-112.4	-112.4	0.22	(1)	6.25	H- C	-296 / 0	0.15 (1)
C- D	-530 / 0	-112.4	-112.4	0.22	(1)	6.25	C- G	-296 / 0	0.15 (1)
D- E	-595 / 0	-112.4	-112.4	0.20	(1)	6.25	G- D	-58 / 89	0.02 (3)
I- A	-904 / 0	0.0	0.0	0.15	(1)	7.81	A- H	0 / 702	0.16 (1)
F- E	-904 / 0	0.0	0.0	0.15	(1)	7.81	G- E	0 / 702	0.16 (1)
I- H	0 / 0	-28.0	-28.0	0.21	(3)	10.00			
H- G	0 / 708	-28.0	-28.0	0.27	(2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.21	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.27/1.00 (G-H:2) , WB=0.16/1.00 (A-H:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

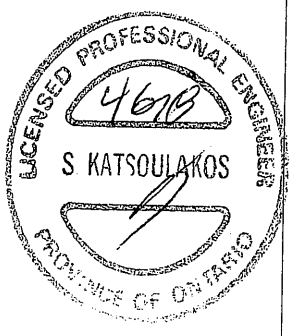
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)



DWG NO. TAM 17821-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T65	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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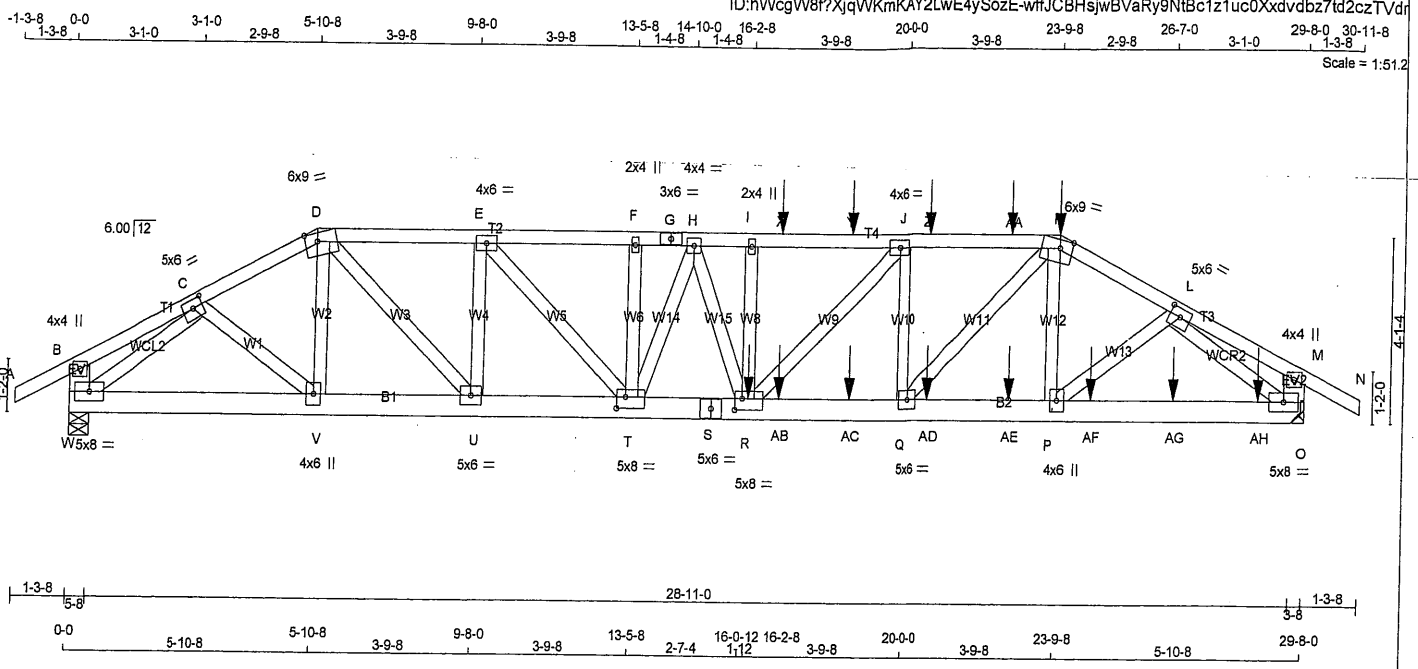

TOTAL WEIGHT = 2 X 163 = 325 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- K	1	12	SIDE(61.0)
K- N	1	12	SIDE(61.0)
W- B	2	12	TOP
O- M	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	SIDE(0.1)
S- O	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
P- K	1	6	SIDE(1.1)
I- R	1	6	SIDE(253.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	3.00
D	TTWW-m	MT20	6.0	9.0	2.25	3.50
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMW+w	MT20	2.0	4.0		
J	TMWW-t	MT20	4.0	6.0		
K	TTWW-m	MT20	6.0	9.0	2.25	3.50
L	TMWW-t	MT20	5.0	6.0	2.25	3.00
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0	3.00	2.00
S	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
W	3337	0	3337	0	0	IN-SX	IN-SX
O	4101	0	4101	0	0	5-8 HANGER BY OTHERS MIN. SEAT SIZE: 3-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2622	1582 / 0	449 / 0	0 / 0	0 / 0	591 / 0	0 / 0
O	3242	1925 / 0	577 / 0	0 / 0	0 / 0	741 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-112.4	-112.4	0.08 (1)	10.00		C- V	0 / 624	0.06 (1)
B- C	-1 / 10	-112.4	-112.4	0.07 (1)	10.00		V- D	-211 / 67	0.02 (1)
C- D	-4930 / 0	-112.4	-112.4	0.15 (1)	4.24		D- U	0 / 3011	0.27 (1)
D- E	-6556 / 0	-112.4	-112.4	0.29 (1)	3.63		U- E	-1993 / 0	0.18 (1)
E- F	-8064 / 0	-112.4	-112.4	0.37 (1)	3.21		E- T	0 / 2136	0.19 (1)
F- G	-8064 / 0	-112.4	-112.4	0.24 (1)	3.31		T- F	-302 / 0	0.03 (1)
G- H	-8064 / 0	-112.4	-112.4	0.24 (1)	3.31		R- I	-456 / 0	0.04 (1)
H- I	-8861 / 0	-112.4	-112.4	0.31 (1)	3.09		R- J	0 / 1601	0.14 (1)
I- X	-8861 / 0	-112.4	-112.4	0.52 (1)	2.89		Q- J	-1936 / 0	0.18 (1)
X- Y	-8861 / 0	-112.4	-112.4	0.52 (1)	2.89		Q- K	0 / 3074	0.27 (1)
Y- J	-8861 / 0	-112.4	-112.4	0.52 (1)	2.89		P- K	-250 / 215	0.02 (1)
J- Z	-7730 / 0	-112.4	-112.4	0.46 (1)	3.17		P- L	0 / 872	0.08 (1)
Z-AA	-7730 / 0	-112.4	-112.4	0.46 (1)	3.17		W- C	-4891 / 0	0.42 (1)
AA- K	-7730 / 0	-112.4	-112.4	0.46 (1)	3.17		L- O	-6061 / 0	0.52 (1)
K- L	-6193 / 0	-112.4	-112.4	0.20 (1)	3.79		T- H	-1174 / 0	0.12 (1)
L- M	-2 / 8	-112.4	-112.4	0.08 (1)	10.00		H- R	0 / 1129	0.10 (1)
M- N	0 / 34	-112.4	-112.4	0.08 (1)	10.00				
W- B	-298 / 0	0.0	0.0	0.01 (1)	7.81				
O- M	-303 / 0	0.0	0.0	0.01 (1)	7.81				
W- V	0 / 3895	-28.0	-28.0	0.29 (1)	10.00				
V- U	0 / 4387	-28.0	-28.0	0.31 (1)	10.00				
U- T	0 / 6556	-28.0	-28.0	0.47 (1)	10.00				
T- S	0 / 8470	-28.0	-28.0	0.62 (1)	10.00				
S- R	0 / 8470	-28.0	-28.0	0.62 (1)	10.00				
R-AB	0 / 7730	-28.0	-28.0	0.59 (1)	10.00				
AB-AC	0 / 7730	-28.0	-28.0	0.59 (1)	10.00				
AC- Q	0 / 7730	-28.0	-28.0	0.59 (1)	10.00				
Q-AD	0 / 5515	-28.0	-28.0	0.39 (1)	10.00				
AD-AE	0 / 5515	-28.0	-28.0	0.39 (1)	10.00				
AE- P	0 / 5515	-28.0	-28.0	0.39 (1)	10.00				
P-AF	0 / 4826	-28.0	-28.0	0.38 (1)	10.00				
AF-AG	0 / 4826	-28.0	-28.0	0.38 (1)	10.00				
AG-AH	0 / 4826	-28.0	-28.0	0.38 (1)	10.00				
AH- O	0 / 4826	-28.0	-28.0	0.38 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.52/1.00 (I-J:1) , BC=0.62/1.00 (R-T:1) , WB=0.52/1.00 (L-O:1) , SSI=0.19/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.79 (L) (INPUT = 1.00)

PROFESSIONAL ENGINEER 4678 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17022-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T65	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:hWcgW8f?XjqWKmKAY2LwE4ySozE-wffJCBHsjwBVaRy9NtBc1z1uc0Xxdvdbz7td2czTVdr

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWWW-t	MT20	5.0	8.0	3.00	2.50
U	BMWW-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMVW1-t	MT20	5.0	8.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 471.8 lbs FACTORED DOWN AT 23-9-8, 135.3 lbs FACTORED DOWN AT 16-11-4, 135.3 lbs FACTORED DOWN AT 18-7-4, AND 135.3 lbs FACTORED DOWN AT 20-7-4, AND 135.3 lbs FACTORED DOWN AT 22-7-4 ON TOP CHORD, AND 1674.0 lbs FACTORED DOWN AT 16-2-8, 69.9 lbs FACTORED DOWN AT 16-11-4, 69.9 lbs FACTORED DOWN AT 18-7-4, 69.9 lbs FACTORED DOWN AT 20-7-4, 69.9 lbs FACTORED DOWN AT 22-7-4, 69.9 lbs FACTORED DOWN AT 24-7-4, AND 69.9 lbs FACTORED DOWN AT 26-7-4, AND 69.9 lbs FACTORED DOWN AT 28-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	23-9-8	-64	-71	—	FRONT	VERT	DEAD
K	23-9-8	-408	-408	—	FRONT	VERT	SNOW
R	16-2-8	-1674	-1674	—	BACK	VERT	TOTAL
X	16-11-4	-135	-135	—	BACK	VERT	TOTAL
Y	18-7-4	-135	-135	—	BACK	VERT	TOTAL
Z	20-7-4	-135	-135	—	BACK	VERT	TOTAL
AA	22-7-4	-135	-135	—	BACK	VERT	TOTAL
AB	16-11-4	-40	-70	—	BACK	VERT	TOTAL
AC	18-7-4	-40	-70	—	BACK	VERT	TOTAL
AD	20-7-4	-40	-70	—	BACK	VERT	TOTAL
AE	22-7-4	-40	-70	—	BACK	VERT	TOTAL
AF	24-7-4	-40	-70	—	BACK	VERT	TOTAL
AG	26-7-4	-40	-70	—	BACK	VERT	TOTAL
AH	28-7-4	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
461B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17822-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T66	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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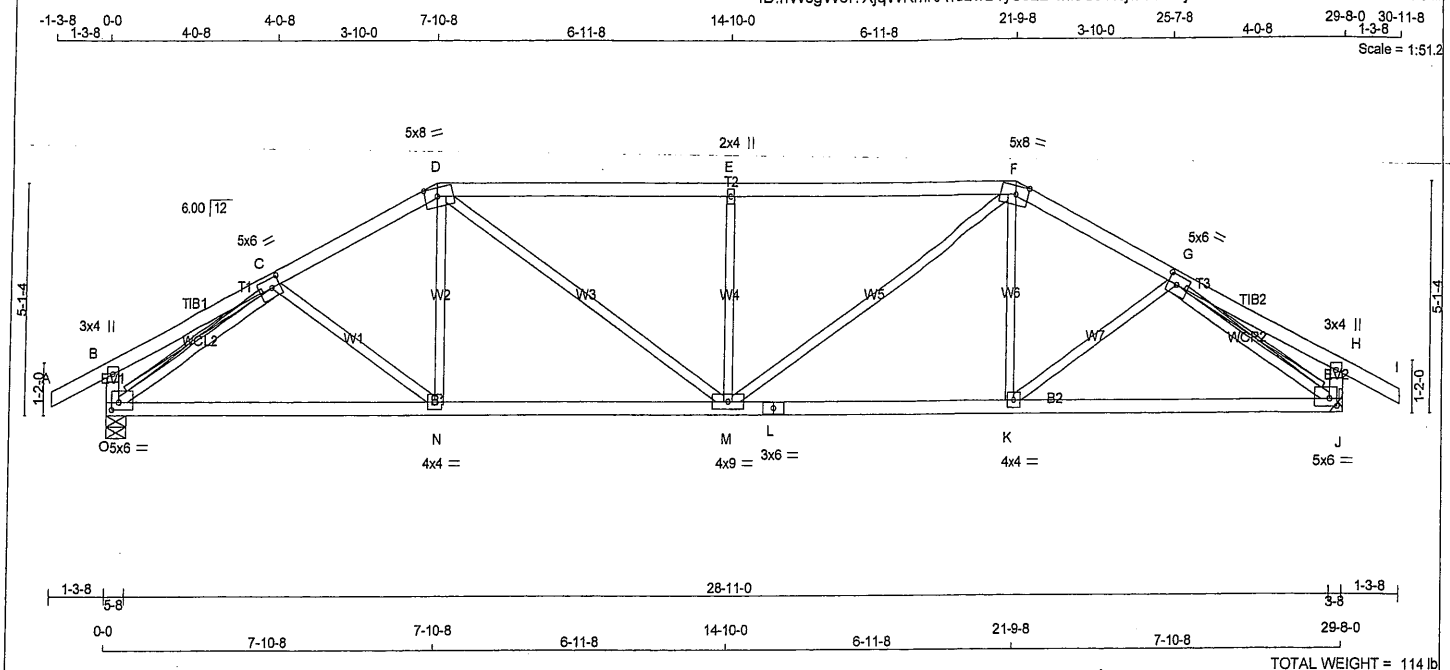

TOTAL WEIGHT = 114 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.00	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0	2.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2236	0	2236	0	0	5-8	5-8
J	2236	0	2236	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0
J	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-O, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 20	-112.4	-112.4	0.24 (1)		10.00
C- D	-2862 / 0	-112.4	-112.4	0.29 (1)		3.89
D- E	-3358 / 0	-112.4	-112.4	0.94 (1)		2.65
E- F	-3358 / 0	-112.4	-112.4	0.94 (1)		2.65
F- G	-2862 / 0	-112.4	-112.4	0.29 (1)		3.89
G- H	0 / 20	-112.4	-112.4	0.24 (1)		10.00
H- I	0 / 34	-112.4	-112.4	0.15 (1)		10.00
O- B	-331 / 0	0.0	0.0	0.03 (1)		7.81
J- H	-331 / 0	0.0	0.0	0.03 (1)		7.81
O- N	0 / 2530	-28.0	-28.0	0.68 (2)		10.00
N- M	0 / 2545	-28.0	-28.0	0.70 (2)		10.00
M- L	0 / 2545	-28.0	-28.0	0.70 (2)		10.00
L- K	0 / 2545	-28.0	-28.0	0.70 (2)		10.00
K- J	0 / 2530	-28.0	-28.0	0.68 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- N	0 / 167	0.04 (3)
N- D	0 / 346	0.08 (3)
D- M	0 / 988	0.22 (1)
M- E	-962 / 0	0.37 (1)
M- F	0 / 988	0.22 (1)
K- F	0 / 346	0.08 (3)
K- G	0 / 167	0.04 (3)
O- C	-3113 / 0	0.65 (1)
G- J	-3113 / 0	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.70/1.00 (K-M:2) , WB=0.65/1.00 (G-J:1) , SSI=0.38/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.77 (G) (INPUT = 1.00)

DWG NO. TAM NB23-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T67	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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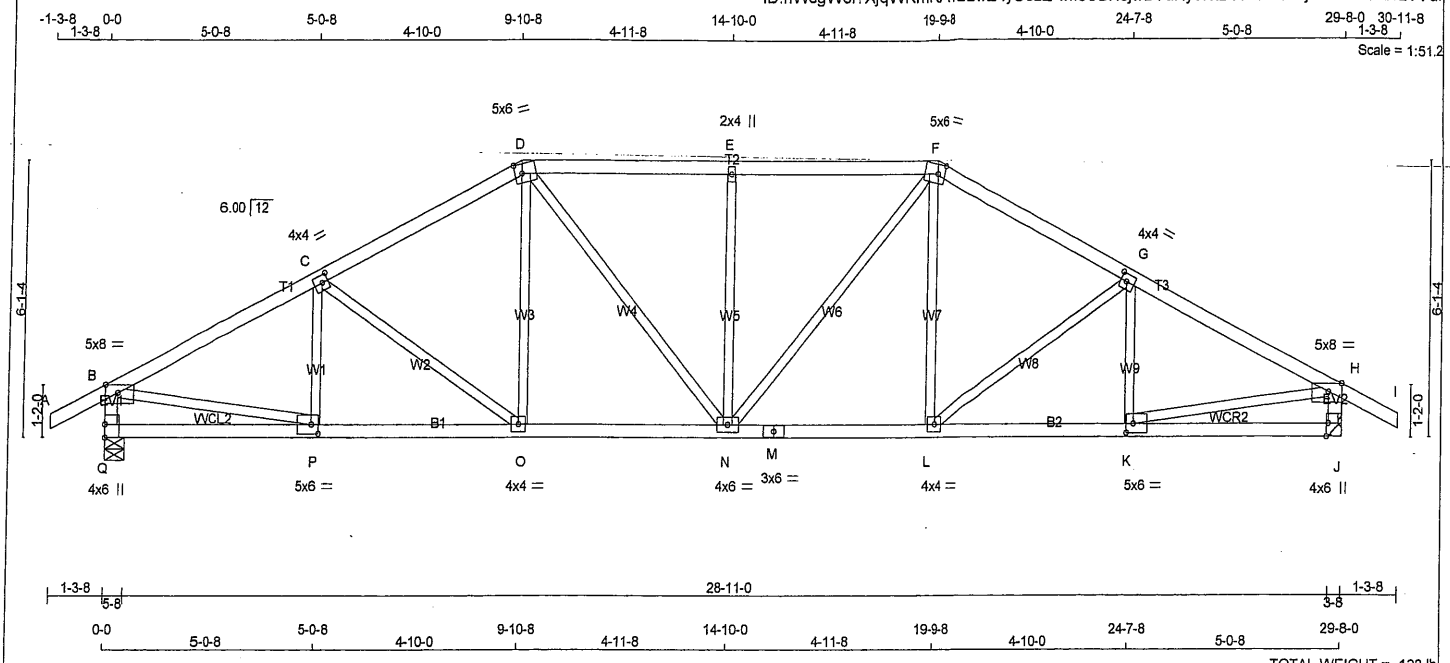

TOTAL WEIGHT = 123 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1+t	MT20	4.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.00
Q	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2236	0	2236	0	0	5-8	5-8
J	2236	0	2236	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0
J	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-2973 / 0	-112.4	-112.4	0.47 (1)	3.64
C- D	-2679 / 0	-112.4	-112.4	0.43 (1)	3.83
D- E	-2721 / 0	-112.4	-112.4	0.44 (1)	3.78
E- F	-2721 / 0	-112.4	-112.4	0.44 (1)	3.78
F- G	-2679 / 0	-112.4	-112.4	0.43 (1)	3.83
G- H	-2973 / 0	-112.4	-112.4	0.47 (1)	3.64
H- I	0 / 34	-112.4	-112.4	0.15 (1)	10.00
Q- B	-2173 / 0	0.0	0.0	0.22 (1)	5.76
J- H	-2173 / 0	0.0	0.0	0.22 (1)	5.76
Q- P	0 / 0	-28.0	-28.0	0.16 (3)	10.00
P- O	0 / 2683	-28.0	-28.0	0.51 (1)	10.00
O- N	0 / 2375	-28.0	-28.0	0.46 (1)	10.00
N- M	0 / 2375	-28.0	-28.0	0.46 (1)	10.00
M- L	0 / 2375	-28.0	-28.0	0.46 (1)	10.00
L- K	0 / 2683	-28.0	-28.0	0.51 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-313 / 64	0.07 (1)
C- O	-381 / 0	0.24 (1)
O- D	0 / 377	0.08 (2)
D- N	0 / 532	0.12 (1)
N- E	-679 / 0	0.40 (1)
N- F	0 / 532	0.12 (1)
L- F	0 / 377	0.08 (2)
L- G	-381 / 0	0.24 (1)
K- G	-313 / 64	0.07 (1)
B- P	0 / 2722	0.61 (1)
K- H	0 / 2722	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.47/1.00 (G-H:1) , BC=0.51/1.00 (K-L:1) , WB=0.61/1.00 (H-K:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.67 (K) (INPUT = 1.00)

DWG NO. TAM 17824-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T68	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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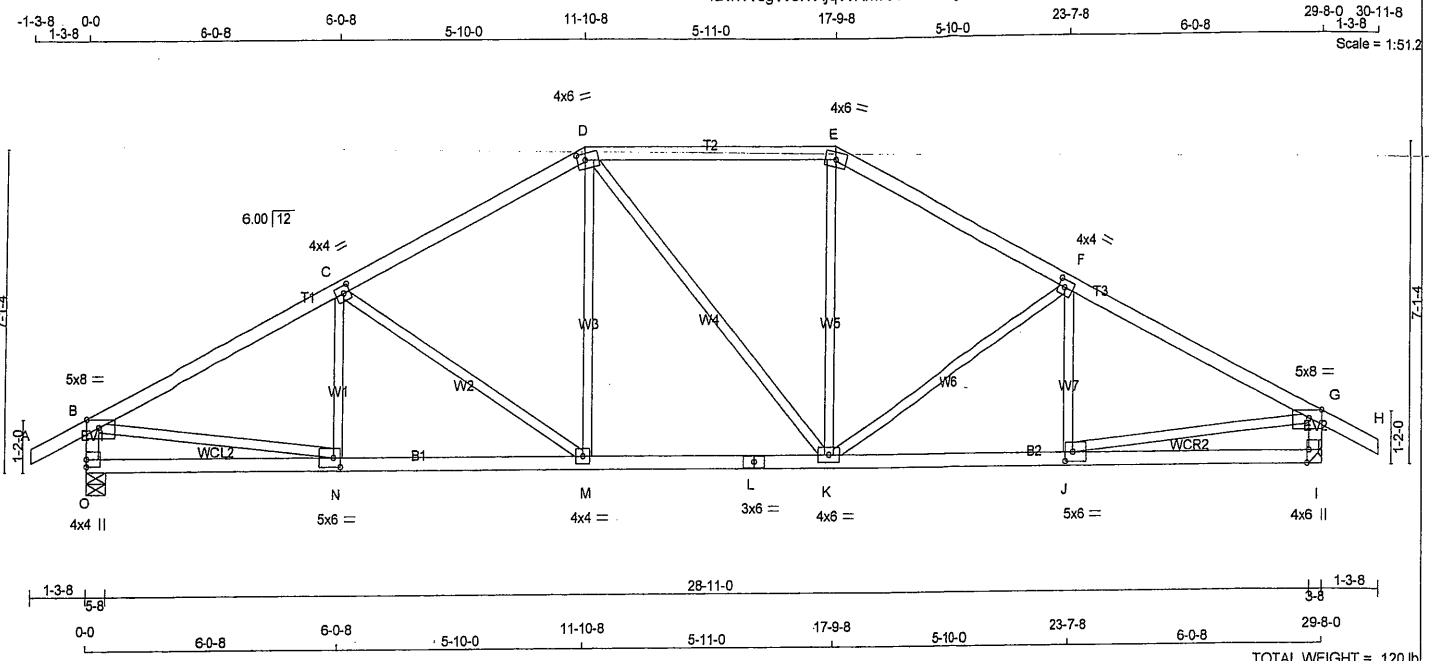


TOTAL WEIGHT = 120 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
I	BMV1+t	MT20	4.0	6.0	Edge	0.50
J	BMWW-t	MT20	5.0	6.0	2.50	2.00
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2236	0	2236	0	0	IN-SX	IN-SX
I	2236	0	2236	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0
I	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

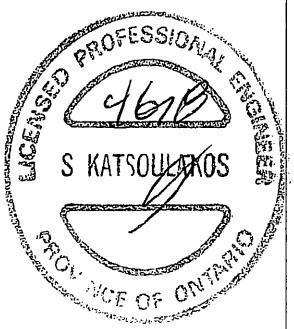
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
B- C	-3011 / 0	-112.4	-112.4	0.69 (1)	3.34	
C- D	-2479 / 0	-112.4	-112.4	0.61 (1)	3.72	
D- E	-2193 / 0	-112.4	-112.4	0.58 (1)	3.93	
E- F	-2480 / 0	-112.4	-112.4	0.61 (1)	3.72	
F- G	-3010 / 0	-112.4	-112.4	0.69 (1)	3.34	
G- H	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
O- B	-2164 / 0	0.0	0.0	0.22 (1)	5.77	
I- G	-2163 / 0	0.0	0.0	0.22 (1)	5.77	
O- N	0 / 0	-28.0	-28.0	0.25 (3)	10.00	
N- M	0 / 2724	-28.0	-28.0	0.54 (1)	10.00	
M- L	0 / 2192	-28.0	-28.0	0.46 (1)	10.00	
L- K	0 / 2192	-28.0	-28.0	0.46 (1)	10.00	
K- J	0 / 2723	-28.0	-28.0	0.54 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.25 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-206 / 154	0.05 (1)
C- M	-644 / 0	0.62 (1)
M- D	0 / 514	0.12 (1)
D- K	0 / 1	0.00 (1)
K- E	0 / 515	0.12 (1)
K- F	-643 / 0	0.62 (1)
J- F	-207 / 154	0.05 (1)
B- N	0 / 2751	0.62 (1)
J- G	0 / 2751	0.62 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.54/1.00 (J-K:1) , WB=0.62/1.00 (C-M:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.68 (N) (INPUT = 1.00)

DWG NO. TAM 17825-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T69	1	1	OBC2012_6TCDL TRUSS DESC.	

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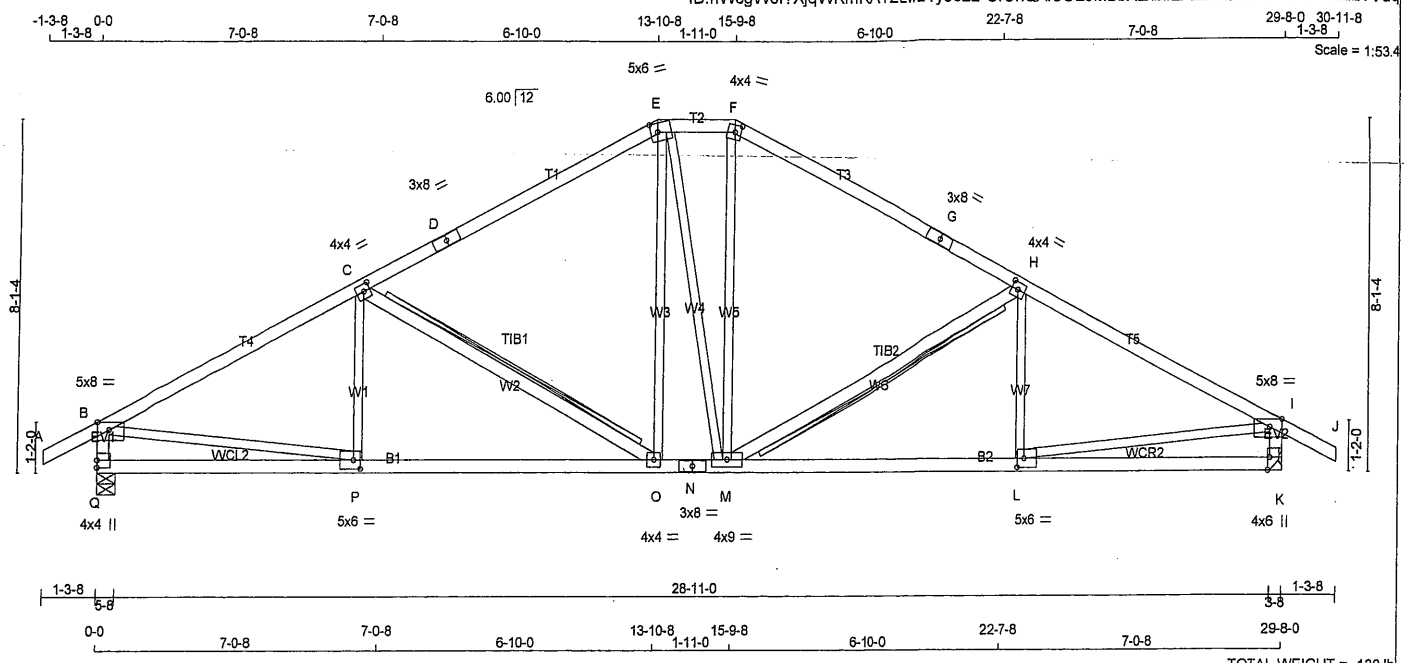

TOTAL WEIGHT = 130 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	3.50
K	BMV1+t	MT20	4.0	6.0	Edge	0.50
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.00
Q	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE *TOUCHES EDGE OF CHORD.*

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2236	0	2236	0	0	5-8	5-8
K	2236	0	2236	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0
K	1765	1052 / 0	312 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-O, H-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-3042 / 0	-112.4	-112.4	0.92 (1)	2.86
C- D	-2250 / 0	-112.4	-112.4	0.91 (1)	3.27
D- E	-2250 / 0	-112.4	-112.4	0.91 (1)	3.27
E- F	-1985 / 0	-112.4	-112.4	0.09 (1)	4.74
F- G	-2252 / 0	-112.4	-112.4	0.91 (1)	3.27
G- H	-2252 / 0	-112.4	-112.4	0.91 (1)	3.27
H- I	-3041 / 0	-112.4	-112.4	0.92 (1)	2.86
I- J	0 / 34	-112.4	-112.4	0.15 (1)	10.00
Q- B	-2162 / 0	0.0	0.0	0.22 (1)	5.78
K- I	-2162 / 0	0.0	0.0	0.22 (1)	5.78
Q- P	0 / 0	-28.0	-28.0	0.36 (3)	10.00
P- O	0 / 2761	-28.0	-28.0	0.63 (2)	10.00
O- N	0 / 1983	-28.0	-28.0	0.46 (1)	10.00
N- M	0 / 1983	-28.0	-28.0	0.46 (1)	10.00
M- L	0 / 2760	-28.0	-28.0	0.63 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.35 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-143 / 229	0.05 (3)
C- O	-899 / 0	0.89 (1)
O- E	0 / 548	0.12 (1)
E- M	0 / 12	0.00 (1)
M- F	0 / 561	0.13 (1)
M- H	-895 / 0	0.88 (1)
L- H	-147 / 226	0.05 (3)
B- P	0 / 2785	0.63 (1)
L- I	0 / 2784	0.63 (1)

LICENSED PROFESSIONAL ENGINEER 4618 S KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.92/1.00 (B-C:1) , BC=0.63/1.00 (O-P:2) , WB=0.89/1.00 (C-O:1) , SSI=0.35/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.69 (P) (INPUT = 1.00)

DWG NO. TAM 17826-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T70	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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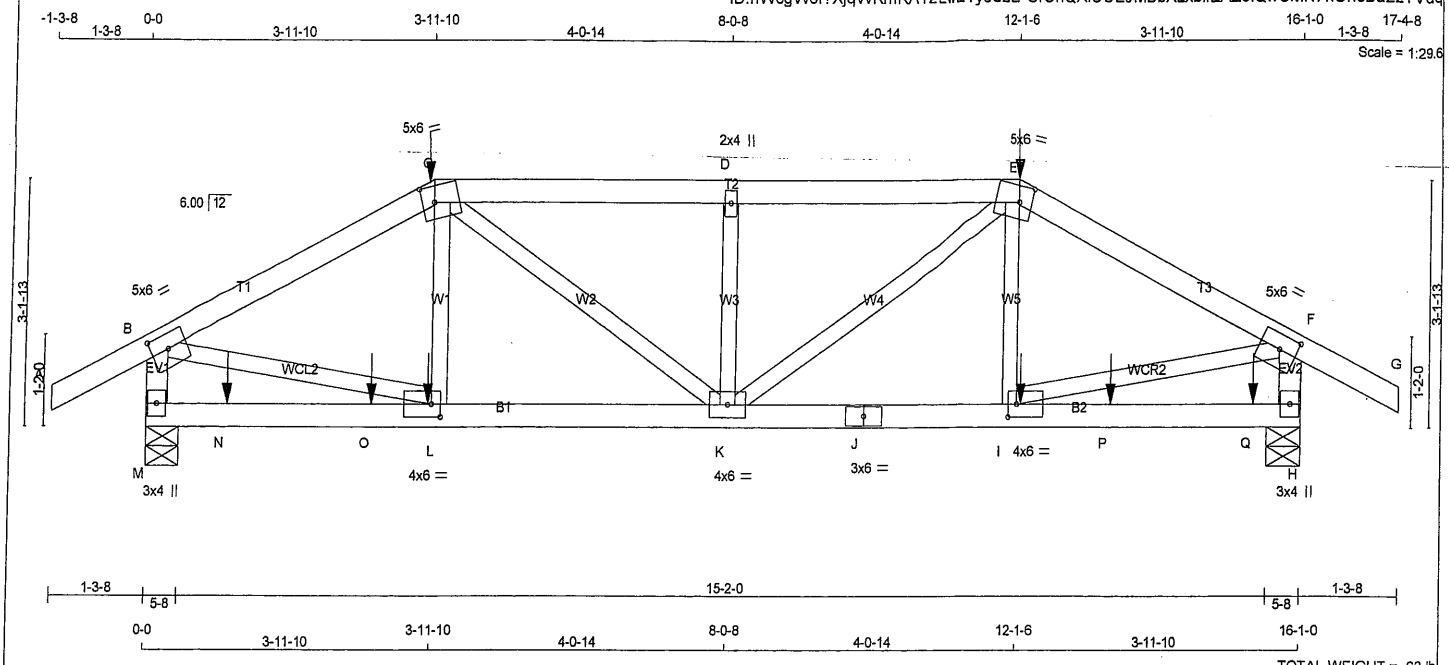

TOTAL WEIGHT = 63 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TTWW-m	MT20	5.0	6.0	2.50	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.00
F	TMVW-t	MT20	5.0	6.0	2.25	2.75
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 108.0 lbs FACTORED DOWN AT 3-11-10, AND 108.0 lbs FACTORED DOWN AT 12-1-6 ON TOP CHORD, AND 8.1 lbs FACTORED DOWN AT 1-1-11, 8.1 lbs FACTORED DOWN AT 3-1-11, 427.0 lbs FACTORED DOWN AT 3-11-0, 427.0 lbs FACTORED DOWN AT 12-2-0, AND 8.1 lbs FACTORED DOWN AT 13-5-5, AND 16.9 lbs FACTORED DOWN AT 15-5-5 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1849	0	1849	0	0	5-8	5-8
H	1854	0	1854	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1436	894 / 0	226 / 0	0 / 0	0 / 0	317 / 0	0 / 0
H	1443	894 / 0	230 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 34	-112.4	-112.4	0.16 (1)	10.00
B- C		-2230 / 0	-112.4	-112.4	0.44 (1)	4.06
C- D		-2481 / 0	-112.4	-112.4	0.42 (1)	3.90
D- E		-2481 / 0	-112.4	-112.4	0.42 (1)	3.90
E- F		-2230 / 0	-112.4	-112.4	0.44 (1)	4.06
F- G		0 / 34	-112.4	-112.4	0.16 (1)	10.00
M- B		-1796 / 0	0.0	0.0	0.20 (1)	6.19
H- F		-1796 / 0	0.0	0.0	0.20 (1)	6.19
M- N		0 / 0	-28.0	-28.0	0.13 (3)	10.00
N- O		0 / 0	-28.0	-28.0	0.13 (3)	10.00
O- L		0 / 0	-28.0	-28.0	0.13 (3)	10.00
L- K		0 / 1998	-33.9	-33.9	0.41 (1)	10.00
K- J		0 / 1998	-33.9	-33.9	0.41 (1)	10.00
J- I		0 / 1998	-33.9	-33.9	0.41 (1)	10.00
I- P		0 / 0	-28.0	-28.0	0.13 (3)	10.00
P- Q		0 / 0	-28.0	-28.0	0.13 (3)	10.00
Q- H		0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C		0 / 228	0.06 (3)
C- K		0 / 589	0.15 (1)
K- D		-547 / 0	0.11 (1)
K- E		0 / 589	0.15 (1)
I- E		0 / 228	0.06 (3)
B- L		0 / 2037	0.50 (1)
I- F		0 / 2037	0.50 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-11-10	-15	-17	—	FRONT	VERT	DEAD
C	3-11-10	-93	-93	—	FRONT	VERT	SNOW
E	12-1-6	-15	-17	—	FRONT	VERT	DEAD
E	12-1-6	-93	-93	—	FRONT	VERT	SNOW
I	12-2-0	-427	-427	—	BACK	VERT	TOTAL
L	3-11-0	-427	-427	—	BACK	VERT	TOTAL
N	1-1-11	-5	-8	—	BACK	VERT	TOTAL
O	3-1-11	-5	-8	—	BACK	VERT	TOTAL
P	13-5-5	-5	-8	—	BACK	VERT	TOTAL
Q	15-5-5	-10	-17	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 3-11-0
START SPAN CARRIED = 2-7-8
END DISTANCE = 12-2-0
END SPAN CARRIED = 2-7-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.41/1.00 (K-L:1) , WB=0.50/1.00 (B-L:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

DWG NO. TAM 17827-T70

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	T70	1	1	TRUSS DESC.	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17827-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T71	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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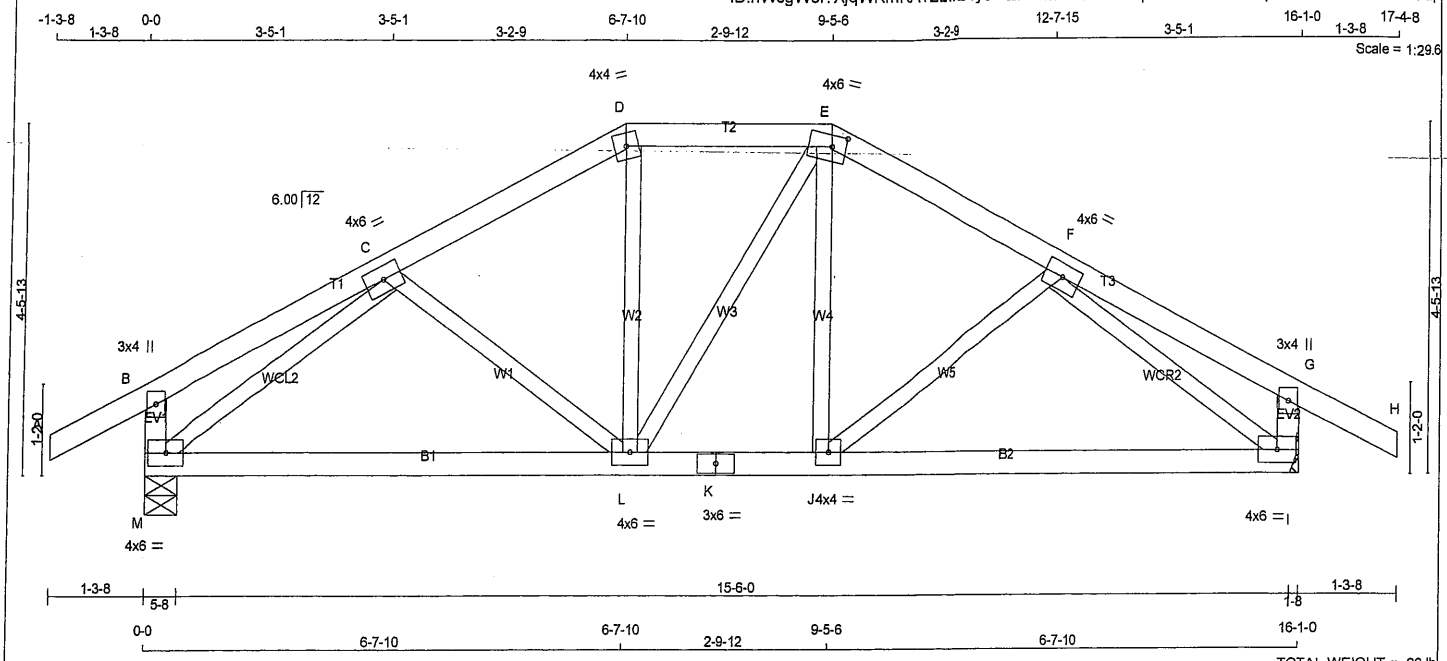

TOTAL WEIGHT = 66 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1282	0	1282	0	0	5-8	5-8
I	1282	0	1282	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1005	610 / 0	169 / 0	0 / 0	0 / 0	225 / 0	0 / 0
I	1005	610 / 0	169 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	10.00
B- C	0 / 17	-112.4	-112.4	0.18 (1)	10.00	10.00
C- D	-1215 / 0	-112.4	-112.4	0.15 (1)	5.66	5.66
D- E	-1076 / 0	-112.4	-112.4	0.13 (1)	5.95	5.95
E- F	-1215 / 0	-112.4	-112.4	0.15 (1)	5.66	5.66
F- G	0 / 17	-112.4	-112.4	0.18 (1)	10.00	10.00
G- H	0 / 34	-112.4	-112.4	0.15 (1)	10.00	10.00
M- B	-301 / 0	0.0	0.0	0.03 (1)	7.81	
I- G	-301 / 0	0.0	0.0	0.03 (1)	7.81	
M- L	0 / 1189	-28.0	-28.0	0.39 (2)	10.00	
L- K	0 / 1075	-28.0	-28.0	0.37 (2)	10.00	
K- J	0 / 1075	-28.0	-28.0	0.37 (2)	10.00	
J- I	0 / 1189	-28.0	-28.0	0.39 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-148 / 63	0.05 (1)
L- D	0 / 252	0.06 (2)
L- E	0 / 1	0.00 (2)
J- E	0 / 251	0.06 (2)
J- F	-148 / 63	0.05 (1)
M- C	-1492 / 0	0.43 (1)
F- I	-1491 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.18/1.00 (F-G:1) , BC=0.39/1.00 (I-J:2) , WB=0.43/1.00 (C-M:1) , SSI=0.16/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.51 (K) (INPUT = 1.00)

DWG NO. TAM 17B26-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287657	T72	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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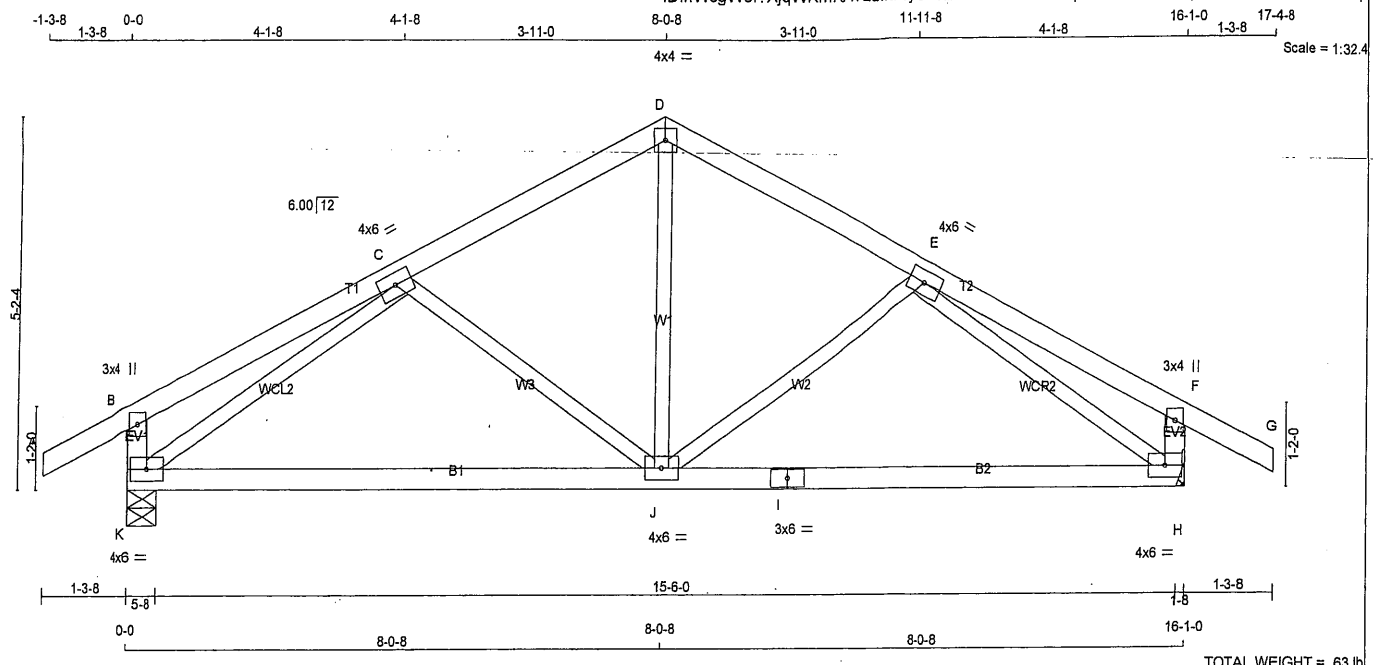

TOTAL WEIGHT = 63 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-p	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	6.0	
J	BMWWW-t	MT20	4.0	6.0	
K	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1282	0	1282	0	0	5-8	5-8
H	1282	0	1282	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1005	610 / 0	169 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	1005	610 / 0	169 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.28 (1)	10.00
C- D	-1112 / 0	-112.4	-112.4	0.23 (1)	5.75
D- E	-1112 / 0	-112.4	-112.4	0.23 (1)	5.75
E- F	0 / 22	-112.4	-112.4	0.28 (1)	10.00
F- G	0 / 34	-112.4	-112.4	0.15 (1)	10.00
K- B	-329 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-329 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1216	-28.0	-28.0	0.65 (2)	10.00
J- I	0 / 1216	-28.0	-28.0	0.65 (2)	10.00
I- H	0 / 1216	-28.0	-28.0	0.65 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 625	0.14 (1)
J- E	-298 / 31	0.12 (1)
C- J	-298 / 31	0.12 (1)
K- C	-1493 / 0	0.59 (1)
E- H	-1493 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 903 (0.21")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.65/1.00 (J-K:2) , WB=0.59/1.00 (C-K:1) , SSI=0.20/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)

DWG NO. TAM 17824-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T100C	5	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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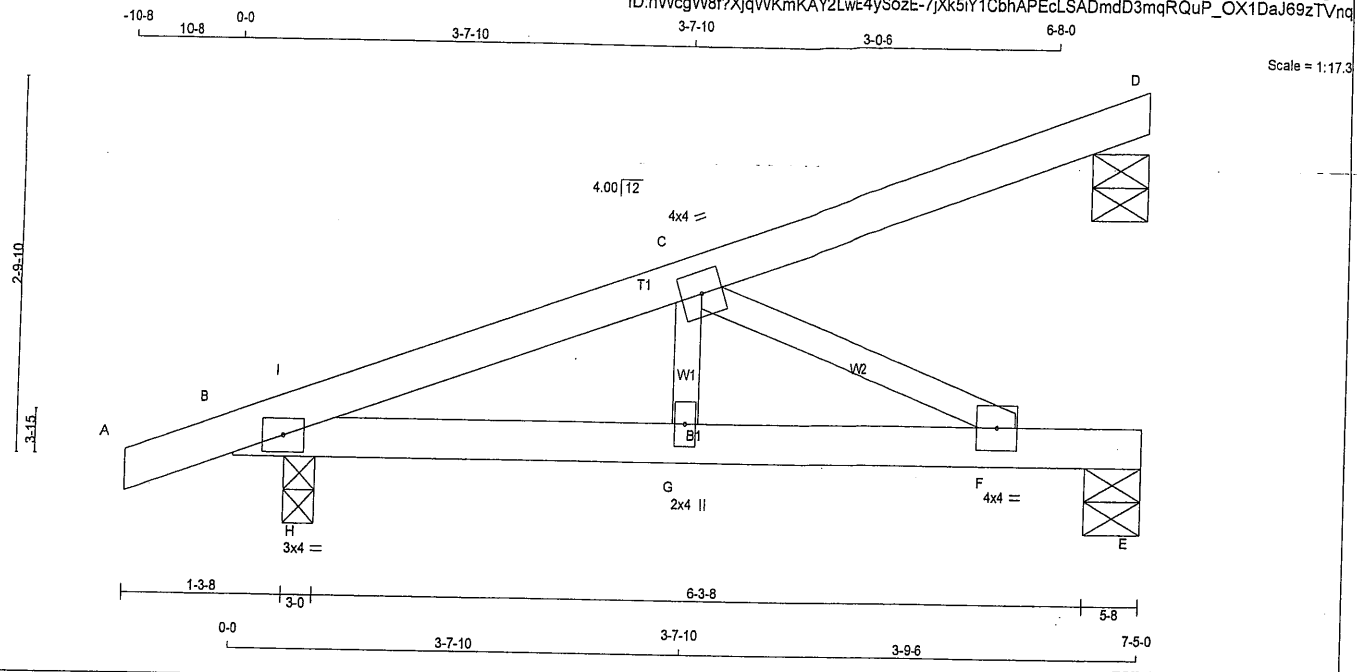

TOTAL WEIGHT = 5 X 22 = 108 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
F	BMW-t	MT20	4.0	4.0	
G	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	191	0	191	0	0	5-8	5-8
E	330	0	330	0	0	5-8	5-8
B	624	0	624	0	0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	134	107 / 0	4 / 0	0 / 0	0 / 0	22 / 0	0 / 0
E	281	133 / 0	74 / 0	0 / 0	0 / 0	74 / 0	0 / 0
B	486	301 / 0	78 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, E, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-112.4	-112.4	0.07 (1)	10.00
B- I	-985 / 0	-112.4	-112.4	0.02 (1)	6.25
I- C	-967 / 0	-112.4	-112.4	0.12 (1)	6.20
C- D	-7 / 2	-112.4	-112.4	0.20 (1)	10.00
B- H	0 / 924	-28.0	-28.0	0.26 (1)	10.00
H- G	0 / 924	-28.0	-28.0	0.28 (1)	10.00
G- F	0 / 924	-28.0	-28.0	0.66 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.51 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 338	0.08 (2)
C- F	-1025 / 0	0.20 (1)
H- I	-148 / 9	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 717 (0.12")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.66/1.00 (F-G:1) , WB=0.20/1.00 (C-F:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (C) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM17769-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	T101C	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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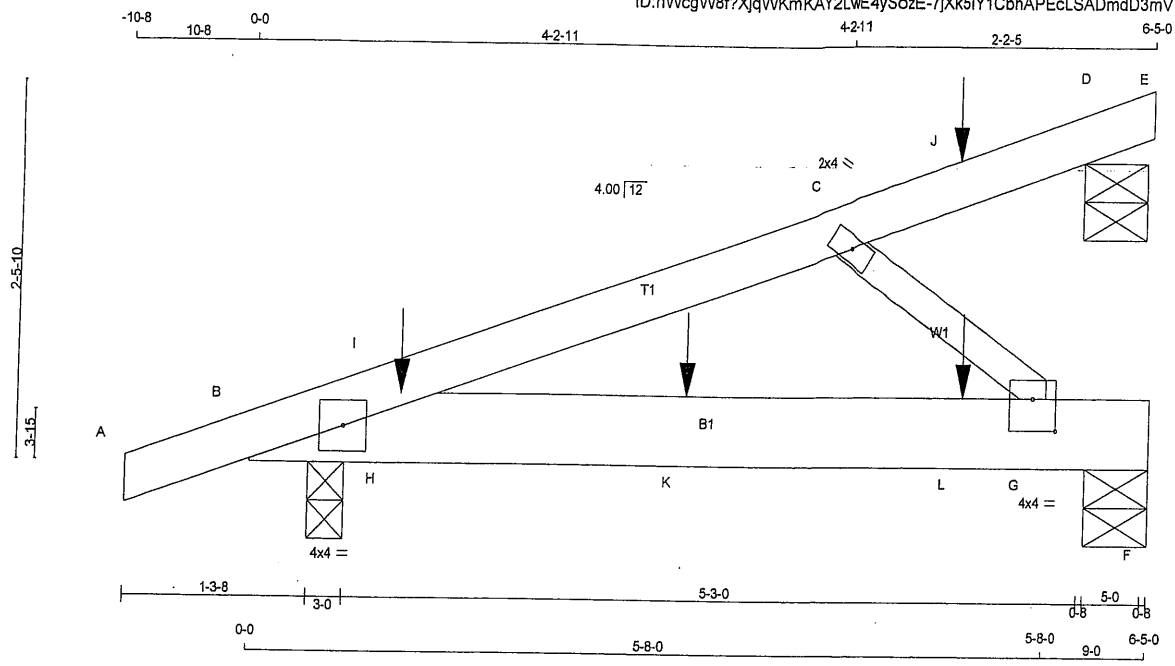

TOTAL WEIGHT = 2 X 22 = 44 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0		
C	TMW+w	MT20	2.0	4.0		
G	BMW-t	MT20	4.0	4.0	2.50	2.00

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4.6 lbs FACTORED DOWN AT 5-0-12 ON TOP CHORD, AND 293.5 lbs FACTORED DOWN AT 1-0-12, AND 293.5 lbs FACTORED DOWN AT 3-0-12, AND 293.5 lbs FACTORED DOWN AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	782	0	782	0	0	5-8	5-8
B	1016	0	1016	0	0	3-0	3-0
D	97	0	97	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	592	369 / 0	68 / 0	0 / 0	0 / 0	155 / 0	0 / 0
B	752	503 / 0	67 / 0	0 / 0	0 / 0	182 / 0	0 / 0
D	66	56 / 0	0 / -1	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-112.4	-112.4	0.04 (1)	10.00
B- I	-469 / 0	-112.4	-112.4	0.21 (1)	6.25
I- C	-340 / 0	-112.4	-112.4	0.23 (1)	6.25
C- J	-16 / 0	-112.4	-112.4	0.08 (1)	6.25
J- D	-16 / 0	-112.4	-112.4	0.08 (1)	6.25
D- E	-6 / 0	-112.4	-112.4	0.00 (1)	10.00
B- H	0 / 334	-28.0	-28.0	0.19 (1)	10.00
H- K	0 / 358	-28.0	-28.0	0.40 (1)	10.00
K- L	0 / 358	-28.0	-28.0	0.40 (1)	10.00
L- G	0 / 358	-28.0	-28.0	0.40 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.24 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-497 / 0	0.04 (1)
H- I	0 / 174	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	1-0-12	-293	-293	—	FRONT	VERT	TOTAL
J	5-0-12	-4	-4	—	TOP	VERT	TOTAL
K	3-0-12	-293	-293	—	FRONT	VERT	TOTAL
L	5-0-12	-293	-293	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4678
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 687 (0.11")

CSI: TC=0.23/1.00 (C-I:1) , BC=0.40/1.00 (G-H:1) , WB=0.04/1.00 (C-G:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (G) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM 17770-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	T102	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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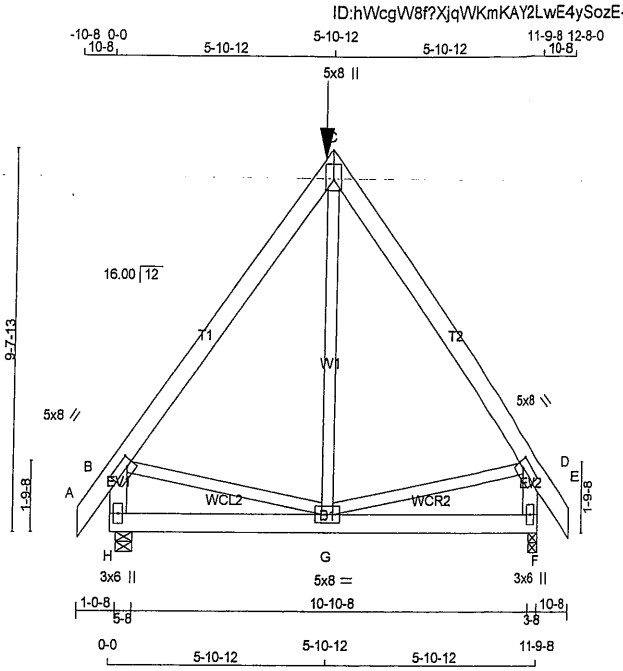


TOTAL WEIGHT = 2 X 90 = 180 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(61.0)
C- E	2	12	SIDE(61.0)
H- B	2	12	TOP
F- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	2	12	SIDE(21.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- C	1	6	SIDE(131.4)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.25
C	TTW+p	MT20	5.0	8.0	3.25	2.50
D	TMVW-t	MT20	5.0	8.0	2.50	2.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 971.3 lbs FACTORED DOWN AT 5-10-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1574	0	1574	0	0	5-8	5-8
F	1574	0	1574	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1251	732 / 0	230 / 0	0 / 0	0 / 0	289 / 0	0 / 0
F	1251	732 / 0	230 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

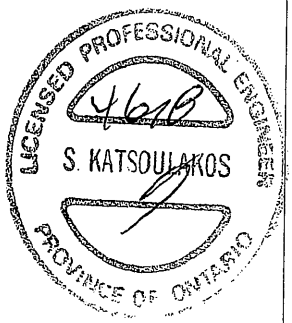
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 48	-112.4	-112.4	0.03 (1)	10.00	G- C	0 / 549	0.05 (3)
B- C	-1091 / 0	-112.4	-112.4	0.19 (1)	6.25	B- G	0 / 668	0.06 (1)
C- D	-1091 / 0	-112.4	-112.4	0.19 (1)	6.25	G- D	0 / 668	0.06 (1)
D- E	0 / 48	-112.4	-112.4	0.03 (1)	10.00			
H- B	-1454 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-1454 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-52.1	-52.1	0.14 (3)	10.00			
G- F	0 / 0	-52.1	-52.1	0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-12	-971	-971	---	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-12
END SETBACK = 5-10-12
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.14/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.12/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (G) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

DWG NO. TAM17796-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287687	PB1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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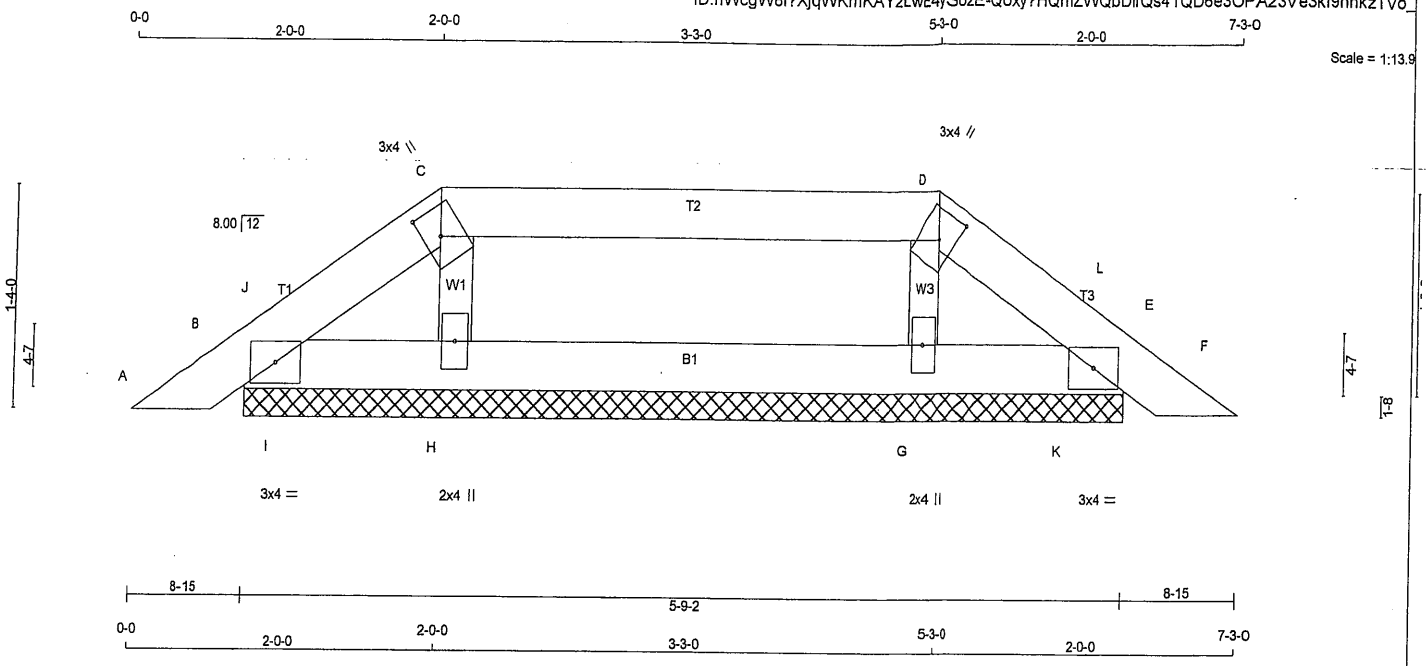

TOTAL WEIGHT = 17 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	180	0	180	0	0	5-9-2	5-9-2
E	180	0	180	0	0	5-9-2	5-9-2
H	299	0	299	0	0	5-9-2	5-9-2
G	299	0	299	0	0	5-9-2	5-9-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	126	101 / 0	4 / 0	0 / 0	0 / 0	21 / 0	0 / 0
E	126	101 / 0	4 / 0	0 / 0	0 / 0	21 / 0	0 / 0
H	247	129 / 0	57 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	247	129 / 0	57 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- J	-50 / 0	-112.4	-112.4	0.00 (3)	6.25
J- C	-51 / 0	-112.4	-112.4	0.02 (1)	6.25
C- D	-17 / 0	-112.4	-112.4	0.20 (1)	6.25
D- L	-51 / 0	-112.4	-112.4	0.02 (1)	6.25
L- E	-50 / 0	-112.4	-112.4	0.00 (3)	6.25
E- F	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- I	0 / 41	-28.0	-28.0	0.03 (1)	10.00
I- H	0 / 41	-28.0	-28.0	0.05 (3)	10.00
H- G	0 / 17	-28.0	-28.0	0.05 (3)	10.00
G- K	0 / 41	-28.0	-28.0	0.05 (3)	10.00
K- E	0 / 41	-28.0	-28.0	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-208 / 0	0.03 (1)
G- D	-208 / 0	0.03 (1)
I- J	-72 / 0	0.00 (1)
K- L	-72 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1) , BC=0.05/1.00 (H-I:3) , WB=0.03/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.04 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4616
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17772-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	PB2	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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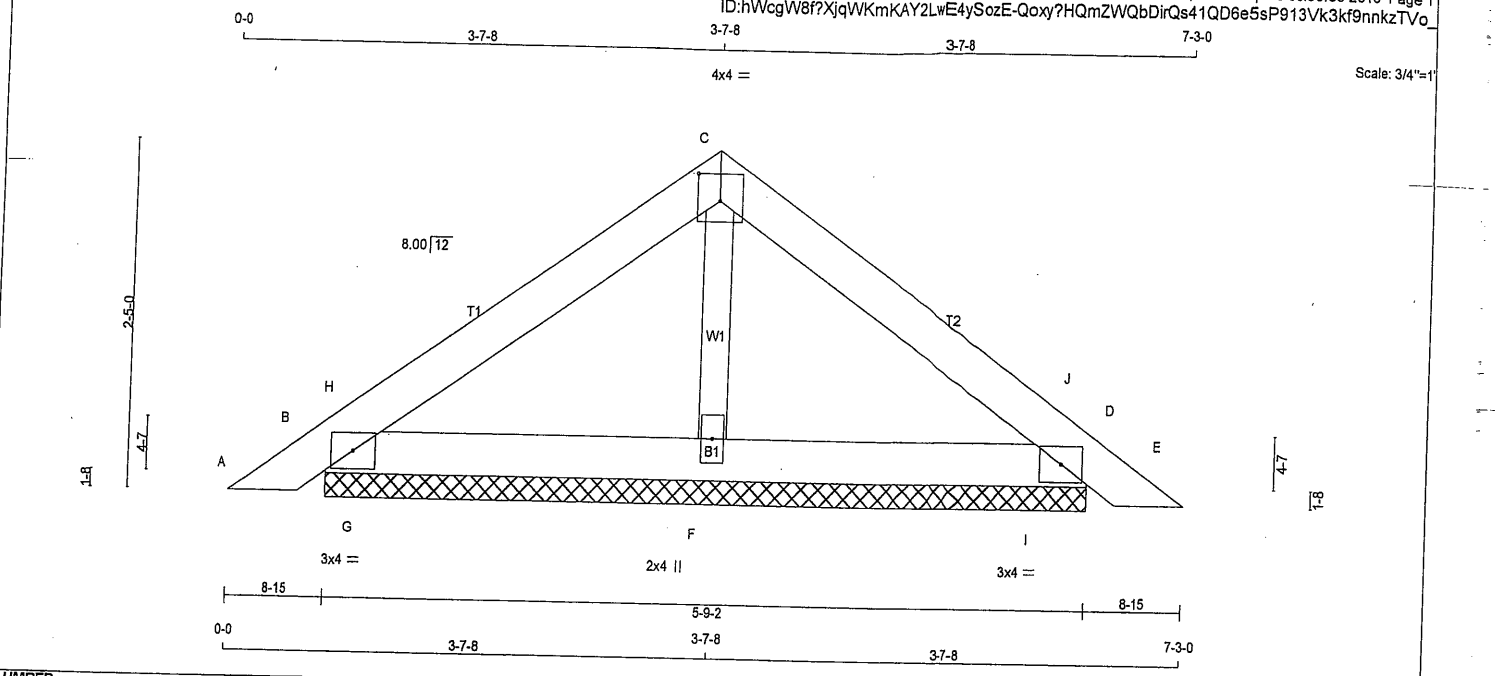


TOTAL WEIGHT = 2 X 18 = 36 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	312	0	312	0	0	5-9-2	5-9-2
D	312	0	312	0	0	5-9-2	5-9-2
F	334	0	334	0	0	5-9-2	5-9-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	234	159 / 0	27 / 0	0 / 0	0 / 0	48 / 0	0 / 0
D	234	159 / 0	27 / 0	0 / 0	0 / 0	48 / 0	0 / 0
F	279	142 / 0	67 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- H	-55 / 0	-112.4	-112.4	0.04 (1)	6.25
H- C	-110 / 0	-112.4	-112.4	0.11 (1)	6.25
C- J	-110 / 0	-112.4	-112.4	0.11 (1)	6.25
J- D	-55 / 0	-112.4	-112.4	0.04 (1)	6.25
D- E	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- G	0 / 86	-28.0	-28.0	0.11 (1)	10.00
G- F	0 / 86	-28.0	-28.0	0.11 (1)	10.00
F- I	0 / 86	-28.0	-28.0	0.11 (1)	10.00
I- D	0 / 86	-28.0	-28.0	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-152 / 0	0.02 (1)
G- H	-255 / 15	0.00 (1)
I- J	-255 / 15	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-J:1) , BC=0.11/1.00 (D-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.19/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

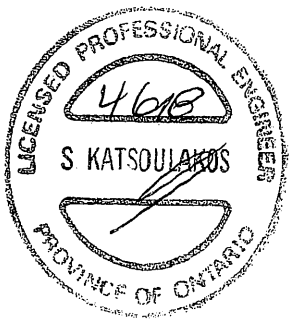
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAM 17773-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	PB3	10	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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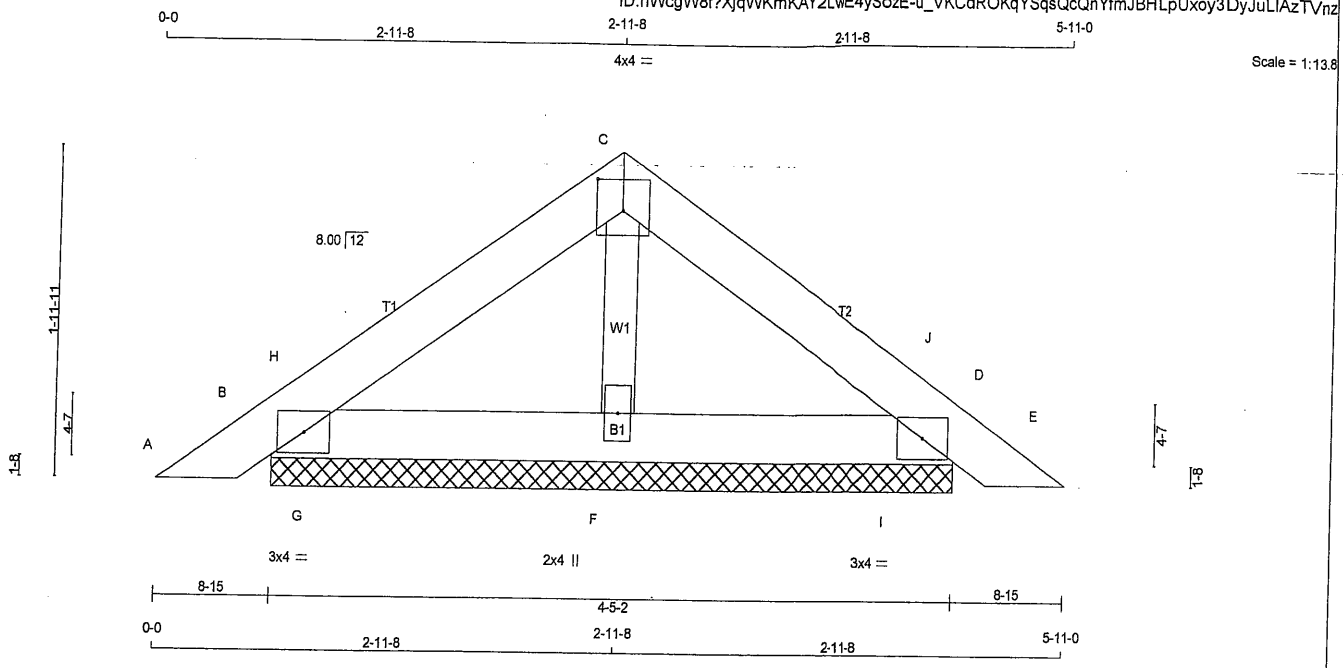


TOTAL WEIGHT = 10 X 14 = 141 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	251	0	251	0	0	4-5-2	4-5-2
D	251	0	251	0	0	4-5-2	4-5-2
F	268	0	268	0	0	4-5-2	4-5-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	187	129 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
D	187	129 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
F	223	115 / 0	52 / 0	0 / 0	0 / 0	56 / 0	0 / 0

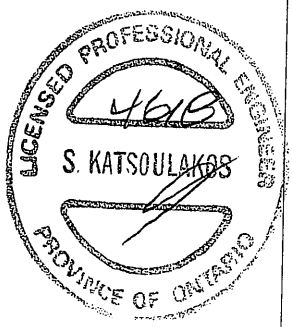
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00	F- C	-124 / 0	0.02 (1)
B- H	-50 / 0	-112.4	-112.4	0.02 (1)	6.25	G- H	-162 / 9	0.00 (1)
H- C	-73 / 0	-112.4	-112.4	0.06 (1)	6.25	I- J	-162 / 9	0.00 (1)
C- J	-73 / 0	-112.4	-112.4	0.06 (1)	6.25			
J- D	-50 / 0	-112.4	-112.4	0.02 (1)	6.25			
D- E	0 / 19	-112.4	-112.4	0.04 (1)	10.00			
B- G	0 / 58	-28.0	-28.0	0.07 (1)	10.00			
G- F	0 / 58	-28.0	-28.0	0.07 (1)	10.00			
F- I	0 / 58	-28.0	-28.0	0.07 (1)	10.00			
I- D	0 / 58	-28.0	-28.0	0.07 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-J:1) , BC=0.07/1.00 (D-I:1) , WB=0.02/1.00 (C-F:1) , SSi=0.12/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 17774-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	PB3Z	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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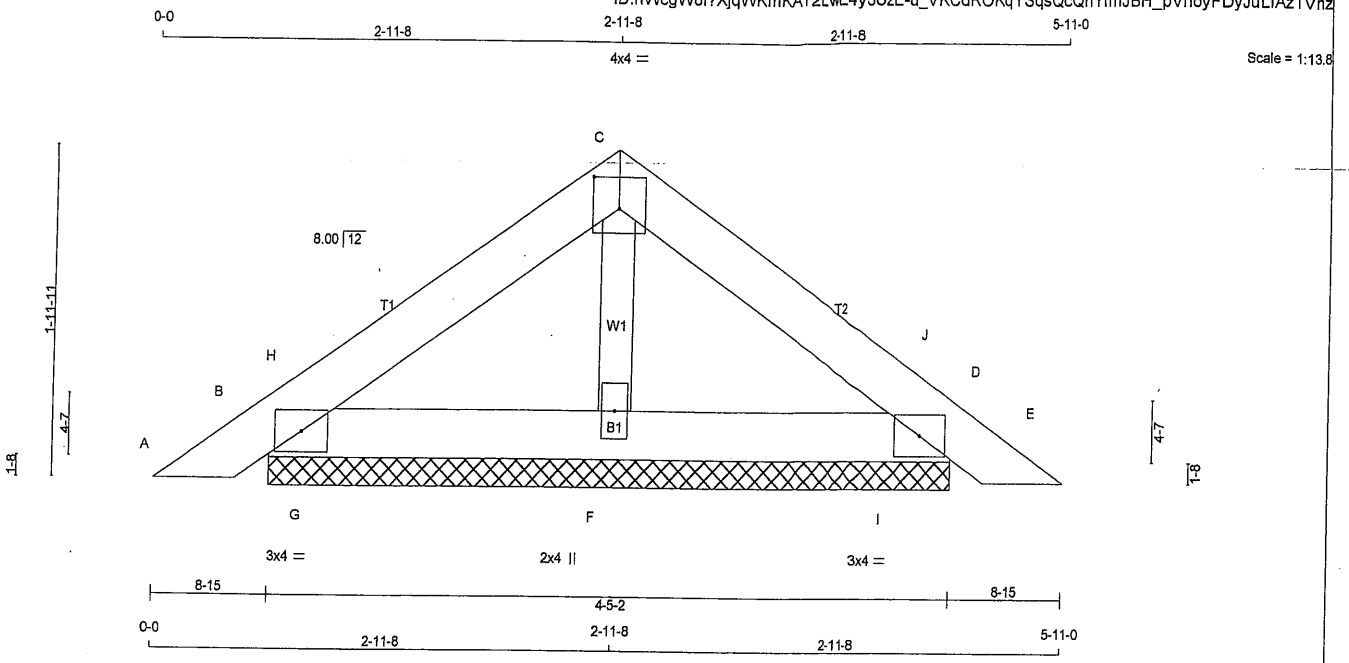

TOTAL WEIGHT = 3 X 14 = 42 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	253	0	253	0	0	4-5-2	4-5-2
D	253	0	253	0	0	4-5-2	4-5-2
F	266	0	266	0	0	4-5-2	4-5-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	188	130 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
D	188	130 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
F	221	113 / 0	52 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-112.4	-112.4	0.01 (1)		10.00	F- C	-124 / 0	0.01 (1)
B- H	-54 / 0	-112.4	-112.4	0.00 (1)		6.25	G- H	-156 / 9	0.00 (1)
H- C	-74 / 0	-112.4	-112.4	0.02 (1)		6.25	I- J	-156 / 9	0.00 (1)
C- J	-74 / 0	-112.4	-112.4	0.02 (1)		6.25			
J- D	-54 / 0	-112.4	-112.4	0.00 (1)		6.25			
D- E	0 / 19	-112.4	-112.4	0.01 (1)		10.00			
B- G	0 / 59	-28.0	-28.0	0.02 (1)		10.00			
G- F	0 / 59	-28.0	-28.0	0.02 (1)		10.00			
F- I	0 / 59	-28.0	-28.0	0.02 (1)		10.00			
I- D	0 / 59	-28.0	-28.0	0.02 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.02/1.00 (C-H:1) , BC=0.02/1.00 (B-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.04/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

DWG NO. TAM 177258
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	PB3Z2	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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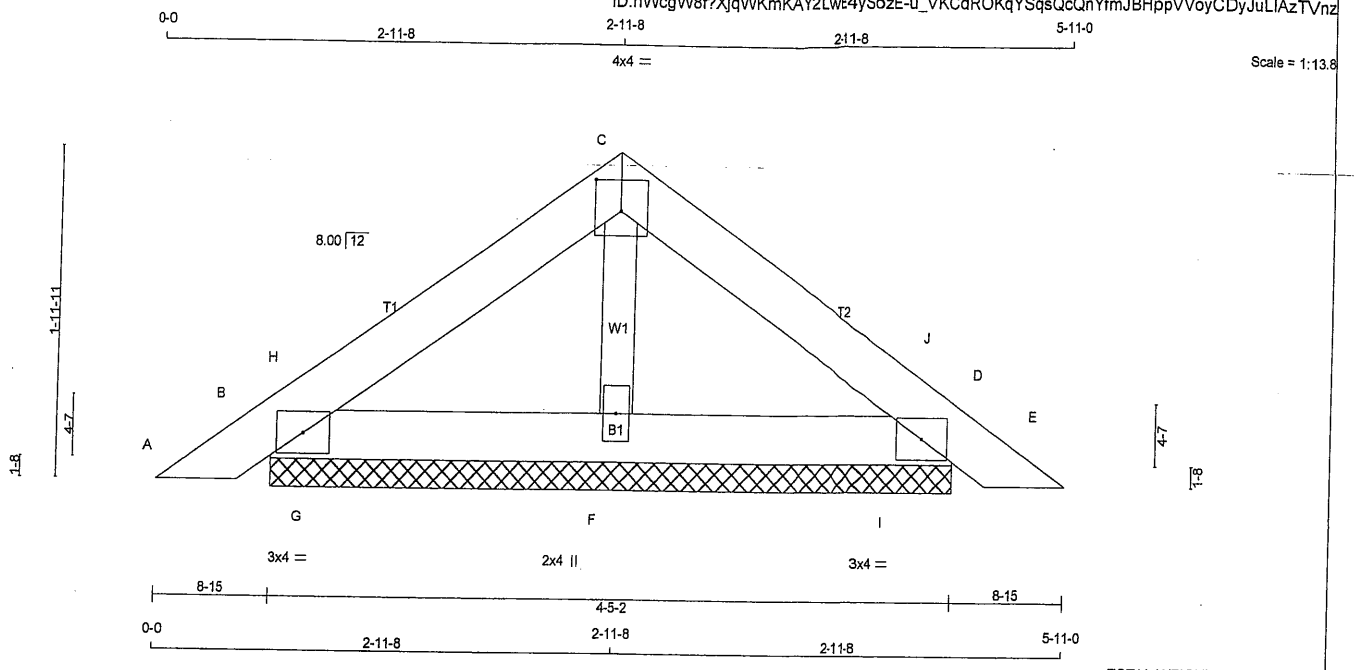


TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	252	0	252	0	0	4-5-2	4-5-2
D	252	0	252	0	0	4-5-2	4-5-2
F	267	0	267	0	0	4-5-2	4-5-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	188	130 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
D	188	130 / 0	20 / 0	0 / 0	0 / 0	38 / 0	0 / 0
F	222	114 / 0	52 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.02 (1)	10.00
B- H	-52 / 0	-112.4	-112.4	0.01 (1)	6.25
H- C	-74 / 0	-112.4	-112.4	0.03 (1)	6.25
C- J	-74 / 0	-112.4	-112.4	0.03 (1)	6.25
J- D	-52 / 0	-112.4	-112.4	0.01 (1)	6.25
D- E	0 / 19	-112.4	-112.4	0.02 (1)	10.00
B- G	0 / 59	-28.0	-28.0	0.03 (1)	10.00
G- F	0 / 59	-28.0	-28.0	0.03 (1)	10.00
F- I	0 / 59	-28.0	-28.0	0.03 (1)	10.00
I- D	0 / 59	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-124 / 0	0.01 (1)
G- H	-159 / 9	0.00 (1)
I- J	-159 / 9	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH.
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-H:1) , BC=0.03/1.00 (B-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.06/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

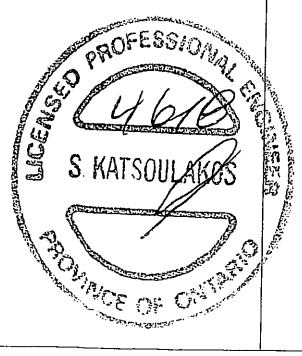
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI CRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.02 (D) (INPUT = 1.00)



DWG NO. TAM 17776-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	PB31	1	1	OBC2012_6TCDL TRUSS DESC.	

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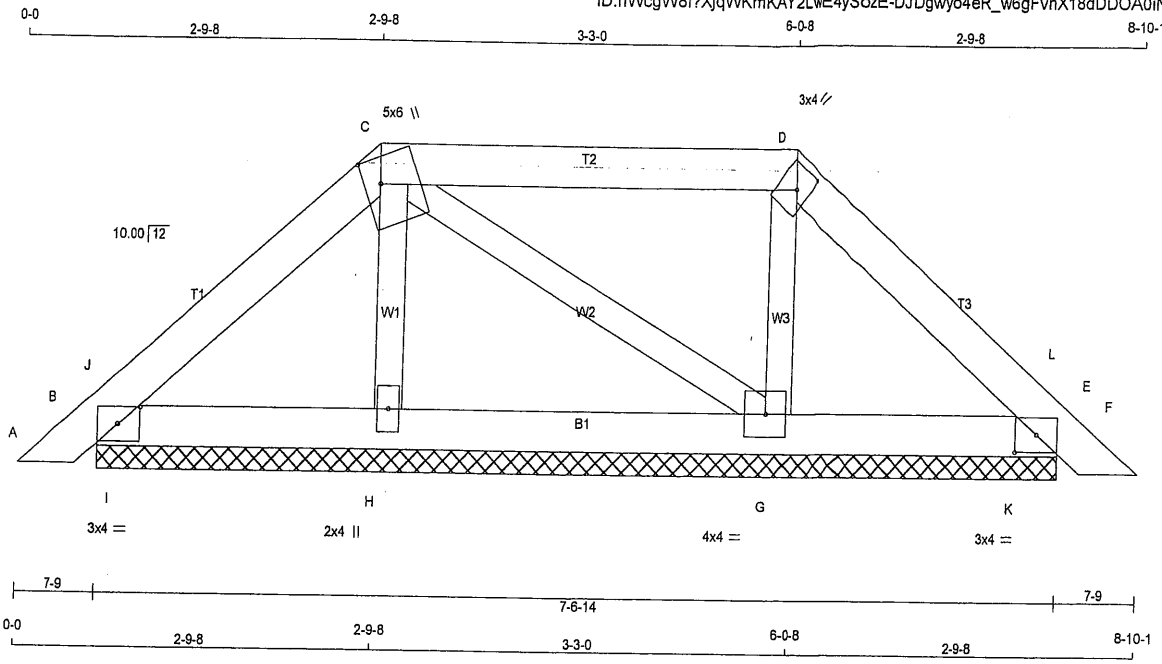

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	276	0	276	0	0	7-6-14	7-6-14
E	257	0	257	0	0	7-6-14	7-6-14
H	302	0	302	0	0	7-6-14	7-6-14
G	350	0	350	0	0	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	203	145 / 0	19 / 0	0 / 0	0 / 0	39 / 0	0 / 0
E	190	134 / 0	19 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	252	127 / 0	61 / 0	0 / 0	0 / 0	64 / 0	0 / 0
G	285	155 / 0	61 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- J	-83 / 0	-112.4	-112.4	0.02 (1)	6.25
J- C	-115 / 0	-112.4	-112.4	0.06 (1)	6.25
C- D	-49 / 0	-112.4	-112.4	0.20 (1)	6.25
D- L	-86 / 0	-112.4	-112.4	0.06 (1)	6.25
L- E	-52 / 0	-112.4	-112.4	0.02 (1)	6.25
E- F	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- I	0 / 84	-28.0	-28.0	0.07 (1)	10.00
I- H	0 / 84	-28.0	-28.0	0.07 (1)	10.00
H- G	0 / 73	-28.0	-28.0	0.06 (2)	10.00
G- K	0 / 62	-28.0	-28.0	0.06 (1)	10.00
K- E	0 / 62	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-198 / 0	0.03 (1)
C- G	-28 / 0	0.01 (1)
G- D	-231 / 0	0.03 (1)
I- J	-164 / 7	0.00 (1)
K- L	-168 / 6	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1) , BC=0.07/1.00 (B-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM 17299-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287688	PB32	2	1	TRUSS DESC.	

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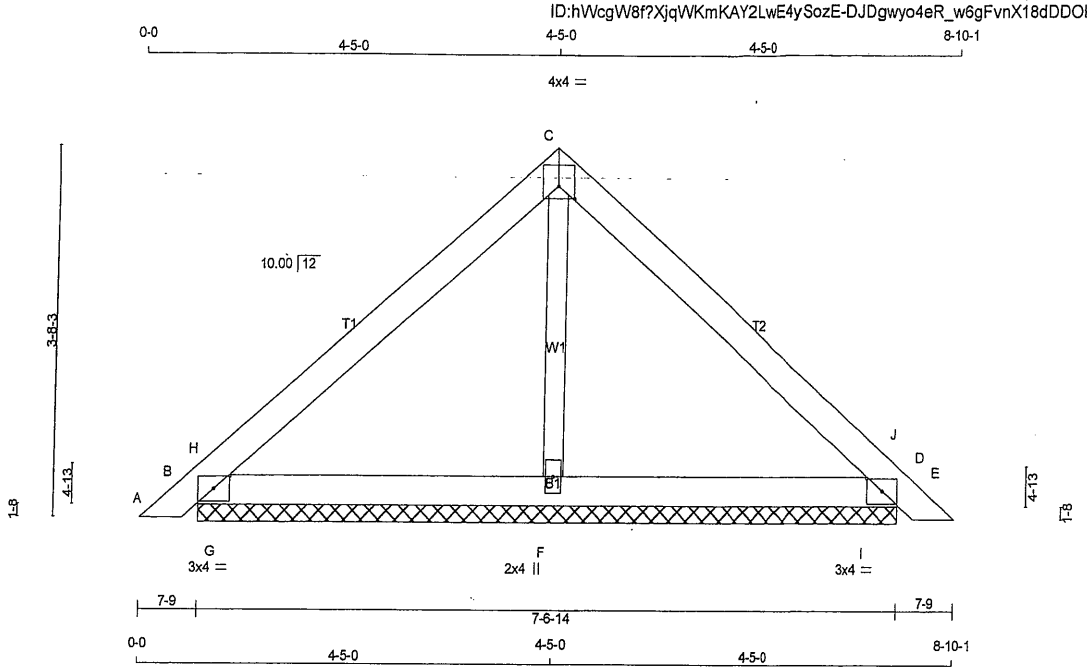

TOTAL WEIGHT = 2 X 24 = 48 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	415	0	415	0	0	7-6-14	7-6-14
D	415	0	415	0	0	7-6-14	7-6-14
F	354	0	354	0	0	7-6-14	7-6-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	313	211 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D	313	211 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
F	305	141 / 0	83 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- H	-84 / 26	-112.4	-112.4	0.11 (1)	6.25
H- C	-196 / 0	-112.4	-112.4	0.19 (1)	6.25
C- J	-196 / 0	-112.4	-112.4	0.19 (1)	6.25
J- D	-84 / 26	-112.4	-112.4	0.11 (1)	6.25
D- E	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- G	0 / 140	-28.0	-28.0	0.19 (1)	10.00
G- F	0 / 140	-28.0	-28.0	0.19 (1)	10.00
F- I	0 / 140	-28.0	-28.0	0.19 (1)	10.00
I- D	0 / 140	-28.0	-28.0	0.19 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-122 / 1	0.02 (1)
G- H	-513 / 25	0.00 (1)
I- J	-513 / 25	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-H:1) , BC=0.19/1.00 (F-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.39/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 17600-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	PB33	11	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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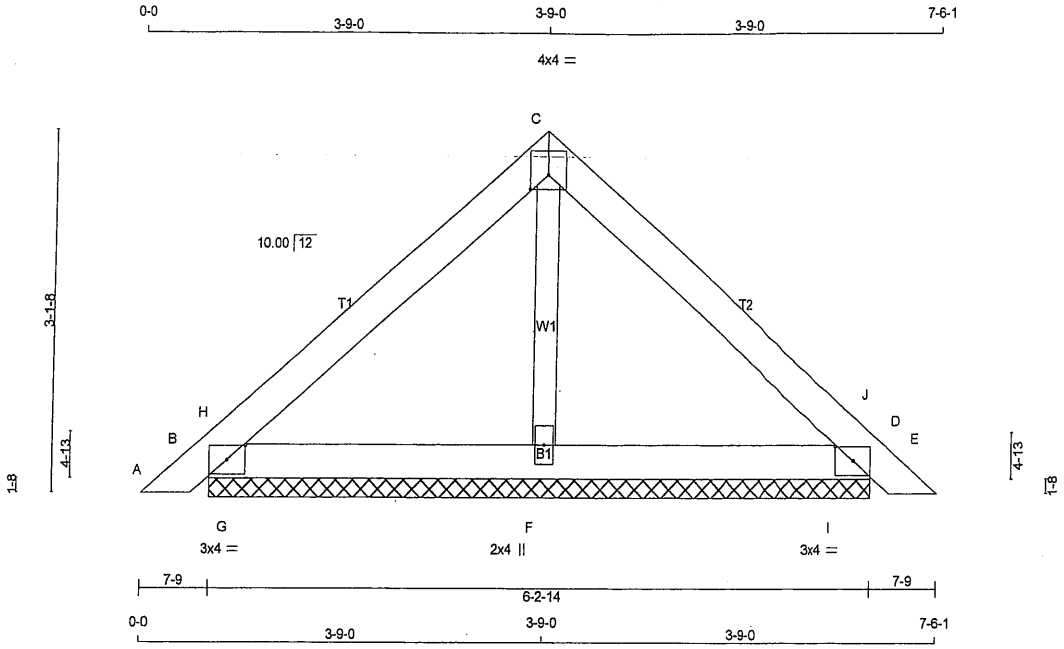

TOTAL WEIGHT = 11 X 20 = 222 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	344	0	344	0	0	6-2-14	6-2-14
D	344	0	344	0	0	6-2-14	6-2-14
F	309	0	309	0	0	6-2-14	6-2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	258	175 / 0	30 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	258	175 / 0	30 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	265	125 / 0	70 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- H	-61 / 0	-112.4	-112.4	0.06 (1)	6.25
H- C	-146 / 0	-112.4	-112.4	0.13 (1)	6.25
C- J	-146 / 0	-112.4	-112.4	0.13 (1)	6.25
J- D	-61 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- G	0 / 104	-28.0	-28.0	0.13 (1)	10.00
G- F	0 / 104	-28.0	-28.0	0.13 (1)	10.00
F- I	0 / 104	-28.0	-28.0	0.13 (1)	10.00
I- D	0 / 104	-28.0	-28.0	0.13 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-117 / 0	0.02 (1)
G- H	-357 / 17	0.00 (1)
I- J	-357 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-H:1) , BC=0.13/1.00 (F-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.27/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM 17801-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	PB33Z	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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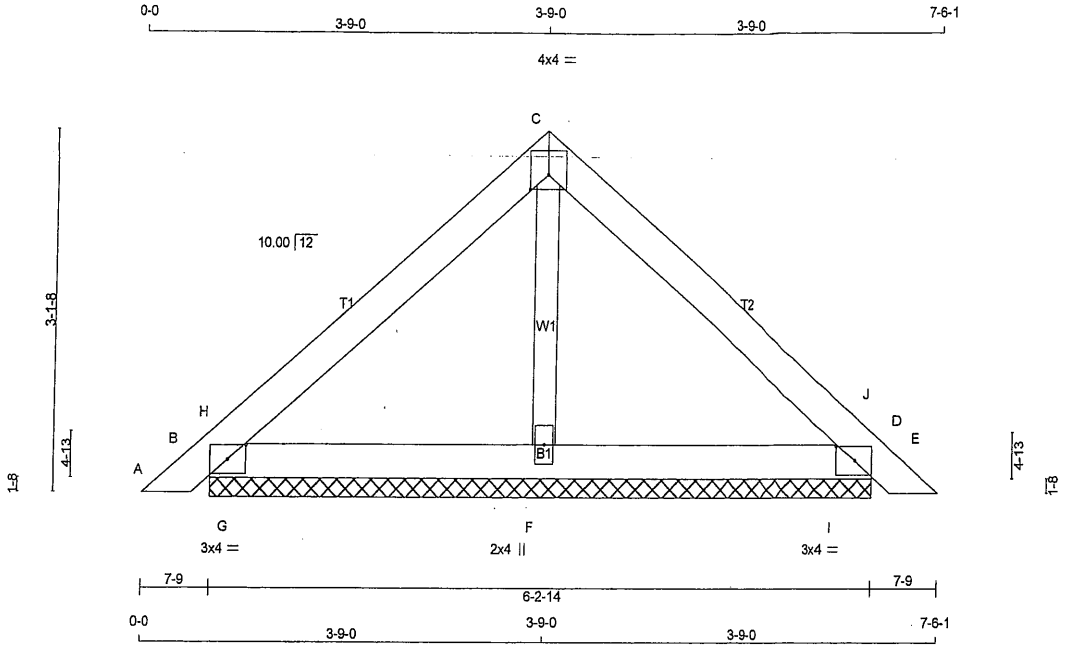

TOTAL WEIGHT = 3 X 20 = 60 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- E 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
B- D 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	346	0	346	0	0	6-2-14	6-2-14
D	346	0	346	0	0	6-2-14	6-2-14
F	305	0	305	0	0	6-2-14	6-2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	260	177 / 0	30 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	260	177 / 0	30 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	262	122 / 0	71 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	LC	MAX CSI
A- B	0 / 18	-112.4	-112.4	0.01	(1)	10.00	F- C	-115 / 0		0.01 (1)
B- H	-61 / 0	-112.4	-112.4	0.02	(1)	6.25	G- H	-345 / 17		0.00 (1)
H- C	-149 / 0	-112.4	-112.4	0.04	(1)	6.25	I- J	-345 / 17		0.00 (1)
C- J	-149 / 0	-112.4	-112.4	0.04	(1)	6.25				
J- D	-61 / 0	-112.4	-112.4	0.02	(1)	6.25				
D- E	0 / 18	-112.4	-112.4	0.01	(1)	10.00				
B- G	0 / 107	-28.0	-28.0	0.04	(1)	10.00				
G- F	0 / 107	-28.0	-28.0	0.04	(1)	10.00				
F- I	0 / 107	-28.0	-28.0	0.04	(1)	10.00				
I- D	0 / 107	-28.0	-28.0	0.04	(1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-H:1) , BC=0.04/1.00 (B-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.09/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (D) (INPUT = 0.90)
JSI METAL= 0.02 (D) (INPUT = 1.00)

DWG NO. TAM 17B02-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	PB34	1	1	OBC2012_6TCDL TRUSS DESC.	

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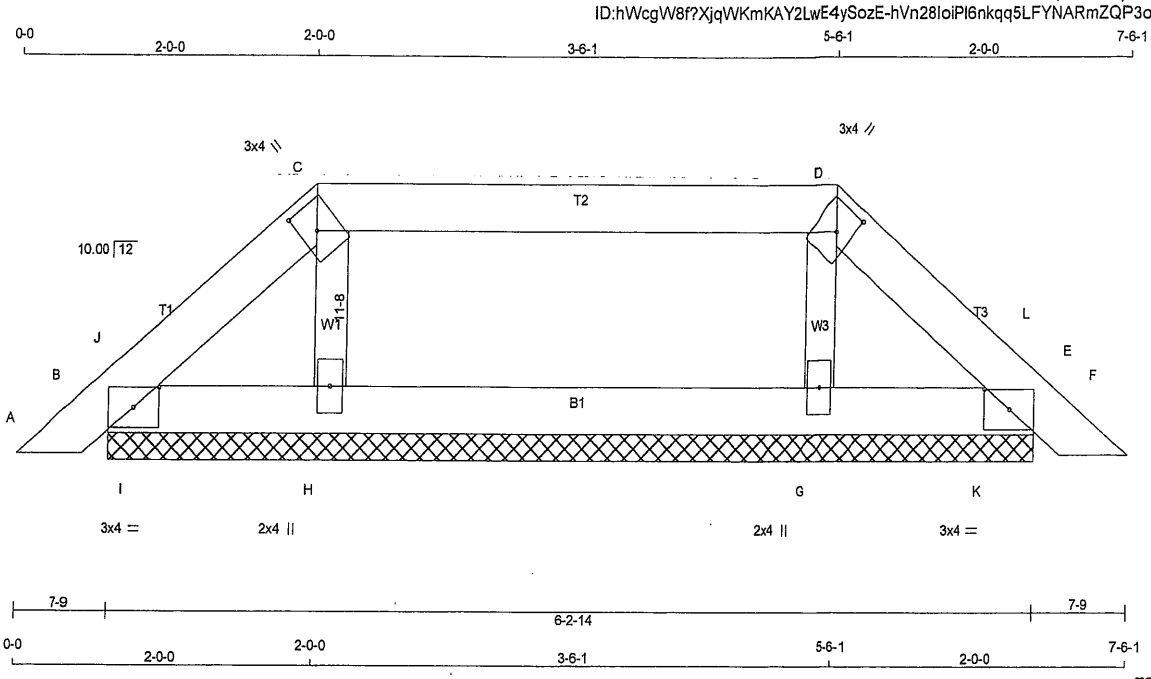

TOTAL WEIGHT = 19 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	189	0	189	0	0	6-2-14	6-2-14
E	189	0	189	0	0	6-2-14	6-2-14
H	310	0	310	0	0	6-2-14	6-2-14
G	310	0	310	0	0	6-2-14	6-2-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	132	106 / 0	4 / 0	0 / 0	0 / 0	22 / 0	0 / 0
E	132	106 / 0	4 / 0	0 / 0	0 / 0	22 / 0	0 / 0
H	258	131 / 0	62 / 0	0 / 0	0 / 0	65 / 0	0 / 0
G	258	131 / 0	62 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00	H- C	-212 / 0	0.03 (1)
B- J	-59 / 2	-112.4	-112.4	0.01 (1)	6.25	G- D	-212 / 0	0.03 (1)
J- C	-71 / 0	-112.4	-112.4	0.02 (1)	6.25	I- J	-93 / 0	0.00 (1)
C- D	-33 / 0	-112.4	-112.4	0.23 (1)	6.25	K- L	-93 / 0	0.00 (1)
D- L	-71 / 0	-112.4	-112.4	0.02 (1)	6.25			
L- E	-59 / 2	-112.4	-112.4	0.01 (1)	6.25			
E- F	0 / 18	-112.4	-112.4	0.02 (1)	10.00			
B- I	0 / 51	-28.0	-28.0	0.03 (1)	10.00			
I- H	0 / 51	-28.0	-28.0	0.06 (3)	10.00			
H- G	0 / 33	-28.0	-28.0	0.06 (3)	10.00			
G- K	0 / 51	-28.0	-28.0	0.06 (3)	10.00			
K- E	0 / 51	-28.0	-28.0	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1) , BC=0.06/1.00 (H-I:3) , WB=0.03/1.00 (D-G:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.04 (H) (INPUT = 1.00)

DWG NO. TAM 17803-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	PB35	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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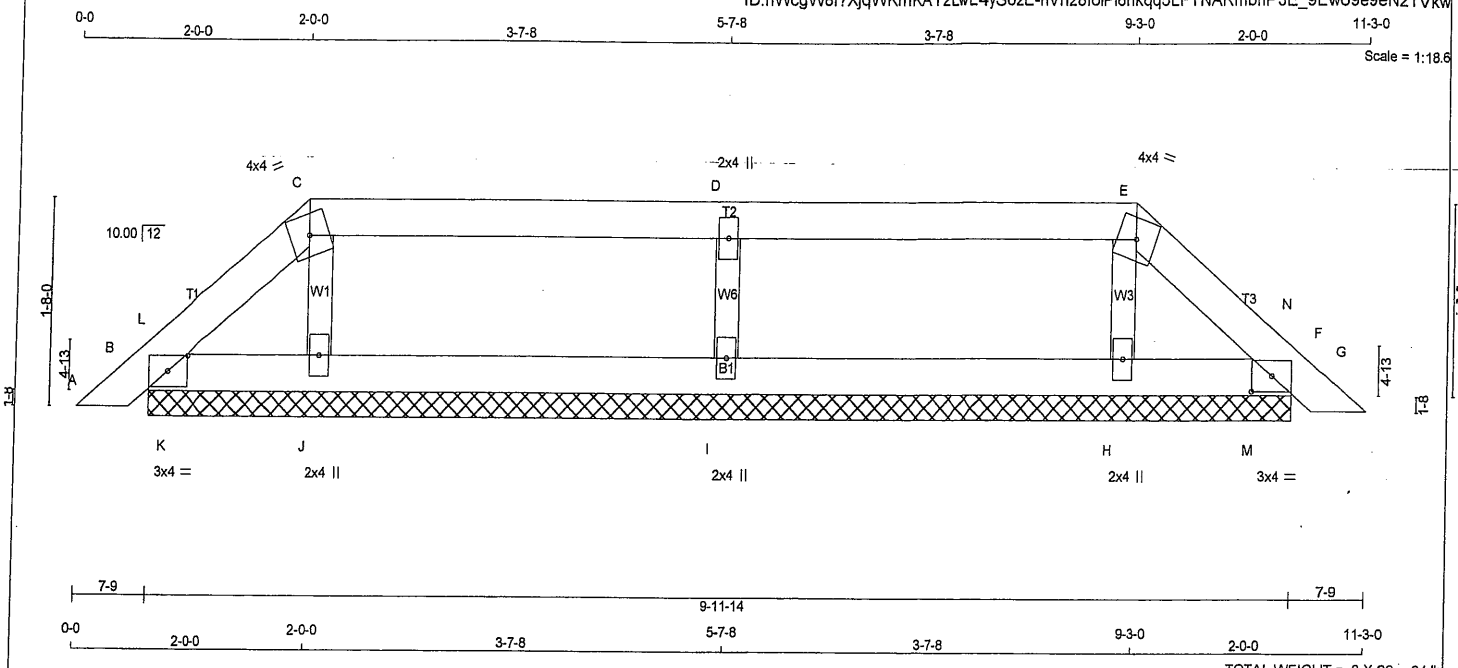

TOTAL WEIGHT = 3 X 28 = 84 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	TOP
C- E 1	12	TOP
E- G 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
B- F 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	175	0	175	0	0	9-11-14	9-11-14
F	175	0	175	0	0	9-11-14	9-11-14
J	282	0	282	0	0	9-11-14	9-11-14
H	282	0	282	0	0	9-11-14	9-11-14
I	611	0	611	0	0	9-11-14	9-11-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	123	98 / 0	4 / 0	0 / 0	0 / 0	21 / 0	0 / 0
F	123	98 / 0	4 / 0	0 / 0	0 / 0	21 / 0	0 / 0
J	239	115 / 0	61 / 0	0 / 0	0 / 0	62 / 0	0 / 0
H	239	115 / 0	61 / 0	0 / 0	0 / 0	62 / 0	0 / 0
I	477	292 / 0	79 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.01 (1)		10.00
B- L	-42 / 3	-112.4	-112.4	0.00 (1)		6.25
L- C	-50 / 0	-112.4	-112.4	0.01 (1)		6.25
C- D	-21 / 0	-112.4	-112.4	0.08 (1)		6.25
D- E	-21 / 0	-112.4	-112.4	0.08 (1)		6.25
E- N	-50 / 0	-112.4	-112.4	0.01 (1)		6.25
N- F	-42 / 3	-112.4	-112.4	0.00 (1)		6.25
F- G	0 / 18	-112.4	-112.4	0.01 (1)		10.00
B- K	0 / 36	-28.0	-28.0	0.01 (1)		10.00
K- J	0 / 36	-28.0	-28.0	0.02 (3)		10.00
J- I	0 / 21	-28.0	-28.0	0.03 (3)		10.00
I- H	0 / 21	-28.0	-28.0	0.03 (3)		10.00
H- M	0 / 36	-28.0	-28.0	0.02 (3)		10.00
M- F	0 / 36	-28.0	-28.0	0.01 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-183 / 0	0.01 (1)
H- E	-183 / 0	0.01 (1)
I- D	-509 / 0	0.02 (1)
K- L	-88 / 0	0.00 (1)
M- N	-88 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1) , BC=0.03/1.00 (I-J:3) , WB=0.02/1.00 (D-I:1) , SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (D) (INPUT = 0.90)
JSI METAL= 0.03 (D) (INPUT = 1.00)

DWG NO. TAM 17804-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287687	J1S	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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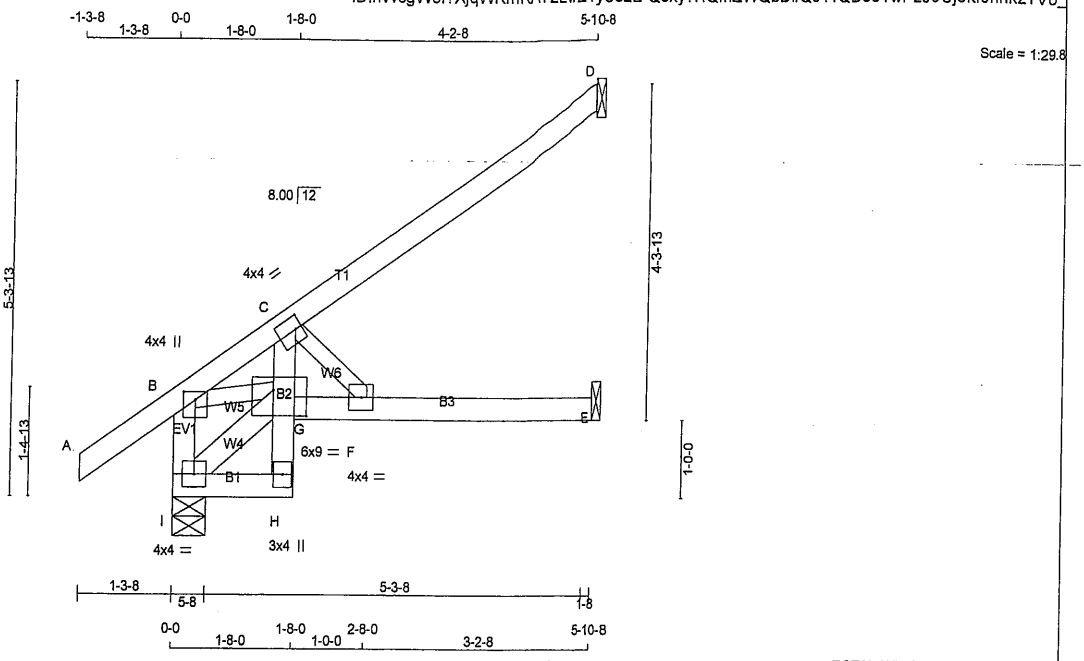


TOTAL WEIGHT = 7 X 24 = 166 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-I	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	585	0	585	0	0	5-8	5-8
D	244	0	244	0	0	1-8 (1-7)	1-8
E	151	0	157	0	0	1-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	447	290 / 0	62 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	174	134 / 0	9 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	143	46 / 0	53 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, D

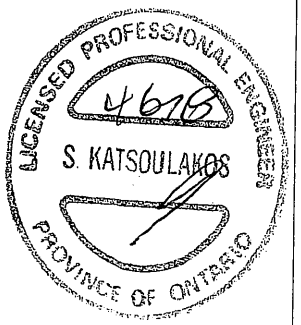
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-546 / 0	0.0	0.0	0.06 (1)	7.81
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-507 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	0 / 11	-112.4	-112.4	0.36 (1)	10.00
I- H	0 / 23	-28.0	-28.0	0.02 (3)	10.00
H- G	0 / 37	0.0	0.0	0.04 (1)	10.00
G- C	0 / 427	0.0	0.0	0.11 (1)	10.00
G- F	0 / 474	-28.0	-28.0	0.54 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.47 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-29 / 0	0.00 (1)
B- G	0 / 391	0.09 (1)
C- F	-601 / 0	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 672 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 398 (0.18")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.54/1.00 (F-G:1) , WB=0.09/1.00 (B-G:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.18 (F) (INPUT = 1.00)

DWG NO. TAM 17771-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287688	J31S	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Fri Apr 6 08:34:10 2018 Page 1
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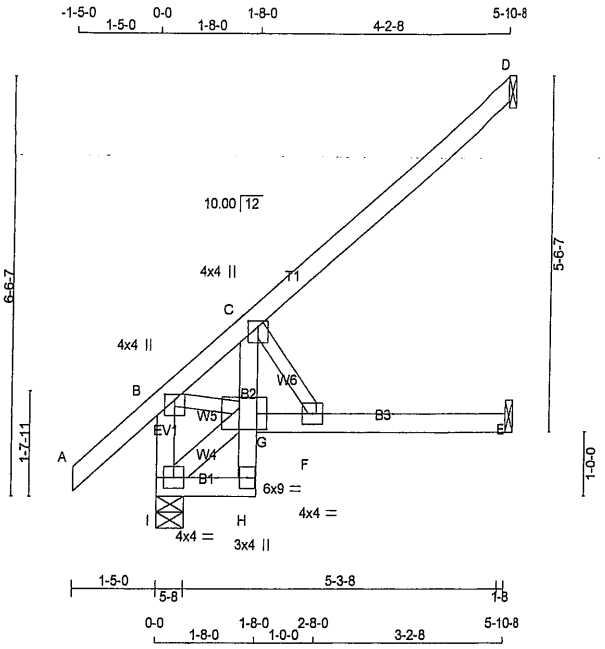


TOTAL WEIGHT = 7 X 26 = 181 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-I	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	604	0	604	0	0	5-8	5-8
D	240	0	240	0	0	1-8	1-8
E	151	0	158	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	460	301 / 0	62 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	171	132 / 0	8 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	143	46 / 0	53 / 0	0 / 0	0 / 0	44 / 0	0 / 0

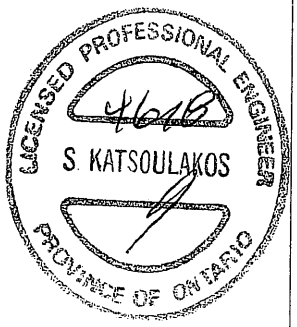
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-573 / 0	0.0	0.0	0.06 (1)	7.81	I- G	-17 / 0	0.00 (1)
A- B	0 / 54	-112.4	-112.4	0.18 (1)	10.00	B- G	0 / 268	0.06 (1)
B- C	-392 / 0	-112.4	-112.4	0.18 (1)	6.25	C- F	-490 / 0	0.07 (1)
C- D	-3 / 11	-112.4	-112.4	0.35 (1)	10.00			
I- H	0 / 13	-28.0	-28.0	0.02 (3)	10.00			
H- G	0 / 37	0.0	0.0	0.02 (1)	10.00			
G- C	0 / 361	0.0	0.0	0.08 (1)	10.00			
G- F	0 / 328	-28.0	-28.0	0.52 (1)	10.00			
F- E	0 / 0	-28.0	-28.0	0.47 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 683 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 402 (0.18")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.52/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

DWG NO. TAM 1779B-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287657	J55S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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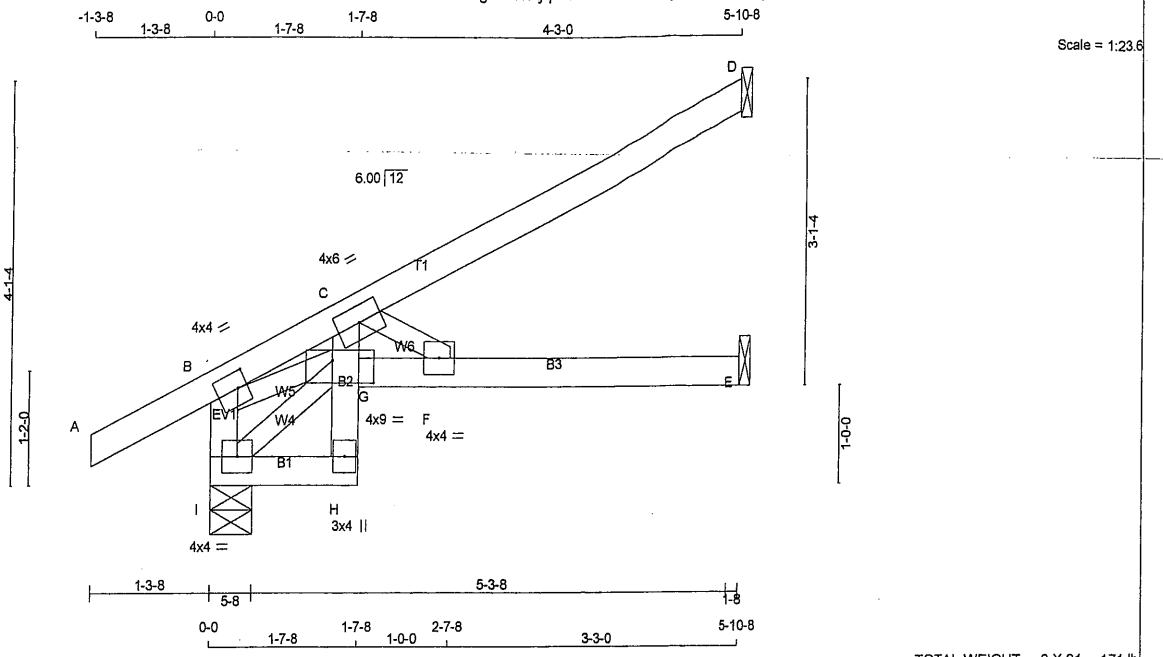


TOTAL WEIGHT = 8 X 21 = 171 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-I	MT20	4.0	9.0	2.75	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	583	0	583	0	0	5-8	5-8
D	251	0	251	0	0	1-8	1-8
E	144	0	152	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	445	289 / 0	62 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	179	138 / 0	10 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	137	43 / 0	52 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-513 / 0	0.0	0.0	0.05 (1)		7.81
A- B	0 / 34	-112.4	-112.4	0.15 (1)		10.00
B- C	-694 / 0	-112.4	-112.4	0.15 (1)		6.25
C- D	0 / 10	-112.4	-112.4	0.38 (1)		10.00
I- H	0 / 65	-28.0	-28.0	0.02 (2)		10.00
H- G	0 / 36	0.0	0.0	0.10 (1)		10.00
G- C	0 / 478	0.0	0.0	0.17 (1)		10.00
G- F	0 / 771	-28.0	-28.0	0.57 (1)		10.00
F- E	0 / 0	-28.0	-28.0	0.44 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-81 / 0	0.01 (1)
B- G	0 / 596	0.13 (1)
C- F	-850 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 673 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 399 (0.18")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.57/1.00 (F-G:1) , WB=0.13/1.00 (B-G:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.76 (B) (INPUT = 0.90)
SI METAL= 0.30 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4610
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM V7830-18
STRUCTURAL COMPONENT ONLY

Site Copy

Boise Cascade

Triple 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP

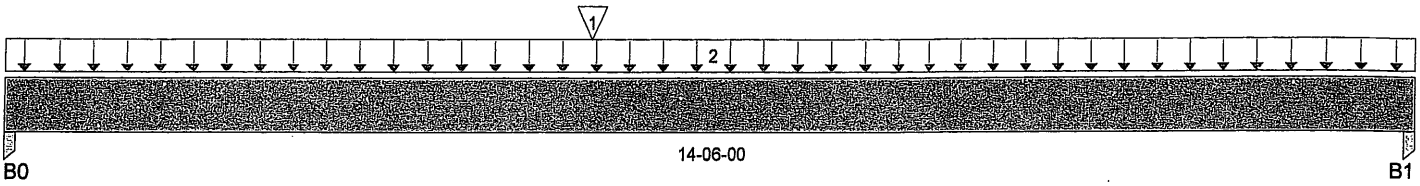
BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

April 5, 2018 09:56:05

Build 6536

Job Name:	BAYVIEW WELLINGTON HOMES	File Name:	BC CALC Project
Address:	S42-8C-EL:C (BM54)	Description:	Designs\FB01
City, Province, Postal Code:	BRADFORD,	Specifier:	
Customer:		Designer:	SUNNY
Code reports:	CCMC 12472-R	Company:	
		Misc:	



Total Horizontal Product Length = 14-06-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 5-1/2"	1,616 / 0	2,095 / 0	4,937 / 0	
B1, 5-1/2"	1,511 / 0	1,962 / 0	4,594 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	POINT LOAD	Conc. Pt. (lbs)	L	06-00-00	06-00-00	575	732	1,875		n/a
2	TYING IN SPAN	Unf. Area (lb/ft^2)	L	00-00-00	14-06-00	11	13	33		16-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	39,795 ft-lbs	75,349 ft-lbs	52.8%	5	06-00-00
End Shear	8,936 lbs	25,578 lbs	34.9%	5	01-07-08
Total Load Defl.	L/426 (0.386")	0.685"	56.4%	13	07-02-03
Live Load Defl.	L/580 (0.284")	0.457"	62.1%	17	07-02-03
Max Defl.	0.386"	1"	38.6%	13	07-02-03
Span / Depth	11.8	n/a	n/a		00-00-00

Bearing Supports

		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	5-1/2" x 5-1/4"	10,832 lbs	n/a	30.7%	Unspecified
B1	Post	5-1/2" x 5-1/4"	10,099 lbs	n/a	28.7%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
Conforms To OBC 2012
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9

LICENSED PROFESSIONAL ENGINEER
4616
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PROVINCE OF ONTARIO

DWG NO. TAM 17834-17
STRUCTURAL
COMPONENT ONLY

Site Copy

Boise Cascade

Triple 1-3/4" x 14" VERSA-LAM® 2.0 3100 SP

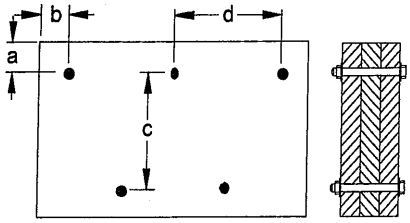
Dry | 1 span | No cantilevers | 0/12 slope (deg)

April 5, 2018 09:56:05

BC CALC® Design Report

Build 6536		File Name:	BC CALC Project
Job Name:	BAYVIEW WELLINGTON HOMES	Description:	Designs\FB01
Address:	S42-8C EL:C (BM54)	Specifier:	
City, Province, Postal Code:	BRADFORD,	Designer:	SUNNY
Customer:		Company:	
Code reports:	CCMC 12472-R	Misc:	

Connection Diagram



a minimum = 2½" c = 10"
b minimum = 2-1/2" d = 12"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.
Bolts are assumed to be Grade A307 or Grade 2 or higher.
Member has no side loads.
Connectors are: 1/2 in. Staggered Through Bolt

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

LICENSED PROFESSIONAL ENGINEER
4678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17831 -18
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

THGQ/THGQH – Truss Girder Hangers

SIMPSON Strong-Tie®

Material: THGQ—7 gauge, THGQH—3 gauge

Finish: THGQ—G90 Galvanized,
THGQH—Simpson Strong-Tie® gray paint

Design:

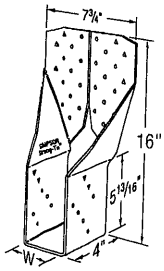
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15% for short-term load duration. No further increase is allowed.
- Designer must ensure that vertical web member supporting a hanger is capable of resisting loads based on net cross-section.
- Girder truss must be a minimum of 2 plies.
- Bearing assumes Q_r/A_b and Q'_r/A'_b = 812 psi D.Fir-L and 615 psi S-P-F. Truss plates on supported member must be as per 6.5.4 and 7.5.9 TPIC 2014 to archieve values shown.
- All multiple members must be fastened together to act as a single unit independent of the hanger fasteners.
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses.

Installation:

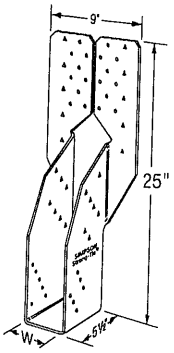
- Use all specified fasteners.
- Fill all round holes for min values and all round and triangle holes for max values.
- Strong-Drive® SDS screws driven through truss plates must be approved by the truss designer. Pre-drilling, using a 5⁄32" bit, is required.
- Connector must be installed, centred on girder vertical web.

Options:

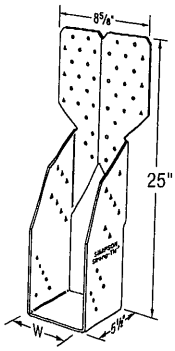
- These hangers may be skewed 45°
- See current catalogue for options



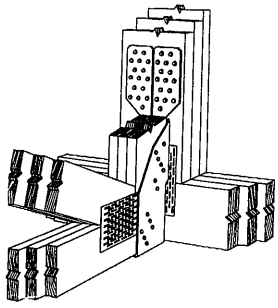
THGQ2-SDS3
(THGQ3-SDS3 similar)



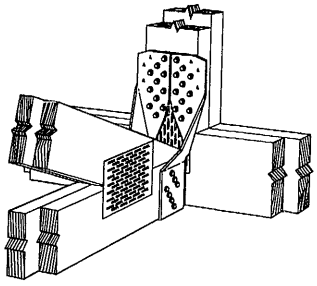
THGQH2-SDS3



THGQH3-SDS4.5
(THGQH4-SDS6 similar)



Typical THGQH3-SDS4.5 Installation



Typical THGQ2-SDS3 Installation

Model No.	Width (in.)	Max. B.C. Depth	Min. Vert Web Size	Fasteners		Factored Resistance (lb.)			
						D.Fir-L		S-P-F	
				Header	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
THGQ2-SDS3 (Min.)	3 5⁄16	2x6	2x8	(22) SDS ¼" x 3"	(10) SDS ¼" x 3"	5205	11655	3750	8395
THGQ2-SDS3 (Max.)	3 5⁄16	2x6	2x10	(28) SDS ¼" x 3"	(14) SDS ¼" x 3"	6555	18055	4720	13000
THGQH2-SDS3 (Min.)	3 5⁄16	2x10	2x8	(18) SDS ¼" x 3"	(14) SDS ¼" x 3"	5790	12555	4170	9040
THGQH2-SDS3 (Max.)	3 5⁄16	2x10	2x10	(28) SDS ¼" x 3"	(26) SDS ¼" x 3"	14190	18455	10215	15160
THGQ3-SDS4.5 (Min.)	4 15⁄16	2x6	2x8	(22) SDS ¼" x 4½"	(10) SDS ¼" x 4½"	5205	11655	3750	8395
THGQ3-SDS4.5 (Max.)	4 15⁄16	2x6	2x10	(28) SDS ¼" x 4½"	(14) SDS ¼" x 4½"	6555	17760	4720	12785
THGQH3-SDS4.5 (Min.)	4 15⁄16	2x10	2x10	(32) SDS ¼" x 4½"	(14) SDS ¼" x 4½"	5790	17860	4170	12860
THGQH3-SDS4.5 (Max.)	4 15⁄16	2x10	2x12	(38) SDS ¼" x 4½"	(26) SDS ¼" x 4½"	14190	21055	10215	15160
THGQH4-SDS6 (Min.)	6 9⁄16	2x12	2x10	(34) SDS ¼" x 6"	(14) SDS ¼" x 6"	5790	17860	4170	12860
THGQH4-SDS6 (Max.)	6 9⁄16	2x12	2x12	(40) SDS ¼" x 6"	(26) SDS ¼" x 6"	14190	24870	10215	17905

1. Factored resistances have been increased 15% for earthquake or wind load with no further increase allowed; reduce where other loads govern.
2. Minimum 2-ply girder required.
3. Connector must be installed centred on girder vertical webs.
4. The thickness of the supporting girder must be equal to or greater than the screw length. For applications where the length of the supplied screws exceeds the thickness of the supporting girder, 3" or 4½" screws may be substituted for the longer length screws with no load reduction, or a shim block may used as approved by the Designer.

LIMIT STATES DESIGN

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T-SPECTHGQ17 3/17 exp. 6/19

(800) 999-5099
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TECHNICAL BULLETIN

CP Crush Plates – Bearing Enhancers

SIMPSON Strong-Tie®

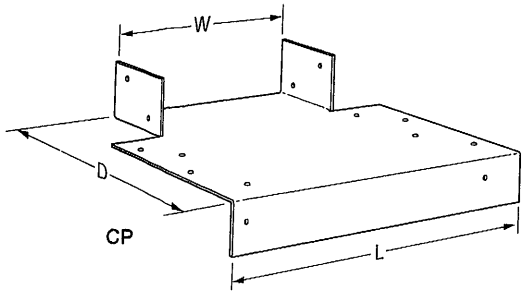
The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

Finish: G90 galvanized

Design:

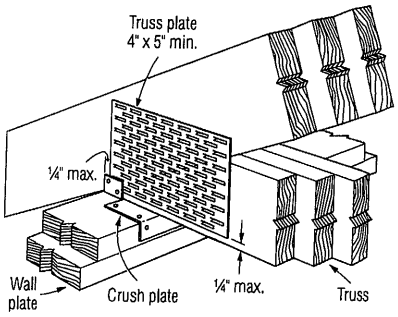
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and Q'_r/A'_b = 812 psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.



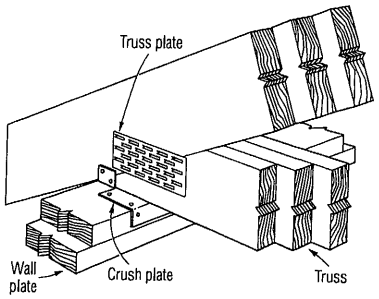
CP

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.) W	D	L	Fasteners Wall Plate	Truss	Factored Resistance (lb.) D.Fir-L Uplift (K_D=1.15)	D.Fir-L Bearing (K_D=1.00)	S-P-F Uplift (K_D=1.15)	S-P-F Bearing (K_D=1.00)
CASE 1 (Truss Plate Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390
CASE 2 (No Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.

LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

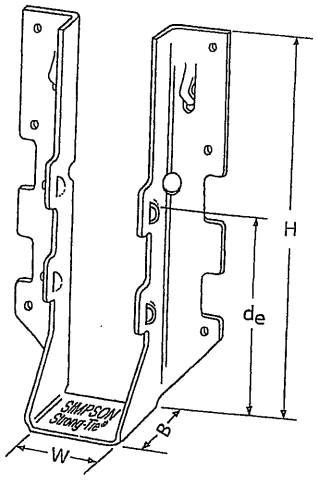
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

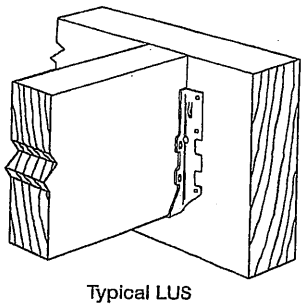
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

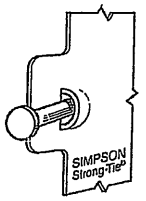


LUS28


Typical LUS Installation

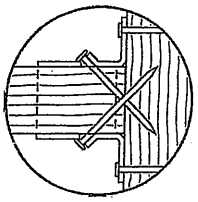
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LIMIT STATES DESIGN

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T-SPECLUS17 3/17 exp. 6/19

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

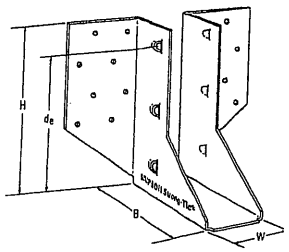
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

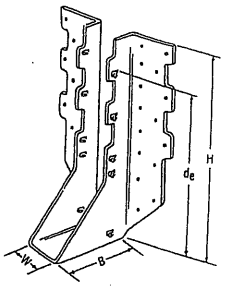
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

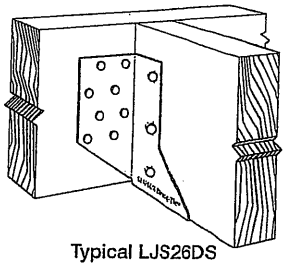
- See current catalogue for options



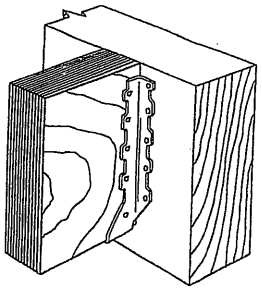
LJS26DS



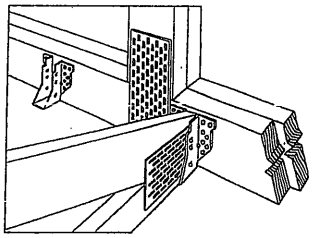
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



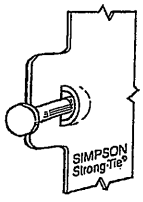
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

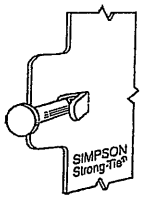
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

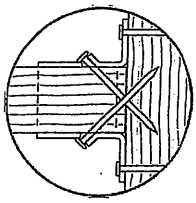


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

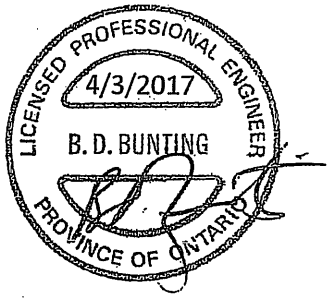
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHUS17 3/17 exp. 6/19

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

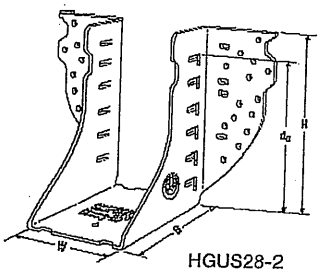
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

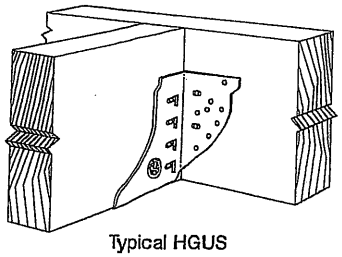
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

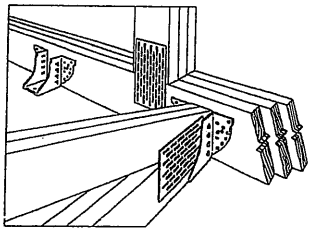
Options:

- See current catalogue for options



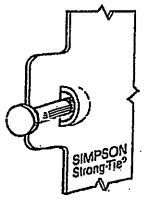
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

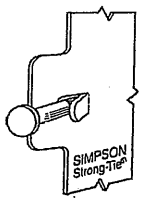
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

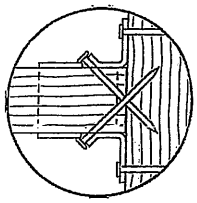


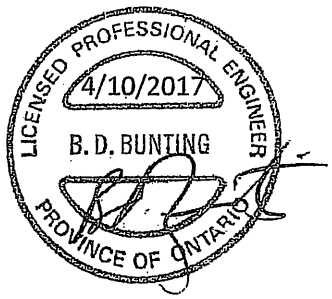
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

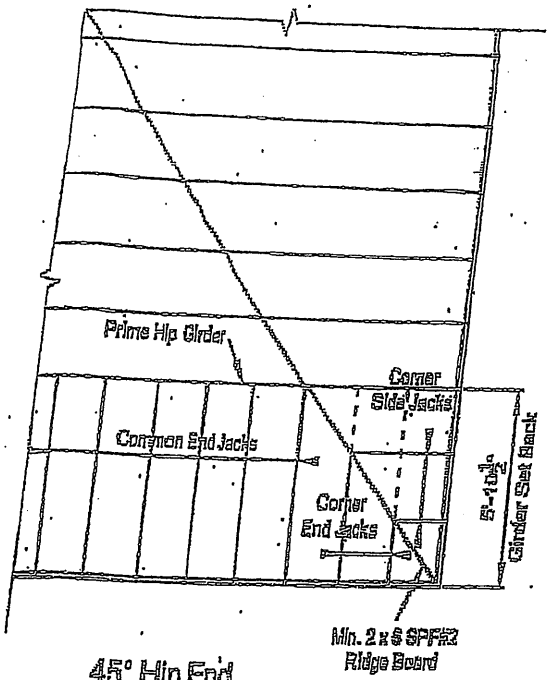
This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017.
This information is updated periodically and should not be relied upon after June 30, 2019.
Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

T-SPECHGUS17 3/17 exp 6/19

(800) 999-5099
strongtie.com

ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

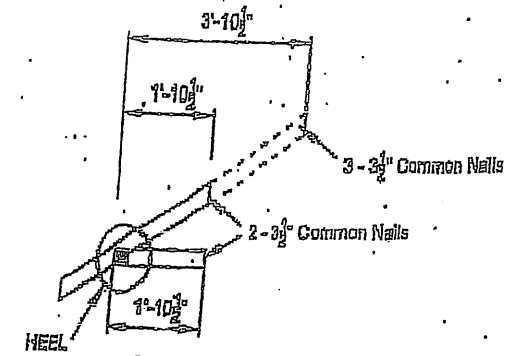
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

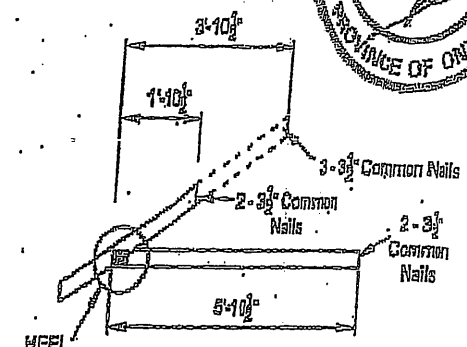
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	= 59.9 P.S.F.

DWG NO T&M 24175.14
STRUCTURAL COMPONENT ONLY

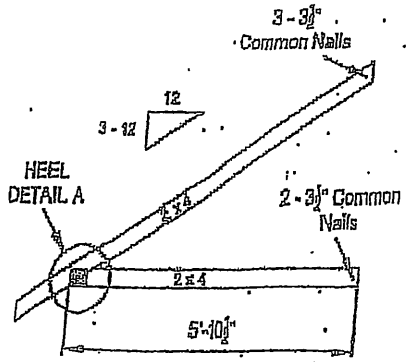
LICENSED PROFESSIONAL ENGINEER
1-2017
S. KATSOULAKOS
PROVINCE OF ONTARIO



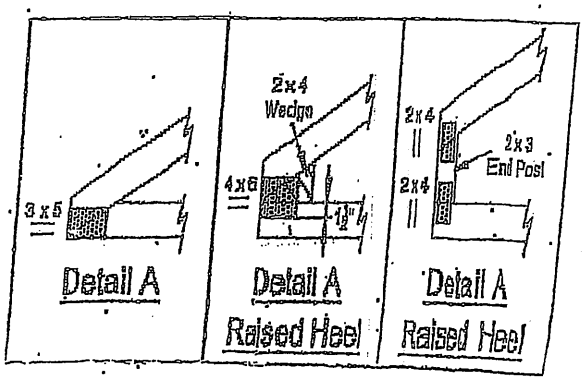
Corner Side Jacks



Corner End Jacks



Common End Jacks



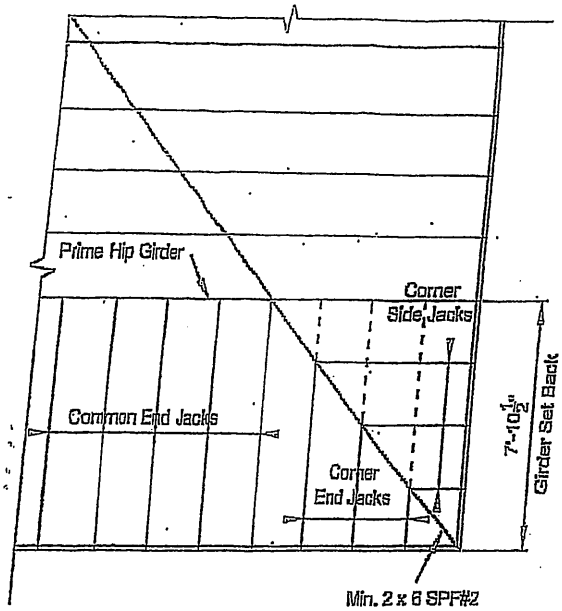
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Site Copy

ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



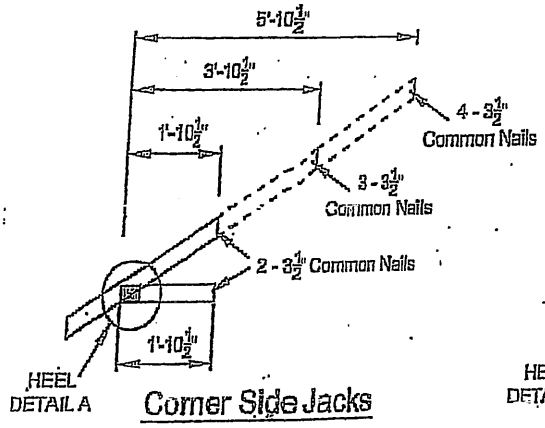
45° Hip End

DESIGN LOAD:

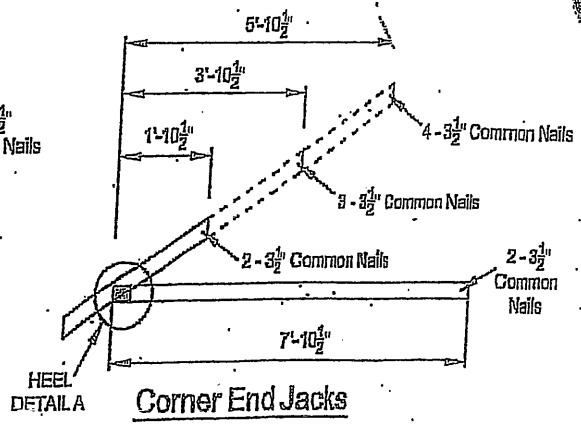
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD,	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503 T4
STRUCTURAL
COMPONENT ONLY

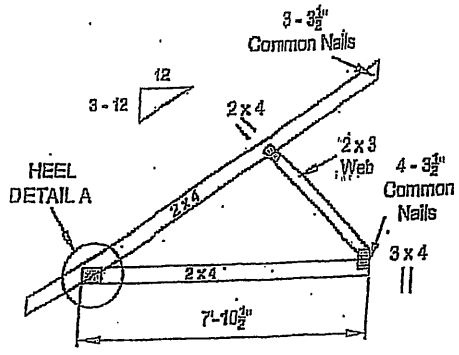
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



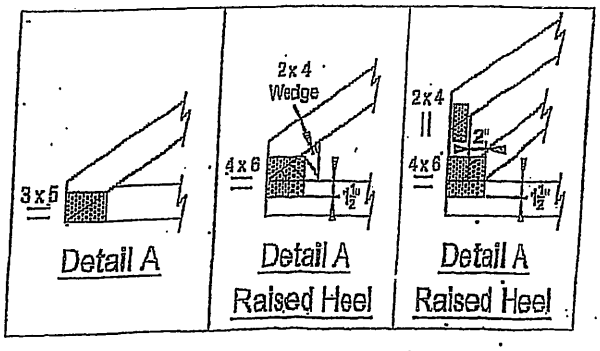
Corner Side Jacks



Corner End Jacks



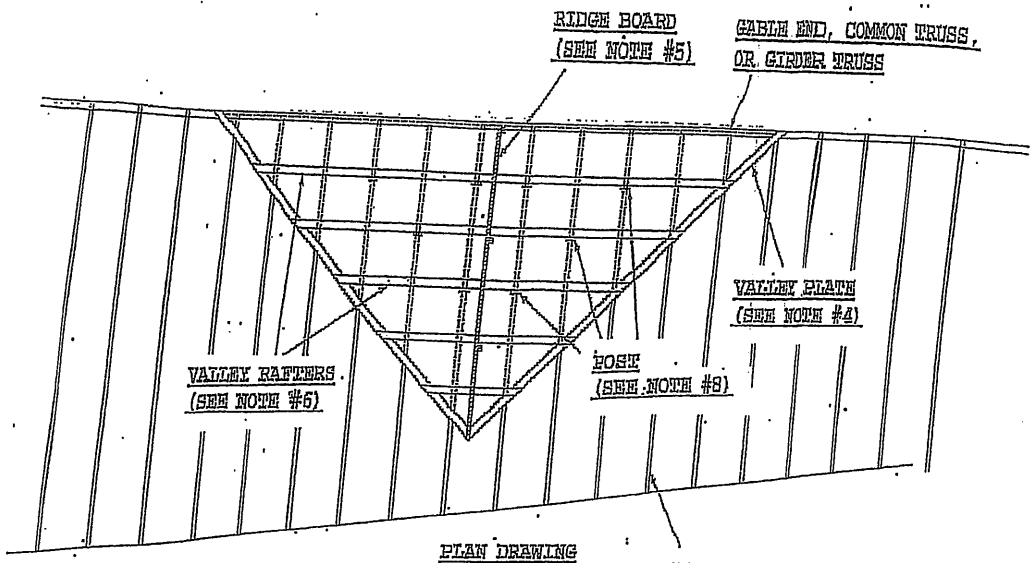
Common End Jacks



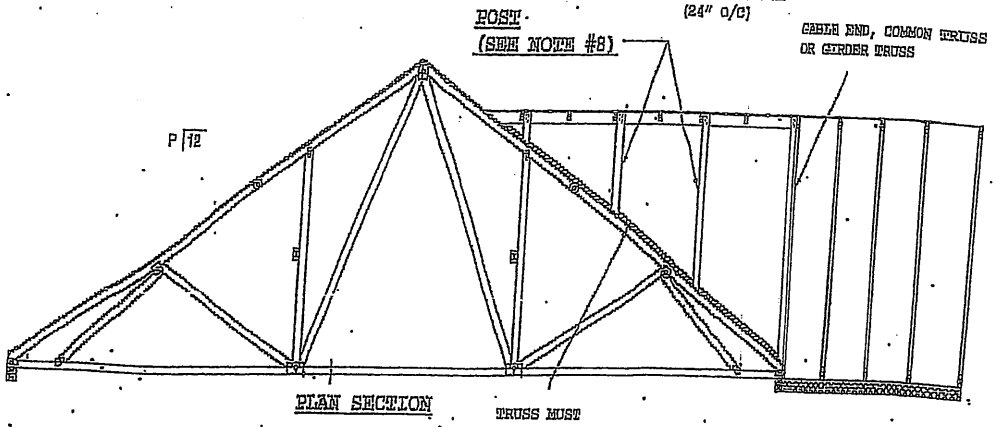
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

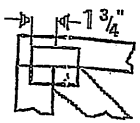
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

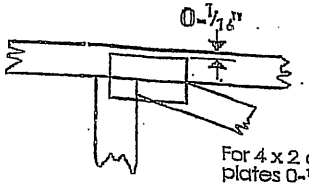
DWG NO TAM 6305.14
STRUCTURAL COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

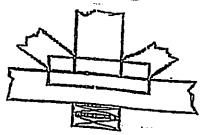
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

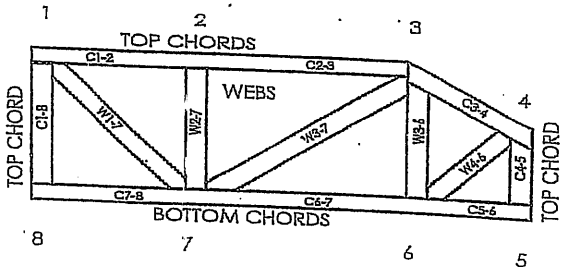
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.