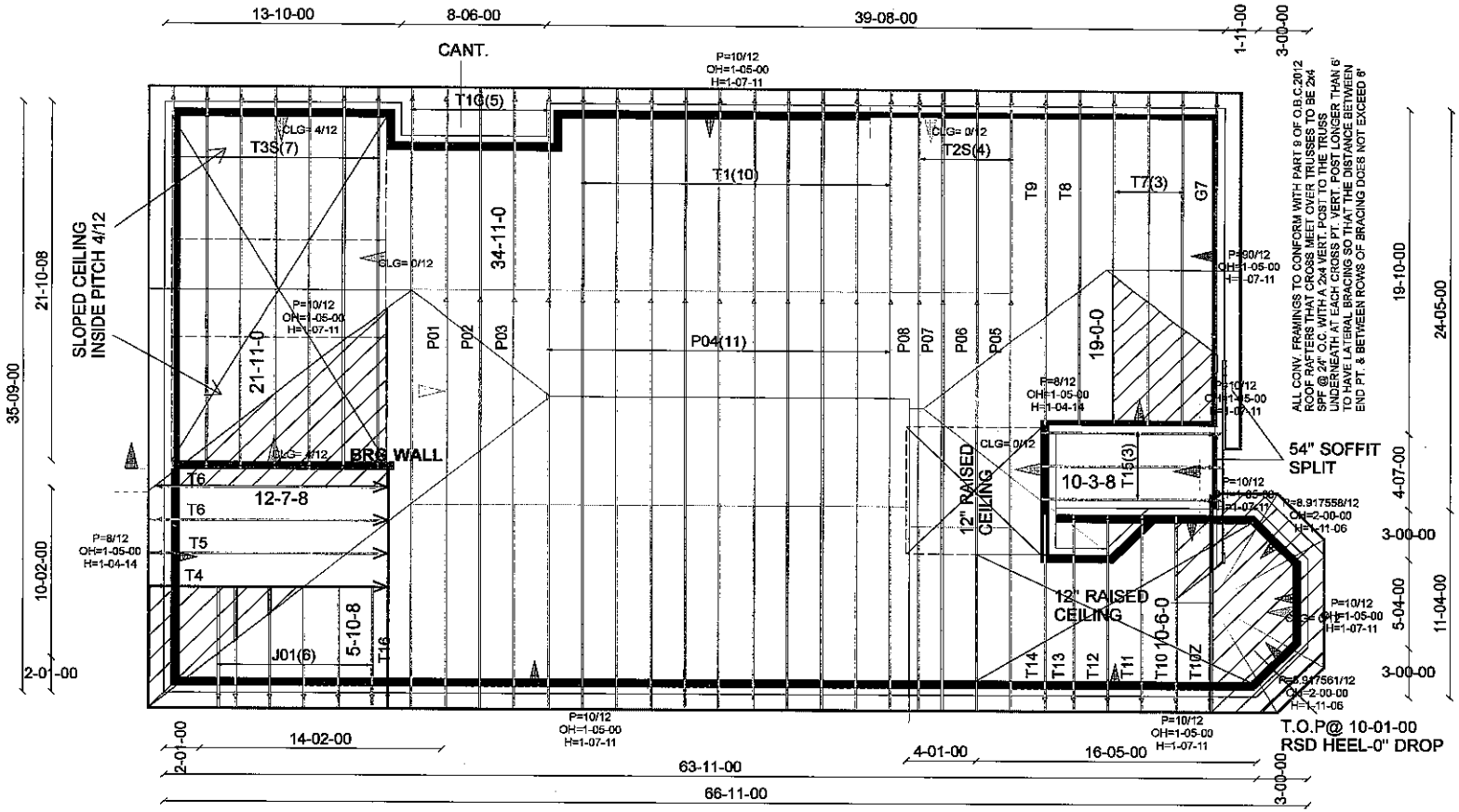


ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)

DENOTES
CONV.
FRAMING

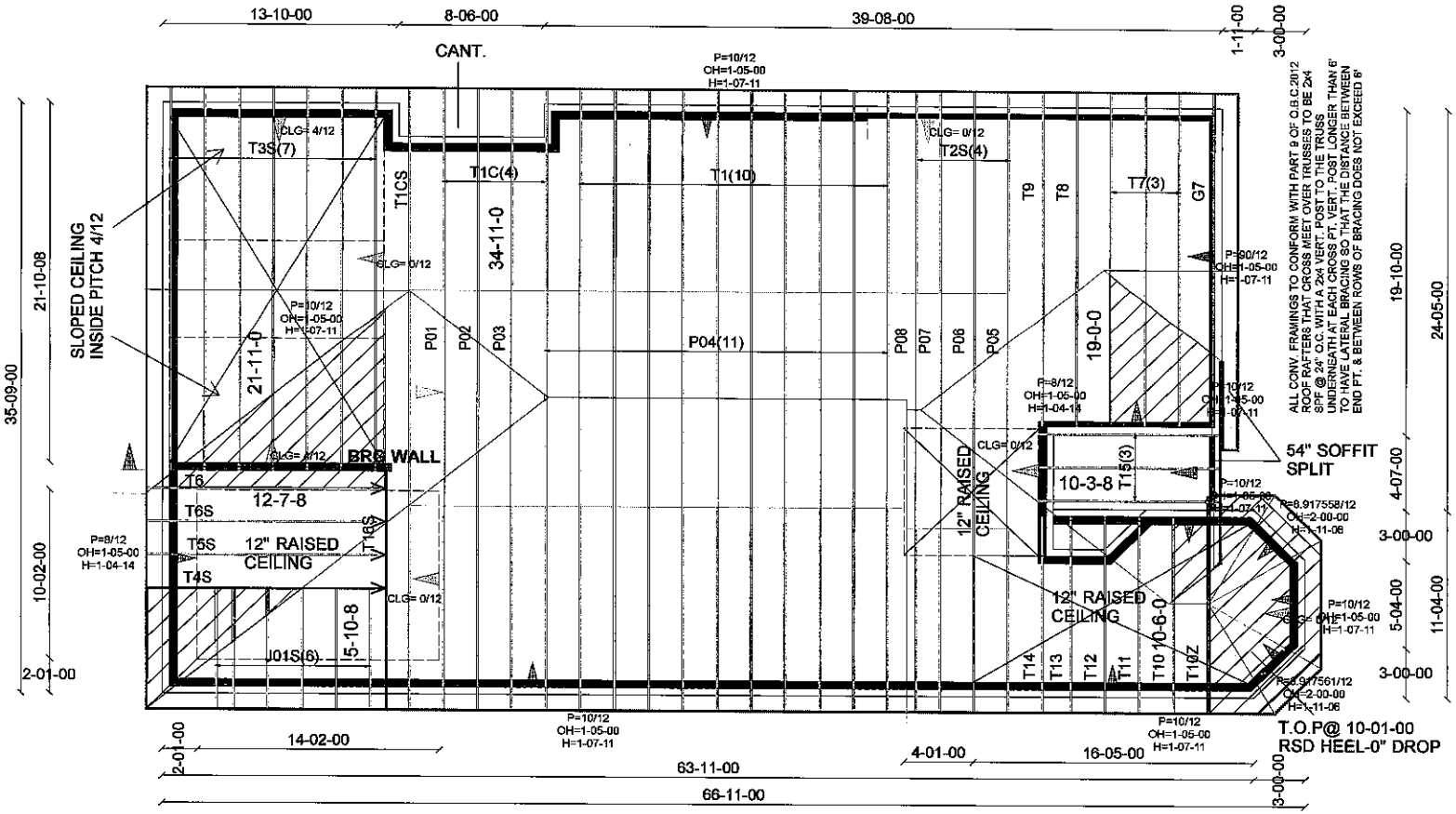
DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-1B / A STD COFF OR REAR UPGRADE**
	Layout ID: **287664**	Project: **GREEN VALLEY ESTATES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: **94071**	Date: **12-Mar-18** Designer: AC	Mitek ver 7.5.0

Site Copy



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-1B / A OPT OR REAR UPGRADE**
	Layout ID: **287663**	Project: **GREEN VALLEY ESTATES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: **94071**	Date: **12-Mar-18** Designer: AC	Mitek ver 7.5.0

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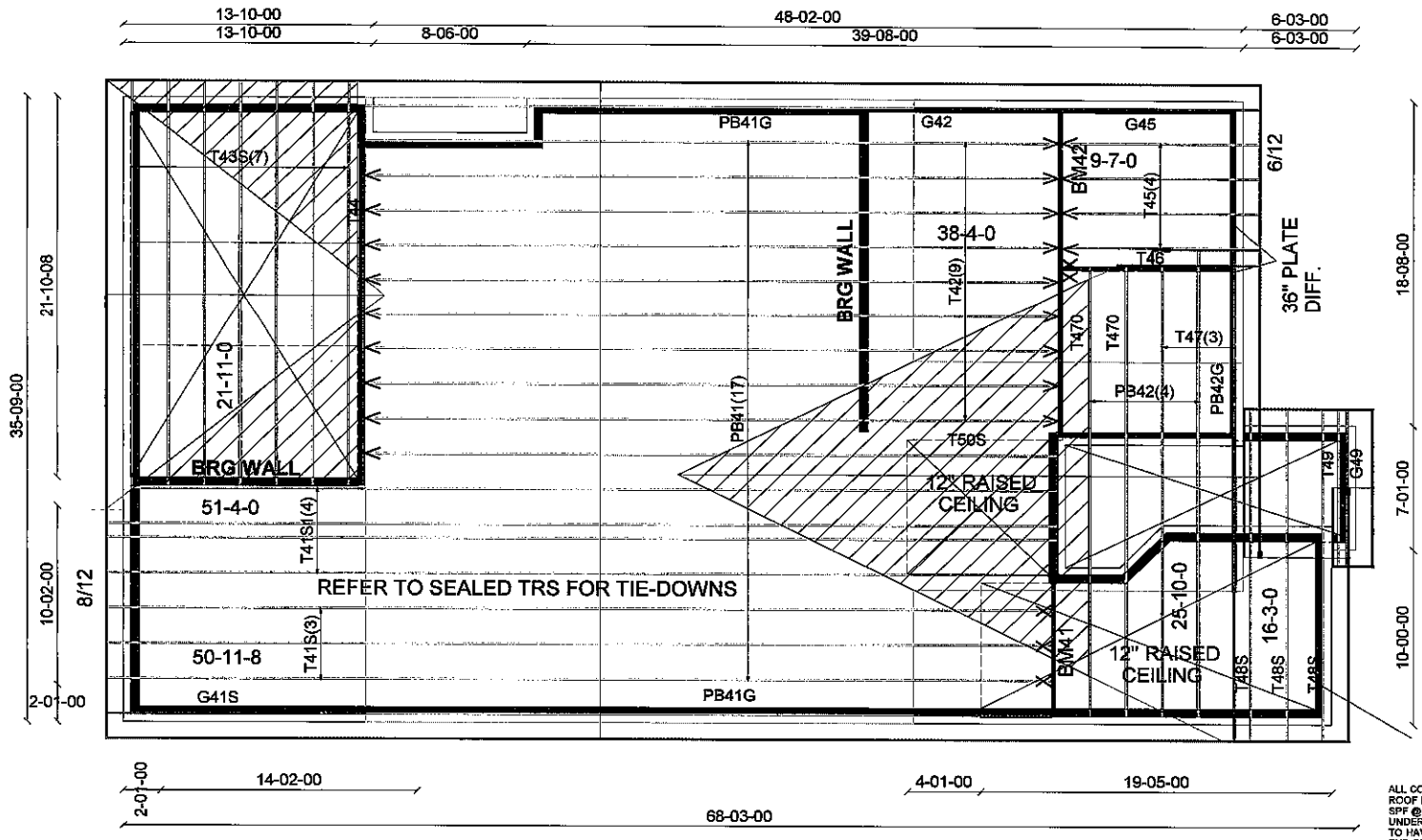


ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26 - (X)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM41 = 2PLY - 1 3/4" X 9 1/2"LVL2.0E
BM42 = STEEL BEAM A.P.P.

T.O.P@ 10-01-00
RSD HEEL-0" DROP

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

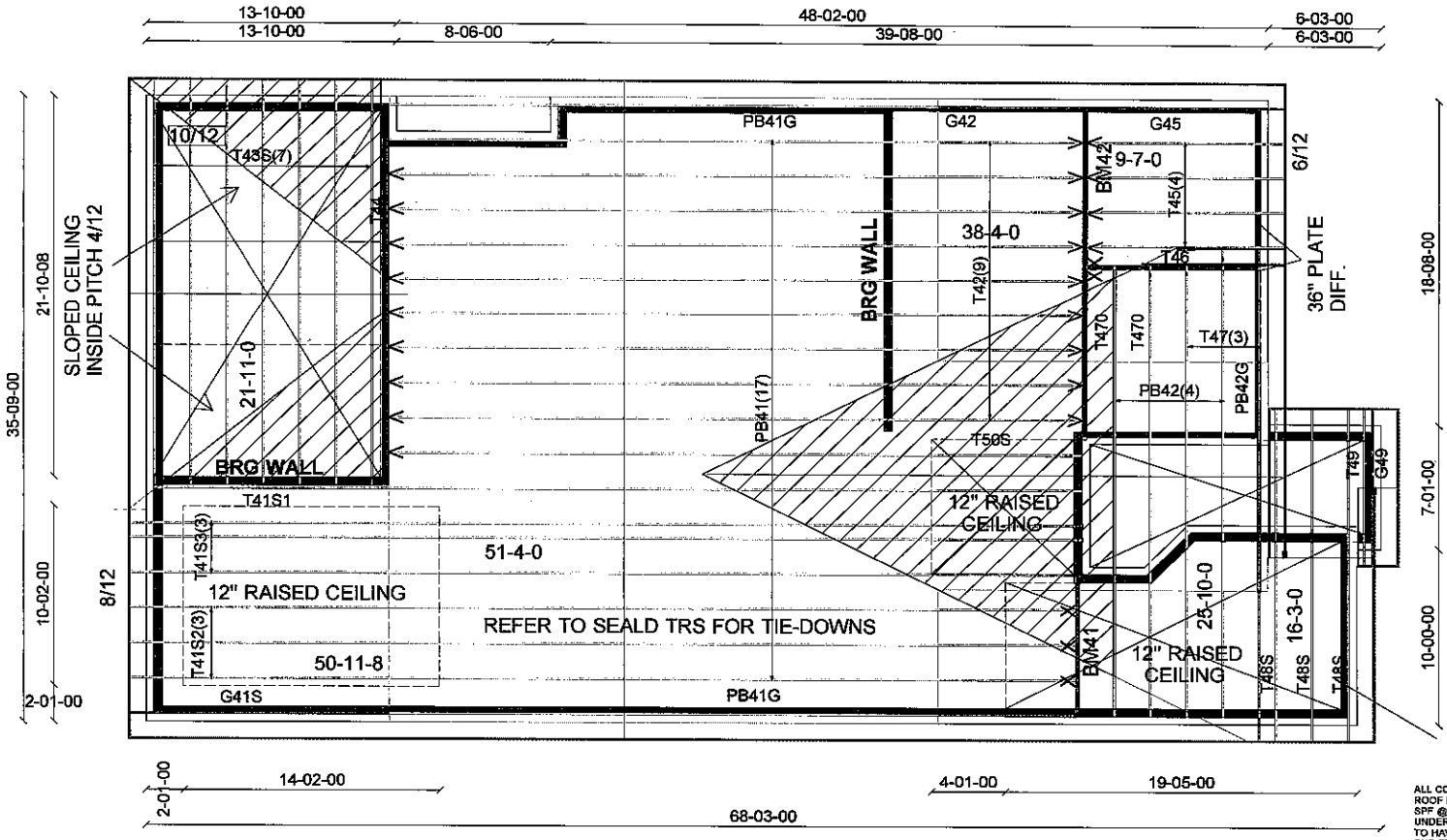
M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-1B / B STD OR REAR UPGD**
	Layout ID: **287665**	Project: **GREEN VALLEY ESTATES**	
	Plan Log: **94071**	Date: **12-Mar-18** Designer: AC	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

Site Copy



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26 - (X)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM41 = 2PLY - 1 3/4" X 9 1/2"LVL2.0E
BM42 = STEEL BEAM A.P.P.

T.O.P@ 10-01-00
RSD HEEL-0" DROP

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,500

TAMARACK LUMBER INC.
AEPA LUMBER GROUP

Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-1B / B OPT COFF**
Layout ID: **287666**	Project: **GREEN VALLEY ESTATES**	
Plan Log: **94071**	Date: **12-Mar-18** Designer: AC	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

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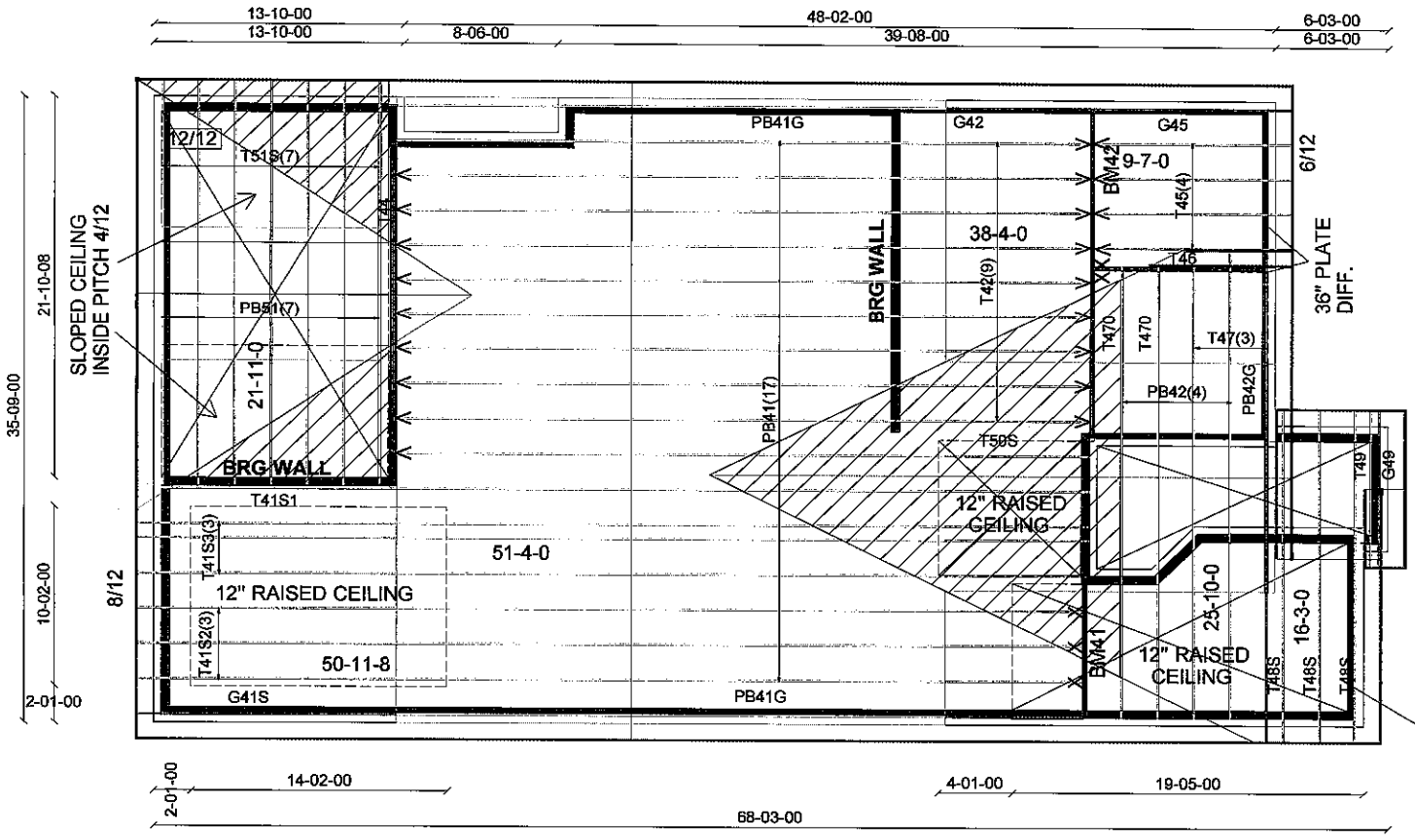


ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26 - (X)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

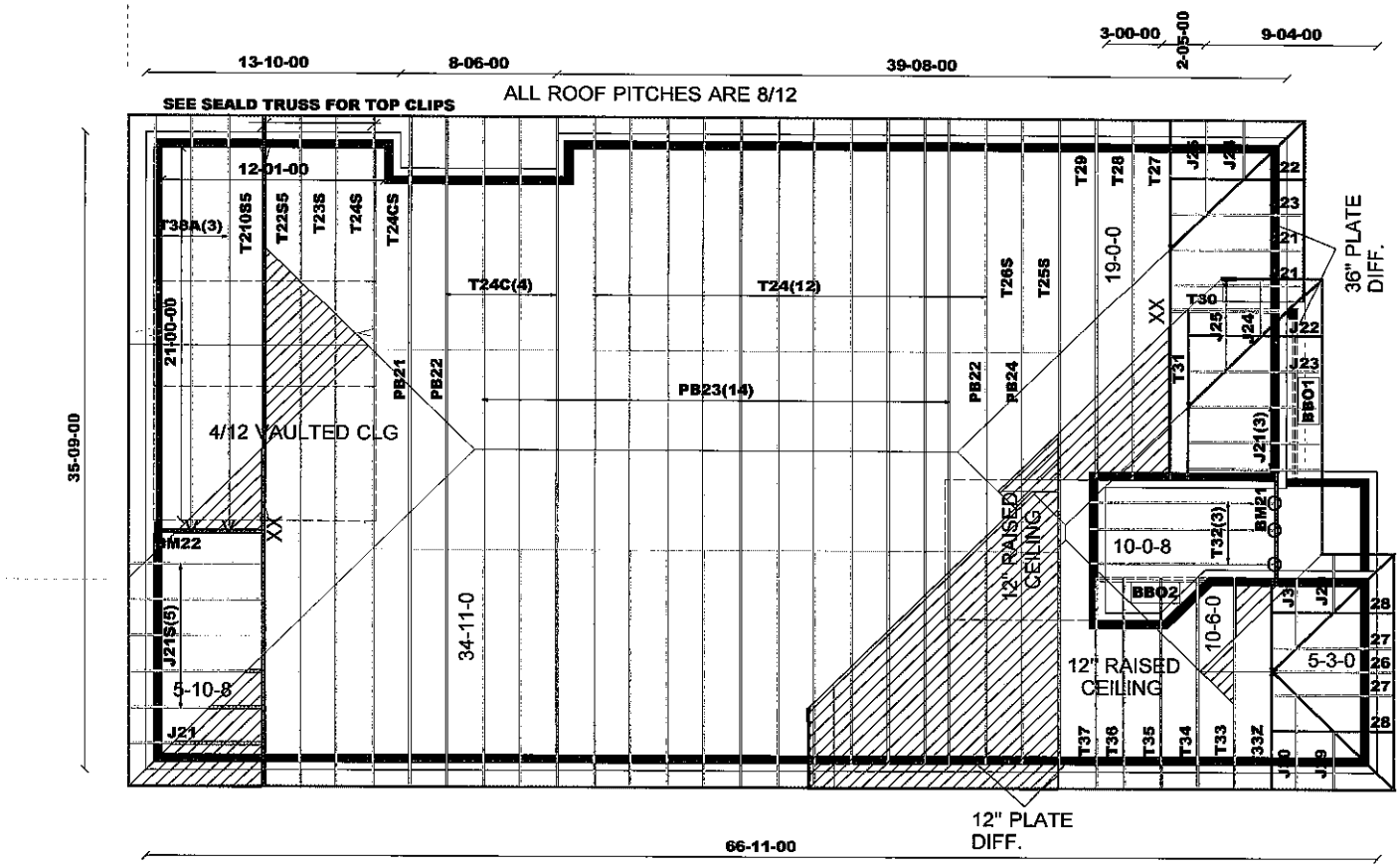
BM41 = 2PLY - 1 3/4" X 9 1/2"LVL2.0E
BM42 = STEEL BEAM A.P.P.

T.O.P@ 10-01-00
RSD HEEL-0" DROP

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-1B / B OPT COFF W/ REAR UPGRADE**
	Layout ID: **289210 D**	Project: **GREEN VALLEY ESTATES**	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: **94071**	Date: **12-Mar-18** Designer: AC	Mitek ver 7.5.0



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM21 = BM22 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B / C_STD OPT COFF
	Layout ID: 295556	Project: GREEN VALLEY EAST	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: 93746	Date: 13-Mar-18 Designer: AC	Mitek ver 7.5.0

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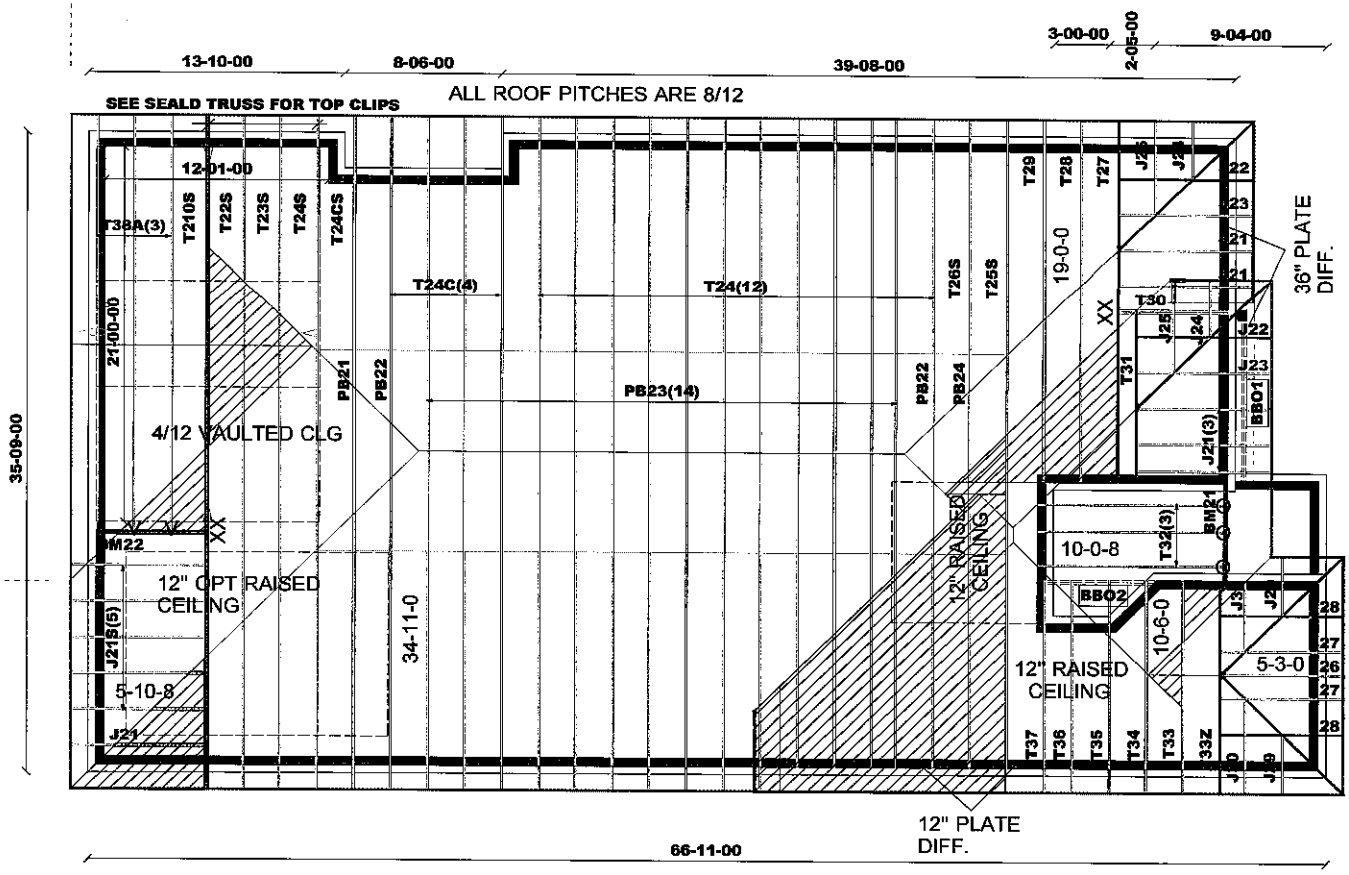

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM21 = BM22 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,500

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**		Model / Elevation: **S42-1B / C OPT COFF**
Layout ID: **287646**	Project: **GREEN VALLEY EAST**		
Plan Log: **94071**	Date: 13-Mar-18	Designer: AC	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287664 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** A OPT COFF OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	T1 PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1935.20 / 1210.00		
	5	T1C PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	967.60 / 605.00		
	4	T2S PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	786.84 / 492.00		
	7	T3S ROOF	10.00 / 4.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	772.38 / 501.69		
	1	T4 HALF HIP	8.00 / 0.00	12-07-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 06-06-07	61.25 / 38.00		
	1	T5 HALF HIP	8.00 / 0.00	12-07-08	08-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 08-02-07	68.32 / 43.17		
	2	T6 HALF HIP	8.00 / 0.00	12-07-08	09-09-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 09-09-14	121.14 / 74.66		
	3	T7 ROOF	10.00 / 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	258.51 / 163.50		
	1	G7 ROOF	10.00 / 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	98.26 / 63.50		
	1	T8 ROOF	10.00 / 0.00	19-00-00	10-06-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 10-06-14	109.58 / 67.67		
	1	T9 ROOF	10.00 / 0.00	19-00-00	11-10-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 11-10-14	120.74 / 75.00		
	1	T10 COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	47.23 / 31.00		
	1	T10Z COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	47.23 / 31.00		
	1	T11 HALF HIP	10.00 / 0.00	10-06-00	07-01-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 07-01-09	56.55 / 35.83		
	1	T12 HALF HIP	10.00 / 0.00	10-06-00	08-05-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-05-09	61.95 / 39.17		
	1	T13 HALF HIP	10.00 / 0.00	10-06-00	09-09-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 09-09-09	68.73 / 43.83		
	1	T14 MONOPITCH	10.00 / 0.00	10-06-00	10-04-11	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 10-04-11	56.66 / 36.00		
	3	T15 MONOPITCH	8.00 / 0.00	10-03-08	11-01-03	2 X 4	2 X 4	00-03-08 / 00-00-00	04-02-14 / 11-01-03	181.20 / 114.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

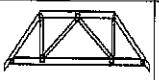
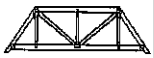
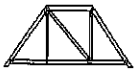
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287664 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B ELEVATION: A OPT COFF OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T16 HALF HIP	10.00 / 0.00	13-05-08	09-09-14	2 X 4	2 X 8	01-03-08 / 00-00-00	01-07-11 / 09-09-14	92.27 / 56.00		
	1	P01 PIGGYBACK	10.00 / 0.00	11-09-14	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	38.59 / 25.17		
	1	P02 PIGGYBACK	10.00 / 0.00	12-02-01	02-08-00	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-09 / 00-04-09	41.07 / 27.00		
	1	P03 PIGGYBACK	10.00 / 0.00	11-09-14	03-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	40.69 / 26.50		
	11	P04 PIGGYBACK	10.00 / 0.00	11-09-14	05-03-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	403.15 / 247.50		
	1	P05 PIGGYBACK	10.00 / 0.00	13-00-04	02-04-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	44.42 / 28.67		
	1	P06 PIGGYBACK	10.00 / 0.00	13-00-04	03-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.93 / 28.33		
	1	P07 PIGGYBACK	10.00 / 0.00	13-00-04	05-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.44 / 30.17		
	1	P08 PIGGYBACK	10.00 / 0.00	13-00-04	05-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.69 / 29.67		
	6	J01 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	151.38 / 96.00		
	6	J01S JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-06-07	157.02 / 111.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **4371.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6925.02** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287663 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** A STD OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	10	T1 PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1935.20 / 1210.00		
	4	T1C PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	774.08 / 484.00		
	1	T1CS PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	198.26 / 127.16		
	4	T2S PIGGYBACK	10.00 / 0.00	34-11-00	10-08-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	786.84 / 492.00		
	7	T3S ROOF	10.00 / 4.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	772.38 / 501.69		
	1	T4S HALF HIP	8.00 / 0.00	12-07-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 05-06-07	65.13 / 41.50		
	1	T5S HALF HIP	8.00 / 0.00	12-07-08	08-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 07-02-07	68.61 / 44.17		
	1	T6 HALF HIP	8.00 / 0.00	12-07-08	09-09-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 09-09-14	60.57 / 38.00		
	1	T6S HALF HIP	8.00 / 0.00	12-07-08	09-09-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 08-09-14	62.46 / 40.00		
	3	T7 ROOF	10.00 / 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	258.51 / 163.50		
	1	T8 ROOF	10.00 / 0.00	19-00-00	10-06-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 10-06-14	109.58 / 67.67		
	1	T9 ROOF	10.00 / 0.00	19-00-00	11-10-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 11-10-14	120.74 / 75.00		
	1	T10 COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	47.23 / 31.00		
	1	T10Z COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	47.23 / 31.00		
	1	T11 HALF HIP	10.00 / 0.00	10-06-00	07-01-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 07-01-09	56.55 / 35.83		
	1	T12 HALF HIP	10.00 / 0.00	10-06-00	08-05-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-05-09	61.95 / 39.17		
	1	T13 HALF HIP	10.00 / 0.00	10-06-00	09-09-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 09-09-09	68.73 / 43.83		
	1	T14 MONOPITCH	10.00 / 0.00	10-06-00	10-04-11	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 10-04-11	56.66 / 36.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

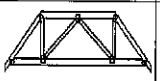
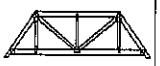
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287663 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** A STD OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T15 MONOPITCH	8.00 / 0.00	10-03-08	11-01-03	2 X 4	2 X 4	00-03-08 / 00-00-00	04-02-14 / 11-01-03	181.20 / 114.00		
	1	T16S HALF HIP	10.00 / 0.00	13-05-08	09-09-14	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 09-09-14	111.15 / 74.00		
	1	G7 ROOF	10.00 / 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	98.26 / 63.50		
	1	P01 PIGGYBACK	10.00 / 0.00	11-09-14	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	38.59 / 25.17		
	1	P02 PIGGYBACK	10.00 / 0.00	12-02-01	02-08-00	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-09 / 00-04-09	41.07 / 27.00		
	1	P03 PIGGYBACK	10.00 / 0.00	11-09-14	03-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	40.69 / 26.50		
	11	P04 PIGGYBACK	10.00 / 0.00	11-09-14	05-03-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	403.15 / 247.50		
	1	P05 PIGGYBACK	10.00 / 0.00	13-00-04	02-04-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	44.42 / 28.67		
	1	P06 PIGGYBACK	10.00 / 0.00	13-00-04	10-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.93 / 28.33		
	1	P07 PIGGYBACK	10.00 / 0.00	13-00-04	05-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.44 / 30.17		
	1	P08 PIGGYBACK	10.00 / 0.00	13-00-04	05-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.69 / 29.67		
	6	J01S JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-06-07	157.02 / 111.00		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **4307.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6803.32** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

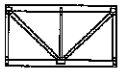
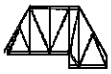
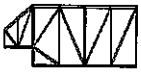
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287665 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** B STD OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T41S ROOF	8.00 / 0.00	50-11-08	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 05-03-04	1054.20 / 631.02		
	4	T41S1 PIGGYBACK	8.00 / 0.00	51-04-00	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 03-01-00	1392.24 / 844.00		
	9	T42 PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 10-00-14	2054.16 / 1260.00		
	7	T43S ROOF	10.00 / 4.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	740.81 / 485.31		
	1 / 3 Ply	T44 FLAT GIRDER	0.00 / 0.00	19-11-00	09-09-14	2 X 6	2 X 6	00-00-00 / 00-00-00	00-00-00 / 09-09-14	537.99 / 312.00		
	4	T45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	169.00 / 108.00		
	1 / 2 Ply	T46 FLAT GIRDER	0.00 / 0.00	09-07-00	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	75.02 / 49.66		
	3	T47 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-10-08 / 00-00-00	01-05-08 / 01-10-08	409.17 / 262.50		
	3	T48S ROOF	12.00 / 0.00	16-03-00	11-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	280.53 / 177.51		
	1	T49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	33.91 / 23.17		
	1	T50S PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 6	2 X 4	00-00-00 / 00-00-00	03-01-00 / 10-00-14	245.46 / 150.17		
	2	T470 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	01-05-08 / 01-10-08	255.76 / 163.66		
	17	P25 PIGGYBACK	6.00 / 0.00	28-08-00	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-07	1787.72 / 1102.11		
	1	G41S GABLE	8.00 / 0.00	50-11-08	10-00-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 05-03-04	320.94 / 202.84		
	1	G42 GABLE	6.00 / 0.00	38-09-08	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 00-03-08	273.13 / 171.33		
	1	G45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	45.70 / 30.00		
	1	G49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	35.50 / 24.17		
	17	PB41 PIGGYBACK	6.00 / 0.00	28-08-00	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-07	1788.91 / 1102.11		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287665 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** B STD OR REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB41G GABLE	6.00 / 0.00	28-08-14	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-15 / 00-04-03	234.52 / 145.00		
	4	PB42 PIGGYBACK	12.00 / 0.00	11-10-14	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	182.36 / 113.32		
	1	PB42G GABLE	12.00 / 0.00	11-11-06	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-15 / 00-04-15	48.33 / 30.33		

TOTAL # TRUSS= **87.00** TOTAL BFT OF ALL TRUSSES= **7388.21** BFT. TOTAL WEIGHT OF ALL TRUSSES= **11965.36** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
11	Hangers	H2.5A	
3	Hangers	HGUS26	
1	Hangers	HGUS26-2	
22	Hangers	LJS26DS	

TOTAL # ITEMS= **37.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

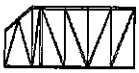
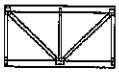
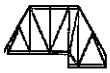
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287666 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** B OPT COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T41S1 PIGGYBACK	8.00 / 0.00	51-04-00	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 03-01-00	348.06 / 211.00		
	3	T41S2 PIGGYBACK	8.00 / 0.00	50-11-08	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 05-03-04	1134.84 / 699.00		
	3	T41S3 PIGGYBACK	8.00 / 0.00	51-04-00	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 05-01-00	1114.68 / 693.00		
	9	T42 PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 10-00-14	2054.16 / 1260.00		
	7	T43S ROOF	10.00 / 4.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	740.81 / 485.31		
	1 / 3 Ply	T44 FLAT GIRDER	0.00 / 0.00	19-11-00	09-09-14	2 X 4	2 X 6	00-00-00 / 00-00-00	00-00-00 / 09-09-14	504.63 / 308.01		
	4	T45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	169.00 / 108.00		
	1 / 2 Ply	T46 FLAT GIRDER	0.00 / 0.00	09-07-00	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	75.02 / 49.66		
	3	T47 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-10-08 / 00-00-00	01-05-08 / 01-10-08	409.17 / 262.50		
	3	T48S ROOF	12.00 / 0.00	16-03-00	11-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	280.53 / 177.51		
	1	T49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	33.91 / 23.17		
	1	T50S PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 6	2 X 4	00-00-00 / 00-00-00	03-01-00 / 10-00-14	245.39 / 150.17		
	2	T470 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	01-05-08 / 01-10-08	255.76 / 163.66		
	17	P25 PIGGYBACK	6.00 / 0.00	28-08-00	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-07	1787.72 / 1102.11		
	1	G41S GABLE	8.00 / 0.00	50-11-08	10-00-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 05-03-04	320.94 / 202.84		
	1	G42 GABLE	6.00 / 0.00	38-09-08	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 00-03-08	273.13 / 171.33		
	1	G45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	41.28 / 27.00		
	1	G49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	35.50 / 24.17		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287666 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** B OPT COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	17	PB41 PIGGYBACK	6.00 / 0.00	28-08-00	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-07	1788.91 / 1102.11		
	2	PB41G GABLE	6.00 / 0.00	28-08-14	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-15 / 00-04-03	234.52 / 145.00		
	4	PB42 PIGGYBACK	12.00 / 0.00	11-10-14	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	182.36 / 113.32		
	1	PB42G GABLE	12.00 / 0.00	11-11-06	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-15 / 00-04-15	48.33 / 30.33		

TOTAL # TRUSS= **87.00** TOTAL BFT OF ALL TRUSSES= **7509.20** BFT. TOTAL WEIGHT OF ALL TRUSSES= **12078.65** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	H2.5A	
3	Hangers	HGUS26	
1	Hangers	HGUS26-2	
22	Hangers	LJS26DS	

TOTAL # ITEMS= **35.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

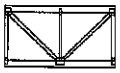
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289210 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON SUB-BUILDER:
MODEL: S42-1B ELEVATION: B OPT COFF W/REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T41S1 PIGGYBACK	8.00 / 0.00	51-04-00	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 03-01-00	348.06 / 211.00		
	3	T41S2 PIGGYBACK	8.00 / 0.00	50-11-08	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 05-03-04	1134.84 / 699.00		
	3	T41S3 PIGGYBACK	8.00 / 0.00	51-04-00	10-00-14	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-14 / 05-01-00	1114.68 / 693.00		
	9	T42 PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 10-00-14	2054.16 / 1260.00		
	1 / 3 Ply	T44 FLAT GIRDER	0.00 / 0.00	19-11-00	09-09-14	2 X 4	2 X 6	00-00-00 / 00-00-00	00-00-00 / 09-09-14	504.63 / 308.01		
	4	T45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	169.00 / 108.00		
	1 / 2 Ply	T46 FLAT GIRDER	0.00 / 0.00	09-07-00	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	75.02 / 49.66		
	3	T47 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-10-08 / 00-00-00	01-05-08 / 01-10-08	409.17 / 262.50		
	3	T48S ROOF	12.00 / 0.00	16-03-00	11-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	280.53 / 177.51		
	1	T49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	33.91 / 23.17		
	1	T50S PIGGYBACK	6.00 / 0.00	38-04-00	10-00-14	2 X 6	2 X 4	00-00-00 / 00-00-00	03-01-00 / 10-00-14	245.27 / 150.17		
	7	T51S PIGGYBACK	12.00 / 4.00	21-11-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	743.61 / 471.31		
	2	T470 PIGGYBACK	12.00 / 0.00	25-10-00	09-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	01-05-08 / 01-10-08	255.76 / 163.66		
	1	G41S GABLE	8.00 / 0.00	50-11-08	10-00-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 05-03-04	320.94 / 202.84		
	1	G42 GABLE	6.00 / 0.00	38-09-08	10-00-14	2 X 4	2 X 4	00-00-00 / 00-00-00	06-01-00 / 00-03-08	273.13 / 171.33		
	1	G45 ROOF	6.00 / 0.00	09-07-00	05-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-11-08	41.28 / 27.00		
	1	G49 ROOF	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	35.50 / 24.17		
	17	PB41 PIGGYBACK	6.00 / 0.00	28-08-00	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 00-04-07	1788.91 / 1102.11		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289210 D LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON SUB-BUILDER:
MODEL: S42-1B ELEVATION: B OPT COFF W/REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB41G GABLE	6.00 / 0.00	28-08-14	08-06-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-15 / 00-04-03	234.52 / 145.00		
	4	PB42 PIGGYBACK	12.00 / 0.00	11-10-14	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	182.36 / 113.32		
	1	PB42G GABLE	12.00 / 0.00	11-11-06	06-04-10	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-15 / 00-04-15	48.33 / 30.33		
	7	PB51 PIGGYBACK	12.00 / 0.00	06-06-10	03-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	155.96 / 102.69		

TOTAL # TRUSS= **77.00** TOTAL BFT OF ALL TRUSSES= **6495.78** BFT. TOTAL WEIGHT OF ALL TRUSSES= **10449.57** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
9	Hangers	H2.5A	
3	Hangers	HGUS26	
1	Hangers	HGUS26-2	
22	Hangers	LJS26DS	

TOTAL # ITEMS= **35.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

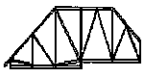
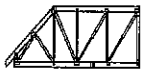
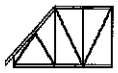
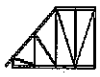
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287647 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B-LOFT ELEVATION: C OPT COFF W/ REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T21S HIP GIRDER	8.00 / 0.00	34-11-00	05-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	313.52 / 199.66		
	1	T22S HIP	8.00 / 0.00	34-11-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	164.15 / 106.50		
	1	T23S HIP	8.00 / 0.00	34-11-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	171.69 / 109.50		
	12	T24 PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	2016.96 / 1286.04		
	4	T24C PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	672.32 / 428.68		
	1	T24CS PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	174.07 / 111.83		
	1	T24S PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	174.07 / 111.83		
	1	T25S HIP	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T26S PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T27 HALF HIP	8.00 / 0.00	19-00-00	05-03-13	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 05-03-13	111.12 / 68.83		
	1	T28 HALF HIP	8.00 / 0.00	19-00-00	06-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-07-13	86.39 / 55.67		
	1	T29 HALF HIP	8.00 / 0.00	19-00-00	07-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-11-13	95.47 / 60.83		
	1 / 2 Ply	T30 MONOPITCH	0.00 / 0.00	05-10-08	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	53.18 / 35.00		
	1	T31 HALF HIP	8.00 / 0.00	09-08-00	05-03-13	2 X 4	2 X 4	00-00-00 / 00-00-00	01-08-02 / 05-03-13	43.41 / 28.33		
	3	T32 MONOPITCH	8.00 / 0.00	10-00-08	10-11-02	2 X 4	2 X 4	00-00-00 / 00-00-00	04-02-13 / 10-11-02	194.16 / 123.00		
	1	T33 COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	51.44 / 32.83		
	1	T33Z COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	51.44 / 32.83		
	1	T34 HALF HIP	8.00 / 0.00	10-06-00	06-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-11-08	54.19 / 34.50		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

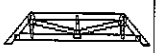
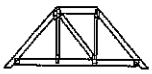
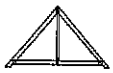
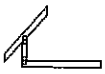
Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287647 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B-LOFT **ELEVATION:** C OPT COFF W/ REAR UPGD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T35 HALF HIP	8.00 / 0.00	10-06-00	08-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 08-03-08	62.99 / 40.17		
	1	T36 HALF HIP	8.00 / 0.00	10-06-00	09-07-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 09-07-08	71.84 / 46.50		
	1	T37 GABLE	8.00 / 0.00	10-06-00	10-04-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 10-04-13	59.13 / 36.50		
	1	PB21 PIGGYBACK	8.00 / 0.00	09-08-02	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	31.63 / 21.50		
	2	PB22 PIGGYBACK	8.00 / 0.00	09-08-02	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	63.10 / 40.00		
	14	PB23 PIGGYBACK	8.00 / 0.00	09-08-02	03-07-03	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	402.36 / 245.00		
	1	PB24 PIGGYBACK	8.00 / 0.00	14-02-02	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	45.53 / 29.50		
	15	J21 JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	267.45 / 160.05		
	5	J21S JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-03-13	121.50 / 82.50		
	3	J22 JACK-OPEN	8.00 / 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 / -04-01-01	01-04-13 / 00-03-08	36.69 / 21.99		
	3	J23 JACK-OPEN	8.00 / 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / -02-01-01	01-04-13 / 00-03-08	44.91 / 27.99		
	3	J24 JACK-OPEN	8.00 / 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 / -00-01-01	01-04-13 / 00-03-08	23.01 / 14.01		
	3	J25 JACK-OPEN	8.00 / 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	01-04-13 / 00-05-03	31.23 / 20.01		
	1	J26 JACK-OPEN	8.00 / 0.00	05-03-00	06-10-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-10-13	18.54 / 12.00		
	2	J27 JACK-OPEN	8.00 / 0.00	05-03-00	05-11-02	2 X 4	2 X 4	01-03-08 / -01-05-09	03-04-13 / 00-03-08	33.06 / 21.34		
	2	J28 JACK-OPEN	8.00 / 0.00	05-03-00	04-07-02	2 X 4	2 X 4	01-03-08 / -03-05-09	03-04-13 / 00-03-08	27.60 / 17.34		
	2	J29 JACK-OPEN	8.00 / 0.00	01-10-08	04-07-02	2 X 4	2 X 4	01-03-08 / -00-01-01	03-04-13 / 00-03-08	19.90 / 12.00		
	2	J30 JACK-OPEN	8.00 / 0.00	01-10-08	05-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	03-04-13 / 00-05-03	25.38 / 16.00		

TOTAL # TRUSS= **98.00** TOTAL BFT OF ALL TRUSSES= **3904.94** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6158.87** LBS.

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TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/12/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287647 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B-LOFT **ELEVATION:** C OPT COFF W/ REAR UPGD

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

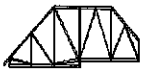
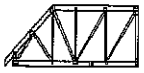
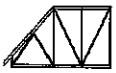
Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295556 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B ELEVATION: C_STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T22S5 HIP	8.00 / 4.00	34-11-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	147.33 / 95.83		
	1	T23S HIP	8.00 / 4.00	34-11-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	162.40 / 104.83		
	12	T24 PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	2016.96 / 1278.00		
	4	T24C PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	666.56 / 422.00		
	1	T24CS PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	166.63 / 106.17		
	1	T24S HIP	8.00 / 4.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	166.59 / 107.17		
	1	T25S HIP	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T26S PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T27 HALF HIP	8.00 / 0.00	19-00-00	05-03-13	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 05-03-13	111.12 / 68.83		
	1	T28 HALF HIP	8.00 / 0.00	19-00-00	06-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-07-13	86.39 / 55.67		
	1	T29 HALF HIP	8.00 / 0.00	19-00-00	07-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-11-13	95.47 / 60.83		
	1 / 2 Ply	T30 MONOPITCH	0.00 / 0.00	05-10-08	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	53.18 / 35.00		
	1	T31 HALF HIP	8.00 / 0.00	09-08-00	05-03-13	2 X 4	2 X 4	00-00-00 / 00-00-00	01-08-02 / 05-03-13	43.41 / 28.33		
	3	T32 MONOPITCH	8.00 / 0.00	10-00-08	10-11-02	2 X 4	2 X 4	00-00-00 / 00-00-00	04-02-13 / 10-11-02	194.16 / 123.00		
	1	T33 COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 4	00-00-00 / 01-03-08	03-04-13 / 03-04-13	49.67 / 31.50		
	1	T33Z COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 6	00-00-00 / 01-03-08	03-04-13 / 03-04-13	55.86 / 36.17		
	1	T34 HALF HIP	8.00 / 0.00	10-06-00	06-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-11-08	54.19 / 34.50		
	1	T35 HALF HIP	8.00 / 0.00	10-06-00	08-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 08-03-08	62.99 / 40.17		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

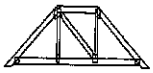
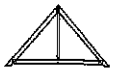
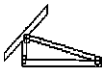
Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295556 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B ELEVATION: C_STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T36 HALF HIP	8.00 / 0.00	10-06-00	09-07-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 09-07-08	64.83 / 41.50		
	1	T37 MONOPITCH	8.00 / 0.00	10-06-00	10-04-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 10-04-13	59.13 / 36.50		
	3	T38A ROOF	8.00 / 4.00	21-11-00	09-02-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-04-02	317.73 / 207.99		
	1 / 2 Ply	T210S5 HIP GIRDER	8.00 / 4.00	34-11-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	346.58 / 220.00		
	1	PB21 PIGGYBACK	8.00 / 0.00	09-08-02	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	31.63 / 21.50		
	2	PB22 PIGGYBACK	8.00 / 0.00	09-08-02	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	63.10 / 40.00		
	14	PB23 PIGGYBACK	8.00 / 0.00	09-08-02	03-07-03	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	402.36 / 245.00		
	1	PB24 PIGGYBACK	8.00 / 0.00	14-02-02	08-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	45.53 / 29.50		
	6	J21 JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	106.98 / 64.02		
	5	J21S JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	97.10 / 58.35		
	2	J22 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 04-01-01	01-04-13 / 00-03-08	33.68 / 20.66		
	2	J23 JACK-OPEN	8.00 / 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 / 02-01-01	01-04-13 / 00-03-08	39.16 / 24.66		
	2	J24 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	01-04-13 / 00-03-08	18.34 / 11.34		
	2	J25 JACK-OPEN	8.00 / 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	00-04-03 / 00-05-03	20.82 / 13.34		
	1	J26 JACK-OPEN	8.00 / 0.00	05-03-00	06-10-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-10-13	18.54 / 12.00		
	2	J27 JACK-OPEN	8.00 / 0.00	03-09-07	05-11-02	2 X 4	2 X 4	01-03-08 / 01-05-09	03-04-13 / 00-03-08	33.06 / 21.34		
	2	J28 JACK-OPEN	8.00 / 0.00	01-09-07	04-07-02	2 X 4	2 X 4	01-03-08 / 03-05-09	03-04-13 / 00-03-08	27.60 / 17.34		
	2	J29 JACK-OPEN	8.00 / 0.00	01-09-07	04-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	03-04-13 / 00-03-08	19.90 / 12.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295556 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** C_STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J30 / JACK-OPEN	8.00 / 0.00	01-10-08	05-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	03-04-13 / 00-05-03	25.38 / 16.00		

TOTAL # TRUSS= **88.00** TOTAL BFT OF ALL TRUSSES= **3955.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6249.80** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
3	Hangers	LUS24	
8	Top Clips	TC26	

TOTAL # ITEMS= **15.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

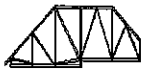
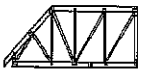
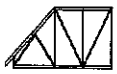
Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287646 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T22S HIP	8.00 / 4.00	34-11-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	152.00 / 100.17		
	1	T23S HIP	8.00 / 4.00	34-11-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	162.61 / 104.50		
	12	T24 PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	2016.96 / 1286.04		
	4	T24C PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	672.32 / 428.68		
	1	T24CS PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	174.07 / 111.83		
	1	T24S HIP	8.00 / 4.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	171.64 / 112.33		
	1	T25S HIP	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T26S PIGGYBACK	8.00 / 0.00	34-11-00	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-04-13	172.72 / 107.34		
	1	T27 HALF HIP	8.00 / 0.00	19-00-00	05-03-13	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 05-03-13	111.12 / 68.83		
	1	T28 HALF HIP	8.00 / 0.00	19-00-00	06-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-07-13	86.39 / 55.67		
	1	T29 HALF HIP	8.00 / 0.00	19-00-00	07-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-11-13	95.47 / 60.83		
	1 / 2 Ply	T30 MONOPITCH	0.00 / 0.00	05-10-08	03-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	03-00-00 / 03-00-00	53.18 / 35.00		
	1	T31 HALF HIP	8.00 / 0.00	09-08-00	05-03-13	2 X 4	2 X 4	00-00-00 / 00-00-00	01-08-02 / 05-03-13	43.41 / 28.33		
	3	T32 MONOPITCH	8.00 / 0.00	10-00-08	10-11-02	2 X 4	2 X 4	00-00-00 / 00-00-00	04-02-13 / 10-11-02	194.16 / 123.00		
	1	T33 COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	51.44 / 32.83		
	1	T33Z COMMON	8.00 / 0.00	10-06-00	06-10-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	51.44 / 32.83		
	1	T34 HALF HIP	8.00 / 0.00	10-06-00	06-11-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-11-08	54.19 / 34.50		
	1	T35 HALF HIP	8.00 / 0.00	10-06-00	08-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 08-03-08	62.99 / 40.17		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

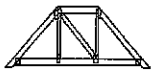
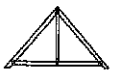
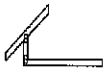
Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287646 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T36 HALF HIP	8.00 / 0.00	10-06-00	09-07-08	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 09-07-08	71.84 / 46.50		
	1	T37 GABLE	8.00 / 0.00	10-06-00	10-04-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 10-04-13	59.13 / 36.50		
	3	T38A ROOF	8.00 / 4.00	21-11-00	09-02-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-04-02	317.73 / 207.99		
	1 / 2 Ply	T210S HIP GIRDER	8.00 / 4.00	34-11-00	05-03-13	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	389.84 / 242.66		
	1	PB21 PIGGYBACK	8.00 / 0.00	09-08-02	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	31.63 / 21.50		
	2	PB22 PIGGYBACK	8.00 / 0.00	09-08-02	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	63.10 / 40.00		
	14	PB23 PIGGYBACK	8.00 / 0.00	09-08-02	03-07-03	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	402.36 / 245.00		
	1	PB24 PIGGYBACK	8.00 / 0.00	14-02-02	01-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	45.53 / 29.50		
	6	J21 JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	106.98 / 64.02		
	5	J21S JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-03-13	126.30 / 85.00		
	2	J22 JACK-OPEN	8.00 / 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 / -04-01-01	01-04-13 / 00-03-08	24.46 / 14.66		
	2	J23 JACK-OPEN	8.00 / 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / -02-01-01	01-04-13 / 00-03-08	29.94 / 18.66		
	2	J24 JACK-OPEN	8.00 / 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 / -00-01-01	01-04-13 / 00-03-08	15.34 / 9.34		
	2	J25 JACK-OPEN	8.00 / 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	01-04-13 / 00-05-03	20.82 / 13.34		
	1	J26 JACK-OPEN	8.00 / 0.00	05-03-00	06-10-13	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 06-10-13	18.54 / 12.00		
	2	J27 JACK-OPEN	8.00 / 0.00	05-03-00	05-11-02	2 X 4	2 X 4	01-03-08 / -01-05-09	03-04-13 / 00-03-08	33.06 / 21.34		
	2	J28 JACK-OPEN	8.00 / 0.00	05-03-00	04-07-02	2 X 4	2 X 4	01-03-08 / -03-05-09	03-04-13 / 00-03-08	27.60 / 17.34		
	2	J29 JACK-OPEN	8.00 / 0.00	01-10-08	04-07-02	2 X 4	2 X 4	01-03-08 / -00-01-01	03-04-13 / 00-03-08	19.90 / 12.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/13/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287646 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B **ELEVATION:** C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
1	2	J30 JACK-OPEN	8.00 / 0.00	01-10-08	05-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	03-04-13 / 00-05-03	25.38 / 16.00		

TOTAL # TRUSS= **88.00** TOTAL BFT OF ALL TRUSSES= **4023.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6328.31** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
3	Hangers	LUS24	
8	Top Clips	TC26	

TOTAL # ITEMS= **15.00**

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T1	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed Mar 7 10:51:10 2018 Page 1

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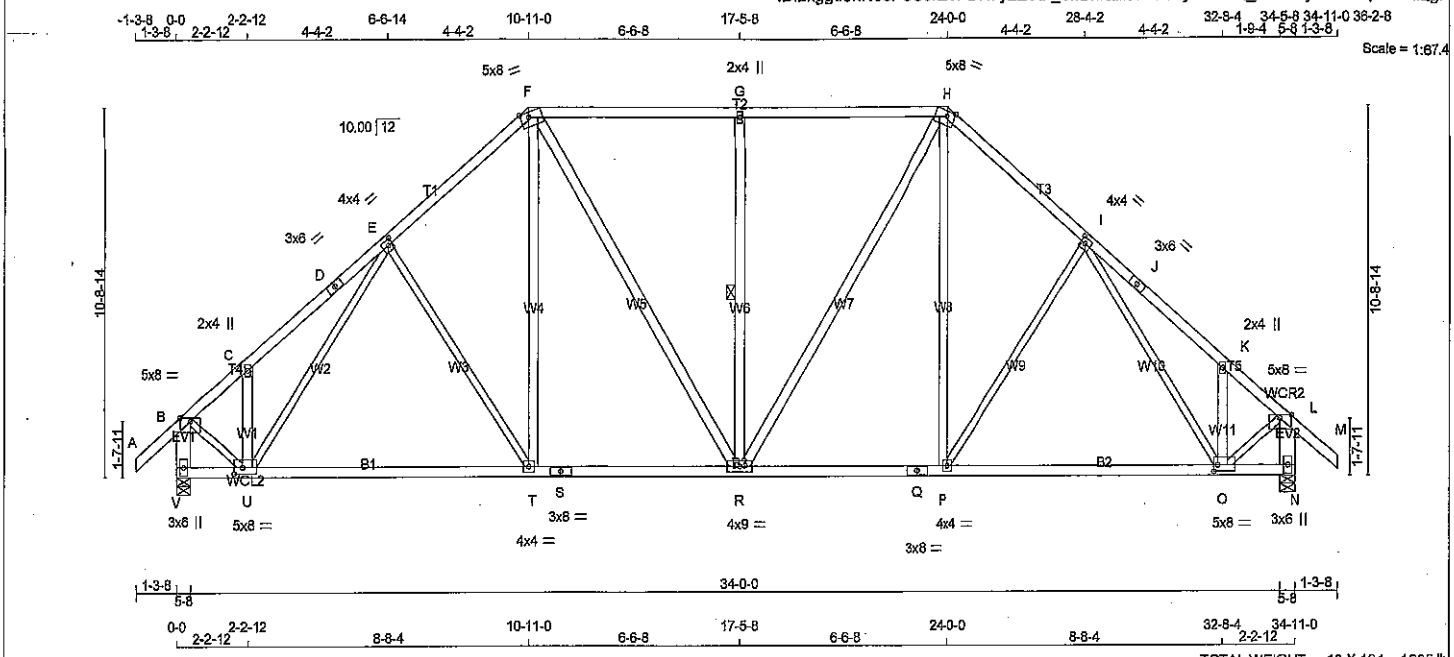

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - E	2x3	DRY	No.2	SPF
E - T	2x3	DRY	No.2	SPF
P - I	2x3	DRY	No.2	SPF
I - O	2x3	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
O - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, G, K						
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TS-t	MT20	3.0	6.0		
L	TMVW-p	MT20	5.0	8.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0	2.25	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWWW-t	MT20	5.0	8.0	2.25	3.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2515	0	2515	0	0	5-8	5-8
N	2515	0	2515	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1987	1224 / 0	367 / 0	0 / 0	0 / 0	397 / 0	0 / 0
N	1987	1224 / 0	367 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

FOR SECTION F-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-2113 / 0	-104.9	-104.9	0.15 (1)	4.55
C- D	-2112 / 0	-104.9	-104.9	0.30 (1)	4.42
D- E	-2112 / 0	-104.9	-104.9	0.30 (1)	4.42
E- F	-2292 / 0	-104.9	-104.9	0.31 (1)	4.25
F- G	-2067 / 0	-112.4	-112.4	0.70 (1)	2.00
G- H	-2067 / 0	-112.4	-112.4	0.70 (1)	2.00
H- I	-2292 / 0	-104.9	-104.9	0.31 (1)	4.25
I- J	-2112 / 0	-104.9	-104.9	0.30 (1)	4.42
J- K	-2112 / 0	-104.9	-104.9	0.30 (1)	4.42
K- L	-2113 / 0	-104.9	-104.9	0.15 (1)	4.55
L- M	0 / 47	-104.9	-104.9	0.14 (1)	10.00
V- B	-2545 / 0	0.0	0.0	0.18 (1)	6.50
N- L	-2545 / 0	0.0	0.0	0.18 (1)	6.50
V- U	0 / 0	-28.0	-28.0	0.37 (3)	10.00
U- T	0 / 1887	-28.0	-28.0	0.58 (2)	10.00
T- S	0 / 1739	-28.0	-28.0	0.57 (2)	10.00
S- R	0 / 1739	-28.0	-28.0	0.57 (2)	10.00
R- Q	0 / 1739	-28.0	-28.0	0.57 (2)	10.00
Q- P	0 / 1739	-28.0	-28.0	0.57 (2)	10.00
P- O	0 / 1887	-28.0	-28.0	0.58 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.37 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-357 / 0	0.05 (1)
U- E	-466 / 0	0.63 (1)
E- T	-260 / 0	0.39 (1)
T- F	0 / 491	0.08 (2)
F- R	0 / 618	0.10 (1)
R- G	-901 / 0	0.53 (1)
R- H	0 / 618	0.10 (1)
P- H	0 / 491	0.08 (2)
P- I	-260 / 0	0.39 (1)
I- O	-466 / 0	0.63 (1)
O- K	-357 / 0	0.05 (1)
B- U	0 / 1908	0.43 (1)
O- L	0 / 1908	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.70/1.00 (F-G:1) , BC=0.58/1.00 (T-U:2) , WB=0.63/1.00 (E-U:1) , SSI=0.36/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.62 (Q) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 13167-08
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T1C	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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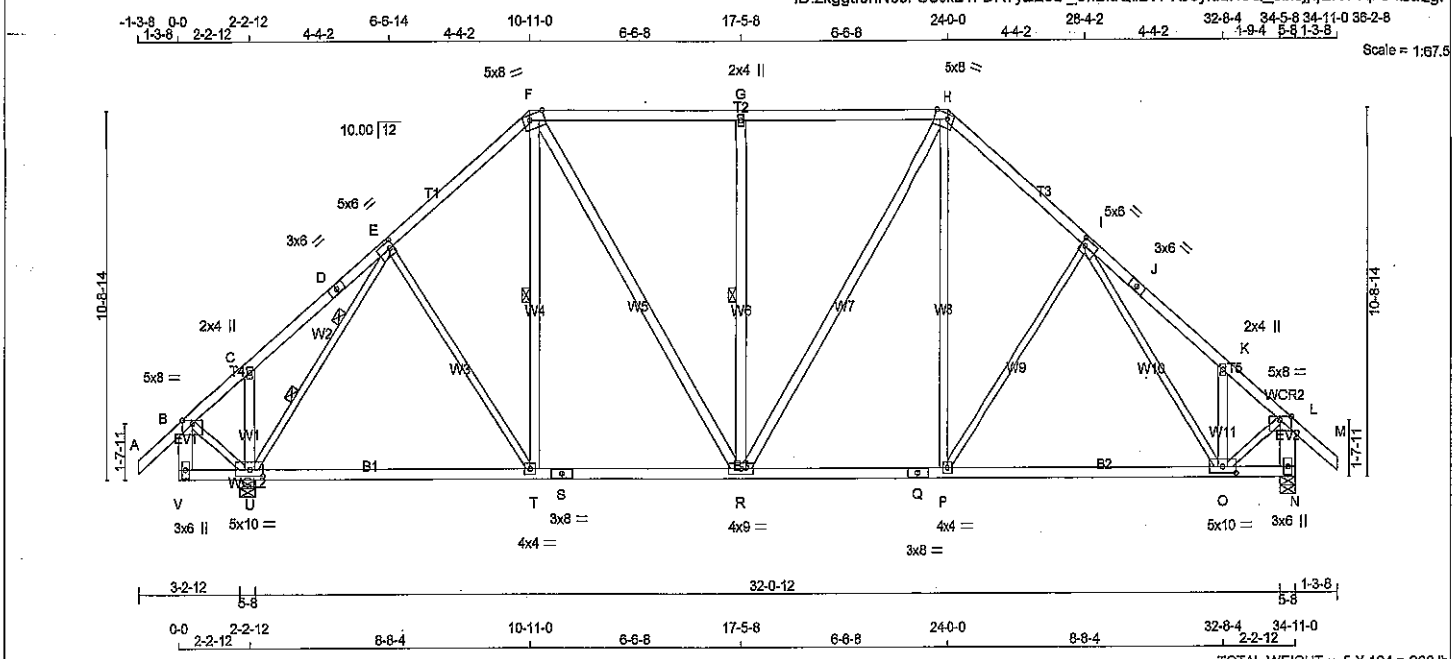

TOTAL WEIGHT = 5 X 194 = 968 lb

[M](F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
U - E	2x3	DRY	No.2	SPF
E - T	2x3	DRY	No.2	SPF
P - I	2x3	DRY	No.2	SPF
I - O	2x3	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
O - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C, G, K						
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	1.50
F	TTWW-m	MT20	5.0	8.0	Edge	5.50
H	TTWW-m	MT20	5.0	8.0	Edge	5.50
I	TMWW-t	MT20	5.0	6.0	2.50	1.50
J	TS-t	MT20	3.0	6.0		
L	TMW-p	MT20	5.0	8.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	10.0	2.25	5.00
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWWW1-t	MT20	5.0	10.0	2.25	5.00
V	BMV+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2686	0	2686	0	0	5-8	5-8
N	2343	0	2343	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2123	1307 / 0	392 / 0	0 / 0	0 / 0	424 / 0	0 / 0
N	1852	1140 / 0	342 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, N

BRACING
FOR SECTION F-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF E-U.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-54 / 166	-104.9	-104.9	0.25 (1)	6.25
C- D	-63 / 137	-104.9	-104.9	0.29 (1)	6.25
D- E	-63 / 137	-104.9	-104.9	0.29 (1)	6.25
E- F	-1781 / 0	-104.9	-104.9	0.27 (1)	4.76
F- G	-1781 / 0	-112.4	-112.4	0.68 (1)	2.00
G- H	-1781 / 0	-112.4	-112.4	0.68 (1)	2.00
H- I	-2058 / 0	-104.9	-104.9	0.30 (1)	4.47
I- J	-1955 / 0	-104.9	-104.9	0.29 (1)	4.57
J- K	-1955 / 0	-104.9	-104.9	0.29 (1)	4.57
K- L	-1956 / 0	-104.9	-104.9	0.15 (1)	4.71
L- M	0 / 47	-104.9	-104.9	0.14 (1)	10.00
V- B	-77 / 0	0.0	0.0	0.01 (2)	7.81
N- L	-2375 / 0	0.0	0.0	0.16 (1)	6.68
V- U	0 / 0	-28.0	-28.0	0.40 (2)	10.00
U- T	0 / 1189	-28.0	-28.0	0.52 (2)	10.00
T- S	0 / 1342	-28.0	-28.0	0.52 (2)	10.00
S- R	0 / 1342	-28.0	-28.0	0.52 (2)	10.00
R- Q	0 / 1559	-28.0	-28.0	0.55 (2)	10.00
Q- P	0 / 1559	-28.0	-28.0	0.55 (2)	10.00
P- O	0 / 1721	-28.0	-28.0	0.56 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.37 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-405 / 0	0.06 (1)
U- E	-2357 / 0	0.94 (1)
E- T	0 / 314	0.07 (1)
T- F	-40 / 206	0.03 (3)
F- R	0 / 826	0.13 (1)
R- G	-903 / 0	0.53 (1)
R- H	0 / 418	0.07 (1)
P- H	0 / 504	0.06 (2)
P- I	-305 / 0	0.42 (1)
I- O	-381 / 0	0.51 (1)
O- K	-357 / 0	0.05 (1)
B- U	-104 / 55	0.02 (1)
O- L	0 / 1768	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.68/1.00 (F-G:1) , BC=0.56/1.00 (O-P:2) , WB=0.94/1.00 (E-U:1) , SSI=0.36/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3816
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13166-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T1CS	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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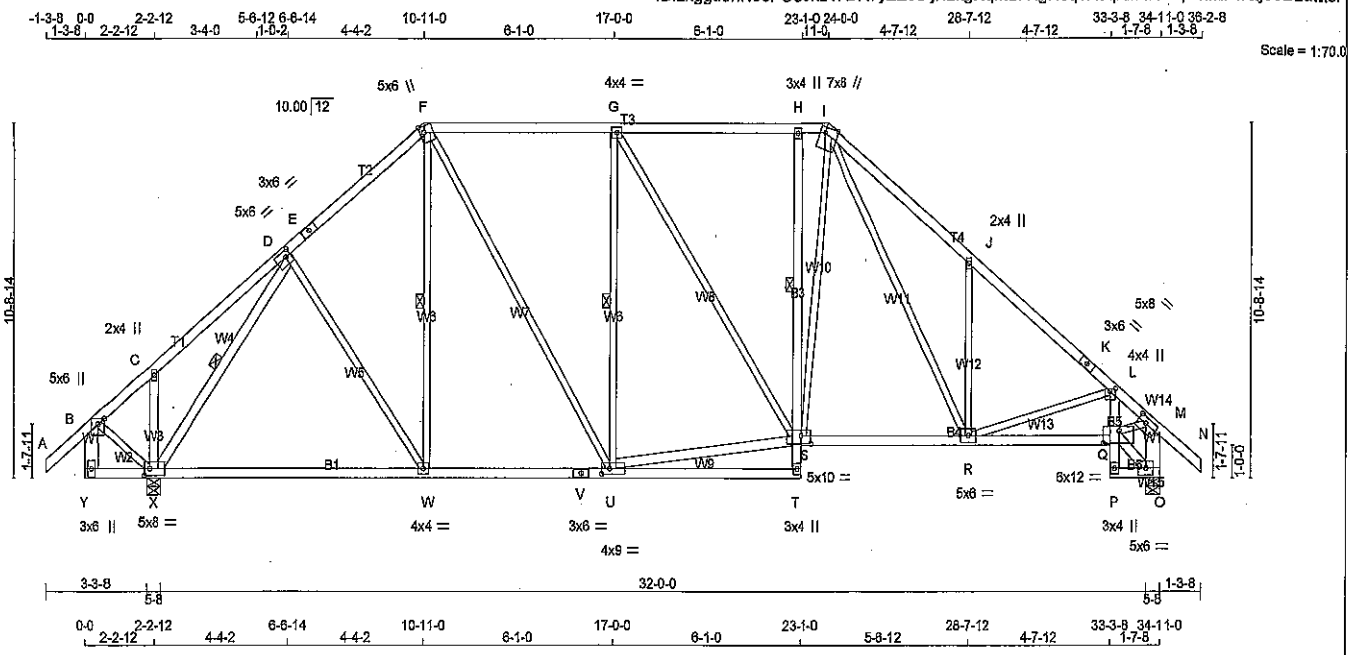

TOTAL WEIGHT = 198 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
X - C	2x4	DRY	No.2	SPF
X - D	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	5.0	6.0	2.25	2.00
E	TS-t	MT20	3.0	6.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	4.75
J	TMW+w	MT20	2.0	4.0		
K	TS-t	MT20	3.0	6.0		
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
M	TMVW-t	MT20	5.0	8.0	2.00	3.25
O	BMVW1-t	MT20	5.0	6.0		
P	BMV+p	MT20	3.0	4.0		
Q	BVMWW-t	MT20	6.0	12.0	4.50	6.25
R	BMWWW-t	MT20	5.0	6.0		
S	BVMWWW-t	MT20	5.0	10.0	3.00	4.25
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	4.0	9.0	2.00	3.00
V	BS-t	MT20	3.0	6.0		
W	BMWW-t	MT20	4.0	4.0		
X	BMWWW1-t	MT20	5.0	8.0	2.50	2.00
Y	BMV+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2631	0	2631	0	0	5-8	5-8
O	2301	0	2301	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2079	1305 / 0	392 / 0	0 / 0	0 / 0	382 / 0	0 / 0
O	1817	1142 / 0	342 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-S, F-W, G-U, D-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-54 / 172	-104.9	-104.9	0.21 (1)	6.25
C- D	-60 / 157	-104.9	-104.9	0.31 (1)	6.25
D- E	-1720 / 0	-104.9	-104.9	0.29 (1)	4.80
E- F	-1720 / 0	-104.9	-104.9	0.29 (1)	4.80
F- G	-1690 / 0	-104.9	-104.9	0.50 (1)	4.52
G- H	-1727 / 0	-104.9	-104.9	0.50 (1)	4.49
H- I	-1730 / 0	-104.9	-104.9	0.20 (1)	4.88
I- J	-2648 / 0	-104.9	-104.9	0.41 (1)	3.92
J- K	-2603 / 0	-104.9	-104.9	0.39 (1)	3.97
K- L	-2603 / 0	-104.9	-104.9	0.39 (1)	3.97
L- M	-2906 / 0	-104.9	-104.9	0.19 (1)	3.94
M- N	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Y- B	-77 / 0	0.0	0.0	0.01 (2)	7.61
O- M	-2224 / 0	0.0	0.0	0.15 (1)	6.85
Y- X	0 / 0	-28.0	-28.0	0.40 (2)	10.00
X- W	0 / 1120	-28.0	-28.0	0.52 (2)	10.00
W- V	0 / 1295	-28.0	-28.0	0.48 (2)	10.00
V- U	0 / 1295	-28.0	-28.0	0.48 (2)	10.00
U- T	0 / 27	-28.0	-28.0	0.21 (2)	10.00
T- S	0 / 121	0.0	0.0	0.05 (1)	10.00
S- H	-480 / 0	0.0	0.0	0.09 (1)	6.25
S- R	0 / 1640	-28.0	-28.0	0.39 (2)	10.00
R- Q	0 / 2288	-28.0	-28.0	0.48 (1)	10.00
P- Q	0 / 36	0.0	0.0	0.12 (1)	10.00
Q- L	-10 / 85	0.0	0.0	0.09 (1)	10.00
P- O	0 / 69	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- F	-44 / 198	0.04 (3)
F- U	0 / 791	0.18 (1)
U- G	-790 / 0	0.65 (1)
R- J	-581 / 0	0.31 (1)
R- L	-281 / 0	0.12 (1)
X- C	-375 / 0	0.06 (1)
B- X	-117 / 53	0.02 (1)
X- D	-2312 / 0	0.70 (1)
D- W	0 / 308	0.07 (1)
U- S	0 / 1691	0.27 (1)
G- S	0 / 61	0.01 (1)
S- I	0 / 847	0.19 (1)
I- R	0 / 826	0.19 (1)
Q- O	-89 / 0	0.01 (1)
Q- M	0 / 2226	0.50 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.50/1.00 (F-G:1) , BC=0.52/1.00 (W-X:2) , WB=0.70/1.00 (D-X:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.57 (M) (INPUT = 1.00)

DWG NO. TAM 13/96-98

STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T2S	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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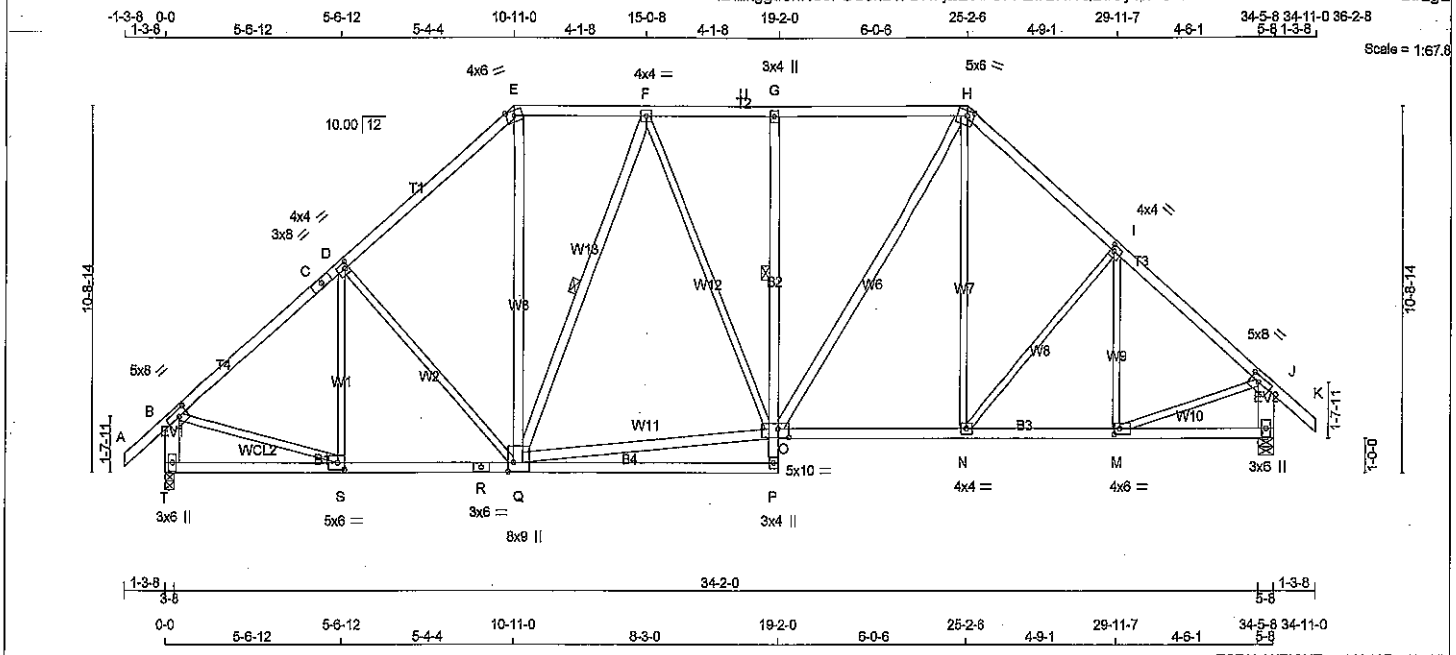

TOTAL WEIGHT = 4 X 197 = 787 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.25
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	2.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	8.0	2.00	3.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.00
N	BMWW-t	MT20	4.0	4.0		
O	BVMWWW-t	MT20	5.0	10.0	3.00	4.00
P	BMV+p	MT20	3.0	4.0		
Q	BMWWWW*+m	MT20	8.0	9.0	3.25	2.00
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2518	0	2518	0	0	3-8	3-8
L	2521	0	2521	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1989	1224 / 0	367 / 0	0 / 0	0 / 0	399 / 0	0 / 0
L	1992	1224 / 0	367 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

FOR SECTION E-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2491 / 0	-104.9	-104.9	0.55 (1)	3.86	
C- D	-2491 / 0	-104.9	-104.9	0.55 (1)	3.86	
D- E	-2308 / 0	-104.9	-104.9	0.51 (1)	4.02	
E- F	-1750 / 0	-112.4	-112.4	0.26 (1)	2.00	
F- U	-2240 / 0	-112.4	-112.4	0.45 (1)	2.00	
U- G	-2240 / 0	-112.4	-112.4	0.45 (1)	2.00	
G- H	-2244 / 0	-112.4	-112.4	0.59 (1)	2.00	
H- I	-2377 / 0	-104.9	-104.9	0.41 (1)	4.08	
I- J	-2472 / 0	-104.9	-104.9	0.43 (1)	4.00	
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
T- B	-2449 / 0	0.0	0.0	0.17 (1)	6.60	
L- J	-2461 / 0	0.0	0.0	0.17 (1)	6.59	
T- S	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
S- R	0 / 1947	-28.0	-28.0	0.59 (2)	10.00	
R- Q	0 / 1947	-28.0	-28.0	0.59 (2)	10.00	
Q- P	0 / 41	-28.0	-28.0	0.39 (3)	10.00	
P- O	0 / 159	0.0	0.0	0.07 (1)	10.00	
O- G	-683 / 0	0.0	0.0	0.17 (1)	6.25	
O- N	0 / 1795	-28.0	-28.0	0.42 (2)	10.00	
N- M	0 / 1928	-28.0	-28.0	0.44 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- D	-313 / 61	0.20 (1)
D- Q	-311 / 0	0.43 (1)
Q- E	0 / 1033	0.17 (1)
O- H	0 / 820	0.13 (1)
N- H	0 / 393	0.09 (2)
N- I	-206 / 0	0.21 (1)
M- I	-371 / 25	0.19 (1)
M- J	0 / 1994	0.45 (1)
B- S	0 / 2000	0.45 (1)
Q- O	0 / 2041	0.33 (1)
F- O	0 / 448	0.10 (1)
Q- F	-889 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.59/1.00 (G-H:1) , BC=0.59/1.00 (Q-S:2) , WB=0.57/1.00 (F-Q:1) , SSI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (O) (INPUT = 0.90)
JSI METAL= 0.63 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKIS
PROVINCE OF ONTARIO

DWG NO. TAM 13169 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T3S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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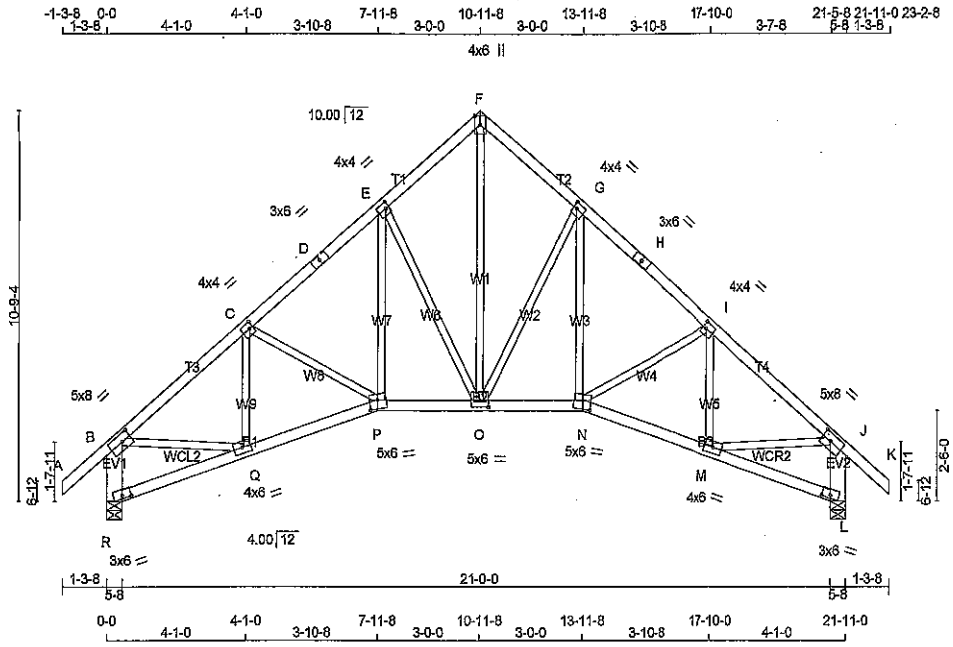

TOTAL WEIGHT = 7 X 110 = 772 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	3.25
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	8.0	2.50	3.25
L	BVM1-I	MT20	3.0	6.0	1.00	3.00
M	BMWW-t	MT20	4.0	6.0		
N	BBWW-m	MT20	5.0	6.0	3.00	3.00
O	BMWWW-t	MT20	5.0	6.0	2.25	3.00
P	BBWW-m	MT20	5.0	6.0	3.00	3.00
Q	BMWW-t	MT20	4.0	6.0		
R	BVM1-I	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1602	0	1602	0	0	5-8	5-8
L	1602	0	1602	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1259	802 / 0	230 / 0	0 / 0	0 / 0	227 / 0	0 / 0
L	1259	802 / 0	230 / 0	0 / 0	0 / 0	227 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BEARING SIZE FACTOR = 1.15 AT JNT(S) R, L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1855 / 0	-104.9	-104.9	0.24 (1)	4.71
C- D	-1799 / 0	-104.9	-104.9	0.23 (1)	4.79
D- E	-1799 / 0	-104.9	-104.9	0.23 (1)	4.79
E- F	-1335 / 0	-104.9	-104.9	0.12 (1)	5.50
F- G	-1335 / 0	-104.9	-104.9	0.12 (1)	5.50
G- H	-1799 / 0	-104.9	-104.9	0.23 (1)	4.79
H- I	-1799 / 0	-104.9	-104.9	0.23 (1)	4.79
I- J	-1855 / 0	-104.9	-104.9	0.24 (1)	4.71
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
R- B	-1554 / 0	0.0	0.0	0.11 (1)	7.81
L- J	-1554 / 0	0.0	0.0	0.11 (1)	7.81
R- Q	0 / 7	-28.0	-28.0	0.12 (3)	10.00
Q- P	0 / 1520	-28.0	-28.0	0.30 (1)	10.00
P- O	0 / 1361	-28.0	-28.0	0.27 (1)	10.00
O- N	0 / 1361	-28.0	-28.0	0.27 (1)	10.00
N- M	0 / 1520	-28.0	-28.0	0.30 (1)	10.00
M- L	0 / 7	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1444	0.32 (1)
O- G	-756 / 0	0.53 (1)
N- G	0 / 598	0.13 (1)
N- I	-84 / 0	0.03 (1)
M- I	-382 / 21	0.09 (1)
E- O	-756 / 0	0.53 (1)
P- E	0 / 598	0.13 (1)
C- P	-84 / 0	0.03 (1)
Q- C	-382 / 21	0.09 (1)
B- Q	0 / 1446	0.33 (1)
M- J	0 / 1446	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.30/1.00 (P-Q:1) , WB=0.53/1.00 (E-O:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (Q) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3876
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13170-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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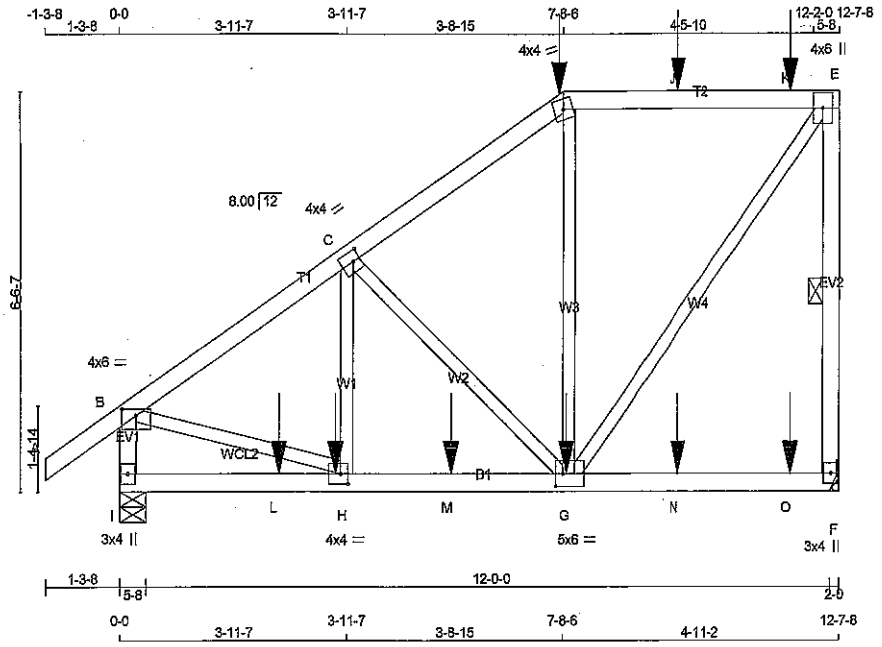

TOTAL WEIGHT = 61 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 189.7 lbs FACTORED DOWN AT 7-8-6, AND 186.2 lbs FACTORED DOWN AT 9-9-2, AND 193.3 lbs FACTORED DOWN AT 11-9-2 ON TOP CHORD, AND 92.5 lbs FACTORED DOWN AT 2-9-2, 92.5 lbs FACTORED DOWN AT 3-9-2, 92.5 lbs FACTORED DOWN AT 5-9-2, 92.5 lbs FACTORED DOWN AT 7-9-2, AND 92.5 lbs FACTORED DOWN AT 9-9-2, AND 95.8 lbs FACTORED DOWN AT 11-9-2 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1455	0	1455	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-0	
I	1256	0	1255	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1187	681 / 0	265 / 0	0 / 0	0 / 0	241 / 0	0 / 0
I	1032	578 / 0	239 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00
B- C	-1113 / 0	-104.9	-104.9	0.31 (1)	5.58
C- D	-823 / 0	-104.9	-104.9	0.30 (1)	6.25
D- J	-655 / 0	-104.9	-104.9	0.94 (1)	4.30
J- K	-655 / 0	-104.9	-104.9	0.94 (1)	4.30
K- E	-655 / 0	-104.9	-104.9	0.94 (1)	4.30
F- E	-1342 / 0	0.0	0.0	0.28 (1)	5.57
I- B	-1197 / 0	0.0	0.0	0.14 (1)	7.28
I- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00
L- H	0 / 0	-28.0	-28.0	0.15 (3)	10.00
H- M	0 / 948	-28.0	-28.0	0.41 (2)	10.00
M- G	0 / 948	-28.0	-28.0	0.41 (2)	10.00
G- N	0 / 0	-28.0	-28.0	0.28 (3)	10.00
N- O	0 / 0	-28.0	-28.0	0.28 (3)	10.00
O- F	0 / 0	-28.0	-28.0	0.28 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-33 / 254	0.06 (3)
C- G	-400 / 0	0.20 (1)
G- D	-297 / 182	0.21 (1)
G- E	0 / 1072	0.27 (1)
B- H	0 / 993	0.24 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	7-8-6	-190	-190	---	FRONT	VERT	TOTAL
G	7-9-2	-53	-92	---	FRONT	VERT	TOTAL
H	3-9-2	-53	-92	---	FRONT	VERT	TOTAL
J	9-9-2	-186	-186	---	FRONT	VERT	TOTAL
K	11-9-2	-193	-193	---	FRONT	VERT	TOTAL
L	2-9-2	-53	-92	---	FRONT	VERT	TOTAL
M	5-9-2	-53	-92	---	FRONT	VERT	TOTAL
N	9-9-2	-53	-92	---	FRONT	VERT	TOTAL
O	11-9-2	-55	-96	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.41/1.00 (G-H:2) , WB=0.27/1.00 (E-G:1) , SSI=0.43/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.36 (H) (INPUT = 1.00)

DWG NO. TAM 13719 -28
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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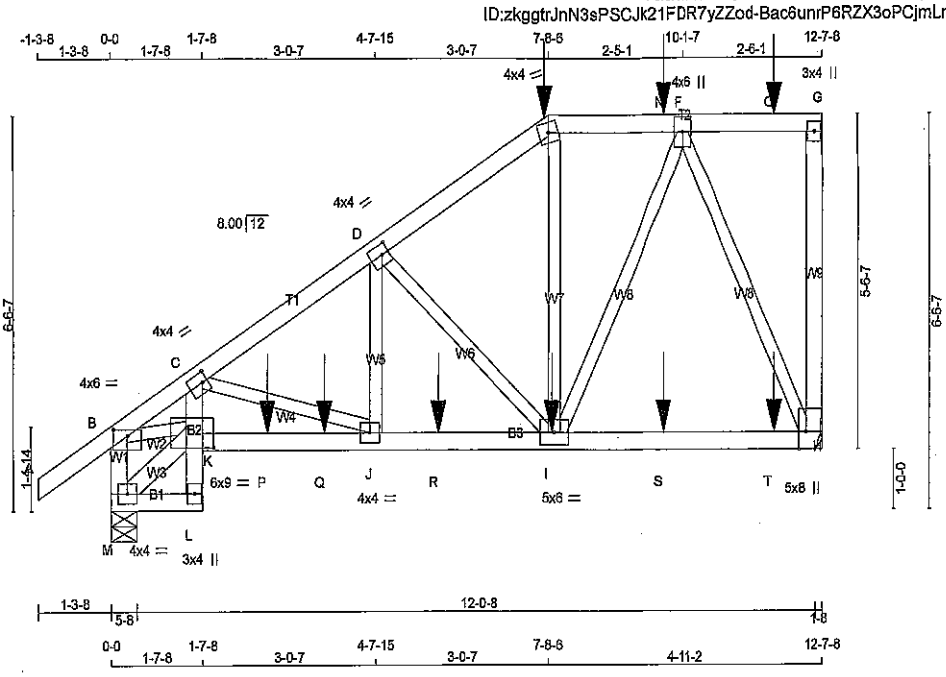

TOTAL WEIGHT = 65 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.25	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+t	MT20	5.0	8.0	Edge 1.50	
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BVMWW-l	MT20	6.0	9.0	4.00	5.75
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 586.8 lbs FACTORED DOWN AT 7-8-6, AND 206.9 lbs FACTORED DOWN AT 9-9-2, AND 214.0 lbs FACTORED DOWN AT 11-9-2 ON TOP CHORD, AND 58.7 lbs FACTORED DOWN AT 2-9-2, 58.7 lbs FACTORED DOWN AT 3-9-2, 58.7 lbs FACTORED DOWN AT 5-9-2, 58.7 lbs FACTORED DOWN AT 7-9-2, AND 58.7 lbs FACTORED DOWN AT 9-9-2, AND 62.0 lbs FACTORED DOWN AT 11-9-2 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1661	0	1661	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-13	
M	1372	0	1372	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
H	1344	788 / 0	269 / 0	0 / 0	0 / 0	267 / 0	0 / 0
M	1108	655 / 0	235 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00
B- C	-1996 / 0	-104.9	-104.9	0.20 (1)	4.55
C- D	-1572 / 0	-104.9	-104.9	0.23 (1)	4.99
D- E	-1195 / 0	-104.9	-104.9	0.21 (1)	5.57
E- N	-969 / 0	-104.9	-104.9	0.22 (1)	6.00
N- F	-969 / 0	-104.9	-104.9	0.22 (1)	6.00
F- O	0 / 0	-104.9	-104.9	0.22 (1)	10.00
O- G	0 / 0	-104.9	-104.9	0.22 (1)	10.00
H- G	-215 / 0	0.0	0.0	0.12 (1)	7.81
M- B	-1287 / 0	0.0	0.0	0.15 (1)	7.07
M- L	0 / 81	-28.0	-28.0	0.03 (2)	10.00
L- K	0 / 36	0.0	0.0	0.15 (1)	10.00
K- C	0 / 243	0.0	0.0	0.18 (1)	10.00
K- P	0 / 1732	-28.0	-28.0	0.40 (1)	10.00
P- Q	0 / 1732	-28.0	-28.0	0.40 (1)	10.00
Q- J	0 / 1732	-28.0	-28.0	0.40 (1)	10.00
J- R	0 / 1329	-28.0	-28.0	0.40 (2)	10.00
R- I	0 / 1329	-28.0	-28.0	0.40 (2)	10.00
I- S	0 / 645	-28.0	-28.0	0.31 (2)	10.00
S- T	0 / 645	-28.0	-28.0	0.31 (2)	10.00
T- H	0 / 645	-28.0	-28.0	0.31 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-423 / 0	0.10 (1)
J- D	0 / 265	0.07 (2)
D- I	-507 / 0	0.17 (1)
I- E	-158 / 92	0.08 (1)
M- K	-104 / 0	0.01 (1)
B- K	0 / 1626	0.40 (1)
I- F	0 / 803	0.20 (1)
F- H	-1498 / 0	0.86 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
E	7-8-6	-587	-587	---	FRONT	VERT	TOTAL
I	7-9-2	-34	-59	---	FRONT	VERT	TOTAL
N	9-9-2	-207	-207	---	FRONT	VERT	TOTAL
O	11-9-2	-214	-214	---	FRONT	VERT	TOTAL
P	2-9-2	-34	-59	---	FRONT	VERT	TOTAL
Q	3-9-2	-34	-59	---	FRONT	VERT	TOTAL
R	5-9-2	-34	-59	---	FRONT	VERT	TOTAL
S	9-9-2	-34	-59	---	FRONT	VERT	TOTAL
T	11-9-2	-35	-62	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.40/1.00 (J-K:1) , WB=0.86/1.00 (F-H:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

LICENSED PROFESSIONAL ENGINEER 3876 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 13197-B
STRUCTURAL COMPONENT ONLY

Pg 1/6

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T4S	1	1	TRUSS DESC.	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3575
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13192-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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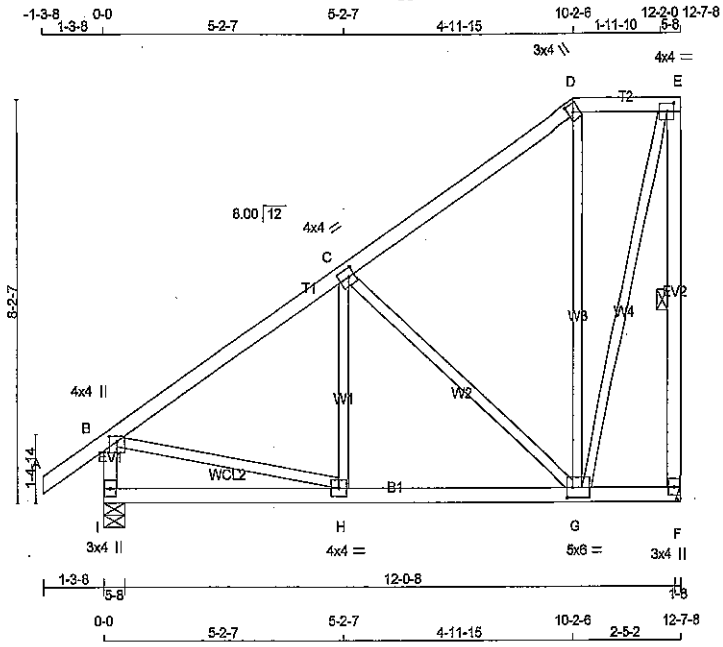

TOTAL WEIGHT = 68 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	839	0	839	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	983	0	983	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	669	410 / 0	133 / 0	0 / 0	0 / 0	126 / 0	0 / 0
I	766	499 / 0	133 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-750 / 0	-104.9	-104.9	0.36 (1)	6.25	
C- D	-285 / 0	-104.9	-104.9	0.35 (1)	6.25	
D- E	-203 / 0	-104.9	-104.9	0.08 (1)	6.25	
F- E	-818 / 0	0.0	0.0	0.25 (1)	6.25	
I- B	-925 / 0	0.0	0.0	0.10 (1)	7.81	
I- H	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
H- G	0 / 654	-28.0	-28.0	0.26 (2)	10.00	
G- F	0 / 0	-28.0	-28.0	0.09 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 237	0.05 (3)
C- G	-601 / 0	0.52 (1)
G- D	-186 / 38	0.24 (1)
G- E	0 / 721	0.16 (1)
B- H	0 / 668	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.26/1.00 (G-H:2) , WB=0.52/1.00 (C-G:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.25 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3628
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13172-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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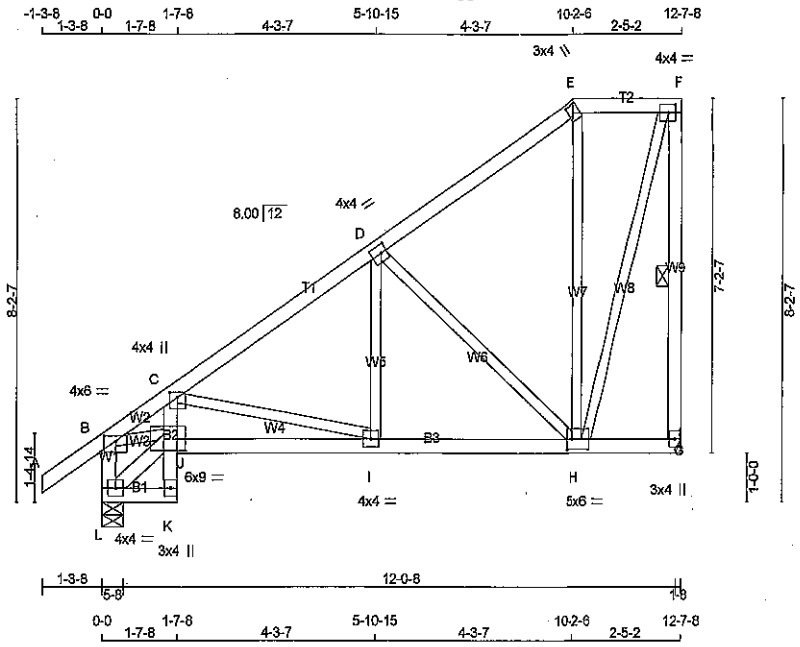

TOTAL WEIGHT = 69 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.25	3.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW+h	MT20	3.0	4.0	2.00	1.25
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	5.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	831	0	831	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
L	991	0	991	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	664	405 / 0	133 / 0	0 / 0	0 / 0	126 / 0	0 / 0
L	772	504 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1356 / 0	-104.9	-104.9	0.12 (1)	5.46
C- D	-835 / 0	-104.9	-104.9	0.25 (1)	6.25
D- E	-308 / 0	-104.9	-104.9	0.24 (1)	6.25
E- F	-229 / 0	-104.9	-104.9	0.08 (1)	6.25
G- F	-807 / 0	0.0	0.0	0.19 (1)	6.25
L- B	-926 / 0	0.0	0.0	0.10 (1)	7.81
L- K	0 / 56	-28.0	-28.0	0.02 (2)	10.00
K- J	0 / 36	0.0	0.0	0.09 (1)	10.00
J- C	0 / 168	0.0	0.0	0.11 (1)	10.00
J- I	0 / 1188	-28.0	-28.0	0.27 (1)	10.00
I- H	0 / 713	-28.0	-28.0	0.22 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-488 / 0	0.17 (1)
I- D	0 / 295	0.07 (2)
D- H	-654 / 0	0.40 (1)
H- E	-145 / 48	0.13 (1)
H- F	0 / 718	0.16 (1)
L- J	-71 / 0	0.01 (1)
B- J	0 / 1117	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.27/1.00 (I-J:1) , WB=0.40/1.00 (D-H:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13198-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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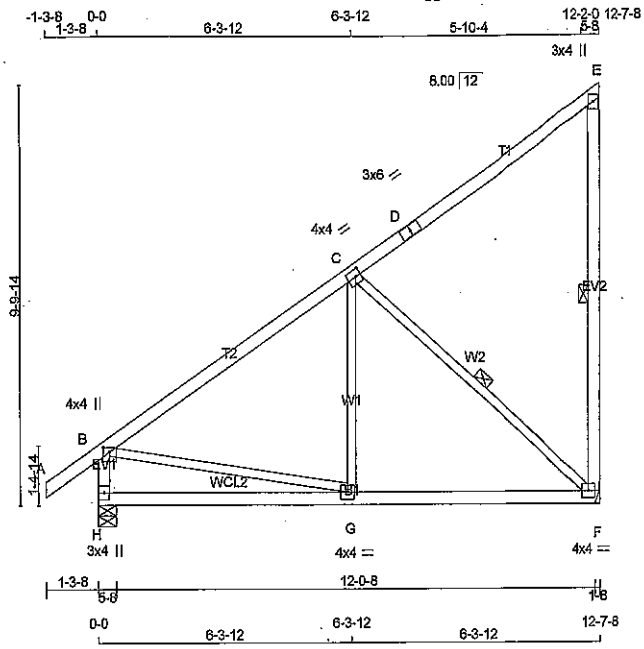

TOTAL WEIGHT = 2 X 61 = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	839	0	839	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	983	0	983	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	669	410 / 0	133 / 0	0 / 0	0 / 0	126 / 0	0 / 0
H	766	499 / 0	133 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-703 / 0	-104.9	-104.9	0.56 (1)		6.16
C- D	-45 / 0	-104.9	-104.9	0.55 (1)		6.25
D- E	-45 / 0	-104.9	-104.9	0.55 (1)		6.25
F- E	-249 / 0	0.0	0.0	0.11 (1)		6.25
H- B	-815 / 0	0.0	0.0	0.09 (1)		7.81
H- G	0 / 0	-28.0	-28.0	0.33 (3)		10.00
G- F	0 / 623	-28.0	-28.0	0.39 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 340	0.06 (3)
C- F	-812 / 0	0.36 (1)
B- G	0 / 632	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.39/1.00 (F-G:2) , WB=0.36/1.00 (C-F:1) , SSI=0.27/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13173 -18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T6S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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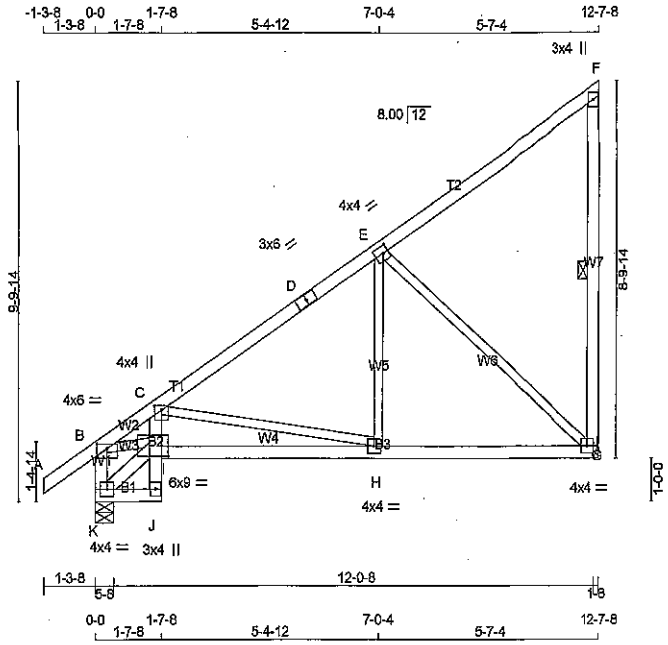

Scale = 1:54.1

TOTAL WEIGHT = 62 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.25	3.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BVMWW-l	MT20	6.0	9.0	4.00	5.50
J	BMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	831	0	831	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
K	991	0	991	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	664	405 / 0	133 / 0	0 / 0	0 / 0	126 / 0	0 / 0
K	772	504 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1411 / 0	-104.9	-104.9	0.15 (1)	5.35	
C- D	-743 / 0	-104.9	-104.9	0.40 (1)	6.25	
D- E	-743 / 0	-104.9	-104.9	0.40 (1)	6.25	
E- F	-37 / 0	-104.9	-104.9	0.40 (1)	6.25	
G- F	-227 / 0	0.0	0.0	0.08 (1)	6.25	
K- B	-924 / 0	0.0	0.0	0.10 (1)	7.81	
K- J	0 / 58	-28.0	-28.0	0.02 (2)	10.00	
J- I	0 / 36	0.0	0.0	0.10 (1)	10.00	
I- C	0 / 180	0.0	0.0	0.12 (1)	10.00	
I- H	0 / 1256	-28.0	-28.0	0.38 (2)	10.00	
H- G	0 / 640	-28.0	-28.0	0.32 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-627 / 0	0.34 (1)
H- E	0 / 387	0.09 (2)
E- G	-639 / 0	0.84 (1)
K- I	-74 / 0	0.01 (1)
B- I	0 / 1181	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.40/1.00 (C-E:1) , BC=0.38/1.00 (H-I:2) , WB=0.84/1.00 (E-G:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.77 (I) (INPUT = 0.90)
ISI METAL= 0.31 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13199 -B
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T7	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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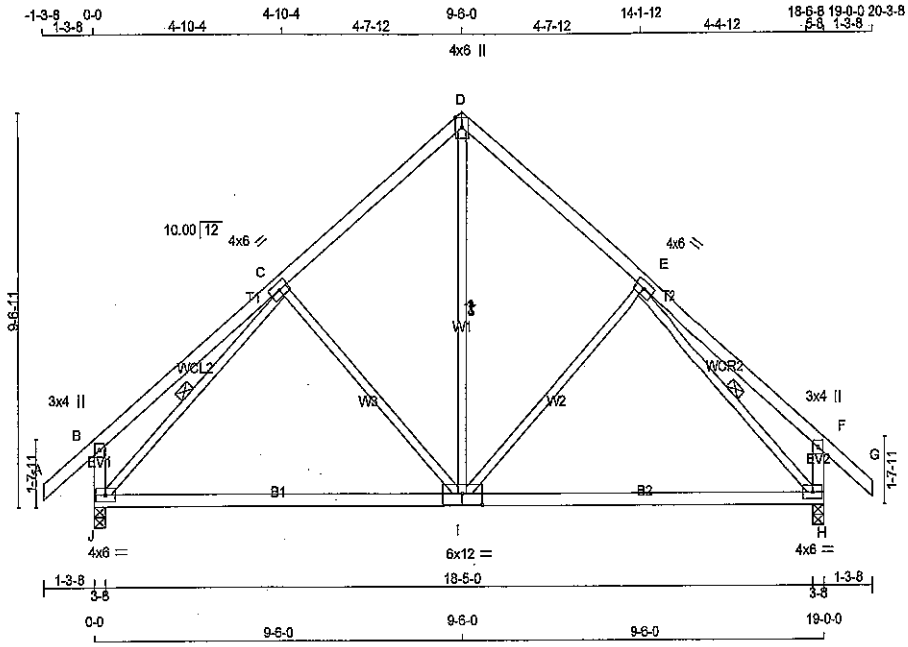

TOTAL WEIGHT = 3 X 86 = 258 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BSWWW-t	MT20	6.0	12.0	3.25	6.00
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1408	0	1408	0	0	3-8	3-8
H	1408	0	1408	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1105	707 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	1105	707 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-J, E-H. D-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 38	-104.9	-104.9	0.39 (1)	10.00
C- D	-928 / 0	-104.9	-104.9	0.31 (1)	6.03
D- E	-928 / 0	-104.9	-104.9	0.31 (1)	6.03
E- F	0 / 38	-104.9	-104.9	0.39 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- B	-334 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-334 / 0	0.0	0.0	0.04 (1)	7.81
J- I	0 / 877	-28.0	-28.0	0.83 (2)	10.00
I- H	0 / 877	-28.0	-28.0	0.83 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 756	0.17 (1)
I- E	-288 / 34	0.28 (1)
C- I	-288 / 34	0.28 (1)
J- C	-1311 / 0	0.44 (1)
E- H	-1311 / 0	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 977 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.39")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.83/1.00 (I-J:2) , WB=0.44/1.00 (C-J:1) , SSI=0.23/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.53 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13174-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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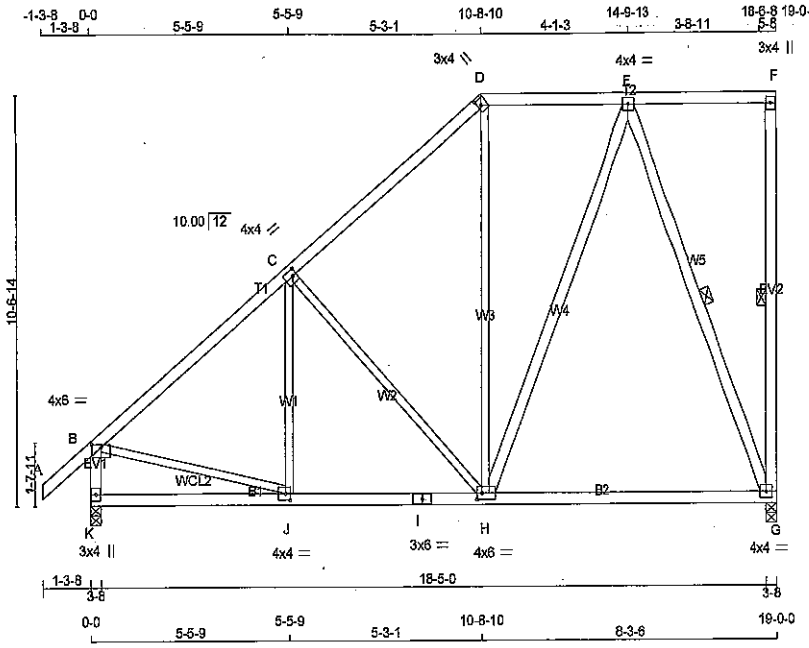

TOTAL WEIGHT = 110 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1263	0	1263	0	0	3-8	3-8
K	1408	0	1408	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1006	617 / 0	200 / 0	0 / 0	0 / 0	190 / 0	0 / 0
K	1105	707 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1142 / 0	-104.9	-104.9	0.42 (1)	5.42
C- D	-810 / 0	-104.9	-104.9	0.40 (1)	6.18
D- E	-588 / 0	-104.9	-104.9	0.23 (1)	6.25
E- F	0 / 0	-104.9	-104.9	0.30 (1)	10.00
G- F	-167 / 0	0.0	0.0	0.09 (1)	6.25
K- B	-1341 / 0	0.0	0.0	0.14 (1)	7.00
K- J	0 / 0	-28.0	-28.0	0.19 (2)	10.00
J- I	0 / 911	-28.0	-28.0	0.50 (2)	10.00
I- H	0 / 911	-28.0	-28.0	0.50 (2)	10.00
H- G	0 / 408	-28.0	-28.0	0.44 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-78 / 157	0.05 (1)
C- H	-480 / 0	0.63 (1)
H- D	0 / 200	0.05 (2)
H- E	0 / 498	0.08 (1)
E- G	-1081 / 0	0.72 (1)
B- J	0 / 936	0.21 (1)

LICENSED PROFESSIONAL ENGINEER
3 07B
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 792 (0.29")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.50/1.00 (H-J:2) , WB=0.72/1.00 (E-G:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

DWG NO. TAM 13175-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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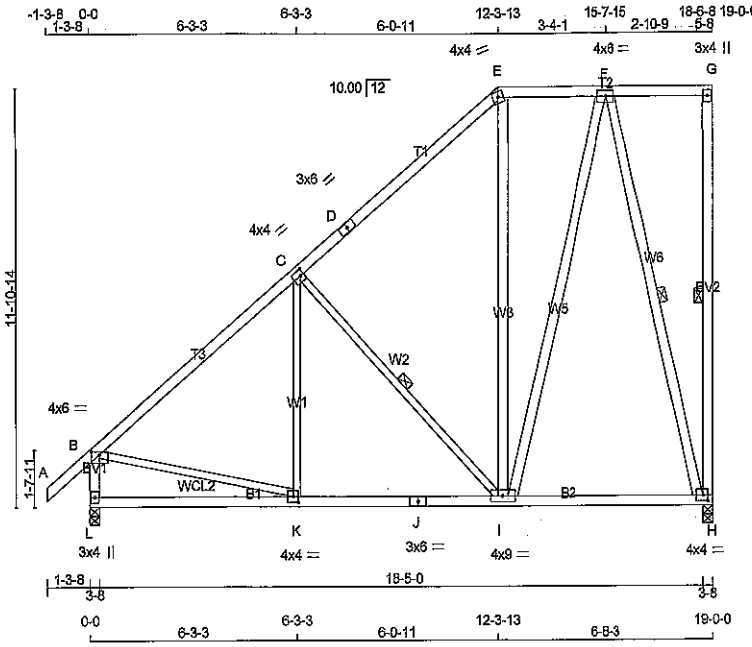

TOTAL WEIGHT = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
K - C	2x3	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	2.00	1.75
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1263	0	1263	0	0	3-8	3-8
L	1408	0	1408	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1006	617 / 0	200 / 0	0 / 0	0 / 0	190 / 0	0 / 0
L	1105	707 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1125 / 0	-104.9	-104.9	0.57 (1)	5.18
C- D	-671 / 0	-104.9	-104.9	0.53 (1)	6.25
D- E	-671 / 0	-104.9	-104.9	0.53 (1)	6.25
E- F	-475 / 0	-104.9	-104.9	0.15 (1)	6.25
F- G	0 / 0	-104.9	-104.9	0.19 (1)	10.00
H- G	-133 / 0	0.0	0.0	0.10 (1)	6.25
L- B	-1336 / 0	0.0	0.0	0.14 (1)	7.00
L- K	0 / 0	-28.0	-28.0	0.25 (3)	10.00
K- J	0 / 903	-28.0	-28.0	0.37 (2)	10.00
J- I	0 / 903	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 304	-28.0	-28.0	0.31 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 252	0.08 (3)
C- I	-621 / 0	0.35 (1)
I- E	0 / 150	0.02 (3)
B- K	0 / 922	0.21 (1)
I- F	0 / 647	0.10 (1)
F- H	-1098 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.37/1.00 (I-K:2) , WB=0.93/1.00 (F-H:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13176 -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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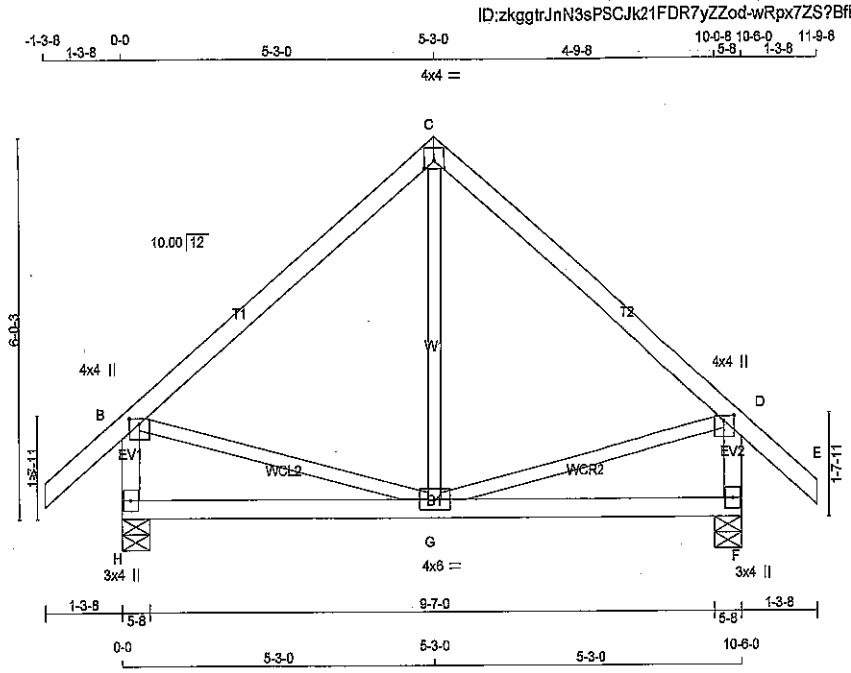

TOTAL WEIGHT = 47 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	843	0	843	0	0	5-8	5-8
F	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0
F	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-441 / 0	-104.9	-104.9	0.37 (1)	6.25
C- D	-441 / 0	-104.9	-104.9	0.37 (1)	6.25
D- E	0 / 47	-104.9	-104.9	0.14 (1)	10.00
H- B	-787 / 0	0.0	0.0	0.08 (1)	7.81
F- D	-787 / 0	0.0	0.0	0.08 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 238	0.05 (3)
B- G	0 / 349	0.08 (1)
G- D	0 / 349	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.23/1.00 (F-G:3) , WB=0.08/1.00 (B-G:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

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DWG NO. TAM 13177 -18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T10Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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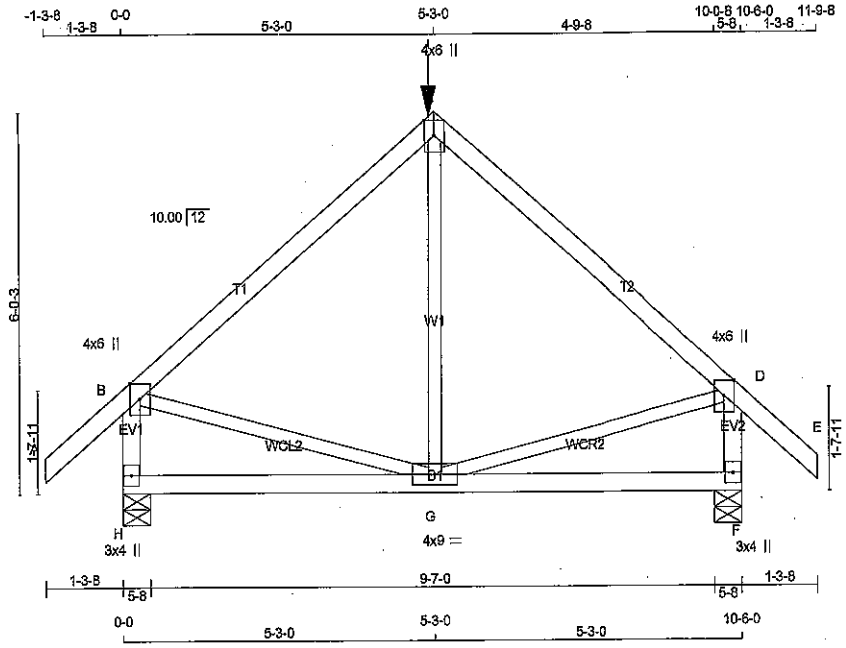

TOTAL WEIGHT = 47 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 688.5 lbs FACTORED DOWN AT 5-3-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1307	0	1307	0	0	5-8	5-8
F	1307	0	1307	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1037	644 / 0	200 / 0	0 / 0	0 / 0	193 / 0	0 / 0
F	1037	644 / 0	200 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-943 / 0	-104.9	-104.9	0.61 (1)	5.38
C- D	-943 / 0	-104.9	-104.9	0.61 (1)	5.38
D- E	0 / 47	-104.9	-104.9	0.16 (1)	10.00
H- B	-1205 / 0	0.0	0.0	0.14 (1)	7.26
F- D	-1205 / 0	0.0	0.0	0.14 (1)	7.26
H- G	0 / 0	-50.8	-50.8	0.45 (3)	10.00
G- F	0 / 0	-50.8	-50.8	0.45 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-31 / 427	0.11 (3)
B- G	0 / 747	0.18 (1)
G- D	0 / 747	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-3-0	-689	-689	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-3-0
END SETBACK = 5-3-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.61/1.00 (C-D:1) , BC=0.45/1.00 (F-G:3) , WB=0.18/1.00 (B-G:1) , SSI=0.25/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

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DWG NO. TAM 13178-13
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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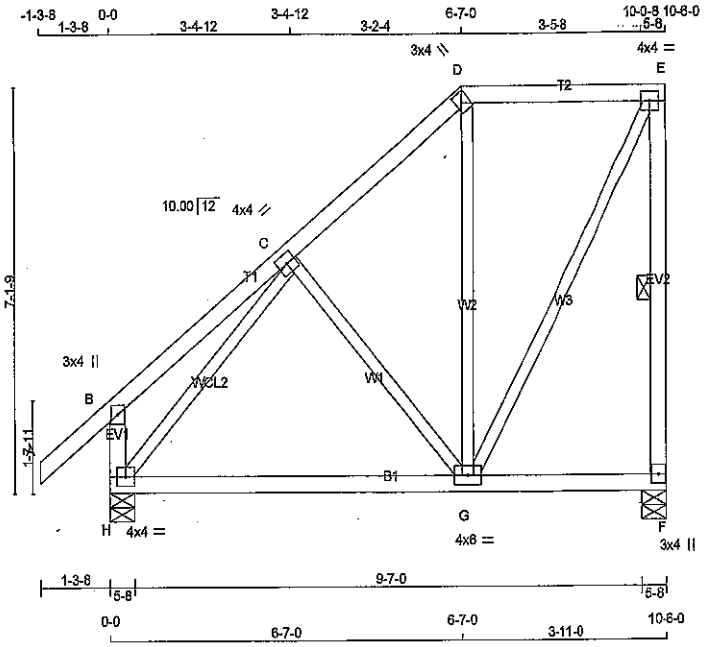

TOTAL WEIGHT = 57 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	698	0	698	0	0	5-8	5-8
H	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.19 (1)	10.00
C- D	-370 / 0	-104.9	-104.9	0.14 (1)	6.25
D- E	-262 / 0	-104.9	-104.9	0.21 (1)	6.25
F- E	-673 / 0	0.0	0.0	0.15 (1)	6.25
H- B	-275 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 398	-28.0	-28.0	0.31 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.28 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-208 / 15	0.10 (1)
G- D	-119 / 87	0.11 (1)
G- E	0 / 537	0.12 (1)
H- C	-632 / 0	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 988 (0.13")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.31/1.00 (G-H:2) , WB=0.27/1.00 (C-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 13179 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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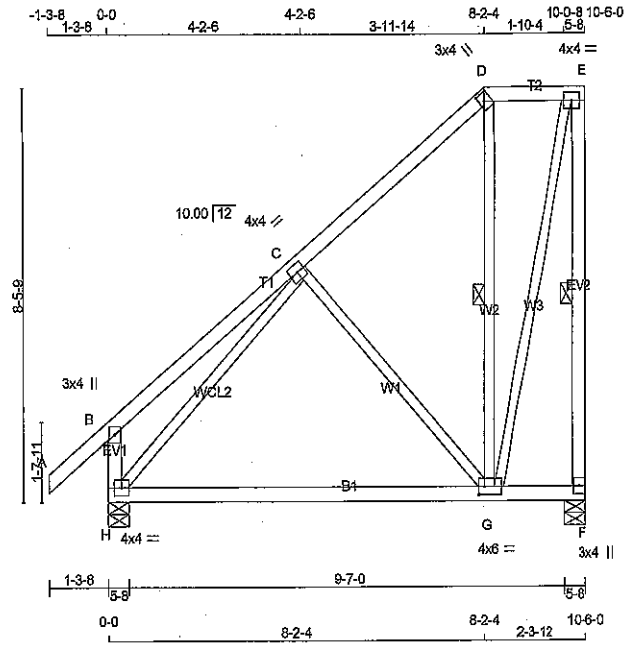

TOTAL WEIGHT = 62 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0	2.00	1.50
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	698	0	698	0	0	5-8	5-8
H	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 34	-104.9	-104.9	0.30 (1)	10.00
C- D	-257 / 0	-104.9	-104.9	0.22 (1)	6.25
D- E	-170 / 0	-104.9	-104.9	0.07 (1)	6.25
F- E	-747 / 0	0.0	0.0	0.24 (1)	6.25
H- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 363	-28.0	-28.0	0.48 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.45 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- G	-317 / 0	0.22 (1)
G- D	-143 / 65	0.07 (1)
G- E	0 / 650	0.15 (1)
H- C	-586 / 0	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 742 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 447 (0.28")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.48/1.00 (G-H:2) , WB=0.39/1.00 (C-H:1) , SSI=0.19/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13180 -B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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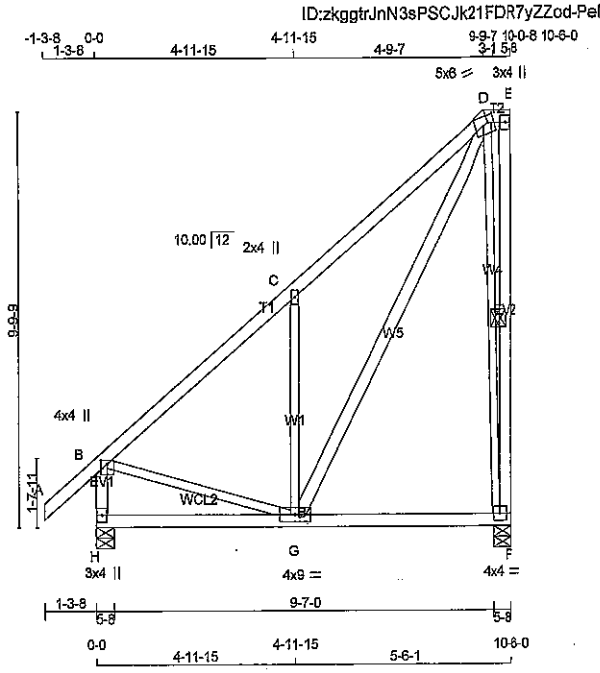

TOTAL WEIGHT = 69 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	698	0	698	0	0	5-8	5-8
H	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-495 / 0	-104.9	-104.9	0.32 (1)	6.25
C- D	-560 / 0	-104.9	-104.9	0.33 (1)	6.25
D- E	0 / 0	-104.9	-104.9	0.01 (1)	10.00
F- E	-35 / 0	0.0	0.0	0.02 (1)	6.25
H- B	-790 / 0	0.0	0.0	0.08 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00
G- F	0 / 42	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-631 / 0	0.33 (1)
B- G	0 / 424	0.10 (1)
D- F	-603 / 0	0.39 (1)
G- D	0 / 788	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.23/1.00 (F-G:3) , WB=0.39/1.00 (D-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (D) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13/8 / 18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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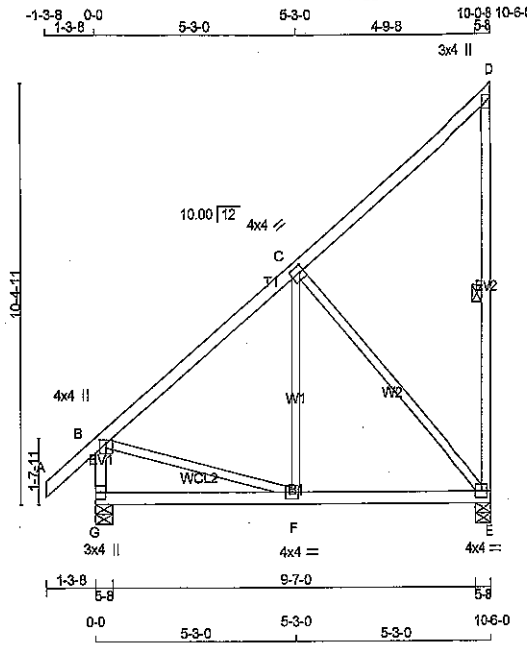

TOTAL WEIGHT = 57 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	695	0	698	0	0	5-8	5-8
G	843	0	843	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
G	654	431 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-479 / 0	-104.9	-104.9	0.38 (1)	6.25
C- D	-44 / 0	-104.9	-104.9	0.37 (1)	6.25
E- D	-207 / 0	0.0	0.0	0.11 (1)	6.25
G- B	-787 / 0	0.0	0.0	0.08 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00
F- E	0 / 401	-28.0	-28.0	0.26 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 278	0.06 (3)
C- E	-591 / 0	0.69 (1)
B- F	0 / 413	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.26/1.00 (E-F:2) , WB=0.69/1.00 (C-E:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13183-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T15	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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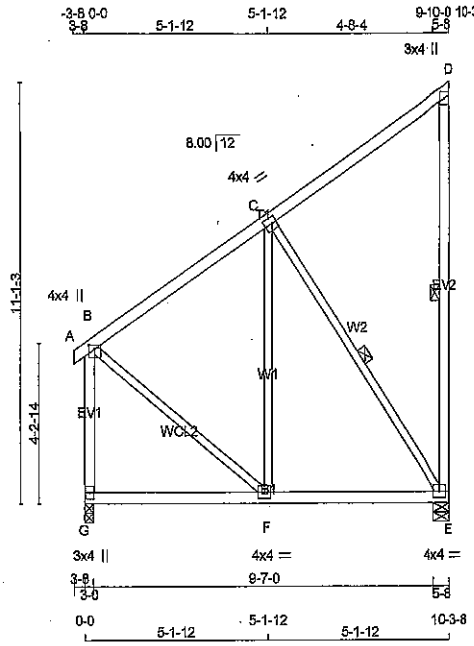

TOTAL WEIGHT = 3 X 60 = 181 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	684	0	684	0	0	5-8	5-8
G	723	0	723	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	545	334 / 0	108 / 0	0 / 0	0 / 0	103 / 0	0 / 0
G	572	358 / 0	108 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 11	-104.9	-104.9	0.01 (1)	10.00
B- C	-321 / 0	-104.9	-104.9	0.36 (1)	6.25
C- D	-37 / 0	-104.9	-104.9	0.36 (1)	6.25
E- D	-203 / 0	0.0	0.0	0.13 (1)	6.25
G- B	-668 / 0	0.0	0.0	0.20 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00
F- E	0 / 298	-28.0	-28.0	0.25 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-49 / 217	0.05 (1)
C- E	-520 / 0	0.28 (1)
B- F	0 / 375	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.25/1.00 (E-F:2) , WB=0.28/1.00 (C-E:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13164-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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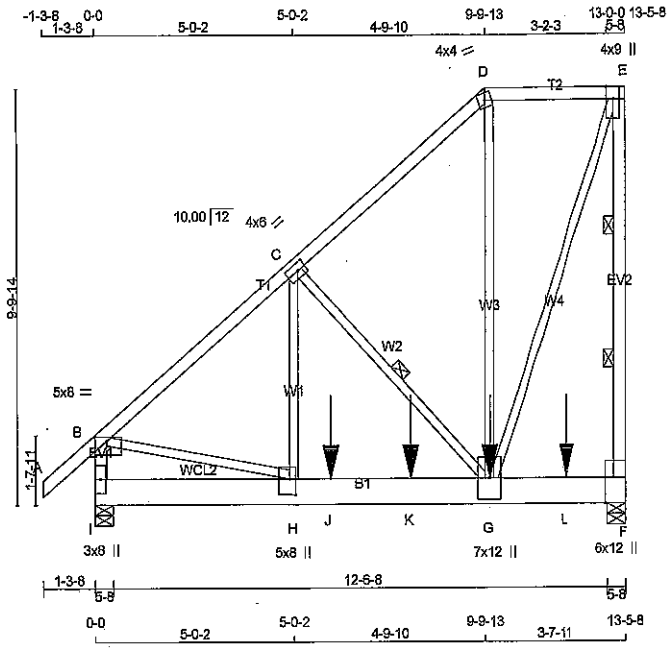

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	9.0	Edge	
F	BMV1+t	MT20	6.0	12.0	Edge	2.50
G	BMWWW+t	MT20	7.0	12.0		
H	BMWW+t	MT20	5.0	8.0	4.50	1.75
I	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1427.4 lbs FACTORED DOWN AT 5-11-4, 811.1 lbs FACTORED DOWN AT 7-11-4, AND 811.1 lbs FACTORED DOWN AT 9-11-4, AND 811.1 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3320	0	3320	0	0	5-8	5-8
I	2475	0	2475	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2625	1645 / 0	498 / 0	0 / 0	0 / 0	483 / 0	0 / 0
I	1953	1230 / 0	365 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF E-F.
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-2594 / 0	-104.9	-104.9	0.71 (1)	3.56
C- D	-1400 / 0	-104.9	-104.9	0.54 (1)	4.79
D- E	-1048 / 0	-104.9	-104.9	0.28 (1)	5.74
F- E	-2982 / 0	0.0	0.0	0.65 (1)	3.96
I- B	-2497 / 0	0.0	0.0	0.28 (1)	5.36
I- H	0 / 0	-28.0	-28.0	0.23 (1)	10.00
H- J	0 / 2022	-28.0	-28.0	0.74 (1)	10.00
J- K	0 / 2022	-28.0	-28.0	0.74 (1)	10.00
K- G	0 / 2022	-28.0	-28.0	0.74 (1)	10.00
G- L	0 / 0	-28.0	-28.0	0.27 (1)	10.00
L- F	0 / 0	-28.0	-28.0	0.27 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 1237	0.31 (1)
C- G	-1444 / 0	0.52 (1)
G- D	0 / 486	0.12 (1)
G- E	0 / 2969	0.73 (1)
B- H	0 / 2074	0.51 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	9-11-4	-811	-811	---	BACK	VERT	TOTAL
J	5-11-4	-1427	-1427	---	BACK	VERT	TOTAL
K	7-11-4	-811	-811	---	BACK	VERT	TOTAL
L	11-11-4	-811	-811	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 992 (0.16")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.74/1.00 (G-H:1) , WB=0.73/1.00 (E-G:1) , SSI=0.90/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)

DWG NO. TAM 13185-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287663	T16S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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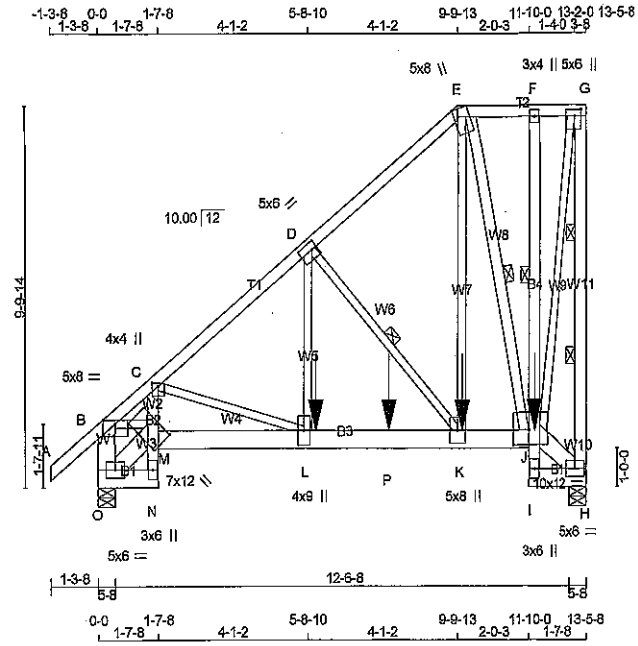

TOTAL WEIGHT = 111 lb

[M]

LUMBER

N.L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - J	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMWW-t	MT20	5.0	6.0	2.25	1.75
E	TTWW+m	MT20	5.0	8.0	Edge	1.25
F	TMV+p	MT20	3.0	4.0		
G	TMVW+p	MT20	5.0	6.0	2.00	2.00
H	BMVW1-t	MT20	5.0	6.0		
I	BMV+p	MT20	3.0	6.0		
J	BVMWWW-l	MT20	10.0	12.0	4.50	5.75
K	BMWW+t	MT20	5.0	8.0		
L	BMWW+t	MT20	4.0	9.0		
M	BVMWW-w	MT20	7.0	12.0	2.75	7.50
N	BMV+p	MT20	3.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1632.6 lbs FACTORED DOWN AT 5-11-4, 803.3 lbs FACTORED DOWN AT 7-11-4, AND 803.3 lbs FACTORED DOWN AT 9-11-4, AND 824.5 lbs FACTORED DOWN AT 11-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	3381	0	3381	0	0	5-8	5-8
O	2617	0	2617	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2678	1670 / 0	512 / 0	0 / 0	0 / 0	496 / 0	0 / 0
O	2064	1303 / 0	384 / 0	0 / 0	0 / 0	377 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF G-H.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-J, D-K, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	
B- C	-3588 / 0	-104.9	-104.9	0.31 (1)	3.40	
C- D	-3168 / 0	-104.9	-104.9	0.52 (1)	3.46	
D- E	-1492 / 0	-104.9	-104.9	0.34 (1)	4.96	
E- F	-568 / 0	-104.9	-104.9	0.14 (1)	6.25	
F- G	-555 / 0	-104.9	-104.9	0.10 (1)	6.25	
H- G	-3288 / 0	0.0	0.0	0.74 (1)	3.75	
O- B	-2519 / 0	0.0	0.0	0.19 (1)	6.48	
O- N	0 / 93	-28.0	-28.0	0.02 (2)	10.00	
N- M	0 / 36	0.0	0.0	0.18 (1)	10.00	
M- C	0 / 172	0.0	0.0	0.20 (1)	10.00	
M- L	0 / 2830	-28.0	-28.0	0.50 (1)	10.00	
L- P	0 / 2452	-28.0	-28.0	0.89 (1)	10.00	
P- K	0 / 2452	-28.0	-28.0	0.89 (1)	10.00	
K- J	0 / 1157	-28.0	-28.0	0.22 (1)	10.00	
I- J	0 / 36	0.0	0.0	0.16 (1)	10.00	
J- F	-120 / 28	0.0	0.0	0.16 (1)	6.25	
I- H	0 / 107	-28.0	-28.0	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-398 / 0	0.14 (1)
L- D	0 / 2165	0.54 (1)
D- K	-2067 / 0	0.63 (1)
K- E	0 / 2918	0.72 (1)
E- J	-2368 / 0	0.88 (1)
O- M	-121 / 0	0.01 (1)
B- M	0 / 2734	0.68 (1)
J- H	-129 / 0	0.01 (1)
J- G	0 / 3220	0.80 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	
J	11-11-12	-825	-825	---	BACK	VERT	TOTAL
K	9-11-4	-803	-803	---	BACK	VERT	TOTAL
L	5-11-4	-1633	-1633	---	BACK	VERT	TOTAL
P	7-11-4	-803	-803	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 682 (0.24")

CSI: TC=0.74/1.00 (G-H:1) , BC=0.89/1.00 (K-L:1) , WB=0.68/1.00 (E-J:1) , SSI=0.31/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

DWG NO. TAM 13200.18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T22S5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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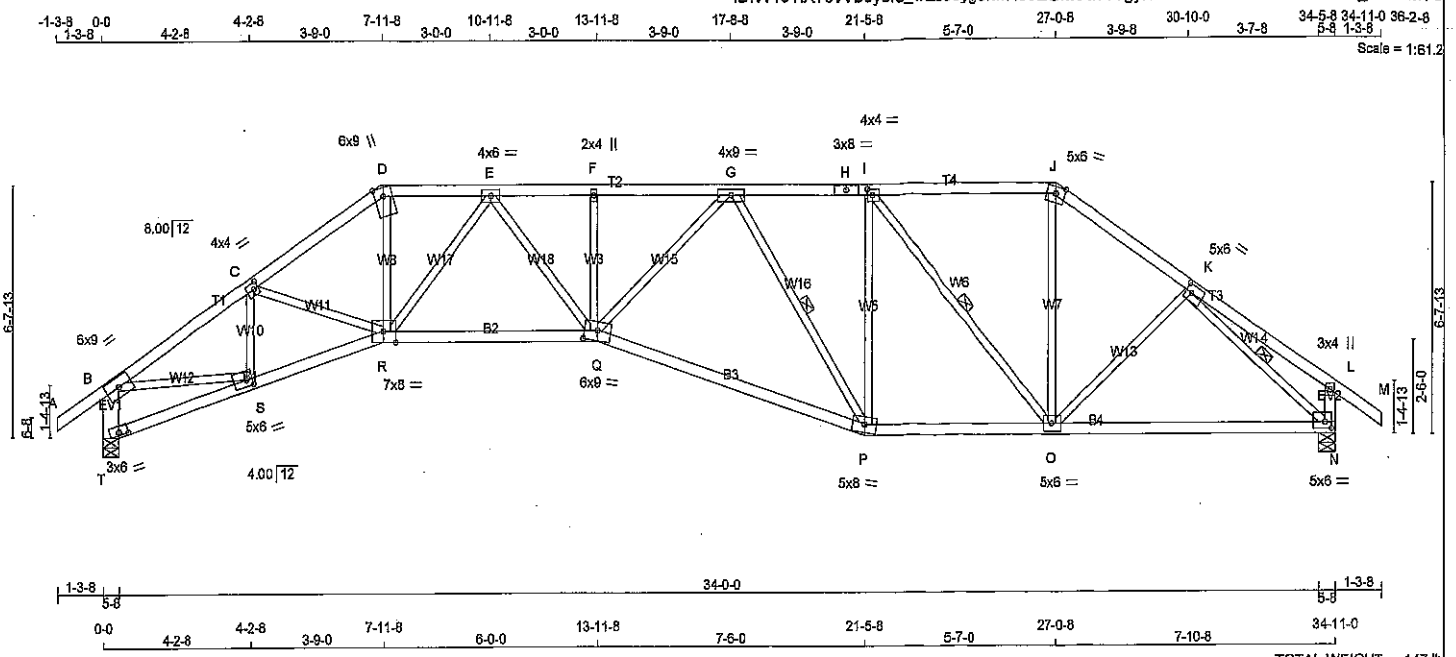

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+m	MT20	6.0	9.0	Edge	3.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	9.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TTW-m	MT20	5.0	6.0	Edge	
K	TMWW-t	MT20	5.0	6.0	2.50	2.00
L	TMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	5.0	6.0	2.00	2.25
O	BMWWW-t	MT20	5.0	6.0		
P	BBWW-m	MT20	5.0	8.0		
Q	BBWWW-m	MT20	6.0	9.0	3.25	4.50
R	BBWWW-p	MT20	7.0	8.0	Edge	4.25
S	BMWW-t	MT20	5.0	6.0	2.00	2.00
T	BVM1-l	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2465	0	2465	0	0	5-8	5-8
N	2465	0	2465	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
N	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-O, K-N, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-4009 / 0	-104.9	-104.9	0.50 (1)	3.17
C- D	-4446 / 0	-104.9	-104.9	0.50 (1)	2.98
D- E	-3686 / 0	-104.9	-104.9	0.29 (1)	3.45
E- F	-5066 / 0	-104.9	-104.9	0.40 (1)	2.86
F- G	-5057 / 0	-104.9	-104.9	0.52 (1)	2.78
G- H	-3061 / 0	-104.9	-104.9	0.55 (1)	3.41
H- I	-3061 / 0	-104.9	-104.9	0.55 (1)	3.41
I- J	-2273 / 0	-104.9	-104.9	0.53 (1)	3.98
J- K	-2739 / 0	-104.9	-104.9	0.35 (1)	3.89
K- L	0 / 23	-104.9	-104.9	0.23 (1)	10.00
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T- B	-2406 / 0	0.0	0.0	0.17 (1)	6.65
N- L	-311 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 6	-28.0	-28.0	0.11 (2)	10.00
S- R	0 / 3510	-28.0	-28.0	0.62 (1)	10.00
R- Q	0 / 4508	-28.0	-28.0	0.90 (1)	10.00
Q- P	0 / 4151	-28.0	-28.0	0.94 (1)	10.00
P- O	0 / 3056	-28.0	-28.0	0.73 (2)	10.00
O- N	0 / 2197	-28.0	-28.0	0.63 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- F	-360 / 0	0.09 (1)
P- I	0 / 343	0.08 (2)
I- O	-1212 / 0	0.57 (1)
O- J	0 / 1108	0.25 (1)
R- D	0 / 2166	0.49 (1)
S- C	-835 / 0	0.15 (1)
C- R	0 / 378	0.09 (1)
B- S	0 / 3356	0.76 (1)
O- K	0 / 174	0.04 (3)
K- N	-3014 / 0	0.67 (1)
Q- G	0 / 1600	0.36 (1)
G- P	-1732 / 0	0.65 (1)
R- E	-1338 / 0	0.52 (1)
E- Q	0 / 910	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 931 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 545 (0.77")

CSI: TC=0.55/1.00 (G-I:1) , BC=0.94/1.00 (P-Q:1) , WB=0.76/1.00 (B-S:1) , SSI=0.27/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (S) (INPUT = 0.90)
JSI METAL= 0.87 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3876
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 132288
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287647	T21S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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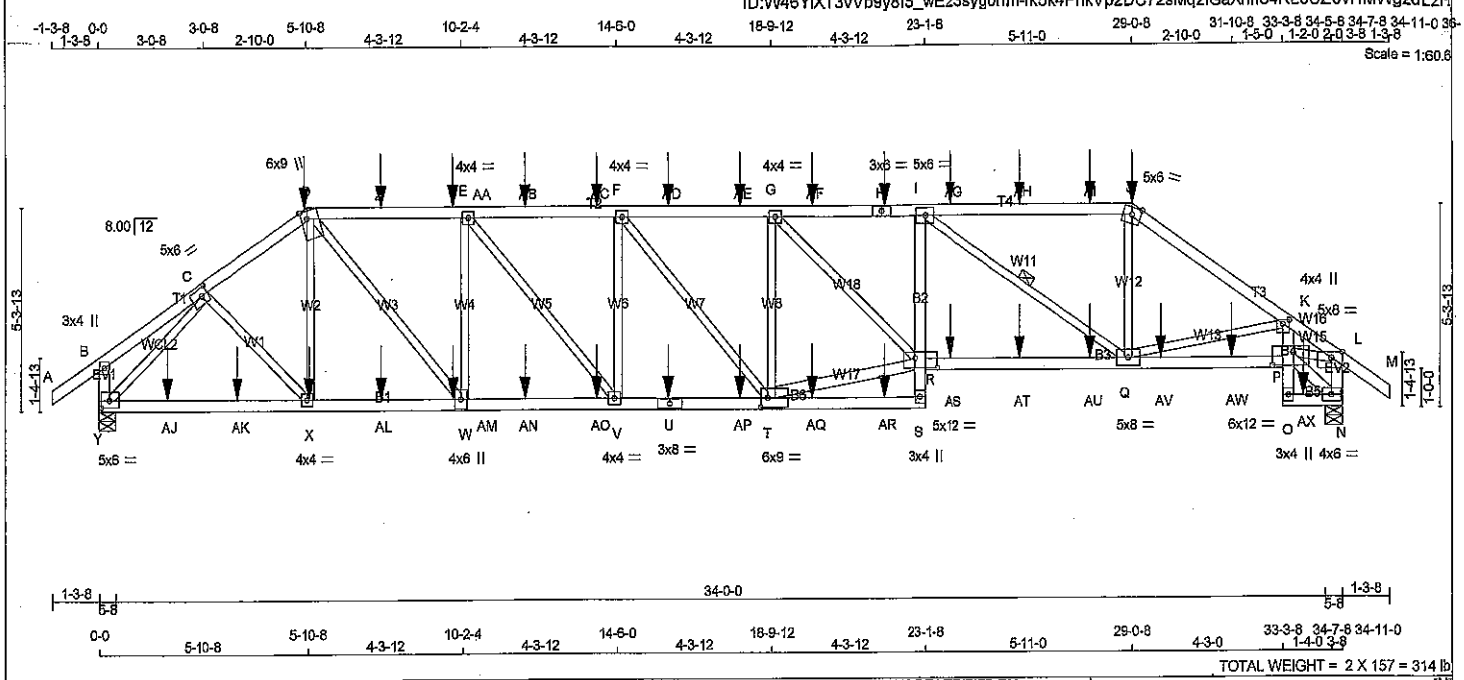

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
J- M	1	12	SIDE(61.0)
Y- B	1	12	TOP
N- L	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- U	1	12	SIDE(61.0)
U- S	1	12	SIDE(0.0)
S- I	1	12	TOP
R- P	1	12	SIDE(0.0)
K- O	1	12	TOP
O- N	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
X- D	1	6	SIDE(16.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E, F, G						
E	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	4053	0	4053	0	0	5-8	5-8
N	4172	0	4172	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	3240	1970 / 0	653 / 0	0 / 0	0 / 0	617 / 0	0 / 0
N	3370	1991 / 0	717 / 0	0 / 0	0 / 0	662 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-4 / 9	-104.9	-104.9	0.07 (1)	10.00	
C- D	-4964 / 0	-104.9	-104.9	0.16 (1)	4.22	
D- Z	-5946 / 0	-104.9	-104.9	0.39 (1)	3.68	
Z-AA	-5946 / 0	-104.9	-104.9	0.39 (1)	3.68	
AA- E	-5946 / 0	-104.9	-104.9	0.39 (1)	3.68	
E-AB	-6918 / 0	-104.9	-104.9	0.44 (1)	3.38	
AB-AC	-6918 / 0	-104.9	-104.9	0.44 (1)	3.38	
AC- F	-6918 / 0	-104.9	-104.9	0.44 (1)	3.38	
F-AD	-7093 / 0	-104.9	-104.9	0.42 (1)	3.37	
AD-AE	-7093 / 0	-104.9	-104.9	0.42 (1)	3.37	
AE- G	-7093 / 0	-104.9	-104.9	0.42 (1)	3.37	
G-AF	-8255 / 0	-104.9	-104.9	0.83 (1)	2.54	
AF- H	-8255 / 0	-104.9	-104.9	0.83 (1)	2.54	
H- I	-8255 / 0	-104.9	-104.9	0.83 (1)	2.54	
I-AG	-5357 / 0	-104.9	-104.9	0.78 (1)	3.37	
AG-AH	-5357 / 0	-104.9	-104.9	0.78 (1)	3.37	
AH-AI	-5357 / 0	-104.9	-104.9	0.78 (1)	3.37	
AI- J	-5357 / 0	-104.9	-104.9	0.78 (1)	3.37	
J- K	-6307 / 0	-104.9	-104.9	0.42 (1)	3.58	
K- L	-7203 / 0	-104.9	-104.9	0.23 (1)	3.45	
L- M	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
Y- B	-281 / 0	0.0	0.0	0.02 (1)	7.81	
N- L	-3907 / 0	0.0	0.0	0.22 (1)	5.[illegible]	
Y-AJ	0 / 3544	-28.0	-28.0	0.42 (1)	10.00	
AJ-AK	0 / 3544	-28.0	-28.0	0.42 (1)	10.00	
AK- X	0 / 3544	-28.0	-28.0	0.42 (1)	10.00	
X-AL	0 / 4108	-28.0	-28.0	0.46 (1)	10.00	
AL-AM	0 / 4108	-28.0	-28.0	0.46 (1)	10.00	
AM- W	0 / 4108	-28.0	-28.0	0.46 (1)	10.00	
W-AN	0 / 5947	-28.0	-28.0	0.58 (1)	10.00	
AN-AO	0 / 5947	-28.0	-28.0	0.58 (1)	10.00	
AO- V	0 / 5947	-28.0	-28.0	0.58 (1)	10.00	
V- U	0 / 6918	-28.0	-28.0	0.67 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- X	0 / 803	0.10 (1)
X- D	-270 / 205	0.06 (1)
D- W	0 / 2794	0.35 (1)
W- E	-1920 / 0	0.42 (1)
E- V	0 / 1499	0.19 (1)
V- F	-945 / 0	0.21 (1)
F- T	0 / 272	0.03 (1)
T- G	-1658 / 0	0.36 (1)
I- Q	-3574 / 0	0.64 (1)
Q- J	0 / 2291	0.28 (1)
Q- K	-1118 / 0	0.21 (1)
Y- C	-5101 / 0	0.77 (1)
P- N	-372 / 0	0.02 (1)
P- L	0 / 5962	0.74 (1)
T- R	0 / 7128	0.63 (1)
Q- R	0 / 1578	0.20 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 918 (0.46")

CSI: TC=0.83/1.00 (G-I:1) , BC=0.85/1.00 (Q-R:1) , WB=0.77/1.00 (C-Y:1) , SSI=0.32/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

S. KATSOULAKOS
PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO

DWG NO. TAM 13254-B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287647	T21S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TTW-m	MT20	5.0	6.0	Edge	
K	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	TMVW-p	MT20	5.0	8.0	Edge	3.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-l	MT20	6.0	12.0	4.00	7.00
Q	BMWWW-t	MT20	5.0	8.0		
R	BVMWW-l	MT20	5.0	12.0	3.00	7.75
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	9.0	3.00	2.25
U	BS-t	MT20	3.0	8.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMWW-t	MT20	4.0	4.0		
Y	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 464.2 lbs FACTORED DOWN AT 29-0-8, 530.6 lbs FACTORED DOWN AT 5-10-8, 126.3 lbs FACTORED DOWN AT 7-11-4, 126.3 lbs FACTORED DOWN AT 9-11-4, 126.3 lbs FACTORED DOWN AT 11-11-4, 126.3 lbs FACTORED DOWN AT 13-11-4, 126.3 lbs FACTORED DOWN AT 15-11-4, 126.3 lbs FACTORED DOWN AT 17-11-4, 126.3 lbs FACTORED DOWN AT 19-11-4, 126.3 lbs FACTORED DOWN AT 21-11-4, 207.2 lbs FACTORED DOWN AT 23-10-4, AND 207.2 lbs FACTORED DOWN AT 25-10-4, AND 207.2 lbs FACTORED DOWN AT 27-10-4 ON TOP CHORD, AND 76.0 lbs FACTORED DOWN AT 1-11-4, 71.7 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, 71.7 lbs FACTORED DOWN AT 11-11-4, 71.7 lbs FACTORED DOWN AT 13-11-4, 71.7 lbs FACTORED DOWN AT 15-11-4, 71.7 lbs FACTORED DOWN AT 17-11-4, 71.7 lbs FACTORED DOWN AT 19-11-4, 71.7 lbs FACTORED DOWN AT 21-11-4, 61.8 lbs FACTORED DOWN AT 23-10-4, 58.7 lbs FACTORED DOWN AT 25-10-4, 58.7 lbs FACTORED DOWN AT 27-10-4, 58.7 lbs FACTORED DOWN AT 29-10-4, AND 58.7 lbs FACTORED DOWN AT 31-10-4, AND 86.0 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U-AP	0 / 6918	-28.0	-28.0	0.67 (1)	10.00
AP- T	0 / 6918	-28.0	-28.0	0.67 (1)	10.00
T-AQ	0 / 190	-28.0	-28.0	0.13 (2)	10.00
AQ-AR	0 / 190	-28.0	-28.0	0.13 (2)	10.00
AR- S	0 / 190	-28.0	-28.0	0.13 (2)	10.00
S- R	0 / 142	0.0	0.0	0.15 (1)	10.00
R- I	0 / 822	0.0	0.0	0.22 (1)	10.00
R-AS	0 / 8304	-28.0	-28.0	0.85 (1)	10.00
AS-AT	0 / 8304	-28.0	-28.0	0.85 (1)	10.00
AT-AU	0 / 8304	-28.0	-28.0	0.85 (1)	10.00
AU- Q	0 / 8304	-28.0	-28.0	0.85 (1)	10.00
Q-AV	0 / 6336	-28.0	-28.0	0.67 (1)	10.00
AV-AW	0 / 6336	-28.0	-28.0	0.67 (1)	10.00
AW- P	0 / 6336	-28.0	-28.0	0.67 (1)	10.00
O- P	0 / 98	0.0	0.0	0.28 (1)	10.00
P- K	0 / 500	0.0	0.0	0.30 (1)	10.00
O-AX	0 / 291	-28.0	-28.0	0.05 (2)	10.00
AX- N	0 / 291	-28.0	-28.0	0.05 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-531	-531	---	FRONT	VERT	TOTAL
H	21-11-4	-126	-126	---	FRONT	VERT	TOTAL
J	29-0-8	-464	-464	---	FRONT	VERT	TOTAL
U	15-11-4	-41	-72	---	FRONT	VERT	TOTAL
X	5-11-4	-41	-72	---	FRONT	VERT	TOTAL
Z	7-11-4	-126	-126	---	FRONT	VERT	TOTAL
AA	9-11-4	-126	-126	---	FRONT	VERT	TOTAL
AB	11-11-4	-126	-126	---	FRONT	VERT	TOTAL
AC	13-11-4	-126	-126	---	FRONT	VERT	TOTAL
AD	15-11-4	-126	-126	---	FRONT	VERT	TOTAL
AE	17-11-4	-126	-126	---	FRONT	VERT	TOTAL
AF	19-11-4	-126	-126	---	FRONT	VERT	TOTAL
AG	23-10-4	-207	-207	---	FRONT	VERT	TOTAL
AH	25-10-4	-207	-207	---	FRONT	VERT	TOTAL
AI	27-10-4	-207	-207	---	FRONT	VERT	TOTAL
AJ	1-11-4	-40	-76	---	FRONT	VERT	TOTAL
AK	3-11-4	-41	-72	---	FRONT	VERT	TOTAL
AL	7-11-4	-41	-72	---	FRONT	VERT	TOTAL
AM	9-11-4	-41	-72	---	FRONT	VERT	TOTAL
AN	11-11-4	-41	-72	---	FRONT	VERT	TOTAL
AO	13-11-4	-41	-72	---	FRONT	VERT	TOTAL
AP	17-11-4	-41	-72	---	FRONT	VERT	TOTAL
AQ	19-11-4	-41	-72	---	FRONT	VERT	TOTAL
AR	21-11-4	-41	-72	---	FRONT	VERT	TOTAL
AS	23-10-4	-35	-62	---	FRONT	VERT	TOTAL
AT	25-10-4	-34	-59	---	FRONT	VERT	TOTAL
AU	27-10-4	-34	-59	---	FRONT	VERT	TOTAL
AV	29-10-4	-34	-59	---	FRONT	VERT	TOTAL
AW	31-10-4	-34	-59	---	FRONT	VERT	TOTAL
AX	33-10-4	-49	-86	---	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (W) (INPUT = 0.90)
JSI METAL= 0.91 (U) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13254-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287646	T22S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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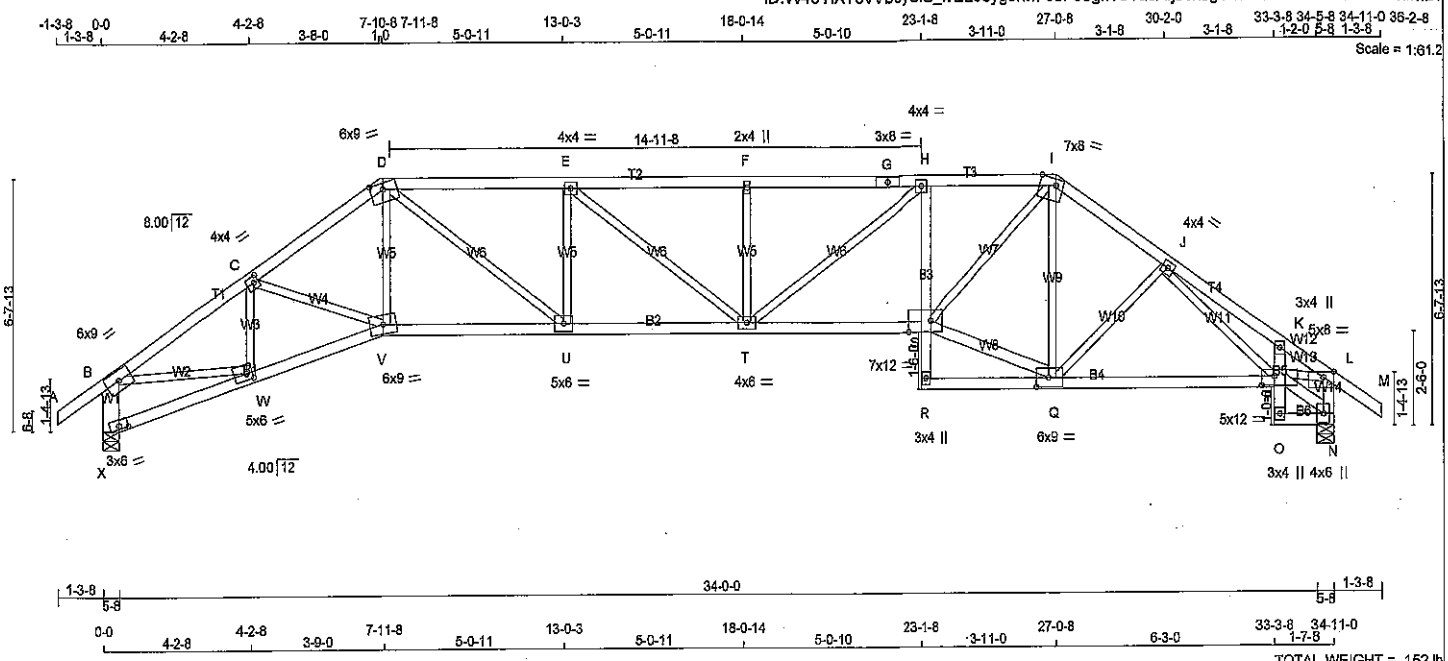

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - Q	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	6.0	9.0	2.00	4.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	7.0	8.0	Edge	5.50
J	TMWW-t	MT20	4.0	4.0		
K	TMV+p	MT20	3.0	4.0		
L	TMWW-p	MT20	5.0	8.0	1.75	3.50
N	BMVW1+p	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-l	MT20	5.0	12.0	3.00	4.25
Q	BMWWW-t	MT20	6.0	9.0	3.00	4.25
R	BMV+p	MT20	3.0	4.0		
S	BVMWW-l	MT20	7.0	12.0	3.50	7.75
T	BMWWW-t	MT20	4.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BBWW-m	MT20	6.0	9.0		
W	BMWW-t	MT20	5.0	6.0	2.00	2.00
X	BVM1-l	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2462	0	2462	0	0	5-8	5-8
N	2468	0	2468	0	0	5-8	5-8

ALLOW FOR 0.5" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD SIMPSON TC26 ex4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1945	1221 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
N	1949	1225 / 0	367 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-4005 / 0	-104.9	-104.9	0.50 (1)	3.17
C- D	-4439 / 0	-104.9	-104.9	0.51 (1)	2.98
D- E	-4996 / 0	-104.9	-104.9	0.85 (1)	2.50
E- F	-5266 / 0	-104.9	-104.9	0.90 (1)	2.37
F- G	-5266 / 0	-104.9	-104.9	0.78 (1)	2.54
G- H	-5266 / 0	-104.9	-104.9	0.78 (1)	2.54
H- I	-4748 / 0	-104.9	-104.9	0.56 (1)	2.80
I- J	-3218 / 0	-104.9	-104.9	0.33 (1)	3.63
J- K	-3482 / 0	-104.9	-104.9	0.35 (1)	3.48
K- L	-3432 / 0	-104.9	-104.9	0.26 (1)	3.58
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
X- B	-2404 / 0	0.0	0.0	0.17 (1)	6.65
N- L	-2354 / 0	0.0	0.0	0.24 (1)	5.57
X- W	0 / 6	-28.0	-28.0	0.11 (2)	10.00
W- V	0 / 3506	-28.0	-28.0	0.62 (1)	10.00
V- U	0 / 3680	-28.0	-28.0	0.69 (1)	10.00
U- T	0 / 4998	-28.0	-28.0	0.87 (1)	10.00
T- S	0 / 4771	-28.0	-28.0	0.87 (1)	10.00
R- S	0 / 41	0.0	0.0	0.12 (1)	10.00
S- H	-862 / 0	0.0	0.0	0.19 (1)	7.81
R- Q	0 / 56	-28.0	-28.0	0.26 (2)	10.00
Q- P	0 / 2930	-28.0	-28.0	0.62 (1)	10.00
O- P	0 / 36	0.0	0.0	0.19 (1)	10.00
P- K	-136 / 6	0.0	0.0	0.19 (1)	7.81
O- N	0 / 138	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- I	-623 / 0	0.30 (1)
V- D	0 / 1091	0.25 (1)
W- C	-834 / 0	0.15 (1)
B- W	0 / 3353	0.75 (1)
C- V	0 / 375	0.08 (1)
S- Q	0 / 2809	0.45 (1)
S- I	0 / 2973	0.67 (1)
P- N	-166 / 0	0.02 (1)
P- L	0 / 2806	0.63 (1)
J- P	-6 / 81	0.02 (3)
Q- J	-388 / 0	0.14 (1)
U- E	-834 / 0	0.22 (1)
T- F	-522 / 0	0.14 (1)
D- U	0 / 1644	0.37 (1)
E- T	0 / 366	0.08 (1)
T- H	0 / 647	0.15 (1)

LICENSED PROFESSIONAL ENGINEER 3826 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 940 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 607 (0.69")

CSI: TC=0.90/1.00 (E-F:1) , BC=0.87/1.00 (T-U:1) , WB=0.75/1.00 (B-W:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.87 (W) (INPUT = 1.00)

DWG NO. TAM 1325618
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T23S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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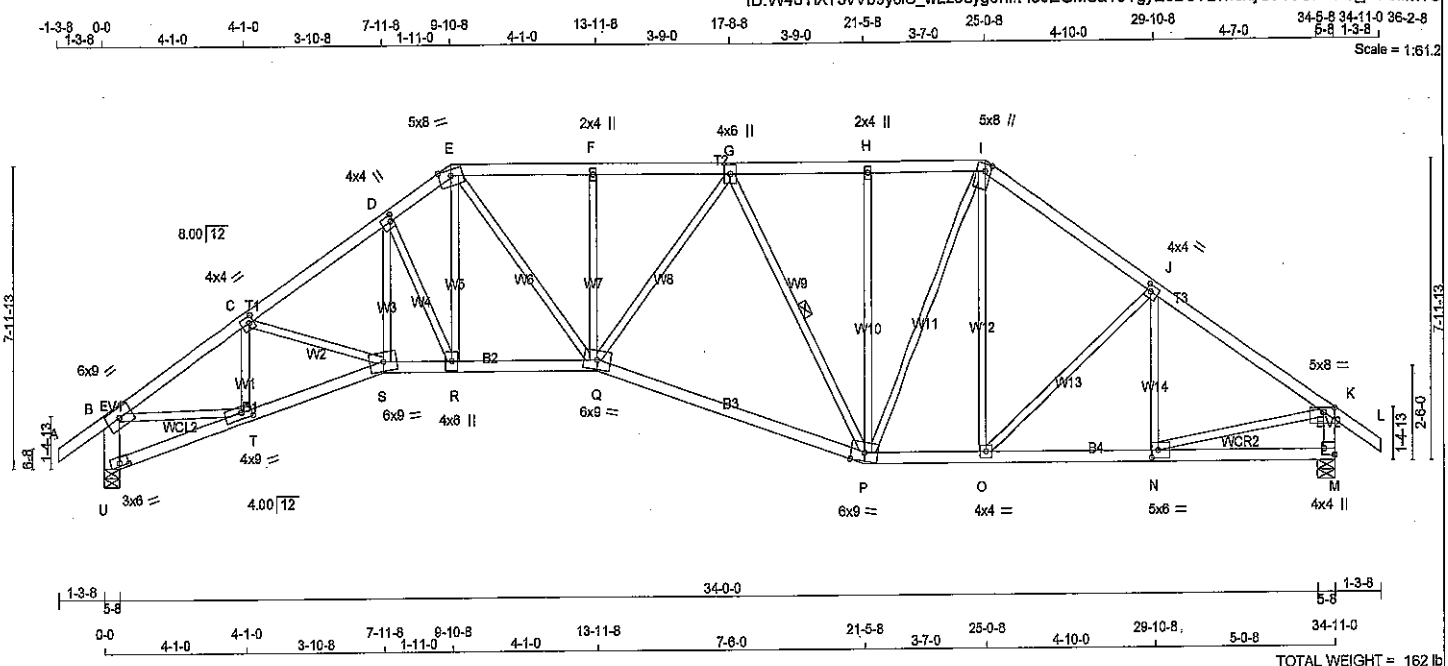

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
BEARING BLOCKS				
EV1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMKW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW+t	MT20	4.0	4.0	2.00	1.00
E	TTWW-m	MT20	5.0	8.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	5.0	8.0	2.25	2.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMWW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	4.0	4.0	2.00	Edge
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	4.0	4.0		
P	BBWWW-m	MT20	6.0	9.0	Edge	
Q	BBWWW-m	MT20	6.0	9.0		
R	BMWW+t	MT20	4.0	6.0		
S	BBWW-m	MT20	6.0	9.0		
T	BMWW-t	MT20	4.0	9.0	2.00	3.25
U	BKM1-I	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD, __

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2465	0	2465	0	0	5-8	5-8
M	2465	0	2465	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD 06/

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
M	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-3941 / 0	-104.9	-104.9	0.41 (1)	3.27
C- D	-4439 / 0	-104.9	-104.9	0.42 (1)	3.08
D- E	-3831 / 0	-104.9	-104.9	0.21 (1)	3.47
E- F	-3779 / 0	-104.9	-104.9	0.36 (1)	3.36
F- G	-3770 / 0	-104.9	-104.9	0.32 (1)	3.41
G- H	-2512 / 0	-104.9	-104.9	0.22 (1)	4.17
H- I	-2517 / 0	-104.9	-104.9	0.22 (1)	4.17
I- J	-2615 / 0	-104.9	-104.9	0.43 (1)	3.90
J- K	-2771 / 0	-104.9	-104.9	0.45 (1)	3.78
K- L	0 / 40	-104.9	-104.9	0.14 (1)	10.00
U- B	-2409 / 0	0.0	0.0	0.17 (1)	6.64
M- K	-2404 / 0	0.0	0.0	0.25 (1)	5.52
U- T	0 / 6	-28.0	-28.0	0.10 (3)	10.00
T- S	0 / 3473	-28.0	-28.0	0.63 (1)	10.00
S- R	0 / 3656	-28.0	-28.0	0.65 (1)	10.00
R- Q	0 / 3184	-28.0	-28.0	0.57 (1)	10.00
Q- P	0 / 3301	-28.0	-28.0	0.82 (2)	10.00
P- O	0 / 2148	-28.0	-28.0	0.41 (1)	10.00
O- N	0 / 2333	-28.0	-28.0	0.44 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-870 / 0	0.15 (1)
C- S	0 / 408	0.09 (1)
S- D	0 / 1032	0.23 (1)
D- R	-1133 / 0	0.36 (1)
R- E	0 / 1127	0.25 (1)
E- Q	0 / 960	0.22 (1)
Q- F	-439 / 0	0.20 (1)
Q- G	0 / 1089	0.25 (1)
G- P	-1415 / 0	0.72 (1)
P- H	-422 / 0	0.51 (1)
P- I	0 / 874	0.20 (1)
O- I	0 / 327	0.07 (2)
O- J	-254 / 0	0.20 (1)
N- J	-350 / 65	0.12 (1)
B- T	0 / 3298	0.74 (1)
N- K	0 / 2385	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 667 (0.63")

CSI: TC=0.45/1.00 (J-K:1) , BC=0.82/1.00 (P-Q:2) , WB=0.74/1.00 (B-T:1) , SSI=0.21/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.69 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3626
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13229 R8
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T24	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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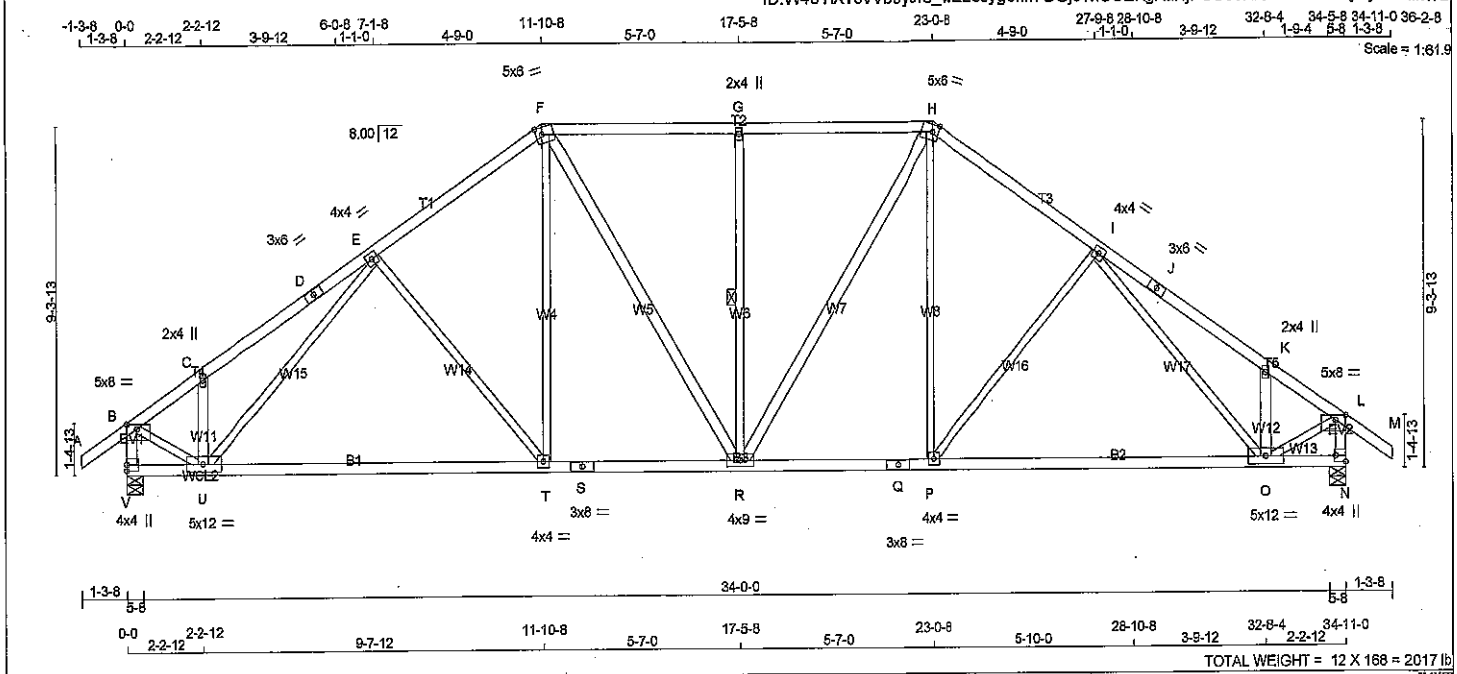

TOTAL WEIGHT = 12 X 168 = 2017 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C, G, K						
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0		
J	TS-t	MT20	3.0	6.0		
L	TMVW-p	MT20	5.0	8.0	1.50	3.50
N	BMV1+p	MT20	4.0	4.0	2.00	Edge
O	BMWWW-t	MT20	5.0	12.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWWW-t	MT20	5.0	12.0		
V	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2465	0	2465	0	0	5-8	5-8
N	2465	0	2465	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
N	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2380 / 0	-104.9	-104.9	0.16 (1)	4.31
C- D	-2377 / 0	-104.9	-104.9	0.38 (1)	4.12
D- E	-2377 / 0	-104.9	-104.9	0.38 (1)	4.12
E- F	-2476 / 0	-104.9	-104.9	0.39 (1)	4.03
F- G	-2289 / 0	-104.9	-104.9	0.49 (1)	4.00
G- H	-2289 / 0	-104.9	-104.9	0.49 (1)	4.00
H- I	-2476 / 0	-104.9	-104.9	0.39 (1)	4.03
I- J	-2377 / 0	-104.9	-104.9	0.38 (1)	4.12
J- K	-2377 / 0	-104.9	-104.9	0.38 (1)	4.12
K- L	-2380 / 0	-104.9	-104.9	0.16 (1)	4.31
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
V- B	-2511 / 0	0.0	0.0	0.26 (1)	5.41
N- L	-2511 / 0	0.0	0.0	0.26 (1)	5.41
V- U	0 / 0	-28.0	-28.0	0.46 (3)	10.00
U- T	0 / 2280	-28.0	-28.0	0.69 (2)	10.00
T- S	0 / 2040	-28.0	-28.0	0.66 (2)	10.00
S- R	0 / 2040	-28.0	-28.0	0.66 (2)	10.00
R- Q	0 / 2040	-28.0	-28.0	0.66 (2)	10.00
Q- P	0 / 2040	-28.0	-28.0	0.66 (2)	10.00
P- O	0 / 2280	-28.0	-28.0	0.69 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.46 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- F	0 / 547	0.12 (2)
F- R	0 / 472	0.08 (1)
R- G	-715 / 0	0.41 (1)
R- H	0 / 472	0.08 (1)
P- H	0 / 547	0.12 (2)
B- U	0 / 2227	0.50 (1)
U- C	-361 / 0	0.05 (1)
O- K	-361 / 0	0.05 (1)
O- L	0 / 2227	0.50 (1)
E- T	-376 / 0	0.44 (1)
U- E	-450 / 16	0.50 (1)
P- I	-376 / 0	0.44 (1)
I- O	-450 / 16	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.69/1.00 (T-U:2) , WB=0.50/1.00 (E-U:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (V) (INPUT = 0.90)
JSI METAL= 0.72 (Q) (INPUT = 1.00)

PROFESSIONAL ENGINEER
LICENSED
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM13230-B
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287646	T24S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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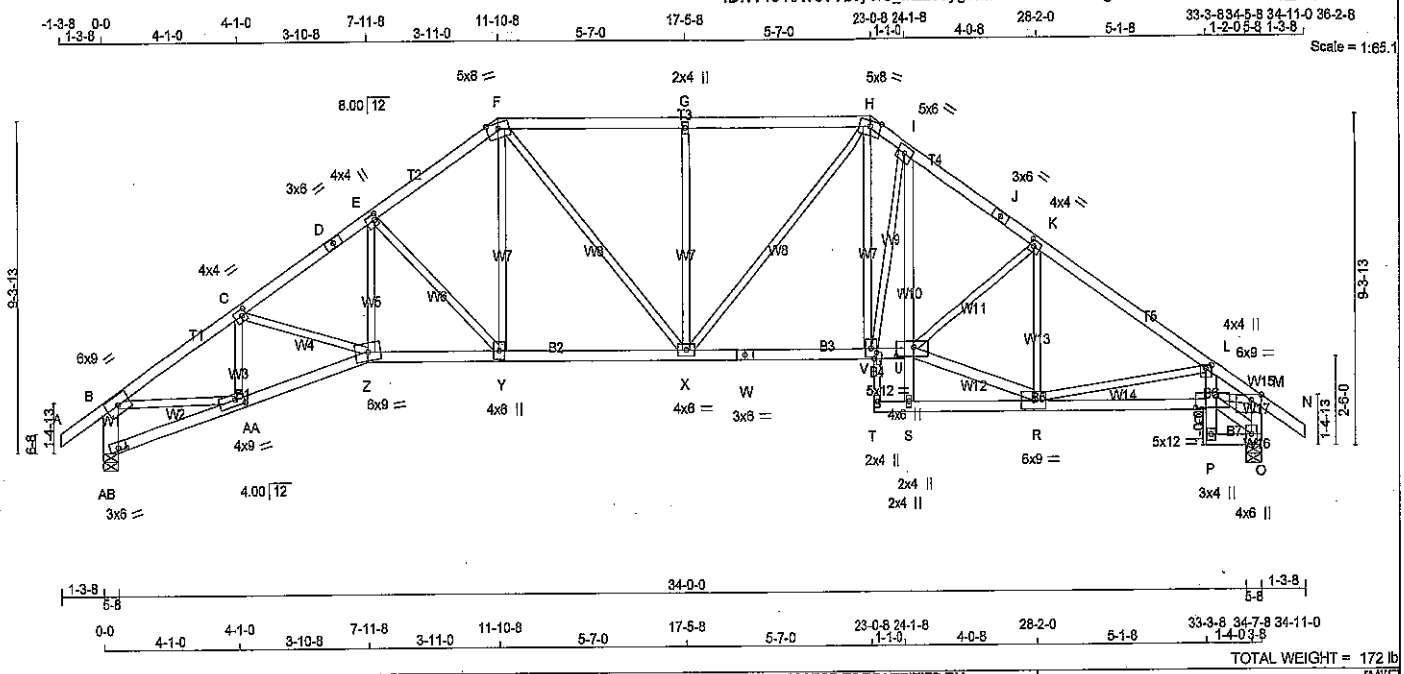

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
AB- Z	2x4	DRY	No.2	SPF
Z - W	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - I	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	4.0	2.00	1.00
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	4.0	4.0	2.00	1.50
L	TMW+p	MT20	4.0	4.0	1.25	2.00
M	TMW-p	MT20	6.0	9.0	Edge	3.50
O	BMW1+p	MT20	4.0	6.0		
P	BMV+p	MT20	3.0	4.0		
Q	BVMWW-l	MT20	5.0	12.0	3.75	7.25
R	BMWWW-t	MT20	6.0	9.0		
S	BMW+w	MT20	2.0	4.0		
T	NP+w	MT20	2.0	4.0		
U	BWMWW*-l	MT20	5.0	12.0	3.00	7.00
V	BMWW+t	MT20	4.0	6.0		
V	NP+w	MT20	2.0	4.0	2.00	1.00
W	BS-t	MT20	3.0	6.0		
X	BMWWW-t	MT20	4.0	6.0		
Y	BMWW+t	MT20	4.0	6.0		
Z	BBWW-m	MT20	6.0	9.0		
AA	BMWW-t	MT20	4.0	9.0	2.00	3.25
AB	BVM1-l	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	2472	0	2472	0	0	5-8	5-8
O	2490	0	2490	0	0	5-8	5-8

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

Simpson Force @ AB

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	1957	1221 / 0	374 / 0	0 / 0	0 / 0	362 / 0	0 / 0
O	1977	1225 / 0	393 / 0	0 / 0	0 / 0	369 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-3948 / 0	-104.9	-104.9	0.42 (1)	3.25
C- D	-4466 / 0	-104.9	-104.9	0.41 (1)	3.09
D- E	-4466 / 0	-104.9	-104.9	0.41 (1)	3.09
E- F	-3390 / 0	-104.9	-104.9	0.32 (1)	3.58
F- G	-3191 / 0	-104.9	-104.9	0.57 (1)	3.38
G- H	-3191 / 0	-104.9	-104.9	0.57 (1)	3.38
H- I	-3437 / 0	-104.9	-104.9	0.14 (1)	3.71
I- J	-3727 / 0	-104.9	-104.9	0.47 (1)	3.27
J- K	-3727 / 0	-104.9	-104.9	0.47 (1)	3.27
K- L	-3401 / 0	-104.9	-104.9	0.52 (1)	3.42
L- M	-4007 / 0	-104.9	-104.9	0.27 (1)	3.34
M- N	0 / 40	-104.9	-104.9	0.14 (1)	10.00
AB- B	-2416 / 0	0.0	0.0	0.17 (1)	6.64
O- M	-2346 / 0	0.0	0.0	0.24 (1)	5.58
AB-AA	0 / 6	-28.0	-28.0	0.10 (3)	10.00
AA- Z	0 / 3477	-28.0	-28.0	0.63 (1)	10.00
Z- Y	0 / 3688	-28.0	-28.0	0.66 (1)	10.00
Y- X	0 / 2816	-28.0	-28.0	0.54 (1)	10.00
X- W	0 / 2831	-28.0	-28.0	0.59 (1)	10.00
W- V	0 / 2831	-28.0	-28.0	0.59 (1)	10.00
V- U	0 / 3036	-28.0	-28.0	0.62 (1)	10.00
T- S	0 / 0	-28.0	-28.0	0.04 (2)	10.00
S- R	0 / 29	-28.0	-28.0	0.18 (3)	10.00
R- Q	0 / 3520	-28.0	-28.0	0.66 (1)	10.00
P- Q	0 / 36	0.0	0.0	0.25 (1)	10.00
Q- L	0 / 191	0.0	0.0	0.27 (1)	10.00
P- O	0 / 161	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-872 / 0	0.15 (1)
C- Z	0 / 439	0.10 (1)
Z- E	0 / 1055	0.24 (1)
E- Y	-1263 / 0	0.66 (1)
Y- F	0 / 1042	0.23 (1)
R- K	-918 / 0	0.33 (1)
R- L	-686 / 0	0.34 (1)
B-AA	0 / 3302	0.74 (1)
S- U	0 / 119	0.07 (1)
U- I	0 / 1105	0.24 (1)
U- R	0 / 3035	0.49 (1)
U- K	0 / 281	0.06 (1)
Q- O	-203 / 0	0.02 (1)
Q- M	0 / 3327	0.75 (1)
V- H	0 / 1221	0.27 (1)
V- I	-1200 / 0	0.74 (1)
X- G	-708 / 0	0.55 (1)
F- X	0 / 576	0.13 (1)
X- H	0 / 553	0.12 (1)

PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 963 (0.43")

CSI: TC=0.57/1.00 (F-G:1) , BC=0.68/1.00 (Y-Z:1) , WB=0.75/1.00 (M-Q:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.75 (W) (INPUT = 1.00)

DWG NO. TAM 13258-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T24C	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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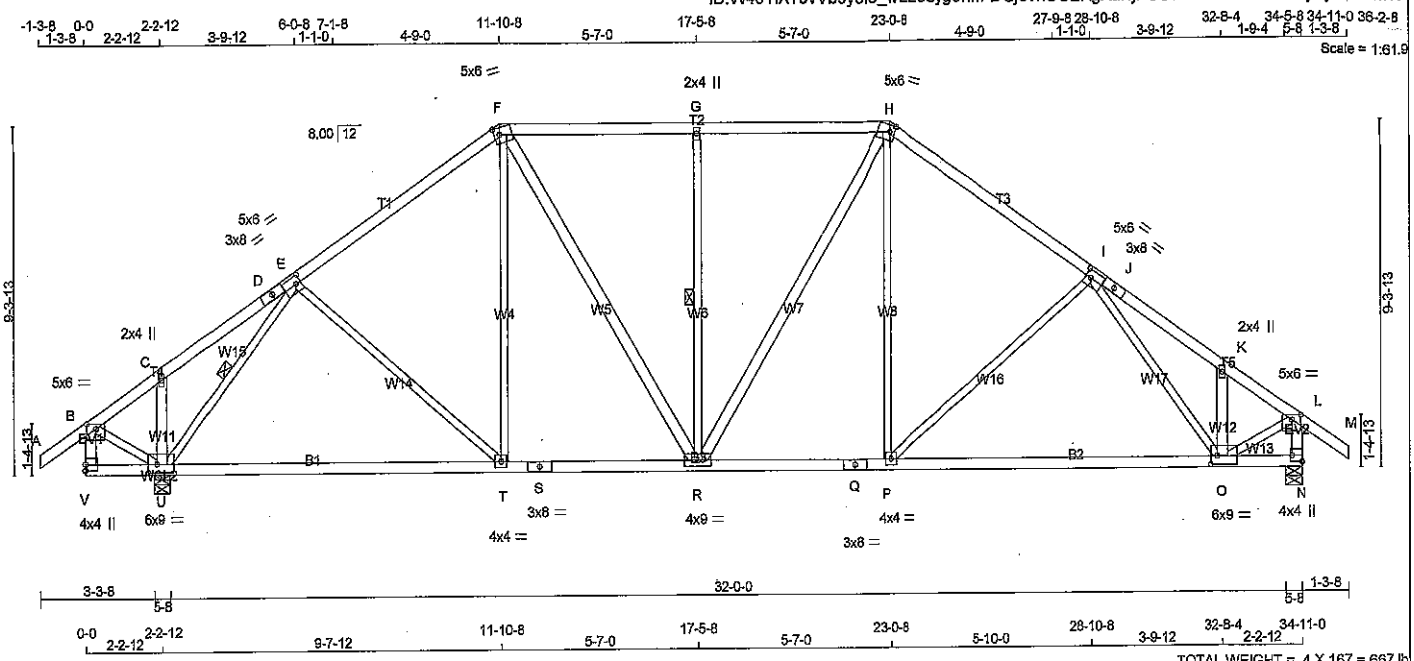

[M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C, G, K						
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	5.0	6.0	2.50	1.75
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
H	TTWW-m	MT20	5.0	6.0	2.25	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TS-t	MT20	3.0	8.0		
L	TMVW-p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	4.0	4.0	2.00	Edge
O	BMWWW-t	MT20	6.0	9.0	2.75	2.25
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWWW1-t	MT20	6.0	9.0	2.75	3.25
V	BMV+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2633	0	2633	0	0	5-8	5-8
N	2297	0	2297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2080	1307 / 0	392 / 0	0 / 0	0 / 0	382 / 0	0 / 0
N	1814	1140 / 0	342 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R, E-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-94 / 176	-104.9	-104.9	0.15 (1)	6.25
C- D	-98 / 194	-104.9	-104.9	0.45 (1)	6.25
D- E	-98 / 194	-104.9	-104.9	0.45 (1)	6.25
E- F	-1965 / 0	-104.9	-104.9	0.51 (1)	4.28
F- G	-1964 / 0	-104.9	-104.9	0.47 (1)	4.30
G- H	-1964 / 0	-104.9	-104.9	0.47 (1)	4.30
H- I	-2212 / 0	-104.9	-104.9	0.52 (1)	4.07
I- J	-2136 / 0	-104.9	-104.9	0.41 (1)	4.24
J- K	-2136 / 0	-104.9	-104.9	0.41 (1)	4.24
K- L	-2185 / 0	-104.9	-104.9	0.11 (1)	4.54
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
V- B	-107 / 0	0.0	0.0	0.01 (2)	7.81
N- L	-2345 / 0	0.0	0.0	0.24 (1)	5.58
V- U	0 / 0	-28.0	-28.0	0.49 (2)	10.00
U- T	0 / 1358	-28.0	-28.0	0.63 (2)	10.00
T- S	0 / 1608	-28.0	-28.0	0.60 (2)	10.00
S- R	0 / 1608	-28.0	-28.0	0.60 (2)	10.00
R- Q	0 / 1818	-28.0	-28.0	0.64 (2)	10.00
Q- P	0 / 1818	-28.0	-28.0	0.64 (2)	10.00
P- O	0 / 2109	-28.0	-28.0	0.67 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.46 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- F	0 / 271	0.06 (3)
F- R	0 / 875	0.11 (1)
R- G	-717 / 0	0.41 (1)
R- H	0 / 277	0.04 (1)
P- H	0 / 540	0.12 (2)
B- U	-140 / 91	0.02 (1)
U- C	-297 / 0	0.04 (1)
O- K	-242 / 0	0.03 (1)
O- L	0 / 2024	0.46 (1)
E- T	0 / 330	0.07 (1)
U- E	-2498 / 0	0.70 (1)
P- I	-391 / 0	0.50 (1)
I- O	-493 / 41	0.35 (1)

LICENSED PROFESSIONAL ENGINEER
3976
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 987 (0.40")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.52/1.00 (H-I:1) , BC=0.67/1.00 (O-P:2) , WB=0.70/1.00 (E-U:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.68 (Q) (INPUT = 1.00)

DWG NO. TAM 1323/08
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T24CS	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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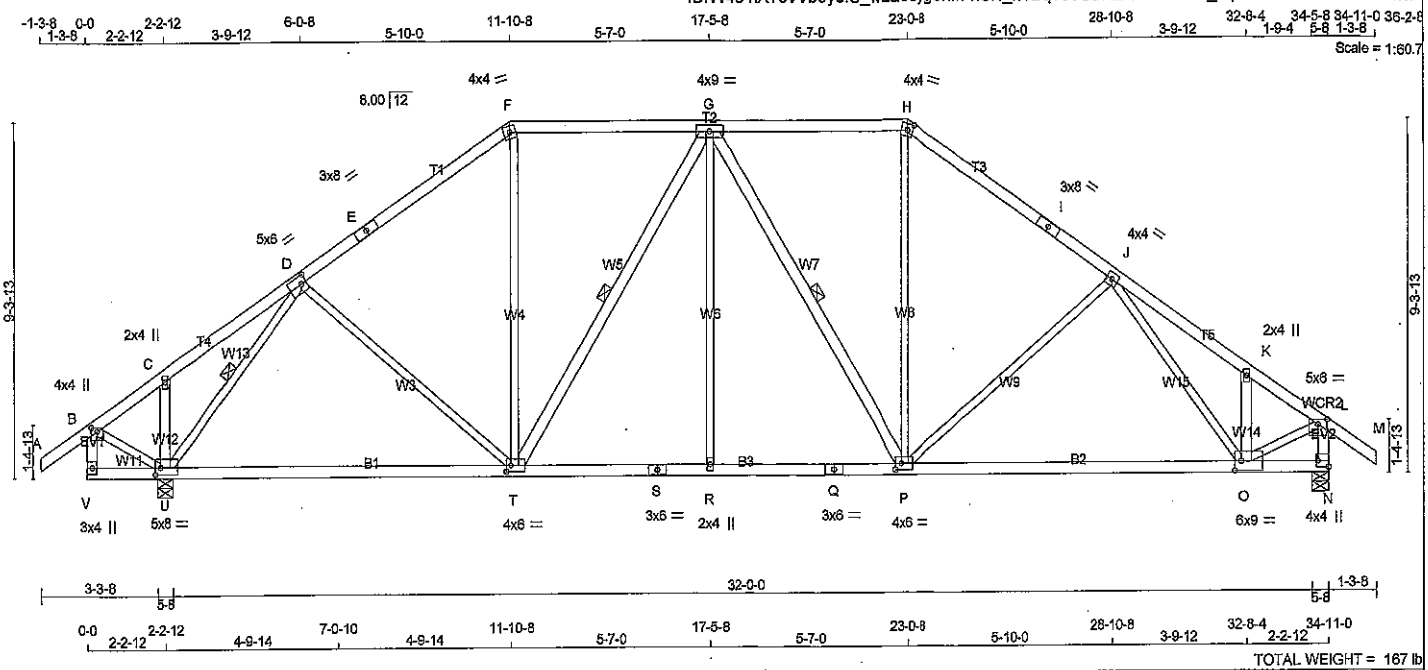

TOTAL WEIGHT = 167 lb

[M]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - G	2x4	DRY	No.2	SPF
G - P	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	5.0	6.0	2.50	1.75
E	TS-t	MT20	3.0	8.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWWW-t	MT20	4.0	9.0		
H	TTW-m	MT20	4.0	4.0	2.00	1.75
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	4.0		
K	TMW+w	MT20	2.0	4.0		
L	TMVW-p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	4.0	4.0	2.00	Edge
O	BMWWW-t	MT20	6.0	9.0	3.00	2.00
P	BMWWW-t	MT20	4.0	6.0	2.00	2.00
Q	BS-t	MT20	3.0	6.0		
R	BMW+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
T	BMWWW-t	MT20	4.0	6.0	2.00	1.50
U	BMWWW1-t	MT20	5.0	8.0	2.25	1.75
V	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2297	0	2297	0	0	5-8	5-8
U	2633	0	2633	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1814	1140 / 0	342 / 0	0 / 0	0 / 0	333 / 0	0 / 0
U	2080	1307 / 0	392 / 0	0 / 0	0 / 0	382 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T, G-P, D-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-93 / 176	-104.9	-104.9	0.15 (1)	6.25
C- D	-97 / 195	-104.9	-104.9	0.46 (1)	6.25
D- E	-1965 / 0	-104.9	-104.9	0.51 (1)	4.28
E- F	-1965 / 0	-104.9	-104.9	0.51 (1)	4.28
F- G	-1615 / 0	-104.9	-104.9	0.45 (1)	4.68
G- H	-1821 / 0	-104.9	-104.9	0.47 (1)	4.44
H- I	-2212 / 0	-104.9	-104.9	0.53 (1)	4.06
I- J	-2212 / 0	-104.9	-104.9	0.53 (1)	4.06
J- K	-2148 / 0	-104.9	-104.9	0.41 (1)	4.22
K- L	-2205 / 0	-104.9	-104.9	0.11 (1)	4.52
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
V- B	-107 / 0	0.0	0.0	0.01 (2)	7.81
N- L	-2339 / 0	0.0	0.0	0.24 (1)	5.58
V- U	0 / 0	-28.0	-28.0	0.49 (2)	10.00
U- T	0 / 1359	-28.0	-28.0	0.63 (2)	10.00
T- S	0 / 1964	-28.0	-28.0	0.64 (2)	10.00
S- R	0 / 1964	-28.0	-28.0	0.64 (2)	10.00
R- Q	0 / 1964	-28.0	-28.0	0.65 (2)	10.00
Q- P	0 / 1964	-28.0	-28.0	0.65 (2)	10.00
P- O	0 / 2110	-28.0	-28.0	0.66 (2)	10.00
O- N	0 / 0	-28.0	-28.0	0.45 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- T	0 / 329	0.07 (1)
T- F	0 / 597	0.13 (1)
T- G	-672 / 0	0.40 (1)
R- G	0 / 159	0.04 (2)
G- P	-275 / 0	0.16 (1)
P- H	0 / 737	0.17 (1)
P- J	-393 / 0	0.50 (1)
O- L	0 / 2032	0.46 (1)
B- U	-141 / 90	0.02 (1)
U- C	-297 / 0	0.04 (1)
U- D	-2499 / 0	0.70 (1)
O- K	-236 / 0	0.03 (1)
J- O	-482 / 42	0.35 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.53/1.00 (H-J:1) , BC=0.66/1.00 (O-P:2) , WB=0.70/1.00 (D-U:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.61 (D) (INPUT = 1.00)

DWG NO. TAM 1323248
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T24S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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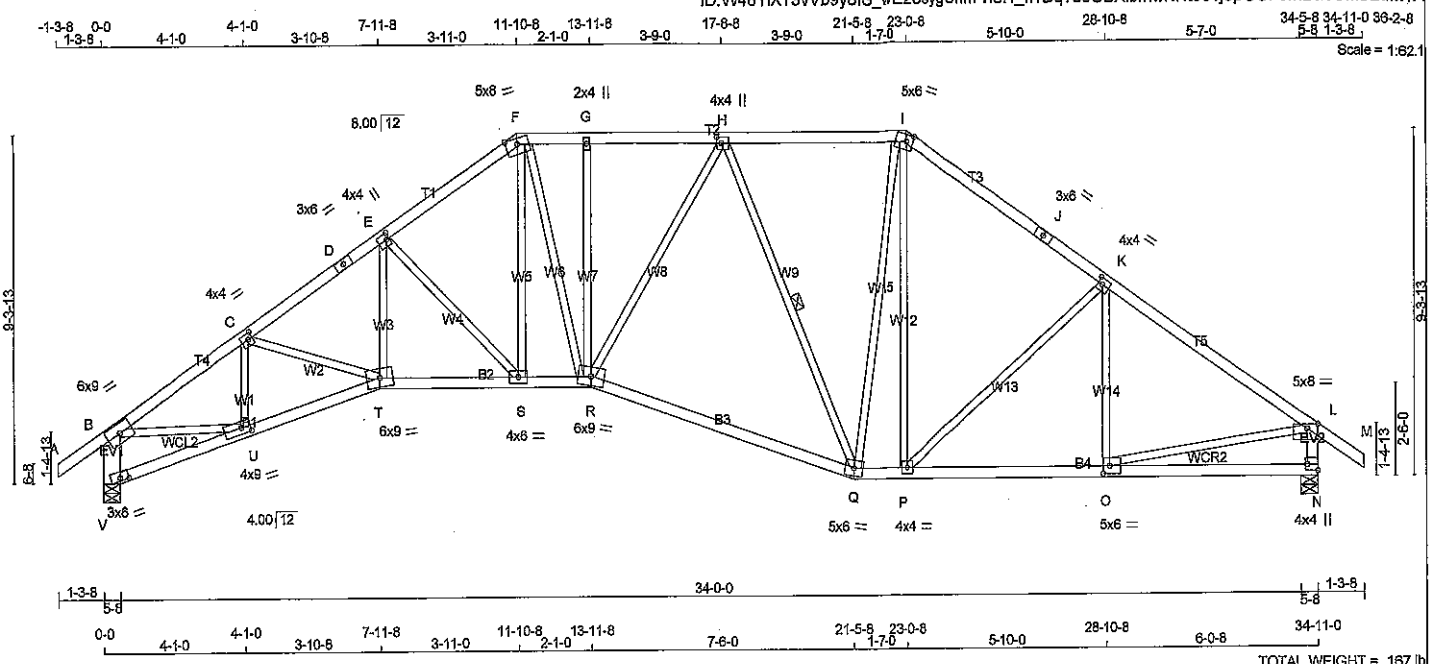

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	4.0	2.00	1.00
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	4.0	2.00	1.50
I	TTWW-m	MT20	5.0	6.0	2.25	2.25
J	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	4.0	4.0	2.00	1.50
L	TMW-p	MT20	5.0	8.0	1.50	3.50
N	BMV1+p	MT20	4.0	4.0	2.00	Edge
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-m	MT20	5.0	6.0		
R	BBWWW-m	MT20	6.0	9.0		
S	BMWW-t	MT20	4.0	6.0		
T	BBWW-m	MT20	6.0	9.0		
U	BMWW-t	MT20	4.0	9.0	2.00	3.25
V	BVM1-l	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2465	0	2465	0	0	5-8	5-8
N	2465	0	2465	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
N	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-3935 / 0	-104.9	-104.9	0.42 (1)	3.25
C- D	-4449 / 0	-104.9	-104.9	0.41 (1)	3.09
D- E	-4449 / 0	-104.9	-104.9	0.41 (1)	3.09
E- F	-3369 / 0	-104.9	-104.9	0.32 (1)	3.59
F- G	-2986 / 0	-104.9	-104.9	0.14 (1)	3.96
G- H	-2963 / 0	-104.9	-104.9	0.39 (1)	3.68
H- I	-2105 / 0	-104.9	-104.9	0.39 (1)	4.30
I- J	-2473 / 0	-104.9	-104.9	0.61 (1)	3.77
J- K	-2473 / 0	-104.9	-104.9	0.61 (1)	3.77
K- L	-2813 / 0	-104.9	-104.9	0.67 (1)	3.50
L- M	0 / 40	-104.9	-104.9	0.14 (1)	10.00
V- B	-2409 / 0	0.0	0.0	0.17 (1)	6.64
N- L	-2395 / 0	0.0	0.0	0.25 (1)	5.52
V- U	0 / 6	-28.0	-28.0	0.10 (3)	10.00
U- T	0 / 3465	-28.0	-28.0	0.62 (1)	10.00
T- S	0 / 3674	-28.0	-28.0	0.65 (1)	10.00
S- R	0 / 2799	-28.0	-28.0	0.49 (1)	10.00
R- Q	0 / 2751	-28.0	-28.0	0.76 (2)	10.00
Q- P	0 / 2025	-28.0	-28.0	0.40 (1)	10.00
P- O	0 / 2375	-28.0	-28.0	0.50 (1)	10.00
O- N	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-888 / 0	0.15 (1)
C- T	0 / 436	0.10 (1)
T- E	0 / 1059	0.24 (1)
E- S	-1267 / 0	0.66 (1)
S- F	0 / 1000	0.22 (1)
F- R	0 / 616	0.14 (1)
R- G	-234 / 0	0.18 (1)
R- H	0 / 740	0.17 (1)
H- Q	-1331 / 0	0.92 (1)
P- I	0 / 438	0.10 (1)
P- K	-468 / 0	0.59 (1)
O- K	-237 / 160	0.11 (1)
B- U	0 / 3291	0.74 (1)
O- L	0 / 2412	0.54 (1)
Q- I	0 / 461	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 706 (0.59")

CSI: TC=0.67/1.00 (K-L:1) , BC=0.76/1.00 (Q-R:2) , WB=0.92/1.00 (H-Q:1) , SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.69 (B) (INPUT = 1.00)

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DWG NO. TAM 13233-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T25S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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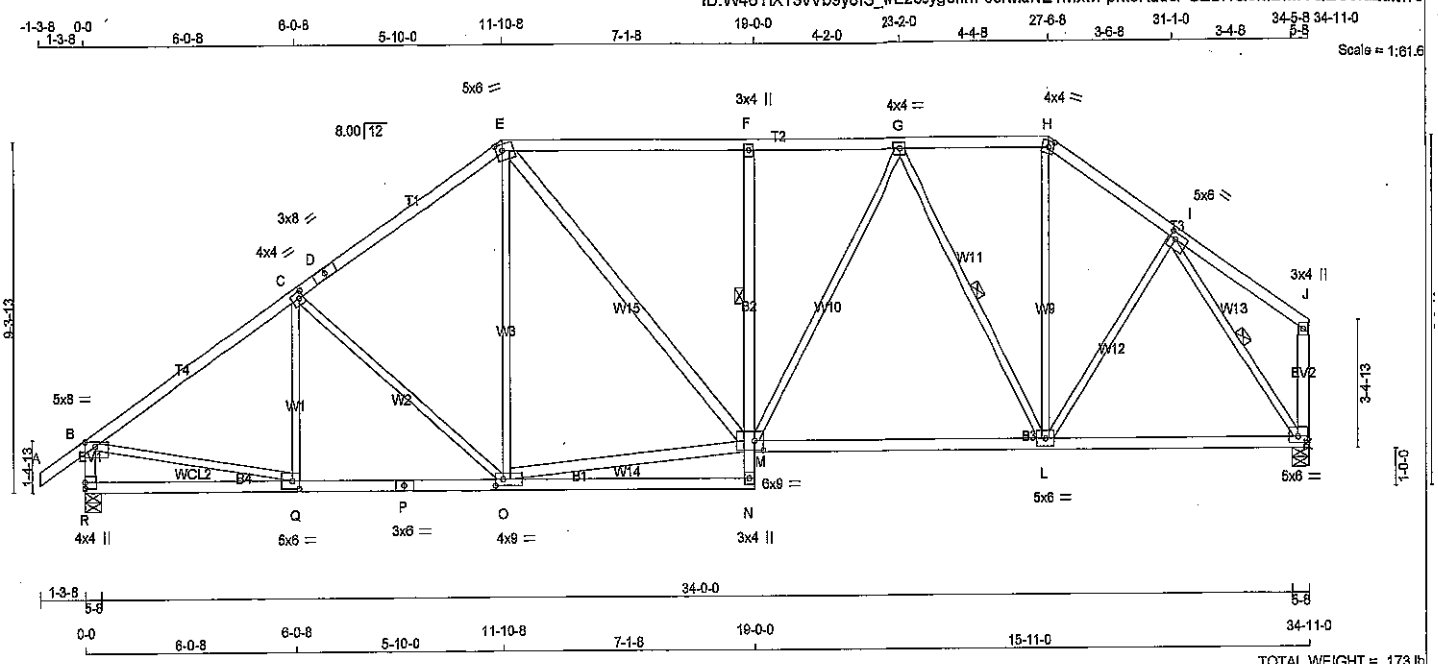

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.00	2.00
J	TMV+p	MT20	3.0	4.0		
K	BMWV1-t	MT20	5.0	6.0	2.00	3.00
L	BMWWW-t	MT20	5.0	6.0		
M	BVMWWW-t	MT20	6.0	9.0	3.00	3.00
N	BMV+p	MT20	3.0	4.0		
O	BMWWW-t	MT20	4.0	9.0	2.00	2.50
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2465	0	2465	0	0	5-8	5-8
K	2321	0	2321	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
K	1850	1134 / 0	367 / 0	0 / 0	0 / 0	349 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-M, G-L, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2809 / 0	-104.9	-104.9	0.67 (1)	3.51	
C- D	-2486 / 0	-104.9	-104.9	0.61 (1)	3.76	
D- E	-2486 / 0	-104.9	-104.9	0.61 (1)	3.76	
E- F	-2556 / 0	-104.9	-104.9	0.75 (1)	3.44	
F- G	-2563 / 0	-104.9	-104.9	0.55 (1)	3.70	
G- H	-1727 / 0	-104.9	-104.9	0.28 (1)	4.79	
H- I	-2090 / 0	-104.9	-104.9	0.21 (1)	4.52	
I- J	0 / 23	-104.9	-104.9	0.21 (1)	10.00	
R- B	-2392 / 0	0.0	0.0	0.25 (1)	5.52	
K- J	-154 / 0	0.0	0.0	0.03 (1)	7.81	
R- Q	0 / 0	-28.0	-28.0	0.21 (2)	10.00	
Q- P	0 / 2371	-28.0	-28.0	0.55 (1)	10.00	
P- O	0 / 2371	-28.0	-28.0	0.55 (1)	10.00	
O- N	0 / 41	-28.0	-28.0	0.30 (2)	10.00	
N- M	0 / 139	0.0	0.0	0.07 (1)	10.00	
M- F	-725 / 0	0.0	0.0	0.11 (1)	6.25	
M- L	0 / 2252	-28.0	-28.0	0.78 (2)	10.00	
L- K	0 / 1397	-28.0	-28.0	0.68 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-255 / 127	0.11 (1)
C- O	-451 / 0	0.57 (1)
O- E	0 / 344	0.06 (2)
L- H	0 / 794	0.18 (1)
B- Q	0 / 2409	0.54 (1)
M- G	0 / 659	0.15 (1)
G- L	-1116 / 0	0.65 (1)
L- I	0 / 569	0.13 (1)
I- K	-2515 / 0	0.76 (1)
O- M	0 / 2011	0.32 (1)
E- M	0 / 802	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.42")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.76/1.00 (L-M:2) , WB=0.76/1.00 (I-K:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13234-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T26S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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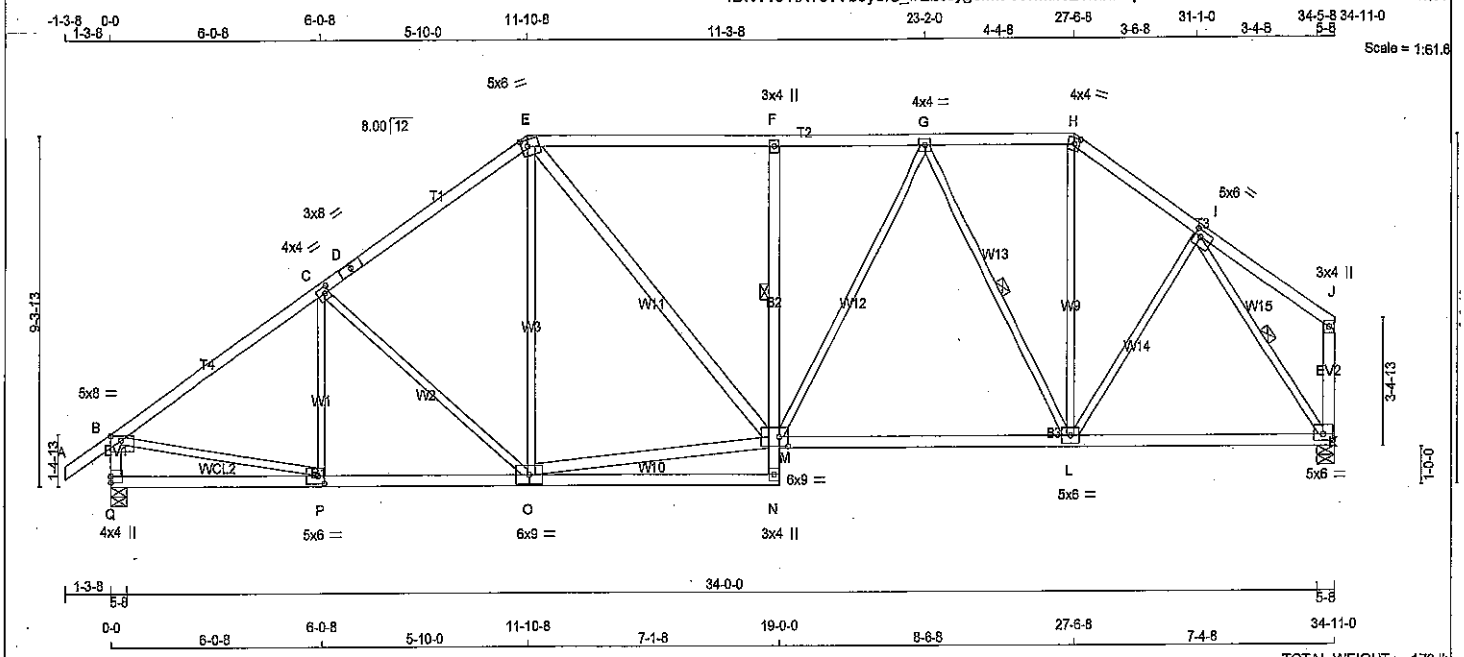

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.00	2.00
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.00	3.00
L	BMWWW-t	MT20	5.0	6.0		
M	BVMWWW-l	MT20	6.0	9.0	3.00	3.00
N	BMV+p	MT20	3.0	4.0		
O	BSWWW-l	MT20	6.0	9.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2465	0	2465	0	0	5-8	5-8
K	2321	0	2321	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1947	1223 / 0	367 / 0	0 / 0	0 / 0	357 / 0	0 / 0
K	1850	1134 / 0	367 / 0	0 / 0	0 / 0	349 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-M, G-L, I-K

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2809 / 0	-104.9	-104.9	0.67 (1)	3.51
C- D	-2466 / 0	-104.9	-104.9	0.61 (1)	3.76
D- E	-2466 / 0	-104.9	-104.9	0.61 (1)	3.76
E- F	-2558 / 0	-104.9	-104.9	0.75 (1)	3.44
F- G	-2563 / 0	-104.9	-104.9	0.55 (1)	3.70
G- H	-1727 / 0	-104.9	-104.9	0.28 (1)	4.79
H- I	-2090 / 0	-104.9	-104.9	0.21 (1)	4.52
I- J	0 / 23	-104.9	-104.9	0.21 (1)	10.00
Q- B	-2392 / 0	0.0	0.0	0.25 (1)	5.52
K- J	-154 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 0	-28.0	-28.0	0.21 (2)	10.00
P- O	0 / 2371	-28.0	-28.0	0.55 (1)	10.00
O- N	0 / 41	-28.0	-28.0	0.30 (2)	10.00
N- M	0 / 139	0.0	0.0	0.07 (1)	10.00
M- F	-725 / 0	0.0	0.0	0.11 (1)	6.25
M- L	0 / 2252	-28.0	-28.0	0.78 (2)	10.00
L- K	0 / 1397	-28.0	-28.0	0.66 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-255 / 127	0.11 (1)
C- O	-451 / 0	0.57 (1)
O- E	0 / 344	0.08 (2)
L- H	0 / 794	0.18 (1)
B- P	0 / 2409	0.54 (1)
O- M	0 / 2011	0.32 (1)
E- M	0 / 802	0.13 (1)
M- G	0 / 659	0.15 (1)
G- L	-1116 / 0	0.65 (1)
L- I	0 / 569	0.13 (1)
I- K	-2515 / 0	0.76 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.42")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.78/1.00 (L-M:2) , WB=0.76/1.00 (I-K:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3076
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13235-B
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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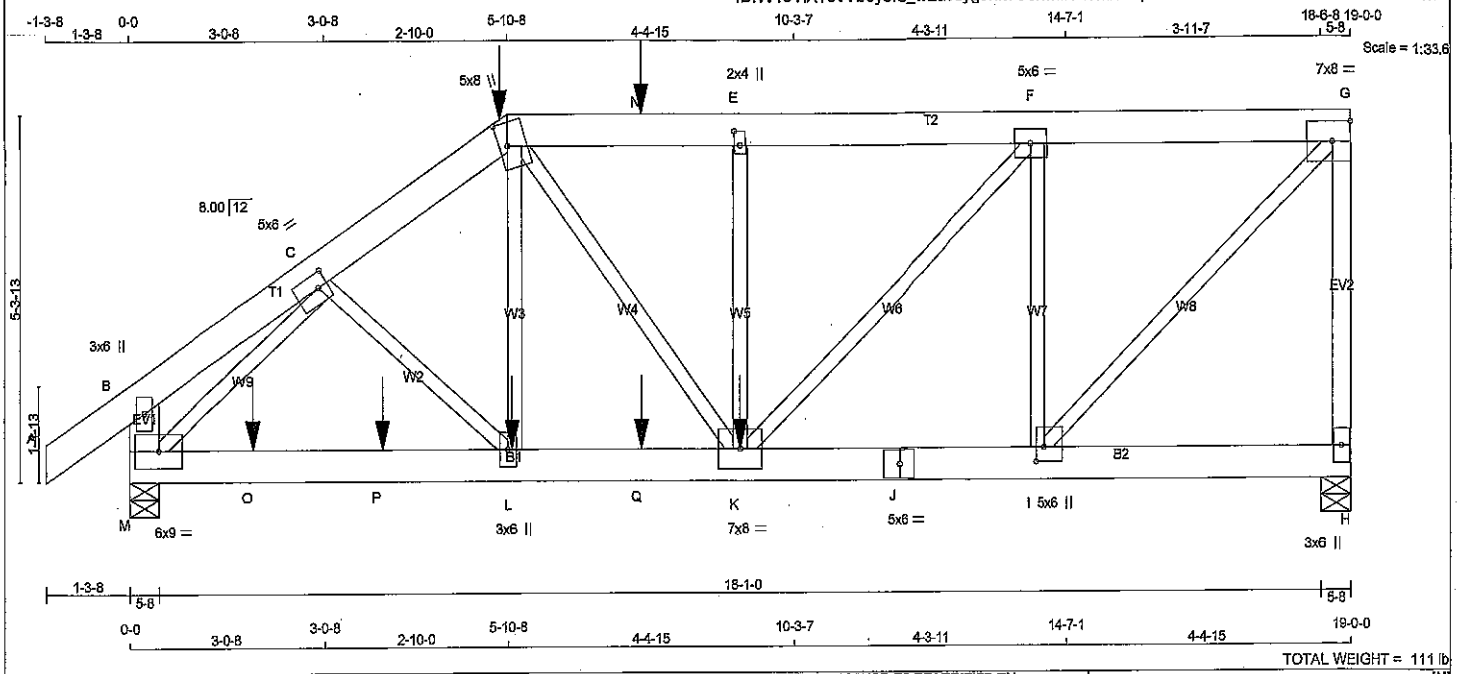

TOTAL WEIGHT = 111 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	5.0	8.0	4.00	1.50
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	7.0	8.0		Edge
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	6.0	2.50	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	7.0	8.0		
L	BMWW+t	MT20	3.0	6.0		
M	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 561.1 lbs FACTORED DOWN AT 5-10-8, AND 126.3 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, AND 71.7 lbs FACTORED DOWN AT 7-11-4, AND 1341.0 lbs FACTORED DOWN AT 9-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2197	0	2197	0	0	5-8	5-8
M	2639	0	2639	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1744	1080 / 0	337 / 0	0 / 0	0 / 0	327 / 0	0 / 0
M	2096	1293 / 0	406 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-104.9	-104.9	0.08 (1)	10.00
B- C	-10 / 0	-104.9	-104.9	0.09 (1)	6.25
C- D	-3073 / 0	-104.9	-104.9	0.15 (1)	4.67
D- N	-3068 / 0	-104.9	-104.9	0.22 (1)	4.60
N- E	-3068 / 0	-104.9	-104.9	0.22 (1)	4.60
E- F	-3068 / 0	-104.9	-104.9	0.22 (1)	4.60
F- G	-1932 / 0	-104.9	-104.9	0.18 (1)	5.57
H- G	-2141 / 0	0.0	0.0	0.91 (1)	5.74
M- B	-311 / 0	0.0	0.0	0.02 (1)	7.81
M- O	0 / 2295	-28.0	-28.0	0.39 (1)	10.00
O- P	0 / 2295	-28.0	-28.0	0.39 (1)	10.00
P- L	0 / 2295	-28.0	-28.0	0.39 (1)	10.00
L- Q	0 / 2554	-28.0	-28.0	0.41 (1)	10.00
Q- K	0 / 2554	-28.0	-28.0	0.41 (1)	10.00
K- J	0 / 1932	-28.0	-28.0	0.33 (1)	10.00
J- I	0 / 1932	-28.0	-28.0	0.33 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.09 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	0 / 349	0.09 (1)
L- D	0 / 375	0.09 (3)
D- K	0 / 864	0.21 (1)
K- E	-486 / 0	0.18 (1)
K- F	0 / 1639	0.41 (1)
I- F	-1751 / 0	0.68 (1)
I- G	0 / 2731	0.68 (1)
M- C	-3209 / 0	0.85 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-561	-561	---	BACK	VERT	TOTAL
K	9-5-8	-1341	-1341	---	BACK	VERT	TOTAL
L	5-11-4	-41	-72	---	BACK	VERT	TOTAL
N	7-11-4	-126	-126	---	BACK	VERT	TOTAL
O	1-11-4	-1	-87	---	BACK	VERT	TOTAL
P	3-11-4	-50	-87	---	BACK	VERT	TOTAL
Q	7-11-4	-41	-72	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.91/1.00 (G-H:1) , BC=0.41/1.00 (K-L:1) , WB=0.85/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.81 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13236-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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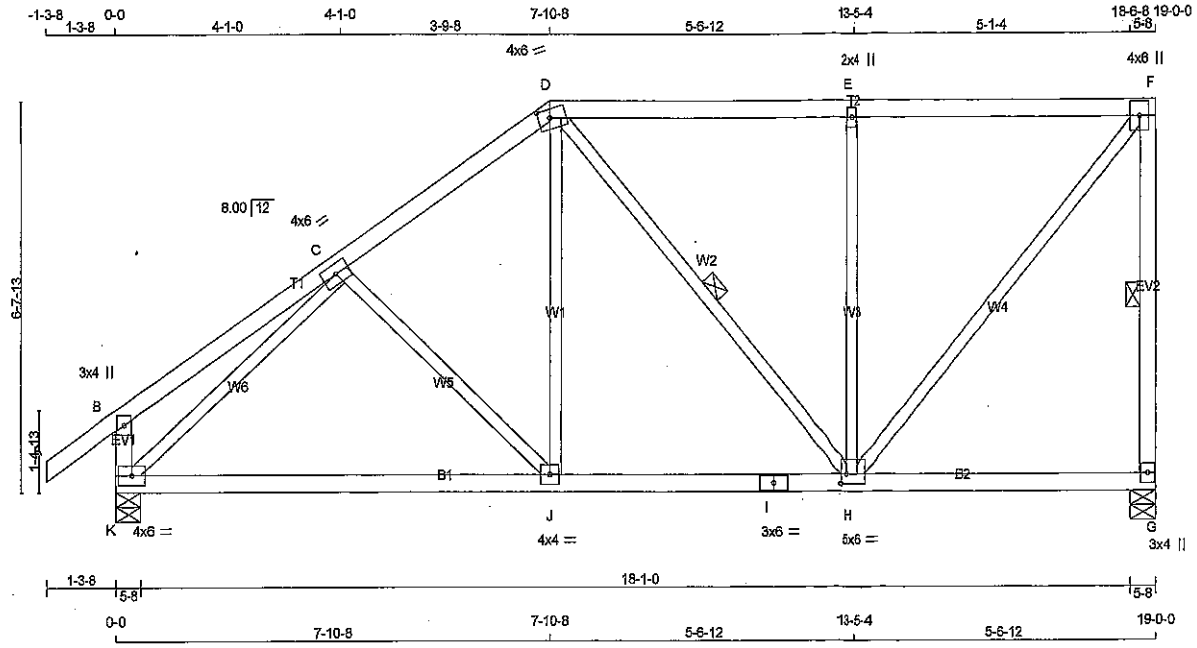

TOTAL WEIGHT = 86 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1263	0	1263	0	0	5-8	5-8
K	1407	0	1407	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1007	617 / 0	200 / 0	0 / 0	0 / 0	190 / 0	0 / 0
K	1104	706 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.26 (1)	10.00
C- D	-1155 / 0	-104.9	-104.9	0.28 (1)	5.57
D- E	-855 / 0	-104.9	-104.9	0.57 (1)	5.65
E- F	-855 / 0	-104.9	-104.9	0.57 (1)	5.65
G- F	-1195 / 0	0.0	0.0	0.24 (1)	5.85
K- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1074	-28.0	-28.0	0.48 (2)	10.00
J- I	0 / 943	-28.0	-28.0	0.48 (2)	10.00
I- H	0 / 943	-28.0	-28.0	0.48 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.19 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 400	0.09 (2)
D- H	-144 / 0	0.07 (2)
H- E	-722 / 0	0.53 (1)
H- F	0 / 1294	0.29 (1)
C- J	-187 / 60	0.09 (1)
K- C	-1473 / 0	0.71 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 831 (0.27")

CSI: TC=0.57/1.00 (E-F:1) , BC=0.48/1.00 (J-K:2) , WB=0.71/1.00 (C-K:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13237-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T29	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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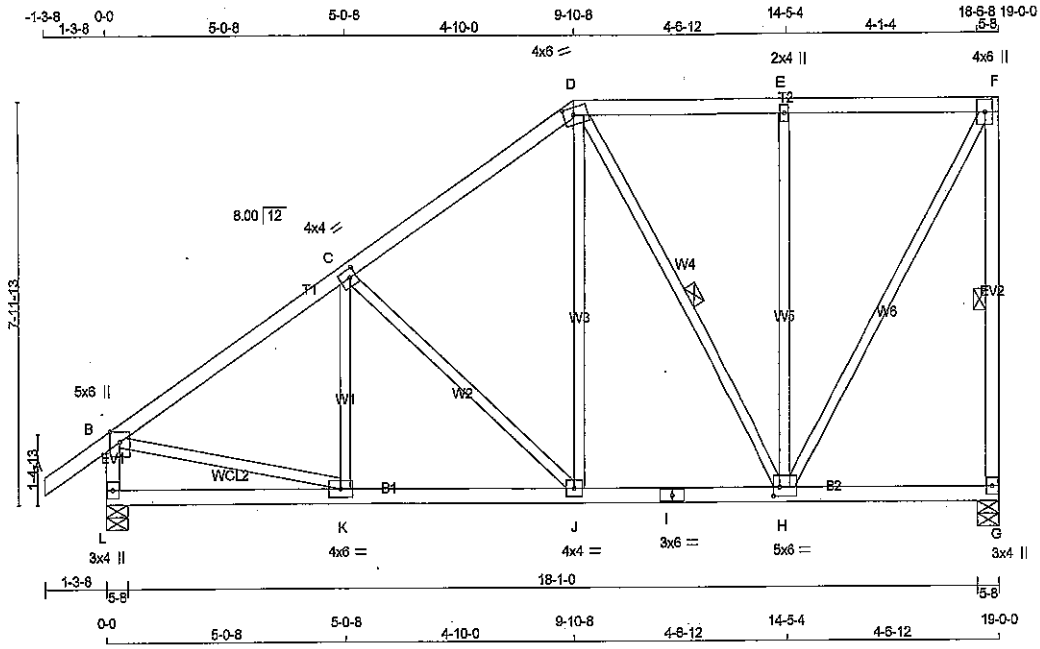

TOTAL WEIGHT = 95 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1263	0	1263	0	0	5-8	5-8
L	1407	0	1407	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1007	617 / 0	200 / 0	0 / 0	0 / 0	190 / 0	0 / 0
L	1104	706 / 0	200 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1332 / 0	-104.9	-104.9	0.35 (1)	5.21
C- D	-974 / 0	-104.9	-104.9	0.34 (1)	5.88
D- E	-613 / 0	-104.9	-104.9	0.28 (1)	6.25
E- F	-613 / 0	-104.9	-104.9	0.28 (1)	6.25
G- F	-1209 / 0	0.0	0.0	0.35 (1)	5.82
L- B	-1349 / 0	0.0	0.0	0.14 (1)	6.98
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00
K- J	0 / 1136	-28.0	-28.0	0.29 (2)	10.00
J- I	0 / 786	-28.0	-28.0	0.22 (2)	10.00
I- H	0 / 786	-28.0	-28.0	0.22 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-87 / 178	0.04 (3)
C- J	-479 / 0	0.39 (1)
J- D	0 / 445	0.10 (1)
D- H	-342 / 0	0.18 (1)
H- E	-590 / 0	0.72 (1)
H- F	0 / 1193	0.27 (1)
B- K	0 / 1162	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.29/1.00 (J-K:2) , WB=0.72/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.28 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1323 B-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T30	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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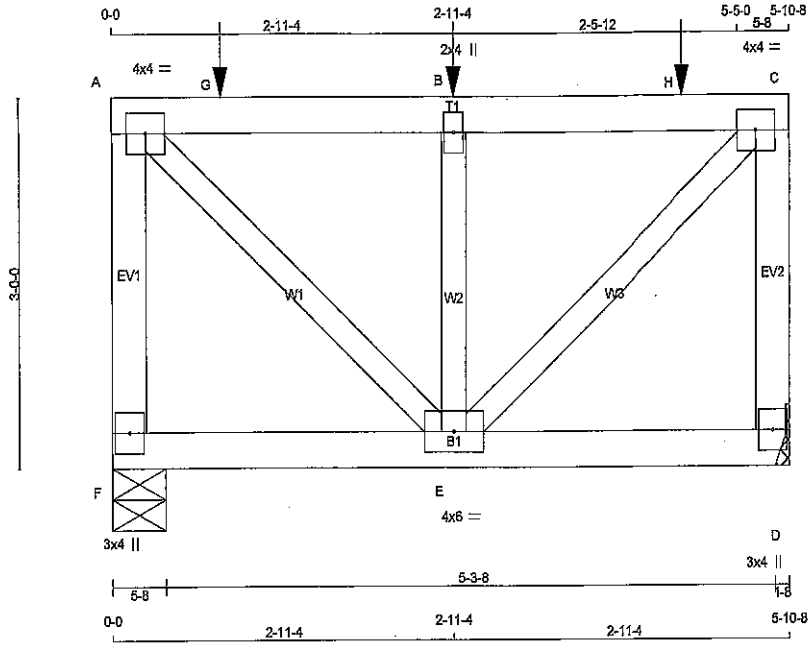

TOTAL WEIGHT = 2 X 27 = 53 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(0.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 237.4 lbs FACTORED DOWN AT 11-4, AND 394.0 lbs FACTORED DOWN AT 2-11-4, AND 884.9 lbs FACTORED DOWN AT 4-11-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	928	0	928	0	0	5-8	5-8
D	1369	0	1369	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	720	475 / 0	120 / 0	0 / 0	0 / 0	124 / 0	0 / 0
D	1101	656 / 0	229 / 0	0 / 0	0 / 0	216 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-886 / 0	0.0	0.0	0.07 (1)	7.81	
A- G	-734 / 0	-104.9	-104.9	0.21 (1)	6.25	
G- B	-734 / 0	-104.9	-104.9	0.21 (1)	6.25	
B- H	-734 / 0	-104.9	-104.9	0.39 (1)	6.25	
H- C	-734 / 0	-104.9	-104.9	0.39 (1)	6.25	
D- C	-1327 / 0	0.0	0.0	0.10 (1)	7.81	
F- E	0 / 0	-28.0	-28.0	0.03 (3)	10.00	
E- D	0 / 0	-28.0	-28.0	0.03 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 998	0.11 (1)
E- B	-1273 / 0	0.11 (1)
E- C	0 / 998	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-11-4	-394	-394	—	TOP	VERT	TOTAL
G	11-4	-237	-237	—	FRONT	VERT	TOTAL
H	4-11-4	-885	-885	—	TOP	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.03/1.00 (D-E:3) , WB=0.11/1.00 (B-E:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (E) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13239-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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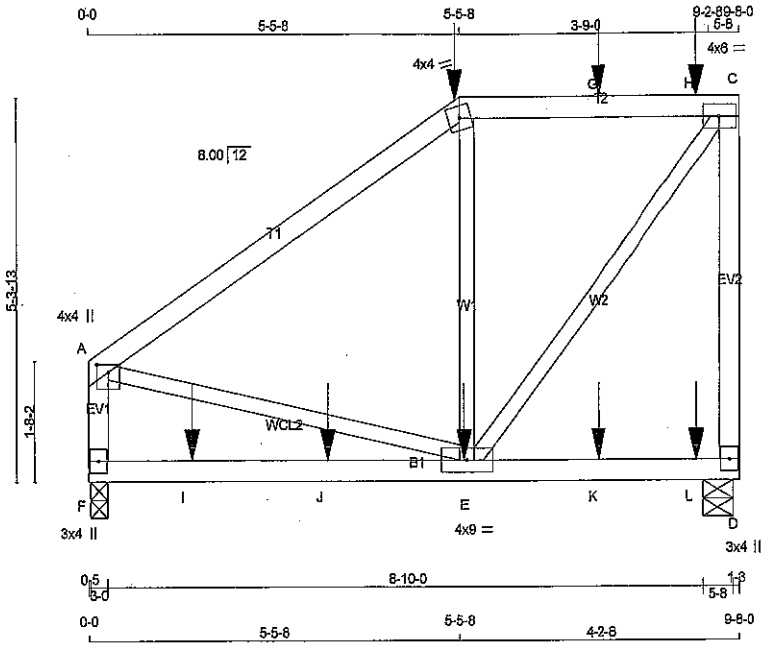

TOTAL WEIGHT = 43 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 561.1 lbs FACTORED DOWN AT 5-5-8, AND 126.3 lbs FACTORED DOWN AT 7-6-4, AND 144.9 lbs FACTORED DOWN AT 9-0-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-6-4, 87.4 lbs FACTORED DOWN AT 3-6-4, 71.7 lbs FACTORED DOWN AT 5-6-4, AND 71.7 lbs FACTORED DOWN AT 7-6-4, AND 80.4 lbs FACTORED DOWN AT 9-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1310	0	1310	0	0	5-8	5-8
F	987	0	987	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1035	648 / 0	195 / 0	0 / 0	0 / 0	192 / 0	0 / 0
F	804	460 / 0	179 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-624 / 0	-104.9	-104.9	0.64 (1)	5.55
B- G	-661 / 0	-104.9	-104.9	0.63 (1)	5.96
G- H	-861 / 0	-104.9	-104.9	0.63 (1)	5.96
H- C	-661 / 0	-104.9	-104.9	0.63 (1)	5.96
D- C	-1226 / 0	0.0	0.0	0.62 (1)	7.20
F- A	-910 / 0	0.0	0.0	0.11 (1)	7.61
F- I	0 / 0	-28.0	-28.0	0.41 (3)	10.00
I- J	0 / 0	-28.0	-28.0	0.41 (3)	10.00
J- E	0 / 0	-28.0	-28.0	0.41 (3)	10.00
E- K	0 / 0	-28.0	-28.0	0.41 (3)	10.00
K- L	0 / 0	-28.0	-28.0	0.41 (3)	10.00
L- D	0 / 0	-28.0	-28.0	0.41 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-699 / 111	0.30 (1)
E- C	0 / 1056	0.26 (1)
A- E	0 / 705	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-5-8	-561	-561	—	BACK	VERT	TOTAL
E	5-6-4	-41	-72	—	BACK	VERT	TOTAL
G	7-6-4	-126	-126	—	BACK	VERT	TOTAL
H	9-0-4	-145	-145	—	BACK	VERT	TOTAL
I	1-6-4	-1	-87	—	BACK	VERT	TOTAL
J	3-6-4	-50	-87	—	BACK	VERT	TOTAL
K	7-6-4	-41	-72	—	BACK	VERT	TOTAL
L	9-0-4	-46	-80	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.64/1.00 (A-B:1) , BC=0.41/1.00 (E-F:3) , WB=0.30/1.00 (B-E:1) , SSI=0.35/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2-876
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1324028
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T32	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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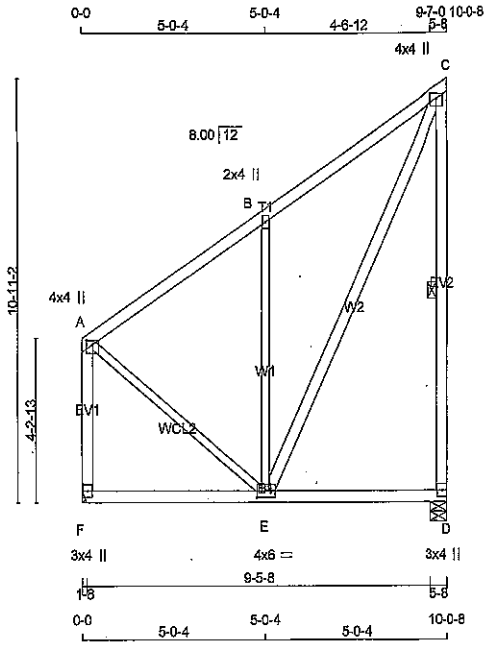

TOTAL WEIGHT = 3 X 65 = 194 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMW+w	MT20	2.0	4.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0	2.00	1.50
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	667	0	667	0	0	5-8	5-8
F	667	0	667	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	532	326 / 0	105 / 0	0 / 0	0 / 0	100 / 0	0 / 0
F	532	326 / 0	105 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX MAX. UNBRAC LENGTH
A- B	-311 / 0	-104.9	-104.9	0.34 (1)	6.25
B- C	-373 / 0	-104.9	-104.9	0.34 (1)	6.25
D- C	-612 / 0	0.0	0.0	0.37 (1)	6.25
F- A	-612 / 0	0.0	0.0	0.18 (1)	7.81
F- E	0 / 0	-28.0	-28.0	0.20 (3)	10.00
E- D	0 / 0	-28.0	-28.0	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-649 / 0	0.68 (1)
E- C	0 / 658	0.11 (1)
A- E	0 / 366	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.20/1.00 (D-E:3) , WB=0.68/1.00 (B-E:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13241 -18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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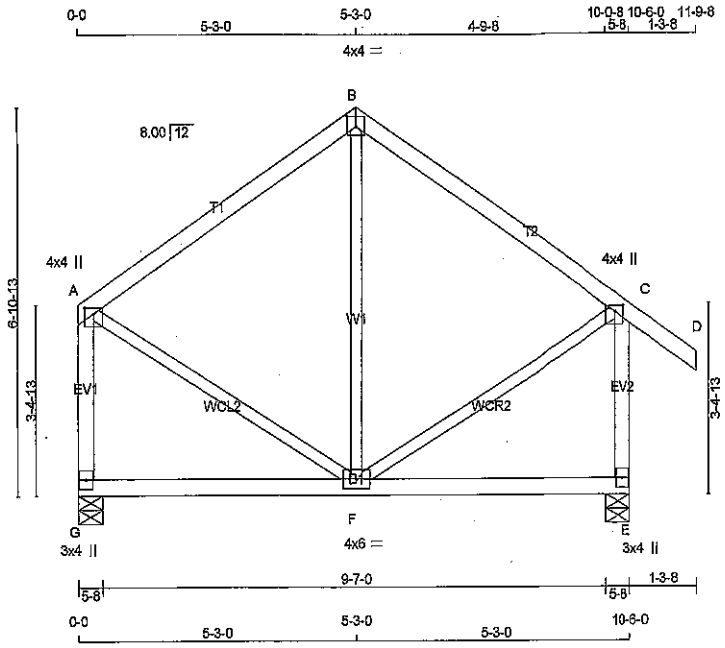

TOTAL WEIGHT = 50 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMWW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	698	0	698	0	0	5-8	5-8
E	842	0	842	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
E	654	430 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-351 / 0	-104.9	-104.9	0.37 (1)	6.25
B- C	-351 / 0	-104.9	-104.9	0.37 (1)	6.25
C- D	0 / 40	-104.9	-104.9	0.14 (1)	10.00
G- A	-642 / 0	0.0	0.0	0.13 (1)	7.81
E- C	-786 / 0	0.0	0.0	0.15 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.23 (3)	10.00
F- E	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-161 / 152	0.13 (1)
A- F	0 / 339	0.08 (1)
F- C	0 / 339	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (A-B:1) , BC=0.23/1.00 (E-F:3) , WB=0.13/1.00 (B-F:1) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (A) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13242-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T33Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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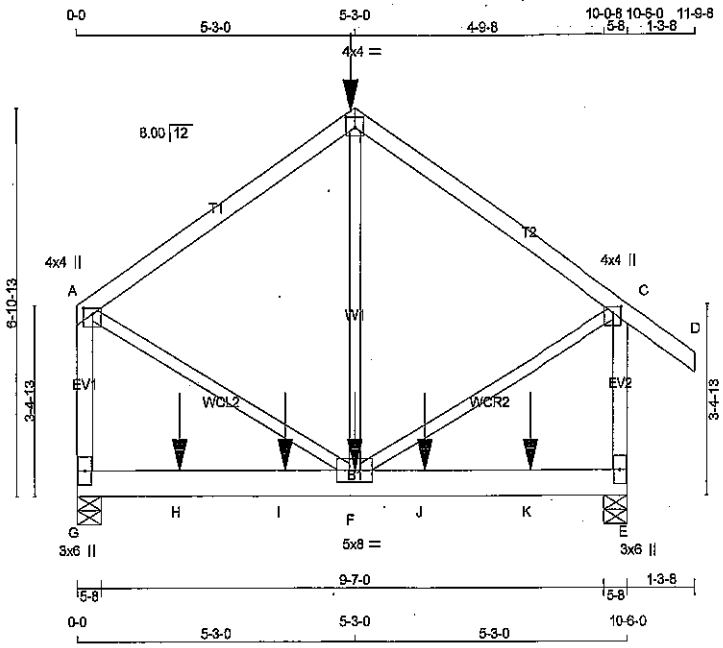

TOTAL WEIGHT = 56 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 775.5 lbs FACTORED DOWN AT 5-3-0 ON TOP CHORD, AND 69.4 lbs FACTORED DOWN AT 1-11-4, 68.0 lbs FACTORED DOWN AT 3-11-4, 68.0 lbs FACTORED DOWN AT 5-3-0, AND 68.0 lbs FACTORED DOWN AT 6-6-12, AND 69.4 lbs FACTORED DOWN AT 8-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1181	0	1181	0	0	5-8	5-8
E	1325	0	1325	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	943	576 / 0	190 / 0	0 / 0	0 / 0	177 / 0	0 / 0
E	1041	665 / 0	190 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-799 / 0	-104.9	-104.9	0.59 (1)	5.75	
B- C	-799 / 0	-104.9	-104.9	0.59 (1)	5.75	
C- D	0 / 40	-104.9	-104.9	0.15 (1)	10.00	
G- A	-1098 / 0	0.0	0.0	0.21 (1)	7.52	
E- C	-1242 / 0	0.0	0.0	0.24 (1)	7.17	
G- H	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
H- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
I- F	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
F- J	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
J- K	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
K- E	0 / 0	-28.0	-28.0	0.19 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-440 / 258	0.33 (1)
A- F	0 / 755	0.19 (1)
F- C	0 / 785	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-3-0	-776	-776	---	BACK	VERT	TOTAL
F	5-3-0	-39	-68	---	BACK	VERT	TOTAL
H	1-11-4	-37	-69	---	BACK	VERT	TOTAL
I	3-11-4	-39	-68	---	BACK	VERT	TOTAL
J	6-6-12	-39	-68	---	BACK	VERT	TOTAL
K	8-6-12	-37	-69	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.59/1.00 (A-B:1) , BC=0.19/1.00 (F-G:3) , WB=0.33/1.00 (B-F:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13243-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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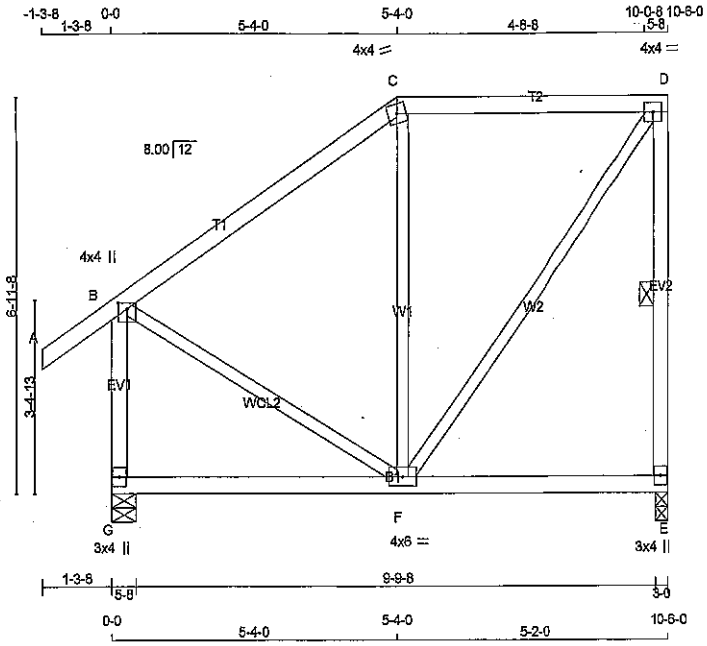

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	698	0	698	0	0	3-0	3-0
G	842	0	842	0	0	5-6	5-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
G	654	430 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-349 / 0	-104.9	-104.9	0.38 (1)	6.25
C- D	-284 / 0	-104.9	-104.9	0.36 (1)	6.25
E- D	-645 / 0	0.0	0.0	0.14 (1)	6.25
G- B	-762 / 0	0.0	0.0	0.15 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.23 (3)	10.00
F- E	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-358 / 54	0.30 (1)
F- D	0 / 470	0.11 (1)
B- F	0 / 333	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.23/1.00 (F-G:3) , WB=0.30/1.00 (C-F:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (F) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13244-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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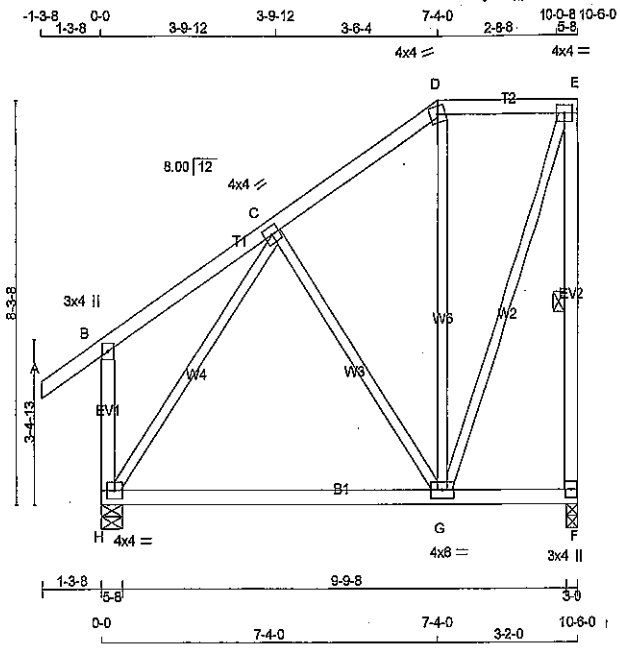

TOTAL WEIGHT = 63 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWWW-t	MT20	4.0	6.0	
H	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	698	0	698	0	0	3-0	3-0
H	842	0	842	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	654	430 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.24 (1)	10.00
C- D	-275 / 0	-104.9	-104.9	0.18 (1)	6.25
D- E	-205 / 0	-104.9	-104.9	0.13 (1)	6.25
F- E	-699 / 0	0.0	0.0	0.22 (1)	6.25
H- B	-291 / 0	0.0	0.0	0.06 (1)	7.81
H- G	0 / 310	-28.0	-28.0	0.37 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.34 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 572	0.13 (1)
C- G	-183 / 35	0.15 (1)
H- C	-559 / 0	0.44 (1)
G- D	-173 / 52	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 857 (0.19")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.37/1.00 (G-H:2) , WB=0.44/1.00 (C-H:1) , SSI=0.17/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/3245-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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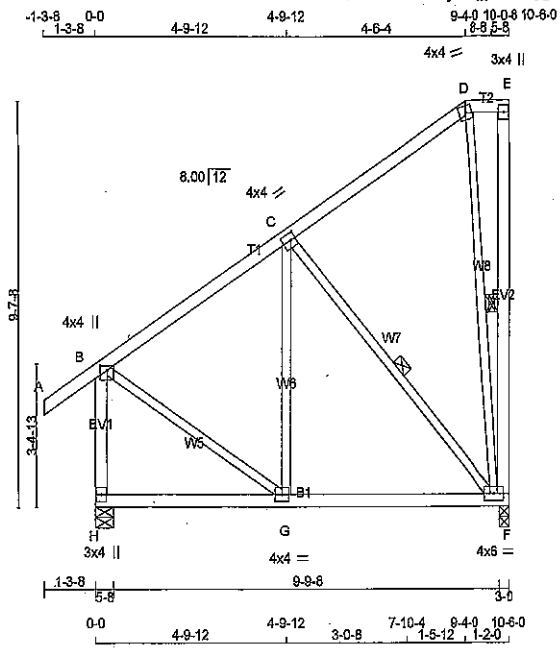

TOTAL WEIGHT = 65 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	698	0	698	0	0	3-0	3-0
H	842	0	842	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	558	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
H	654	430 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-387 / 0	-104.9	-104.9	0.30 (1)	6.25
C- D	-68 / 0	-104.9	-104.9	0.30 (1)	6.25
D- E	0 / 0	-104.9	-104.9	0.02 (1)	10.00
F- E	-61 / 0	0.0	0.0	0.03 (1)	6.25
H- B	-794 / 0	0.0	0.0	0.15 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00
G- F	0 / 349	-28.0	-28.0	0.27 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	0 / 414	0.09 (1)
G- C	-41 / 224	0.05 (3)
C- F	-479 / 0	0.22 (1)
D- F	-219 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.27/1.00 (F-G:2) , WB=0.22/1.00 (C-F:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13246 68
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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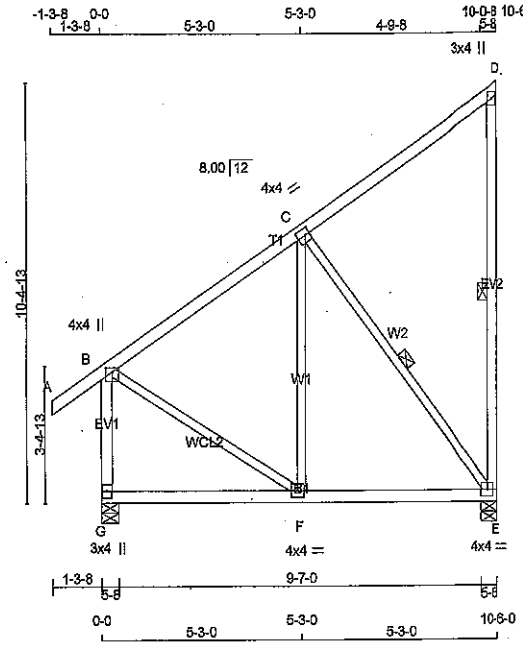

TOTAL WEIGHT = 59 lb

[M](F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	698	0	698	0	0	5-8	5-8
G	842	0	842	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	556	341 / 0	110 / 0	0 / 0	0 / 0	105 / 0	0 / 0
G	654	430 / 0	110 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-379 / 0	-104.9	-104.9	0.38 (1)	6.25	
C- D	-38 / 0	-104.9	-104.9	0.37 (1)	6.25	
E- D	-207 / 0	0.0	0.0	0.11 (1)	6.25	
G- B	-785 / 0	0.0	0.0	0.15 (1)	7.81	
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00	
F- E	0 / 346	-28.0	-28.0	0.26 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-22 / 234	0.05 (3)
C- E	-555 / 0	0.26 (1)
B- F	0 / 402	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.26/1.00 (E-F:2) , WB=0.26/1.00 (C-E:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13247-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T38A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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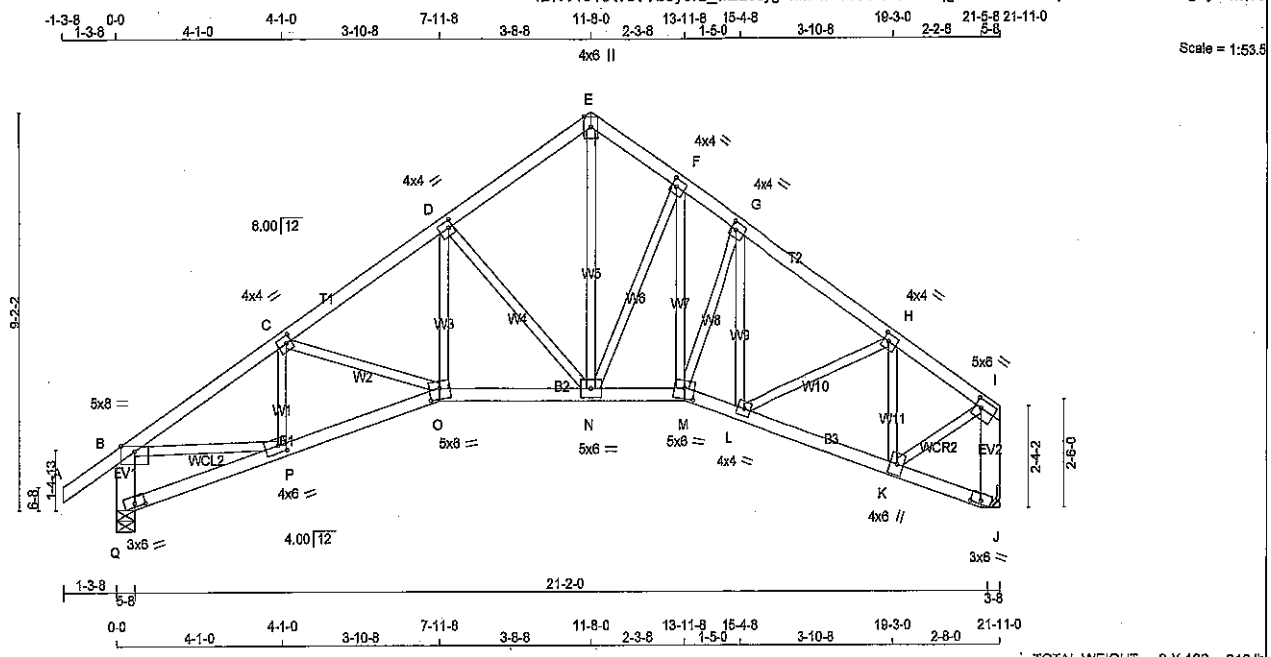

TOTAL WEIGHT = 3 X 106 = 318 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C, F, G, H						
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW+p	MT20	4.0	6.0	Edge	
I	TMW-t	MT20	5.0	6.0	2.25	1.75
J	BVM1-I	MT20	3.0	6.0	1.00	3.00
K	BMWW+t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BBWW-m	MT20	5.0	6.0	3.00	3.00
N	BMWWW-t	MT20	5.0	6.0		
O	BBWW-m	MT20	5.0	6.0	3.00	3.00
P	BMWW-t	MT20	4.0	6.0	2.00	2.25
Q	BVM1-I	MT20	3.0	6.0	1.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1601	0	1601	0	0	5-8	5-8
J	1457	0	1457	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1258	801 / 0	230 / 0	0 / 0	0 / 0	227 / 0	0 / 0
J	1161	712 / 0	230 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2311 / 0	-104.9	-104.9	0.27 (1)		4.28
C- D	-2322 / 0	-104.9	-104.9	0.23 (1)		4.32
D- E	-1530 / 0	-104.9	-104.9	0.17 (1)		5.16
E- F	-1510 / 0	-104.9	-104.9	0.10 (1)		5.27
F- G	-1869 / 0	-104.9	-104.9	0.12 (1)		4.82
G- H	-1879 / 0	-104.9	-104.9	0.17 (1)		4.76
H- I	-1372 / 0	-104.9	-104.9	0.14 (1)		5.41
Q- B	-1550 / 0	0.0	0.0	0.11 (1)		7.81
J- I	-1429 / 0	0.0	0.0	0.13 (1)		7.81
Q- P	0 / 6	-28.0	-28.0	0.11 (3)		10.00
P- O	0 / 2042	-28.0	-28.0	0.38 (1)		10.00
O- N	0 / 1910	-28.0	-28.0	0.36 (1)		10.00
N- M	0 / 1516	-28.0	-28.0	0.30 (1)		10.00
M- L	0 / 1640	-28.0	-28.0	0.29 (1)		10.00
L- K	0 / 1224	-28.0	-28.0	0.24 (1)		10.00
K- J	0 / 6	-28.0	-28.0	0.07 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-456 / 0	0.08 (1)
C- O	-20 / 0	0.01 (3)
O- D	0 / 739	0.17 (1)
D- N	-969 / 0	0.46 (1)
N- E	0 / 1407	0.32 (1)
N- F	-636 / 0	0.31 (1)
M- F	0 / 625	0.14 (1)
M- G	-90 / 0	0.03 (1)
L- G	-227 / 25	0.07 (1)
L- H	0 / 438	0.10 (1)
K- H	-841 / 0	0.17 (1)
B- P	0 / 1941	0.44 (1)
K- I	0 / 1289	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.38/1.00 (O-P:1) , WB=0.46/1.00 (D-N:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.50 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
386
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 132488
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T41S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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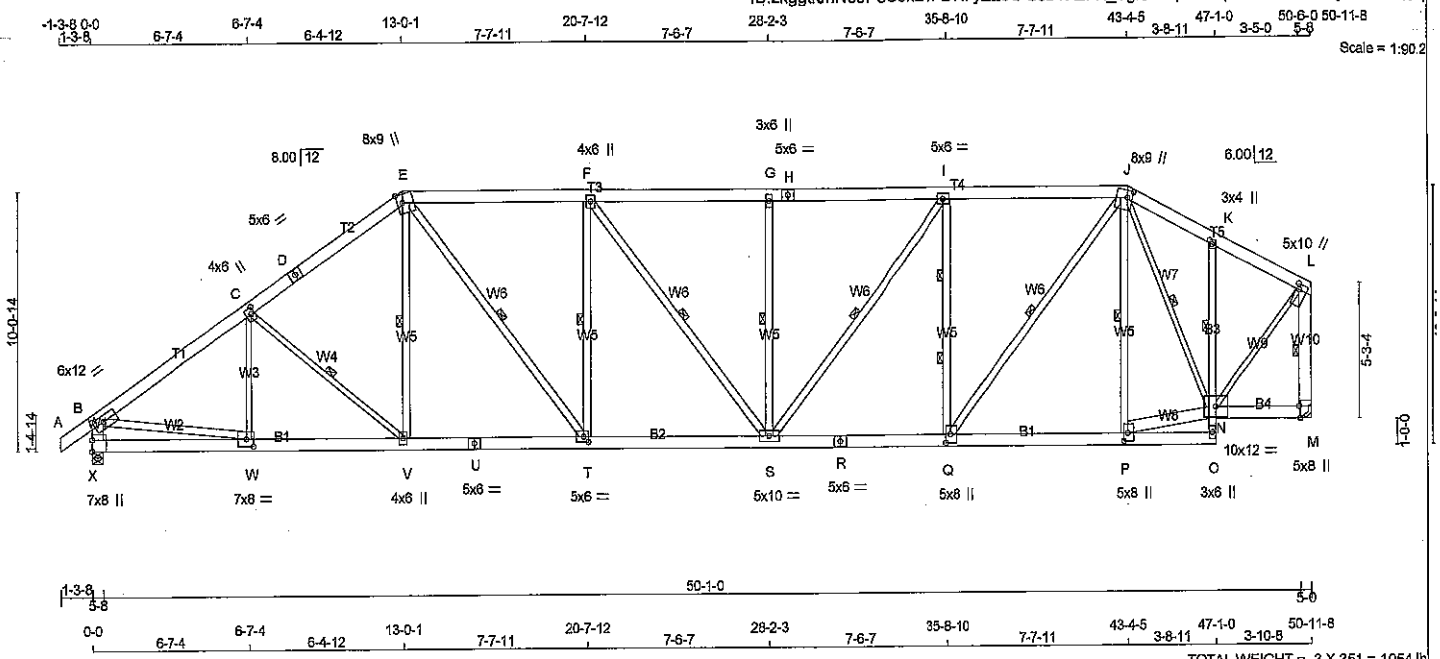

TOTAL WEIGHT = 3 X 351 = 1054 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	2100F 1.8E	SPF
H - J	2x6	DRY	2100F 1.8E	SPF
J - L	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - C	2x3	DRY	No.2	SPF
C - V	2x3	DRY	No.2	SPF
N - L	2x3	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
J - N	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	12.0	2.75	4.75
C	TMWW+t	MT20	4.0	6.0	3.00	1.50
D	TS-t	MT20	5.0	6.0		
E	TTWW+m	MT20	8.0	9.0	Edge	2.50
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWWW+m	MT20	8.0	9.0	3.25	2.50
K	TMV+p	MT20	3.0	4.0	2.00	1.25
L	TMVW+t	MT20	5.0	10.0	2.50	1.25
M	BMV1+t	MT20	5.0	8.0	Edge	0.50
N	BVMWWW-l	MT20	10.0	12.0		
O	BMV+p	MT20	3.0	6.0		
P	BMWW+t	MT20	5.0	8.0	3.75	2.00
Q	BMWW+t	MT20	5.0	8.0	4.00	2.00
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	10.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW-t	MT20	7.0	8.0	3.50	3.50
X	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	4724	0	4724	240	-243	5-8	5-8
M	4562	0	4562	0	-152	HANGER BY OTHERS MIN. SEAT SIZE: 5-0	

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 243 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 152 LBS FACTORED UPLIFT

Simpson H1-HL-STA. LSS26GS.

PROVIDE FOR 240 LBS FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3633	2337 / 0	535 / 0	0 / 0	287 / -663	761 / 0	0 / 0
M	3528	2214 / 0	535 / 0	0 / 0	177 / -610	779 / 0	0 / 0
HORIZONTAL REACTIONS							
X	—	0 / 0	0 / 0	0 / 0	171 / -168	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X
BEARING SIZE FACTOR = 1.15 AT JNT(S) X (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF K-N, C-V, E-V, E-T, S-T, F-S, G-S, L-S, J-O, J-P, L-M, J-N.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 62	-142.8	-142.8	0.11 (2)	10.00	
B- C	-6020 / 283	-142.8	-142.8	0.79 (2)	2.93	
C- D	-5633 / 310	-142.8	-142.8	0.73 (1)	3.06	
D- E	-5633 / 310	-142.8	-142.8	0.73 (1)	3.06	
E- F	-5993 / 310	-155.3	-155.3	0.49 (2)	2.00	
F- G	-6116 / 231	-155.3	-155.3	0.47 (2)	2.00	
G- H	-6116 / 231	-155.3	-155.3	0.46 (1)	2.00	
H- I	-6116 / 231	-155.3	-155.3	0.46 (1)	2.00	
I- J	-5240 / 228	-155.3	-155.3	0.46 (3)	2.00	
J- K	-2846 / 246	-142.8	-142.8	0.19 (3)	4.82	
K- L	-2806 / 154	-142.8	-142.8	0.19 (3)	4.85	
X- B	-4633 / 275	0.0	0.0	0.30 (1)	5.03	
M- L	-4506 / 178	0.0	0.0	0.41 (1)	4.08	
X- W	-230 / 208	-28.0	-28.0	0.14 (17)	6.25	
W- V	-305 / 5056	-28.0	-28.0	0.69 (1)	6.25	
V- U	-128 / 4640	-28.0	-28.0	0.64 (1)	6.25	
U- T	-128 / 4640	-28.0	-28.0	0.64 (1)	6.25	
T- S	-114 / 5993	-28.0	-28.0	0.78 (1)	6.25	
S- R	0 / 5240	-28.0	-28.0	0.69 (1)	10.00	
R- Q	0 / 5240	-28.0	-28.0	0.69 (1)	10.00	
Q- P	0 / 3112	-28.0	-28.0	0.45 (1)	10.00	
P- O	-2 / 11	-28.0	-28.0	0.13 (17)	10.00	
O- N	0 / 44	0.0	0.0	0.02 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-573 / 112	0.24 (7)
C- V	-709 / 230	0.33 (2)
V- E	-60 / 664	0.11 (2)
E- T	-19 / 2168	0.35 (1)
T- F	-1502 / 111	0.67 (1)
F- S	-775 / 1073	0.65 (2)
S- G	-1095 / 89	0.49 (1)
S- I	-123 / 1888	0.30 (2)
Q- I	-2452 / 140	0.53 (1)
Q- J	-54 / 3412	0.55 (1)
P- J	-671 / 70	0.30 (7)
B- W	-131 / 5102	0.82 (1)
N- L	-83 / 3935	0.89 (1)
P- N	0 / 3222	0.38 (1)
J- N	-1568 / 0	0.91 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	43.5	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	65.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 8.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/180 (3.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.79/1.00 (B-C:2) , BC=0.78/1.00 (S-T:1) , WB=0.91/1.00 (J-N:1) , SSI=0.48/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

PROFESSIONAL ENGINEER J. S. KATSOULAKIS PROVINCE OF ONTARIO

DWG NO. TAM 13202

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T41S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:zkggtrJnN3sPSCJk21FDR7yZZod-GJbTK2FH_TgiUsBqaIKHxpPfxw0?nrROIIj18CzdMSq

LOADING
TOTAL LOAD CASES: (18)

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.95 (W) (INPUT = 1.00)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- K	-773 / 177	0.0	0.0	0.05 (3)	6.25			
N- M	-27 / 57	-28.0	-28.0	0.07 (17)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13202018
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T41S1	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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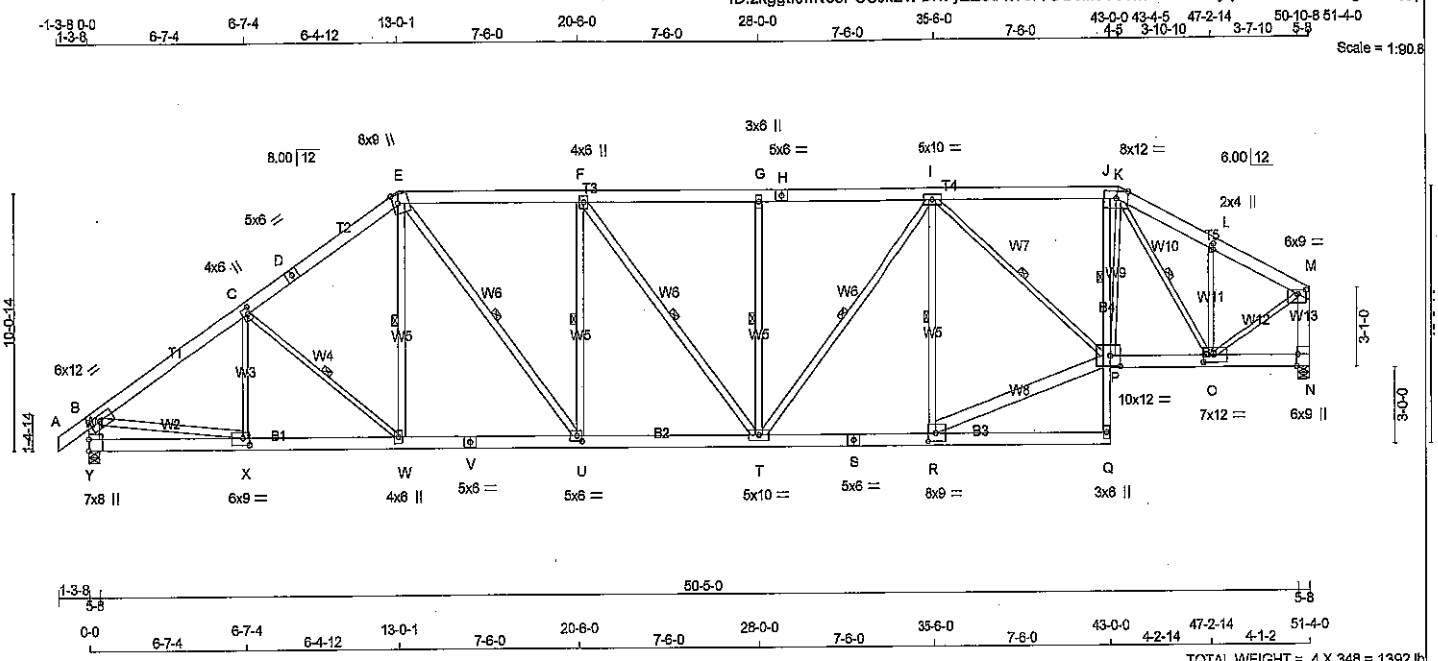

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	2100F 1.6E	SPF
H - K	2x6	DRY	2100F 1.6E	SPF
K - M	2x6	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - J	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
X - C	2x3	DRY	No.2	SPF
C - W	2x3	DRY	No.2	SPF
O - L	2x3	DRY	No.2	SPF
O - M	2x3	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
I - P	2x3	DRY	No.2	SPF
P - K	2x3	DRY	No.2	SPF
K - O	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	12.0	2.75	4.75
C	TMWW+t	MT20	4.0	6.0	3.00	1.50
D	TS-t	MT20	5.0	6.0		
E	TTWW+m	MT20	8.0	9.0	Edge	2.50
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWWW-t	MT20	5.0	10.0		
J						
K						
K	TTVWW*-w	MT20	8.0	12.0	Edge	
L	TMW+w	MT20	2.0	4.0	3.00	
M	TMVW-p	MT20	6.0	9.0	2.00	4.00
N	BMV1+t	MT20	6.0	9.0	Edge	0.50
O	BMWWW-t	MT20	7.0	12.0	3.50	2.50
P	BVMWWW-l	MT20	10.0	12.0	5.00	5.00
Q	BMV+p	MT20	3.0	6.0		
R	BMWW-t	MT20	8.0	9.0	3.75	4.00
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	10.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BS-t	MT20	5.0	6.0		
W	BMWW+t	MT20	4.0	6.0		
X	BMWW-t	MT20	6.0	9.0	3.25	3.25
Y	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	4488	0	4488	341	-694	5-8	5-8
N	4402	0	4402	0	-581	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT Y FOR 694 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 581 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 341 LBS FACTORED HORIZONTAL REACTION AT JOINT Y

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	3462	2182 / 0	513 / 0	0 / 0	426 / -989	767 / 0	0 / 0
N	3407	2112 / 0	513 / 0	0 / 0	269 / -918	782 / 0	0 / 0
HORIZONTAL REACTIONS							
Y	—	0 / 0	0 / 0	0 / 0	244 / -246	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) Y, N (BASED ON SUPPORT DEPTH = 6)

BRACING
FOR SECTION E-K, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-P, C-W, E-W, E-U, F-U, F-T, G-T, I-T, I-R, P-K, K-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (16)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 62	-133.1	-133.1	0.11 (2)		10.00
B- C	-5732 / 862	-133.1	-133.1	0.78 (2)		3.04
C- D	-5374 / 884	-133.1	-133.1	0.73 (2)		3.21
D- E	-5374 / 884	-133.1	-133.1	0.73 (2)		3.21
E- F	-5726 / 921	-145.6	-145.8	0.48 (2)		2.00
F- G	-5884 / 821	-145.6	-145.6	0.46 (2)		2.00
G- H	-5884 / 821	-145.6	-145.6	0.46 (3)		2.00
H- I	-5884 / 821	-145.6	-145.6	0.46 (3)		2.00
I- J	-4681 / 731	-145.6	-145.6	0.46 (3)		2.00
J- K	-4685 / 732	-145.6	-145.6	0.12 (1)		2.00
K- L	-4070 / 713	-142.8	-142.8	0.24 (3)		4.11
L- M	-4038 / 570	-142.8	-142.8	0.24 (3)		4.13
Y- B	-4399 / 724	0.0	0.0	0.28 (1)		5.15
N- M	-4356 / 598	0.0	0.0	0.41 (1)		5.17
Y- X	-326 / 239	-27.5	-27.5	0.14 (17)		6.25
X- W	-888 / 4813	-27.5	-27.5	0.65 (1)		6.25
W- V	-608 / 4428	-27.5	-27.5	0.61 (1)		6.25
V- U	-608 / 4428	-27.5	-27.5	0.61 (1)		6.25
U- T	-696 / 5726	-27.5	-27.5	0.75 (1)		6.25
T- S	-448 / 5145	-27.5	-27.5	0.72 (1)		6.25
S- R	-448 / 5145	-27.5	-27.5	0.72 (1)		6.25
R- Q	-1 / 10	-27.5	-27.5	0.20 (17)		10.00
Q- P	0 / 139	0.0	0.0	0.05 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-547 / 176	0.23 (7)
C- W	-707 / 364	0.33 (2)
W- E	-149 / 657	0.11 (2)
E- U	-210 / 2095	0.34 (1)
U- F	-1468 / 260	0.65 (1)
F- T	-737 / 1122	0.61 (2)
T- G	-1010 / 191	0.45 (8)
T- I	-289 / 1794	0.29 (2)
R- I	-1840 / 285	0.83 (1)
O- L	-801 / 297	0.27 (3)
B- X	-567 / 4857	0.78 (1)
O- M	-498 / 4305	0.97 (1)
R- P	-462 / 5538	0.65 (1)
I- P	-792 / 145	0.54 (2)
P- K	-311 / 2575	0.58 (1)
K- O	-1744 / 100	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	40.2	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 8.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(73 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 40.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (3.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.78/1.00 (B-C:2) , BC=0.75/1.00 (T-U:1) , WB=0.97/1.00 (M-O:1) , SSI=0.48/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.94 (R) (INPUT = 1.00)

PROFESSIONAL ENGINEER
S. S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1320348
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- J	-327 / 463	0.0	0.0	0.10 (2)	6.25			
P- O	-397 / 4511	-27.5	-27.5	0.56 (1)	6.25			
O- N	-22 / 46	-27.5	-27.5	0.07 (17)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13203-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287666	T41S2	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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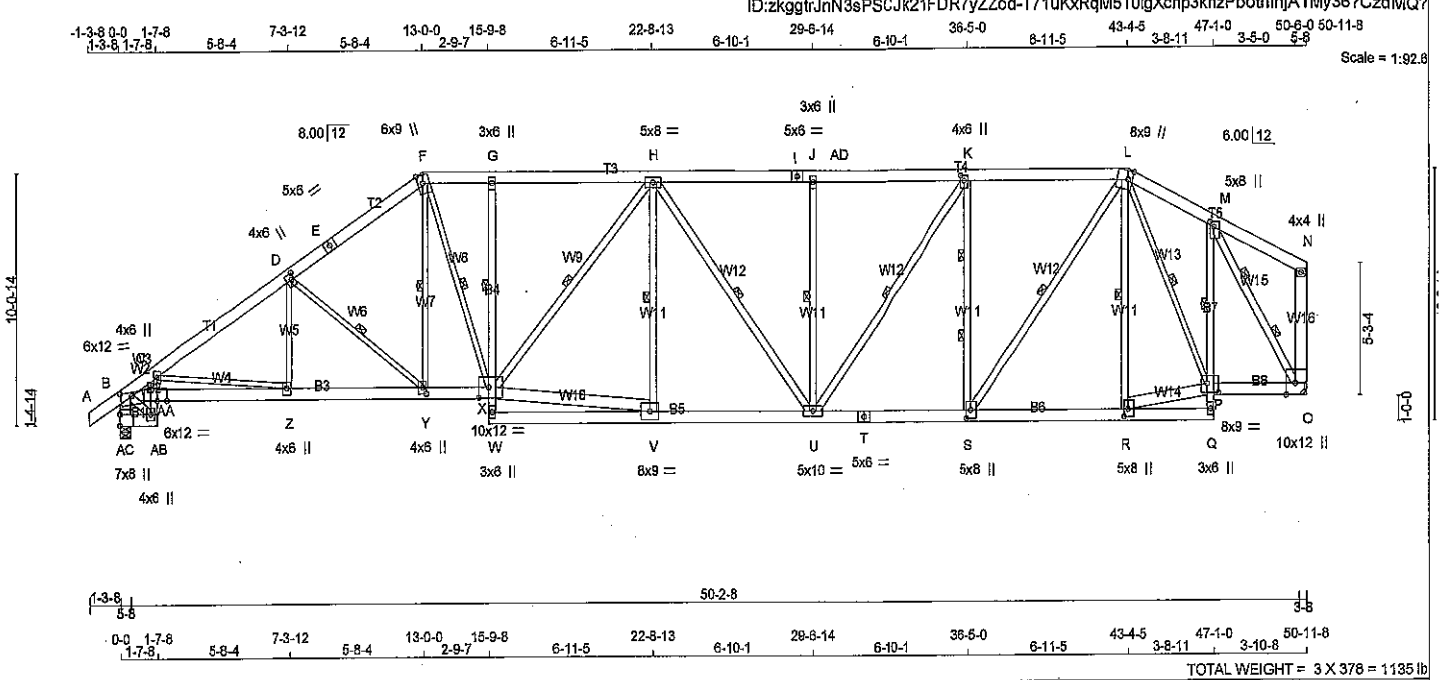

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
AC- AB	2x6	DRY	No.2	SPF
AB- C	2x4	DRY	No.2	SPF
AA- X	2x6	DRY	No.2	SPF
W - G	2x4	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
C - Z	2x3	DRY	No.2	SPF
Z - D	2x3	DRY	No.2	SPF
D - Y	2x3	DRY	No.2	SPF
Y - F	2x3	DRY	No.2	SPF
F - X	2x3	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
L - P	2x3	DRY	No.2	SPF
B - AA	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWWm	MT20	6.0	12.0	1.75	6.00
C	TMVW+p	MT20	4.0	6.0		
D	TMWW+t	MT20	4.0	6.0	3.00	1.50
E	TS-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.50
G	TMV+p	MT20	3.0	6.0		
H	TMWWW-t	MT20	5.0	8.0		
I	TS-t	MT20	5.0	6.0		
J	TMV+w	MT20	3.0	6.0		
K	TMWW+t	MT20	4.0	6.0	2.50	2.00
L	TTWWW+m	MT20	8.0	9.0	4.25	2.25
M	TMVW+p	MT20	5.0	8.0	2.50	2.00
N	TMV+p	MT20	4.0	4.0		
O	BMVW1+p	MT20	10.0	12.0	Edge	4.50
P	BVMWW-l	MT20	8.0	9.0	4.50	6.00
Q	BMV+p	MT20	3.0	6.0		
R	BMWW+t	MT20	5.0	8.0	3.75	2.00
S	BMWW+t	MT20	5.0	8.0	4.00	2.25
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	5.0	10.0		
V	BMWW-t	MT20	8.0	9.0		
W	BMV+p	MT20	3.0	6.0		
X	BVMWWW-l	MT20	10.0	12.0	5.00	5.00
Y	BMWW+t	MT20	4.0	6.0	3.00	1.75
Z	BMWW+t	MT20	4.0	6.0		
AA	BVMW-l	MT20	6.0	12.0		5.25
AB	BMVW+p	MT20	4.0	6.0		
AC	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AC	4459	0	4459	356	-696	5-8	5-8
O	4368	0	4368	0	-567	HANGER BY OTHERS	
						MIN. SEAT SIZE: 5-0	

PROVIDE ANCHORAGE AT BEARING JOINT AC FOR 696 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 567 LBS FACTORED UPLIFT

SIMPSON 1-HGUS26

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 356 LBS FACTORED HORIZONTAL REACTION AT JOINT AC

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AC	3439	2168 / 0	510 / 0	0 / 0	427 / -987	782 / 0	0 / 0
O	3361	2093 / 0	510 / 0	0 / 0	263 / -906	779 / 0	0 / 0
HORIZONTAL REACTIONS							
AC	---	0 / 0	0 / 0	0 / 0	254 / -249	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AC
BEARING SIZE FACTOR = 1.15 AT JNT(S) AC (BASED ON SUPPORT DEPTH = 5.8)

BRACING

FOR SECTION F-L, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-X, M-P, D-Y, F-Y, F-X, H-V, H-U, J-U, K-U, D-S, L-R, H-X, L-P.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF K-S, M-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 62	-133.1	-133.1	0.11 (2)	10.00
B- C	-7586 / 1449	-133.1	-133.1	0.45 (1)	2.87
C- D	-6801 / 1063	-133.1	-133.1	0.74 (2)	2.83
D- E	-5917 / 973	-133.1	-133.1	0.65 (2)	3.11
E- F	-5917 / 973	-133.1	-133.1	0.65 (2)	3.11
F- G	-5543 / 958	-145.6	-145.6	0.38 (2)	2.00
G- H	-5536 / 958	-145.6	-145.6	0.74 (2)	2.00
H-AD	-5687 / 786	-145.6	-145.6	0.71 (1)	2.00
AD- I	-5687 / 786	-145.6	-145.6	0.71 (1)	2.00
I- J	-5687 / 786	-145.6	-145.6	0.71 (1)	2.00
J- K	-5687 / 786	-145.6	-145.6	0.75 (1)	2.00
K- L	-4812 / 737	-145.6	-145.6	0.74 (3)	2.00
L- M	-2747 / 587	-142.8	-142.8	0.22 (3)	4.85
M- N	-38 / 141	-142.8	-142.8	0.19 (3)	6.25
AC- B	-4439 / 705	0.0	0.0	0.29 (1)	5.13
O- N	-270 / 140	0.0	0.0	0.08 (3)	7.81
AC-AB	-341 / 309	-27.5	-27.5	0.03 (11)	6.25
AB-AA	-189 / 166	0.0	0.0	0.36 (1)	7.81

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Z	-872 / 495	0.53 (2)
Z- D	0 / 354	0.08 (17)
D- Y	-1260 / 443	0.46 (2)
Y- F	-227 / 971	0.22 (2)
F- X	-169 / 2049	0.46 (1)
V- H	-679 / 185	0.30 (7)
H- U	-1124 / 888	0.85 (2)
U- J	-987 / 206	0.43 (8)
U- K	-288 / 1870	0.30 (2)
S- K	-2363 / 361	0.51 (1)
S- L	-337 / 3158	0.51 (1)
R- L	-665 / 124	0.29 (7)
M- O	-4766 / 528	0.73 (1)
X- V	-613 / 5783	0.68 (1)
X- H	-773 / 172	0.46 (3)
R- P	-201 / 3071	0.36 (1)
L- P	-1408 / 85	0.82 (1)
B-AB	-301 / 389	0.05 (8)
B-AA	-1305 / 5983	0.70 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	40.2	PSF
	DL =	5.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 8.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(73 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 40.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/180 (3.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.45")

CSI: TC=0.75/1.00 (J-K:1) , BC=0.85/1.00 (Z-AA:1) , WB=0.85/1.00 (H-U:2) , SSI=0.43/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

PROFESSIONAL ENGINEER J. S. KAI SUULARUS PROVINCE OF ONTARIO 3/16

DWG NO. TAM 13224-B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287666	T41S2	3	1	TRUSS DESC..	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AA- C	-192 / 373	0.0	0.0	0.41 (1)	7.81
AA- Z	-1435 / 6531	-27.5	-27.5	0.85 (1)	6.25
Z- Y	-945 / 5722	-27.5	-27.5	0.75 (1)	6.25
Y- X	-606 / 4881	-27.5	-27.5	0.65 (1)	6.25
W- X	0 / 130	0.0	0.0	0.10 (1)	10.00
X- G	-847 / 199	0.0	0.0	0.20 (2)	6.25
W- V	-8 / 62	-27.5	-27.5	0.17 (17)	10.00
V- U	-615 / 5784	-27.5	-27.5	0.77 (1)	6.25
U- T	-362 / 4812	-27.5	-27.5	0.63 (1)	6.25
T- S	-362 / 4812	-27.5	-27.5	0.63 (1)	6.25
S- R	-194 / 2958	-27.5	-27.5	0.42 (1)	6.25
R- Q	-3 / 8	-27.5	-27.5	0.11 (17)	10.00
Q- P	0 / 49	0.0	0.0	0.01 (3)	10.00
P- M	-57 / 2207	0.0	0.0	0.36 (1)	6.25
P- O	-190 / 2427	-27.5	-27.5	0.32 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.95 (V) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13224 -B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287666	T41S3	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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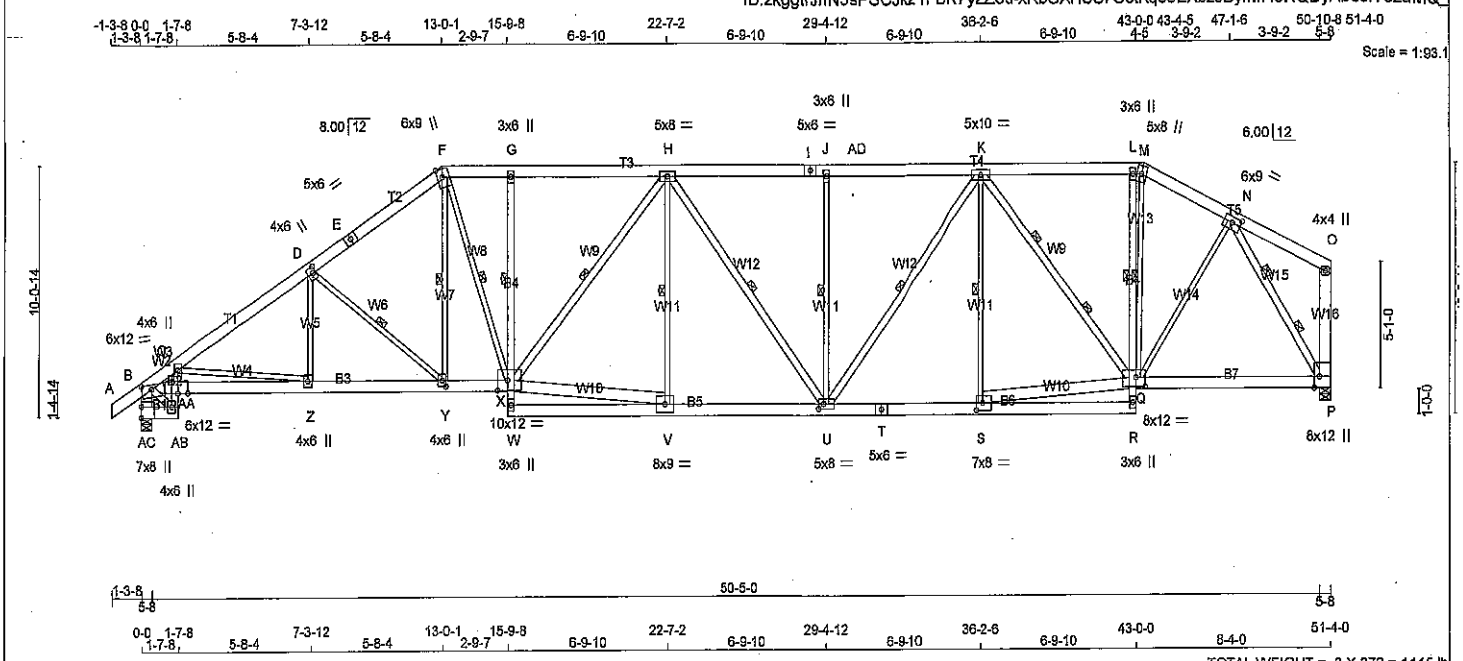

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - M	2x6	DRY	No.2	SPF
M - O	2x6	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
AC- AB	2x6	DRY	No.2	SPF
AB- C	2x4	DRY	No.2	SPF
AA- X	2x6	DRY	No.2	SPF
W - G	2x4	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - U	2x4	DRY	No.2	SPF
U - K	2x4	DRY	No.2	SPF
B - AB	2x4	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
X - H	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
K - Q	2x4	DRY	No.2	SPF
N - P	2x4	DRY	No.2	SPF
B - AA	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWWm	MT20	6.0	12.0	2.25	5.25
C	TMVW+p	MT20	4.0	6.0		
D	TMWW+t	MT20	4.0	6.0	3.00	1.50
E	TS-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.50
G	TMV+p	MT20	3.0	6.0		
H	TMWWW-t	MT20	5.0	8.0		
I	TS-t	MT20	5.0	6.0		
J	TMW+w	MT20	3.0	6.0		
K	TMWWW-t	MT20	5.0	10.0		
L	TMV+p	MT20	3.0	6.0		
M	TTW+m	MT20	5.0	8.0		
N	TMWW-t	MT20	6.0	9.0	2.75	4.50
O	TMV+p	MT20	4.0	4.0		
P	BMVW1+t	MT20	8.0	12.0	Edge	2.50
Q	BVMWWWW*	MT20	8.0	12.0	4.50	5.00
R	BMV+p	MT20	3.0	6.0		
S	BMWW-t	MT20	7.0	8.0	3.50	3.50
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	5.0	8.0	2.50	2.50
V	BMWW-t	MT20	8.0	9.0		
W	BMV+p	MT20	3.0	6.0		
X	BVMWWW-I	MT20	10.0	12.0	5.00	5.00
Y	BMWW+t	MT20	4.0	6.0	3.00	1.75
Z	BMWW+t	MT20	4.0	6.0		
AA	BVMW-I	MT20	6.0	12.0		5.25
AB	BMWW+p	MT20	4.0	6.0		
AC	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AC	4491	0	4491	349	-699	5-8	5-8
P	4399	0	4399	0	-577	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT AC FOR 699 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT P FOR 577 LBS FACTORED UPLIFT
SIMPSON H2.5A / H2.5A

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 349 LBS FACTORED HORIZONTAL REACTION AT JOINT AC

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AC	3464	2184 / 0	513 / 0	0 / 0	427 / -992	767 / 0	0 / 0
P	3405	2110 / 0	513 / 0	0 / 0	265 / -915	782 / 0	0 / 0
HORIZONTAL REACTIONS							
AC	---	0 / 0	0 / 0	0 / 0	249 / -249	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AC, P
BEARING SIZE FACTOR = 1.15 AT JNT(S) AC, P (BASED ON SUPPORT DEPTH = 4.8)

BRACING
FOR SECTION F-M, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-X, L-Q, D-Y, F-Y, F-X, H-V, H-U, S-U, K-L, K-S, X-M-Q.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF K-Q, N-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 62	-133.1	-133.1	0.11 (2)	10.00	
B- C	-7645 / 1453	-133.1	-133.1	0.45 (1)	2.86	
C- D	-6859 / 1067	-133.1	-133.1	0.74 (2)	2.81	
D- E	-5975 / 978	-133.1	-133.1	0.65 (2)	3.09	
E- F	-5975 / 978	-133.1	-133.1	0.65 (2)	3.09	
F- G	-5599 / 962	-145.6	-145.6	0.36 (2)	2.00	
G- H	-5592 / 962	-145.6	-145.6	0.73 (2)	2.00	
H-AD	-5796 / 798	-145.6	-145.6	0.71 (1)	2.00	
AD- I	-5796 / 798	-145.6	-145.6	0.71 (1)	2.00	
I- J	-5796 / 798	-145.6	-145.6	0.71 (1)	2.00	
J- K	-5796 / 798	-145.6	-145.6	0.71 (1)	2.00	
K- L	-3609 / 636	-145.6	-145.6	0.67 (3)	2.00	
L- M	-3614 / 637	-145.6	-145.6	0.11 (1)	2.00	
M- N	-3980 / 648	-142.8	-142.8	0.22 (3)	4.17	
N- O	-42 / 138	-142.8	-142.8	0.17 (3)	6.25	
AC- B	-4471 / 708	0.0	0.0	0.29 (1)	5.11	
P- O	-286 / 139	0.0	0.0	0.07 (3)	7.81	
AC-AB	-334 / 310	-27.5	-27.5	0.03 (11)	6.25	
AB-AA	-189 / 166	0.0	0.0	0.36 (1)	7.81	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Z	-872 / 495	0.53 (2)
Z- D	0 / 364	0.08 (17)
D- Y	-1261 / 443	0.46 (2)
Y- F	-227 / 971	0.22 (2)
F- X	-191 / 2072	0.47 (1)
V- H	-709 / 166	0.44 (7)
H- U	-1067 / 939	0.81 (2)
U- J	-981 / 203	0.59 (8)
U- K	-283 / 1822	0.29 (2)
S- K	-573 / 150	0.35 (7)
B-AB	-301 / 389	0.05 (8)
X- V	-620 / 5853	0.69 (1)
X- H	-784 / 152	0.48 (3)
S- Q	-377 / 4987	0.59 (1)
K- Q	-2235 / 269	0.61 (1)
Q- M	-229 / 1398	0.31 (1)
Q- N	-62 / 1917	0.43 (1)
N- P	-4739 / 576	0.74 (1)
B-AA	-1309 / 6026	0.71 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	40.2	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.2	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 8.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(73 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 40.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/180 (3.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.77/1.00 (J-K:1) , BC=0.85/1.00 (Z-AA:1) , WB=0.81/1.00 (H-U:2) , SSI=0.42/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 13225-8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287666	T41S3	3	1	TRUSS DESC.	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING
TOTAL LOAD CASES: (18)

JSI GRIP= 0.89 (X) (INPUT = 0.90)
JSI METAL= 1.00 (V) (INPUT = 1.00)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-187 / 375	0.0	0.0	0.41 (1)	7.81			
AA- Z	-1438 / 6578	-27.5	-27.5	0.85 (1)	6.25			
Z- Y	-949 / 5771	-27.5	-27.5	0.76 (1)	6.25			
Y- X	-610 / 4930	-27.5	-27.5	0.66 (1)	6.25			
W- X	0 / 126	0.0	0.0	0.10 (1)	10.00			
X- G	-817 / 193	0.0	0.0	0.19 (2)	6.25			
W- V	-8 / 63	-27.5	-27.5	0.16 (17)	10.00			
V- U	-622 / 5850	-27.5	-27.5	0.77 (1)	6.25			
U- T	-377 / 4975	-27.5	-27.5	0.67 (1)	6.25			
T- S	-377 / 4975	-27.5	-27.5	0.67 (1)	6.25			
S- R	-4 / 43	-27.5	-27.5	0.16 (17)	10.00			
R- Q	0 / 126	0.0	0.0	0.07 (1)	10.00			
Q- L	-511 / 152	0.0	0.0	0.09 (3)	6.25			
Q- P	-227 / 2486	-27.5	-27.5	0.47 (4)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
3-8-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13225-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T42	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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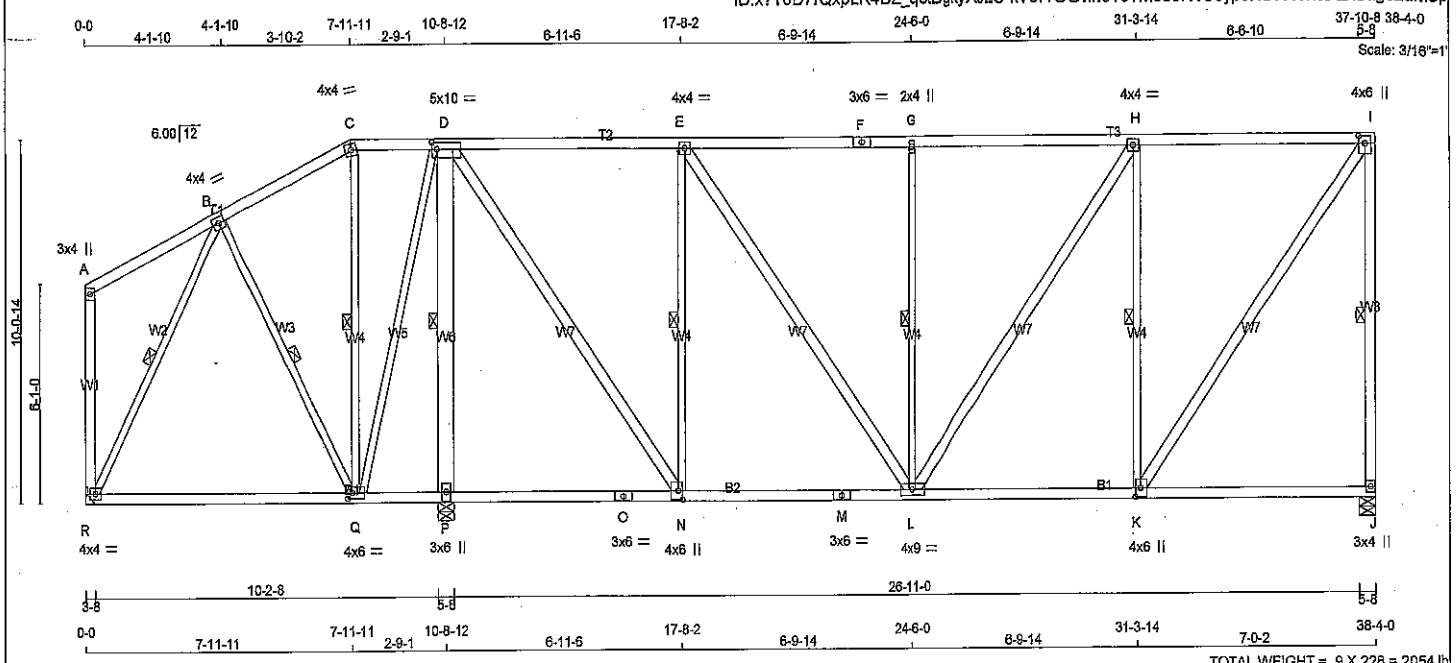

TOTAL WEIGHT = 9 X 228 = 2054 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
P - D	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
E - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, E, H						
B	TMWW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWWW-t	MT20	5.0	10.0	2.25	1.75
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
I	TMVW+p	MT20	4.0	6.0	2.50	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0	3.00	1.75
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.50
O	BS-t	MT20	3.0	6.0		
P	BMW1+w	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0	2.00	1.50
R	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1875	0	1875	0	0	5-8	5-8
P	2661	0	2661	0	0	5-8	5-8
R	568	0	568	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1488	870 / 0	269 / 0	0 / 0	0 / 0	349 / 0	0 / 0
P	2265	1341 / 0	407 / 0	0 / 0	0 / 0	517 / 0	0 / 0
R	452	279 / 0	90 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING

FOR SECTION C-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, C-Q, D-P, H-K, E-N, G-L, B-Q, B-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-104.9	-104.9	0.28 (1)	10.00
B- C	-72 / 0	-104.9	-104.9	0.21 (1)	6.25
C- D	-48 / 0	-112.4	-112.4	0.39 (1)	2.00
D- E	-967 / 0	-112.4	-112.4	0.52 (1)	2.00
E- F	-1330 / 0	-112.4	-112.4	0.54 (1)	2.00
F- G	-1330 / 0	-112.4	-112.4	0.54 (1)	2.00
G- H	-1330 / 0	-112.4	-112.4	0.64 (1)	2.00
H- I	-1065 / 0	-112.4	-112.4	0.67 (1)	2.00
J- I	-1798 / 0	0.0	0.0	0.89 (1)	4.99
R- A	-163 / 0	0.0	0.0	0.12 (1)	7.81
R- Q	0 / 162	-27.5	-27.5	0.40 (3)	10.00
Q- P	-111 / 0	-27.5	-27.5	0.31 (3)	6.25
P- O	-111 / 0	-27.5	-27.5	0.23 (3)	6.25
O- N	-111 / 0	-27.5	-27.5	0.23 (3)	6.25
N- M	0 / 967	-27.5	-27.5	0.39 (2)	10.00
M- L	0 / 967	-27.5	-27.5	0.39 (2)	10.00
L- K	0 / 1065	-27.5	-27.5	0.44 (2)	10.00
K- J	0 / 0	-27.5	-27.5	0.35 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-151 / 10	0.10 (1)
Q- D	0 / 583	0.13 (1)
P- D	-2745 / 0	0.86 (1)
K- I	0 / 1827	0.29 (1)
D- N	0 / 1861	0.30 (1)
K- H	-1268 / 0	0.88 (1)
N- E	-1323 / 0	0.92 (1)
L- H	0 / 465	0.07 (1)
E- L	0 / 636	0.10 (1)
L- G	-727 / 0	0.50 (1)
B- Q	-260 / 13	0.14 (1)
R- B	-351 / 0	0.18 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 804 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 474 (0.27")

CSI: TC=0.89/1.00 (I-J:1) , BC=0.44/1.00 (K-L:2) , WB=0.92/1.00 (E-N:1) , SSI=0.37/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.35 (N) (INPUT = 1.00)

DWG NO. TAM 13204-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T43S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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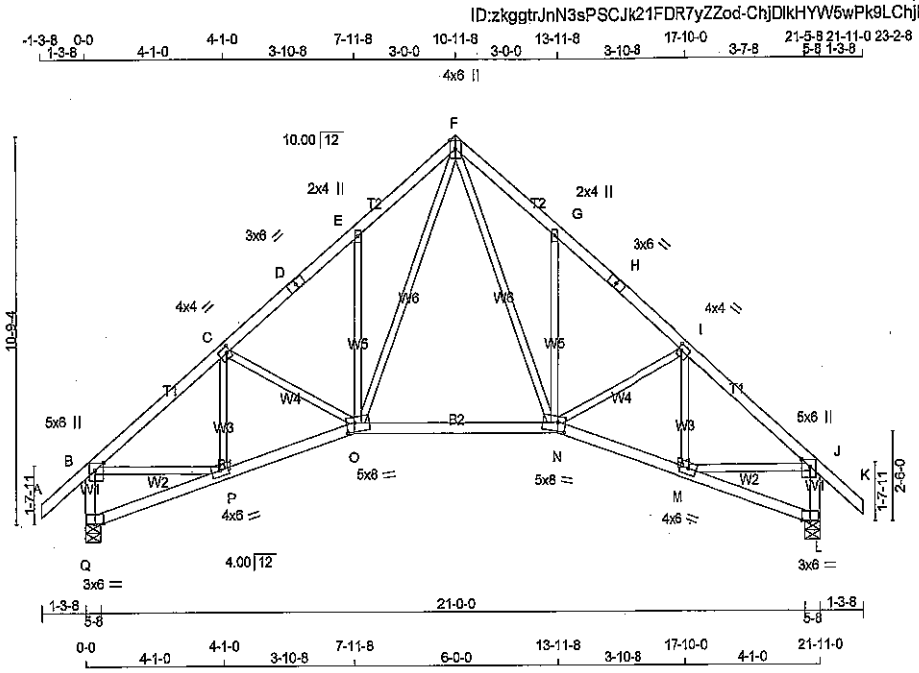

TOTAL WEIGHT = 7 X 106 = 741 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW+p	MT20	5.0	6.0	Edge	3.00
L	BVM1-p	MT20	3.0	6.0	Edge	
M	BMWW-t	MT20	4.0	6.0		
N	BBWWW-m	MT20	5.0	8.0	3.00	2.75
O	BBWWW-m	MT20	5.0	8.0	3.00	2.75
P	BMWW-t	MT20	4.0	6.0		
Q	BVM1-p	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1602	0	1602	0	0	5-8	5-8
L	1602	0	1602	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1259	802 / 0	230 / 0	0 / 0	0 / 0	227 / 0	0 / 0
L	1259	802 / 0	230 / 0	0 / 0	0 / 0	227 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1855 / 0	-104.9	-104.9	0.24 (1)	4.71
C- D	-1803 / 0	-104.9	-104.9	0.23 (1)	4.79
D- E	-1803 / 0	-104.9	-104.9	0.23 (1)	4.79
E- F	-1808 / 0	-104.9	-104.9	0.14 (1)	4.87
F- G	-1808 / 0	-104.9	-104.9	0.14 (1)	4.87
G- H	-1803 / 0	-104.9	-104.9	0.23 (1)	4.79
H- I	-1803 / 0	-104.9	-104.9	0.23 (1)	4.79
I- J	-1855 / 0	-104.9	-104.9	0.24 (1)	4.71
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1554 / 0	0.0	0.0	0.17 (1)	6.61
L- J	-1554 / 0	0.0	0.0	0.17 (1)	6.61
Q- P	0 / 7	-28.0	-28.0	0.12 (3)	10.00
P- O	0 / 1521	-28.0	-28.0	0.29 (1)	10.00
O- N	0 / 990	-28.0	-28.0	0.40 (2)	10.00
N- M	0 / 1521	-28.0	-28.0	0.29 (1)	10.00
M- L	0 / 7	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- N	0 / 1088	0.24 (1)
N- G	-367 / 0	0.19 (1)
N- I	-83 / 0	0.03 (1)
M- I	-382 / 21	0.09 (1)
O- F	0 / 1088	0.24 (1)
O- E	-367 / 0	0.19 (1)
C- O	-83 / 0	0.03 (1)
P- C	-382 / 21	0.09 (1)
B- P	0 / 1446	0.33 (1)
M- J	0 / 1446	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.40/1.00 (N-O:2) , WB=0.33/1.00 (B-P:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.67 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3 878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1320648
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T44	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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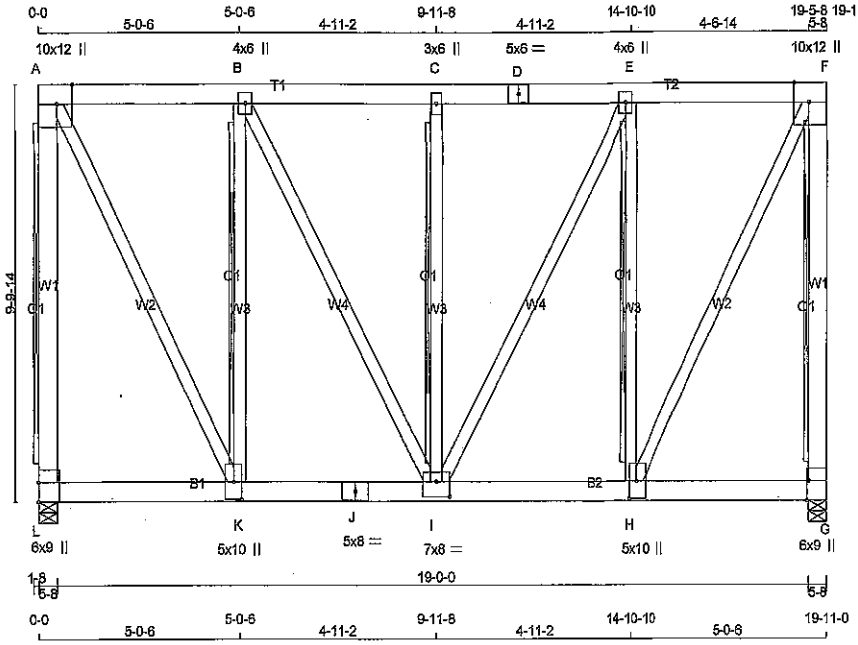

Scale = 1:54.5

TOTAL WEIGHT = 3 X 179 = 538 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L- A	2	12	TOP
A- D	2	12	TOP
D- F	2	12	TOP
F- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	3	5	SIDE(805.0)
J- G	3	5	SIDE(805.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	10.0	12.0	Edge	4.50
B	TMWW+t	MT20	4.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMWW+p	MT20	10.0	12.0	Edge	4.50
G	BMV1+t	MT20	6.0	9.0	Edge	0.50
H	BMWW+t	MT20	5.0	10.0	5.00	2.25
I	BMWWW-t	MT20	7.0	8.0	4.25	4.00
J	BS-t	MT20	5.0	8.0		
K	BMWW+t	MT20	5.0	10.0	5.00	2.25
L	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	13423	0	13423	0	0	5-8	5-8
G	13423	0	13423	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	10699	6522 / 0	2109 / 0	0 / 0	0 / 0	2068 / 0	0 / 0
G	10699	6522 / 0	2109 / 0	0 / 0	0 / 0	2068 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BEARING SIZE FACTOR = 1.15 AT JNT(S) L, G (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT A-L, F-G, B-K, C-I, E-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- A	-10932 / 0	0.0	0.0	0.96 (1)	7.81
A- B	-5730 / 0	-112.4	-112.4	0.09 (1)	2.00
B- C	-7352 / 0	-112.4	-112.4	0.09 (1)	2.00
C- D	-7352 / 0	-112.4	-112.4	0.09 (1)	2.00
D- E	-7352 / 0	-112.4	-112.4	0.09 (1)	2.00
E- F	-5730 / 0	-112.4	-112.4	0.09 (1)	2.00
G- F	-10932 / 0	0.0	0.0	0.96 (1)	7.81
L- K	0 / 0	-1235.4	-1235.4	0.69 (1)	10.00
K- J	0 / 5730	-1235.4	-1235.4	0.92 (1)	10.00
J- I	0 / 5730	-1235.4	-1235.4	0.92 (1)	10.00
I- H	0 / 5730	-1235.4	-1235.4	0.92 (1)	10.00
H- G	0 / 0	-1235.4	-1235.4	0.69 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- K	0 / 12106	0.65 (1)
K- B	-3649 / 0	0.39 (1)
B- I	0 / 3481	0.19 (1)
I- C	-567 / 0	0.06 (1)
I- E	0 / 3481	0.19 (1)
H- E	-3649 / 0	0.39 (1)
H- F	0 / 12106	0.65 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 38-4-0
END DISTANCE = 19-11-0
END SPAN CARRIED = 38-4-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.96/1.00 (F-G:1) , BC=0.92/1.00 (I-K:1) , WB=0.65/1.00 (A-K:1) , SSI=0.75/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.63 (J) (INPUT = 1.00)

DWG NO. TAM 132076

STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T45	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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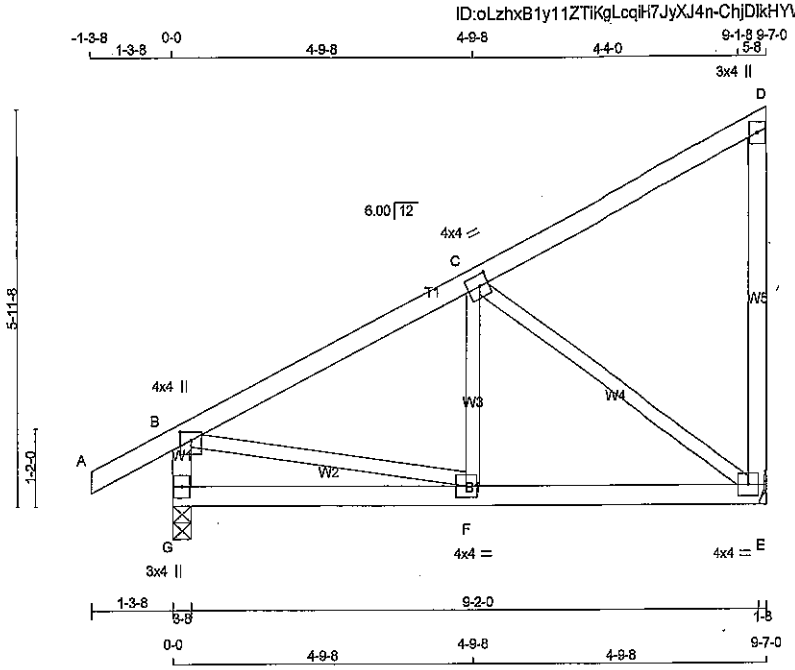

TOTAL WEIGHT = 4 X 42 = 169 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	637	0	637	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	779	0	779	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	508	311 / 0	101 / 0	0 / 0	0 / 0	96 / 0	0 / 0
G	604	399 / 0	101 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00
B- C	-620 / 0	-104.9	-104.9	0.31 (1)	6.25
C- D	-27 / 0	-104.9	-104.9	0.31 (1)	6.25
E- D	-191 / 0	0.0	0.0	0.13 (1)	7.81
G- B	-726 / 0	0.0	0.0	0.07 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.18 (3)	10.00
F- E	0 / 579	-28.0	-28.0	0.24 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 247	0.06 (3)
C- E	-700 / 0	0.38 (1)
B- F	0 / 588	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.24/1.00 (E-F:2) , WB=0.38/1.00 (C-E:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1320818
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T46	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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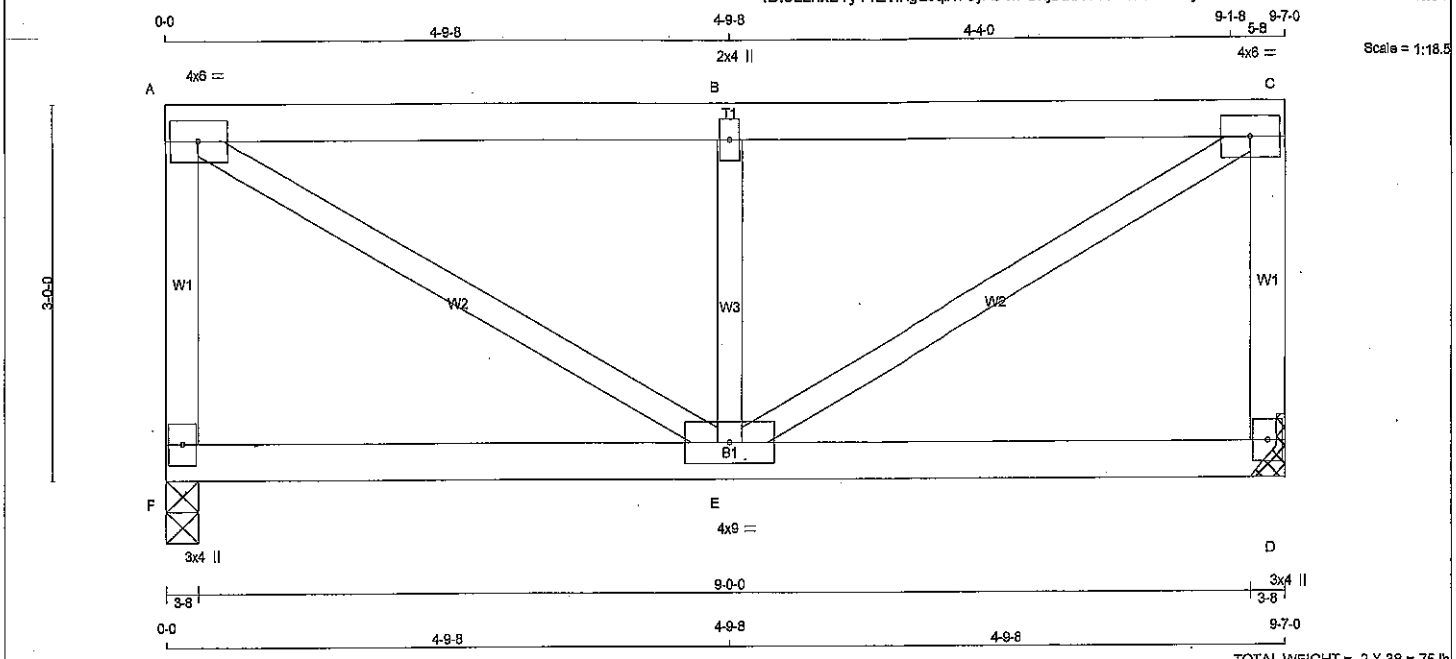

TOTAL WEIGHT = 2 X 38 = 75 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1947	0	1947	0	0	3-8	3-8
D	1947	0	1947	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1552	934 / 0	302 / 0	0 / 0	0 / 0	316 / 0	0 / 0
D	1552	934 / 0	302 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1885 / 0	0.0	0.0	0.15 (1)	7.81
A- B	-2110 / 0	-378.3	-378.3	0.82 (1)	2.00
B- C	-2110 / 0	-378.3	-378.3	0.82 (1)	2.00
D- C	-1885 / 0	0.0	0.0	0.15 (1)	7.81
F- E	0 / 0	-28.0	-28.0	0.09 (3)	10.00
E- D	0 / 0	-28.0	-28.0	0.09 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 2424	0.30 (1)
E- B	-2241 / 0	0.21 (1)
E- C	0 / 2424	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-0-0
END DISTANCE = 9-7-0
END SPAN CARRIED = 10-0-0
END WALL WIDTH = 0-0
APPLIED TO TOP EDGE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.82/1.00 (B-C:1) , BC=0.09/1.00 (D-E:3) , WB=0.30/1.00 (A-E:1) , SSI=0.48/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (A) (INPUT = 0.80)
JSI METAL= 0.26 (C) (INPUT = 1.00).

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 13209-B

STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T47	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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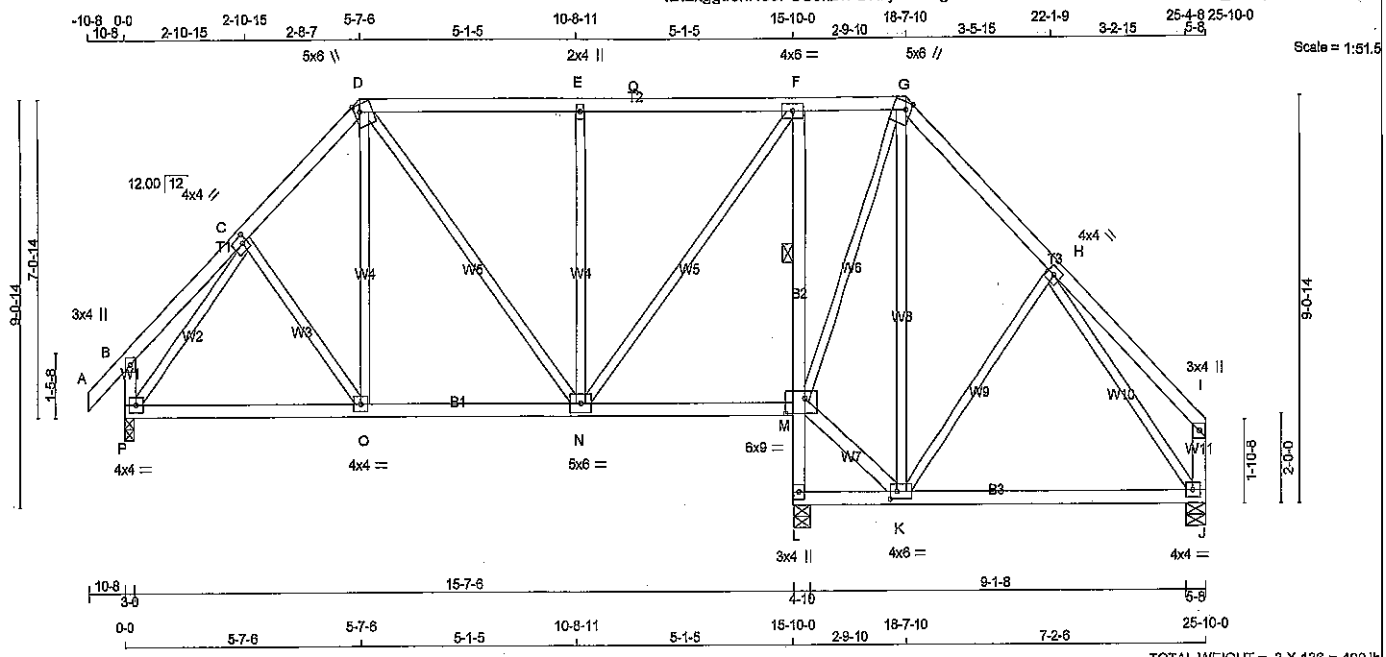

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW-t	MT20	4.0	4.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	9.0	4.00	5.50
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1738	0	1738	0	0	4-10	4-10
P	1208	0	1208	0	0	3-0	3-0
J	688	0	688	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1367	836 / 0	240 / 0	0 / 0	0 / 0	292 / 0	0 / 0
P	958	584 / 0	180 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	560	322 / 0	123 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, P, J

BRACING

FOR SECTION D-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-104.9	-104.9	0.07 (1)	10.00	
B- C	0 / 22	-104.9	-104.9	0.12 (1)	10.00	
C- D	-902 / 0	-104.9	-104.9	0.10 (1)	6.25	
D- E	-657 / 0	-112.4	-112.4	0.35 (1)	2.00	
E- Q	-658 / 0	-112.4	-112.4	0.34 (1)	2.00	
Q- F	-658 / 0	-112.4	-112.4	0.34 (1)	2.00	
F- G	-83 / 0	-112.4	-112.4	0.19 (1)	2.00	
G- H	-293 / 0	-104.9	-104.9	0.17 (1)	6.25	
H- I	0 / 32	-104.9	-104.9	0.23 (1)	10.00	
P- B	-216 / 0	0.0	0.0	0.02 (1)	7.61	
J- I	-141 / 0	0.0	0.0	0.02 (1)	7.61	
P- O	0 / 662	-28.0	-28.0	0.27 (2)	10.00	
O- N	0 / 623	-28.0	-28.0	0.27 (2)	10.00	
N- M	0 / 82	-28.0	-28.0	0.18 (3)	10.00	
L- M	-1754 / 0	0.0	0.0	0.04 (1)	6.29	
M- F	-1250 / 0	0.0	0.0	0.09 (1)	5.74	
L- K	-2 / 0	-28.0	-28.0	0.26 (3)	10.00	
K- J	0 / 324	-28.0	-28.0	0.36 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-70 / 60	0.03 (1)
O- D	0 / 276	0.06 (2)
D- N	0 / 57	0.01 (1)
N- E	-667 / 0	0.58 (1)
N- F	0 / 968	0.22 (1)
K- G	0 / 280	0.08 (2)
K- H	-236 / 13	0.17 (1)
P- C	-1139 / 0	0.42 (1)
H- J	-565 / 0	0.38 (1)
M- K	0 / 247	0.04 (1)
M- G	-304 / 0	0.30 (1)

LICENSED PROFESSIONAL ENGINEER
3378
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 672 (0.18")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.36/1.00 (J-K:2) , WB=0.58/1.00 (E-N:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)

DWG NO. TAM 13210-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T48S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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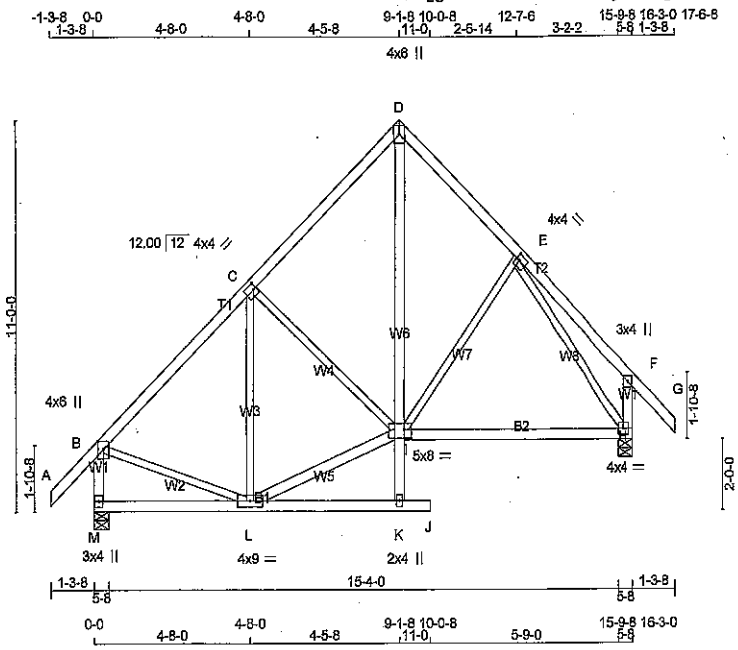

TOTAL WEIGHT = 3 X 94 = 281 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BWMWWW+I	MT20	5.0	8.0	3.00	2.50
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1238	0	1238	0	0	5-8	5-8
H	1241	0	1241	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	974	618 / 0	179 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	978	618 / 0	181 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15	(1)	10.00
B- C	-869 / 0	-104.9	-104.9	0.29	(1)	6.20
C- D	-763 / 0	-104.9	-104.9	0.29	(1)	6.25
D- E	-753 / 0	-104.9	-104.9	0.17	(1)	6.25
E- F	0 / 31	-104.9	-104.9	0.21	(1)	10.00
F- G	0 / 52	-104.9	-104.9	0.15	(1)	10.00
M- B	-1187 / 0	0.0	0.0	0.13	(1)	7.33
H- F	-286 / 0	0.0	0.0	0.03	(1)	7.81
M- L	0 / 0	-28.0	-28.0	0.17	(3)	10.00
L- K	0 / 4	-28.0	-28.0	0.17	(3)	10.00
K- J	0 / 0	-28.0	-28.0	0.03	(2)	10.00
I- H	0 / 595	-28.0	-28.0	0.48	(2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-338 / 59	0.24 (1)
K- I	0 / 127	0.03 (2)
I- D	0 / 696	0.12 (1)
B- L	0 / 676	0.15 (1)
I- E	-143 / 43	0.10 (1)
E- H	-1042 / 0	0.67 (1)
L- I	0 / 699	0.11 (1)
C- I	-162 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 680 (0.29")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.48/1.00 (H-I:2) , WB=0.67/1.00 (E-H:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 1321/ 8
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T49	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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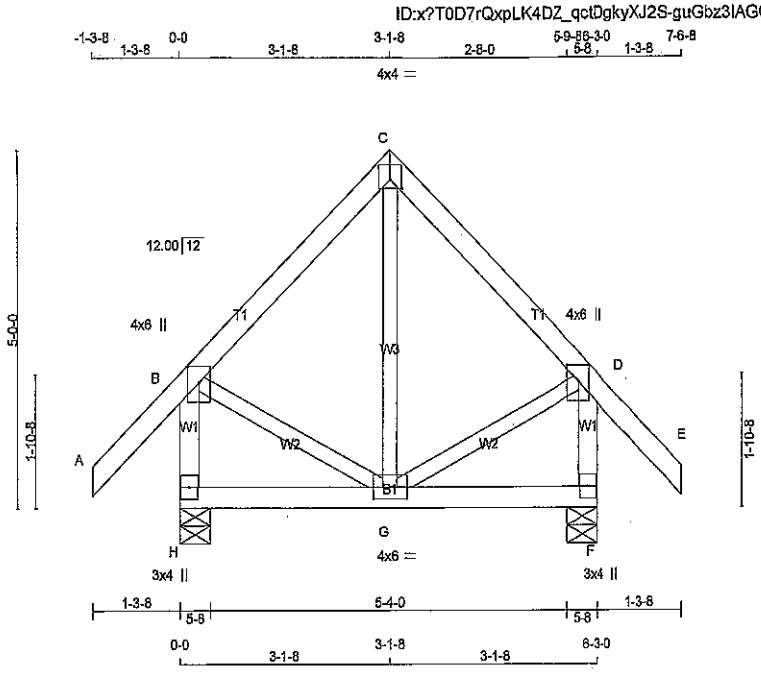

Scale = 1:32.3

TOTAL WEIGHT = 34 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	562	0	562	0	0	5-8	5-8
F	562	0	562	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	430	294 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0
F	430	294 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	
B- C	-206 / 0	-104.9	-104.9	0.18 (1)	6.25	
C- D	-206 / 0	-104.9	-104.9	0.18 (1)	6.25	
D- E	0 / 52	-104.9	-104.9	0.15 (1)	10.00	
H- B	-527 / 0	0.0	0.0	0.06 (1)	7.81	
F- D	-527 / 0	0.0	0.0	0.06 (1)	7.81	
H- G	0 / 0	-28.0	-28.0	0.08 (3)	10.00	
G- F	0 / 0	-28.0	-28.0	0.08 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-36 / 120	0.03 (3)
B- G	0 / 162	0.04 (1)
G- D	0 / 162	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.08/1.00 (G-H:3) , WB=0.04/1.00 (B-G:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (C) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
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PROVINCE OF ONTARIO

DWG NO. TAM 13212-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T50S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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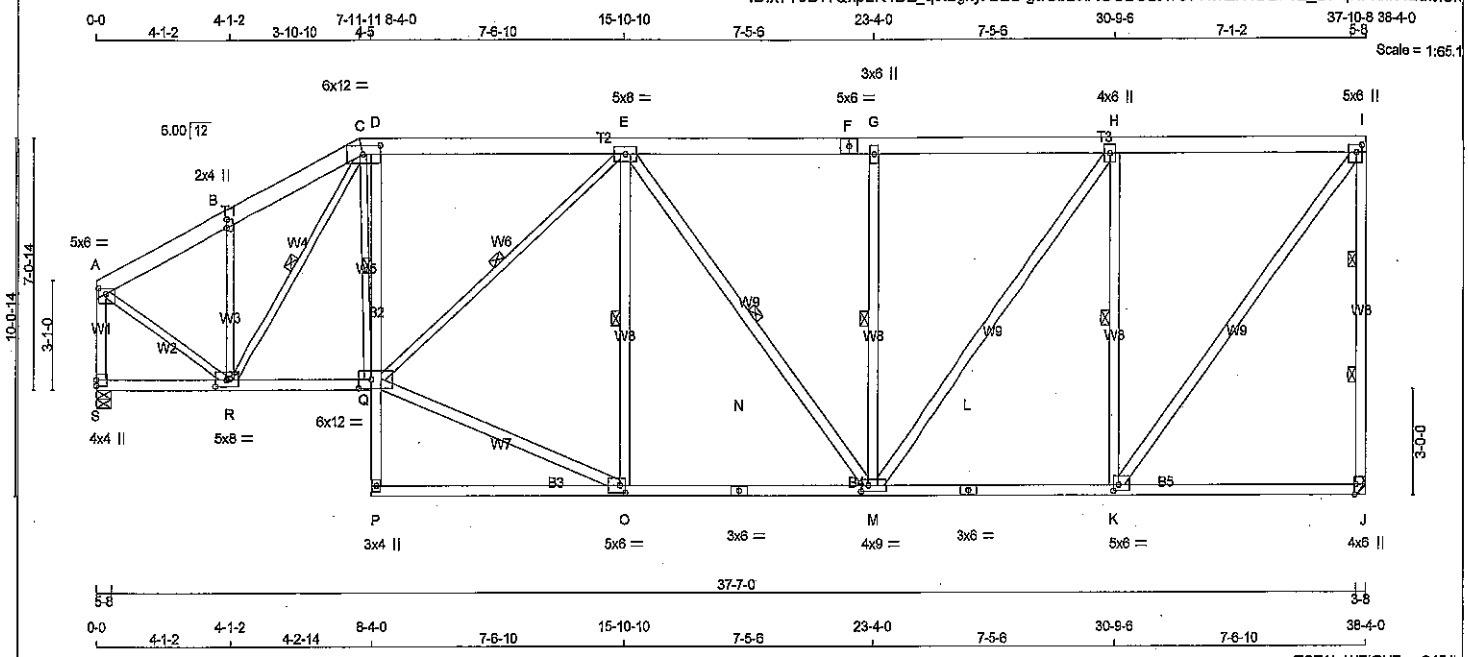

TOTAL WEIGHT = 245 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
R - B	2x3	DRY	No.2	SPF
A - R	2x3	DRY	No.2	SPF
Q - E	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
R - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.75	2.75
B	TMW+w	MT20	2.0	4.0	3.00	
C						
C	TTVWW*-I	MT20	6.0	12.0	3.00	6.25
D						
E	TMWWW-t	MT20	5.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV1+t	MT20	4.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0	2.00	2.50
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMV+p	MT20	3.0	4.0		
Q	BVMWWW-I	MT20	6.0	12.0	3.00	4.50
R	BMWWW-t	MT20	5.0	8.0	2.25	3.75
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2685	0	2685	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
S	2637	0	2637	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2141	1245 / 0	403 / 0	0 / 0	0 / 0	493 / 0	0 / 0
S	2102	1245 / 0	403 / 0	0 / 0	0 / 0	455 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

FOR SECTION C-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF I-J.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, E-O, E-M, G-M, H-K, E-Q, C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2328 / 0	-104.9	-104.9	0.11 (1)	5.30
B- C	-2352 / 0	-104.9	-104.9	0.11 (1)	5.27
C- D	-2625 / 0	-112.4	-112.4	0.05 (2)	2.00
D- E	-2622 / 0	-112.4	-112.4	0.38 (1)	2.00
E- F	-2577 / 0	-112.4	-112.4	0.38 (1)	2.00
F- G	-2577 / 0	-112.4	-112.4	0.38 (1)	2.00
G- H	-2577 / 0	-112.4	-112.4	0.38 (1)	2.00
H- I	-1766 / 0	-112.4	-112.4	0.37 (1)	2.00
J- I	-2601 / 0	0.0	0.0	0.58 (1)	4.26
S- A	-2594 / 0	0.0	0.0	0.40 (1)	5.33
S- R	0 / 0	-28.0	-28.0	0.15 (3)	10.00
R- Q	0 / 2519	-28.0	-28.0	0.48 (1)	10.00
P- Q	0 / 143	0.0	0.0	0.05 (1)	10.00
Q- D	-353 / 123	0.0	0.0	0.04 (1)	6.25
P- O	0 / 8	-28.0	-28.0	0.41 (3)	10.00
O- N	0 / 2652	-28.0	-28.0	0.68 (2)	10.00
N- M	0 / 2652	-28.0	-28.0	0.68 (2)	10.00
M- L	0 / 1766	-28.0	-28.0	0.59 (2)	10.00
L- K	0 / 1766	-28.0	-28.0	0.59 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.42 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-496 / 0	0.18 (1)
O- E	-829 / 0	0.39 (1)
E- M	-124 / 0	0.11 (1)
M- G	-770 / 0	0.37 (1)
M- H	0 / 1334	0.21 (1)
K- H	-2020 / 0	0.96 (1)
K- I	0 / 2869	0.46 (1)
A- R	0 / 2509	0.56 (1)
Q- O	0 / 2852	0.46 (1)
Q- E	-47 / 0	0.03 (2)
C- Q	0 / 1582	0.36 (1)
R- C	-852 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.58/1.00 (I-J:1) , BC=0.68/1.00 (M-O:2) , WB=0.98/1.00 (H-K:1) , SSI=0.31/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.79 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13213-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289210	T51S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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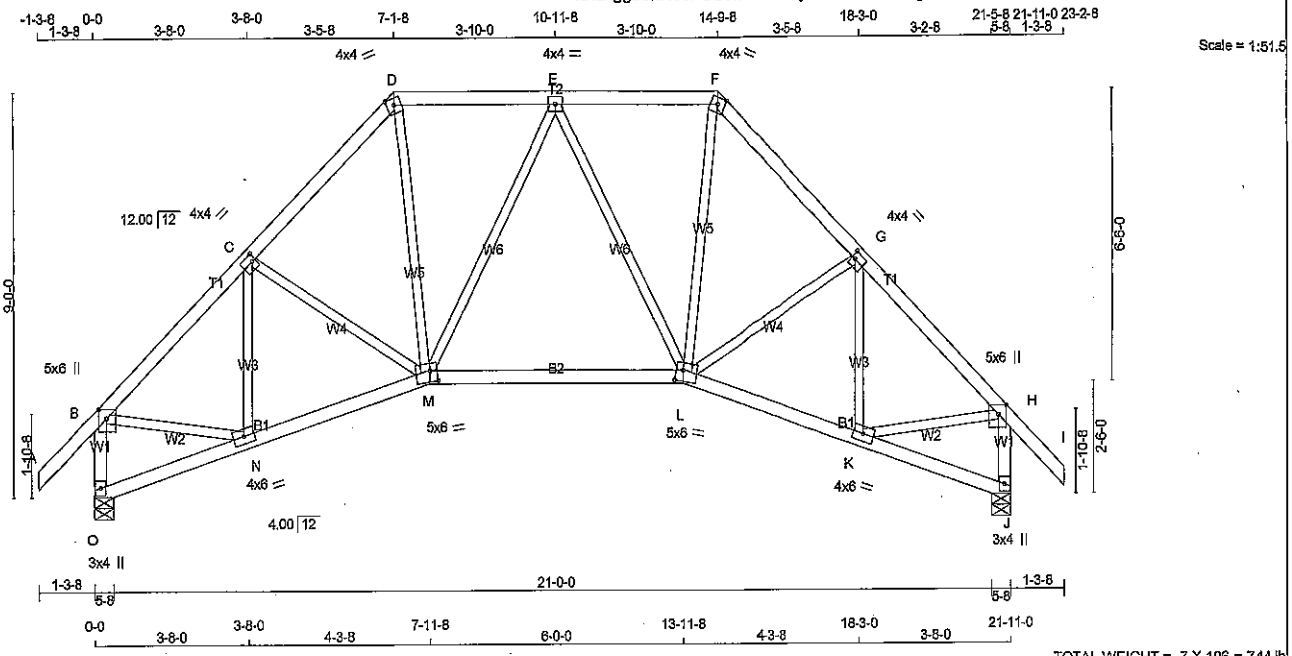

TOTAL WEIGHT = 7 X 106 = 744 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-m	MT20	4.0	4.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	1.75	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.00
H	TMVW+p	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BBWWW-m	MT20	5.0	6.0	3.00	2.00
M	BBWWW-m	MT20	5.0	6.0	3.00	2.00
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1626	0	1626	0	0	5-8	5-8
J	1626	0	1626	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1272	802 / 0	219 / 0	0 / 0	0 / 0	251 / 0	0 / 0
J	1272	802 / 0	219 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	-1597 / 0	-104.9	-104.9	0.20 (1)	5.05
C- D	-1597 / 0	-104.9	-104.9	0.19 (1)	5.05
D- E	-1210 / 0	-112.4	-112.4	0.21 (1)	2.00
E- F	-1210 / 0	-112.4	-112.4	0.21 (1)	2.00
F- G	-1597 / 0	-104.9	-104.9	0.19 (1)	5.05
G- H	-1597 / 0	-104.9	-104.9	0.20 (1)	5.05
H- I	0 / 52	-104.9	-104.9	0.15 (1)	10.00
O- B	-1587 / 0	0.0	0.0	0.18 (1)	6.55
J- H	-1587 / 0	0.0	0.0	0.18 (1)	6.55
O- N	0 / 8	-27.5	-27.5	0.13 (3)	10.00
N- M	0 / 1209	-27.5	-27.5	0.25 (1)	10.00
M- L	0 / 1334	-27.5	-27.5	0.43 (2)	10.00
L- K	0 / 1209	-27.5	-27.5	0.25 (1)	10.00
K- J	0 / 8	-27.5	-27.5	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-391 / 9	0.12 (1)
C- M	-56 / 0	0.02 (1)
D- M	0 / 806	0.18 (1)
M- E	-283 / 0	0.26 (1)
E- L	-283 / 0	0.26 (1)
L- F	0 / 806	0.18 (1)
L- G	-56 / 0	0.02 (1)
K- G	-391 / 9	0.12 (1)
B- N	0 / 1158	0.26 (1)
K- H	0 / 1159	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.43/1.00 (L-M:2) , WB=0.26/1.00 (B-N:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1322 [illegible]
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287646	T210S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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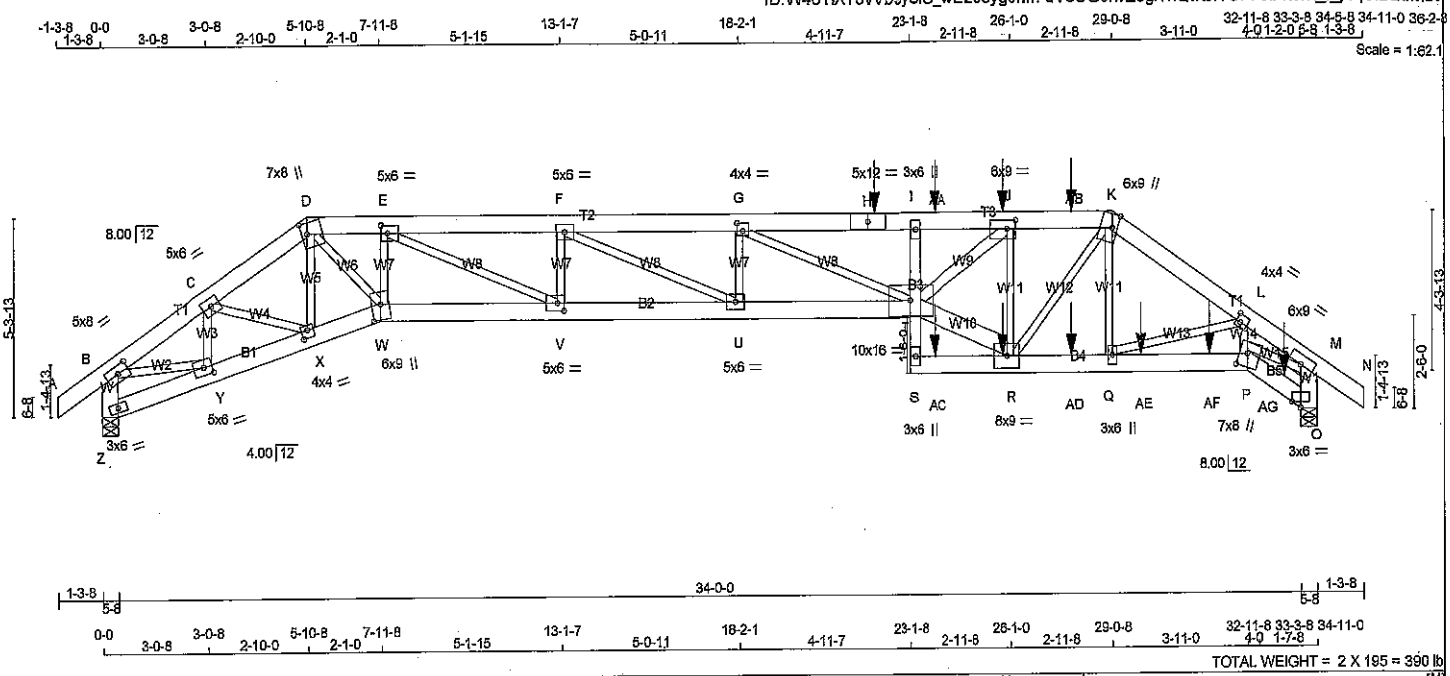

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	1650F 1.5E	SPF
D - H	2x6	DRY	1650F 1.5E	SPF
H - K	2x6	DRY	1650F 1.5E	SPF
K - N	2x6	DRY	1650F 1.5E	SPF
Z - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
S - I	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
T - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- H	2	12	SIDE(0.0)
H- K	2	12	SIDE(183.1)
K- N	2	12	TOP
Z- B	2	12	TOP
O- M	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- W	2	12	TOP
W- T	2	12	TOP
S- P	2	12	SIDE(183.1)
O- P	2	12	SIDE(0.0)
I- S	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	3.75
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	7.0	8.0	3.50	1.75
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMWW-t	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.50	2.00
H	TS-t	MT20	5.0	12.0		
I	TMV+p	MT20	3.0	6.0		
J	TMWW-t	MT20	6.0	9.0	2.75	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	3381	0	3381	0	0	5-8	5-8
O	4590	0	4590	0	0	5-8	5-8

ALLOW FOR 0.5" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

SIMPSON 2-TC26 @ 9.2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2881	1666 / 0	515 / 0	0 / 0	0 / 0	500 / 0	0 / 0
O	3876	2220 / 0	746 / 0	0 / 0	0 / 0	710 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-104.9	-104.9	0.03 (1)	10.00	
B- C	-5324 / 0	-104.9	-104.9	0.06 (1)	5.63	
C- D	-6729 / 0	-104.9	-104.9	0.07 (1)	5.15	
D- E	-9171 / 0	-104.9	-104.9	0.07 (1)	4.57	
E- F	-13011 / 0	-104.9	-104.9	0.22 (1)	3.84	
F- G	-15620 / 0	-104.9	-104.9	0.27 (1)	3.50	
G- H	-15810 / 0	-104.9	-104.9	0.60 (1)	3.08	
H- I	-15810 / 0	-104.9	-104.9	0.60 (1)	3.08	
I-AA	-15678 / 0	-104.9	-104.9	0.31 (1)	3.43	
AA- J	-15678 / 0	-104.9	-104.9	0.31 (1)	3.43	
J-AB	-7940 / 0	-104.9	-104.9	0.13 (1)	4.77	
AB- K	-7940 / 0	-104.9	-104.9	0.13 (1)	4.77	
K- L	-7138 / 0	-104.9	-104.9	0.15 (1)	4.94	
L- M	-8261 / 0	-104.9	-104.9	0.14 (1)	4.67	
M- N	0 / 41	-104.9	-104.9	0.03 (1)	10.00	
Z- B	-3276 / 0	0.0	0.0	0.12 (1)	7.67	
O- M	-4544 / 0	0.0	0.0	0.17 (1)	6.76	
Z- Y	-19 / 0	-28.0	-28.0	0.07 (1)	6.25	
Y- X	0 / 4611	-28.0	-28.0	0.37 (1)	10.00	
X- W	0 / 5852	-28.0	-28.0	0.42 (1)	10.00	
W- V	0 / 9263	-28.0	-28.0	0.43 (1)	10.00	
V- U	0 / 13011	-28.0	-28.0	0.62 (1)	10.00	
U- T	0 / 15620	-28.0	-28.0	0.71 (1)	10.00	
S- T	0 / 198	0.0	0.0	0.25 (1)	10.00	
T- I	-2076 / 0	0.0	0.0	0.26 (1)	7.26	
S-AC	0 / 207	-28.0	-28.0	0.05 (1)	10.00	
AC- R	0 / 207	-28.0	-28.0	0.05 (1)	10.00	
R-AD	0 / 5995	-28.0	-28.0	0.47 (1)	10.00	
AD- Q	0 / 5995	-28.0	-28.0	0.47 (1)	10.00	
Q-AE	0 / 6716	-28.0	-28.0	0.52 (1)	10.00	
AE-AF	0 / 6716	-28.0	-28.0	0.52 (1)	10.00	
AF- P	0 / 6716	-28.0	-28.0	0.52 (1)	10.00	
P-AG	0 / 2	-28.0	-28.0	0.02 (2)	10.00	
AG- O	0 / 2	-28.0	-28.0	0.02 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-1525 / 0	0.13 (1)
C- X	0 / 1336	0.17 (1)
X- D	-614 / 0	0.08 (1)
D- W	0 / 5394	0.67 (1)
W- E	-2100 / 0	0.18 (1)
R- J	-6223 / 0	0.80 (1)
R- K	0 / 3226	0.40 (1)
Q- K	0 / 793	0.10 (1)
B- Y	0 / 4412	0.55 (1)
T- R	0 / 8839	0.57 (1)
T- J	0 / 9963	0.88 (1)
E- V	0 / 4135	0.51 (1)
V- F	-1620 / 0	0.14 (1)
F- U	0 / 2880	0.36 (1)
U- G	-1164 / 0	0.10 (1)
G- T	-773 / 209	0.20 (5)
P- L	0 / 753	0.09 (1)
P- M	0 / 6793	0.84 (1)
Q- L	-781 / 0	0.12 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 757 (0.55")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 486 (0.86")

CSI: TC=0.60/1.00 (G-I:1) , BC=0.71/1.00 (T-U:1) , WB=0.88/1.00 (J-T:1) , SSI=0.53/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.95 (P) (INPUT = 1.00)

P6/6

DWG NO. TAM 13259.48
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287646	T210S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TTWW+m	MT20	6.0	9.0	Edge	1.75
L	TMWW-t	MT20	4.0	4.0	2.50	1.50
M	TMVW-t	MT20	6.0	9.0		
O	BVM1-p	MT20	3.0	6.0	Edge	3.00
P	BBWW+m	MT20	7.0	8.0	4.25	2.75
Q	BMWW+t	MT20	3.0	6.0		
R	BMWWW-t	MT20	8.0	9.0		
S	BMV+p	MT20	3.0	6.0		
T	BVMWWW-l	MT20	10.0	16.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.25
W	BBWW+m	MT20	6.0	9.0	5.00	3.00
X	BMWW-t	MT20	4.0	4.0	2.50	2.00
Y	BMWW-t	MT20	5.0	6.0	2.50	2.75
Z	BVM1-l	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1775.0 lbs FACTORED DOWN AT 22-0-8, 11.5 lbs FACTORED DOWN AND 9.4 lbs FACTORED UP AT 23-10-4, AND 11.5 lbs FACTORED DOWN AND 9.4 lbs FACTORED UP AT 25-10-4, AND 11.5 lbs FACTORED DOWN AND 9.4 lbs FACTORED UP AT 27-10-4 ON TOP CHORD, AND 240.8 lbs FACTORED DOWN AT 23-10-4, 235.0 lbs FACTORED DOWN AT 25-10-4, 235.0 lbs FACTORED DOWN AT 27-10-4, 235.0 lbs FACTORED DOWN AT 29-10-4, AND 235.0 lbs FACTORED DOWN AT 31-10-4, AND 74.3 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	22-0-8	-1775	-1775	—	FRONT	VERT	TOTAL
J	25-10-4	-11	-11	9	FRONT	VERT	TOTAL
R	25-10-4	-235	-235	—	FRONT	VERT	TOTAL
AA	23-10-4	-11	-11	9	FRONT	VERT	TOTAL
AB	27-10-4	-11	-11	9	FRONT	VERT	TOTAL
AC	23-10-4	-241	-241	—	FRONT	VERT	TOTAL
AD	27-10-4	-235	-235	—	FRONT	VERT	TOTAL
AE	29-10-4	-235	-235	—	FRONT	VERT	TOTAL
AF	31-10-4	-235	-235	—	FRONT	VERT	TOTAL
AG	33-10-4	-42	-74	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13259.18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T210S5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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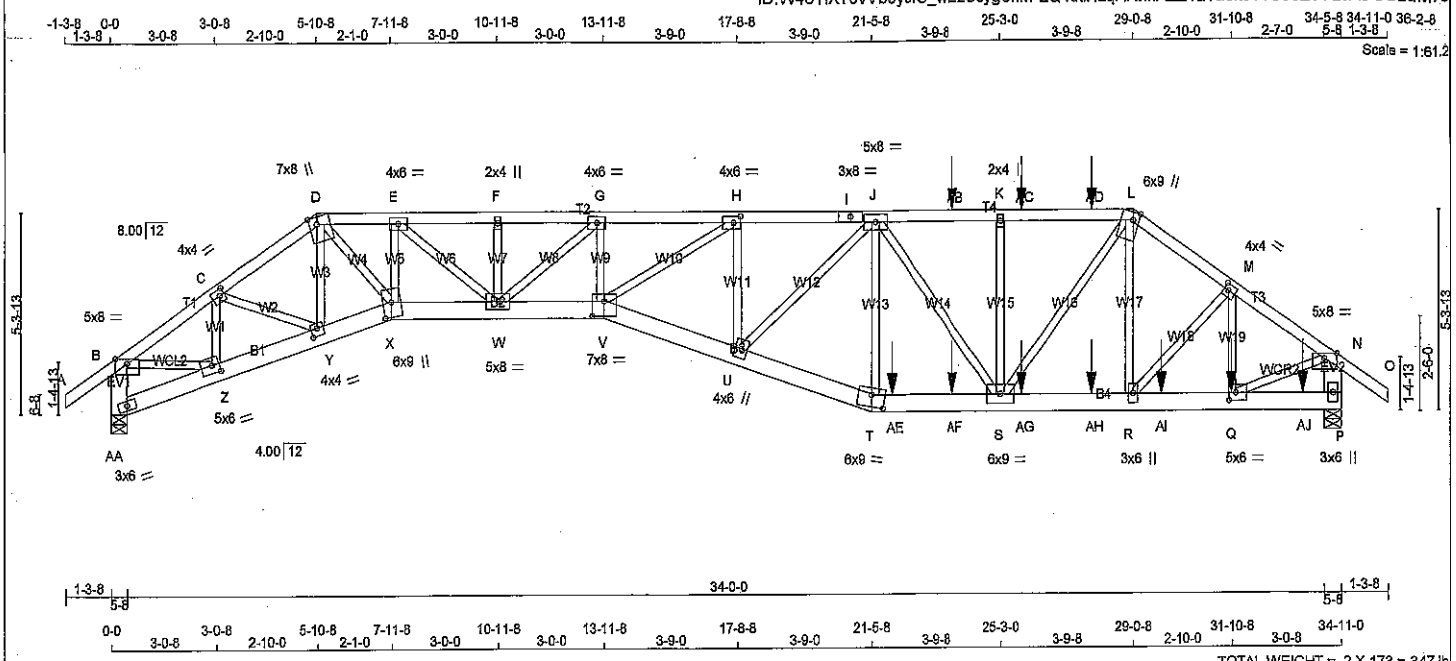

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- X	2x6	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- I	1	12	TOP
I- L	1	12	SIDE(0.0)
L- O	1	12	TOP
AA- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- X	2	12	TOP
X- V	2	12	TOP
V- T	2	12	TOP
T- P	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	4.0	6.0	2.00	2.25
I	TS-t	MT20	3.0	8.0		
J	TMWWW-t	MT20	5.0	8.0		
K	TMW+w	MT20	2.0	4.0		
L	TTWW+m	MT20	6.0	9.0	Edge	2.00
M	TMWW-t	MT20	4.0	4.0	2.00	1.50
N	TMVW-p	MT20	5.0	8.0	Edge	
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AA	3360	0	3360	0	0	5-8	5-8
P	4328	0	4328	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	2656	1665 / 0	502 / 0	0 / 0	0 / 0	489 / 0	0 / 0
P	3471	2092 / 0	712 / 0	0 / 0	0 / 0	668 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD LC1 (PLF) TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-5022 / 0	-104.9	-104.9	0.16 (1)	4.19
C- D	-6499 / 0	-104.9	-104.9	0.21 (1)	3.70
D- E	-8770 / 0	-104.9	-104.9	0.33 (1)	3.12
E- F	-11022 / 0	-104.9	-104.9	0.53 (1)	2.59
F- G	-11022 / 0	-104.9	-104.9	0.54 (1)	2.58
G- H	-13091 / 0	-104.9	-104.9	0.83 (1)	2.08
H- I	-9803 / 0	-104.9	-104.9	0.51 (1)	2.81
I- J	-9803 / 0	-104.9	-104.9	0.51 (1)	2.81
J-AB	-6661 / 0	-104.9	-104.9	0.39 (1)	3.47
AB- K	-6661 / 0	-104.9	-104.9	0.39 (1)	3.47
K-AC	-6660 / 0	-104.9	-104.9	0.41 (1)	3.46
AC-AD	-6660 / 0	-104.9	-104.9	0.41 (1)	3.46
AD- L	-6660 / 0	-104.9	-104.9	0.41 (1)	3.46
L- M	-5402 / 0	-104.9	-104.9	0.16 (1)	4.08
M- N	-4744 / 0	-104.9	-104.9	0.15 (1)	4.32
N- O	0 / 40	-104.9	-104.9	0.08 (1)	10.00
AA- B	-3247 / 0	0.0	0.0	0.12 (1)	7.69
P- N	-4215 / 0	0.0	0.0	0.15 (1)	6.96
AA- Z	-22 / 0	-28.0	-28.0	0.08 (1)	6.25
Z- Y	0 / 4444	-28.0	-28.0	0.37 (1)	10.00
Y- X	0 / 5637	-28.0	-28.0	0.39 (1)	10.00
X- W	0 / 8856	-28.0	-28.0	0.71 (1)	10.00
W- V	0 / 13032	-28.0	-28.0	0.98 (1)	10.00
V- U	0 / 10338	-28.0	-28.0	0.72 (1)	10.00
U- T	0 / 8177	-28.0	-28.0	0.58 (1)	10.00
T-AE	0 / 7783	-28.0	-28.0	0.85 (1)	10.00
AE-AF	0 / 7783	-28.0	-28.0	0.85 (1)	10.00
AF- S	0 / 7783	-28.0	-28.0	0.85 (1)	10.00
S-AG	0 / 4466	-28.0	-28.0	0.34 (1)	10.00
AG-AH	0 / 4466	-28.0	-28.0	0.34 (1)	10.00
AH- R	0 / 4466	-28.0	-28.0	0.34 (1)	10.00
R-AI	0 / 3958	-28.0	-28.0	0.31 (1)	10.00
AI- Q	0 / 3958	-28.0	-28.0	0.31 (1)	10.00
Q-AJ	0 / 0	-28.0	-28.0	0.05 (1)	10.00
AJ- P	0 / 0	-28.0	-28.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- C	-1650 / 0	0.14 (1)
C- Y	0 / 1292	0.16 (1)
Y- D	-649 / 0	0.07 (1)
D- X	0 / 5221	0.65 (1)
X- E	-2021 / 0	0.18 (1)
E- W	0 / 2834	0.35 (1)
W- F	-329 / 0	0.03 (1)
W- G	-2629 / 0	0.34 (1)
V- G	0 / 1388	0.17 (1)
V- H	0 / 3921	0.49 (1)
U- H	-2560 / 0	0.32 (1)
U- J	0 / 2820	0.35 (1)
T- J	-896 / 0	0.18 (1)
J- S	-1875 / 0	0.61 (1)
S- K	-832 / 0	0.17 (1)
S- L	0 / 3599	0.45 (1)
R- L	-297 / 129	0.06 (1)
R- M	0 / 738	0.09 (1)
Q- M	-1253 / 0	0.13 (1)
B- Z	0 / 4190	0.52 (1)
Q- N	0 / 4165	0.52 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 867 (0.47")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 573 (0.73")

CSI: TC=0.83/1.00 (G-H:1) , BC=0.98/1.00 (V-W:1) , WB=0.65/1.00 (D-X:1) , SSI=0.53/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.72 (V) (INPUT = 1.00)

P6 1/2

DWG NO. TAM 132498
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	T210S5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW+t	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BBW-m	MT20	6.0	9.0	3.75	4.50
U	BMWW+t	MT20	4.0	6.0		
V	BBWW-p	MT20	7.0	8.0	4.75	4.00
W	BMWWW-t	MT20	5.0	8.0		
X	BBWW+m	MT20	6.0	9.0	4.75	2.75
Y	BMWW-t	MT20	4.0	4.0	2.50	2.00
Z	BMWW-t	MT20	5.0	6.0	2.50	2.25
AA	BVM1-l	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 203.3 lbs FACTORED DOWN AT 23-10-4, AND 203.3 lbs FACTORED DOWN AT 25-10-4, AND 203.3 lbs FACTORED DOWN AT 27-10-4 ON TOP CHORD, AND 1858.0 lbs FACTORED DOWN AT 22-0-8, 87.4 lbs FACTORED DOWN AT 23-10-4, 87.4 lbs FACTORED DOWN AT 25-10-4, 87.4 lbs FACTORED DOWN AT 27-10-4, 87.4 lbs FACTORED DOWN AT 29-10-4, AND 87.4 lbs FACTORED DOWN AT 31-10-4, AND 71.7 lbs FACTORED DOWN AT 33-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	31-10-4	-50	-87	—	FRONT	VERT	TOTAL
AB	23-10-4	-203	-203	—	FRONT	VERT	TOTAL
AC	25-10-4	-203	-203	—	FRONT	VERT	TOTAL
AD	27-10-4	-203	-203	—	FRONT	VERT	TOTAL
AE	22-0-8	-1858	-1858	—	FRONT	VERT	TOTAL
AF	23-10-4	-50	-87	—	FRONT	VERT	TOTAL
AG	25-10-4	-50	-87	—	FRONT	VERT	TOTAL
AH	27-10-4	-50	-87	—	FRONT	VERT	TOTAL
AI	29-10-4	-50	-87	—	FRONT	VERT	TOTAL
AJ	33-10-4	-41	-72	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3-8-18
S KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO. TAM 1324/9-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	T470	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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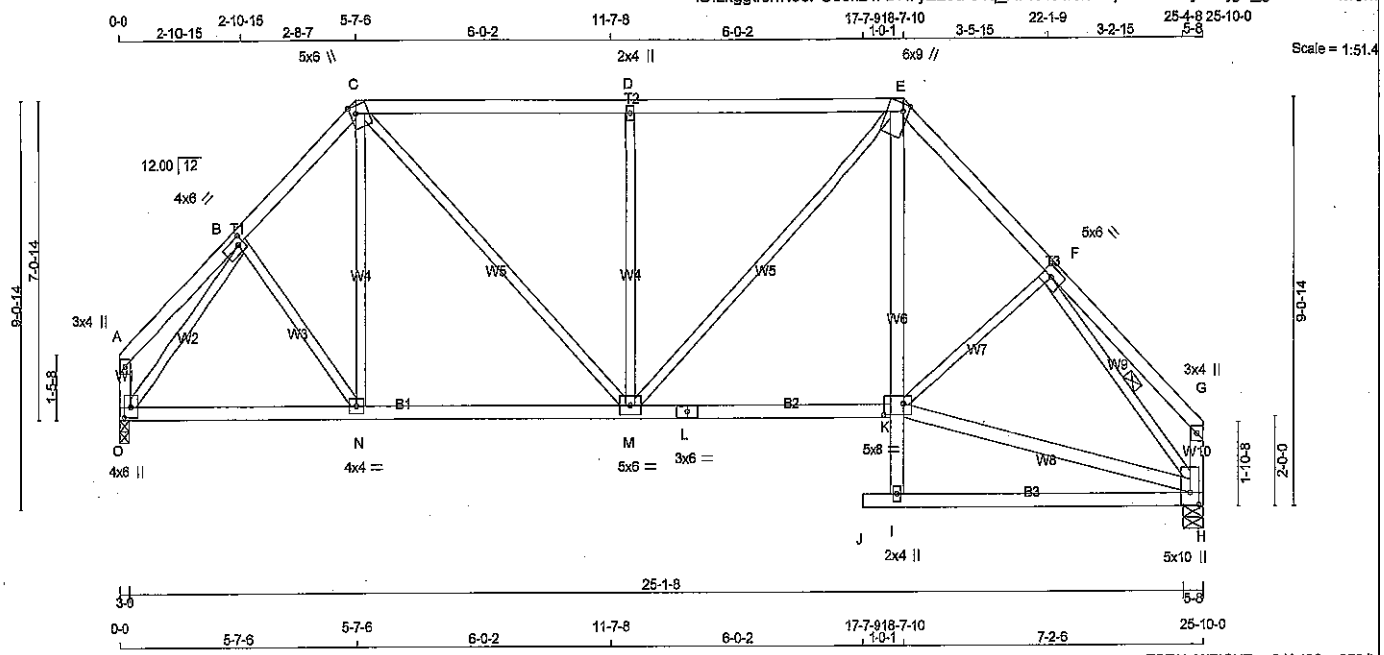

TOTAL WEIGHT = 2 X 128 = 256 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMWW-t	MT20	5.0	6.0	2.50	2.75
G	TMV+p	MT20	3.0	4.0		
H	BMVWW1+p	MT20	5.0	10.0	3.25	2.25
I	BMW+w	MT20	2.0	4.0		
K	BWMWW*-I	MT20	5.0	6.0	3.00	5.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0	2.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1769	0	1769	0	0	3-0	3-0
H	1773	0	1773	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1405	839 / 0	263 / 0	0 / 0	0 / 0	303 / 0	0 / 0
H	1413	839 / 0	271 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-104.9	-104.9	0.11 (1)	10.00	
B- C	-1690 / 0	-104.9	-104.9	0.12 (1)	5.02	
C- D	-1803 / 0	-112.4	-112.4	0.67 (1)	2.00	
D- E	-1803 / 0	-112.4	-112.4	0.67 (1)	2.00	
E- F	-1983 / 0	-104.9	-104.9	0.21 (1)	4.63	
F- G	0 / 28	-104.9	-104.9	0.20 (1)	10.00	
O- A	-117 / 0	0.0	0.0	0.01 (1)	7.81	
H- G	-148 / 0	0.0	0.0	0.02 (1)	7.81	
O- N	0 / 1136	-27.5	-27.5	0.34 (2)	10.00	
N- M	0 / 1177	-27.5	-27.5	0.39 (2)	10.00	
M- L	0 / 1393	-27.5	-27.5	0.41 (2)	10.00	
L- K	0 / 1393	-27.5	-27.5	0.41 (2)	10.00	
J- I	0 / 0	-27.5	-27.5	0.02 (3)	10.00	
I- H	0 / 16	-27.5	-27.5	0.45 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- N	0 / 116	0.03 (3)
N- C	0 / 220	0.05 (3)
C- M	0 / 905	0.20 (1)
M- D	-893 / 0	0.77 (1)
I- K	0 / 215	0.06 (1)
K- E	0 / 526	0.13 (1)
O- B	-1954 / 0	0.71 (1)
M- E	0 / 589	0.13 (1)
K- H	0 / 1357	0.22 (1)
K- F	0 / 134	0.03 (3)
F- H	-2305 / 0	0.56 (1)

LICENSED PROFESSIONAL ENGINEER
3-8-16
S. KATSOULIDIS
PROVINCE OF ONTARIO

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 908 (0.34")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.45/1.00 (H-I:3) , WB=0.77/1.00 (D-M:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.57 (F) (INPUT = 1.00)

DWG NO. TAM 13214-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	G7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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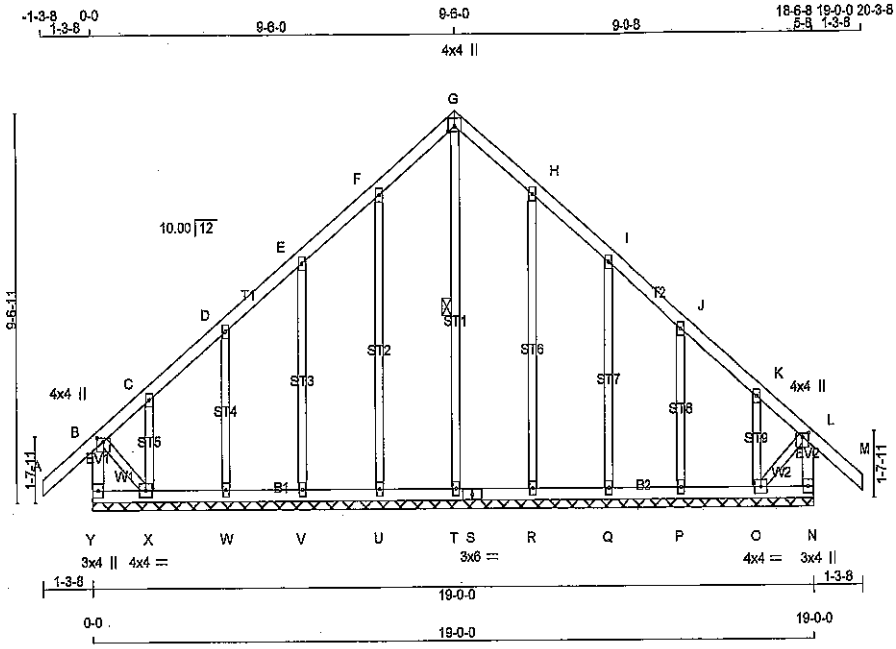

TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-334 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-80 / 0	-104.9	-104.9	0.14 (1)	6.25	
C- D	-26 / 0	-104.9	-104.9	0.05 (1)	6.25	
D- E	-31 / 0	-104.9	-104.9	0.05 (1)	6.25	
E- F	-20 / 0	-104.9	-104.9	0.06 (1)	6.25	
F- G	-34 / 0	-104.9	-104.9	0.06 (1)	6.25	
G- H	-34 / 0	-104.9	-104.9	0.06 (1)	6.25	
H- I	-20 / 0	-104.9	-104.9	0.06 (1)	6.25	
I- J	-31 / 0	-104.9	-104.9	0.05 (1)	6.25	
J- K	-26 / 0	-104.9	-104.9	0.05 (1)	6.25	
K- L	-80 / 0	-104.9	-104.9	0.14 (1)	6.25	
L- M	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
N- L	-334 / 0	0.0	0.0	0.04 (1)	7.81	
Y- X	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
X- W	0 / 27	-28.0	-28.0	0.02 (2)	10.00	
W- V	0 / 22	-28.0	-28.0	0.02 (2)	10.00	
V- U	0 / 19	-28.0	-28.0	0.02 (2)	10.00	
U- T	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
T- S	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
S- R	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
R- Q	0 / 19	-28.0	-28.0	0.02 (2)	10.00	
Q- P	0 / 22	-28.0	-28.0	0.02 (2)	10.00	
P- O	0 / 27	-28.0	-28.0	0.02 (2)	10.00	
O- N	0 / 0	-28.0	-28.0	0.02 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-151 / 0	0.09 (1)
U- F	-239 / 0	0.28 (1)
V- E	-198 / 0	0.12 (1)
W- D	-224 / 0	0.07 (1)
X- C	-98 / 0	0.02 (1)
R- H	-239 / 0	0.28 (1)
Q- I	-198 / 0	0.12 (1)
P- J	-224 / 0	0.07 (1)
O- K	-98 / 0	0.02 (1)
B- X	0 / 40	0.01 (1)
O- L	0 / 40	0.01 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (W-X:2) , WB=0.28/1.00 (H-R:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)

DWG NO. TAM 131868
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G41S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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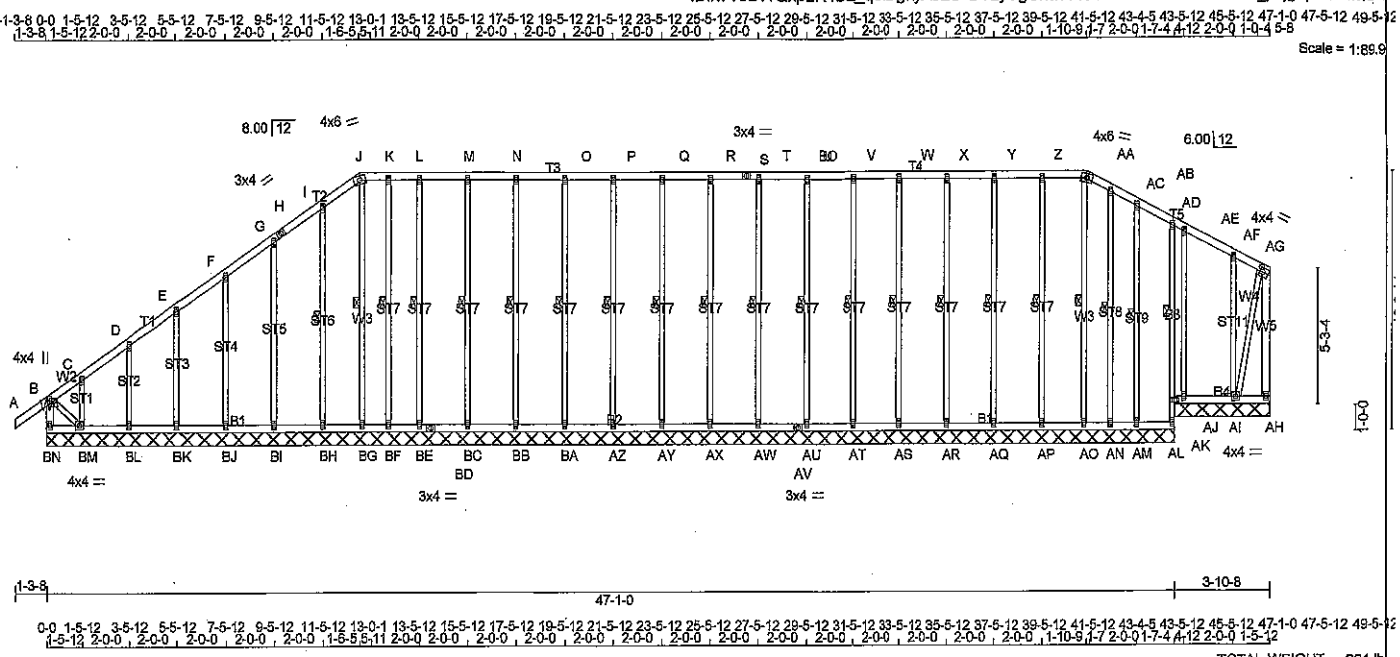

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
BN- B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - S	2x4	DRY	No.2	SPF
S - AA	2x4	DRY	No.2	SPF
AA- AG	2x4	DRY	No.2	SPF
AH- AG	2x4	DRY	No.2	SPF
BN- BD	2x4	DRY	No.2	SPF
BD- AV	2x4	DRY	No.2	SPF
AV- AL	2x4	DRY	No.2	SPF
AL- AD	2x3	DRY	No.2	SPF
AK- AH	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT.	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, G, I, K, L, M, N, O, P, Q, R, T, U, V, W, X, Y, Z, AB, AC, AE, AF						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	4.0		
J	TTW-m	MT20	4.0	6.0		
S	TS-t	MT20	3.0	4.0		
AA	TTW-m	MT20	4.0	6.0		
AD	TMV+p	MT20	2.0	4.0		
AG	TMVW-t	MT20	4.0	4.0	2.00	1.25
AH, AL, BN						
AH	BMV1+p	MT20	2.0	4.0		
AI	BMWW1-t	MT20	4.0	4.0		
AJ, AM, AN, AO, AP, AQ, AR, AS, AT, AU, AW, AX, AY, AZ, BA, BB, BC, BE, BF, BG, BH, BI, BJ, BK, BL						
AJ	BMW1+w	MT20	2.0	4.0		
AK	BVM-l	MT20	2.0	4.0		
AV	BS-t	MT20	3.0	4.0		
BD	BS-t	MT20	3.0	4.0		
BM	BMWW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
3 8/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

FOR SECTION J-AA, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF AD-AK, Q-AY, P-AZ, O-BA, N-BB, M-BC, L-BE, K-BF, I-BH, R-AX, T-AW, U-AU, V-AT, W-AS, X-AR, Y-AQ, Z-AP, AB-AN, AC-AM, J-BG, AA-AO.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
BN- B	-304 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-56 / 0	-104.9	-104.9	0.13 (1)	6.25	
C- D	-8 / 0	-104.9	-104.9	0.05 (1)	10.00	
D- E	-11 / 0	-104.9	-104.9	0.05 (1)	6.25	
E- F	-5 / 0	-104.9	-104.9	0.05 (1)	10.00	
F- G	-2 / 0	-104.9	-104.9	0.05 (1)	10.00	
G- H	0 / 0	-104.9	-104.9	0.05 (1)	10.00	
H- I	0 / 0	-104.9	-104.9	0.05 (1)	10.00	
I- J	-8 / 0	-104.9	-104.9	0.04 (1)	10.00	
J- K	0 / 4	-112.4	-112.4	0.02 (1)	2.00	
K- L	0 / 4	-112.4	-112.4	0.04 (1)	2.00	
L- M	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
M- N	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
N- O	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
O- P	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
P- Q	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
Q- R	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
R-BO	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
BO- S	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
S- T	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
T- U	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
U- V	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
V- W	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
W- X	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
X- Y	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
Y- Z	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
Z-AA	0 / 4	-112.4	-112.4	0.05 (1)	2.00	
AA-AB	-3 / 0	-104.9	-104.9	0.02 (1)	10.00	
AB-AC	0 / 2	-104.9	-104.9	0.02 (1)	10.00	
AC-AD	-2 / 0	-104.9	-104.9	0.03 (1)	10.00	
AD-AE	-5 / 0	-104.9	-104.9	0.04 (1)	10.00	
AE-AF	-2 / 0	-104.9	-104.9	0.04 (1)	10.00	
AF-AG	0 / 4	-104.9	-104.9	0.04 (1)	10.00	
AH-AG	-70 / 0	0.0	0.0	0.04 (1)	7.81	
BN-BM	0 / 0	-28.0	-28.0	0.02 (3)	10.00	
BM-BL	0 / 14	-28.0	-28.0	0.02 (3)	10.00	
BL-BK	0 / 8	-28.0	-28.0	0.02 (3)	10.00	
BK-BJ	0 / 5	-28.0	-28.0	0.02 (3)	10.00	
BJ-BI	0 / 2	-28.0	-28.0	0.02 (3)	10.00	
BI-BH	0 / 0	-28.0	-28.0	0.02 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AY- Q	-225 / 0	0.16 (1)
AZ- P	-225 / 0	0.16 (1)
BA- O	-225 / 0	0.16 (1)
BB- N	-225 / 0	0.16 (1)
BC- M	-231 / 0	0.16 (1)
BE- L	-168 / 0	0.13 (1)
BF- K	-144 / 0	0.10 (1)
BH- I	-205 / 0	0.11 (1)
BI- G	-211 / 0	0.23 (1)
BJ- F	-208 / 0	0.14 (1)
BK- E	-204 / 0	0.08 (1)
BL- D	-224 / 0	0.05 (1)
BM- C	-89 / 0	0.02 (1)
AX- R	-225 / 0	0.16 (1)
AW- T	-225 / 0	0.16 (1)
AU- U	-225 / 0	0.16 (1)
AT- V	-225 / 0	0.16 (1)
AS- W	-225 / 0	0.16 (1)
AR- X	-225 / 0	0.16 (1)
AQ- Y	-225 / 0	0.16 (1)
AP- Z	-239 / 0	0.17 (1)
AN-AB	-127 / 0	0.08 (1)
AM-AC	-130 / 0	0.07 (1)
AJ-AE	-140 / 0	0.12 (1)
BG- J	-120 / 0	0.08 (1)
AO-AA	-130 / 0	0.09 (1)
AI-AG	0 / 14	0.00 (1)
B-BM	0 / 22	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (BB-BC:3) , WB=0.23/1.00 (G-BI:1) , SSI=0.10/1.00 (Z-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (AG) (INPUT = 0.90)
JSI METAL= 0.06 (BN) (INPUT = 1.00)

DWG NO. TAM 13295-A6
STRUCTURAL COMPONENT ONLY

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CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G41S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
BH-BG	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
BG-BF	-4 / 0	-28.0	-28.0	0.01 (3)	10.00
BF-BE	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
BE-BD	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
BD-BC	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
BC-BB	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
BB-BA	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
BA-AZ	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AZ-AY	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AY-AX	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AX-AW	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AW-AV	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AV-AU	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AU-AT	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AT-AS	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AS-AR	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AR-AQ	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AQ-AP	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AP-AO	-4 / 0	-28.0	-28.0	0.02 (3)	10.00
AO-AN	-2 / 0	-28.0	-28.0	0.01 (3)	10.00
AN-AM	-1 / 0	-28.0	-28.0	0.01 (3)	10.00
AM-AL	0 / 0	-28.0	-28.0	0.01 (3)	10.00
AL-AK	-87 / 7	0.0	0.0	0.00 (3)	7.81
AK-AD	-100 / 0	0.0	0.0	0.00 (1)	6.25
AK-AJ	0 / 0	-28.0	-28.0	0.02 (2)	10.00
AJ-AI	0 / 2	-28.0	-28.0	0.02 (3)	10.00
AI-AH	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
3 818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13215 -08
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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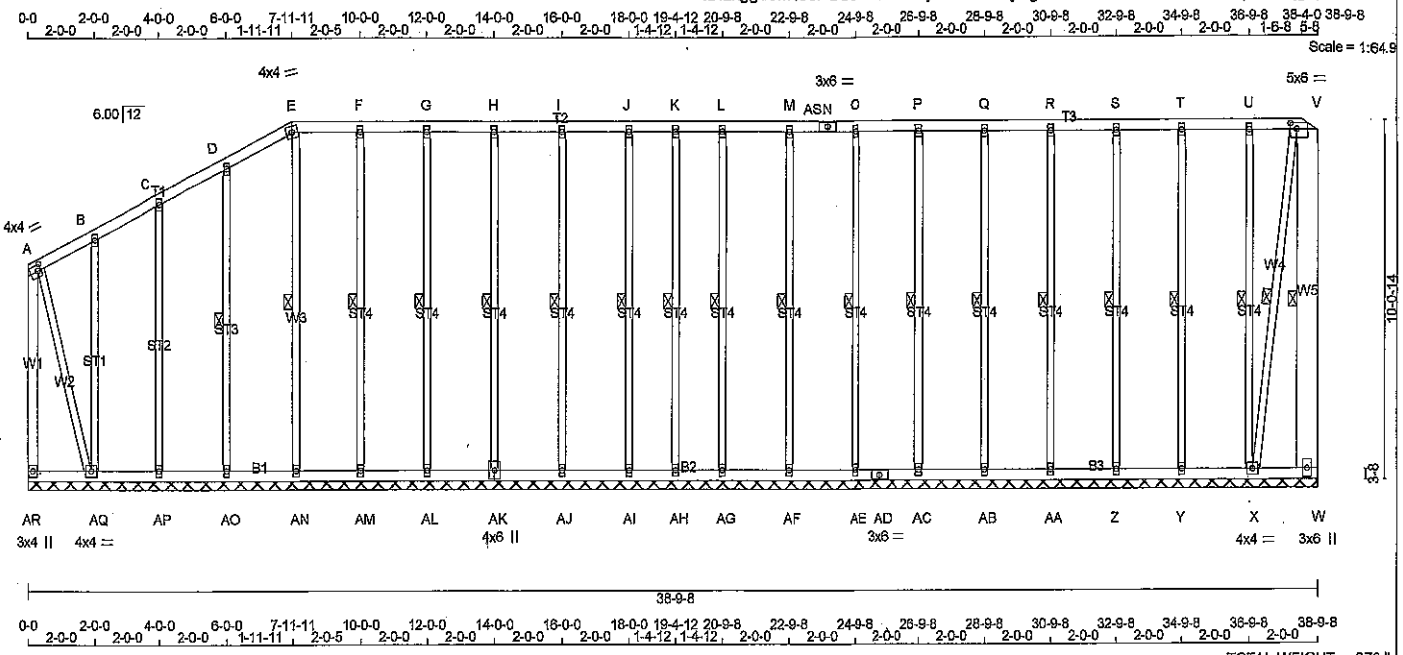

TOTAL WEIGHT = 273 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - V	2x4	DRY	No.2	SPF
AR- A	2x4	DRY	No.2	SPF
W - V	2x6	DRY	No.2	SPF
AR- AK	2x4	DRY	No.2	SPF
AK- AD	2x4	DRY	No.2	SPF
AD- W	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, F, G, H, I, J, K, L, M, O, P, Q, R, S, T, U						
B	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
N	TS-t	MT20	3.0	6.0		
V	TMWW-t	MT20	5.0	6.0	2.00	2.25
W	BMV1+p	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y, Z, AA, AB, AC, AE, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP						
Y	BMW1+w	MT20	2.0	4.0		
AD	BS-t	MT20	3.0	6.0		
AK	BSW1+l	MT20	4.0	6.0		
AQ	BMWW1-t	MT20	4.0	4.0		
AR	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

FOR SECTION E-V, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF V-W, K-AH, U-X, T-Y, S-Z, R-AA, Q-AB, P-AC, O-AE, M-AF, L-AG, D-AO, F-AM, G-AL, H-AK, I-AJ, J-AI, E-AN, V-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 2	-104.9	-104.9	0.05 (1)	10.00
B- C	-5 / 0	-104.9	-104.9	0.05 (1)	10.00
C- D	0 / 0	-104.9	-104.9	0.06 (1)	10.00
D- E	-10 / 0	-104.9	-104.9	0.06 (1)	10.00
E- F	0 / 2	-112.4	-112.4	0.06 (1)	2.00
F- G	0 / 2	-112.4	-112.4	0.06 (1)	2.00
G- H	0 / 2	-112.4	-112.4	0.05 (1)	2.00
H- I	0 / 2	-112.4	-112.4	0.05 (1)	2.00
I- J	0 / 2	-112.4	-112.4	0.05 (1)	2.00
J- K	0 / 2	-112.4	-112.4	0.04 (1)	2.00
K- L	0 / 2	-112.4	-112.4	0.04 (1)	2.00
L- M	0 / 2	-112.4	-112.4	0.05 (1)	2.00
M-AS	0 / 2	-112.4	-112.4	0.05 (1)	2.00
AS- N	0 / 2	-112.4	-112.4	0.05 (1)	2.00
N- O	0 / 2	-112.4	-112.4	0.05 (1)	2.00
O- P	0 / 2	-112.4	-112.4	0.05 (1)	2.00
P- Q	0 / 2	-112.4	-112.4	0.05 (1)	2.00
Q- R	0 / 2	-112.4	-112.4	0.05 (1)	2.00
R- S	0 / 2	-112.4	-112.4	0.05 (1)	2.00
S- T	0 / 2	-112.4	-112.4	0.05 (1)	2.00
T- U	0 / 2	-112.4	-112.4	0.05 (1)	2.00
U- V	0 / 6	-112.4	-112.4	0.05 (1)	2.00
AR- A	-102 / 0	0.0	0.0	0.08 (1)	7.81
W- V	-59 / 0	0.0	0.0	0.01 (1)	6.25
AR-AQ	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AQ-AP	0 / 4	-28.0	-28.0	0.03 (3)	10.00
AP-AO	0 / 2	-28.0	-28.0	0.03 (3)	10.00
AO-AN	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AN-AM	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AM-AL	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AL-AK	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AK-AJ	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AJ-AI	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AI-AH	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AH-AG	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AG-AF	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AF-AE	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AE-AD	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AD-AC	-2 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AH- K	-146 / 0	0.10 (1)
X- U	-244 / 0	0.17 (1)
Y- T	-225 / 0	0.16 (1)
Z- S	-224 / 0	0.16 (1)
AA- R	-225 / 0	0.16 (1)
AB- Q	-225 / 0	0.16 (1)
AC- P	-225 / 0	0.16 (1)
AE- O	-225 / 0	0.16 (1)
AF- M	-230 / 0	0.16 (1)
AG- L	-191 / 0	0.13 (1)
AQ- B	-227 / 0	0.20 (1)
AP- C	-202 / 0	0.26 (1)
AO- D	-236 / 0	0.13 (1)
AM- F	-251 / 0	0.17 (1)
AL- G	-221 / 0	0.15 (1)
AK- H	-225 / 0	0.16 (1)
AJ- I	-230 / 0	0.16 (1)
AI- J	-191 / 0	0.13 (1)
AN- E	-178 / 0	0.12 (1)
A-AQ	0 / 18	0.00 (1)
X- V	-35 / 0	0.02 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-AR:1) , BC=0.03/1.00 (AP-AQ:3) , WB=0.26/1.00 (C-AP:1) , SSI=0.11/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (A) (INPUT = 0.90)
JSI METAL= 0.09 (AK) (INPUT = 1.00)

DWG NO. TAM 13216-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:zkggtrJnN3sPSCJk21FDR7yZZod-OYLyVgCnwFAG?Eu2LTFLnzF81JrprBdoqglq?QzdMSu

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO
AC-AB	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AB-AA	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
AA- Z	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
Z- Y	-2 / 0	-28.0	-28.0	0.03 (3)	10.00
Y- X	-2 / 0	-28.0	-28.0	0.03 (3)	10.00
X- W	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1321608
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G45	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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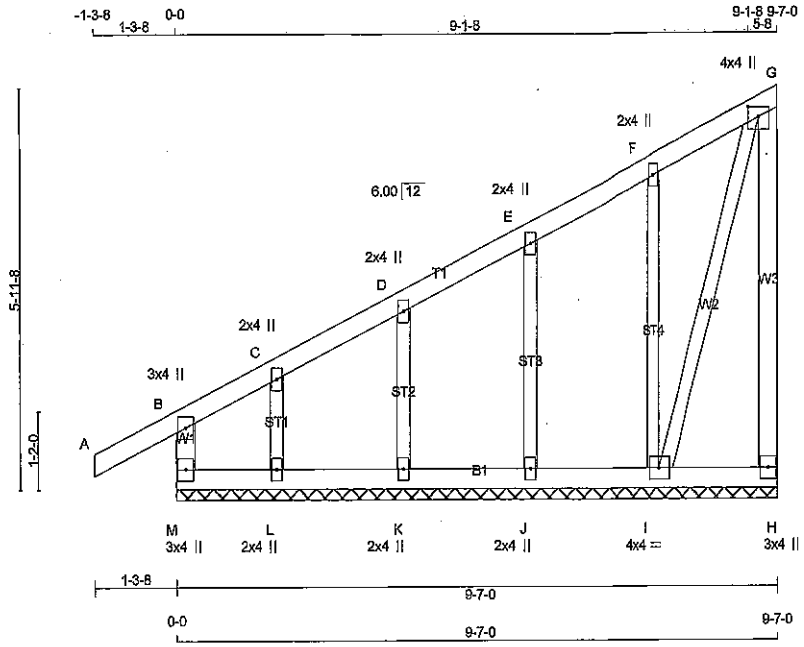

TOTAL WEIGHT = 46 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	4.0	1.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J, K, L						
J	BMW1+w	MT20	2.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
M- B	-278 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00
B- C	-25 / 0	-104.9	-104.9	0.13 (1)	6.25
C- D	0 / 11	-104.9	-104.9	0.06 (1)	10.00
D- E	0 / 10	-104.9	-104.9	0.06 (1)	10.00
E- F	0 / 17	-104.9	-104.9	0.06 (1)	10.00
F- G	0 / 10	-104.9	-104.9	0.06 (1)	10.00
H- G	-48 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00
L- K	-5 / 0	-28.0	-28.0	0.02 (2)	10.00
K- J	-10 / 0	-28.0	-28.0	0.02 (2)	6.25
J- I	-14 / 0	-28.0	-28.0	0.03 (2)	6.25
I- H	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	-237 / 0	0.09 (1)
J- E	-199 / 0	0.05 (1)
K- D	-225 / 0	0.04 (1)
L- C	-112 / 0	0.02 (1)
I- G	-51 / 0	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) ; BC=0.03/1.00 (I-J:2) , WB=0.09/1.00 (F-I:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (F) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13217-S8
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	G49	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed Mar 7 09:57:10 2018 Page 1
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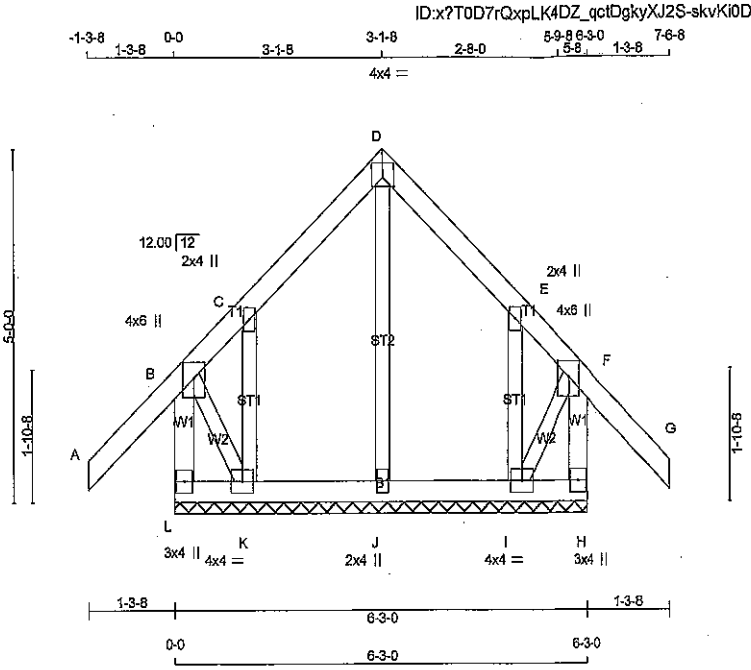

TOTAL WEIGHT = 35 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-285 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	
B- C	-58 / 0	-104.9	-104.9	0.14 (1)	6.25	
C- D	-12 / 0	-104.9	-104.9	0.06 (1)	6.25	
D- E	-12 / 0	-104.9	-104.9	0.06 (1)	6.25	
E- F	-58 / 0	-104.9	-104.9	0.14 (1)	6.25	
F- G	0 / 52	-104.9	-104.9	0.15 (1)	10.00	
H- F	-285 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 0	-28.0	-28.0	0.02 (3)	10.00	
K- J	0 / 2	-28.0	-28.0	0.03 (3)	10.00	
J- I	0 / 2	-28.0	-28.0	0.03 (3)	10.00	
I- H	0 / 0	-28.0	-28.0	0.02 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-190 / 0	0.07 (1)
K- C	-102 / 0	0.02 (1)
I- E	-102 / 0	0.02 (1)
B- K	0 / 10	0.00 (1)
I- F	0 / 10	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.03/1.00 (J-K:3) , WB=0.07/1.00 (D-J:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1328-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P01	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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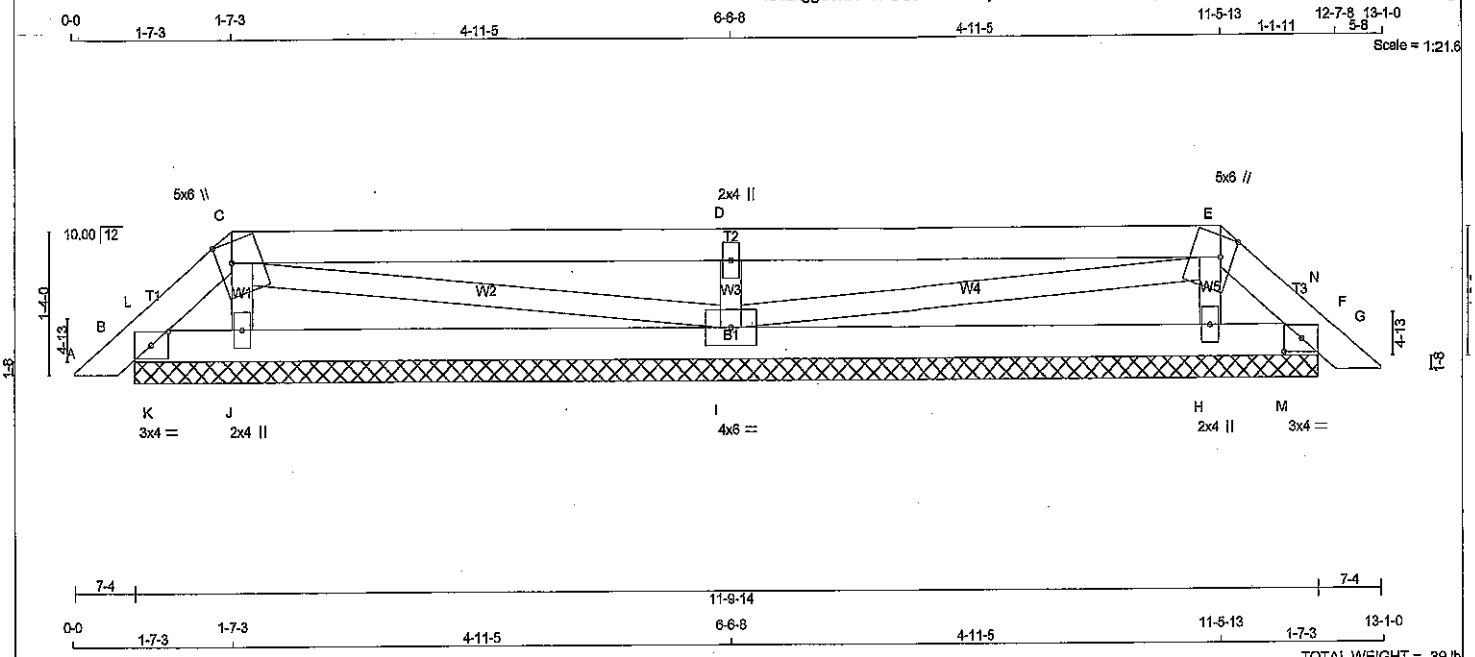

TOTAL WEIGHT = 39 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	112	0	112	0	-39	11-9-14	11-9-14
F	112	0	112	0	-39	11-9-14	11-9-14
J	332	0	332	0	0	11-9-14	11-9-14
I	792	0	792	0	0	11-9-14	11-9-14
H	332	0	332	0	0	11-9-14	11-9-14

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	60	87 / 0	0 / -21	0 / 0	0 / 0	0 / -6	0 / 0
F	60	87 / 0	0 / -21	0 / 0	0 / 0	0 / -6	0 / 0
J	295	130 / 0	92 / 0	0 / 0	0 / 0	74 / 0	0 / 0
I	617	403 / 0	108 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	295	130 / 0	92 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- L	-18 / 53	-104.9	-104.9	0.03 (3)	6.25	
L- C	-55 / 0	-104.9	-104.9	0.02 (2)	6.25	
C- D	-4 / 2	-104.9	-104.9	0.44 (1)	10.00	
D- E	-4 / 2	-104.9	-104.9	0.44 (1)	10.00	
E- N	-55 / 0	-104.9	-104.9	0.02 (2)	6.25	
N- F	-18 / 53	-104.9	-104.9	0.03 (3)	6.25	
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- K	-3 / 34	-28.0	-28.0	0.02 (1)	10.00	
K- J	-3 / 34	-28.0	-28.0	0.11 (2)	10.00	
J- I	-3 / 13	-28.0	-28.0	0.14 (3)	10.00	
I- H	-3 / 13	-28.0	-28.0	0.14 (3)	10.00	
H- M	-3 / 34	-28.0	-28.0	0.11 (2)	10.00	
M- F	-3 / 34	-28.0	-28.0	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-188 / 2	0.03 (1)
C- I	-8 / 1	0.00 (1)
I- D	-648 / 0	0.09 (1)
I- E	-8 / 1	0.00 (1)
H- E	-188 / 2	0.03 (1)
K- L	-107 / 0	0.00 (1)
M- N	-107 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44/1.00 (D-E:1) , BC=0.14/1.00 (I-J:3) , WB=0.09/1.00 (D-I:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3676
S KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 131868
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P02	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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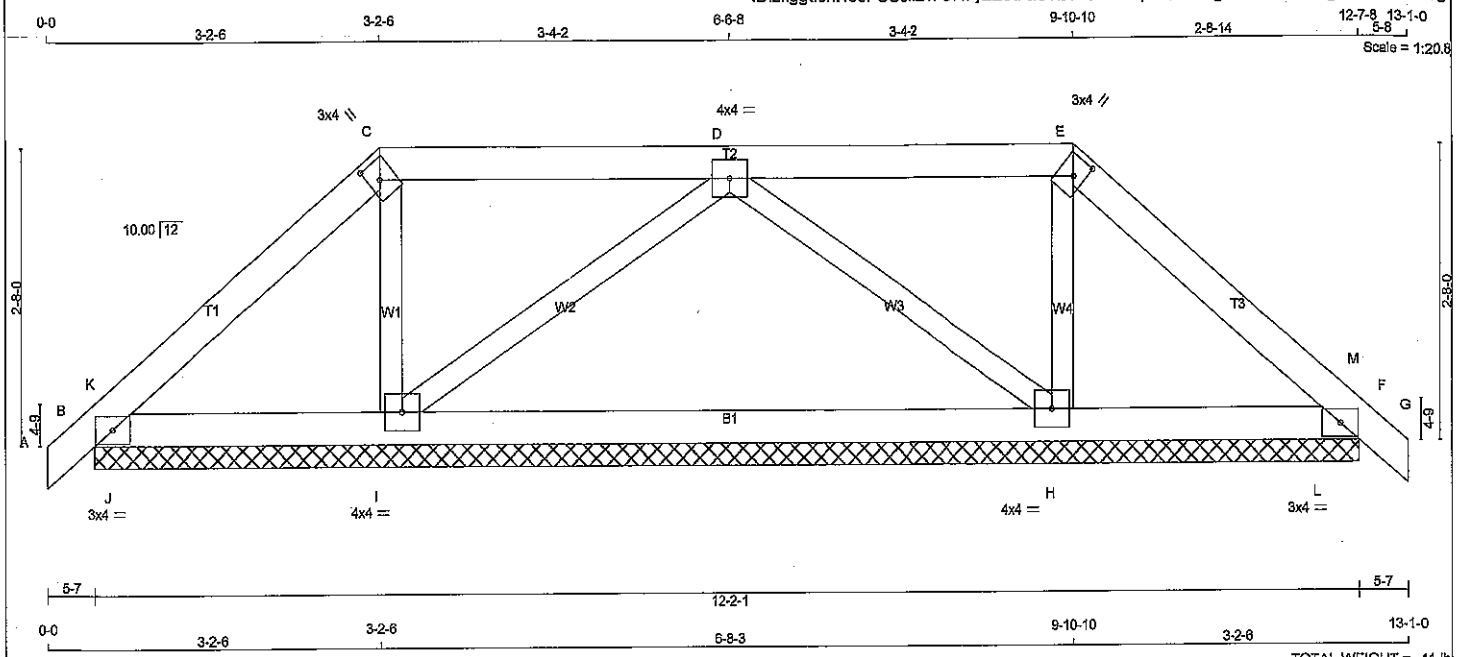

TOTAL WEIGHT = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TTW+h	MT20	3.0	4.0	2.00	1.25
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	214	0	214	0	0	12-2-1	12-2-1
F	214	0	214	0	0	12-2-1	12-2-1
I	652	0	652	0	0	12-2-1	12-2-1
H	652	0	652	0	0	12-2-1	12-2-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	150	127 / 0	7 / 0	0 / 0	0 / 0	16 / 0	0 / 0
F	150	127 / 0	7 / 0	0 / 0	0 / 0	16 / 0	0 / 0
I	534	304 / 0	121 / 0	0 / 0	0 / 0	109 / 0	0 / 0
H	534	304 / 0	121 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-104.9	-104.9	0.02 (1)	10.00
B- K	0 / 84	-104.9	-104.9	0.06 (1)	10.00
K- C	-14 / 2	-104.9	-104.9	0.09 (1)	6.25
C- D	0 / 7	-104.9	-104.9	0.19 (1)	10.00
D- E	0 / 7	-104.9	-104.9	0.19 (1)	10.00
E- M	-14 / 2	-104.9	-104.9	0.09 (1)	6.25
M- F	0 / 84	-104.9	-104.9	0.06 (1)	10.00
F- G	0 / 17	-104.9	-104.9	0.02 (1)	10.00
B- J	-4 / 4	-28.0	-28.0	0.09 (1)	10.00
J- I	-4 / 4	-28.0	-28.0	0.20 (2)	10.00
I- H	0 / 289	-28.0	-28.0	0.22 (2)	10.00
H- L	-4 / 4	-28.0	-28.0	0.20 (2)	10.00
L- F	-4 / 4	-28.0	-28.0	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-247 / 0	0.04 (1)
I- D	-367 / 0	0.10 (1)
D- H	-367 / 0	0.10 (1)
H- E	-247 / 0	0.04 (1)
J- K	-284 / 0	0.00 (1)
L- M	-284 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-D:1) , BC=0.22/1.00 (H-I:2) , WB=0.10/1.00 (D-I:1) , SSI=0.21/1.00 (F-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (I) (INPUT = 0.90)
JSI METAL= 0.11 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/018
S KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 131898
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P03	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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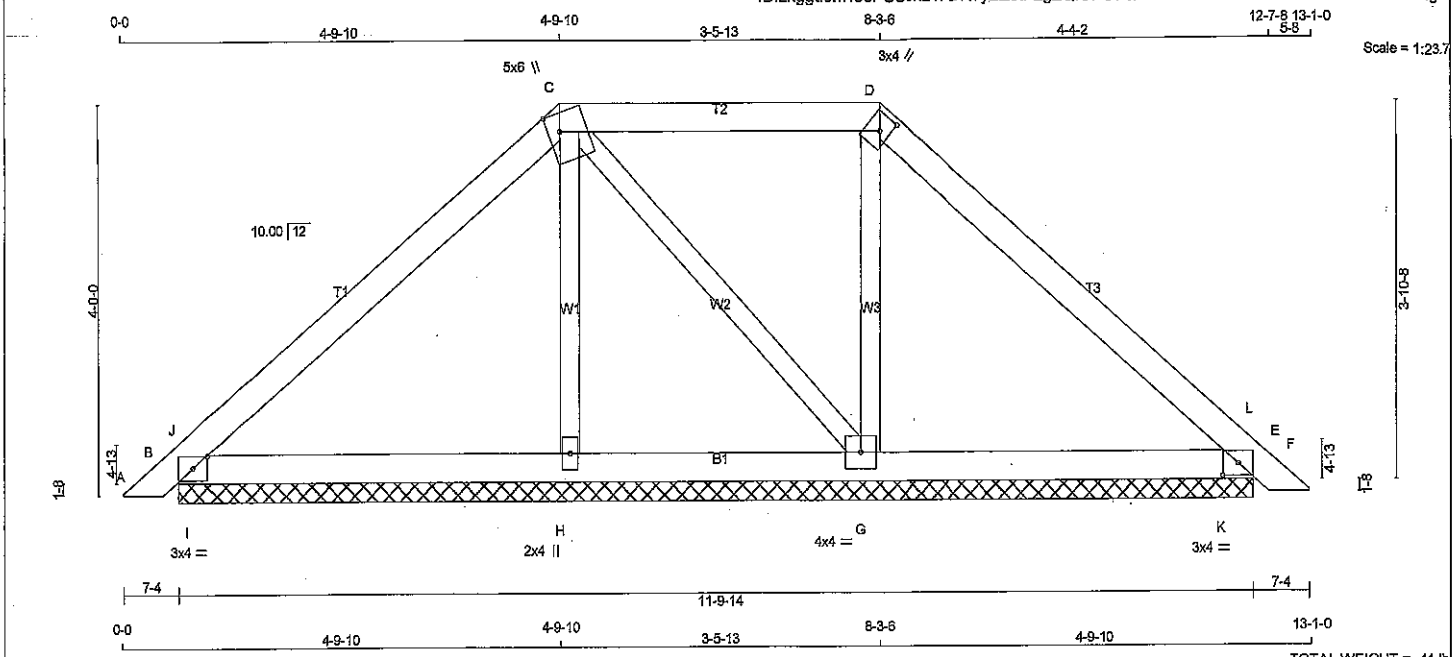

TOTAL WEIGHT = 41 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	468	0	468	0	0	11-9-14	11-9-14
E	438	0	438	0	0	11-9-14	11-9-14
H	333	0	333	0	0	11-9-14	11-9-14
G	441	0	441	0	0	11-9-14	11-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	353	250 / 0	48 / 0	0 / 0	0 / 0	55 / 0	0 / 0
E	333	231 / 0	48 / 0	0 / 0	0 / 0	54 / 0	0 / 0
H	285	143 / 0	77 / 0	0 / 0	0 / 0	65 / 0	0 / 0
G	355	211 / 0	75 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-142 / 0	-104.9	-104.9	0.10 (1)	6.25
J- C	-243 / 0	-104.9	-104.9	0.23 (1)	6.25
C- D	-134 / 0	-104.9	-104.9	0.22 (1)	6.25
D- L	-197 / 0	-104.9	-104.9	0.23 (1)	6.25
L- E	-141 / 13	-104.9	-104.9	0.10 (1)	6.25
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 176	-28.0	-28.0	0.20 (1)	10.00
I- H	0 / 176	-28.0	-28.0	0.20 (1)	10.00
H- G	0 / 171	-28.0	-28.0	0.13 (1)	10.00
G- K	0 / 142	-28.0	-28.0	0.19 (1)	10.00
K- E	0 / 142	-28.0	-28.0	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-183 / 0	0.04 (1)
C- G	-53 / 0	0.02 (1)
G- D	-251 / 0	0.06 (1)
I- J	-497 / 117	0.00 (1)
K- L	-503 / 117	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-J:1) , BC=0.20/1.00 (H-I:1) , WB=0.06/1.00 (D-G:1) , SSI=0.40/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3876
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13190-A
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P04	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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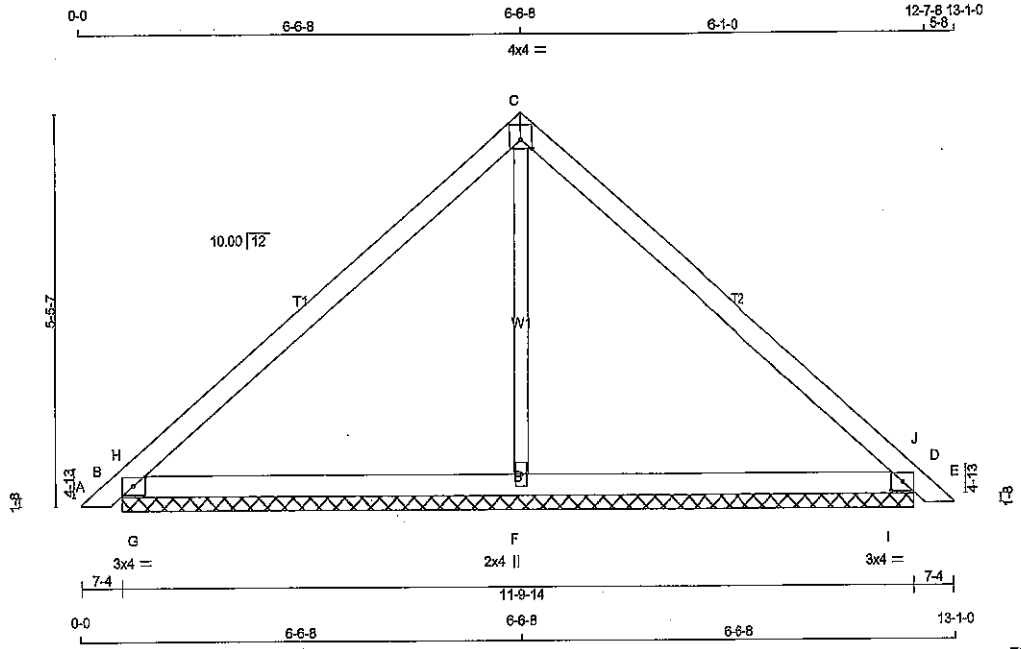

TOTAL WEIGHT = 11 X 37 = 403 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	626	0	626	0	0	11-9-14	11-9-14
D	626	0	626	0	0	11-9-14	11-9-14
F	429	0	429	0	0	11-9-14	11-9-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	473	334 / 0	65 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	473	334 / 0	65 / 0	0 / 0	0 / 0	74 / 0	0 / 0
F	381	167 / 0	119 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- H	-201 / 227	-104.9	-104.9	0.31 (1)	6.25
H- C	-365 / 0	-104.9	-104.9	0.46 (1)	6.25
C- J	-365 / 0	-104.9	-104.9	0.46 (1)	6.25
J- D	-201 / 227	-104.9	-104.9	0.31 (1)	6.25
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- G	0 / 263	-28.0	-28.0	0.41 (1)	10.00
G- F	0 / 263	-28.0	-28.0	0.41 (1)	10.00
F- I	0 / 263	-28.0	-28.0	0.41 (1)	10.00
I- D	0 / 263	-28.0	-28.0	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-84 / 47	0.03 (1)
G- H	-1112 / 148	0.00 (1)
I- J	-1112 / 148	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-J:1) , BC=0.41/1.00 (F-I:1) , WB=0.03/1.00 (C-F:1) , SSI=0.86/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/8/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1319 / -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P05	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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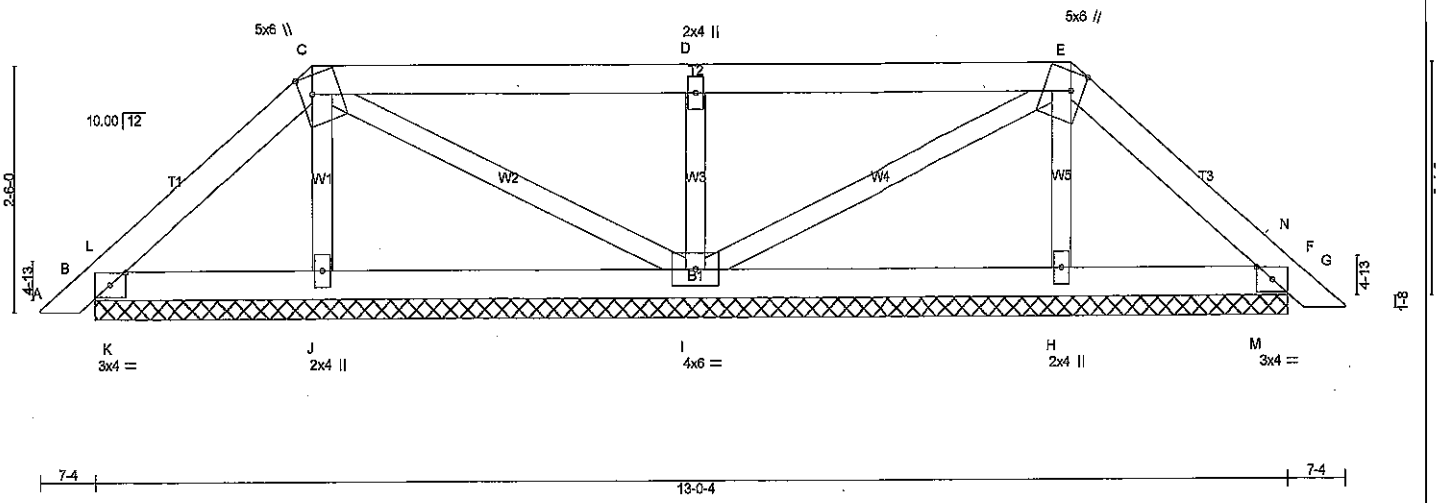

7-4 | 13-0-4 | 7-4

0-0 | 3-0-0 | 3-0-0 | 4-1-11 | 7-1-11 | 4-1-11 | 11-3-6 | 3-0-0 | 14-3-6

TOTAL WEIGHT = 44 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	253	0	253	0	0	13-0-4	13-0-4
F	253	0	253	0	0	13-0-4	13-0-4
J	321	0	321	0	0	13-0-4	13-0-4
I	692	0	692	0	0	13-0-4	13-0-4
H	321	0	321	0	0	13-0-4	13-0-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	185	142 / 0	18 / 0	0 / 0	0 / 0	25 / 0	0 / 0
F	185	142 / 0	18 / 0	0 / 0	0 / 0	25 / 0	0 / 0
J	274	138 / 0	74 / 0	0 / 0	0 / 0	62 / 0	0 / 0
I	537	355 / 0	90 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	274	138 / 0	74 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- L	-37 / 0	-104.9	-104.9	0.03 (1)	6.25
L- C	-88 / 0	-104.9	-104.9	0.07 (1)	6.25
C- D	-13 / 0	-104.9	-104.9	0.30 (1)	6.25
D- E	-13 / 0	-104.9	-104.9	0.30 (1)	6.25
E- N	-88 / 0	-104.9	-104.9	0.07 (1)	6.25
N- F	-37 / 0	-104.9	-104.9	0.03 (1)	6.25
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- K	0 / 63	-28.0	-28.0	0.07 (1)	10.00
K- J	0 / 63	-28.0	-28.0	0.08 (2)	10.00
J- I	0 / 53	-28.0	-28.0	0.10 (2)	10.00
I- H	0 / 53	-28.0	-28.0	0.10 (2)	10.00
H- M	0 / 63	-28.0	-28.0	0.08 (2)	10.00
M- F	0 / 63	-28.0	-28.0	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-194 / 0	0.03 (1)
C- I	-44 / 0	0.02 (1)
I- D	-542 / 0	0.08 (1)
I- E	-44 / 0	0.02 (1)
H- E	-194 / 0	0.03 (1)
K- L	-193 / 9	0.00 (1)
M- N	-193 / 9	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.10/1.00 (I-J:2) , WB=0.08/1.00 (D-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/13/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13192-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P06	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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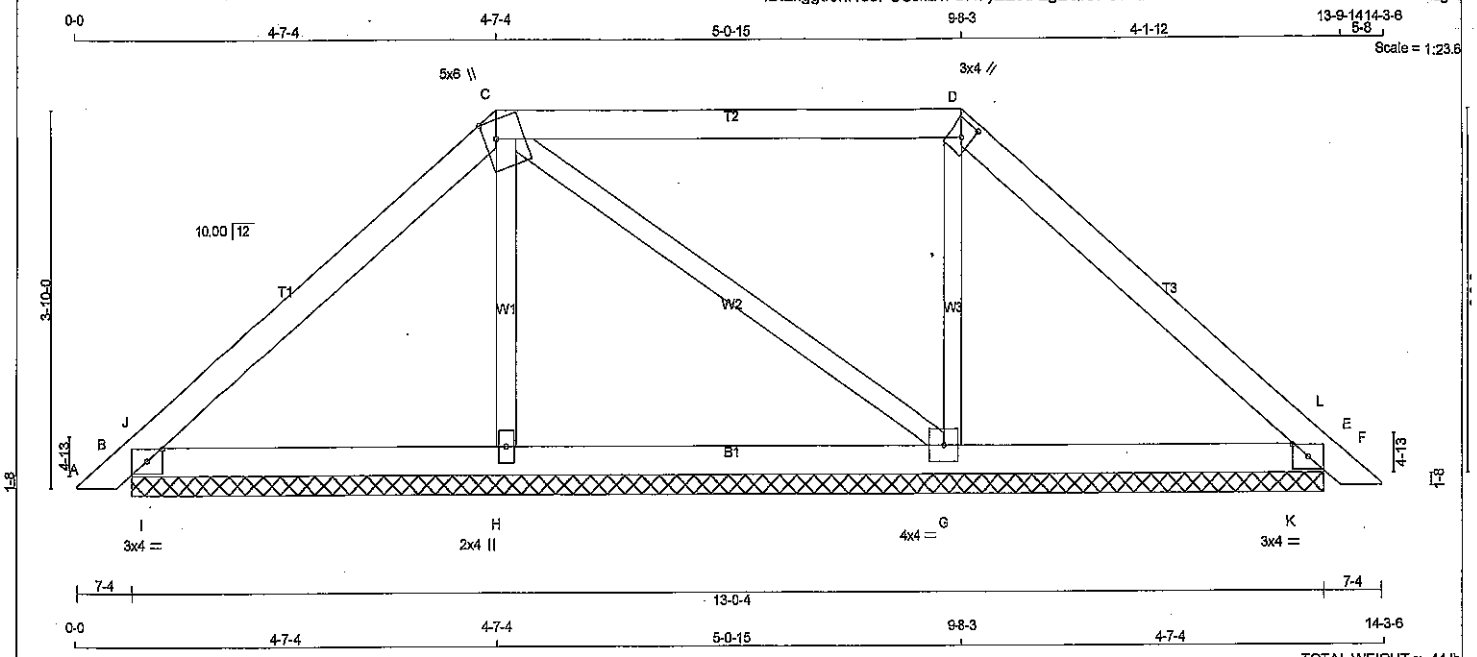

TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	448	0	448	0	0	13-0-4	13-0-4
E	417	0	417	0	0	13-0-4	13-0-4
H	447	0	447	0	0	13-0-4	13-0-4
G	529	0	529	0	0	13-0-4	13-0-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	333	245 / 0	39 / 0	0 / 0	0 / 0	49 / 0	0 / 0
E	312	226 / 0	39 / 0	0 / 0	0 / 0	47 / 0	0 / 0
H	377	196 / 0	98 / 0	0 / 0	0 / 0	83 / 0	0 / 0
G	432	247 / 0	97 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-98 / 0	-104.9	-104.9	0.09 (1)	6.25
J- C	-235 / 0	-104.9	-104.9	0.21 (1)	6.25
C- D	-125 / 0	-104.9	-104.9	0.46 (1)	6.25
D- L	-188 / 0	-104.9	-104.9	0.21 (1)	6.25
L- E	-96 / 6	-104.9	-104.9	0.09 (1)	6.25
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 171	-28.0	-28.0	0.18 (1)	10.00
I- H	0 / 171	-28.0	-28.0	0.18 (1)	10.00
H- G	0 / 162	-28.0	-28.0	0.15 (2)	10.00
G- K	0 / 135	-28.0	-28.0	0.18 (1)	10.00
K- E	0 / 135	-28.0	-28.0	0.18 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-275 / 0	0.06 (1)
C- G	-45 / 0	0.03 (1)
G- D	-331 / 0	0.07 (1)
I- J	-465 / 73	0.00 (1)
K- L	-471 / 73	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-D:1) , BC=0.18/1.00 (H-I:1) , WB=0.07/1.00 (D-G:1) , SSI=0.38/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13193-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P07	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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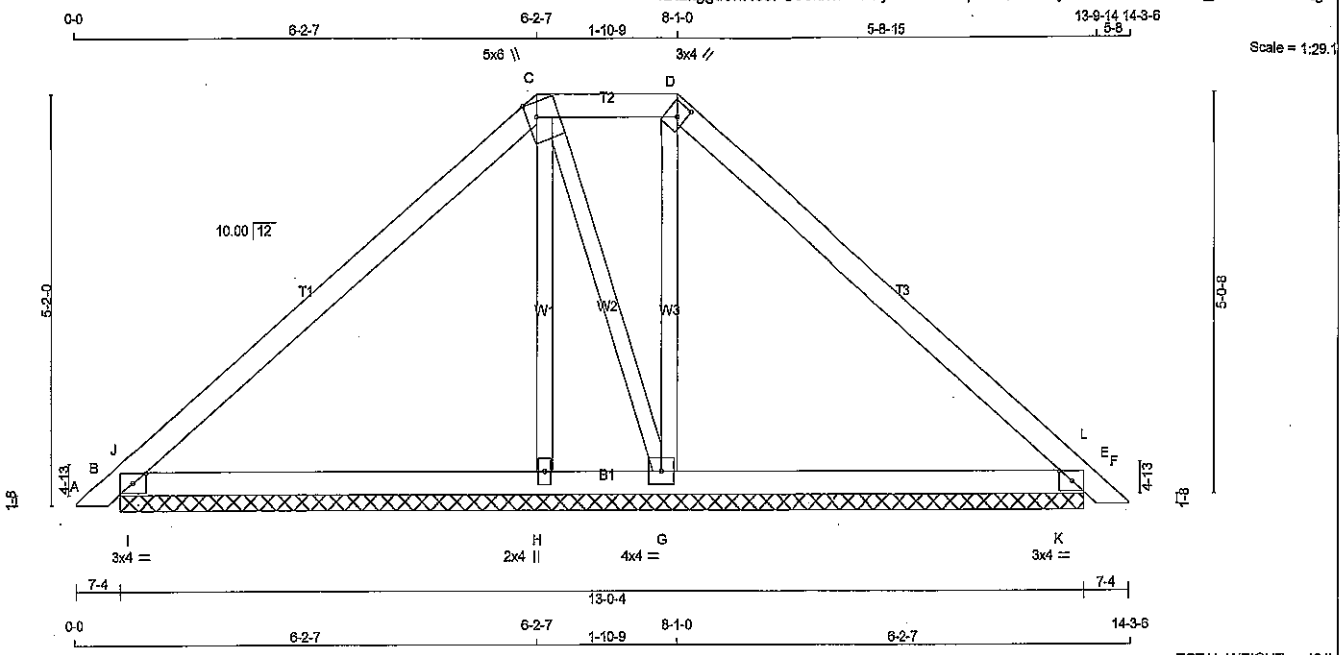

TOTAL WEIGHT = 46 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	612	0	612	0	0	13-0-4	13-0-4
E	597	0	597	0	0	13-0-4	13-0-4
H	254	0	254	0	0	13-0-4	13-0-4
G	377	0	377	0	0	13-0-4	13-0-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	467	321 / 0	70 / 0	0 / 0	0 / 0	76 / 0	0 / 0
E	458	310 / 0	71 / 0	0 / 0	0 / 0	76 / 0	0 / 0
H	227	98 / 0	72 / 0	0 / 0	0 / 0	57 / 0	0 / 0
G	301	184 / 0	60 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-244 / 108	-104.9	-104.9	0.23 (1)	6.25
J- C	-346 / 0	-104.9	-104.9	0.42 (1)	6.25
C- D	-229 / 0	-104.9	-104.9	0.06 (1)	6.25
D- L	-322 / 0	-104.9	-104.9	0.42 (1)	6.25
L- E	-248 / 134	-104.9	-104.9	0.24 (1)	6.25
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 250	-28.0	-28.0	0.34 (1)	10.00
I- H	0 / 250	-28.0	-28.0	0.34 (1)	10.00
H- G	0 / 249	-28.0	-28.0	0.25 (1)	10.00
G- K	0 / 232	-28.0	-28.0	0.34 (1)	10.00
K- E	0 / 232	-28.0	-28.0	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-85 / 13	0.03 (1)
C- G	-57 / 9	0.02 (1)
G- D	-154 / 24	0.06 (1)
I- J	-905 / 199	0.00 (1)
K- L	-909 / 199	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (C-J:1) , BC=0.34/1.00 (H-I:1) , WB=0.06/1.00 (D-G:1) , SSI=0.72/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13194-B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	P08	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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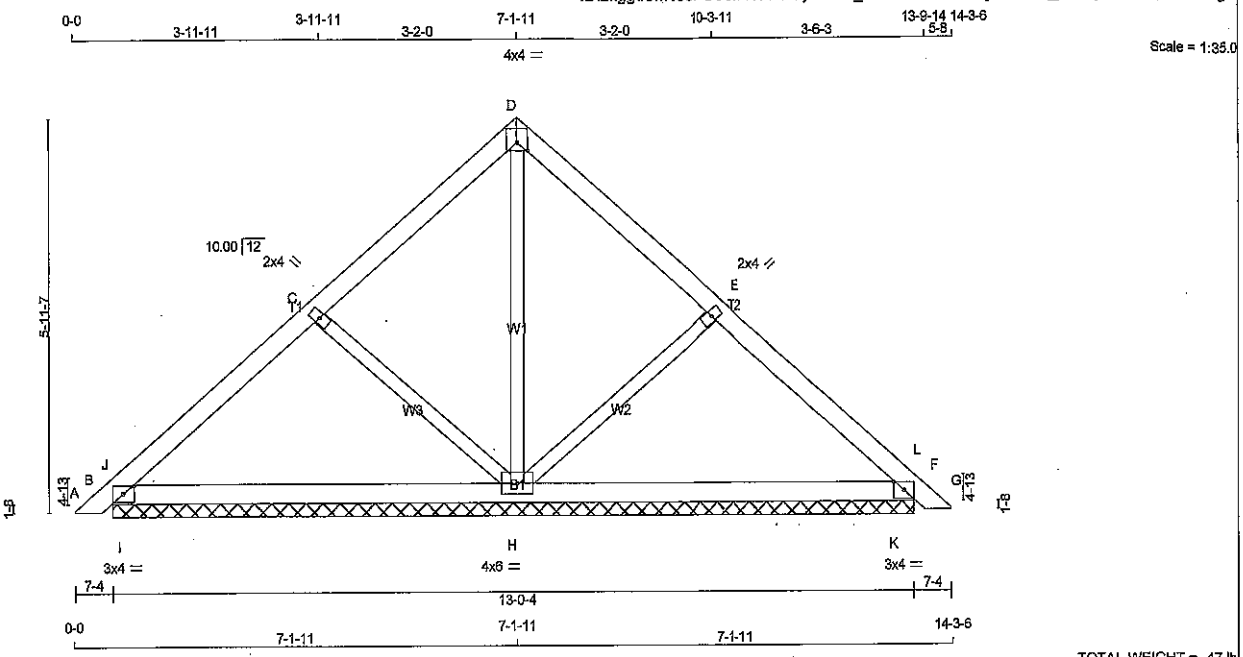

TOTAL WEIGHT = 47 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMWWW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	444	0	444	0	0	13-0-4	13-0-4
F	444	0	444	0	0	13-0-4	13-0-4
H	953	0	953	0	0	13-0-4	13-0-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	346	225 / 0	60 / 0	0 / 0	0 / 0	61 / 0	0 / 0
F	346	225 / 0	60 / 0	0 / 0	0 / 0	61 / 0	0 / 0
H	761	464 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-461 / 0	-104.9	-104.9	0.13 (3)	6.25
J- C	-260 / 0	-104.9	-104.9	0.19 (1)	6.25
C- D	-23 / 8	-104.9	-104.9	0.19 (1)	6.25
D- E	-23 / 8	-104.9	-104.9	0.19 (1)	6.25
E- L	-260 / 0	-104.9	-104.9	0.19 (1)	6.25
L- F	-461 / 0	-104.9	-104.9	0.13 (3)	6.25
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 229	-28.0	-28.0	0.11 (3)	10.00
I- H	0 / 229	-28.0	-28.0	0.31 (2)	10.00
H- K	0 / 229	-28.0	-28.0	0.31 (2)	10.00
K- F	0 / 229	-28.0	-28.0	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	-302 / 18	0.15 (1)
H- E	-337 / 0	0.10 (1)
C- H	-337 / 0	0.10 (1)
I- J	0 / 443	0.00 (1)
K- L	0 / 443	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (E-L:1) , BC=0.31/1.00 (H-K:2) , WB=0.15/1.00 (D-H:1) , SSI=0.24/1.00 (F-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13195 P8
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	P25	17	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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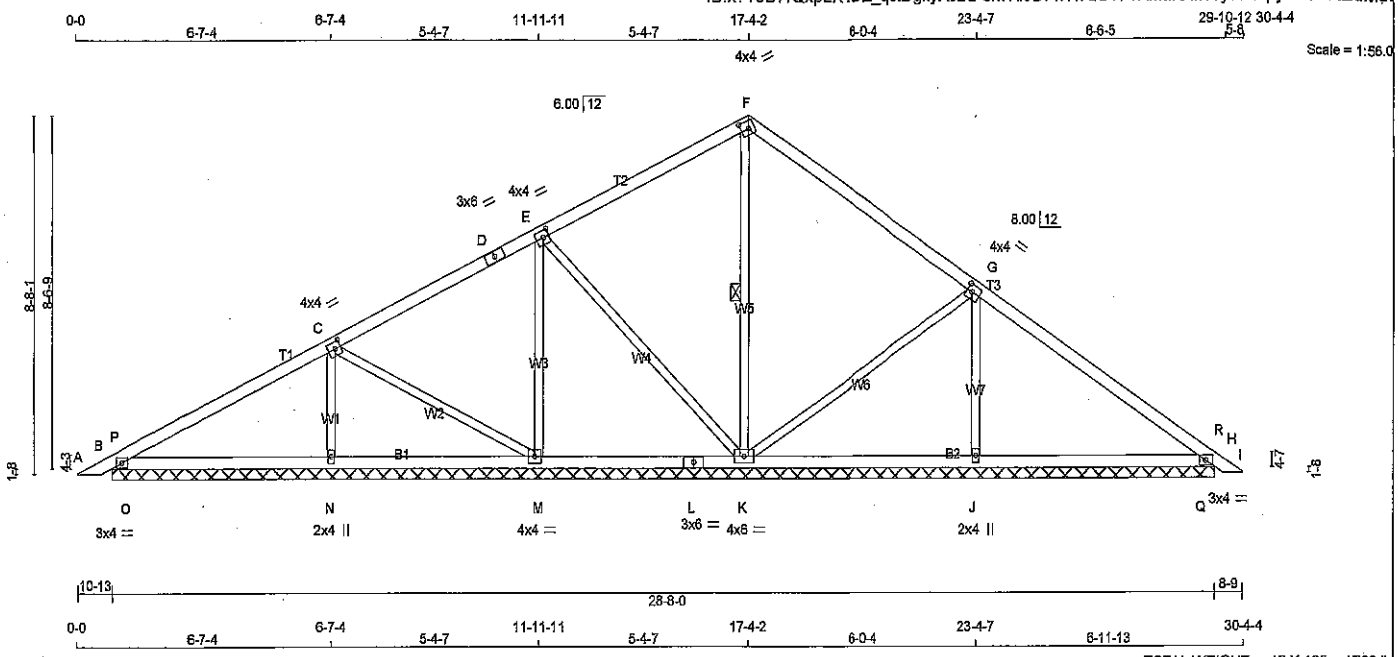

TOTAL WEIGHT = 17 X 105 = 1788 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-I	MT20	4.0	4.0	2.25	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMB1-I	MT20	3.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWWW1-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	833	0	732	204	-47	28-8-0 (13-8-0	8-0
N	971	0	1109	0	-88	28-8-0 (13-8-0	8-0
M	797	0	901	0	-88	28-8-0 (13-8-0	8-0
K	1046	0	1046	0	-81	28-8-0 (13-8-0	8-0
J	980	0	1062	0	-77	28-8-0 (13-8-0	8-0
H	879	0	739	0	-65	28-8-0 (13-8-0	8-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT K FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT J FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	470	409 / 0	52 / 0	0 / 0	20 / -82	74 / 0	0 / 0
N	755	561 / 0	126 / 0	0 / 0	36 / -153	140 / 0	0 / 0
M	624	466 / 0	109 / 0	0 / 0	29 / -139	116 / 0	0 / 0
K	800	543 / 0	117 / 0	0 / 0	34 / -148	140 / 0	0 / 0
J	769	539 / -2	137 / 0	0 / 0	76 / -149	147 / 0	0 / 0
H	507	404 / -1	60 / 0	0 / 0	46 / -99	82 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	145 / -91	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-K.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 30	-142.8	-142.8	0.09 (2)	10.00
B- P	-214 / 16	-142.8	-142.8	0.13 (2)	6.25
P- C	-305 / 92	-142.8	-142.8	0.64 (2)	6.25
C- D	-219 / 97	-142.8	-142.8	0.68 (2)	6.25
D- E	-219 / 97	-142.8	-142.8	0.68 (2)	6.25
E- F	-84 / 119	-142.8	-142.8	0.67 (2)	6.25
F- G	-112 / 85	-142.8	-142.8	0.93 (3)	6.25
G- R	-300 / 41	-142.8	-142.8	0.95 (3)	5.76
R- H	-203 / 0	-142.8	-142.8	0.16 (3)	6.25
H- I	0 / 26	-142.8	-142.8	0.05 (3)	10.00
B- O	-79 / 277	-28.0	-28.0	0.38 (2)	6.25
O- N	-79 / 277	-28.0	-28.0	0.38 (2)	6.25
N- M	-79 / 277	-28.0	-28.0	0.26 (2)	6.25
M- L	-49 / 179	-28.0	-28.0	0.19 (5)	6.25
L- K	-49 / 179	-28.0	-28.0	0.19 (5)	6.25
K- J	0 / 293	-28.0	-28.0	0.31 (6)	10.00
J- Q	0 / 293	-28.0	-28.0	0.36 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-860 / 143	0.16 (2)
C- M	-111 / 78	0.07 (2)
M- E	-729 / 120	0.39 (2)
E- K	-260 / 106	0.31 (2)
K- F	-603 / 28	0.28 (1)
K- G	-307 / 171	0.34 (3)
J- G	-805 / 140	0.24 (3)
O- P	-647 / 218	0.00 (1)
Q- R	-785 / 337	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	43.5	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	65.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(60 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 43.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.95/1.00 (G-R:3) , BC=0.38/1.00 (B-O:2) , WB=0.39/1.00 (E-M:2) , SSI=0.59/1.00 (H-Q:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13219-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- H	0 / 293	-28.0	-28.0	0.36 (3)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13219-88
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	PB21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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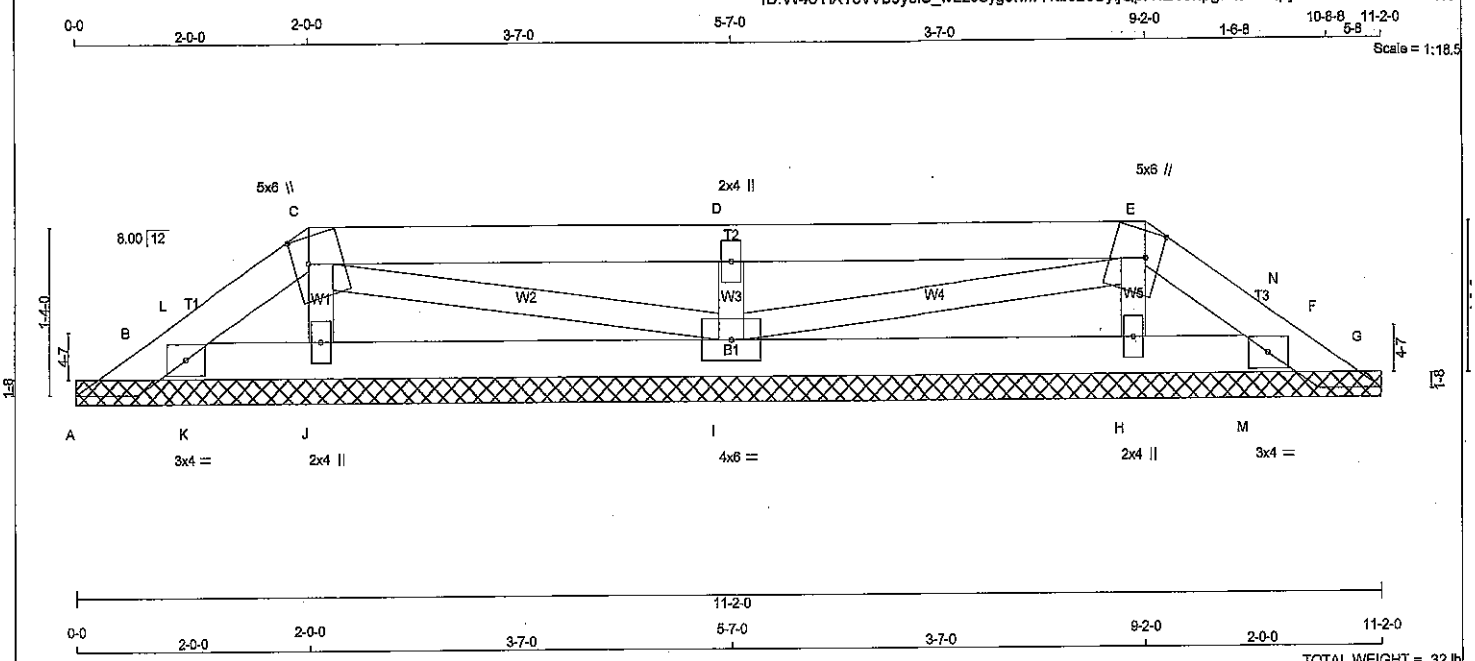

TOTAL WEIGHT = 32 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	27	0	27	0	0	11-2-0 (9-8-2)	11-2-0
G	27	0	27	0	0	11-2-0 (9-8-2)	11-2-0
B	152	0	152	0	0	11-2-0 (9-8-2)	11-2-0
F	152	0	152	0	0	11-2-0 (9-8-2)	11-2-0
J	258	0	258	0	0	11-2-0 (9-8-2)	11-2-0
I	576	0	576	0	0	11-2-0 (9-8-2)	11-2-0
H	258	0	258	0	0	11-2-0 (9-8-2)	11-2-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , G

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	20	12 / 0	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0
G	20	12 / 0	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0
B	103	91 / 0	0 / -1	0 / 0	0 / 0	13 / 0	0 / 0
F	103	91 / 0	0 / -1	0 / 0	0 / 0	13 / 0	0 / 0
J	222	109 / 0	62 / 0	0 / 0	0 / 0	51 / 0	0 / 0
I	449	293 / 0	77 / 0	0 / 0	0 / 0	79 / 0	0 / 0
H	222	109 / 0	62 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2 / 7	-122.4	-122.4	0.01 (1)	10.00
B- L	-47 / 10	-104.9	-104.9	0.01 (1)	6.25
L- C	-39 / 0	-104.9	-104.9	0.01 (1)	6.25
C- D	0 / 0	-104.9	-104.9	0.23 (1)	10.00
D- E	0 / 0	-104.9	-104.9	0.23 (1)	10.00
E- N	-39 / 0	-104.9	-104.9	0.01 (1)	6.25
N- F	-47 / 10	-104.9	-104.9	0.01 (1)	6.25
F- G	-2 / 7	-122.4	-122.4	0.01 (1)	10.00
B- K	0 / 29	-28.0	-28.0	0.02 (1)	10.00
K- J	0 / 29	-28.0	-28.0	0.06 (3)	10.00
J- I	0 / 11	-28.0	-28.0	0.09 (3)	10.00
I- H	0 / 11	-28.0	-28.0	0.09 (3)	10.00
H- M	0 / 29	-28.0	-28.0	0.06 (3)	10.00
M- F	0 / 29	-28.0	-28.0	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-164 / 0	0.02 (1)
C- I	-10 / 0	0.00 (1)
I- D	-470 / 0	0.07 (1)
I- E	-10 / 0	0.00 (1)
H- E	-164 / 0	0.02 (1)
K- L	-53 / 0	0.00 (1)
M- N	-53 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (D-E:1) , BC=0.09/1.00 (H-I:3) , WB=0.07/1.00 (D-I:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 13250-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	PB22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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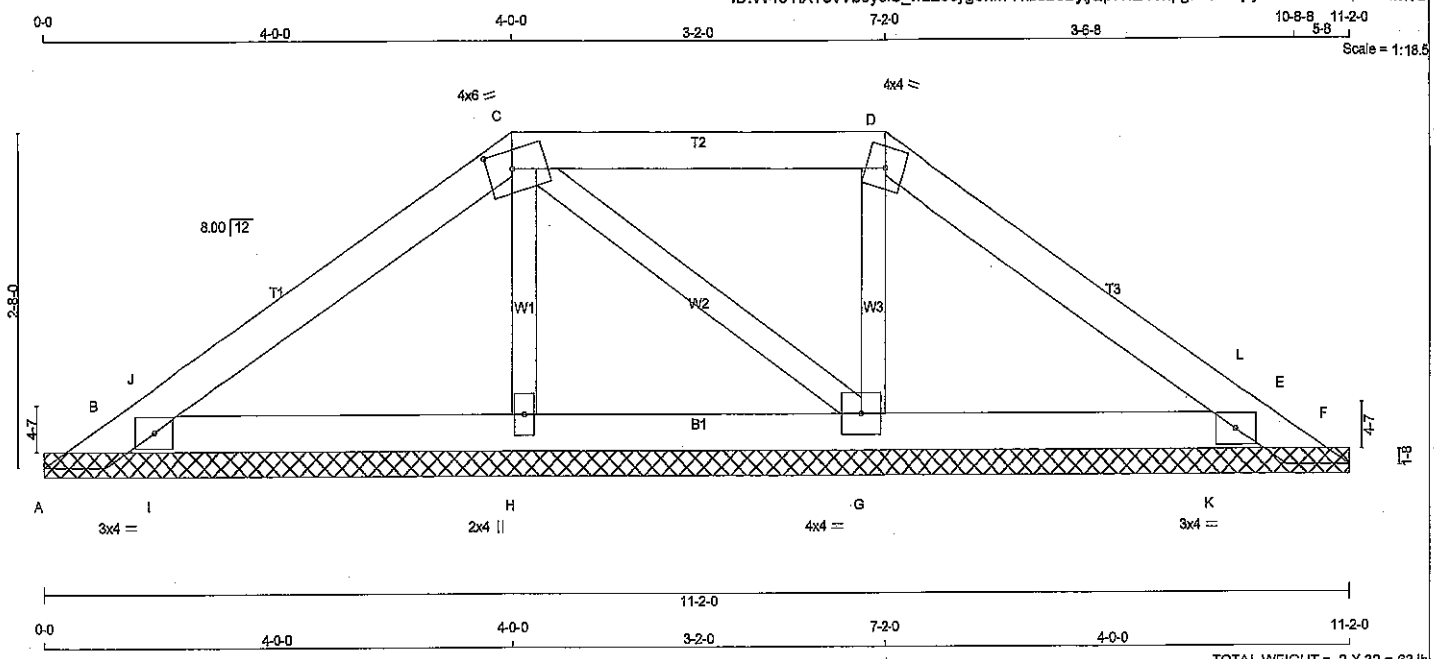

TOTAL WEIGHT = 2 X 32 = 63 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-111	0	0	0	-111	11-2-0 (9-8-2)	11-2-0
F	-112	0	0	0	-112	11-2-0 (9-8-2)	11-2-0
B	484	0	484	0	0	11-2-0 (9-8-2)	11-2-0
E	464	0	464	0	0	11-2-0 (9-8-2)	11-2-0
H	326	0	326	0	0	11-2-0 (9-8-2)	11-2-0
G	397	0	397	0	0	11-2-0 (9-8-2)	11-2-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , F

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-90	0 / -55	0 / -20	0 / 0	0 / 0	0 / -14	0 / 0
F	-91	0 / -56	0 / -20	0 / 0	0 / 0	0 / -14	0 / 0
B	370	250 / 0	55 / 0	0 / 0	0 / 0	65 / 0	0 / 0
E	357	238 / 0	55 / 0	0 / 0	0 / 0	64 / 0	0 / 0
H	271	147 / 0	66 / 0	0 / 0	0 / 0	58 / 0	0 / 0
G	319	191 / 0	67 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 77	-122.4	-122.4	0.15 (1)	10.00	
B- J	-177 / 0	-104.9	-104.9	0.14 (1)	6.25	
J- C	-105 / 0	-104.9	-104.9	0.12 (1)	6.25	
C- D	-33 / 0	-104.9	-104.9	0.18 (1)	6.25	
D- L	-67 / 0	-104.9	-104.9	0.12 (1)	6.25	
L- E	-139 / 0	-104.9	-104.9	0.14 (1)	6.25	
E- F	0 / 78	-122.4	-122.4	0.15 (1)	10.00	
B- I	0 / 77	-28.0	-28.0	0.05 (1)	10.00	
I- H	0 / 77	-28.0	-28.0	0.07 (3)	10.00	
H- G	0 / 67	-28.0	-28.0	0.07 (3)	10.00	
G- K	0 / 46	-28.0	-28.0	0.07 (3)	10.00	
K- E	0 / 46	-28.0	-28.0	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-223 / 0	0.04 (1)
C- G	-42 / 0	0.01 (1)
G- D	-269 / 0	0.04 (1)
I- J	-60 / 134	0.00 (1)
K- L	-61 / 134	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.07/1.00 (H-I:3) , WB=0.04/1.00 (D-G:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1325 / -18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	PB23	14	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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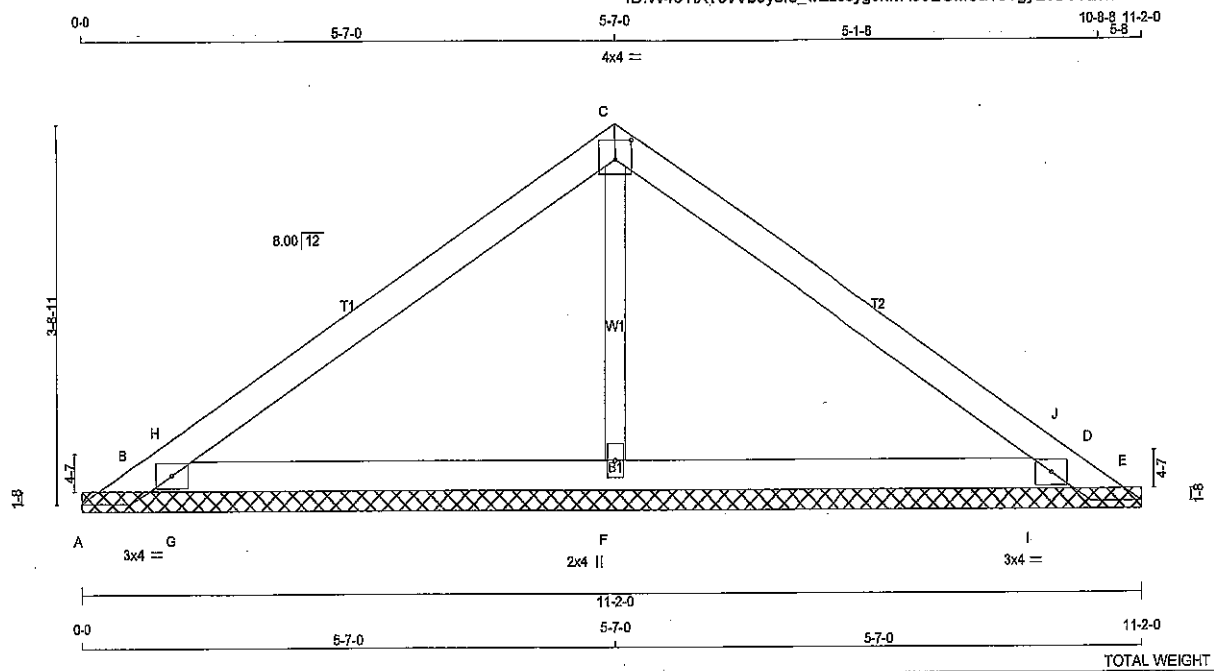

TOTAL WEIGHT = 14 X 29 = 402 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-314	0	0	0	-314	11-2-0 (9-8-2)	1-2-0
E	-314	0	0	0	-314	11-2-0 (9-8-2)	1-2-0
B	812	0	812	0	0	11-2-0 (9-8-2)	1-2-0
D	812	0	812	0	0	11-2-0 (9-8-2)	1-2-0
F	453	0	453	0	0	11-2-0 (9-8-2)	1-2-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , E

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 314 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 314 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-245	0 / -162	0 / -43	0 / 0	0 / 0	0 / -40	0 / 0
E	-245	0 / -162	0 / -43	0 / 0	0 / 0	0 / -40	0 / 0
B	621	422 / 0	93 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	621	422 / 0	93 / 0	0 / 0	0 / 0	106 / 0	0 / 0
F	385	195 / 0	103 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 180	-122.4	-122.4	0.35 (1)		10.00
B- H	-232 / 0	-104.9	-104.9	0.32 (1)		6.25
H- C	-143 / 0	-104.9	-104.9	0.26 (1)		6.25
C- J	-143 / 0	-104.9	-104.9	0.26 (1)		6.25
J- D	-232 / 0	-104.9	-104.9	0.32 (1)		6.25
D- E	0 / 180	-122.4	-122.4	0.35 (1)		10.00
B- G	0 / 100	-28.0	-28.0	0.08 (1)		10.00
G- F	0 / 100	-28.0	-28.0	0.17 (2)		10.00
F- I	0 / 100	-28.0	-28.0	0.17 (2)		10.00
I- D	0 / 100	-28.0	-28.0	0.08 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-260 / 0	0.05 (1)
G- H	-123 / 259	0.00 (1)
I- J	-123 / 259	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (D-E:1) , BC=0.17/1.00 (F-G:2) , WB=0.05/1.00 (C-F:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

DWG NO. TAM 13252-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295556	PB24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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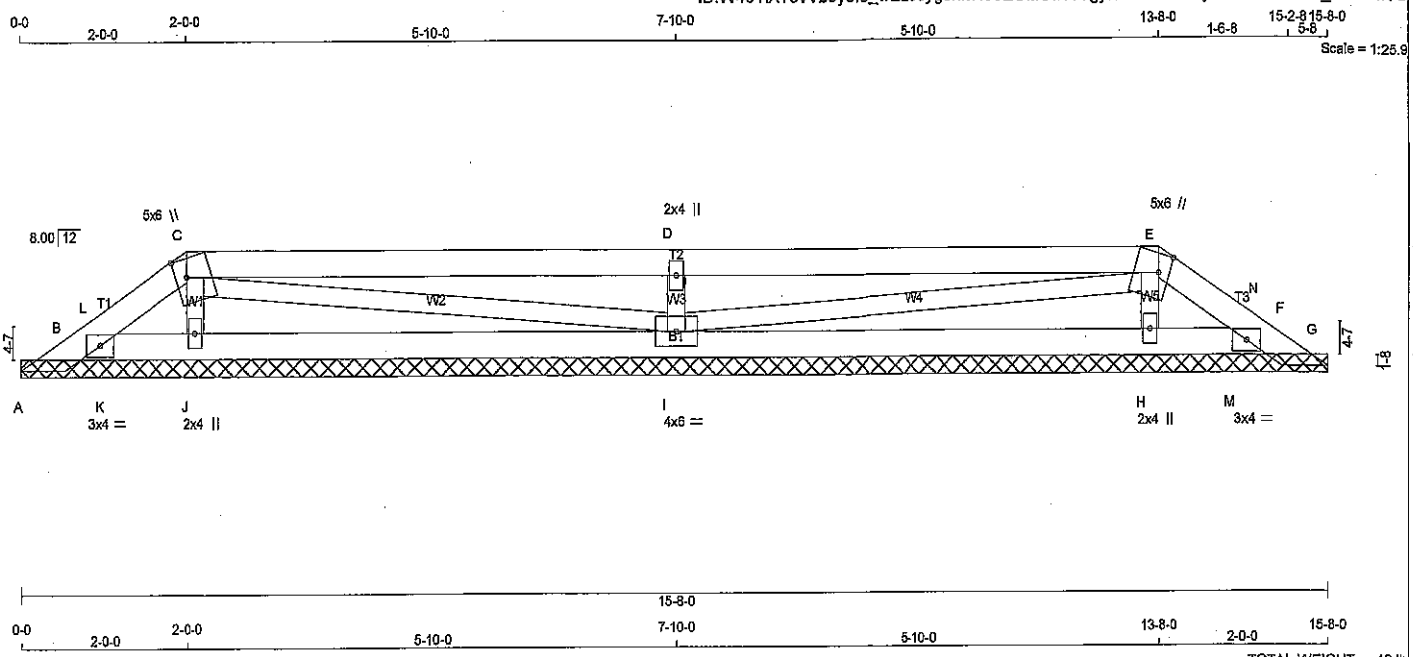

TOTAL WEIGHT = 46 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	39	0	40	0	0	15-8-0 (14-2-2	5-8-0
G	39	0	40	0	0	15-8-0 (14-2-2	5-8-0
B	109	0	109	0	-72	15-8-0 (14-2-2	5-8-0
F	109	0	109	0	-72	15-8-0 (14-2-2	5-8-0
J	408	0	408	0	0	15-8-0 (14-2-2	5-8-0
I	934	0	934	0	0	15-8-0 (14-2-2	5-8-0
H	408	0	408	0	0	15-8-0 (14-2-2	5-8-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , G

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	36	11 / 0	12 / 0	0 / 0	0 / 0	13 / 0	0 / 0
G	36	11 / 0	12 / 0	0 / 0	0 / 0	13 / 0	0 / 0
B	45	95 / 0	0 / -38	0 / 0	0 / 0	0 / -11	0 / 0
F	45	95 / 0	0 / -38	0 / 0	0 / 0	0 / -11	0 / 0
J	362	160 / 0	113 / 0	0 / 0	0 / 0	90 / 0	0 / 0
I	728	474 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	362	160 / 0	113 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-13 / 0	-122.4	-122.4	0.03 (3)	6.25
B- L	-20 / 76	-104.9	-104.9	0.04 (3)	6.25
L- C	-53 / 0	-104.9	-104.9	0.03 (3)	6.25
C- D	-2 / 0	-104.9	-104.9	0.61 (1)	10.00
D- E	-2 / 0	-104.9	-104.9	0.61 (1)	10.00
E- N	-53 / 0	-104.9	-104.9	0.03 (3)	6.25
N- F	-20 / 76	-104.9	-104.9	0.04 (3)	6.25
F- G	-13 / 0	-122.4	-122.4	0.03 (3)	6.25
B- K	-1 / 37	-28.0	-28.0	0.03 (1)	10.00
K- J	-1 / 37	-28.0	-28.0	0.17 (3)	10.00
J- I	-1 / 9	-28.0	-28.0	0.23 (3)	10.00
I- H	-1 / 9	-28.0	-28.0	0.23 (3)	10.00
H- M	-1 / 37	-28.0	-28.0	0.17 (3)	10.00
M- F	-1 / 37	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-240 / 0	0.03 (1)
C- I	-7 / 0	0.00 (1)
I- D	-765 / 0	0.11 (1)
I- E	-7 / 0	0.00 (1)
H- E	-240 / 0	0.03 (1)
K- L	-153 / 0	0.00 (1)
M- N	-153 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.61/1.00 (C-D:1) , BC=0.23/1.00 (H-I:3) , WB=0.11/1.00 (D-I:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

DWG NO. TAM 132538
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	PB41	17	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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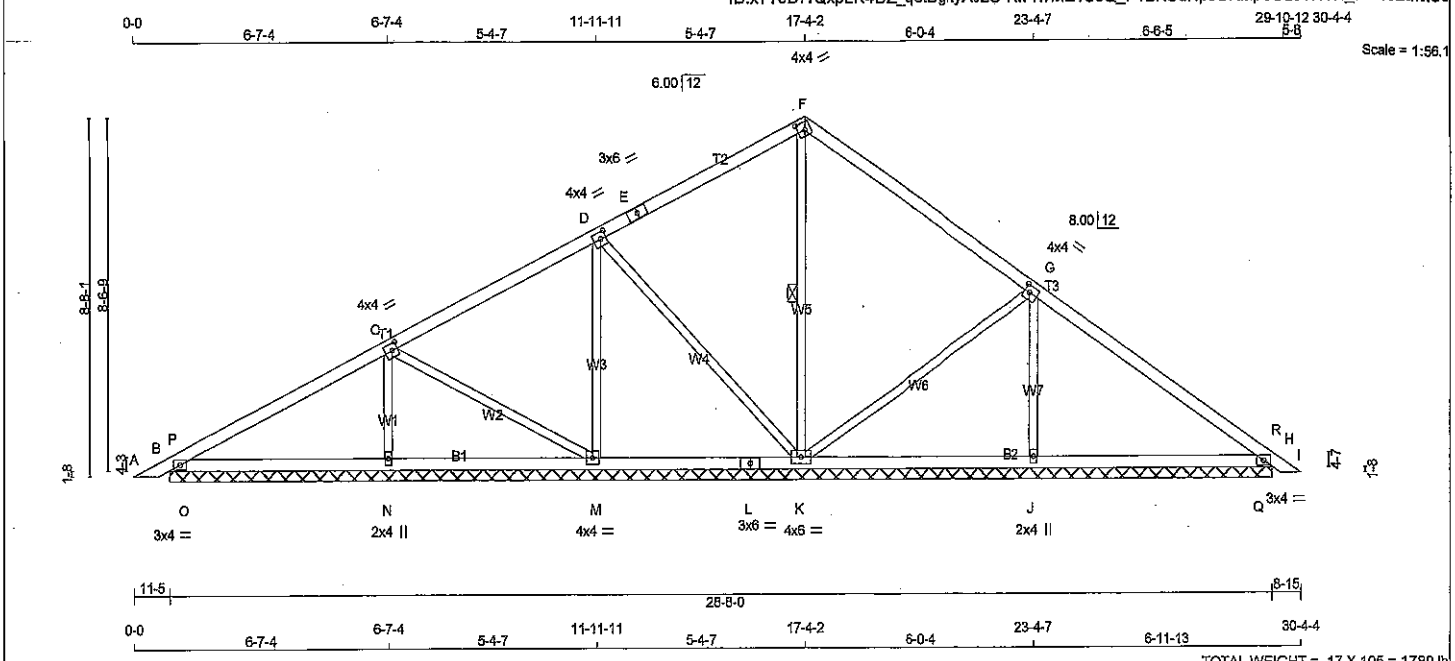

TOTAL WEIGHT = 17 X 105 = 1789 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTW-I	MT20	4.0	4.0	2.25	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMB1-I	MT20	3.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWWW1-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	486	0	486	0	0	28-8-0 (13-8-4)	8-0
N	756	0	756	0	0	28-8-0 (13-8-4)	8-0
M	622	0	622	0	0	28-8-0 (13-8-4)	8-0
K	807	0	807	0	0	28-8-0 (13-8-4)	8-0
J	766	0	766	0	0	28-8-0 (13-8-4)	8-0
H	521	0	521	0	0	28-8-0 (13-8-4)	8-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	367	259 / 0	50 / 0	0 / 0	0 / 0	59 / 0	0 / 0
N	604	365 / 0	120 / 0	0 / 0	0 / 0	118 / 0	0 / 0
M	500	297 / 0	104 / 0	0 / 0	0 / 0	100 / 0	0 / 0
K	632	405 / 0	111 / 0	0 / 0	0 / 0	115 / 0	0 / 0
J	618	363 / 0	131 / 0	0 / 0	0 / 0	125 / 0	0 / 0
H	397	274 / 0	58 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-K.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-104.9	-104.9	0.06 (1)	10.00
B- P	-157 / 0	-104.9	-104.9	0.08 (3)	6.25
P- C	-184 / 0	-104.9	-104.9	0.40 (1)	6.25
C- D	-140 / 0	-104.9	-104.9	0.42 (1)	6.25
D- E	-51 / 0	-104.9	-104.9	0.42 (1)	6.25
E- F	-51 / 0	-104.9	-104.9	0.42 (1)	6.25
F- G	-76 / 0	-104.9	-104.9	0.63 (1)	6.25
G- R	-205 / 0	-104.9	-104.9	0.63 (1)	6.25
R- H	-194 / 0	-104.9	-104.9	0.09 (1)	6.25
H- I	0 / 18	-104.9	-104.9	0.03 (1)	10.00
B- O	0 / 183	-27.5	-27.5	0.23 (1)	10.00
O- N	0 / 183	-27.5	-27.5	0.24 (1)	10.00
N- M	0 / 183	-27.5	-27.5	0.23 (2)	10.00
M- L	0 / 127	-27.5	-27.5	0.17 (2)	10.00
L- K	0 / 127	-27.5	-27.5	0.17 (2)	10.00
K- J	0 / 201	-27.5	-27.5	0.28 (2)	10.00
J- Q	0 / 201	-27.5	-27.5	0.28 (2)	10.00
Q- H	0 / 201	-27.5	-27.5	0.23 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-542 / 0	0.10 (1)
C- M	-64 / 0	0.04 (1)
M- D	-463 / 0	0.25 (1)
D- K	-151 / 0	0.18 (1)
K- F	-434 / 0	0.20 (1)
K- G	-209 / 0	0.23 (1)
J- G	-536 / 0	0.16 (1)
O- P	-342 / 144	0.00 (1)
Q- R	-442 / 211	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
3 676
S KATSOULAKOS
PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63/1.00 (G-R:1) , BC=0.28/1.00 (J-K:2) , WB=0.25/1.00 (D-M:1) , SSI=0.38/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (K) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)

DWG NO. TAM 13220-478
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	PB41G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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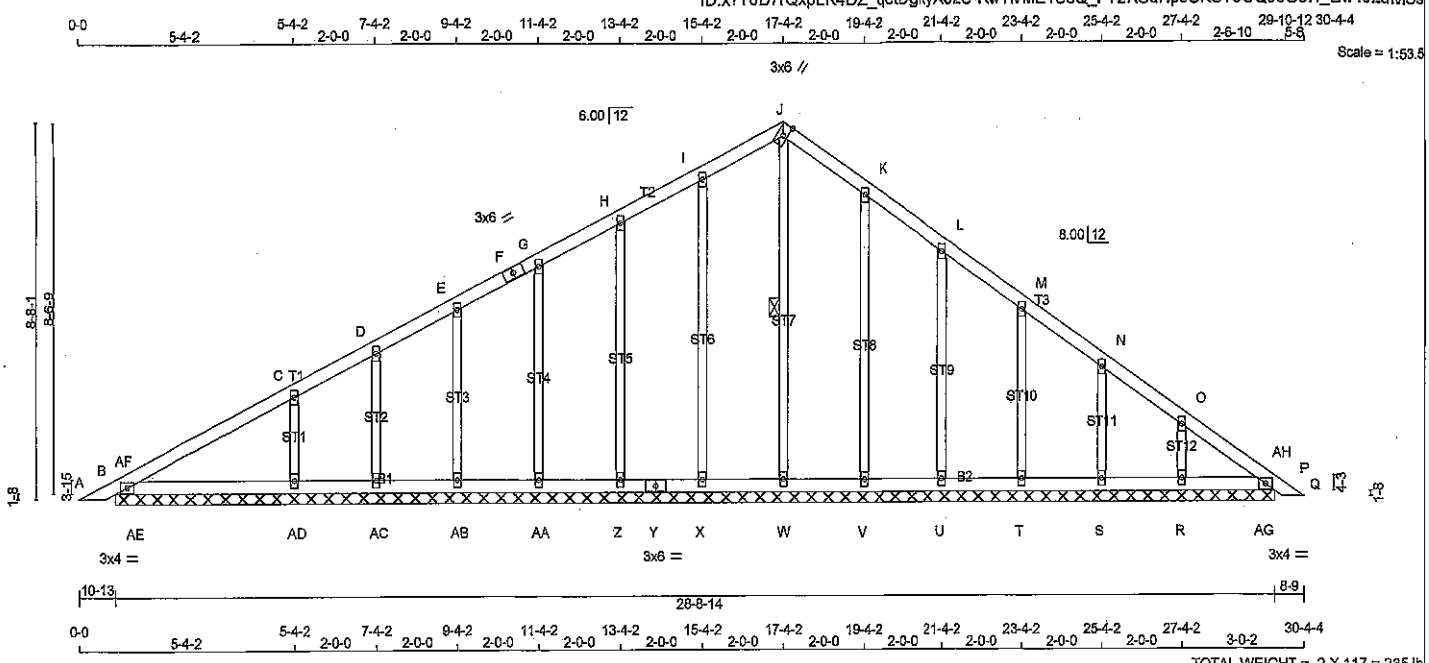

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - Q	2x4	DRY	No.2	SPF
B - Y	2x4	DRY	No.2	SPF
Y - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, G, H, I, K, L, M, N, O						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTW+h	MT20	3.0	6.0	3.25	1.25
P	TMB1-I	MT20	3.0	4.0		
R, S, T, U, V, W, X, Z, AA, AB, AC, AD						
R	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-W.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-104.9	-104.9	0.05 (1)	10.00
B-AF	-58 / 0	-104.9	-104.9	0.06 (3)	6.25
AF- C	-37 / 2	-104.9	-104.9	0.21 (1)	6.25
C- D	-72 / 0	-104.9	-104.9	0.21 (1)	6.25
D- E	-30 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-35 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-35 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-30 / 0	-104.9	-104.9	0.05 (1)	6.25
H- I	-26 / 0	-104.9	-104.9	0.06 (1)	6.25
I- J	-35 / 0	-104.9	-104.9	0.06 (1)	6.25
J- K	-39 / 0	-104.9	-104.9	0.06 (1)	6.25
K- L	-28 / 0	-104.9	-104.9	0.06 (1)	6.25
L- M	-35 / 0	-104.9	-104.9	0.05 (1)	6.25
M- N	-38 / 0	-104.9	-104.9	0.05 (1)	6.25
N- O	-49 / 0	-104.9	-104.9	0.06 (1)	6.25
O-AH	-47 / 0	-104.9	-104.9	0.06 (1)	6.25
AH- P	-69 / 0	-104.9	-104.9	0.02 (3)	6.25
P- Q	0 / 17	-104.9	-104.9	0.03 (1)	10.00
B-AE	0 / 48	-28.0	-28.0	0.15 (1)	10.00
AE-AD	0 / 48	-28.0	-28.0	0.15 (1)	10.00
AD-AC	0 / 36	-28.0	-28.0	0.12 (2)	10.00
AC-AB	0 / 33	-28.0	-28.0	0.03 (2)	10.00
AB-AA	0 / 30	-28.0	-28.0	0.03 (2)	10.00
AA-Z	0 / 27	-28.0	-28.0	0.02 (2)	10.00
Z- Y	0 / 25	-28.0	-28.0	0.02 (2)	10.00
Y- X	0 / 25	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 23	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 23	-28.0	-28.0	0.02 (2)	10.00
V- U	0 / 26	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 29	-28.0	-28.0	0.03 (2)	10.00
T- S	0 / 33	-28.0	-28.0	0.03 (2)	10.00
S- R	0 / 38	-28.0	-28.0	0.04 (2)	10.00
R-AG	0 / 50	-28.0	-28.0	0.05 (1)	10.00
AG- P	0 / 50	-28.0	-28.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- J	-149 / 0	0.07 (1)
X- I	-237 / 0	0.25 (1)
Z- H	-204 / 0	0.14 (1)
AA- G	-205 / 0	0.10 (1)
AB- E	-228 / 0	0.07 (1)
AC- D	-120 / 5	0.03 (1)
AD- C	-414 / 0	0.07 (1)
V- K	-236 / 0	0.22 (1)
U- L	-202 / 0	0.11 (1)
T- M	-210 / 0	0.06 (1)
S- N	-199 / 0	0.04 (1)
R- O	-226 / 0	0.03 (1)
AE-AF	-219 / 103	0.00 (1)
AG-AH	-77 / 35	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-AF:1) , BC=0.15/1.00 (AD-AE:1) , WB=0.25/1.00 (I-X:1) , SSI=0.19/1.00 (B-AE:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (J) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM 13221-08
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	PB42	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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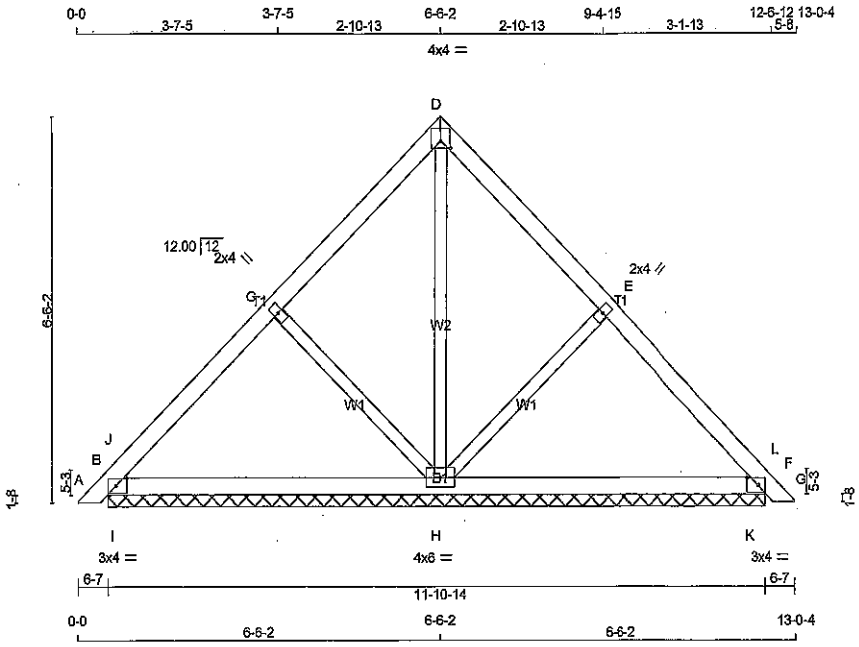

TOTAL WEIGHT = 4 X 46 = 182 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMWWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	427	0	427	0	0	11-10-14	11-10-14
F	427	0	427	0	0	11-10-14	11-10-14
H	822	0	822	0	0	11-10-14	11-10-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	332	218 / 0	56 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	332	218 / 0	56 / 0	0 / 0	0 / 0	57 / 0	0 / 0
H	661	395 / 0	138 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)		10.00
B- J	-461 / 0	-104.9	-104.9	0.10 (3)		6.25
J- C	-245 / 0	-104.9	-104.9	0.16 (1)		6.25
C- D	-33 / 0	-104.9	-104.9	0.16 (1)		6.25
D- E	-33 / 0	-104.9	-104.9	0.16 (1)		6.25
E- L	-245 / 0	-104.9	-104.9	0.16 (1)		6.25
L- F	-461 / 0	-104.9	-104.9	0.10 (3)		6.25
F- G	0 / 15	-104.9	-104.9	0.02 (1)		10.00
B- I	0 / 199	-28.0	-28.0	0.09 (3)		10.00
I- H	0 / 199	-28.0	-28.0	0.26 (2)		10.00
H- K	0 / 199	-28.0	-28.0	0.26 (2)		10.00
K- F	0 / 199	-28.0	-28.0	0.09 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	-231 / 23	0.14 (1)
H- E	-278 / 0	0.08 (1)
C- H	-278 / 0	0.08 (1)
I- J	0 / 420	0.00 (1)
K- L	0 / 420	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-J:1) , BC=0.26/1.00 (H-I:2) , WB=0.14/1.00 (D-H:1) , SSI=0.23/1.00 (B-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.10 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 3222-08
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287665	PB42G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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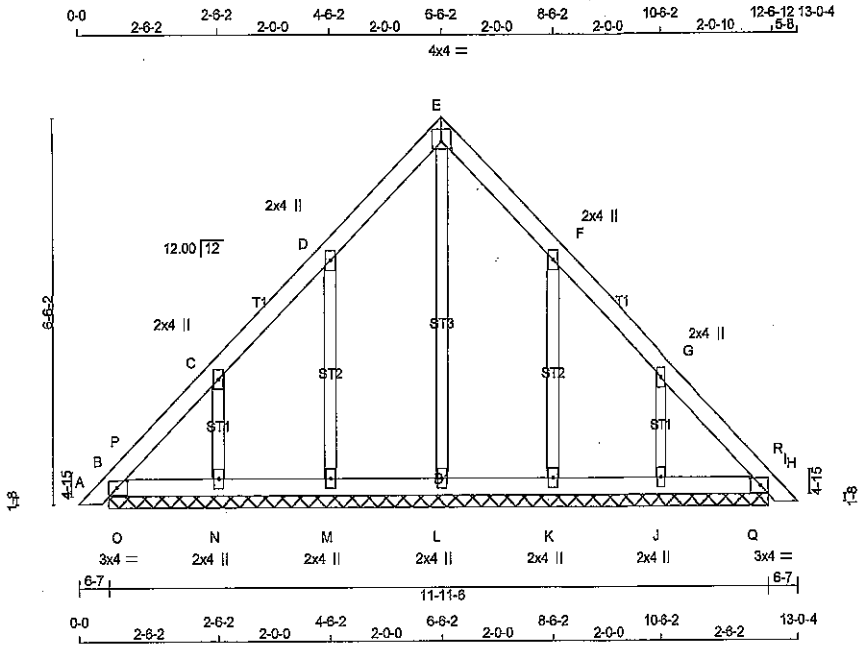

TOTAL WEIGHT = 48 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.25
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.25
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- P	-75 / 0	-104.9	-104.9	0.01 (3)	6.25
P- C	-61 / 0	-104.9	-104.9	0.05 (1)	6.25
C- D	-56 / 0	-104.9	-104.9	0.06 (1)	6.25
D- E	-66 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-66 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-56 / 0	-104.9	-104.9	0.06 (1)	6.25
G- R	-61 / 0	-104.9	-104.9	0.05 (1)	6.25
R- H	-75 / 0	-104.9	-104.9	0.01 (3)	6.25
H- I	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- O	0 / 53	-28.0	-28.0	0.03 (1)	10.00
O- N	0 / 53	-28.0	-28.0	0.03 (1)	10.00
N- M	0 / 42	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 36	-28.0	-28.0	0.03 (2)	10.00
L- K	0 / 36	-28.0	-28.0	0.03 (2)	10.00
K- J	0 / 42	-28.0	-28.0	0.03 (2)	10.00
J- Q	0 / 53	-28.0	-28.0	0.03 (1)	10.00
Q- H	0 / 53	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- E	-106 / 0	0.07 (1)
M- D	-236 / 0	0.07 (1)
N- C	-194 / 0	0.03 (1)
K- F	-236 / 0	0.07 (1)
J- G	-194 / 0	0.03 (1)
O- P	-78 / 29	0.00 (1)
Q- R	-78 / 29	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (D-E:1) , BC=0.03/1.00 (J-Q:1) , WB=0.07/1.00 (D-M:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13223-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289210	PB51	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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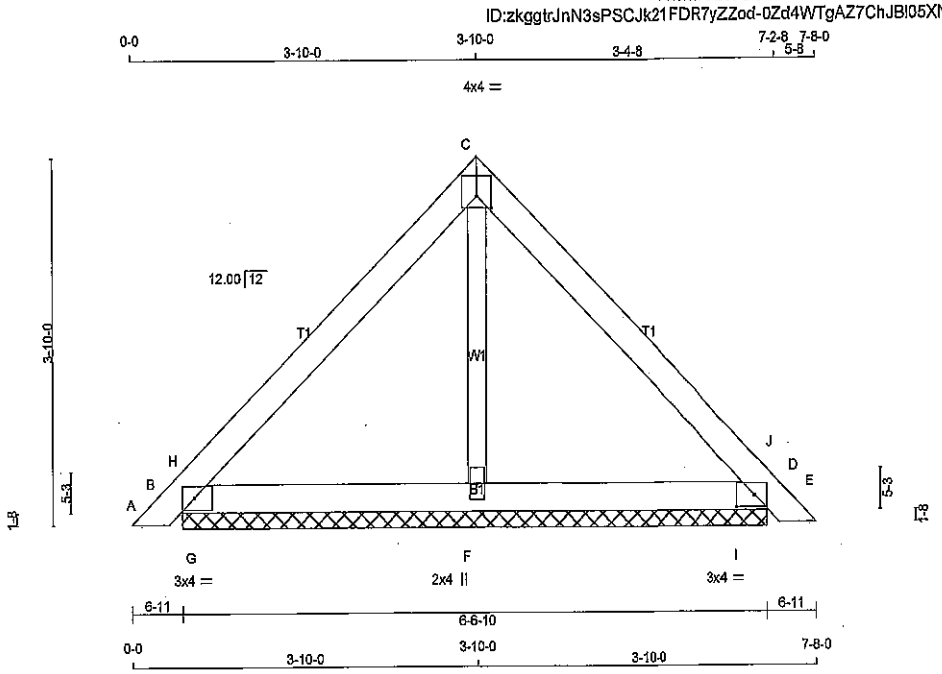

TOTAL WEIGHT = 7 X 22 = 156 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	348	0	348	0	0	6-6-10	6-6-10
D	348	0	348	0	0	6-6-10	6-6-10
F	267	0	267	0	0	6-6-10	6-6-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	259	189 / 0	31 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	259	189 / 0	31 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	233	106 / 0	69 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)		10.00
B- H	-74 / 46	-104.9	-104.9	0.08 (1)		6.25
H- C	-162 / 0	-104.9	-104.9	0.13 (1)		6.25
C- J	-162 / 0	-104.9	-104.9	0.13 (1)		6.25
J- D	-74 / 46	-104.9	-104.9	0.08 (1)		6.25
D- E	0 / 15	-104.9	-104.9	0.02 (1)		10.00
B- G	0 / 105	-27.5	-27.5	0.13 (1)		10.00
G- F	0 / 105	-27.5	-27.5	0.13 (1)		10.00
F- I	0 / 105	-27.5	-27.5	0.13 (1)		10.00
I- D	0 / 105	-27.5	-27.5	0.13 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-75 / 10	0.02 (1)
G- H	-428 / 48	0.00 (1)
I- J	-428 / 48	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-H:1) , BC=0.13/1.00 (F-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.33/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13227-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287647	J21S	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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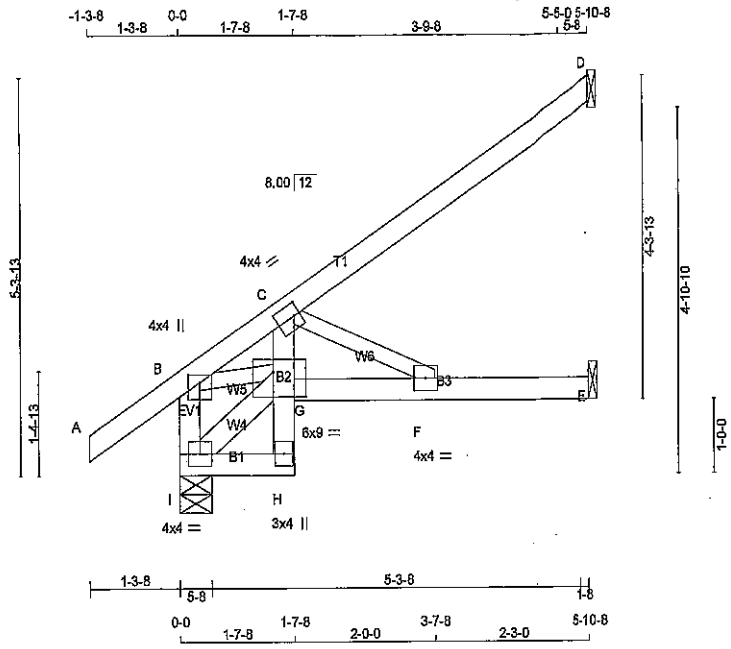

TOTAL WEIGHT = 5 X 24 = 121 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-l	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	551	0	551	0	0	5-8	5-8
D	216	0	216	0	0	1-8	1-8
E	157	0	160	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	420	290 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	152	128 / 0	7 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	148	52 / 0	55 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-511 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-542 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-8 / 8	-104.9	-104.9	0.30 (1)	10.00
I- H	0 / 24	-28.0	-28.0	0.02 (2)	10.00
H- G	0 / 36	0.0	0.0	0.04 (1)	10.00
G- C	0 / 245	0.0	0.0	0.08 (1)	10.00
G- F	0 / 466	-28.0	-28.0	0.48 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.40 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-31 / 0	0.00 (1)
B- G	0 / 440	0.10 (1)
C- F	-533 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 631 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 504 (0.14")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.48/1.00 (F-G:1) , WB=0.10/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.18 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13255-B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287664	J01S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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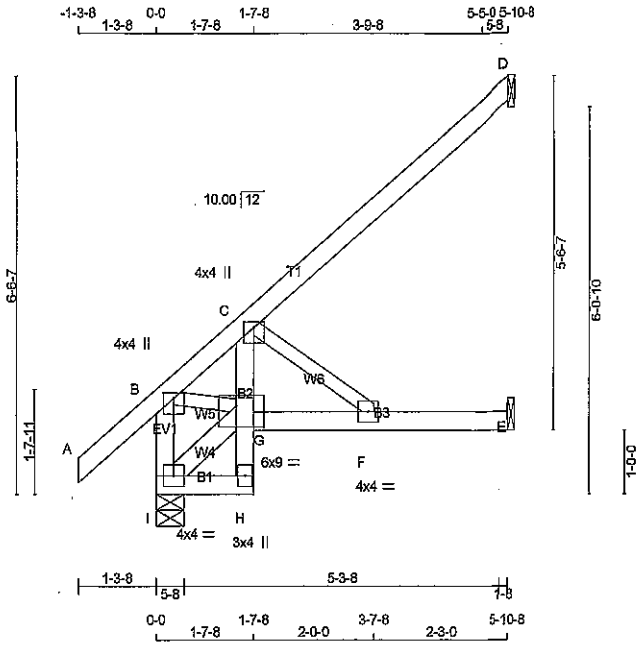

TOTAL WEIGHT = 6 X 26 = 157 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-l	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	553	0	553	0	0	5-8	5-8
D	213	0	213	0	0	1-8	1-8
E	161	0	163	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	421	291 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	148	127 / 0	6 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	151	54 / 0	56 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-522 / 0	0.0	0.0	0.06 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-411 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-12 / 8	-104.9	-104.9	0.29 (1)	6.25
I- H	0 / 13	-28.0	-28.0	0.02 (2)	10.00
H- G	0 / 36	0.0	0.0	0.02 (1)	10.00
G- C	0 / 200	0.0	0.0	0.05 (1)	10.00
G- F	0 / 333	-28.0	-28.0	0.46 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-16 / 0	0.00 (1)
B- G	0 / 310	0.07 (1)
C- F	-399 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 839 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 508 (0.14")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.46/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13187-18
STRUCTURAL
COMPONENT ONLY

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