



Floor Beam\B01

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name:

38514

Address:

City, Province, Postal Code:Bradford, ON Customer:

BAYVIEW WELLINGTON HOMES CCMC 12472-R

Code reports:

GREEN VALLEY ESTATES (\$42-1B)

File Name: S42-1B Description: Designs\B01

Specifier:

Designer:

Alpa Roof Trusses Inc. Company:

Misc:

		z * *
\bowtie	08-10-00	
B0	00-10-00	R1

Total Horizontal Product Length = 08-10-00

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	Dead	Snow	Wind					
B0, 3-1/2"	1,665 / 0	646 / 0							
B1, 3-1/2"	1,788 / 0	692 / 0							

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	04-10-00 40	15		09-02-00
2 FLOOR/STAIRS	Unf. Area (lb/ft^2)	L 04-10-00	08-10-00 40	15	*	10-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,760 ft-lbs	12,704 ft-lbs	0.53	1	04-05-12
End Shear	2,643 lbs	5,785 lbs	0.46	1	07-09-00
Total Load Defl.	L/421 (0.239")	0.419"	0.57	4	04-05-01
Live Load Defl.	L/583 (0.172")	0.279"	0.62	5	04-05-01
Max Defl.	0.239"	1"	0.24	4	04-05-01
Span / Depth	10.6	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material 1
B0	Wall/Plate	3-1/2" x 1-3/4"	3,304 lbs	0.88	0.44	Spruce Pine Fir Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	3,546 lbs	0.94	0.47	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

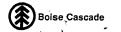
NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS® , VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



T-1508004



Floor Beam\B02

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name:

38514

Address:

City, Province, Postal Code:Bradford, ON

GREEN VALLEY ESTATES (\$42-1B)

BAYVIEW WELLINGTON HOMES

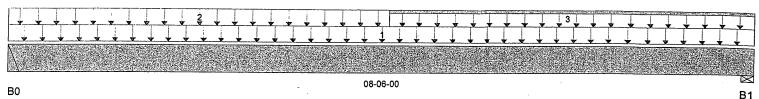
Customer: Code reports: CCMC 12472-R File Name: S42-1B Description: Designs\B02

Specifier:

Designer:

Company: Alps Roof Trusses Inc.

Misc:



B0

Total Horizontal Product Length = 08-06-00

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	Dead	Snow	Wind					
B0	2,095 / 0	863 / 0							
B1, 3-1/2"	1,608 / 0	816 / 0							

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag	Description	 Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
1	FLOOR	Unf. Area (lb/ft^2)	L	00-00-00	08-06-00	40	15			08-00-00
2	FLOOR	Unf. Area (lb/ft^2)	L	00-00-00	04-04-00	40	15			05-08-00
3	WALL	Unf. Lin. (lb/ft)	L	04-04-00	08-06-00		60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	7,460 ft-lbs	12,704 ft-lbs	0.59	1	03-11-01
End Shear	3,094 lbs	5,785 lbs	0.53	1	01-00-08
Total Load Defl.	L/397 (0.244")	0.404"	0.6	4	04-01-08
Live Load Defl.	L/576 (0.168")	0.269"	0.62	5	04-01-08
Max Defl.	0.244"	1"	0.24	4	04-01-08
Span / Depth	10.2	n/a	n/a		00-00-00

Bear	ring Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material 1
B0	Hanger	3" x 1-3/4"	4,221 lbs	0.65	0.66	HUS1.81/10
B1	Wall/Plate	3-1/2" x 1-3/4"	3,432 lbs	0.91	0.46	Spruce Pine Fir

Disclosure

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Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Hanger Manufacturer: Simpson Strong-Tie, Inc.

Resistance Factor phi has been applied to all presented results per CSA 086.

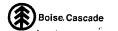
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS



Floor Beam\B03

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

BAYVIEW WELLINGTON HOMES

Customer: Code reports:

CCMC 12472-R

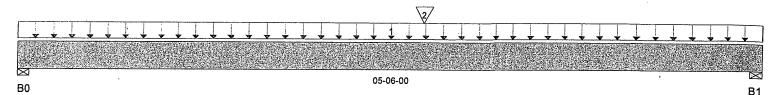
File Name: S42-1B Description: Designs\B03

Specifier:

Designer:

Company: Alps Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 05-06-00

Reaction Summary	(Down / Uplift) (lbs)				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1,090 / 0	457 / 0			
B1. 3-1/2"	1.299 / 0	543 / 0			

Lo	Load Summary					Live	ive Dead	Snow Wind	Trib.	
Tag	Description	Load Type	Ref	. Start	End	1.00	0.65	1.00	1.15	
1	FLOOR	Unf. Area (I	b/ft^2) L	00-00-00	05-06-00	40	15			01-04-00
2	PL B2	Conc. Pt. (I	bs) L	03-00-00	03-00-00	2,095	863			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	5,615 ft-lbs	12,704 ft-lbs	0.44	1	03-00-00
End Shear	2,506 lbs	5,785 lbs	0.43	1	04-05-00
Total Load Defl.	L/999 (0.058")	n/a	n/a	. 4	02-09-13
Live Load Defl.	L/999 (0.041")	n/a	n/a	5	02-09-13
Max Defl.	0.058"	n/a	n/a	4	02-09-13
Span / Depth	6.4	n/a	n/a		00-00-00

			·	Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	2,205 lbs	0.59	0.3	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	2,626 lbs	0.7	0.35	Spruce Pine Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

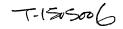
User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.







Floor Beam\B04

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

Customer:

Code reports:

38514

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

BAYVIEW WELLINGTON HOMES CCMC 12472-R

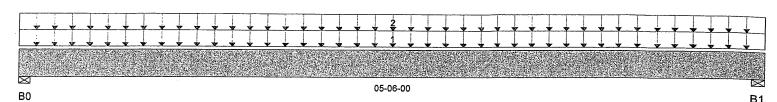
File Name: S42-1B Description: Designs\B04

Specifier:

Designer: F.C.

Company: Alps Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 05-06-00

Reaction Summary	(Down / Uplift) (lbs)				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1,192 / 0	473 / 0			
B1, 3-1/2"	1,192 / 0	473 / 0			

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	05-06-00 40	15		08-04-00
2 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	05-06-00 40	15		02-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,749 ft-lbs	25,408 ft-lbs	0.11	1	02-09-00
End Shear	1,442 lbs	11,571 lbs	0.12	1	01-01-00
Total Load Defl.	L/999 (0.018")	n/a	n/a	4	02-09-00
Live Load Defl.	L/999 (0.013")	n/a	n/a	5	02-09-00
Max Defl.	0.018"	n/a	n/a	4	02-09-00
Span / Depth	6.4	n/a	n/a		00-00-00

Bearing Supports		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2,379 lbs	0.32	0.16	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2,379 lbs	0.32	0.16	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS

O.C., STAGGERED IN TWO ROWS (Top WADED)





Floor Beam\B05

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

Customer:

Code reports:

38514

GREEN VALLEY ESTATES (\$42-1B)

CCMC 12472-R

BAYVIEW WELLINGTON HOMES

City, Province, Postal Code:Bradford, ON

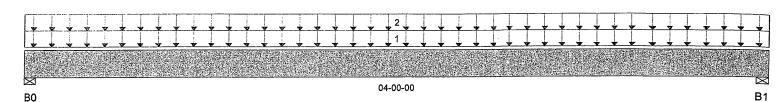
Description: Designs\B05 Specifier:

File Name: S42-1B

Designer:

Company: Alps Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 04-00-00

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	Dead	Snow	Wind					
B0, 3-1/2"	733 / 0	294 / 0							
B1. 3-1/2"	733 / 0	294 / 0							

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	04-00-00 40	15		06-08-00
2 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	04-00-00 40	15		02-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,151 ft-lbs	25,408 ft-lbs	0.05	1	02-00-00
End Shear	673 lbs	11,571 lbs	0.06	1	01-01-00
Total Load Defl.	L/999 (0.004")	n/a	n/a	4	02-00-00
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	02-00-00
Max Defl.	0.004"	n/a	n/a	4	02-00-00
Span / Depth	4.5	n/a	n/a		00-00-00

Bearing Supports		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1,468 lbs	0.19	0.1	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1,468 lbs	0.19	0.1	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

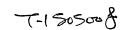
Design based on Dry Service Condition.

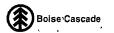
Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS 121 O.C., STAGGERED IN TWO ROWS







Floor Beam\B06

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports: **BAYVIEW WELLINGTON HOMES**

CCMC 12472-R

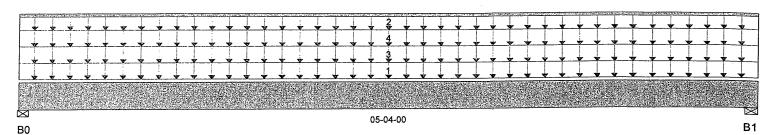
File Name: S42-1B Description: Designs\B06

Specifier:

Designer: F.C.

Company: Alps Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 05-04-00

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	Dead	Snow	Wind					
B0, 3"	631 / 0	720 / 0	1,681 / 0						
B1, 3-1/2"	641 / 0	731 / 0	1,707 / 0						

Load Summary				Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	05-04-00	40	15			00-08-00
2 WALL	Unf. Lin. (lb/ft)	L 00-00-00	05-04-00		60			n/a
3 ROOF	Unf. Area (lb/ft^2)	L 00-00-00	05-04-00	11	10	33		15-03-00
4 ROOF	Unf. Area (lb/ft^2)	L 00-00-00	05-04-00	11	. 10	33		104-00-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4.267 ft-lbs	25,408 ft-lbs	0.17	5	02-08-00
End Shear	2,265 lbs	11,571 lbs	0.2	5	01-00-08
Total Load Defl.	L/999 (0.025")	n/a	n/a	13	02-08-00
Live Load Defl.	L/999 (0.018")	n/a	n/a	17	02-08-00
Max Defl.	0.025"	n/a	n/a	13	02-08-00
Span / Depth	6.2	n/a	n/a		00-00-00

Bearin	ng Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3" x 3-1/2"	3,737 lbs	0.58	0.29	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	3,795 lbs	0.5	0.25	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

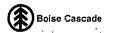
User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS @ (2' O.C., STAGGERED IN TWO ROWS

O.C., STAGGERED IN TWO ROWS (TO P WADED)







Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

Floor Beam\B07

Dry | 1 span | No cantilevers | 0/12 slope (deg)

BC CALC® Design Report

December-04-14

Build 3272

Job Name: Address:

Customer:

Code reports:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

BAYVIEW WELLINGTON HOMES

CCMC 12472-R

File Name: S42-1B

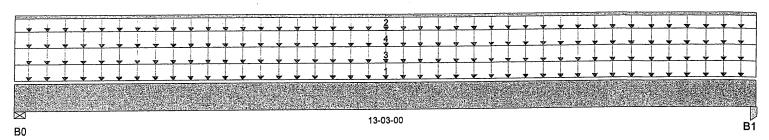
Description: Designs\B07

Specifier:

Designer: F.C. Company:

Alps Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 13-03-00

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	Dead	Snow	Wind				
B0, 3-1/2"	1,749 / 0	2,014 / 0	4,715 / 0					
B1, 3"	1,738 / 0	2,001 / 0	4,686 / 0					

Load Summary				Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
1 FLOOR 2 WALL 3 ROOF	Unf. Area (lb/ft^2) Unf. Lin. (lb/ft) Unf. Area (lb/ft^2)	L 00-00-00 L 00-00-00 L 00-00-00	13-03-00 13-03-00 13-03-00	11	15 60 10	33		00-08-00 n/a 15-03-00
4 ROOF	Unf. Area (lb/ft^2)	L 00-00-00	13-03-00	11	10	33		06-03-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	32,417 ft-lbs	60,415 ft-lbs	0.54	5	06-07-08
End Shear	8,447 lbs	21.696 lbs	0.39	5	01-03-06
Total Load Defl.	L/345 (0.447")	0.642"	0.7	13	06-07-08
Live Load Defl.	L/480 (0.321")	0.428"	0.75	17	06-07-08
Max Defl.	0.447"	1"	0.45	13	06-07-08
Span / Depth	13	n/a	n/a		00-00-00

Bearing Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0 Wall/Plate	3-1/2" x 5-1/4"	10,465 lbs	0.93	0.47	Spruce Pine Fir
B1 Post	3" x 3-1/2"	10,399 lbs	0.47	0.81	Douglas Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.



NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS @ 0.C., STAGGERED IN TWO ROWS

O.C., STAGGERED IN TWO ROWS (TOP LOCADED)



T-1505010



Floor Beam\B08

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports:

BAYVIEW WELLINGTON HOMES

CCMC 12472-R

File Name: S42-1B

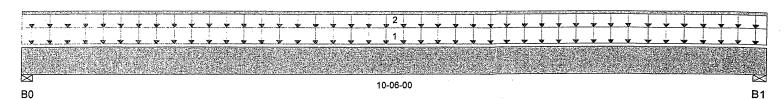
Description: Designs\B08

Specifier:

Designer:

Company: Alps Roof Trusses inc.

Misc:



Total Horizontal Product Length = 10-06-00

Reaction Summary (Down / Uplift) (lbs) Wind Bearing Dead Snow 140 / 0 B0, 3-1/2" 418 / 0 B1, 3-1/2" 140 / 0 418 / 0

Load Summary				Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	10-06-00	40	15			00-08-00
2 WALL	Unf. Lin. (lb/ft)	L 00-00-00	10-06-00		60			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,405 ft-lbs	16,515 ft-lbs	0.09	0	05-03-00
End Shear	465 lbs	7,521 lbs	0.06	0	01-01-00
Total Load Defl.	L/999 (0.049")	n/a	n/a	4	05-03-00
Live Load Defl.	L/999 (0.012")	n/a	n/a	5	05-03-00
Max Defl.	0.049"`	n/a	n/a	4	05-03-00
Span / Depth	12.7	n/a	n/a		00-00-00

Bearing Supports		Dim. (L x W) Demand		Resistance Support	Resistance Member	Material	
B0	Wall/Plate	3-1/2" x 3-1/2"	585 lbs	0.12	0.06	Spruce Pine Fir	•
B1	Wall/Plate	3-1/2" x 3-1/2"	585 lbs	0.12	0.06	Spruce Pine Fir	•

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS (Z' O.C., STAGGERED IN TWO ROWS





Floor Beam\B09

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

City, Province, Postal Code:Bradford, ON

Customer: Code reports: GREEN VALLEY ESTATES (\$42-1B)

BAYVIEW WELLINGTON HOMES

CCMC 12472-R

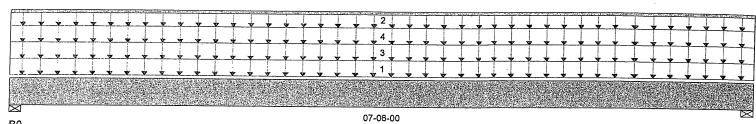
File Name: S42-1B Description: Designs\B09

Specifier:

Designer: F.C.

Alps Roof Trusses Inc. Company:

Misc:



B0

B1

Total Horizontal Product Length = 07-06-00

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	Dead	Snow	Wind	·				
B0, 3-1/2"	901/0	1,027 / 0	2,403 / 0						
B1, 3-1/2"	901 / 0	1,027 / 0	2,403 / 0						

Load Summary				Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	07-06-00	40	15			00-8-00
2 WALL	Unf. Lin. (lb/ft)	L 00-00-00	07-06-00		60			n/a
3 ROOF	Unf. Area (lb/ft^2)	L 00-00-00	07-06-00	11	10	33		04-02-00
4 ROOF	Unf. Area (lb/ft^2)	L 00-00-00	07-06-00	11	10	33		15-03-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	8,823 ft-lbs	25,408 ft-lbs	0.35	5	03-09-00
End Shear	3,796 lbs	11,571 lbs	0.33	5	01-01-00
Total Load Defl.	L/999 (0.107")	n/a	n/a	13	03-09-00
Live Load Defl.	L/999 (0.077")	n/a	n/a	17	03-09-00
Max Defl.	0.107"	n/a	n/a	13	03-09-00
Span / Depth	8.9	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	5,338 lbs	0.71	0.36	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5,338 lbs	0.71	0.36	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS

O.C., STAGGERED IN TWO ROWS (TOP WADES)





Floor Beam\B10

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

06-00-00

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports:

STAIRS

BAYVIEW WELLINGTON HOMES

CCMC 12472-R

File Name: S42-1B

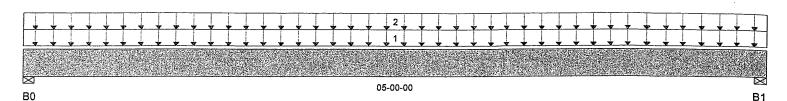
Description: Designs\B10

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 05-00-00

Reaction Summary (Down / Uplift) (lbs) Wind Dead Snow Live B0. 3-1/2" 967 / 0 420 / 0 B1, 3-1/2" 967 / 0 420 / 0

Load Summary Tag Description	Load Type	Ref. Start		ive .00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	05-00-00 40	0	20			03-08-00

00-00-00

05-00-00 40

Factored		Factored	Factored Demand /		Location	
Controls Summary	Demand	Resistance	Resistance	Case		
Pos. Moment	2,037 ft-lbs	12,704 ft-lbs	0.16	1	02-06-00	
End Shear	1,119 lbs	5,785 lbs	0.19	1	01-01-00	
Total Load Defl.	L/999 (0.021")	n/a	n/a	4	02-06-00	
Live Load Defl.	L/999 (0.015")	n/a	n/a	5	02-06-00	
Max Defl.	0.021"	n/a	n/a	4	02-06-00	
Span / Depth	5.7	n/a	n/a		00-00-00	

Unf. Area (lb/ft^2)

Beari	ng Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B0	Wall/Plate	3-1/2" x 1-3/4"	1,975 lbs	0.52	0.26	Spruce Pine	
B1	Wall/Plate	3-1/2" x 1-3/4"	1,975 lbs	0.52	0.26	Spruce Pine	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS Disclosure

15

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER® , AJS™, ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.





Floor Beam\B11

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

BAYVIEW WELLINGTON HOMES

Customer: Code reports:

CCMC 12472-R

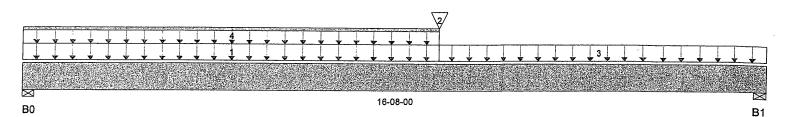
File Name: S42-1B Description: Designs\B11

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 16-08-00

Reaction Summary (Down / Uplift) (lbs) Bearing Wind Live Dead Snow B0, 3-1/2" 622 / 0 770 / 0 B1, 3-1/2" 825 / 0 611 / 0

Lo	ad Summary			•		Live	Dead	Snow	Wind	Trib.
Tag	Description	Load Type	Ref	. Start	End	1.00	0.65	1.00	1.15	
1	FLOOR	Unf. Area (lb/ft^2)	L	00-00-00	09-04-00	40	20			00-06-00
2	PL B10	Conc. Pt. (lbs)	L	09-04-00	09-04-00	967	420			n/a
3	FLOOR	Unf. Area (lb/ft^2)	L	09-04-00	16-08-00	40	20			01-00-00
4	WALL	Unf. Lin. (lb/ft)	L	00-00-00	09-04-00		60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	11,611 ft-lbs	25,408 ft-lbs	0.46	1	09-04-00
End Shear	1,896 lbs	11,571 lbs	0.16	1	15-07-00
Total Load Defl.	L/283 (0.688")	0.81"	0.85	4	08-06-11
Live Load Defl.	L/515 (0.378")	0.54"	0.7	5	08-06-11
Max Defl.	0.688"	1"	0.69	4	08-06-11
Span / Depth	20.5	n/a	n/a		00-00-00

Bear	ing Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1,895 lbs	0.25	0.13	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2,001 lbs	0.27	0.13	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

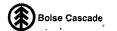
Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS



T-15050K



Floor Beam\B12

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

Customer:

Code reports:

38514

GREEN VALLEY ESTATES (\$42-1B)

City, Province, Postal Code:Bradford, ON

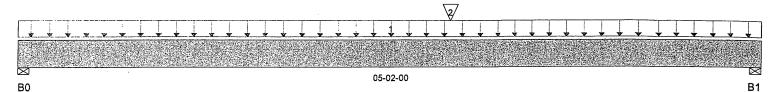
BAYVIEW WELLINGTON HOMES CCMC 12472-R

File Name: S42-1B Description: Designs\B12

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.



Total Horizontal Product Length = 05-02-00

Reaction Summary (D	own / Uplift) (lbs)				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	535 / 0	237 / 0			
B1, 3-1/2"	707 / 0	311 / 0			

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	05-02-00 40	15		01-04-00
2 PL B10	Conc. Pt. (lbs)	L 03-00-00	03-00-00 967	420		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,549 ft-lbs	12,704 ft-lbs	0.2	1	03-00-00
End Shear	1,330 lbs	5,785 lbs	0.23	1	04-01-00
Total Load Defl.	L/999 (0.023")	n/a	n/a	4	02-08-06
Live Load Defl.	L/999 (0.016")	n/a	n/a	5	02-08-06
Max Defl.	0.023"	n/a	n/a	4	02-08-06
Span / Depth	5.9	n/a	n/a		00-00-00

Beari	ng Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1,099 lbs	0.29	0.15	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1,450 lbs	0.38	0.19	Spruce Pine Fir

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD $^{\intercal}$, BCI®, BOISE GLULAM $^{\intercal}$, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS





Floor Beam\B13

BC CALC® Design Report Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name:

38514

Address: GRE

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

Customer:

BAYVIEW WELLINGTON HOMES

Code reports: CCMC 12472-R

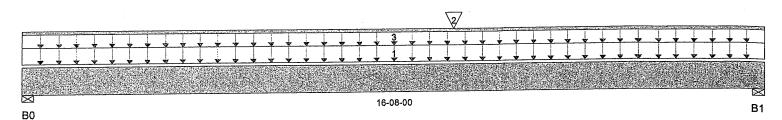
File Name: S42-1B Description: Designs\B13

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 16-08-00

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	Dead	Snow	Wind				
B0, 3-1/2"	1,029 / 0	1,057 / 0						
B1, 3-1/2"	1,303 / 0	1,163 / 0						

Land Company				Live	Dead	Snow	Wind	Trib.
Load Summary	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
Tag Description			40.00.00	40	20			01-00-00
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	16-08-00	40.				- · · · ·
	•	L 09-08-00	09-08-00	1.665	646			n/a
2 PL B1	Conc. Pt. (lbs)		••••	1,000				n/o
3 \N/ALL	Unf. Lin. (lb/ft)	L 00-00-00	16-08-00		60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	18,718 ft-lbs	39,636 ft-lbs	0.47	1	09-08-00
End Shear	3.216 lbs	17.356 lbs	0.19	1	15-07-00
Total Load Defl.	L/265 (0.733")	0.81"	0.9	4	08-07-02
Live Load Defl.	L/474 (0.411")	0.54"	0.76	5	08-07-02
Max Defl.	0.733"	1"	0.73	4	08-07-02
Span / Depth	20.5	n/a	n/a		00-00-00

Rear	ing Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	2,864 lbs	0.25	0.13	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 5-1/4"	3,408 lbs	0.3	0.15	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4
Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS ② (Zご) O.C., STAGGERED IN TWO ROWS







Floor Beam\B14

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports: **BAYVIEW WELLINGTON HOMES**

CCMC 12472-R

File Name: S42-1B Description: Designs\B14

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

\boxtimes	04-06-00	×
В0	04-00-00	B1

Total Horizontal Product Length = 04-06-00

Reaction Summary	(Down / Uplift) (lbs)				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	405 / 0	213 / 0			
B1, 3-1/2"	405 / 0	213 / 0			

Load Summary				Live	Dead	Snow	Wind	Trib.
Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00	1.15	
1 FLOOR	Unf. Area (lb/ft^2)	L 00-00-00	04-06-00	40	20			04-06-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	793 ft-lbs	12,704 ft-lbs	0.06	1	02-03-00
End Shear	453 lbs	5,785 lbs	0.08	1	01-01-00
Total Load Defl.	L/999 (0.007")	n/a	n/a	4	02-03-00
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	02-03-00
Max Defl.	0.007"	n/a	n/a	4	02-03-00
Span / Depth	5.1	n/a	n/a		00-00-00

Bear	ing Supports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	874 lbs	0.23	0.12	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	874 lbs	0.23	0.12	Spruce Pine Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

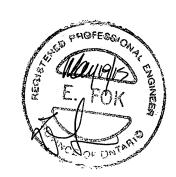
Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods.
Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.







Floor Beam\B15

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name:

38514

Address:

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports:

B1, 3-1/2"

PL B14

BAYVIEW WELLINGTON HOMES

CCMC 12472-R

File Name: S42-1B

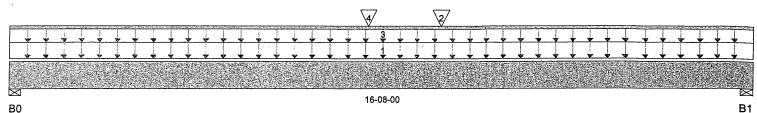
Description: Designs\B15

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



1,497 / 0

Conc. Pt. (lbs)

Total Horizontal Product Length = 16-08-00

Reaction Summary (Down / Uplift) (lbs) Snow Wind Bearing Live Dead B0, 3-1/2" 1,240 / 0 1,208 / 0

1,305 / 0

Wind Trib. Live Dead Snow **Load Summary** Ref. Start 1.00 0.65 1.00 1.15 Tag Description Load Type End FLOOR Unf. Area (lb/ft^2) 00-00-00 16-08-00 40 20 01-00-00 646 2 PL B1 Conc. Pt. (lbs) 09-08-00 09-08-00 1,665 n/a 16-08-00 60 n/a 3 WALL Unf. Lin. (lb/ft) 00-00-00 08-00-00 405 213 n/a

08-00-00

Controls Summary	Factored Demand	Factored Resistance	Demand / Résistance	Load Case	Location
Pos. Moment	21,746 ft-lbs	52,848 ft-lbs	0.41	1	09-08-00
End Shear	3.678 lbs	23,142 lbs	0.16	1	15-07-00
Total Load Defl.	L/299 (0.652")	0.81"	0.8	4	08-07-08
Live Load Defl.	L/526 (0.37") ´	0.54"	0.68	5	08-07-08
Max Defl.	0.652"` ′	1"	0.65	4	08-07-08
Span / Depth	20.5	n/a	n/a		00-00-00

Demand/ Demand/ Resistance Resistance **Bearing Supports** Member Material Dim. (L x W) Demand Support B0 0.11 Spruce Pine Fir Wall/Plate 3-1/2" x 7" 3,369 lbs 0.22 Wall/Plate 3-1/2" x 7" Spruce Pine Fir В1 3,878 lbs 0.26 0.13

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

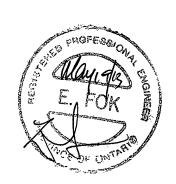
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS

(2' O.C., STAGGERED IN TWO ROWS, Pure 1/2" BOUCS, NOTA e o.c., STALLEGED IN 2 ROWS



T-150504 ite Copy



Floor Beam\B16

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name:

Customer:

38514

Address:

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

BAYVIEW WELLINGTON HOMES CCMC 12472-R

Code reports:

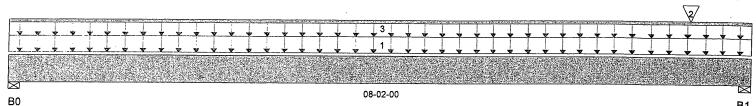
File Name: S42-1B Description: Designs\B16

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



B1

	rotal Horizontal	Product Length =	= 08-02-00

Reaction Summary (Down / Uplift) (Ibs)								
Bearing	Live	Dead	Snow	Wind				
B0, 3-1/2"	132 / 0	318 / 0						
B1, 3-1/2"	491 / 0	506 / 0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag	Description	Load Type	Ref	. Start	End	1.00	0.65	1.00	1.15	
1	FLOOR	Unf. Area (lb/ft^2)	L	00-00-00	08-02-00	40	15			00-88-00
2	PL B14	Conc. Pt. (lbs)	L	07-06-00	07-06-00	405	213			n/a
3	WALL	Unf. Lin. (lb/ft)	L	00-00-00	08-02-00		60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	845 ft-lbs	8,258 ft-lbs	0.1	0	04-03-04
End Shear	765 lbs	5,785 lbs	0.13	1	07-01-00
Total Load Defl.	L/999 (0.039")	n/a	n/a	4	04-02-02
Live Load Defl.	L/999 (0.013")	n/a	n/a	5	04-03-04
Max Defl.	0.039"	n/a	n/a	4	04-02-02
Span / Depth	9.7	n/a	n/a		00-00-00

Bearing Su	pports	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
	/Plate	3-1/2" x 1-3/4"	445 lbs	0.18	0.09	Spruce Pine Fir
	/Plate	3-1/2" x 1-3/4"	1,369 lbs	0.36	0.18	Spruce Pine Fir

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM® VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 4 Deflections less than 1/8" were ignored in the results.

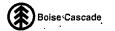
User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS



T-1505015

Site Copy



Floor Beam\B17

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

December-04-14

Build 3272

Job Name: Address:

38514

GREEN VALLEY ESTATES (S42-1B)

City, Province, Postal Code:Bradford, ON

Customer: Code reports: **BAYVIEW WELLINGTON HOMES**

CCMC 12472-R

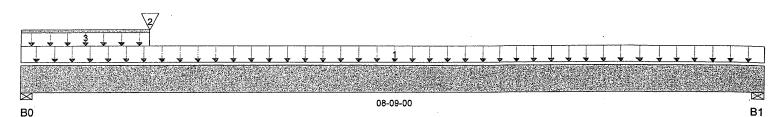
File Name: S42-1B Description: Designs\B17

Specifier:

Designer: F.C.

Company: Alpa Roof Trusses Inc.

Misc:



Total Horizontal Product Length = 08-09-00

	Total Holizolitai i	Todaot Echigin	00 00 00
			المسادات المناسخة
Pagation Summary / Down / Unlift) / Iba			

Reaction Summary (Do	own / Uplift)(lbs)				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	994 / 0	548 / 0			
B1, 3-1/2"	323 / 0	179 / 0			

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	Description	Load Type	Ref	f. Start	End	1.00	0.65	1.00	1.15	
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	08-09-00	40	20			01-00-00
2	PL B10	Conc. Pt. (Ìbs)	L	01-06-00	01-06-00	967	420			n/a
3	WALL	Unf. Lin. (lb/ft)	L	00-00-00	01-06-00		60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,582 ft-lbs	12,704 ft-lbs	0.2	1	01-06-00
End Shear	1,996 lbs	5,785 lbs	0.35	1	01-01-00
Total Load Defl.	L/999 (0.08")	n/a	n/a	4	04-00-01
Live Load Defl.	L/999 (0.053")	n/a	n/a	5	04-00-01
Max Defl.	0.08" `	n/a	n/a	4	04-00-01
Span / Depth	10.5	n/a	n/a		00-00-00

Bear		Dim. (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	2,176 lbs	0.58	0.29	Spruce Pine Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	708 lbs	0.19	0.09	Spruce Pine Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria. Design meets User specified (1") Maximum total load deflection criteria.

Calculations assume Member is Fully Braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4 Deflections less than 1/8" were ignored in the results.

User Notes

NAIL ONE PLY TO ANOTHER WITH 3 1/2" SPIRAL NAILS O.C., STAGGERED IN TWO ROWS @

Disclosure

Products L.L.C.

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.\n\nBC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood







Maximum Floor Spans

Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/360 Deflection Limit 5/8" OSB G&N Sheathing







Depth			В	are			1/2" Gypsum Ceiling					
	Series		On Cent	re Spacing			On Cent	re Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"			
•	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A			
	NI-40×	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A			
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A			
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A			
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A			
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A			
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A			
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A			
11-7/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A			
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A			
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A			
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A			
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A			
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A			
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A			
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A			
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A			
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A			
10	NI-80	23'-11"	22'-1"	.21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A			
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A			

			Mid-Spa	n Blocking		Mid-S	Mid-Span Blocking and 1/2" Gypsum Ceiling					
Depth	Series		On Cent	re Spacing				re Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"			
	NI-20	16'-10"	15'-5"	14'-6"	N/A	17'-1"	15'-5"	14'-6"	N/A			
	NI-40x	17'-11"	16'-11"	16'-4"	N/A	18'-5"	17'-4"	16'-7"	N/A			
9-1/2"	NI-60	18'-2"	17'-1"	16'-6"	N/A	18'-7"	17'-6"		N/A			
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"		N/A			
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	19.2" 14'-6" 16'-7" 16'-10" 17'-7" 17'-8" 17'-6" 19'-3" 19'-6" 20'-5" 20'-8" 21'-2" 21'-7" 21'-11" 22'-11" 23'-9" 24'-2" 25'-2"	N/A			
	NI-20	19'-6"	18'-1"	17'-5"	N/A	20'-2"	18'-8"	17'-6"	N/A			
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-3"	N/A			
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A			
11-7/6	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"		N/A			
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A			
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	17'-6" 19'-3" 19'-6" 20'-5" 20'-8" 21'-2" 21'-7" 21'-11"	N/A			
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A			
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A			
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A			
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A			
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A			
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"		N/A			
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"		N/A			
10	NI-80 ´	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A			
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A			

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/360 and a total load deflection limit of L/240.

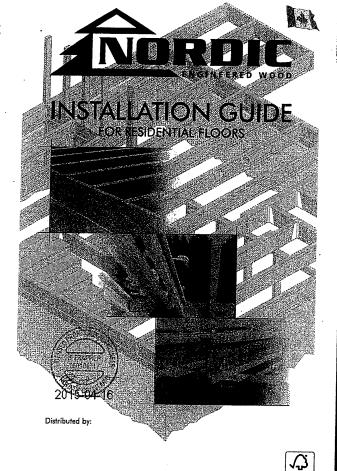
^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, and NBC 2010.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation uidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



SAFETY AND CONSTRUCTION PRECAUTIONS



Never stack building

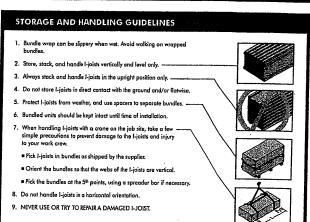
aced, or serious inju

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheafied.

Avoid Accidents by Following these Important Guidelin

- Broce and not each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flonges of the I-joists. Until this sheathing is applied, temporary bracing, aften called struts, or temporary sheathing must be applied to prevent I-joist reliever or buckling.
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1½° nails statened to the top surface of sech I-joist. Nail the bracing to a lateral restraint at the end of each boy. Lop ends of adjoining bracing over at least two I-joist.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure ponels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each 1-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- 5. Never install a damaged I-joist.

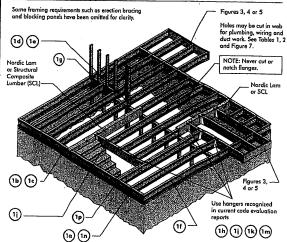
Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic L-joists, failure to follow allowable hade sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



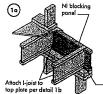
INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that 1-joist flange widths match hanger widths. If not, considerable
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched. 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- I-joists must be anchored securely to supports before floor sheathing is attached, and supports be level.
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Cancentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting flatures, audio equipment and security cameros. Newer suspend nursual or heavy loads from the 1-pist's bottom flange. Whenever possible, suppend all concentrated loads from the top of the 1-pist. Or, ottach the load to blocking that has been securely fastened to the 1-pist webs.
- Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or 1-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to strinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products such as rim board must be cut to fit between the I-joists, and an I-joist-compatible depth selected. 13. Provide permanent lateral support of the bottom flange of all 1-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered 1-joists of the end support next to the cantilever extension. In the completed structure, the gypsum wallboard etiling provides this lateral support. Until the final finished ceiling is applied, temporary backing are structure must be used.
- structure, the gypsum wallboard bracing or struts must be used. 14. If square-edge ponels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeeks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

TYPICAL NORDICI-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



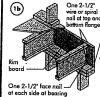
All noils shown in the above details are assumed to be common wire noils unless otherwise noted. 3° (0.122° dia.) common spiral noils may be substituted for 2-1/2° (0.126° dia.) common wire noils. Framing lumber assumed to be Spruce-Rine-Fin No. 2 or better. Individuo Components not shown to scale for clarity.



2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for dacking)

Blacking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (pH)
NI Joists	3,300

*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duretion. It shall not be used in the design of a bending member, such as joist, header, or refiter. For concentrated vertical load transfer, see detail 1d.



-Attach rim board to top plate using 2-1/2° wire or spiral toe-nails at 6° o.c.

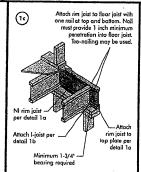
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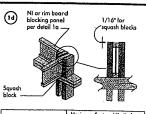
To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Noils tay be driven at an angle to d splitting of bearing plate.

Minimum bearing length shall be 1-3/4* for the end bearings, and 3-1/2* for the intermediate bearings when applicable.

Blocking Panel	Maximum Factored Uniform
or Rim Joist	Vertical Load* (plf)
1-1/8° Rim Board Plus	8,090

The uniform varical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.





Pair of Squash Blocks	Maximum Factored Vertical per Pair of Squash Blocks (lbs)						
·	3-1/2" wide	5-1/2" wide					
2x Lumber	5,500	8,500					
1-1/8° Rim Board Plus	4,300	6,600					



Refer to the Installation Guide for Residential Floors for additional information.

CCMC EVALUATION REPORT 13032-R

NI-90x 1-1<u>2</u> 3-1₂ 4 1-1/2 3-1/2 1 SB 204 -STORESSION. OSB3/8, 1 NI-40x 1.1.7 3.1.7 OSB 3/6* 1-12-17 OSB 3/8"→ ← OSB 3/8° OSB 716*-NI-20 1-1/2 B 3/8° -> 11.7/8* 14* 16* J. PRAPPIER 100108717 FSC 1950f MSR 2100f MSR 1950f MSR 2100f MSR 2400f MSR NPG Lumber 33 pieces 33 pieces 33 pieces 23 pieces 23 gieres 23 pieces 23 pieces per unit per unit per unit

WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- 1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.

 Lipist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- 3. Whenever possible, field-cut holes should be centred on the middle of the web.
 4. The maximum size hole or the maximum depth of a duct chose opening that
- can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
- 5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- the diameter of the maximum round hole permitted at that location.

 6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the langest side of the langest redangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.

 7. A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be
- ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- 8. Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

TABLE 2

Simple Span Only

- 9. A 1-1/2 inch hole or smaller can be placed anywhere in the web A 1-1/2 inch note or stripler can be placed unymere in the web
 provided that it meets the requirements of rule number 6 above.
 All holes and dud chase openings shall be cut in a workman-like
- manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase opening.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Minimum distance from inside face of supports to centre of opening (ft - in.)

TABLE 1

LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

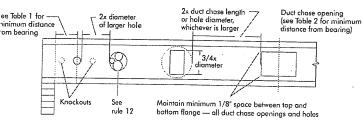
Joist	Inink	Joist Minimum Distance from Inside Face of Any Support to Centre of Hale (ft - in.)														
Depth	Series			_					le Diam							
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4
	NI-20	0'-7"	1'-6"	2'-10"	4'-3"	5'-8"	6'-0"									
	NI-40x	0'-7"	1'-6"	3'-0"	4'-4"	6'-0"	6'-4°									
9-1/2"	NI-60	1'-3"	2'-6"	4'-0"	5'-4"	7'-0"	7'-5"									
	NI-70	2'-0"	3'-4"	4'-9"	6'-3"	8'-0"	8'-4"									
	NI-80	2'-3"	3'-6"	5'-0°	6'-6"	8'-2"	8'-8"		•••							
	NI-20	0'-7"	0'-8"	1'-0"	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"						***
	NI-40x	0'-7"	0'-8"	1'-3"	2'-8"	4'-0"	4'-4"	5'-5"	7'-0"	8'-4"						
	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5'-9"	6'-0"	7'-3"	8'-10"	10'-0"						
11-7/8"	NI-70	1'-3"	2'-6"	4'-0"	5'-4"	6'-9"	7'-2"	8'-4"	10'-0"	11'-2"						
	NI-80	1'-6"	2'-10°	4'-2"	5'-6"	7'-0"	7'-5"	8'-6"	10-3	11'-4"						
	NI-90	0'-7"	0'-8"	1'-5"	3'-2"	4'-10"		6'-9"	8'-9"	10'-2"						
	NI-90x	0'-7"	0'-8"	0'-9"	2'-5"	4'-4"	4'-9"	6'-3"		10-2						
	NI-40x	0'-7"	0'-8"	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10' 0"			
	NI-60	0'-7"	0'-8"	1'-8"	3'-0"	4'-3"	4'-8"	5'-8"	7'-2"	8'-0"	8'-8"	10'-4"	10'-2"			
14"	NI-70	0'-8"	1'-10"	3'-0"	4'-5"	5'-10"	6'-2"	7'-3"	8'-9"	9'-9"	10'-4"					
14	NI-80	0'-10"	2'-0"	3'-4"	4'-9"	6'-2"	6'-5"	7'-6"	9'-0"	10'-0"	10-4	12'-4"	13'-5"			
ı	NI-90	0'-7"	0'-8"	0'-10"	2'-5"	4'-0"	4'-5"	5'-9"	7'-5"	8'-8"	9'-4"		13'-9" 12'-11"			
- 1	NI-90x	0'-7"	0'-8"	0'-8"	2'-0"	3'-9"	4'-2"	5'-5"	7'-3"	8'-5"	9'-2"					
	NI-60	0'-7"	0'-8"	0'-8"	1'-6"	2'-10"	3'-2"	4'-2"	5'-6"	6'-4"		DI C0	01.0"	701.0		
l	NI-70	0'-7"	1'-0"	2'-3"	3'-6"	4'-10"	5'-3"	6'-3"	7'-8"	8'-6"	7'-0"	8'-5"	91-8"	10'-2"		13'-9"
16"	NI-80	0'-7"	1'-3"	2-6"	3'-10"	5'-3"	5'-6"	6'-6"	8'-0"	9'-0"	9'-2"	10'-8"	12'-0"		14'-0"	15'-6"
	NI-90	0'-7"	0'-8"	0'-8"	1'-9"	3'-3⁼	3'-8"	4'-9"	6'-5"		9'-5"	11'-0"	12'-3"		14'-5"	16'-0"
J	NI-90x	0'-7"	0'-8"	0'-9"	2'-0"	3'-6"	4'-0"	5'-0°	6-9	7'-5"	8'-0"	9'-10"	11'-3"		13'-9"	15'-4"
	111-70X			· · ·	2-0	5-0	4.0	J-U	0-7"	7'-9"	8'-4"	10'-2"	11'-6"	12'-0"		

- Above table may be used for 1-joist spacing of 24 inches on centre or less.
 Hole location distance is measured from inside face of supports to centre of hole.
 Distances in this chart are based on uniformly loaded joists.
 The above table is based on the 1-joist being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

Duct Chase Length (in.) 4'-5" 5'-8" 5'-9" 5'-5" 5'-8" 5'-8" 7'-5" 8'-6" 8'-9" 8'-4" 8'-6" 9'-4" 6'-10" 7'-1" 6'-7" NI-40x 9-1/2 NI-60 6'-10" 8'-2" 8'-9" 10'-1" 6'-2" 7'-2" 7'-8" 7'-4" 7'-7" 7'-11" NI-20 7'-5" 8'-6" 9'-0" 8'-7" NI-40 10'-9' 10'-9" 11'-0" 10'-4" 10'-8" 10'-11" 11'-2" 12'-8" 13'-0" 12'-4" NI-60 NI-70 NI-80 NI-90 10'-3" 10'-1" 10'-2" 10'-7" 11-7/8" 8'-10" 9'-2" NI-90 10'-1' 10'-7' 10-1 14" NI-RO 11:5 10'-3" 10'-7" 10'-3" 10'-1" 10'-4" 10'-8" 10'-5" 10'-9" 11'-2" 12'-1" 12'-6" 11'-10" 12'-3" 12'-1" 12'-7" 12'-6" 13'-0" NI-60 14'-10" 12'-8" 13'-1" 13'-6" 13'-9" 16" 11'-10" Above table may be used for I-joist spacing of 24 inches on centre or less.
 Duct chase opening location distance is measured from inside face of supports to centre of opening.
 The above table is based on simple-span joists only. For other applications, contact your local distributor.
 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 pst and dead load of 15 pst, and a live load deflection limit of I/480.
 The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for sharter spans; contact your local distributor.

DUCT CHASE OPENING SIZES AND LOCATIONS

IELD-CUT HOLE LOCATOR





Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-jaist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

AFETY AND CONSTRUCTION PRECAUTIONS



WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- Brace and noil each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends.
 When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover
- or buckling.

 Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each 1-joist. Noil the bracing to a lateral restraint at the end of each bay, Lap ends of adjoining bracing over at least two 1-joists.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of 1-joists at the end of the bay. 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- Install and fully noil permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building
 materials over beams or walls only.
- 5. Never install a damaged l-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic Lipists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



PRODUCT WARRANTY

Chantiers Chibougamau guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibougamau warrants that our products, en utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.





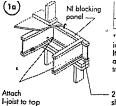


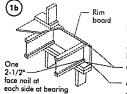
plate per detail 1b

3,300 The uniform vertical load is limited to a joist depth of 16 es or less and is based on standard term load duration It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load

Maximum Factored Uniform

Vertical Load* (plf)

2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for decking)



Blocking Panel	Maximum Factored Uniform
or Rim Joist	Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

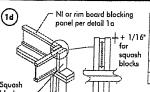
The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

One 2-1/2" wire or spiral nail at top and battom flange

Attach rim board to top plate using 2-1/2" wire or spiral toe-nails at 6" o.c.

To avoid splitting flange, start nails at least 1-1/2° from end of 1-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

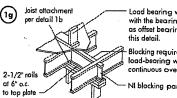


Pair of Squash Blocks	Maximum Factored Vertical Load per Pair of Squash Blocks (lbs				
	3-1/2" wide	5-1/2" wide			
2x Lumber	5,500	8,500			
1-1/8" Rim Board Plus	4,300	6,600			

Provide lateral bracing per detail 1a or 1b



Transfer land from above to bearing below. Install squash blacks ner Match bearing area of blocks below to post



Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by

Blocking required over all interior supports under load-bearing walls or when floor joists are not continuous over support

NI blocking panel per detail 1a

Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a (1h) double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3' nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.

Blocking Panel

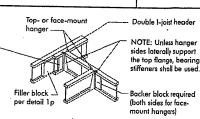
or Rim Joist

NI Joists

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"]"	5-1/2"
3-1/2°	1-1/2"	7-1/4"

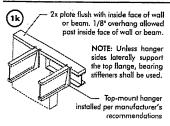
- * Minimum grade for backer block material shall be S-P.F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-0325 or CAN/CSA-0437 Standard.
- ** For face-mount hangers use net joist depth minus 3-1/4* for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".

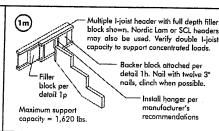


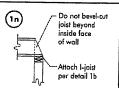
For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

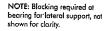
(ii) Nordic Lam or Structural Composite Lumber (SCL) For nailing schedules for multiple beams, see the manufacturer's recommendations. Top- or face-mount hange installed per manufacturer's

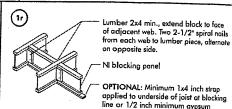
NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.







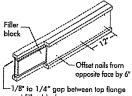




recommendations

OPTIONAL: Minimum 1x4 inch strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists

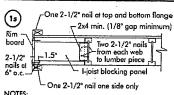




-1/8" to 1/4" gap between top flange and filler block

- 1. Support back of I-joist web during nailing to prevent damage to web/flange connection.
- 2. Leave a 1/8 to 1/4-inch gap between top of filler black and bottom of top I-joist flange.
- 3. Filler block is required between joists for full length
- 4. Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double t-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
 The maximum factored load that may be applied to one
- side of the double joist using this detail is 860 lbf/ft. Verify double I-joist capacity.

Flange Size	Net Depth	Filler Block Size
2-1/2" x 1-1/2"	9-1/2" 11-7/8" 14" 16"	2-1/8" x 6" 2-1/8" x 8" 2-1/8" x 10" 2-1/8" x 12"
3-1/2" x 1-1/2"	9-1/2" 11-7/8" 14" 16"	3" x 6" 3" x 8" 3" x 10" 3" x 12"
3-1/2" x 2"	11-7/8" 14" 16"	3" x 7" 3" x 9" 3" x 11"



NOTES:

In some local codes, blocking is prescriptively required
in the first joist space (or first and second joist space)
next to the starter joist. Where required, see local code
requirements for spacing of the blocking.
 All nails are common spiral in this detail.

WEB STIFFENERS

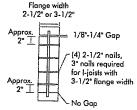
RECOMMENDATIONS.

- A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the 1-joist properties table found of the 1-joist Construction Guide (C101). The gap between the stiffener and the flange is at
- A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top. A load stiffener is required at locations where a factored concentrated
- load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

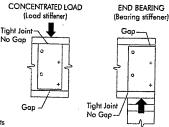
FIGURE 2

WEB STIFFENER INSTALLATION DETAILS

with opposite



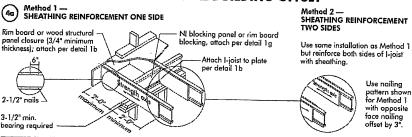
See the adjacent table for web stiffener size requirements



STIFFENER SIZE REQUIREMENTS

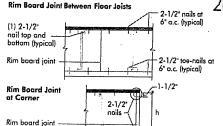
	Flange Width	Web Stiffener Size Each Side of Web
	2-1/2*	1" x 2-5/16" minimum width
	3-1/2°	1-1-10" x 2-5/16"
	197	To the same
		PPAPPIER A
١	18.5	7

CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET

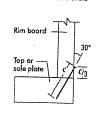


NOTE: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall motch the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach i-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

RIM BOARD INSTALLATION DETA (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT



15 1



WEB HOLES

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- I-joist top and bottom flanges must NEVER be cut, notched, or otherwise madified.
- Whenever possible, field-cut holes should be centred on the middle of the web.
- The maximum size hole or the maximum depth of a dust chase opening that can be out into an I-joist web shall equal the clear distance between the flanges of the I-joist multi 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
- The sides of square holes or langest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 3/4 or me diameter or me maximum route note partnessed at that location. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole for rivice the length of the languest tide of the languest rectangular hole or duct chase opening and each hole and duct chase opening shall be sized and lacated in compliance with the requirements of Tables 1 and 2, respectively.
- A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or dua chase openings.
- Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a contilevered section of a joist. Holes of greater size may be permitted subject to
- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it
 meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

LOCATION OF CIRCULAR HOLES IN JOIST WEBS Simple or Multiple Span for Dead Loads up to 15 psf and Live Leads up to 40 psf

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- Above table may be used for I-joist spacing of 24 inchias on centre or less.
 Hole location distance is measured from inside face of supports to centre of hole.
 Distances in this char are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the L-joists used at their maximum span. If the L-joists are placed at less than their full maximum span (see Maxime minimum distance from the centraline of the hole to the face of any support (D) as given above any be reduced as follows:

Distance from the inside face of any support to centre of hele, reduced for less-than-ma distance shall not be less than 6 inches from the face of the support to edge of the hole.

The actual measured pan distance between the inside faces of supports (fil. 5 ppn Adjustment Factor given in this toble.

The minimum distance for-Dreduced = Lactual x D SAF Where: Dreduced =

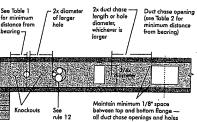
Lactual

The actual measured span distance between the inside foces of supports (ft). Span Adjustment Factor given in this toble.

The minimum distance from the inside face of any support to centre of hole from this table if <u>actual</u> is greater than 1, use 1 in the above calcutation for <u>actual</u>.

SAF

FIELD-CUT HOLE LOCATOR



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.

Knackouts are prescored holes provided for the contractor's convenience to install electrical or small jumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the lipid. Where possible, it is preferable to use knackouts instead of field-ut blade.



Never drill, cut or notch the flange, or over-cut the web.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the cratangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the 1-joist.



5 04 16

- Above table may be used for Ljoist specing of 24 inches on centre or less.

 Duct chase opening location distance is measured from fuside face of supports to centre of opening.

 Duct chase opening location distance is measured from fuside face of supports to centre of opening.

 Duct chase opening to centre of the control of the control opening to centre opening the control opening to centre of the control opening the contro

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- Lay the first ponal with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when topped into place with a black and sledgehammer.
- Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- Apply two lines of glue on 1-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread give in the groove of one or two panels at a time before laying the nest row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a fininer line (1/8 incl) than used on I-joil flanges.
- 8. Tap the second row of panels into place, using a black to protect groove edges.
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2* common nail to assure accurate and consistent spacing.)
- 10. Complete all notifing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue seting.) Use 2' ring- or screw-shook nails for panels 3/4-inch thick or less, and 2-1/2' ring- or screw-shook nails for thicker ponels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked an right away and will carry construction loads without damage to the glue bond.

FASTENERS FOR SHEATHING AND SURFICIONING(1)

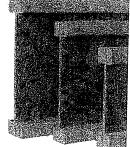
Maximum Minimum		Vail Size and T	уре	Maximur of Fa	n Spacing
Sporing Thickness (in.) (in.)	Wire or Spiral Neils	Nails or Serows	Stoples	Edges	Intern Supports
16" 6 5/8	2*	1-3/4"	2*	6"	12*
20 5/8	2*	1-3/4*	2*	6*	12*
24 3/4	. 2*	1-3/4*	2"	6-	12.

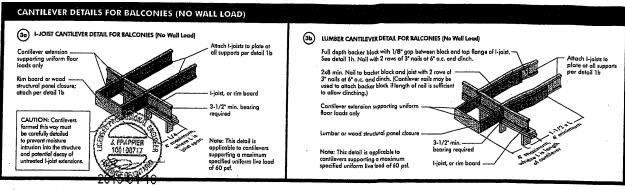
- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- Special conditions may impose heavy traffic and concentrated loads that require construction in excess
 of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGS8-71.26 Standard, Adhesives for Field-Glving Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.
- Ref.: NRC-CNRC, National Building Cade of Canada 2010, Table 9.23.3.5.

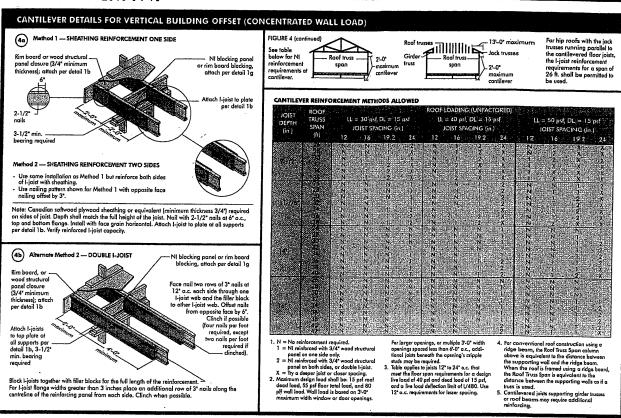
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

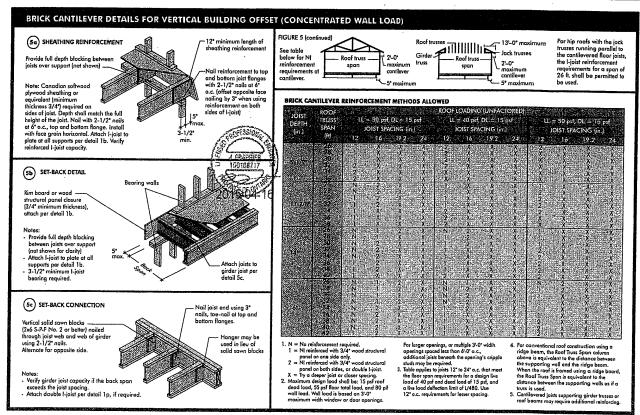
RIM BOARD INSTALLATION DETAILS (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT ard Joint Between Floor Joists 2-1/2" nails at 6" o.c. (typical) Rim board Joint at Corner 1-1/2* (typical) 2-1/2" toe-nails at 6" o.c. (typical) Rim board joint-(86) TOE-NAIL CONNECTION AT RIM BOARD (8c) 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL Exterior sheathing Existing stud wall Remove siding at ledger prior to installation Rim board ----Floor sheathing Continuous flashing extending at least 3" past joist hanger - Staggered 1/2" ameter lag screws or thru-balts with washers Deck joist - FOREISIO Existing foundation wall L FRAFFIER 2x ledger board (preservative-treated); must be greater than or equal to the depth of the deck joist 100100717











MAXIMUM-FLOOR SPANS

- Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration of floor vibration and a live load deflection limit of L/48D. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joint spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGSB-71.26
 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used of gypsum and/or a row of blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spons and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with ather than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI unils conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

MAXIMUM FLOOR SPANS FOR NORDIC 1-JOISTS SIMPLE AND MULTIPLE SPANS

taiot toist			bia spans			Multi	ple spans	
Depth Series		On cor	ifro spaci	ıg :		On con	tre spacing	
	12*	16"	19.2	24	12"	16"	19.2"	24
	317.15	412	1349	0.213/5	翻譯的影		ar ar 210%	E147
1.04404		+ 115.2	14.8				3.1540	15'5
9000	16-3	152	4.10			16.7	16-0	101
		16-1	15 ¢	15.7			16'-9'	16 10
	2173	34 16 3	1548	5.9			910318 AV	77.0
	16.11	14.0	-145.5	15-6		(FE) (17) 35;	6.8	16.7
	18.1	17.0	16-5	16.6			17.9	177.
1 1 1 1 1	184	17:3' 8'-0'	16-7 17-4	16'-9		18.95	18-0	8-1 19:1
	19.9	8.3		17'5		19:11	19.0	
	20.21	8.7	17-6 17-10			20.27	19.3	94
1 100	204	8.9				20 T	19.8	19:11:
SOUTH OF STREET, THE	20-14-3	18:7	32-17-10			20.6	17.8	199
	20.5	18-11	18.1	10.2				20:1
		20 0		17.2			23,000	21-2
		2043				277.50	0115	24 16
100	300	20-8	19.9			22 10		211.10
and the second	22.7	20-11					72.0	22.2
\$200 E 125 E 200 C	22-3	20-8	17.9	19.10		22.0	27.9	21:10
	23.6	2129	20.9	20		24'0"	727115	23'-0'
1 1000	23 11	22-1	2 71 P	21.2			2302	23'4"
14.90	24'5	22.6	21.5	21.6	26411	24-10	23.9	2137.94
2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	24 8	22'-9'	21.9	271.70		25-2	24'0"	24 1

CCMC EVALUATION REPORT 1 3032-R

I-JOIST HANGERS

- Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
- 2. All nailing must meet the hanger manufacturer's recommendations.
- . Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the 1-joist.





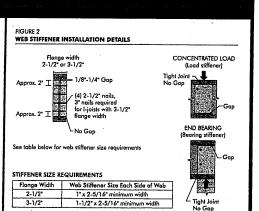
WEB STIFFENERS

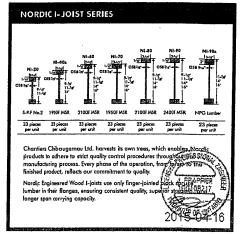
40.

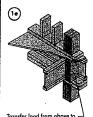
■ A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the Ljoist properties table found of the Ljoist Construction Guide (C101). The gop between the stiffener and the flonge is at the top.

- A bearing stiffener is required when A bearing similar is required when the l-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A load stiffener is required at location A Load shiftener is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flangs between supports, or in the case of a cantilever, anywhere between the confliever lip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitt by the code. The gap between the stiffener and the flange is at the bottom.

SI units conversion: 1 inch = 25.4 mm







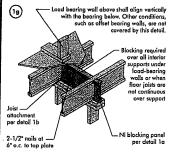
bearing below. Install squast blocks per detail 1d. Match bearing area of blocks below

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Use single I-joist for loads up to 3,300 plf, double I-joists for loads up to 6,600 plf (filler block no o 6,600 plf (tiller block not required). Attach I-joist to top plate using 2-1/2" nails Provide backer for siding attachm unless nailable Wall sheathing, as required ---

Rim board may be used in lieu of I-joists. Backer is not required when rim board is used. Bracing per code shall be carried to the foundation.

(III)

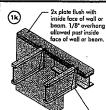


Nordic Lam or SCL (1k)

Top- or face-mount hanger installed per manufacturer

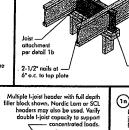
For nailing schedules for multiple beams, see the manufacturer's recommendations.

Note: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.



Top-mount hanger installed per . manufacturer's recommendations

Note: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.



Filler block per detail 1p -Install hanger per manufacturer's recommendations Backer block attached per detail 1h. Nail with twelve 3" nails, clinch when possible.

Maximum support capacity = 1,620 lbs.

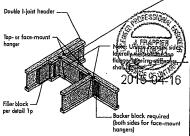
10 Do not bevel-cut joist beyond inside face of wall

l-joist per detail 1 b

Note: Blocking required at bearing for lateral support, not shown for clarity.

Backer block (use if hanger load exceeds 360 lbs)
Before installing a backer block to a double I-joist, drive three additional 3" nails through the wabs and filler block where the backer block will fill. Clinch. Install backer fills to top flongs.

**London 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

BACKER BLOCKS (Blocks must be long enough to permit required

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1,	5-1/2"
3-1/2*	1-1/2*	7-1/4"

- Minimum grade for backer block material shall be S-P.F. No. 2 or better for solid sown lumber and wood structural panels conforming to CAN/CSA-0325 or CAN/CSA-0437 Standard.

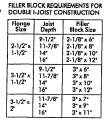
 For face-mount hangers use net joist depth minus 3-1/4" for joist with 1-1/2" thick flanges. For 2" thick flanges use riet depth minus 4-1/4".

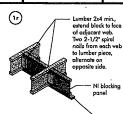
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Offset nails from apposite face by 6"

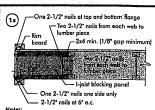
1/8" to 1/4" gap between top flange

- Support back of I-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
- Nail joists together with two rows of 3' nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
- The maximum factored load that may be applied to one side of the double joist using this detail is 860 lbt/ft. Verify double l-joist capacity.





Optional: Minimum 1x4 inch — strop applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists.



Notes:
In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) ned to the starter joist. Where required, see local code requiremen for spacing of the blocking.

All nails are common spiral in this detail.