

31-09-00

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	14
J2	14-00-00	9 1/2" NI-40x	1	32
J3	10-00-00	9 1/2" NI-40x	1	13
J4	8-00-00	9 1/2" NI-40x	1	17
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	6
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary					
Qty Manuf Product					
10	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
6	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			

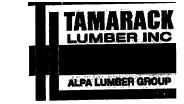
TOWN OF BRADFORD WEST GWILLIMBURY **BUILDING DEPARTMENT** PLANS EXAMINED

ONTARIO BUILDING CODE APPLIES

DATE: 2018-10-24

INSPECTOR: BG

SITE COPY



FROM PLAN DATED: JAN 2017

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: A,C

LOT:

CITY: BRADFORD

SALESMAN: M D **DESIGNER: CZ REVISION:**

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

LOADING:

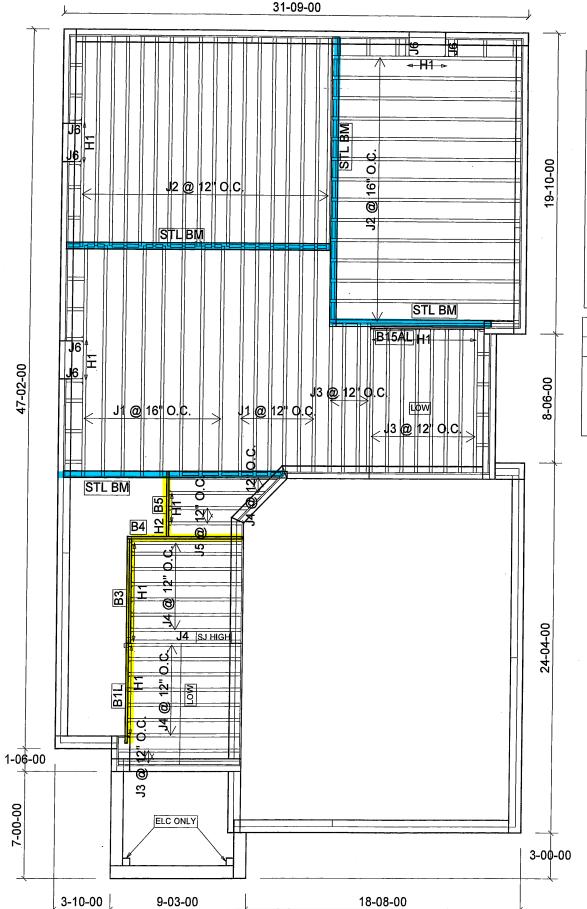
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

1st FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	14
J2	14-00-00	9 1/2" NI-40x	1	32
J3	10-00-00	9 1/2" NI-40x	1	14
J4	8-00-00	9 1/2" NI-40x	1	17
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	6
B15AL	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary					
Qty Manuf Product		Product			
18	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
6	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: A,C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6. LOADING:

DESIGN LOADS: L/480.000

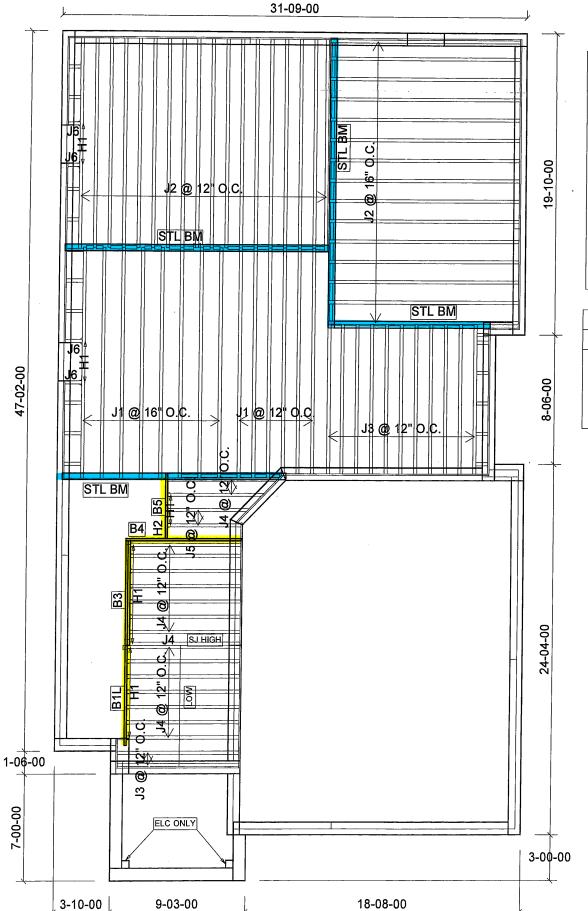
LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

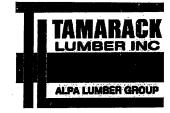
1st FLOOR

SUNKEN



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	14
J2	14-00-00	9 1/2" NI-40x	1	33
J3	10-00-00	9 1/2" NI-40x	1	13
J4	8-00-00	9 1/2" NI-40x	1	17
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	4
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

C	Connector Summary					
Qty Manuf Product		Product				
10	H1	IUS2.56/9.5				
8	H1	IUS2.56/9.5				
4	H1	IUS2.56/9.5				
1	H2	HUS1.81/10				



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: A,C

. . . .

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

LOT:

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6. LOADING:

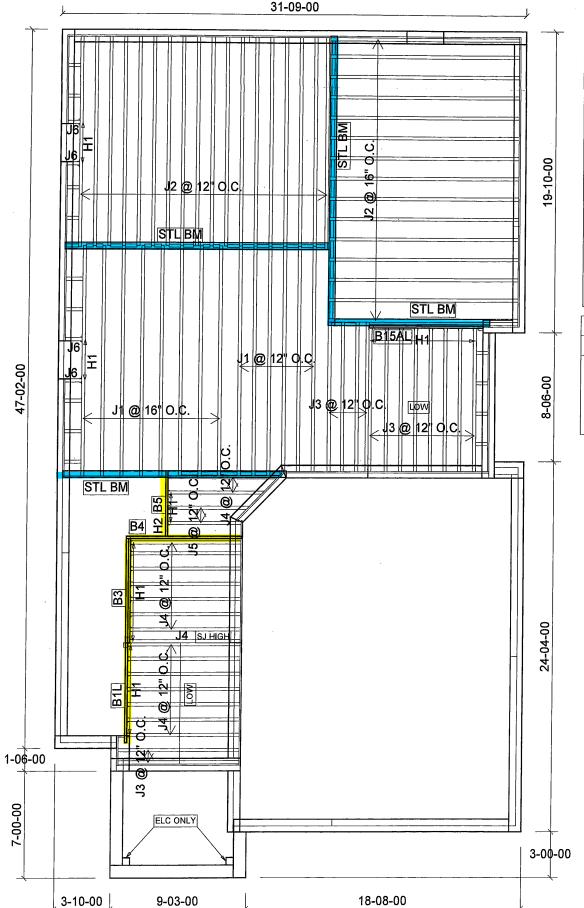
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

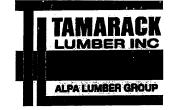
1st FLOOR

STANDARD WITH WOD & WOB



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	14
J2	14-00-00	9 1/2" NI-40x	1	33
J3	10-00-00	9 1/2" NI-40x	1	14
J4	8-00-00	9 1/2" NI-40x	1	17
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	4
B15AL	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary					
Qty	Manuf	Product			
18	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
4	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: A,C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6. LOADING:

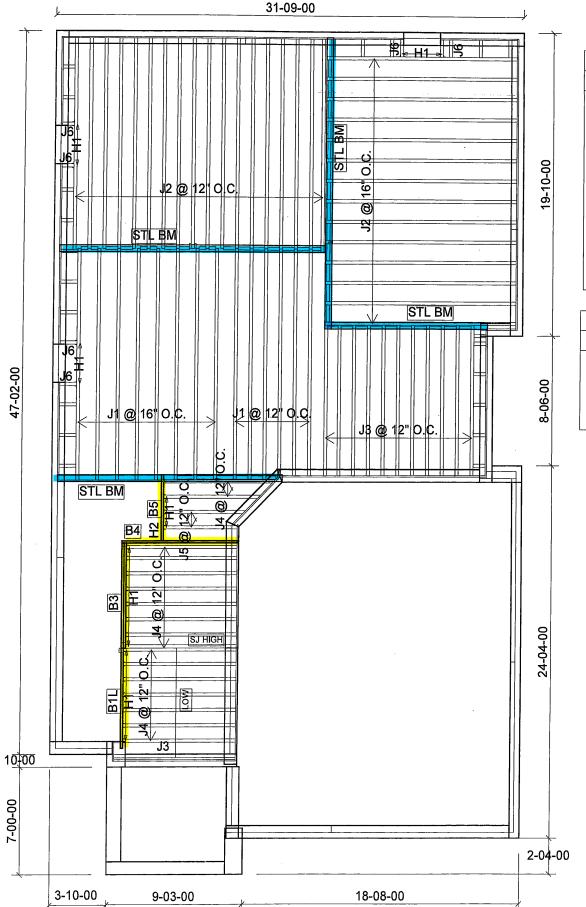
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

1st FLOOR

SUNKEN WITH WOD & WOB



		Products			
PlotID	Length	Product	Plie	S	Net Qty
J1	16-00-00	9 1/2" NI-40x	1		14
J2	14-00-00	9 1/2" NI-40x	1		32
J3	10-00-00	9 1/2" NI-40x	1		12
J4	8-00-00	9 1/2" NI-40x	1	€.	17
J5	6-00-00	9 1/2" NI-40x	1	{	2
J6	2-00-00	9 1/2" NI-40x	1		6
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1		1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1		1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2		2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1		1

Connector Summary					
Qty	Manuf	Product			
10	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
6	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: B

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

LOT:

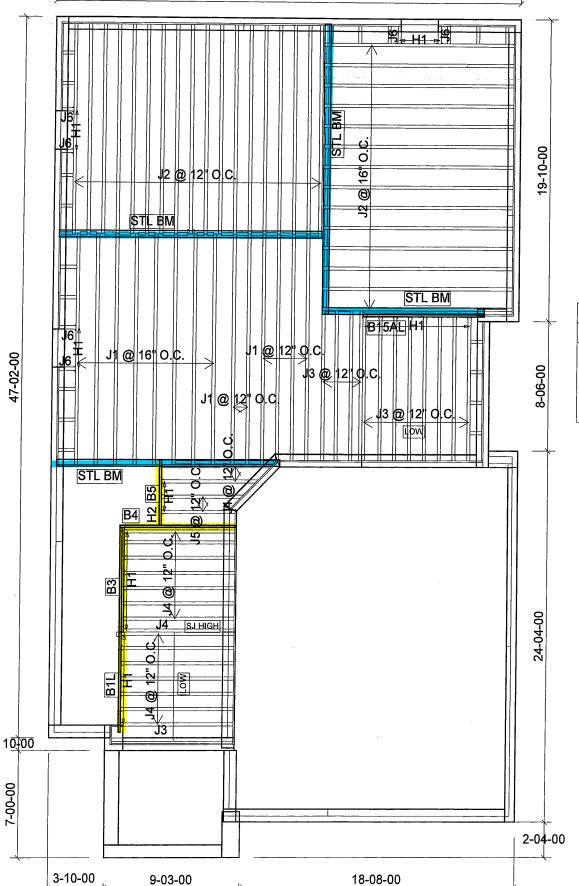
REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6. LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

1st FLOOR



31-09-00

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	16-00-00	9 1/2" NI-40x	1	14
J2	14-00-00	9 1/2" NI-40x	1	32
J3	10-00-00	9 1/2" NI-40x	1	13
J4	8-00-00	9 1/2" NI-40x	1	17
J5	6-00-00	9 1/2" NI-40x	1	2
J6	2-00-00	9 1/2" NI-40x	1	6
B15AL	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary					
Qty	Manuf	Product			
18	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
6	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			



FROM PLAN DATED: JAN 2017

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

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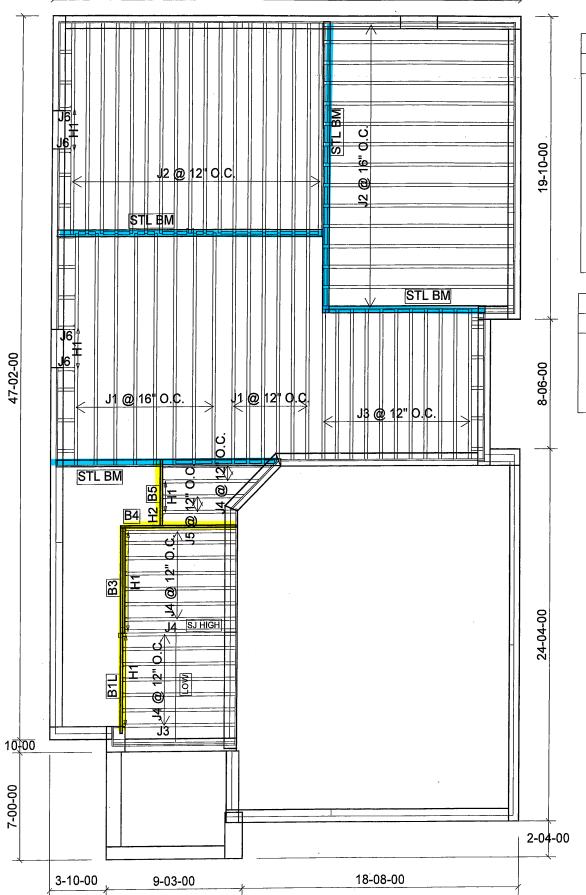
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

1st FLOOR

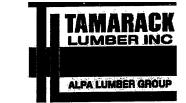
SUNKEN



31-09-00

Products							
PlotID	Length	Product	Plies	Net Qty			
J1	16-00-00	9 1/2" NI-40x	1 -	14			
J2	14-00-00	9 1/2" NI-40x	1	33			
J3	10-00-00	9 1/2" NI-40x	1	12			
J4	8-00-00	9 1/2" NI-40x	1	17			
J5	6-00-00	9 1/2" NI-40x	1	2			
J6	2-00-00	9 1/2" NI-40x	1	4			
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
В3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			

Connector Summary						
Qty Manuf Product						
10	H1	IUS2.56/9.5				
8	H1	IUS2.56/9.5				
4	H1	IUS2.56/9.5				
1	H2	HUS1.81/10				



FROM PLAN DATED: JAN 2017

BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

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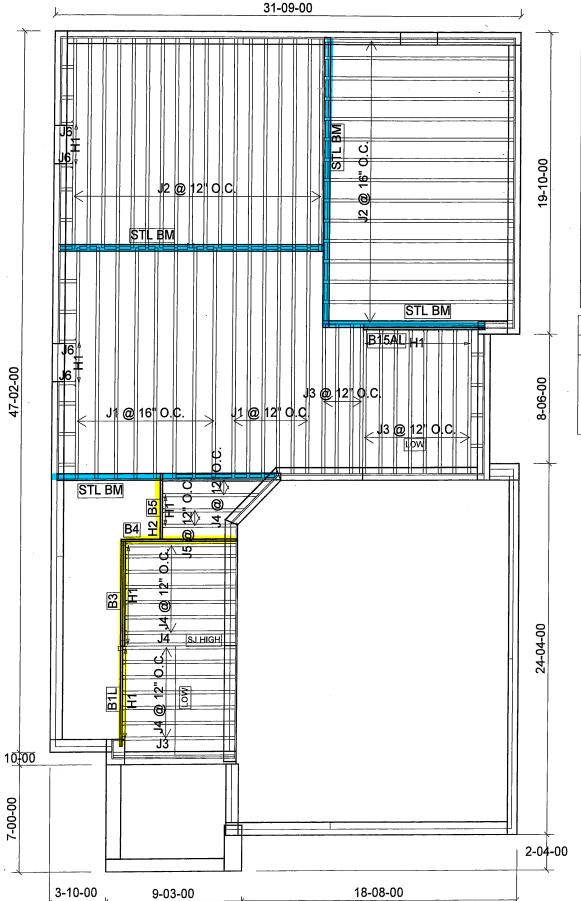
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

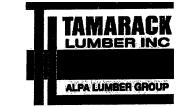
1st FLOOR

STANDARD WITH WOD & WOB



Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	-1	14		
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J5	6-00-00	9 1/2" NI-40x	1	2		
J6	2-00-00	9 1/2" NI-40x	1	4		
B15AL	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B1L	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B4	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B3	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B5	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		

Connector Summary					
Qty Manuf Product					
18	H1	IUS2.56/9.5			
8	H1	IUS2.56/9.5			
4	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

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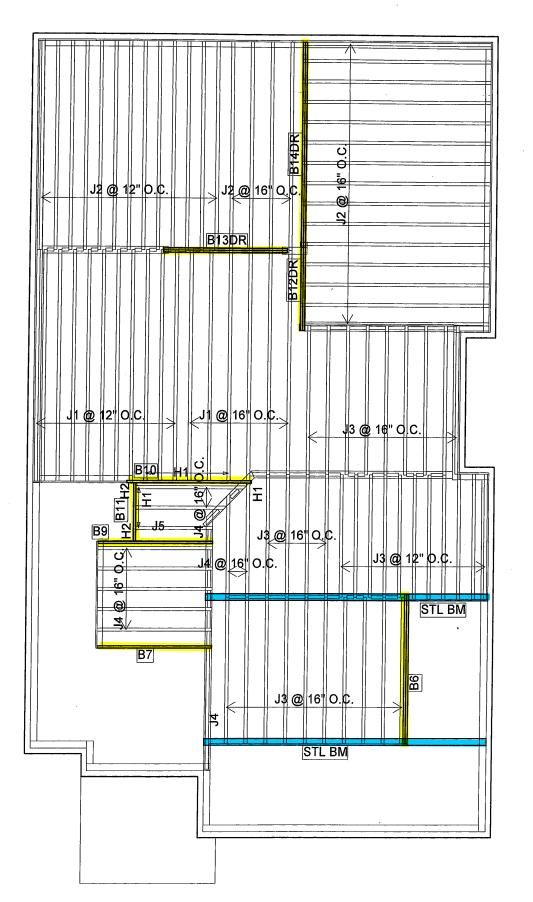
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

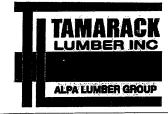
1st FLOOR

SUNKEN WITH WOD & WOB



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	16-00-00	9 1/2" NI-40x	1	17			
J2	14-00-00	9 1/2" NI-40x	1	32			
J3	10-00-00	9 1/2" NI-40x	1	34			
J4	8-00-00	9 1/2" NI-40x	1	10			
J5	6-00-00	9 1/2" NI-40x	1	1			
B14DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B10	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B13DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B6	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B7	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B9	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B12DR	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			

Connector Summary						
Qty Manuf Product						
10	H1	IUS2.56/9.5				
2	H2	HUS1.81/10				



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: A

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

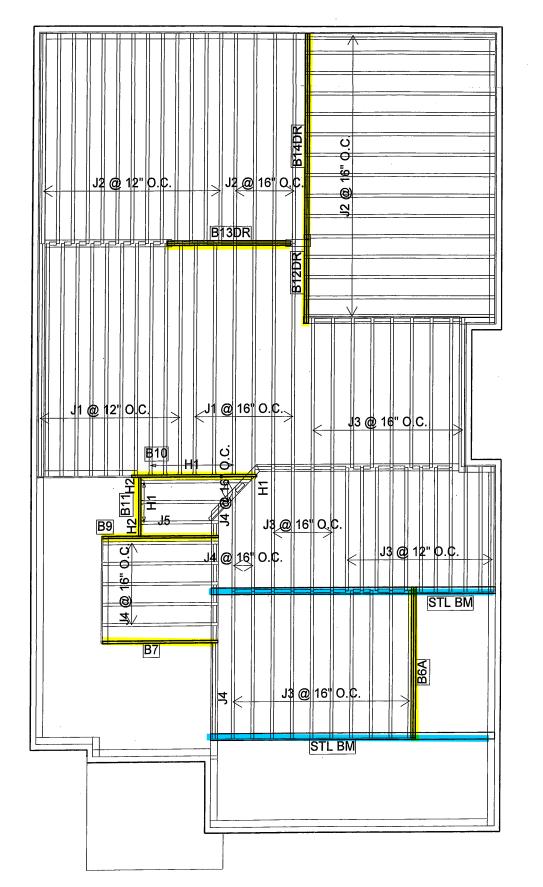
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

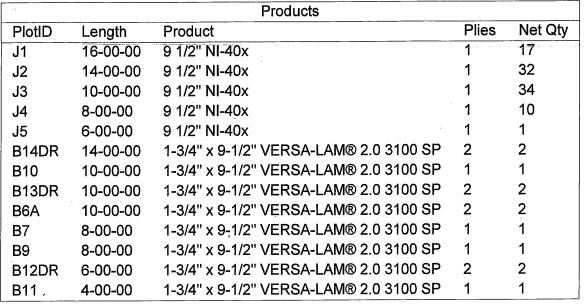
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

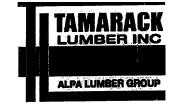
DATE: 16/02/2018

2nd FLOOR





Connector Summary						
Qty Manuf Product						
10	H1	IUS2.56/9.5				
2	H2	HUS1.81/10				



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: B

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

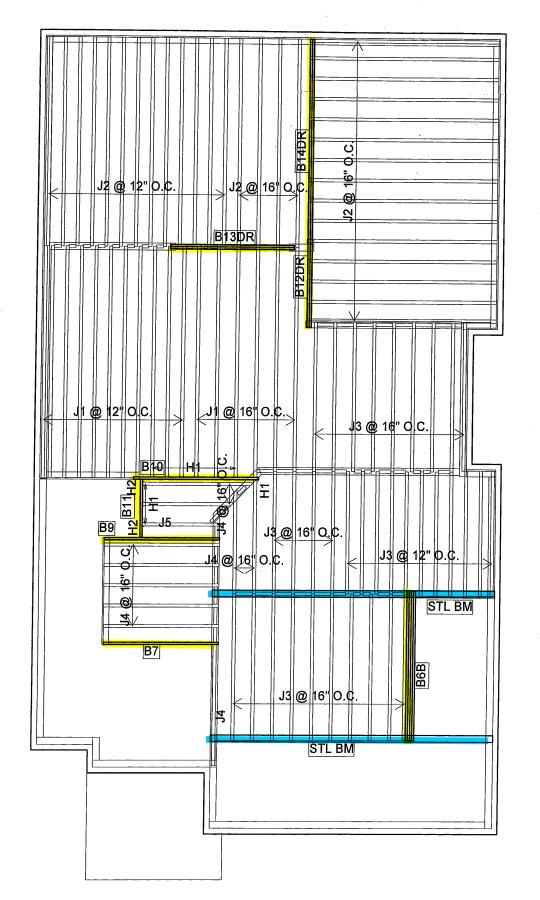
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE, CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

2nd FLOOR



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	16-00-00	9 1/2" NI-40x	1	17			
J2	14-00-00	9 1/2" NI-40x	1	32			
J3	10-00-00	9 1/2" NI-40x	1	34			
J4	8-00-00	9 1/2" NI-40x	1	10			
J5	6-00-00	9 1/2" NI-40x	1	1			
B14DR	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B10	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B13DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B6B	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	4	4			
B7	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	· 1	1			
B9	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B12DR	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			

Connector Summary							
Qty	Qty Manuf Product						
10	H1	IUS2.56/9.5					
2	H2	HUS1.81/10					



BUILDER: BAYVIEW WELLINGTON

SITE: GREEN VALLEY EAST

MODEL: S38-3 BAROSSA 3

ELEVATION: C

LOT:

CITY: BRADFORD

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 16/02/2018

2nd FLOOR



Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1L(i1298)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5 033

Job Name:

Addres s: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

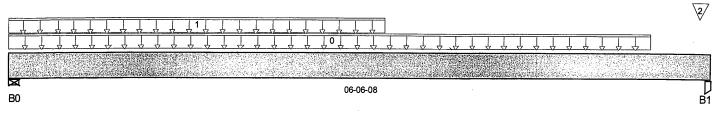
File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1L(i1298

Specifier:

Designer: Company.

Misc:



Total Horizontal Product Length = 06-06-08

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 5-1/2"	1,160 / 0	532/0						
B1, 3-1/2"	685/0	297/0						

	ad Summary Description	Load Type	Po	f. Start	End	Live	Dead	Snow		Trib.
ıa	g Description	Loau Type	Re	ı. Start	Ellu	1.00	0.65	1.00	1.15	
0	Sm oothed Load	Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	154	58			n/a
1	0	Unf. Lin. (lb/ft)	L	00-00-00	03-06-00	240	120			n/a
2	J4(i1017)	Conc. Pt. (lbs)	L	06-05-04	06-05-04	80	30			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,631 ft-lbs	12,704 ft-lbs	20.7%	1	02-10-08
End Shear	1,477 lbs	5,785 lbs	25.5%	1	01-03-00
Total Load Defl.	L/999 (0.045")	n/a	n/a	4	03-03-00
Live Load Defl.	L/999 (0.031")	n/a	n/a	5	03-03-00
Max Defl.	0.045"	n/a	n/a	4	03-03-00
Span / Depth	7.5	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 1-3/4"	2,406 lbs	22.1%	20.5%	Unspecified
B1	Post	3-1/2" x 1-3/4"	1,398 lbs	35.1%	18.7%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®,

VERSA-STITAND® VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. TAM 9675 -17 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1295)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name:

Address:
City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1295)

Specifier:

Designer: Company:

Misc:

12 B1

Total Horizontal Product Length = 07-09-14

Reaction Summary (Down / Uplift) (lbs)										
Be aring	Live	De ad	Sn ow	Wind						
B0, 3-1/2"	88 / 0	201/0								
B1, 4-3/8"	69 / 0	69 / 0 ′								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re f.	Start	En d	1.00	0.65	1.00	1.15	
0	FC1 Floor Material	Unf. Lin. (lb/ft)	L 0	0-00-00	07-09-14	18	7			n/a
1	0	Unf. Lin. (lb/ft)	L 0	0-00-00	02-07-08		60			n/a
2	7(i245)	Conc. Pt. (lbs)	L 0	0-02-12	00-02-12	20	23			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	256 ft-1bs	8,258 ft-lbs	3.1%	0	02-06-05
End Shear	141 ibs	3,761 lbs	3.7%	0	01-01-00
Total Load Defl.	L/999 (0.011")	n/a	n/a	4	03-08-06
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	03-10-11
Max Defl.	0.011"	n/a	n/a	4	03-08-06
Span / Depth	9.2	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	Demand	De man d/ Re s istance Support	De mand/ Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	281 lbs	10.9%	5.8%	Unspecified
B1	Wall/Plate	4-3/8" x 1-3/4"	189 lbs	5.8%	2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA, STRAND®, VERSA-STUD® are tradeligates of Boise, Cascade Wood



DWG NO. TAM 9676 -18
STRUCTURAL
COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1461)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2017 09:17:39

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3WOD.mmdl

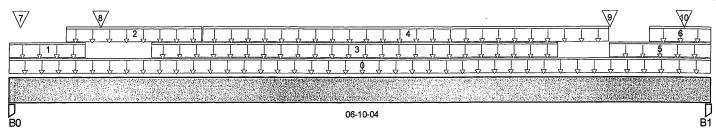
Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1461)

Specifier:

Designer: CZ

Company.

Misc:



Total Horizontal Product Length = 06-10-04

Reaction Summary (Down / Uplift) (Ibs)										
Be aring	Live	Dead	Snow	Wind						
B0, 4-1/2"	1,120/0	891/0								
B1. 2-3/4"	1,432 / 0	893/0								

١٨	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	7(i245)	Unf. Lin. (lb/ft)	L	00-00-00	06-10-04		81			n/a
1	7(i245)	Unf. Lin. (lb/ft)	L	00-00-00	00-08-14	139	77			n/a
2	7(i245)	Unf. Lin. (lb/ft)	L	00-06-08	01-10-08	150	56			n/a
3	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-08	05-04-08	156	59			n/a
4	7(i245)	Unf. Lin. (lb/ft)	L	01-10-08	05-10-08	162	61			n/a
5	7(i245)	Unf. Lin. (lb/ft)	L	05-10-08	06-10-04	128	48			n/a
6	7(i245)	Unf. Lin. (lb/ft)	L	06-03-02	06-10-04	703	352			n/a
7	J4(i1610)	Conc. Pt. (lbs)	L	00-01-04	00-01-04	68	149			n/a
8	J4(i1576)	Conc. Pt. (lbs)	L	00-10-08	00-10-08	147	66			n/a
9	J4(i1549)	Conc. Pt. (lbs)	L	05-10-08	05-10-08	134	50			n/a
10	J4(i1534)	Conc. Pt. (lbs)	L	06-07-01	06-07-01	86	32			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,798 ft-lbs	25,408 ft-lbs	14.9%	1	03-07-08
End Shear	2,575 lbs	11,571 lbs	22.3%	1	05-10-00
Total Load Defl.	L/999 (0.04")	n/a	n/a	4	03-06-00
Live Load Defl.	L/999 (0.024")	n/a	n/a	5	03-06-00
Max Defl.	0.04"	n/a	n/a	4	03-06-00
Span / Depth	8.1	n/a	n/a		00-00-00

Bearin	ng Supports	Dim . (L x W)	Demand	De man d/ Re s is tance Suppor t	Demand/ Resistance Member	Material
B0	Post	4-1/2" x 3-1/2"	2,7931bs	27.3%	14.5%	Unspecified
B1	Post	2-3/4" x 3-1/2"	3,2651bs	52.2%	27.8%	Unspecified

Notes





Pb 4



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1461)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 20, 2017 09:17:39

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3WOD.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i146

Specifier:

Designer: CZ

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

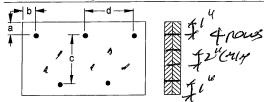
Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

A COS on bu

CONFORMS TO OBC 2012

Connection Diagram



Calculated Side Load = 329.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL

Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BC®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. TAM 9677 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1299)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

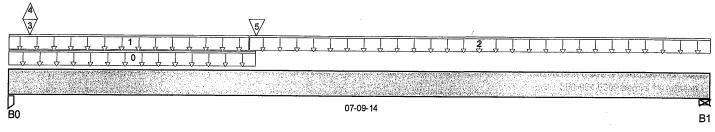
Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1299)

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 07-09-14

Reaction Summary (Down / Uplift) (lbs)										
Be aring	Live	De ad	Snow	Wind						
B0, 3-1/2"	598/1	451/0								
B1, 4-3/8"	313/0	186/0								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type		Ref. Start End		1.00	0.65	1.00	1.15	
0	0	Unf. Lin. (lb/ft)	L	00-00-00	02-08-14		60			n/a
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-08-00	8	3			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-08-00	07-09-14	26	10			n/a
3	7(i245)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	121	70			n/a
4	7(i245)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	-1	-			n/a
5	B5(i791)	Conc. Pt. (lbs)	L	02-08-14	02-08-14	635	307			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,627 ft-lbs	12,704 ft-lbs	20.7%	1	02-08-14
End Shear	1,086 lbs	5,785 lbs	18.8%	1	01-01-00
Total Load Defl.	L/999 (0.059")	n/a	n/a	6	03-07-03
Live Load Defl.	L/999 (0.036")	n/a	n/a	8	03-07-03
Max Defl.	0.059"	n/a	n/a	6	03-07-03
Span / Depth	9.2	n/a	n/a		00-00-00

Bearing Supports		Dim . (L x W)	De man d	De mand/ Resistance Support	Demand/ Resistance Member	Material	
B0	Post	3-1/2" x 1-3/4"	1,460 lbs	36.7%	19.5%	Unspecified	
B1	Wall/Plate	4-3/8" x 1-3/4"	702 lbs	21.5%	7.5%	Unspecified	

Notes



SITE COPY

DWG NO. TAM 9678 -19 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1299)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer: Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i129)

Specifier: Designer:

Company: Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9 **COMPONENT ONLY**

SITE COP'



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i791)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

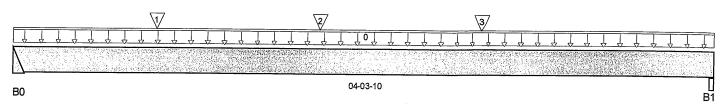
File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i791)

Specifier: Designer:

Company:

Misc:



Total Horizontal Product Length = 04-03-10

Reaction Summary (Down / Uplift) (lbs)										
Bearing	Live	De ad	Snow	Wind						
B0	653/0	316/0								
B1, 5-1/4"	711/0	348/0								

Load Summary				Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 0	Unf. Lin. (lb/ft)	L 00-00-00	04-03-10	240	120		n/a
1 J5(i1284)	Conc. Pt. (lbs)	L 00-10-08	00-10-08	97	36		n/a
2 J5(i1268)	Conc. Pt. (lbs)	L 01-10-08	01-10-08	106	42		n/a
3 J4(i1246)	Conc. Pt. (lbs)	L 02-10-08	02-10-08	124	47		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,338 ft-lbs	12,704 ft-lbs	10.5%	1	01-10-08
End Shear	860 lbs	5,785 lbs	14.9%	1	03-00-14
Total Load Defl.	L/999 (0.01")	n/a	n/a	4	02-00-06
Live Load Defl.	L/999 (0.007")	n/a	n/a	5	02-00-06
Max Defl.	0.01"	n/a	n/a	4	02-00-06
Span / Depth	4.8	n/a	n/a		00-00-00

				De mand/	Demand/		
Bearing Supports		Dim // x \\/\	Dim.(LxW) Demand		Resistance		
Deal	ing Supports	Dilli. (L X VV)	Demanu	Support	Member	Material	
B0	Hanger	2" x 1-3/4"	1,374 lbs	n/a	32.2%	HUS1.81/10	
B1	Beam	5-1/4" x 1-3/4"	1,501 lbs	38.2%	13.4%	Unspecified	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

SITE COPY



DWG NO. TAM 9629 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i791)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name: Address:

City, Province, Postal Code.BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i791

Specifier: Designer:

Company:

Misc:

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BCRIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. TAM 9609 STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i695)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

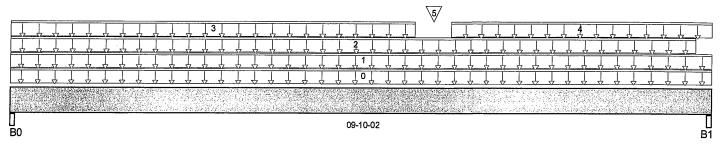
File Name: S38-3 BAROSSA 3.mmdi

Description: Designs\Flush Beams\1st Floor\Flush Beams\86(i695)

Specifier:

Designer: Company:

Misc:



Total Horizontal Product Length = 09-10-02

Reaction Summary (Down / Uplift) (lbs)										
Be aring `	Live	De ad	Snow	Wind						
B0, 4-1/8"	822/0	1,270 / 0								
B1, 5-1/4"	1,325 / 0	1,746 / 0								

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description *	Load Type	Re	f. Start	End	1.00	0.65	1.00	1.15	
0	LOWROOF	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	33	30		99	n/a
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	6				n/a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-07-08	5				n/a
3	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	05-07-14	33	130		99	n/a
4	Us er Load	Unf. Lin. (lb/ft)	L	06-01-14	09-10-02	143	230		429	n/a
5	Us er Load	Conc. Pt. (lbs)	L	05-10-14	05-10-14	1,001	1,003		3,002	n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	10,088 ft-lbs	25,408 ft-lbs	39.7%	1	05-10-14
End Shear	3,404 lbs	11,571 lbs	29.4%	1	08-07-06
Total Load Defl.	L/552 (0.2")	0.459"	43.5%	4	05-01-01
Live Load Defl.	L/999 (0.086")	n/a	n/a	5	05-01-01
Max Defl.	0.2"	1"	20%	4	05-01-01
Span / Depth	11.6	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	De man d	De mand/ Resistance Support	Resistance Member	Material
B0	Beam	4-1/8" x 3-1/2"	2,821 lbs	45.7%	16%	Unspecified
B1	Beam	5-1/4" x 3-1/2"	4,171 lbs	53.1%	18.6%	Unspecified

Notes



DWG NO. TAM 9680 -78
STRUCTURAL
COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i695)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:54

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B6(i695)

Specifier:

Designer: Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

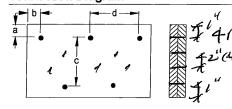
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012**

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = 2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d ·Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9680-18 STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6(i2072)

BC CALC® Design Report

Dry | 1 span | No cant.

February 16, 2018 10:07:54

Build 6215 Job name:

Address:

City, Province, Postal Code: BRA...RD

Customer: Code reports:

CCMC 12472-R

File name:

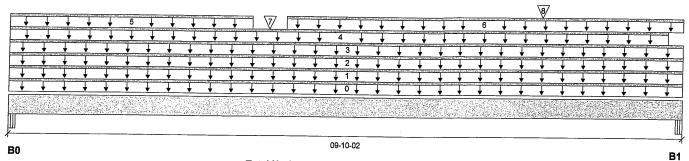
S38-3 BAROSSA 3.mmdl

Description: 1st Floor\Flush Beams\B6(i2072)

Specifier:

Designer: CZ

Company:



Total Horizontal Product Length = 09-10-02

Reaction Summary (Down / Upliff) (lbs)

		, c) (180)			
Bearing	Live	Dead	Snow	Wind	
B0, 4-1/8"	905 / 0	1,341 / 0	2,404 / 0		
B1, 5-1/4"	1,482 / 0	1,782 / 0	3,592 / 0		

Lo	ad Summary					Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	•
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02		10			00-00-00
1	LOW ROOF	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	33	30	99		n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02		100	00		n\a
3	FC3 Floor Material	Unf. Lin. (lb/ft)	Ĺ	00-00-00	09-10-02	6	100			n\a
4	FC3 Floor Material	Unf. Lin. (lb/ft)	Ī.	00-00-00	09-07-08	•				n\a
5	ROOF	Unf. Lin. (lb/ft)	ī	00-00-00		33	30	99		
6	ROOF	Unf. Lin. (lb/ft)	ī	04-00-06		110	100	330		n\a "\a
7	User Load	Conc. Pt. (lbs)	ī	03-09-06	03-09-06					n∖a
8	T12	` '				455	469	1,366		n∖a
9	114	Conc. Pt. (lbs)	L	07-09-09	07-09-09	743	552	1,388		n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	16,620 ft-lbs	23,220 ft-lbs	71.6 %	13	04-08-14
End Shear	7,095 lbs	11,571 lbs	61.3 %	13	08-07-06
Total Load Deflection	L/297 (0.371")	n\a	80.8 %	45	04-11-11
Live Load Deflection	L/421 (0.262")	n\a	85.5 %	61	04-11-11
Max Defl.	0.371"	n\a	37.1 %	45	04-11-11
Span / Depth	11.6				0, ,, ,,

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Beam	4-1/8" x 3-1/2"	5,735 lbs	74.4 %	32.6 %	Unspecified
B1	Beam	5-1/4" x 3-1/2"	8,356 lbs	85.2 %	37.3 %	Unspecified



COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6(i2072)

BC CALC® Design Report

Dry | 1 span | No cant.

February 16, 2018 10:07:54

Build 6215

Job name:

File name:

S38-3 BAROSSA 3.mmdl

Address:

City, Province, Postal Code: BRA...RD

Description:

1st Floor\Flush Beams\B6(i2072)

Specifier: Designer:

Customer: Code reports:

CZ

CCMC 12472-R

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

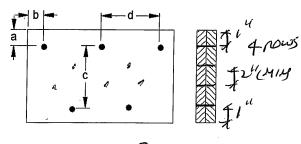
Importance Factor: Normal Part code: Part 9

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads,

please consult a technical representative or professional of Record.

Member has no side loads.

Connection Diagram



a minimum = 1" b minimum = 3" c = 3 - 1/2"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d

3-1/2" ARDOX SPIRAL

Disclosure

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DWG NO. TAM 9661.8 **STRUCTURAL** COMPONENT ONLY



SITE





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6A(i2256)

Dry | 1 span | No cant.

February 16, 2018 14:12:48

Build 6215

Job name: Address:

City, Province, Postal Code: BRA...RD

BC CALC® Design Report

File name:

S38-3 BAROSSA 3ELB.mmdi

Description: 1st Floor\Flush Beams\B6A(i2256)

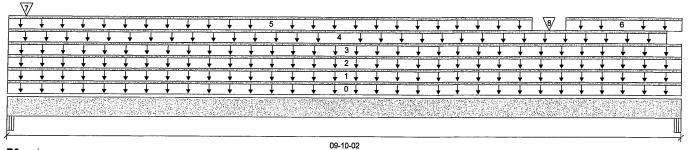
Specifier:

Designer: CZ

Customer: Code reports:

CCMC 12472-R

Company:



B0

Total Horizontal Product Length = 09-10-02

В1

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	
B0, 4-1/8"	854 / 0	1,309 / 0	2,404 / 0		
B1, 5-1/4"	1,044 / 0	1,524 / 0	2,967 / 0		

Loa	ad Summary					Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	Ĺ	00-00-00	09-10-02		10			00-00-00
1	LOW ROOF	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	33	30	99		n\a
2	wall	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02		100			n\a
3	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	6				n\a
4	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-07-08	5				n\a
5	roof	Unf. Lin. (lb/ft)	L	00-00-00	07-07-14	88	80	264		n\a
6	roof	Unf. Lin. (lb/ft)	L	08-01-14	09-10-02	88	80	264		n\a
7	MB22	Conc. Pt. (lbs)	L	00-03-00	00-03-00	132	134	396		n\a
8	t12z	Conc. Pt. (lbs)	L	07-10-14	07-10-14	511	536	1,533		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	11,842 ft-lbs	23,220 ft-lbs	51.0 %	13	05-05-00
End Shear	5,781 lbs	11,571 lbs	50.0 %	13	08-07-06
Total Load Deflection	L/412 (0.268")	n\a	58.3 %	45	05-00-01
Live Load Deflection	L/604 (0.182")	n\a	59.6 %	61	05-00-01
Max Defl.	0.268"	n\a	26.8 %	45	05-00-01
Span / Depth	11.6				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Beam	4-1/8" x 3-1/2"	5,669 lbs	73.5 %	32.2 %	Unspecified
B1	Beam	5-1/4" x 3-1/2"	6,876 lbs	70.1 %	30.7 %	Unspecified





DWG NO. TAM 9602-18 STRUCTURAL COMPONENT ONLY



BC CALC® Design Report

Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6A(i2256)

Dry | 1 span | No cant.

February 16, 2018 14:12:48

Build 6215

Job name: Address:

File name:

S38-3 BAROSSA 3ELB.mmdl

Description:

1st Floor\Flush Beams\B6A(i2256)

City, Province, Postal Code: BRA...RD

Specifier:

Designer: CZ

Customer: Code reports:

CCMC 12472-R

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

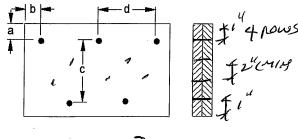
Importance Factor: Normal Part code: Part 9

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads,

please consult a technical representative or professional of Record.

Member has no side loads.

Connection Diagram



a minimum = 2" b minimum = 3"



Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL

Disclosure

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DWG NO. TAM 9682-18 STRUCTURAL COMPONENT ONLY

SITE C





Quadruple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6B(i2504)

BC CALC® Design Report

Build 6215

Dry | 1 span | No cant.

February 16, 2018 14:00:46

Job name:

Address:

City, Province, Postal Code: BRA...RD

Customer:

Code reports:

CCMC 12472-R

File name:

S38-3 BAROSSA 3 EL C-L2.mmdl

Description: 1st Floor\Flush Beams\B6B(i2504)

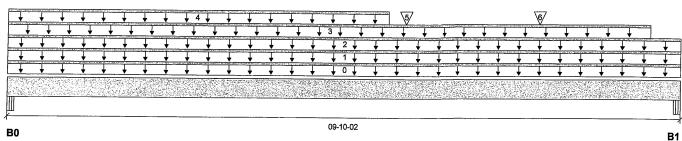
Specifier:

Designer:

CZ

Wind

Company:



B0

Total Horizontal Product Length = 09-10-02

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow B0, 4-1/8" 3,519 / 0 1,839 / 0 1,509 / 0 B1, 5-1/4" 5,681 / 0 2,567 / 0 2,079 / 0

Lo	ad Summary					Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	_
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02		19			00-00-00
1	LOW ROOF	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02	33	30	99		n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-00-00	09-10-02		100			n\a
3	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-04-14	12				n\a
4	ROOF	Unf. Lin. (lb/ft)	L.	00-00-00	05-06-09	33	30	99.		n\a
5	User Load	Conc. Pt. (lbs)	L	05-09-09	05-09-09/	7,264	2,126	1,152)	n\a
6	T12	Conc. Pt. (lbs)	L	07-09-09	07-09-09	1,240	574	912		n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	38,229 ft-lbs	48,297 ft-lbs	79.2 %	1	05-09-09
End Shear	12,382 lbs	23,142 lbs	53.5 %	1	08-07-06
Total Load Deflection	L/315 (0.349")	n\a	76.1 %	35	05-01-09
Live Load Deflection	L/429 (0.257")	n\a	83.9 %	51	05-01-09
Max Defl.	0.349"	n\a	34.9 %	35	05-01-09
Span / Depth	11.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Beam	4-1/8" x 7"	8,331 lbs	54.0 %	23.6 %	Unspecified
B1	Beam	5-1/4" x 7"	12,770 lbs	65.1 %	28.5 %	Unspecified









Quadruple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B6B(i2504)

BC CALC® Design Report

Build 6215

Dry | 1 span | No cant.

February 16, 2018 14:00:46

Job name: Address:

File name:

S38-3 BAROSSA 3 EL C-L2.mmdl

City, Province, Postal Code: BRA...RD

Description:

1st Floor\Flush Beams\B6B(i2504)

Customer:

Specifier:

Code reports:

CCMC 12472-R

Designer: CZ Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

Unbalanced snow loads determined from building geometry were used in selected product's verification. **CONFORMS TO OBC 2012**

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads,

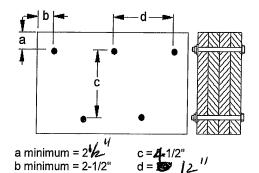
please consult a technical representative or professional of Record.

Beams 7 inches wide will be assumed to be either top-loaded only, or equally loaded from each side.

Bolts are assumed to be Grade A307 or Grade 2 or higher.

Member has no side loads.

Connection Diagram



Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Beams 7 inches wide will be assumed to be either top-loaded only, or equally loaded from each side. Bolts are assumed to be Grade A307 or Grade 2 or higher.

Member has no side loads.

Connectors are: 1/2 in. Staggered Through Bolt

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable purious Guide or ask and applicable purious of the control o Suide and applicable building codes. To ້ຖືບອູ່stions, please call (800)232-0788 before installation.

ALLOS, BU FRAMER® , AJS™,
OALLJOIST® , BC RIM BOARD™, BCI® ,
BOISE GLULAM™ BC Flood (*) VERSA-LAM®, VERSA-RIM PLUS®, POR

DWG NO. TAM 968 3-18 STRUCTURAL COMPONENT ONLY





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i964)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i964)

Specifier: Designer:

Company.

Misc:

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						lan er					i de la company La company La company														
⊠										07-0	9-14												***		. E ≥ B′

Total Horizontal Product Length = 07-09-14

Reaction Summary (Down / Uplift) (lbs)											
Bearing	Live	De ad	Snow	Wind							
B0, 4-3/8"	99 / 0	56 / 0									
B1, 5-1/2"	96 / 0	55 / 0									

Load Summary						Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-07-04	26	10			n/a

	Factored	Factored	Demand /	Load	Location	
Controls Summary	Demand	Resistance	Resistance	Case		
Pos. Moment	358 ft-1bs	12,704 ft-lbs	2.8%	1	03-10-06	
End Shear	153 lbs	5,785 lbs	2.6%	1	01-01-14	
Total Load Defl.	L/999 (0.009")	n/a	n/a	4	03-10-06	
Live Load Defl.	L/999 (0.006")	n/a	n/a	5	03-10-06	
Max Defl.	0.009"	n/a	n/a	4	03-10-06	
Span / Depth	9	n/a	n/a		00-00-00	

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	218 lbs	6.7%	2.3%	Unspecified
B1	Wall/Plate	5-1/2" x 1-3/4"	212 lbs	5.2%	1.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i971)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

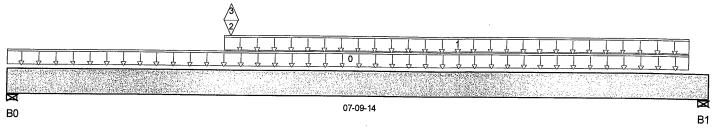
Description: Designs\Flush Beams\1st Floor\Flush Beams\B9(i971)

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 07-09-14

Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 4-3/8"	511/1	256/0						
B1, 5-1/2"	292/1	149/0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-07-04	9	3			n/a
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	02-04-14	07-07-04	19	7			n/a
2	B11(i667)	Conc. Pt. (lbs)	L	02-05-12			302			n/a
3	B11(i667)	Conc. Pt. (lbs)	L	02-05-12			002			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	2,277 ft-lbs	12,704 ft-lbs	17.9%	1	02-05-12	
End Shear	1,053 lbs	5,785 lbs	18.2%	1	01-01-14	
Total Load Defl.	L/999 (0.048")	n/a	n/a	6	03-06-13	
Live Load Defl.	L/999 (0.032")	n/a	n/a	8	03-06-13	
Max Defl.	0.048"	n/a	n/a	6	03-06-13	
Span / Depth	9	n/a	n/a		00-00-00	

Beari	ng Supports	Dim . (L x W)	Demand	De mand/ Resistance Support	Demand/ Resistance Member	· Material
B0	Wall/Plate	4-3/8" x 1-3/4"	1,087 lbs	33.2%	11.6%	Unspecified
B1	Wall/Plate	5-1/2" x 1-3/4"	624 lbs	15.2%	5.3%	Unspecified

Notes

SITE CC

DWG NO. TAM 9685-18 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i971)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B9(i971)

Specifier: Designer:

Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9605._K STRUCTURAL COMPONENT ONLY





Boise Cascado Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B10(i1057)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15;17:55

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

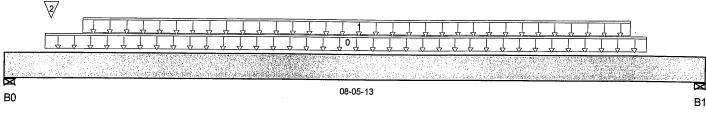
File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1057)

Specifier:

Designer: Company:

Misc:



Total Horizontal Product Length = 08-05-13

Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind					
B0, 4-1/2"	1,658 / 0	706/0							
B1, 7-3/4"	1,107/0	438/0							

	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-05-12	07-09-08	7	3			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-04	07-07-04	304	114			n/a
2	B11(i667)	Conc. Pt. (lbs)	L	00-06-10	00-06-10	684	323			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4,674 ft-lbs	12,704 ft-lbs	36.8%	1	04-03-04
End Shear	2,249 lbs	5,785 lbs	38.9%	1	01-02-00
Total Load Defl.	L/679 (0.134")	0.379"	35.4%	6	04-01-04
Live Load Defl.	L/999 (0.096")	n/a	n/a	8	04-01-04
Max Defl.	0.134"	1"	13.4%	6	04-01-04
Span / Depth	9.6	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	Demand	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	4-1/2" x 1-3/4"	3,369 lbs	99.8%	35%	Unspecified
B1	Wall/Plate	7-3/4" x 1-3/4"	2,207 lbs	38%	13.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM STRUCTURAL COMPONENT ONLY

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CONFORMS TO OBC 2012



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i667)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

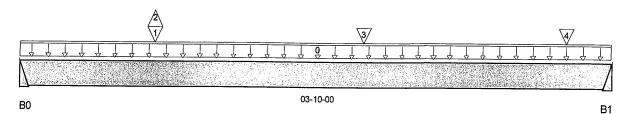
Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i667)

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 03-10-00

Reaction Summary (Do	Reaction Summary (Down / Uplift) (lbs)									
Be aring	Live	De ad	Snow	Wind						
В0	635/2	305/0								
B1	679/0	321/0								

Load Summary			L	Live	Dead	Snow	Wind 1.15	Trib.
Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00		
0 0	Unf. Lin. (lb/ft)	L 00-00-0	0 03-10-00	240	120			n/a
1 J5(i1102)	Conc. Pt. (lbs)	L 00-10-0	8 00-10-08	118	44			n/a
2 J5(i1102)	Conc. Pt. (lbs)	L 00-10-0	8 00-10-08	-2				n/a
3 J4(i1238)	Conc. Pt. (lbs)	L 02-02-0	8 02-02-08	185	69			n/a
4 J4(i1100)	Conc. Pt. (lbs)	L 03-06-0			34			n/a

Controls Summary	Factored F crols Summary Demand F		Demand / Resistance	Load Case	Location
Pos. Moment	1,243 ft-lbs	12,704 ft-lbs	9.8%	1	02-01-09
End Shear	815 lbs	5,785 lbs	14.1%	1	00-11-08
Total Load Defl.	L/999 (0.008")	n/a	n/a	6	01-11-03
Live Load Defl.	L/999 (0.006")	n/a	n/a	8	01-11-03
Max Defl.	0.008"	n/a	n/a	6	01-11-03
Span / Depth	4.6	n/a	n/a		00-00-00

Beari	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Hanger	2" x 1-3/4"	1,334 lbs	n/a	31.2%	HUS1.81/10
B1	Hanger	2" x 1-3/4"	1,419 lbs	n/a	33.2%	HUS1.81/10

Notes



DWG NO. TAM 9687.78
STRUCTURAL
COMPONENT ONLY





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i667)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer: Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i66;

Specifier: Designer: Company:

CONFORMS TO OBC 2012

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 960 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12DR(i1066)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer: Code reports:

le reports: CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B12I

Specifier: Designer: Company: Misc:

V	1 /	2	3	4
X				4 (4.0)
B0		05-05-10		B1

Total Horizontal	l Product	Length	= 05-05-10
------------------	-----------	--------	------------

Reaction Summary (D	own / Uplift) (lbs)			
Be aring	Live	De ad	Snow	Wind
B0, 5-1/8"	658/0	272/0		
B1, 5-1/2"	2,473 / 0	1,023/0		

n/a
n/a
n/a
n/a
n/a
_

Controls Summary	Factored Demand			Load Case	Location
Pos. Moment	1,575 ft-lbs	25,408 ft-lbs	6.2%	1	03-00-02
End Shear	1,169 lbs	11,571 lbs	10.1%	1	04-02-10
Total Load Defl.	L/999 (0.008")	n/a	n/a	4	02-09-03
Live Load Defl.	L/999 (0.006")	n/a	n/a	5	02-09-03
Max Defl.	0.008"	n/a	n/a	4	02-09-03
Span / Depth	5.9	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim. (L x W)	De man d	Support	Member	Material
B0	Wall/Plate	5-1/8" x 3-1/2"	1,327 lbs	11.4%	6.1%	Unspedified
B1	Wall/Plate	5-1/2" x 3-1/2"	4,9891bs	39.9%	21.2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 00-06-12, Bottom: 00-06-12.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012



DWG NO. TAM 9606.73 STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12DR(i1066)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer: Code reports:

CCMC 12472-R

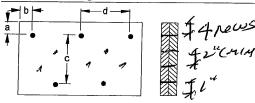
File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier: Designer:

Company: Misc:

Connection Diagram



a minimum = 2"

b minimum = 3"

·Nails

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 968. STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13DR(i652)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

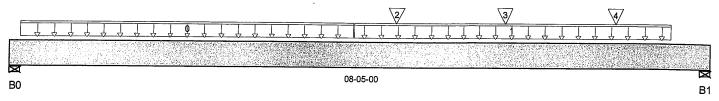
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B13[

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 08-05-00

Reaction Summary (Down / Uplift) (lbs)									
Be aring `	Live	De ad	Snow	Wind					
B0, 4"	2,389/0	937/0							
B1, 4"	2,124/0	838/0							

Lo	oad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re f	. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-01-08	04-01-08	573	215			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	04-01-08	07-11-08	310	116			n/a
2	J2(i1135)	Conc. Pt. (lbs)	L	04-07-08	04-07-08	314	118			n/a
3	J2(i1273)	Conc. Pt. (lbs)	L	05-11-08	05-11-08	359	135			n/a
4	J2(i1133)	Conc. Pt. (lbs)	L	07-03-08	07-03-08	359	135			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	8,902 ft-lbs	25,408 ft-lbs	35%	1	04-07-08	
End Shear	4,219 lbs	11,571 lbs	36.5%	1	07-03-08	
Total Load Defl.	L/683 (0.138")	0.394"	35.1%	4	04-03-00	
Live Load Defl.	L/999 (0.099")	n/a	n/a	5	04-03-00	
Max Defl.	0.138"	1"	13.8%	4	04-03-00	
Span / Depth	9.9	n/a	n/a		00-00-00	

Bear	ing Supports	Dim.(L x W)	Demand	De mand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	4" x 3-1/2"	4,756 lbs	52.3%	27.8%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	4,233 lbs	46.5%	24.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 00-04-02, Bottom: 00-04-02.

Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012





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Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13DR(i652)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 12, 2017 15:17:55

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

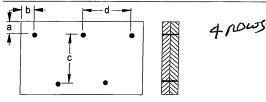
File Name: S38-3 BAROSSA 3.mmdl

Description: Designs \Dropped Beams \1st Floor \Dropped Beams \B1

Specifier: Designer: Company.

Misc:

Connection Diagram



a minimum = 2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d \ Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM Structúral COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14DR(i1700)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 13, 2017 10:38:38

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B14[

Specifier:

Designer: Company.

Misc:

$\overline{\mathbb{V}}$	2/
B0 13-05-06	B1

Total Horizontal Product Length = 13-05-06

Reaction Summary (D	Down / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 4"	1,846 / 0	756/0			
B1, 4-3/8"	1,623 / 0	673/0			

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
Ta	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-10-08	11-06-08	251	94			n/a
1	J2(i1540)	Conc. Pt. (lbs)	L	00-02-08	00-02-08	281	106			n/a
2	J2(i1500)	Conc. Pt. (lbs)	L	12-02-08	12-02-08	306	115			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	11,122 ft-lbs	25,408 ft-lbs	43.8%	1	06-10-08
End Shear	3,252 lbs	11,571 lbs	28.1%	1	12-03-08
Total Load Defl.	L/335 (0.462")	0.644"	71.7%	4	06-09-04
Live Load Defl.	L/472 (0.327")	0.429"	76.3%	5	06-09-04
Max Defl.	0.462"	1"	46.2%	4	06-09-04
Span / Depth	16.3	n/a	n/a		00-00-00

Rear	ring Supports	Dim . (L x W)	Demand	De mand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	4" x 3-1/2"	3,714 lbs	32.7%	21.7%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	3,275 lbs	26.3%	17.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 00-01-15, Bottom: 00-01-15.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9



STRUCTURAL COMPONENT ONLY



SITE COPY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14DR(i1700)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 13, 2017 10:38:38

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

CCMC 12472-R Code reports:

File Name: S38-3 BAROSSA 3.mmdl

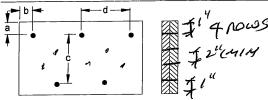
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier:

Designer:

Company. Misc:

Connection Diagram



a minimum = #" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d 7000. Nails

3-1/2" ARDOX SPIRAL

Disclosure

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SITE COPY

DWG NO. TAM STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B15AL(i2100)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 13, 2017 10:47:56

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3ELB-SUNKEN.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B15AL(i21

Specifier:

Designer: CZ Company:

Misc:

2/	3/		5
V	V		
Ј во		08-02-08	⊠ B1

Total Horizontal Product Length = 08-02-08

Reaction Summary (Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 3-1/2"	760/0	400/0			
B1, 5-1/2"	711/0	400/0			

10	ad Summary					Live	Dead	Snow	wina	i rib.
	g Description	Load Type	Ret	f. Start	En d	1.00	0.65	1.00	1.15	<u>.</u>
ō	Smoothed Load	Unf. Lin. (lb/ft)	L	01-08-08	06-08-08	189	95			n/a
1	FC4 Floor Material	Unf. Lin. (lb/ft)	L	07-02-08	08-02-08	18	9			n/a
2	J3(i1946)	Conc. Pt. (lbs)	L	00-01-04	00-01-04	114	57			n/a
3	J3(i1949)	Conc. Pt. (lbs)	L	01-02-08	01-02-08	209	10 4			n/a
4	J3(i2130)	Conc. Pt. (lbs)	L	07-02-08	07-02-08	180	90			n/a
5	E6(i220)	Conc. Pt. (lbs)	L	07-11-12	07-11-12		26			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,971 ft-lbs	12,704 ft-lbs	23.4%	1	04-02-08
End Shear	1,391 lbs	5,785 lbs	24%	1	01-01-00
Total Load Defl.	L/999 (0.086")	n/a	n/a	4	04-00-04
Live Load Defl.	L/999 (0.056")	n/a	n/a	5	04-00-04
Max Defl.	0.086"	n/a	n/a	4	04-00-04
Span / Depth	9.6	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(LxW)	De man d	Resistance Support	Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	1,640 lbs	33%	21.9%	Un specified
B1	Wall/Plate	5-1/2" x 1-3/4"	1,567 lbs	11.5%	13.3%	Un specified

Notes



DWG NO. TAM 9691-18 STRUCTURAL COMPONENT ONLY





Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B15AL(i2100)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

September 13, 2017 10:47:56

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: S38-3 BAROSSA 3ELB-SUNKEN.mmdl

Description: Designs \Flush Beams \Basment\Flush Beams \B15AL(i:

Specifier:

Designer: Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum total load deflection criteria.

Design meets User specified (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012**

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Di sclosure

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DWG NO. TAM 967/ **STRUCTURAL** COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







		Bare					1/2" Gypsum Ceiling				
Depth	Series		On Cen	tre Spacing				re Spacing	-		
	·	12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3 "	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11 7/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18' - 3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20' - 5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spa	n Blocking	Mid-Span Blocking and 1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing				re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19' - 5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17' - 8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
11-7/8"	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	N!-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
4"	NI-70	25' - 3"	23'-4"	22 '- 3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25 '- 7"	23' - 8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23' - 9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
.6"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
.0	NI-80	28' - 2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26' - 2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

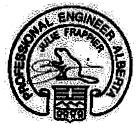
^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			E	are			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing	
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
•	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18' - 2"	17'-5"	16'-9"	20'-2"	18' - 9"	17'-11"	17'-2"
11-7/6	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19' - 9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23' - 0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23' - 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21' - 9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'- 5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22' - 1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/0	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22 '- 3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22' - 9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23 '- 8"	22' - 4"	20'-10"
14"	NI-70	26'-1"	24' - 3"	23' - 2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26' - 6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22' - 9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23 '- 4"
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28' - 8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
10	NI-80 .	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28' - 5"	27'-2"	25'-8"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9 -1/ 2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15' - 5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'- 5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/6	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18' - 9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20' - 3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22' - 3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	Ni-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15' - 7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-//0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22' - 5"	21'-0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22 '- 9"	N/A
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23' - 9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25' - 8"	24'-6"	N/A	28'-5"	26'-5"	25' - 2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'~5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

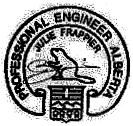
^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







				Bare		.1 _	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10 "	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/8	N!-70	20' - 9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19' - 9"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25 '- 1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
11-7/8"	NI-60	21'-9"	19 '- 8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/0	NI-70	23' - 4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20' - 5"	18'-11"	24'-1"	21'-10"	20'-5"	18' - 11"
	NI-90x	24'-3"	22'-6"	21'-3"	19' - 7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22' - 5"	21'-0"	19'-6"	24'-9"	22' - 5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28' - 8"	26 '- 8"	25'-3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
10	NI-80	29'-1"	27'-0"	25 '- 9"	23'-10"	29' - 8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

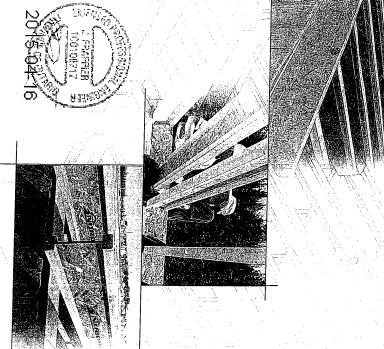
^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

NSTALLATION GUIDE ENGINEERED WOOD

FOR RESIDENTIAL FLOORS



Distributed by:



SAFETY AND CONSTRUCTION PRECAUTIONS

N-C301 / November 2014



until fully fastened and Do not walk on 1-joists

braced, or serious injuries can result.



over-stress I-joist with concentrated loads from Once sheathed, do not materials over unsheathed I-joists. building materials. Never stack building

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

Avoid Accidents by Following these Important Guidelines:

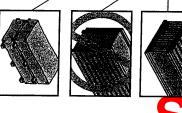
Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When Lipists are applied continuous over interior supports and a load-bearing wall is planned at that location,

- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the Lioists. Until this sheathing is applied, to prevent 1-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied blocking will be required at the interior support.
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Na bracing over at least two I-joists.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of Ljoists at the end of the bay.
- 3. For cantilevered 1-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each Lipist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

can result in serious accidents. Follow these installation guidelines carefully, Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic Lioists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required

STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- Store, stack, and handle I-joists vertically and level only.
- 3. Always stack and handle Ljoists in the upright position only.
- Do not store I-joists in direct contact with the ground and/or flatwise
- Protect I-joists from weather, and use spacers to separate bundles. Bundled units should be kept intact until time of installation.
- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- Pick I-joists in bundles as shipped by the supplier
- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5th points, using a spreader bar if necessary.
- 8. Do not handle Lioists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST





MAXIMUM FLOOR SPANS

- 1. Maximum clear spans applicable to simple-span or or more of the adjacent span. For multiple-span applications, the end spans shall be 40% 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. live load of 40 psf and dead load of 15 psf. The ultimate multiple-span residential floor construction with a design limit states are based on the factored loads of 1.50L +
- of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive Spans are based on a composite floor with glued-nailed assumed. Increased spans may be achieved with the used shall meet the requirements given in CGBS-71.26
- 3. Minimum bearing length shall be 1-3/4 inches for the enc bearings, and 3-1/2 inches for the intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI units conversion: 1 inch = 25.4 mm

MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

Joist	Joist		Simple	spans			Multip	e spans
Depth	Series		On centr	e spacing			On centr	e spacing
A second and the second and the second	Tark (Tark Tark)	12"	16"	19.2	24°	12"	16 ⁿ	19.2"
		15:31		13:9	19.5	E.91	115,48	POISTER.
		16.0	15.4	14-10"			16-55	15-10
			1101	15'-6"	5.57		107	10 -U
		18 17 3 1 18	9 H & S	15'-8" W	15:0			
		36510	1000	1515	15:6	181	7.3	18.78
				16-5	16-6	720 O	8.65	17.9
				16-7	16.9	20:31	18-9	18-0
					1	216	397118	19-0
			27.0				20-2	19:35
		2014	1819	17-11	18:0	22.5	20.9	9.50
			0.0	17-10	4 1/Z-10	20.2	20-6	19.8
			9010	10.1			7.00	20.0
		2001	201-3	19.4	1916	2	22.5	515
			2011	19-9			22-10	21,10
		* O'CC	2018	19:9	201F61F	2477	20.9	22.00
		200	210	20-9	20-101	26.0	24-01	22.11
		74.5	222	21-1	21.2	26,5	24-5	23.3
		24.8	22.0	21.0 21.0	人にも	76-11	24.10	2319

CCMC EVALUATION REPORT 13032-R

I-JOIST HANGERS

- Hangers shown illustrate the three to support 1-joists. most commonly used metal hangers
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based maximum spans. and load capacity based on the on the joist depth, flange width
- 4. Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.





Top Moun



race Mount

WEB STIFFENERS

RECOMMENDATIONS:

- Construction Guide (C101).The gap between the stiffener and the flange is at the top. A bearing stiffener is required in all engineered applications with factored -joist properties table found of the I-joist reactions greater than shown in the
- A bearing stiffener is required when stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and the I-joist is supported in a hanger and the
- by the code. The gap between the stiffener adjusted for other load durations as permitted ■ A load stiffener is required at locations and the flange is at the bottom. standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever between supports, or in the case of a than 2,370 lbs is applied to the top flange where a factored concentrated load greater

SI units conversion: 1 inch = 25.4 mm

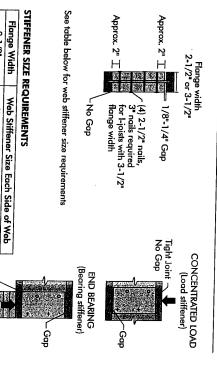
3-1/2" 2-1/2"

1-1/2" x 2-5/16" minimum width 1" x 2-5/16" minimum width

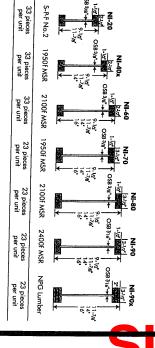
Tight Join

FIGURE 2

WEB STIFFENER INSTALLATION DETAILS



NORDIC I-JOIST SERIES



products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables Narylic finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from forest to the

longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior streலங்கள்ள Nordic Engineered Wood I-joists use only finger-jointed back spruce

2015-04-16

INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, coping you
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.

- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அன்
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 5
- When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Ljoists where they will be permanently exposed to weather, or where they will remain in direct contact with
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or Ljoist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products such as rim board must be cut to fit between the I-joists, and an
- 13. Provide permanent lateral support of the bottom flange of all Lipists at interior supports of multiple-span joists. Similarly, structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary support the bottom flange of all cantilevered Ljoists at the end support next to the cantilever extension. In the completed
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed. minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or

(1

panel NI blocking

€

One 2-1/2" wire or spiral

plate using 2-1/2" wire or

Attach rim board to top

One 2-1/2" face nail board nail at top and avoid splitting of bearing plate. shall be 1-3/4" for the end may be driven at an angle to bearings, and 3-1/2" for Minimum bearing length from end of I-joist. Nails To avoid splitting flange, spiral toe-nails at 6" o.c.

It shall not be used in the design of a bonding auration.	inches or less and is based on standard to a joist depth of 16	*The uniform work - 11 1 1 2 3,300	or Rim Joist Maximum Factored Uniform Vertical Load* (pH)
	1-1/8" Rim Board Plus		
The uniform vertical load is limited to a rim board 1 11.	8,090	Maximum Factored Uniform Vertical Load (plf)	when applicable.

at each side at bearing

the intermediate bearings

such as joist, header, or rafter. For concentrated vertical It shall not be used in the design of a bending member, used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d. or less and is based on standard term load duration. It shall not be *The uniform vertical load is limited to a rim board depth of 16 inches

load transfer, see detail 1d

top plate per detail 1b Attach I-joist to

with same nailing

bearing plate

transter, nail to

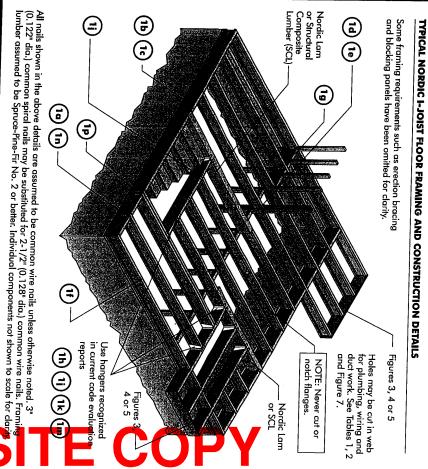
as required for

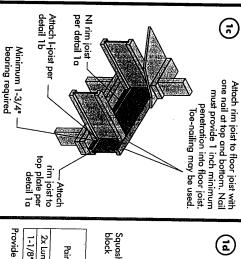
plate (when used tor lateral shear

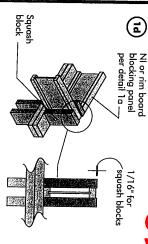
2-1/2" nails at

6" o.c. to top

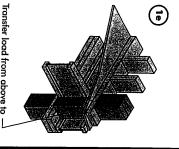
Blocking Panel or Rim Joist







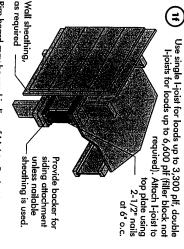
_	. —			
Provide lateral bracing per detail 1a, 1b, or 1c	1-1/8" Kim Board Plus	2x Lumber		Pair of Squash Blocks
r detail 1a, 1b,	4,300	5,500	3-1/2" wide	Pair of Squash Blocks (lbs)
or 1c	6,600	8,500	5-1/2" wide	bred Vertical per th Blocks (lbs)



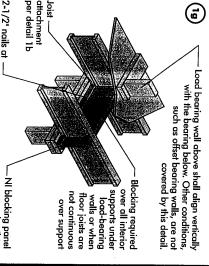
to post above. bearing below. Install squash blocks per detail 1d. Match Transfer load from above to bearing area of blocks below

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Nordic Lam or SCL

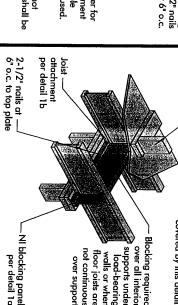


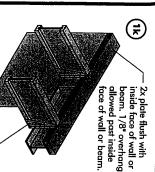
required when rim board is used. Bracing per code shall be Rim board may be used in lieu of Ljoists. Backer is not carried to the foundation.





(I)





Top-mount hanger installed per ___ manufacturer's recommendations

Install hanger per

recommendations manutacturer's

beams, see the manufacturer's

For nailing schedules for multiple

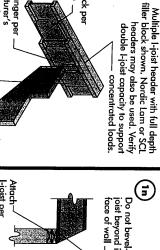
recommendations.

recommendations installed per manufacturer's

Top- or tace-mount hanger

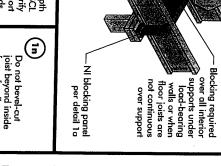
support the top flange, bearing Note: Unless hanger sides laterally stiffeners shall be used

support the top flange, bearing stiffeners shall be used. Note: Unless hanger sides laterally



detail 1p -Filler block per

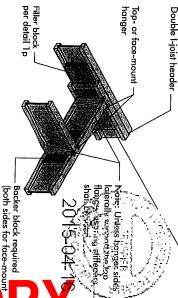
support, not shown at bearing for lateral Note: Blocking required



l-joist per detail 1b

additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Backer block (use if hanger load exceeds 360 lbs)
Before installing a backer block to a double I-joist, drive three resistance for hanger for this detail = 1,620 lbs. Use twelve 3" nails, clinched when possible. Maximum factored

 \bigcirc



Verify double I-joist capacity to support concentrated loads. For hanger capacity see hanger manufacturer's recommendations.

nangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

3-1/2"	2-1/2"	Flange Width
1-1/2"	7"	Material Thickness Required*
7-1/4"	5-1/2"	Minimum Depth**

- to CAN/CSA-O325 or CAN/CSA-O437 Standard better for solid sawn lumber and wood structural panels conforming Minimum grade for backer block material shall be S-P-F No. 2 or
- ** For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth

Notes:

Ŧ

Filler block

- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist prevent damage to web/flange connection
- 3. Filler block is required between joists for full length of span.
- 4. Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot possible) on each side of the double I-joi nails at 12 inches o.c. (clinched when Total of four nails per foot required. If na

-Offset nails from opposite face by 6"

-1/8" to 1/4" gap between top flange The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

and filler block

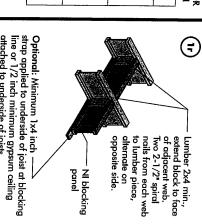
FILLER BLOCK REQUIREMENTS FOR DOUBLE 1-JOIST CONSTRUCTION

Maximum support capacity = 1,620 lbs.

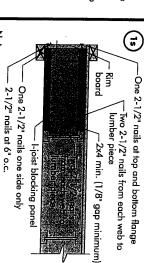
clinch when possible.

detail 1h. Nail with twelve 3" nails, Backer block attached per

ċι						4.		ယ			.2		<u>.</u>
The maximum factored load that may be applied to one side of the double joist	are required.	can be clinched, only two nails per foot	lotal of four nails per foot required. If nails	possible) on each side of the double 1-joist.	nails at 12 inches o.c. (clinched when	4. Nail joists together with two rows of 3"	tull length of span.	Filler block is required between joists for	flange.	of filler block and bottom of top 1-joist	Leave a 1/8 to 1/4-inch gap between top	prevent damage to web/flange connection.	1. Support back of I-joist web during nailing to
2** ×	ر ار ا		1-1/2	3-1/2" x	•		1-1/2	2-1/2" x		Size	Flange	COBE	FILLER BL
14° 5	11-7/8"	16"	14"	11-7/8"	9-1/2"	16.	14"	11-7/8"	9-1/2"	Depth	Joist	CO ISIO	OCK REQU
3 × 9 × 11	3" x 7"	3" x 12"	3" × 10"	သူ × ဇူ	3" x 6"	2-1/8" x 12"	2-1/8" x 10"	2-1/8" x 8"	2-1/8" x 6"	Block Size	Filler	DOUBLE I-JOIST CONSTRUCTION	FILLER BLOCK REQUIREMENTS FOR

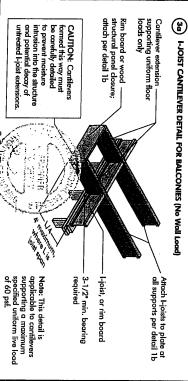


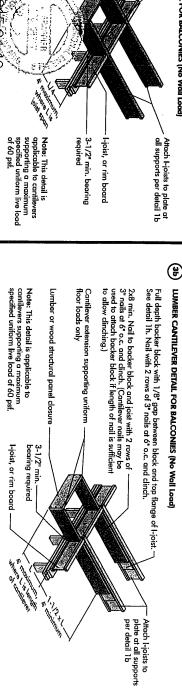
attached to underside of joists.



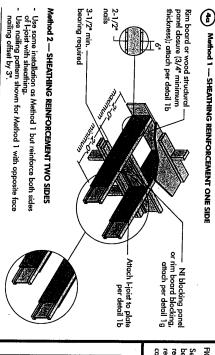
- In some local codes, blocking is prescriptively required in the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to tor spacing of the blocking
- All nails are common spiral in this detail







CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" rails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

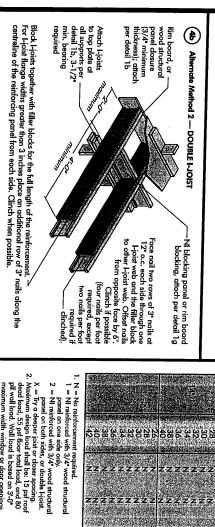


FIGURE 4 (continued) reinforcement requirements at cantilever. below for NI see table Roof truss span cantilever <u>ئ</u> 2

Roof trusses

13'-0" maximum For hip roofs with the jack trusses running parallel to the cantilevered floor joists,

	span	Roof truss
cantilever	2'-0"	Jack trusses
26 ft. shall be permitted to be used.	the I-joist reinforcement requirements for a span of	the cantilevered floor joists,

CANTILEVER REINFORCEMENT METHODS ALLOWED

JOIST DEPTH (in.)	ROOF TRUSS SPAN (#)	LL = 30 psf, DL = 15 p JOIST SPACING (in.) 12 16 19.2	- 8	DOF LOADIN LL = 40 psi JOIST SP 12 16	G (UNFACTO 5, DL = 15 p. ACING (in.) 19.2)RED) :f 24 1:	LL = 50 psf, JOIST SPA 2 16	DL = 15 ps CING (in.) 19.2	# 24
	2 2 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		XXXXX	22722	××082	2.22		****	××××
	26	2			×			X,	×
	28 30	ZZ					Z	, <u>.</u>	×Ν
	9 2			-z			ies ies i	សស់រ	××
	26	ZZ			2			×	××
	28 30	ZZ			ZZZ			ďΖ	33
Ŧ	32	ΖZ			·zz				NN
	588						2	222	< N N
	26 28	N			Ž			22	×:
	320	ZZZ ZZZ ZZZ	Z Z Z	222 222	zzz	: :zz	Z Z	zż	44.
	36 38	ZZZ			ZŹ				υ N =
	40 42	ZZ.			z	0N)			(NN)

4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam, the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a

For larger openings, or multiple 3'-0" width openings spaced less than 6'-0' o.c., additional loss beneath the opening's cripple stude may be required.

3. Table applies to joists 12' to 24" o.c. that meet the floor span requirements for a design live load of 40 pet and dead load of 15 pet, and a five load deflection limit of L480. Use 12" o.c. requirements for lesser spacing.

truss is used.

Cantilevered joists supporting girder trusses or root beams

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. hole or duct chase opening shall be in compliance with the requirements of
- Ņ I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. The maximum size hole or the maximum depth of a duct chase opening that can between the top or bottom of the hole or opening and the adjacent I-joist flange. be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained
- Ċ ٥. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 7. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the opening shall be sized and located in compliance with the requirements of size of the largest square hole (or twice the length of the langest side of the langest rectangular hole or duct chase opening) and each hole and duct chase Tables 1 and 2, respectively.
- A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ cantilevered section of a joist. Holes of greater size may be permitted subject to Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a
- 9 A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

					Depth
1927.00	eto e le le le			4 TO 18 19 19 19 19 19 19 19 19 19 19 19 19 19	
					2
					6
0.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0				i di Tar	und hol
442					
			100		(in.)
	3 e e 200		11.11.1		10-3/4
	100 4100		1 1 1	1111	.in., 12
		is ris	CARL SHAPE OF	CONTRACTOR AND AND	Sı adju 12-3/4 Fa
A I					ctor Straent

- 2. Hole location distance is measured from inside force of supports to centre of hole.

 3. Distances in this chart are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the Hoists used at their maximum span. If the Hoists are placed at less than their full maximum span (see Maximum fico) Spans, the minimum distance from the centreline of the hole to the tace of any support (D) as given above may be reduced as follows:

Dreduced = Lactual x D

Where: Dreduced =

ը Է՞ Lactual

The actual measured span distance between the inside faces of supports (ft). Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (th. The testicated distance shall not be less than 6 inches from the face of the support to edge of the hole. Span Adjustment Factor given in this table.

The minimum distance from the inside face of any support to centre of hole from this table

Lactual is greater than 1, use 1 in the above calculation for Lactual.

SAF

150AY

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS - Simple Span Only Minimum distance from inside face of any support to centre of opening (ft-in.)

Duct chase length 12 14 16 16 20 20 20 20 20 20 20 20 20 20 20 20 20	8 10 12 14 16 18 8 10 12 14 16 18 5 10 5 20 5 20 5 20 5 20 5 20 5 20 5 20	8 10 12 14 16 18 20 1 14 16 18 20 1 15 10 12 14 16 18 20 1 15 10 12 14 16 18 20 1 15 10 12 14 16 18 20 1 15 10 15 10 15 15 15 15 15 15 15 15 15 15 15 15 15
Duct chase lengt 12 14 16 12 14 16 24 10 24 25 25 25 25 25 25 25 25 25 25 25 25 25	Duct chase length (in.) 12 14 16 18 20 0 25 210 25 20 0 25	Duct chase length (in.) 12 14 16 18 20 13 14 16 18 20 14 16 18 20 15 16 18 20 16 18 20 17 18 20 18 20 18 20
Duct chase lengt 14 16 16 24 25 25 25 25 25 25 25 25 25 25 25 25 25	Duct chase length (in.) 14 16 18 25 25 25 25 25 25 25 25 25 25 25 25 25 2	Duct chase length (in.) 14 16 18 20 14 16 18 20 25 20 25 25 25 25 25 25 25 25 25 25 25 25 25
<u> </u>	igh (in.) 18 2.3.3	thase length (in.) 16 18 20 21 23 34 35 35 35 35 35 35 35 35 3
		20 9.00 9.00 9.00 9.00 9.00 9.00 9.00 9.

15:2"

- Above table may be used for Lioist spacing of 24 inches on centre or less.
 Duct chase opening location distance is measured from inside face of supports to centre of opening.
 The above table is based on simple-span joists only. For other applications, contact your local distributor.
 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor.

FIGURE 7

FIELD-CUT HOLE LOCATOR

bearing distance from for minimum See Table 1 Knockouts О 2x diameter of larger hole rule 12 whichever is 2x duct chase all duct chase openings and holes length or hole between top and bottom flange Maintain minimum 1/8" space Duct chase opening from bearing) minimum distance (see Table 2 for

and may be ignored for purposes of calculating minimum distances A knockout is **NOT** considered a hole, may be utilized wherever it occurs

field-cut holes

sharp saw. should be cut with a Holes in webs over-cut the web. notch the flange, or Never drill, cut or spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of

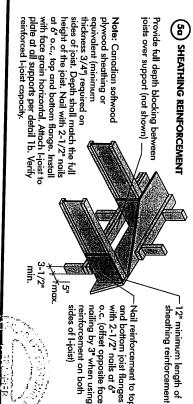
are 1-1/2 inches in diameter, and are

electrical or small plumbing lines. They tor the contractor's convenience to install Knockouts are prescored holes provided

stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch and then making the cuts between the holes is another good method to diameter hole in each of the four corners the corners, as this can cause unnecessary For rectangular holes, avoid over-cutting ninimize damage to the I-joist

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

FIGURE 5 (continued)



Nail reinforcement to top nailing by 3" when using o.c. (offset opposite face and bottom joist flanges with 2-1/2" nails at 6"

> requirements at cantilever. reinforcement below for NI See table

> > Roof truss span

> > > T 2'-0"

(5b) SET-BACK DETAIL

Bearing walls

(3/4" minimum thickness), attach per detail 1b. structural panel closure Rim board or wood

- Provide full depth blocking between joists over support (not shown for clarity)
- supports per detail 1b. 3-1/2" minimum I-joist Attach I-joist to plate at all

bearing required.

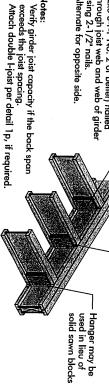


SET-BACK CONNECTION

(F)

through joist web and web of girder using 2-1/2" nails. Vertical solid sawn blocks _____ (2x6 S-P-F No. 2 or better) nailed Alternate for opposite side.

> bottom flanges. Nail joist end using 3" nails, toe-nail at top and



Notes:

- N = No reinforcement required.
 N reinforced with 3/4" wood structural
- panel on one side only. 2 = NI reinforced with 3/4ⁿ wood structural panel on both sides, or double I-joist.
- 2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0" X = Try a deeper joist or closer spacing.maximum width window or door openings.
- For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple
- shuds may be required.

 Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.

Lmaximum -5" maximum cantilever Girder_ Roof trusses Roof trussspan ন 13'-0" maximum Jack trusses 2<u>-</u>6 - maximum cantilever 5" maximum

the cantilevered floor joists, For hip roofs with the jack trusses running parallel to the I-joist reinforcement 26 ft. shall be permitted to requirements for a span of

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

JOIST DEPTH (in.)	ROOF TRUSS SPAN (fi	IL = JO 12	: 30 psf, DIST SPA: 16	DL = 15 CING (in. 19.2	psf) 24	ROOFL LL : Ju 12 2	OADING = 40 psf, OIST SPA 16	(UNFAC DL = 15 CING (in 19.2	LTORED) i psf i.) 24	12 22 4	= 50 ps JOIST SP 16	f, DL = 1 ACING (i 19.2	5 psf n.) 24
	88 208228 28 88 208228	;add2ZZ <mark>222</mark>	XXX BAGAGAXX) (************************************		1 0×× ===00			*****	∠××≗dana×		**************************************	**************************************
	226 30 32 34 34 38 40	<u>ZZZZZ</u>	≛Cododox T	××××××k	*****	ナーコニーニング ターコニーニング データ	<.xx		××××××××××××××××××××××××××××××××××××××	ж	*****	*****	*****
	226 226 32 34 34 40	-zzzzzzzz	30000-1-1	dada****			:×××××××××××××××××××××××××××××××××××××	×××××		NNHHHZ	xxxxx มีที่สั	*****	*****

- For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the Roof Truss Span is equivalent to the When the roof is framed using a ridge board, truss is used. distance between the supporting walls as if a the supporting wall and the ridge beam.
- 5. Cantilevered joists supporting girder trusses or roof beams may require additional reintorcing.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next-panel from damage when topped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- After the first row of panels is in place, spread glue in the groove of one or two panels at a time before loying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.
- Tap the second row of panels into place, using a block to protect groove edges.
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch of all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2' common nail to assure accurate and consistent spacing.)
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the glue bond.

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24 3	20	6	Maximum Joist Spacing (in.)
3/4	1 +5/8 ·*	5/8	Minimum Panel Thickness (in.)
22	2	2**	Common Wire or Spiral Nails
1-3/4"	1-3/4"	1-3/4"	ail Size and Ty Ring Thread Nails or Screws
2	2"	2"	pe Staples
6"	6"	6	Maximum of Fas Edges
12"	12"	12"	n Spacing teners Interm, Supports

- Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter
- Special conditions may impose heavy traffic and concentrated loads that require construction in excess
 of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.
- Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

IMPORTANT NOTE:

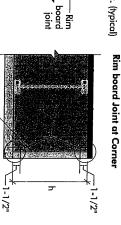
Floor sheathing must be field glued to the Lioist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

RIM BOARD INSTALLATION DETAILS

(8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

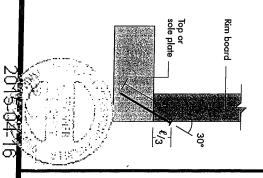
Rim board Joint Between Floor Joists 2-1/2" nails at 6" 6.





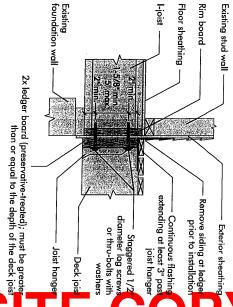


2-1/2" toe-nails at 6" o.c. (typical) —

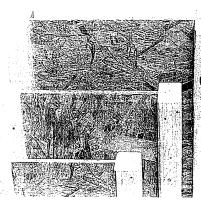




Rim board joint







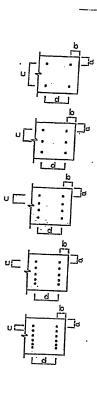
· MICRO CITY

engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

	LVL HEADER AND CONVENTION					
	LVL HEA	BER NAILING	NVENTIONAL DETAILS			
	DETAIL NUMBER					
	. A	2	12			
-	В	2	8			
- -	С	2	6			
L	D	2	4			
⋅ _	1A	3	12			
-	1B	3	8			
╙	1C	3	. 6			
L	1D	. 3:	4			
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L	2B	4	8 .			
L	. 2C	4	6			
	2D	4	4			
_	3A	5	12			
L	3B	5	. 8			
Ŀ		3C 5				
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<u> </u>	4A	6	12			
<u> </u>	4B	6	8			
<u> </u>	4C	6	6			
L	4D	6	4			



NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
 - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
 - (3) MINIMUM NAIL ROW SPACING "c" = 2"
 - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
 - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
 - (6) DO NOT USE AIR-DRIVEN NAILS



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STRUCTURAL

COMPONENT ONLY

TO BE USED ONLY

WITH BEAM CALOS

PEARING THE

STAMP BELOWS

PROVICE NAILING
DETAIL № × SEE
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