TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-11-15
INSPECTOR: BG

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

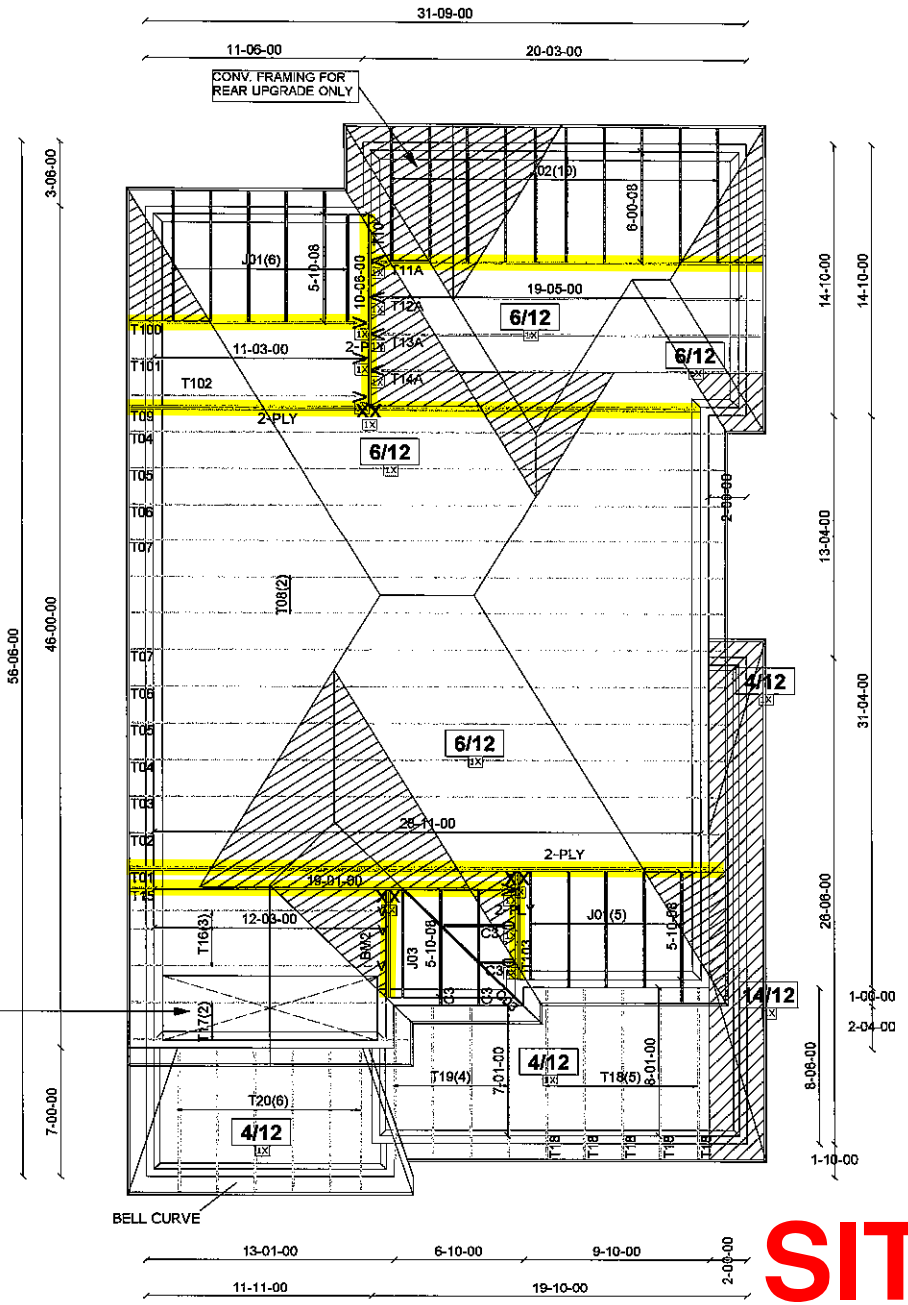
O LUS24
V LJS26DS
XX HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL. (CONFIRM)**
- RAISED 12 "
- INSET 14 "

****4/12 CATH. CL.****

BM2: 2-2X10



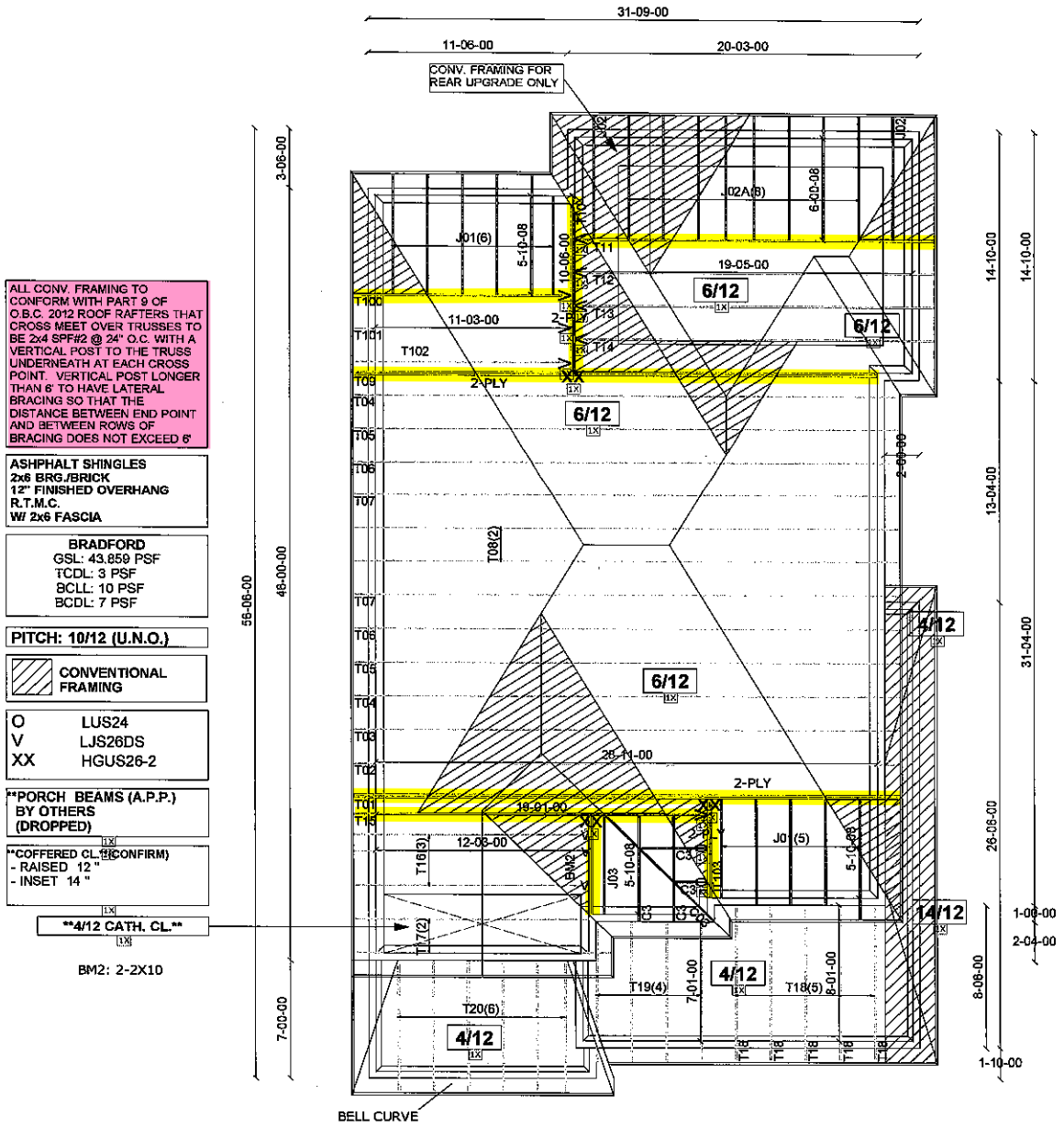
SITE COPY

M11,253

TAMARACK TRUSSES INC.	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-17 / A-STD
	Layout ID: 288690	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/6/2018 Designer: Peter Dahlberg/JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.6.4



SITE COPY

M11,253

TAMARACK LUMBER INC.	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-17 / A OPT.COFF
	Layout ID: 288659	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/6/2018 Designer: Peter Dahlberg/JG	

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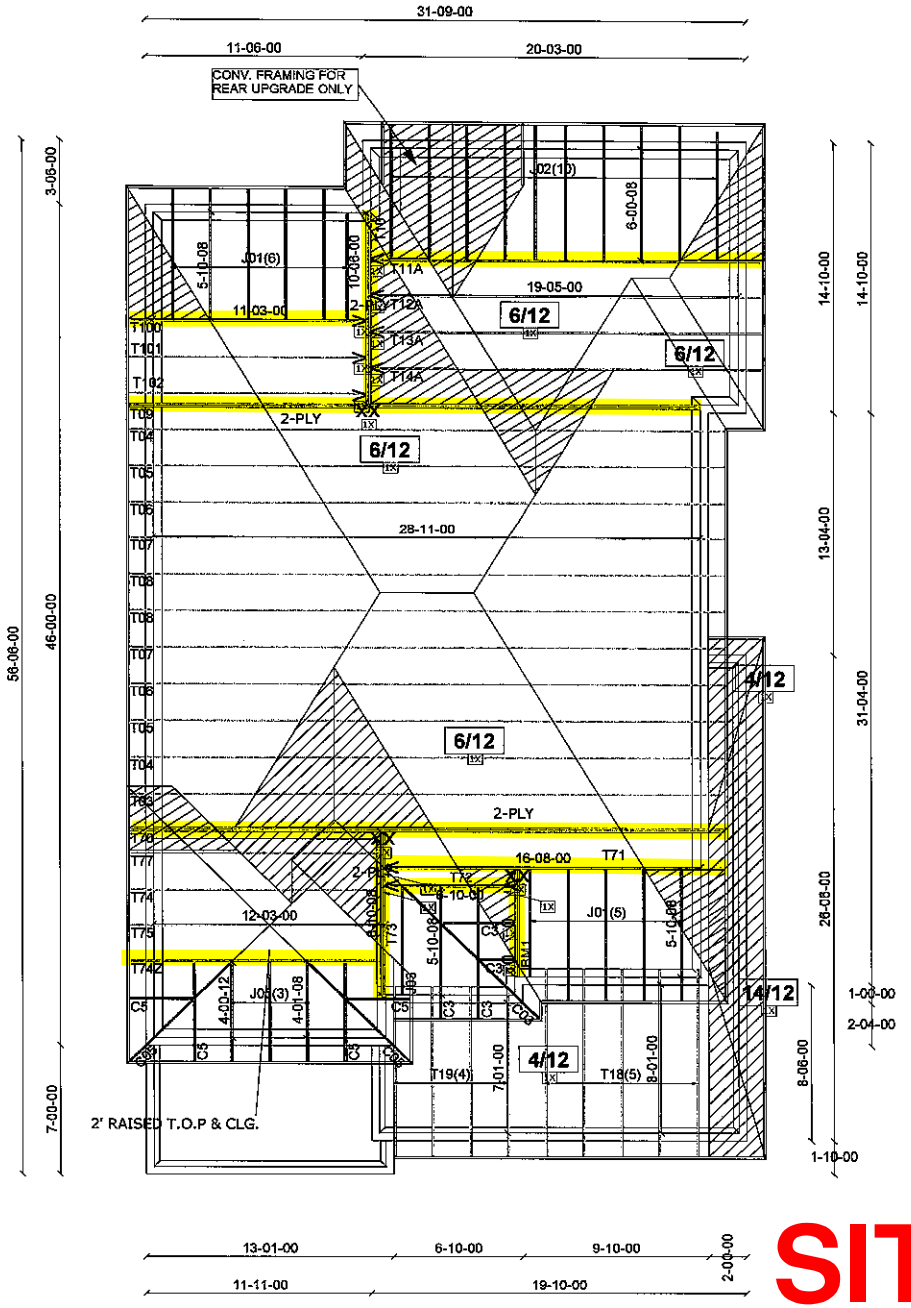
Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)

**COFFERED CL. (CONFIRM)
- RAISED 12 "
- INSET 14 "**

BM1: 2-2X10

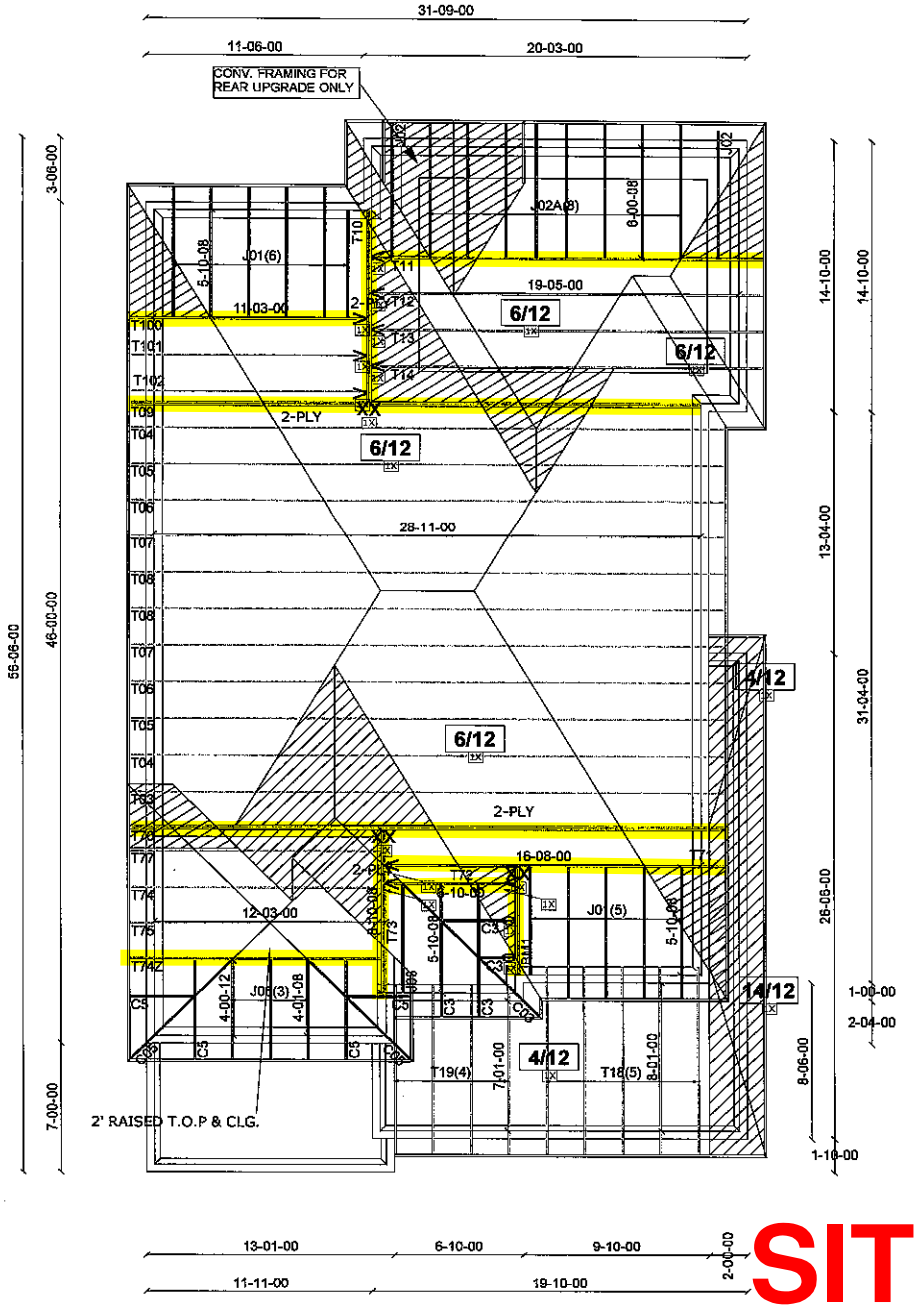
SITE COPY

M11,253

TAMARACK LUMBER INC.	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-18 / B- STD**
	Layout ID: **288706**	Project: **GREEN VALLEY EAST**	
	Plan Log: **96660**	Date: **3/7/2018** Designer: Peter Dahlberg/JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

**PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)

**COFFERED CL. (TO CONFIRM)
- RAISED 12 "
- INSET 14 "

BM1: 2-2X10

SITE COPY

M11,253

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-17 / B OPT.COFF
	Layout ID: 288660	Project: GREEN VALLEY EAST	
	Plan Log: 93767	Date: 3/7/2018 Designer: Peter Dahlberg/JG	

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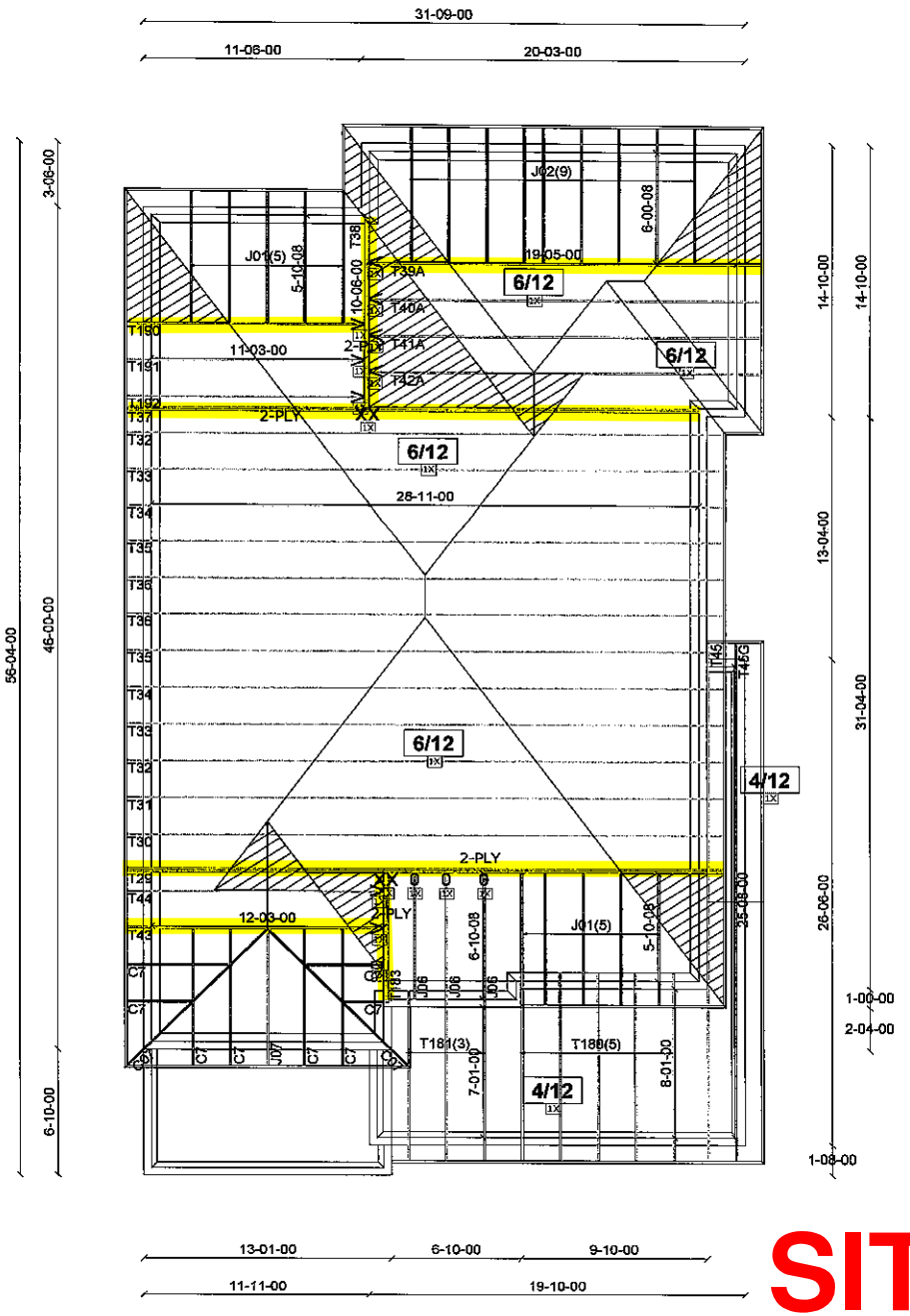
Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

COFFERED CL.(CONFIRM)
- RAISED 12 "
- INSET 14 "

1X

SITE COPY

M11,253

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	288711
Plan Log:	96660

Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**

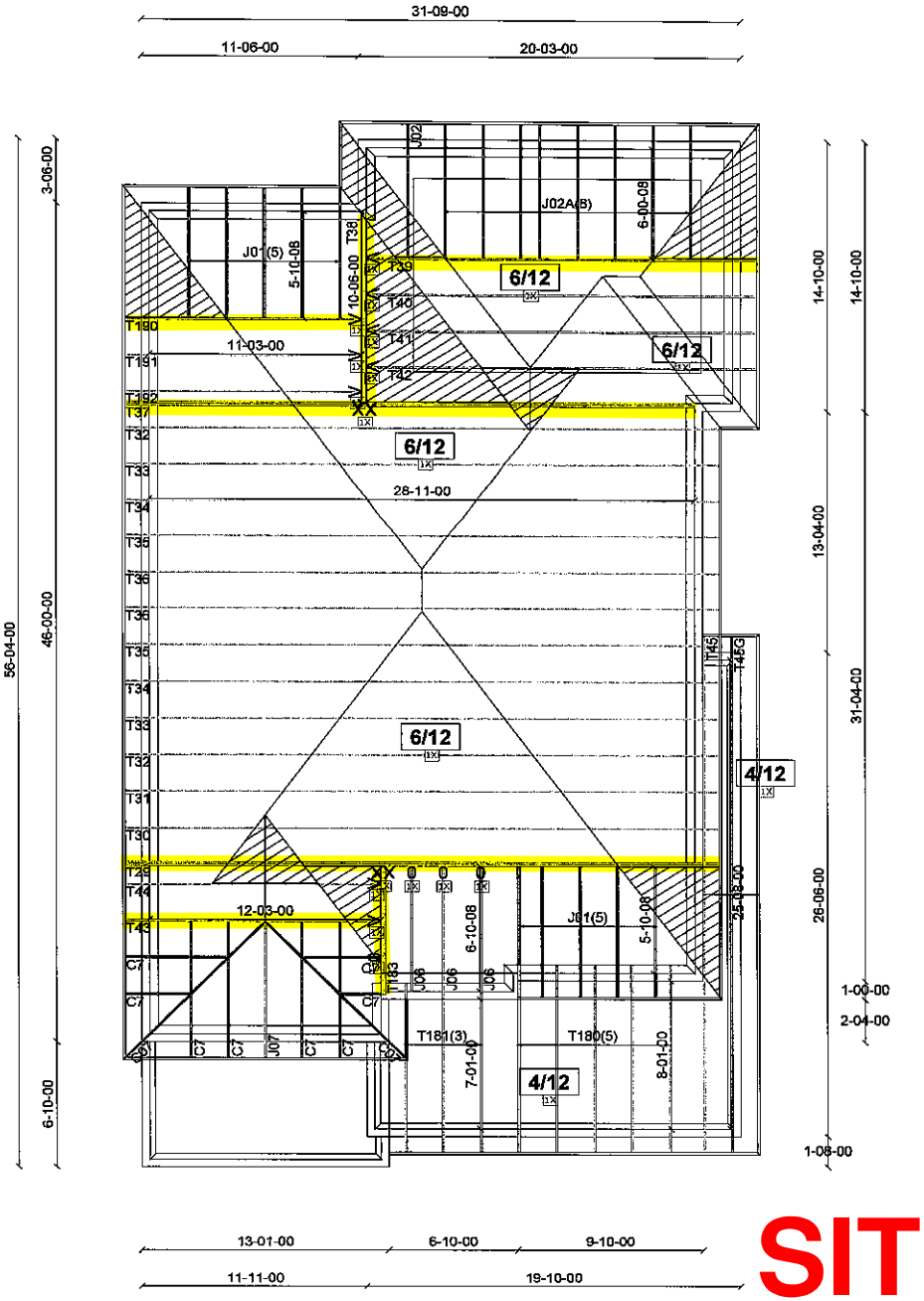
Project: **GREEN VALLEY EAST**

Date: **3/7/2018** Designer: Peter Dahlberg/JG

Model / Elevation: **S38-17 / C-STD.**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

COFFERED CL.(CONFIRM)
- RAISED 12 "
- INSET 14 "

SITE COPY

M11,253

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 288664
Plan Log: 96660

Builder / Location: BAYVIEW WELLINGTON / BRADFORD

Model / Elevation: S38-17 / C OPT.COFF

Project: GREEN VALLEY EAST

Date: 3/7/2018 Designer: Peter Dahlberg/JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.6.4

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

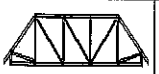
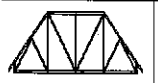
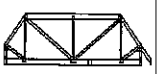
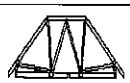
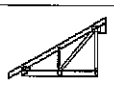
Delivery Shiplist

DATE	03/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288690 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: A-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	327.44 204.00		
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	348.30 217.34		
	1 2 Ply	T10 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T11A HIP GIRDER	10.00 0.00	19-05-00	04-02-04	2 X 4	2 X 6	00-00-00 01-03-08	01-07-11 01-07-11	97.10 61.00		
	1	T12A ROOF	10.00 0.00	19-05-00	05-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	88.86 57.17		
	1	T13A ROOF	10.00 0.00	19-05-00	06-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	87.83 58.17		
	1	T14A ROOF	10.00 0.00	19-05-00	07-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	85.90 56.00		
	1	T15 HIP GIRDER	10.00 0.00	19-01-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	126.87 76.67		
	3	T16 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	177.27 114.51		
	2	T17 SCISSORS	10.00 4.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.74 76.34		
	5	T18 MONOPITCH	4.00 0.00	08-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 03-03-08	163.00 104.15		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

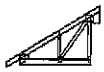
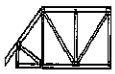
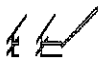
Delivery Shiplist

DATE	03/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288690 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: A-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	T19 MONOPITCH	4.00 / 0.00	07-01-00	03-03-00	2 X 4	2 X 4	01-03-08 / 00-00-00	00-07-03 / 02-11-08	116.28 / 76.00		
	6	T20 MONOPITCH	4.00 / 0.00	06-07-00	02-09-12	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-06-04	153.78 / 99.00		
	1	T100 HALF HIP	10.00 / 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-01-04	50.81 / 33.50		
	1	T101 HALF HIP	10.00 / 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-01-04	51.91 / 34.33		
	1	T102 HALF HIP	10.00 / 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-01-04	52.39 / 33.33		
	1 / 2 Ply	T103 JACK-CLOSED	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 04-01-04	63.56 / 40.66		
	11	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		
	10	J02 JACK-OPEN	6.00 / 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-02-04	171.90 / 113.30		
	1	J03 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	18.99 / 12.00		
	1	C03 JACK-OPEN	10.00 / 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 00-03-08	70.45 / 46.00		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2768.36** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4365.60** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
11	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **16.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

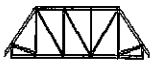
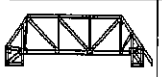
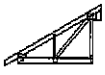
Delivery Shiplist

DATE	03/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288659 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	327.44 204.00		
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	348.30 217.34		
	1 2 Ply	T10 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T11 HIP GIRDER	10.00 0.00	19-05-00	04-02-04	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	86.62 57.83		
	1	T12 ROOF	10.00 0.00	19-05-00	05-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	94.50 63.33		
	1	T13 ROOF	10.00 0.00	19-05-00	06-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	92.13 63.50		
	1	T14 ROOF	10.00 0.00	19-05-00	07-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	93.96 63.50		
	1	T15 HIP GIRDER	10.00 0.00	19-01-00	06-06-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	126.87 76.67		
	3	T16 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	177.27 114.51		
	2	T17 SCISSORS	10.00 4.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.74 76.34		
	5	T18 MONOPITCH	4.00 0.00	08-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 03-03-08	163.00 104.15		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

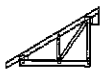
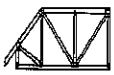
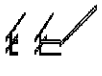
Delivery Shiplist

DATE	03/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288659 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-17 **ELEVATION:** A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	T19 MONOPITCH	4.00 / 0.00	07-01-00	03-03-00	2 X 4	2 X 4	01-03-08 / 00-00-00	00-07-03 / 02-11-08	116.28 / 76.00		
	6	T20 MONOPITCH	4.00 / 0.00	06-07-00	02-09-12	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-06-04	153.78 / 99.00		
	1	T100 HALF HIP	10.00 / 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-01-04	50.81 / 33.50		
	1	T101 HALF HIP	10.00 / 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-01-04	51.91 / 34.33		
	1	T102 HALF HIP	10.00 / 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-01-04	52.39 / 33.33		
	1 / 2 Ply	T103 JACK-CLOSED	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 04-01-04	63.56 / 40.66		
	11	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		
	2	J02 JACK-OPEN	6.00 / 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-02-04	34.38 / 22.66		
	8	J02A JACK-OPEN	6.00 / 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-02-04	177.44 / 128.00		
	1	J03 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	18.99 / 12.00		
	1	C03 JACK-OPEN	10.00 / 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 00-03-08	70.45 / 46.00		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2821.54** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4413.04** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
11	Hangers	LJS26DS	

TOTAL # ITEMS= **14.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

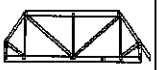
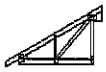
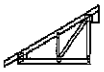
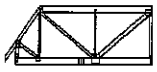
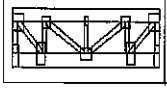
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288706 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: B-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	348.30 217.34		
	1 2 Ply	T10 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T11A HIP GIRDER	10.00 0.00	19-05-00	04-02-04	2 X 4	2 X 6	00-00-00 01-03-08	01-07-11 01-07-11	97.10 61.00		
	1	T12A ROOF	10.00 0.00	19-05-00	05-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	88.86 57.17		
	1	T13A ROOF	10.00 0.00	19-05-00	06-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	87.77 58.17		
	1	T14A ROOF	10.00 0.00	19-05-00	07-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	85.90 56.00		
	5	T18 MONOPITCH	4.00 0.00	08-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 03-03-08	163.00 104.15		
	4	T19 MONOPITCH	4.00 0.00	07-01-00	03-03-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 02-11-08	116.28 76.00		
	1 2 Ply	T70 HIP GIRDER	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	303.44 187.34		
	1	T71 HALF HIP	10.00 0.00	16-08-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	87.59 53.83		
	1	T72 HALF HIP	10.00 0.00	06-10-00	06-06-07	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 06-06-07	40.12 25.00		
	1 2 Ply	T73 FLAT GIRDER	0.00 0.00	08-10-08	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	88.26 57.34		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

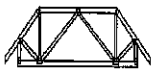
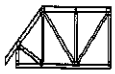
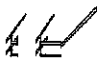
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288706 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-17 **ELEVATION:** B-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T74 HIP	10.00 0.00	12-03-00	05-00-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	56.48 37.67		
	1	T74Z HIP GIRDER	10.00 0.00	12-03-00	05-00-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	56.48 37.67		
	1	T75 HIP	10.00 0.00	12-03-00	06-08-05	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.95 38.17		
	1	T77 HIP	10.00 0.00	12-03-00	03-04-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	52.44 34.33		
	1	T100 HALF HIP	10.00 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	50.81 33.50		
	1	T101 HALF HIP	10.00 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	51.91 34.33		
	1	T102 HALF HIP	10.00 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	52.39 33.33		
	11	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	10	J02 JACK-OPEN	6.00 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-02-04	171.90 113.30		
	1	J03 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	18.99 12.00		
	3	J05 JACK-OPEN	10.00 0.00	04-01-08	05-00-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-00-15	48.69 34.50		
	1	C03 JACK-OPEN	10.00 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	70.45 46.00		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **2584.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4069.66** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
10	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

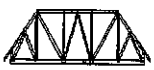
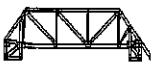
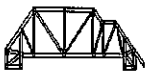
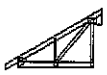
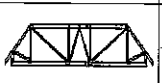
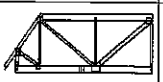
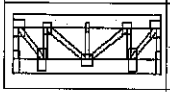
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288660 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	348.30 217.34		
	1 2 Ply	T10 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T11 HIP GIRDER	10.00 0.00	19-05-00	04-02-04	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	86.62 57.83		
	1	T12 ROOF	10.00 0.00	19-05-00	05-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	94.50 63.33		
	1	T13 ROOF	10.00 0.00	19-05-00	06-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	92.13 63.50		
	1	T14 ROOF	10.00 0.00	19-05-00	07-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-07-11 01-07-11	93.96 63.50		
	5	T18 MONOPITCH	4.00 0.00	08-01-00	03-07-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 03-03-08	163.00 104.15		
	4	T19 MONOPITCH	4.00 0.00	07-01-00	03-03-00	2 X 4	2 X 4	01-03-08 00-00-00	00-07-03 02-11-08	116.28 76.00		
	1 2 Ply	T70 HIP GIRDER	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	303.44 187.34		
	1	T71 HALF HIP	10.00 0.00	16-08-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	87.59 53.83		
	1	T72 HALF HIP	10.00 0.00	06-10-00	06-06-07	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 06-06-07	40.12 25.00		
	1 2 Ply	T73 FLAT GIRDER	0.00 0.00	08-10-08	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	88.26 57.34		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

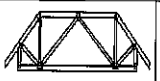
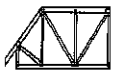
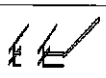
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288660 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-17 **ELEVATION:** B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T74 HIP	10.00 0.00	12-03-00	05-00-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	56.48 37.67		
	1	T74Z HIP GIRDER	10.00 0.00	12-03-00	05-00-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	56.48 37.67		
	1	T75 HIP	10.00 0.00	12-03-00	06-08-05	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.95 38.17		
	1	T77 HIP	10.00 0.00	12-03-00	03-04-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	52.44 34.33		
	1	T100 HALF HIP	10.00 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-04	50.81 33.50		
	1	T101 HALF HIP	10.00 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	51.91 34.33		
	1	T102 HALF HIP	10.00 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	52.39 33.33		
	11	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	2	J02 JACK-OPEN	6.00 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-02-04	34.38 22.66		
	8	J02A JACK-OPEN	6.00 0.00	06-00-08	04-02-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-02-04	177.44 128.00		
	1	J03 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	18.99 12.00		
	3	J05 JACK-OPEN	10.00 0.00	04-01-08	05-00-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-00-15	48.69 34.50		
	1	C03 JACK-OPEN	10.00 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	70.45 46.00		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **2638.04** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4117.16** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

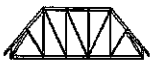
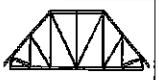
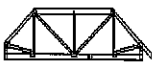
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288711 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: C-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T29 HIP GIRDER	8.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	337.72 212.00		
	1	T30 HIP	8.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	115.80 72.67		
	1	T31 HIP	8.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	128.98 81.00		
	2	T32 HIP	8.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	255.52 161.00		
	2	T33 HIP	8.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	268.64 166.66		
	2	T34 HIP	8.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	263.06 162.68		
	2	T35 HIP	8.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	267.64 167.34		
	2	T36 COMMON	8.00 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	269.58 167.34		
	1 2 Ply	T37 HIP GIRDER	8.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 05-05-13	354.36 216.00		
	1 2 Ply	T38 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T39A HIP GIRDER	8.00 0.00	19-05-00	04-02-04	2 X 4	2 X 6	00-00-00 01-03-08	01-04-13 01-04-13	94.69 59.00		
	1	T40A ROOF	8.00 0.00	19-05-00	05-02-04	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	81.72 52.67		
	1	T41A ROOF	8.00 0.00	19-05-00	06-02-04	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	86.74 55.50		
	1	T42A ROOF	8.00 0.00	19-05-00	07-02-08	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	85.19 55.00		
	1	T43 COMMON	8.00 0.00	12-03-00	05-05-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	54.00 35.84		
	1	T44 COMMON	8.00 0.00	12-03-00	05-05-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	54.00 35.84		
	1	T45 COMMON	4.00 0.00	25-08-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-03-15 00-03-15	80.04 49.83		
	1	T45G COMMON	4.00 0.00	25-08-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	82.18 51.83		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

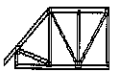
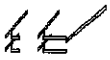
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288711 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: C-STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	T180 ROOF	4.00 / 0.00	08-01-00	03-03-12	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 03-00-04	153.90 / 100.85		
	3	T181 ROOF	4.00 / 0.00	07-01-00	02-11-12	2 X 4	2 X 4	01-03-08 / 00-08-08	00-03-15 / 02-08-04	82.29 / 54.99		
	1 / 2 Ply	T183 JACK-CLOSED	6.00 / 0.00	07-00-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	00-07-00 / 04-01-04	64.00 / 42.66		
	1	T190 HALF HIP	8.00 / 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-01-04	50.37 / 32.33		
	1	T191 HALF HIP	8.00 / 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	48.82 / 31.00		
	1	T192 HALF HIP	8.00 / 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-04	54.07 / 34.83		
	10	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	167.90 / 106.70		
	9	J02 JACK-OPEN	6.00 / 0.00	06-00-00	04-02-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-02-00	153.81 / 96.03		
	3	J06 JACK-PARTIAL	6.00 / 0.00	06-10-08	04-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-00 / 04-01-04	65.64 / 39.99		
	1	J07 JACK-PARTIAL	8.00 / 0.00	06-01-08	05-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-05-13	24.86 / 16.50		
	2	C04 JACK-PARTIAL	8.00 / 0.00	06-01-08	06-11-05	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 00-03-08	138.58 / 88.00		

TOTAL # TRUSS= **64.00** TOTAL BFT OF ALL TRUSSES= **2521.42** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4000.58** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
1	Hangers	LUS24	

TOTAL # ITEMS= **12.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

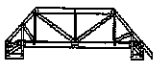
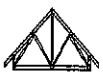
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288664 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-17 **ELEVATION:** C-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T29 HIP GIRDER	8.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	337.72 212.00		
	1	T30 HIP	8.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	115.80 72.67		
	1	T31 HIP	8.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	128.98 81.00		
	2	T32 HIP	8.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	255.52 161.00		
	2	T33 HIP	8.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	268.64 166.66		
	2	T34 HIP	8.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	263.06 162.68		
	2	T35 HIP	8.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	267.64 167.34		
	2	T36 COMMON	8.00 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	269.58 167.34		
	1 2 Ply	T37 HIP GIRDER	8.00 0.00	28-11-00	06-05-00	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 05-05-13	354.36 216.00		
	1 2 Ply	T38 JACK-CLOSED	6.00 0.00	10-06-00	06-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-05-00	116.48 75.34		
	1	T39 HIP GIRDER	8.00 0.00	19-05-00	04-02-04	2 X 4	2 X 6	00-00-00 01-03-08	01-04-13 01-04-13	99.89 64.67		
	1	T40 ROOF	8.00 0.00	19-05-00	05-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	86.25 57.33		
	1	T41 ROOF	8.00 0.00	19-05-00	06-02-00	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	88.00 57.83		
	1	T42 ROOF	8.00 0.00	19-05-00	07-02-08	2 X 4	2 X 4	00-00-00 01-03-08	01-04-13 01-04-13	84.08 55.67		
	1	T43 COMMON	8.00 0.00	12-03-00	05-05-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	54.00 35.84		
	1	T44 COMMON	8.00 0.00	12-03-00	05-05-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	54.00 35.84		
	1	T45 COMMON	4.00 0.00	25-08-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-03-15 00-03-15	80.04 49.83		
	1	T45G COMMON	4.00 0.00	25-08-00	04-07-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 05-05-13	82.18 51.83		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

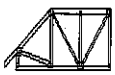
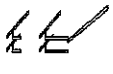
Delivery Shiplist

DATE	03/07/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288664 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-17 ELEVATION: C-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	T180 ROOF	4.00 / 0.00	08-01-00	03-03-12	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 03-00-04	153.90 / 100.85		
	3	T181 ROOF	4.00 / 0.00	07-01-00	02-11-12	2 X 4	2 X 4	01-03-08 / 00-08-08	00-03-15 / 02-08-04	82.29 / 54.99		
	1 / 2 Ply	T183 JACK-CLOSED	6.00 / 0.00	07-00-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	00-07-00 / 04-01-04	64.00 / 42.66		
	1	T190 HALF HIP	8.00 / 0.00	11-03-00	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-01-04	50.37 / 32.33		
	1	T191 HALF HIP	8.00 / 0.00	11-03-00	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	48.82 / 31.00		
	1	T192 HALF HIP	8.00 / 0.00	11-03-00	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-04	54.07 / 34.83		
	10	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	167.90 / 106.70		
	1	J02 JACK-OPEN	6.00 / 0.00	06-00-00	04-02-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-02-00	17.09 / 10.67		
	8	J02A JACK-OPEN	6.00 / 0.00	06-00-00	04-02-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-02-00	174.88 / 128.00		
	3	J06 JACK-PARTIAL	6.00 / 0.00	06-10-08	04-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-00 / 04-01-04	65.64 / 39.99		
	1	J07 JACK-PARTIAL	8.00 / 0.00	06-01-08	05-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-05-13	24.86 / 16.50		
	2	C04 JACK-PARTIAL	8.00 / 0.00	06-01-08	06-11-05	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 00-03-08	138.58 / 88.00		

TOTAL # TRUSS= **64.00** TOTAL BFT OF ALL TRUSSES= **2577.39** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4048.62** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T01	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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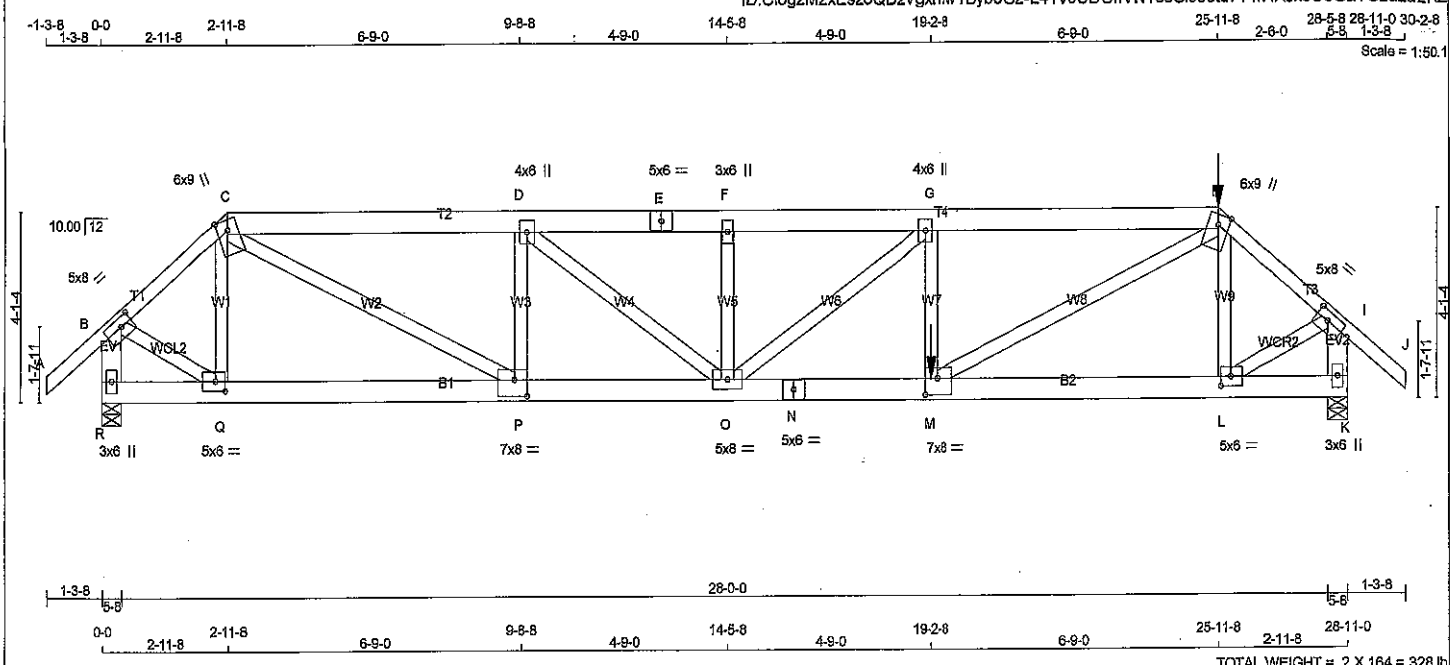

TOTAL WEIGHT = 2 X 164 = 328 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	SIDE(0.0)
C- E	2	12	TOP
E- H	2	12	SIDE(50.8)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(23.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.25
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-t	MT20	5.0	8.0	2.25	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	7.0	8.0	4.25	3.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	7.0	8.0	4.25	3.50
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3421	0	3421	0	0	5-8	5-8
K	5277	0	5277	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2893	1697 / 0	493 / 0	0 / 0	0 / 0	502 / 0	0 / 0
K	4175	2597 / 0	787 / 0	0 / 0	0 / 0	791 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-859 / 0	0.08 (1)
B- C	-3230 / 0	-104.9	-104.9	0.13 (1)	5.07	C- P	0 / 5187	0.46 (1)
C- D	-7082 / 0	-104.9	-104.9	0.27 (1)	4.31	P- D	-2353 / 0	0.20 (1)
D- E	-9163 / 0	-104.9	-104.9	0.27 (1)	3.86	D- O	0 / 2625	0.23 (1)
E- F	-9163 / 0	-104.9	-104.9	0.27 (1)	3.86	O- F	-294 / 0	0.03 (1)
F- G	-9163 / 0	-104.9	-104.9	0.34 (1)	3.77	O- G	-1866 / 0	0.39 (1)
G- H	-10644 / 0	-206.6	-206.6	0.60 (1)	3.26	M- G	-40 / 390	0.03 (3)
H- I	-5160 / 0	-104.9	-104.9	0.19 (1)	4.13	M- H	0 / 7529	0.67 (1)
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	L- H	-1279 / 0	0.11 (1)
R- B	-3440 / 0	0.0	0.0	0.13 (1)	7.52	B- Q	0 / 2714	0.24 (1)
K- I	-5316 / 0	0.0	0.0	0.20 (1)	6.35	L- I	0 / 4336	0.36 (1)
R- Q	0 / 0	-27.5	-27.5	0.07 (2)	10.00			
Q- P	0 / 2524	-27.5	-27.5	0.22 (1)	10.00			
P- O	0 / 7081	-27.5	-27.5	0.50 (1)	10.00			
O- N	0 / 10644	-27.5	-27.5	0.76 (1)	10.00			
N- M	0 / 10644	-27.5	-27.5	0.76 (1)	10.00			
M- L	0 / 4028	-54.1	-54.1	0.38 (1)	10.00			
L- K	0 / 0	-54.1	-54.1	0.15 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	25-11-8	-226	-226	—	FRONT	VERT	TOTAL
M	19-2-8	-3407	-3407	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-8-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.76/1.00 (M-O:1) , WB=0.67/1.00 (H-M:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER 3816 S. KOMLSOULAKOS PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 132698

STRUCTURAL COMPONENT ONLY

P6/6

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T01	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 3406.9 lbs FACTORED DOWN AT 19-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.85 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13260-R
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T02	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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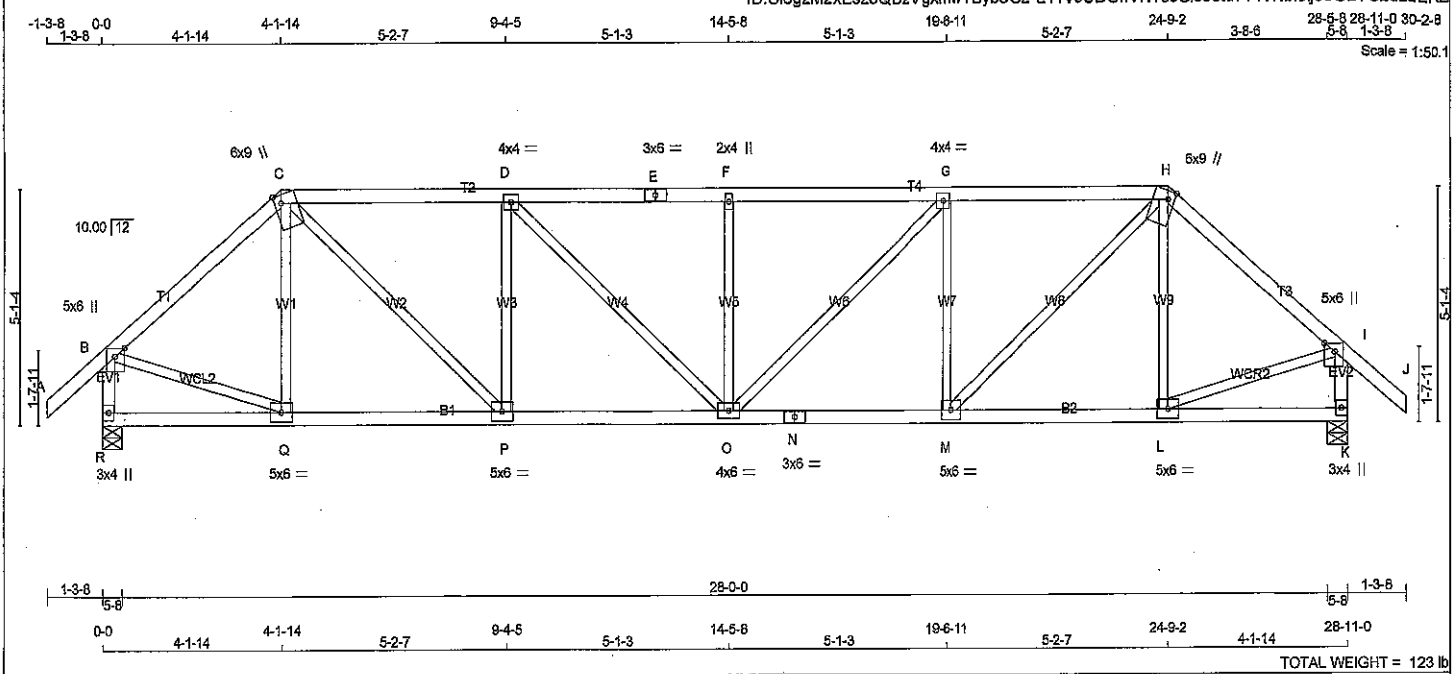

TOTAL WEIGHT = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2060	0	2060	0	0	5-8	5-8
K	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1884 / 0	-104.9	-104.9	0.40 (1)		4.47
C- D	-2588 / 0	-104.9	-104.9	0.55 (1)		3.74
D- E	-2922 / 0	-104.9	-104.9	0.58 (1)		3.51
E- F	-2922 / 0	-104.9	-104.9	0.58 (1)		3.51
F- G	-2922 / 0	-104.9	-104.9	0.58 (1)		3.51
G- H	-2588 / 0	-104.9	-104.9	0.55 (1)		3.74
H- I	-1884 / 0	-104.9	-104.9	0.40 (1)		4.47
I- J	0 / 47	-104.9	-104.9	0.14 (1)		10.00
R- B	-2016 / 0	0.0	0.0	0.21 (1)		5.95
K- I	-2016 / 0	0.0	0.0	0.21 (1)		5.95
R- Q	0 / 0	-27.5	-27.5	0.14 (3)		10.00
Q- P	0 / 1437	-27.5	-27.5	0.31 (1)		10.00
P- O	0 / 2588	-27.5	-27.5	0.48 (1)		10.00
O- N	0 / 2588	-27.5	-27.5	0.48 (1)		10.00
N- M	0 / 2588	-27.5	-27.5	0.48 (1)		10.00
M- L	0 / 1437	-27.5	-27.5	0.31 (1)		10.00
L- K	0 / 0	-27.5	-27.5	0.14 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
Q- C	-298 / 60	0.11 (1)
C- P	0 / 1564	0.35 (1)
P- D	-921 / 0	0.35 (1)
D- O	0 / 459	0.10 (1)
O- F	-493 / 0	0.19 (1)
O- G	0 / 459	0.10 (1)
M- G	-921 / 0	0.35 (1)
M- H	0 / 1564	0.35 (1)
L- H	-298 / 60	0.11 (1)
B- Q	0 / 1510	0.34 (1)
L- I	0 / 1510	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.58/1.00 (F-G:1) , BC=0.48/1.00 (O-P:1) , WB=0.35/1.00 (G-M:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13261-T
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T03	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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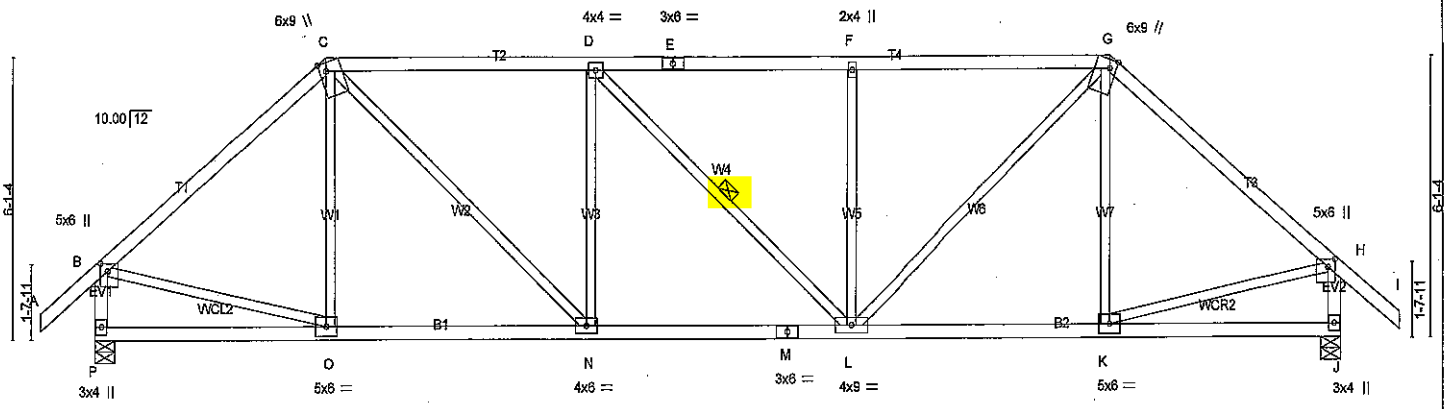

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TOTAL WEIGHT = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1909 / 0	-104.9	-104.9	0.70 (1)	4.06	
C- D	-2355 / 0	-104.9	-104.9	0.71 (1)	3.68	
D- E	-2354 / 0	-104.9	-104.9	0.70 (1)	3.68	
E- F	-2354 / 0	-104.9	-104.9	0.70 (1)	3.68	
F- G	-2354 / 0	-104.9	-104.9	0.70 (1)	3.69	
G- H	-1909 / 0	-104.9	-104.9	0.70 (1)	4.06	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
P- B	-2001 / 0	0.0	0.0	0.21 (1)	5.97	
J- H	-2001 / 0	0.0	0.0	0.21 (1)	5.97	
P- O	0 / 0	-27.5	-27.5	0.22 (3)	10.00	
O- N	0 / 1460	-27.5	-27.5	0.37 (2)	10.00	
N- M	0 / 2355	-27.5	-27.5	0.48 (1)	10.00	
M- L	0 / 2355	-27.5	-27.5	0.48 (1)	10.00	
L- K	0 / 1461	-27.5	-27.5	0.37 (2)	10.00	
K- J	0 / 0	-27.5	-27.5	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-172 / 155	0.10 (1)
C- N	0 / 1234	0.28 (1)
N- D	-687 / 0	0.40 (1)
D- L	-2 / 0	0.00 (1)
L- F	-686 / 0	0.40 (1)
L- G	0 / 1231	0.28 (1)
K- G	-171 / 155	0.10 (1)
B- O	0 / 1505	0.34 (1)
K- H	0 / 1505	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010.

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.48/1.00 (L-N:1) , WB=0.40/1.00 (D-N:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 132628-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T04	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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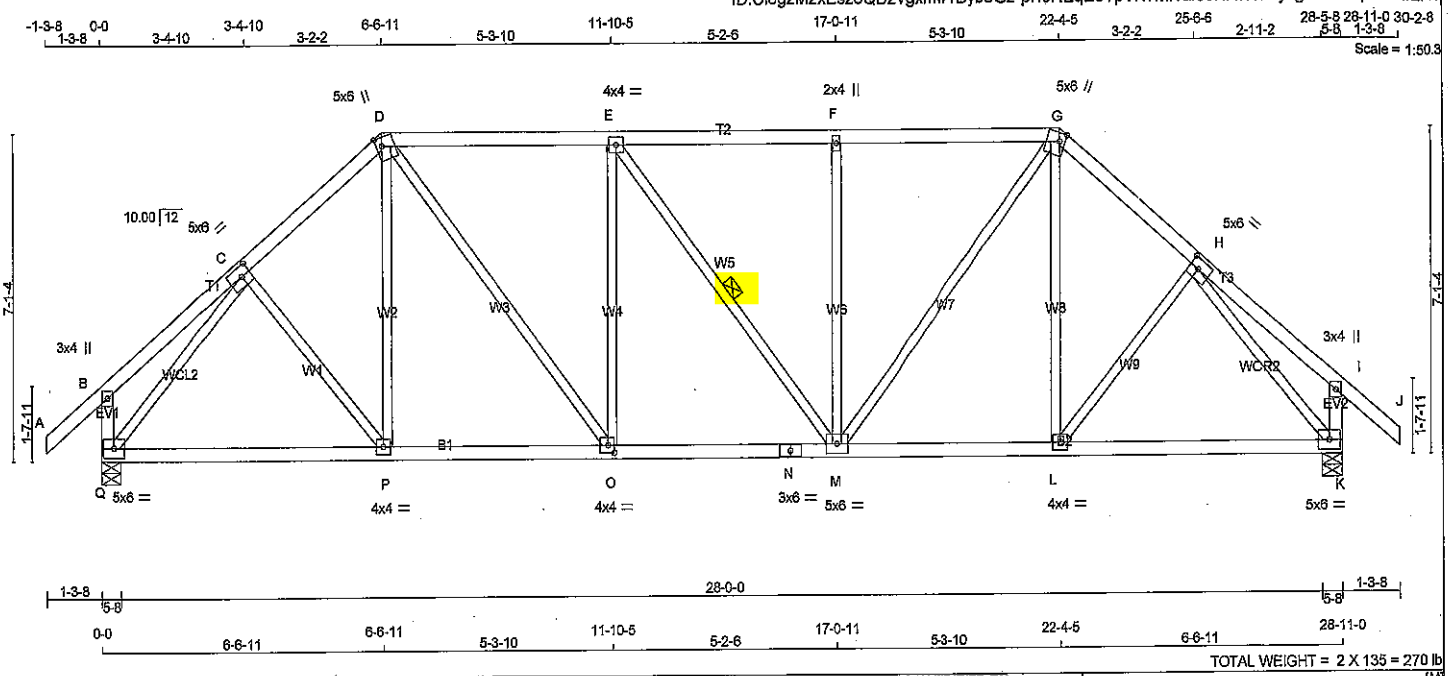

TOTAL WEIGHT = 2 X 135 = 270 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2060	0	2060	0	0	5-8	5-8
K	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00
C- D	-1910 / 0	-104.9	-104.9	0.21 (1)	4.67
D- E	-2014 / 0	-104.9	-104.9	0.49 (1)	4.22
E- F	-2013 / 0	-104.9	-104.9	0.49 (1)	4.22
F- G	-2012 / 0	-104.9	-104.9	0.48 (1)	4.24
G- H	-1910 / 0	-104.9	-104.9	0.21 (1)	4.67
H- I	0 / 23	-104.9	-104.9	0.16 (1)	10.00
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-280 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-280 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 1381	-27.5	-27.5	0.40 (2)	10.00
P- O	0 / 1447	-27.5	-27.5	0.42 (2)	10.00
O- N	0 / 2014	-27.5	-27.5	0.39 (1)	10.00
N- M	0 / 2014	-27.5	-27.5	0.39 (1)	10.00
M- L	0 / 1447	-27.5	-27.5	0.42 (2)	10.00
L- K	0 / 1381	-27.5	-27.5	0.40 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 147	0.03 (2)
P- D	0 / 224	0.05 (3)
D- O	0 / 922	0.21 (1)
O- E	-594 / 0	0.52 (1)
E- M	-3 / 0	0.00 (1)
M- F	-593 / 0	0.52 (1)
M- G	0 / 919	0.21 (1)
L- G	0 / 224	0.05 (3)
L- H	0 / 147	0.03 (2)
Q- C	-2194 / 0	0.94 (1)
H- K	-2194 / 0	0.94 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.42/1.00 (L-M:2) , WB=0.94/1.00 (H-K:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.54 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13263-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T05	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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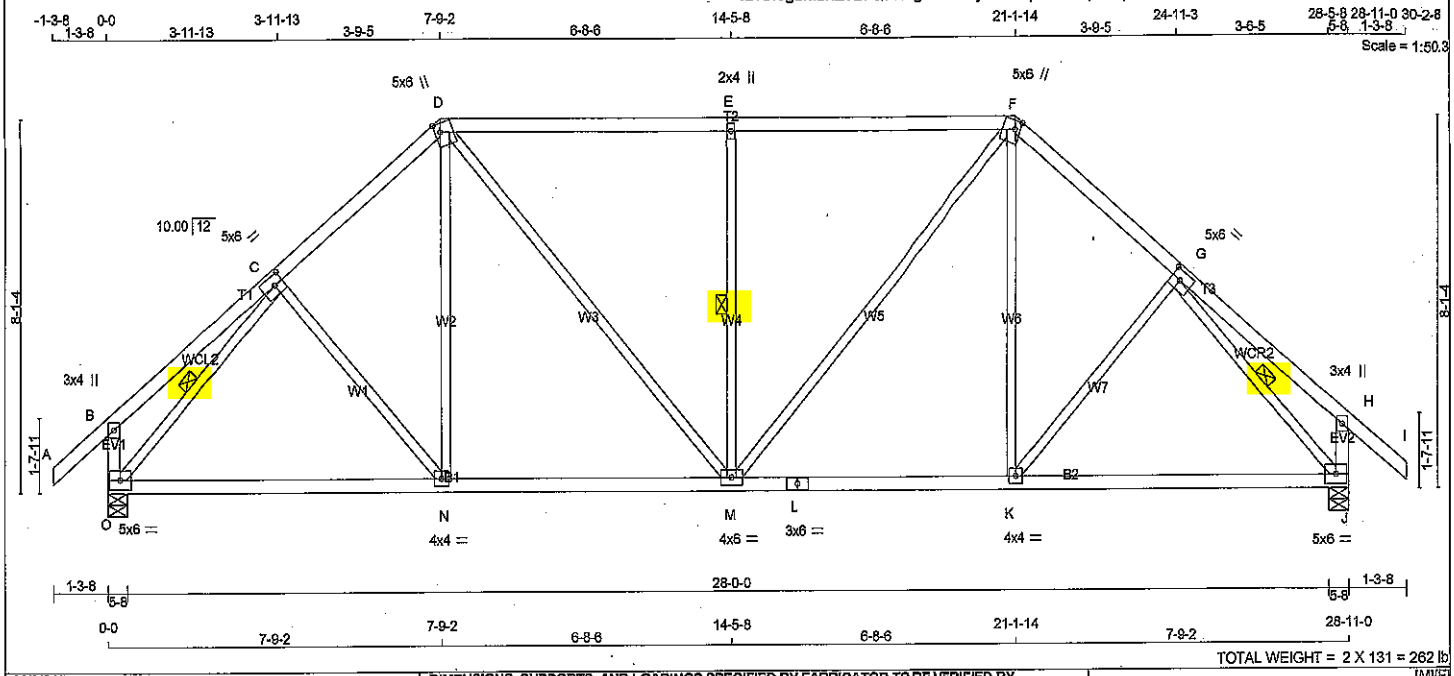

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	-17 / 113	0.03 (3)
B- C	0 / 29	-104.9	-104.9	0.24 (1)	10.00	N- D	0 / 342	0.08 (2)
C- D	-1875 / 0	-104.9	-104.9	0.23 (1)	4.71	D- M	0 / 667	0.15 (1)
D- E	-1852 / 0	-104.9	-104.9	0.67 (1)	4.04	M- E	-862 / 0	0.36 (1)
E- F	-1852 / 0	-104.9	-104.9	0.67 (1)	4.04	M- F	0 / 667	0.15 (1)
F- G	-1875 / 0	-104.9	-104.9	0.23 (1)	4.71	K- F	0 / 342	0.08 (2)
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	K- G	-17 / 113	0.03 (3)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-2196 / 0	0.55 (1)
O- B	-302 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-2196 / 0	0.55 (1)
J- H	-302 / 0	0.0	0.0	0.03 (1)	7.81			
O- N	0 / 1426	-27.5	-27.5	0.53 (2)	10.00			
N- M	0 / 1417	-27.5	-27.5	0.54 (2)	10.00			
M- L	0 / 1417	-27.5	-27.5	0.54 (2)	10.00			
L- K	0 / 1417	-27.5	-27.5	0.54 (2)	10.00			
K- J	0 / 1426	-27.5	-27.5	0.53 (2)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.54/1.00 (M-N:2) , WB=0.55/1.00 (C-O:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3078
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13264-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T06	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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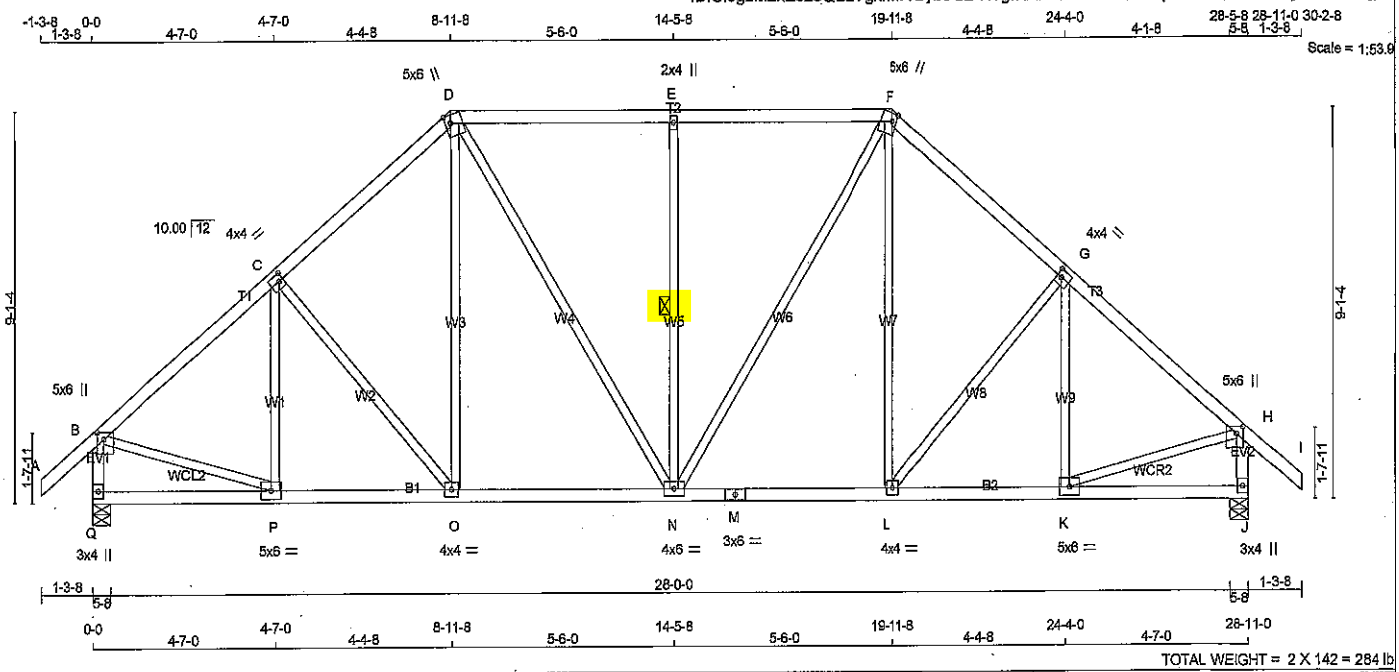

TOTAL WEIGHT = 2 X 142 = 284 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1916 / 0	-104.9	-104.9	0.32 (1)		4.57
C- D	-1799 / 0	-104.9	-104.9	0.31 (1)		4.69
D- E	-1621 / 0	-104.9	-104.9	0.44 (1)		4.69
E- F	-1621 / 0	-104.9	-104.9	0.44 (1)		4.69
F- G	-1799 / 0	-104.9	-104.9	0.31 (1)		4.69
G- H	-1916 / 0	-104.9	-104.9	0.32 (1)		4.57
H- I	0 / 47	-104.9	-104.9	0.14 (1)		10.00
Q- B	-2006 / 0	0.0	0.0	0.21 (1)		5.96
J- H	-2006 / 0	0.0	0.0	0.21 (1)		5.96
Q- P	0 / 0	-27.5	-27.5	0.13 (3)		10.00
P- O	0 / 1498	-27.5	-27.5	0.32 (1)		10.00
O- N	0 / 1354	-27.5	-27.5	0.31 (2)		10.00
N- M	0 / 1354	-27.5	-27.5	0.31 (2)		10.00
M- L	0 / 1354	-27.5	-27.5	0.31 (2)		10.00
L- K	0 / 1498	-27.5	-27.5	0.32 (1)		10.00
K- J	0 / 0	-27.5	-27.5	0.13 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
P- C	-295 / 60	0.13 (1)
C- O	-225 / 0	0.19 (1)
O- D	0 / 332	0.07 (2)
D- N	0 / 504	0.11 (1)
N- E	-703 / 0	0.38 (1)
N- F	0 / 504	0.11 (1)
L- F	0 / 332	0.07 (2)
L- G	-225 / 0	0.19 (1)
K- G	-295 / 60	0.13 (1)
B- P	0 / 1558	0.35 (1)
K- H	0 / 1558	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.32/1.00 (K-L:1) , WB=0.38/1.00 (E-N:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (H) (INPUT = 0.90)
JSI METAL= 0.42 (H) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 13265-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T07	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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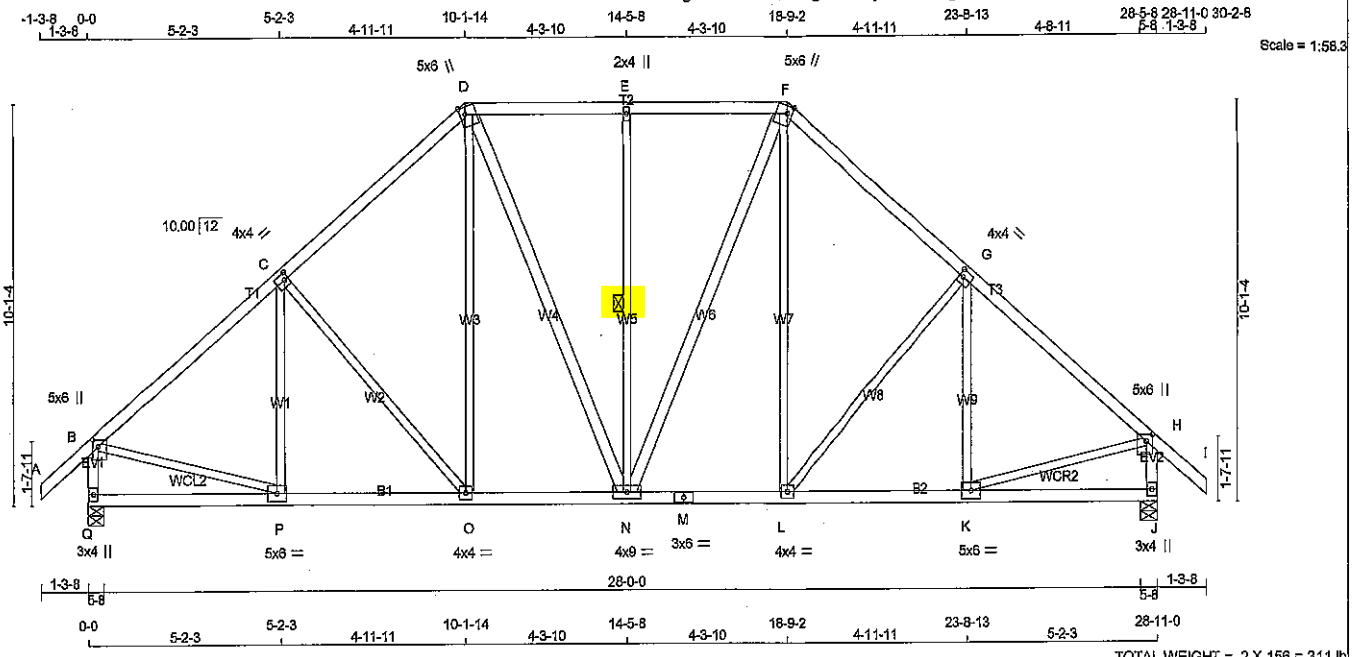

TOTAL WEIGHT = 2 X 156 = 311 lb

[MII]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1936 / 0	-104.9	-104.9	0.42 (1)	4.43
C- D	-1725 / 0	-104.9	-104.9	0.40 (1)	4.66
D- E	-1437 / 0	-104.9	-104.9	0.26 (1)	5.17
E- F	-1437 / 0	-104.9	-104.9	0.26 (1)	5.17
F- G	-1725 / 0	-104.9	-104.9	0.40 (1)	4.66
G- H	-1936 / 0	-104.9	-104.9	0.42 (1)	4.43
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-2001 / 0	0.0	0.0	0.21 (1)	5.97
J- H	-2001 / 0	0.0	0.0	0.21 (1)	5.97
Q- P	0 / 0	-27.5	-27.5	0.18 (3)	10.00
P- O	0 / 1518	-27.5	-27.5	0.33 (2)	10.00
O- N	0 / 1294	-27.5	-27.5	0.27 (1)	10.00
N- M	0 / 1294	-27.5	-27.5	0.27 (1)	10.00
M- L	0 / 1294	-27.5	-27.5	0.27 (1)	10.00
L- K	0 / 1518	-27.5	-27.5	0.33 (2)	10.00
K- J	0 / 0	-27.5	-27.5	0.18 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-224 / 121	0.12 (1)
C- O	-340 / 0	0.39 (1)
O- D	0 / 378	0.09 (1)
D- N	0 / 357	0.08 (1)
N- E	-542 / 0	0.38 (1)
N- F	0 / 357	0.08 (1)
L- F	0 / 378	0.09 (1)
L- G	-340 / 0	0.39 (1)
K- G	-224 / 121	0.12 (1)
B- P	0 / 1565	0.35 (1)
K- H	0 / 1565	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.33/1.00 (K-L:2) , WB=0.39/1.00 (C-O:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1326618
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T08	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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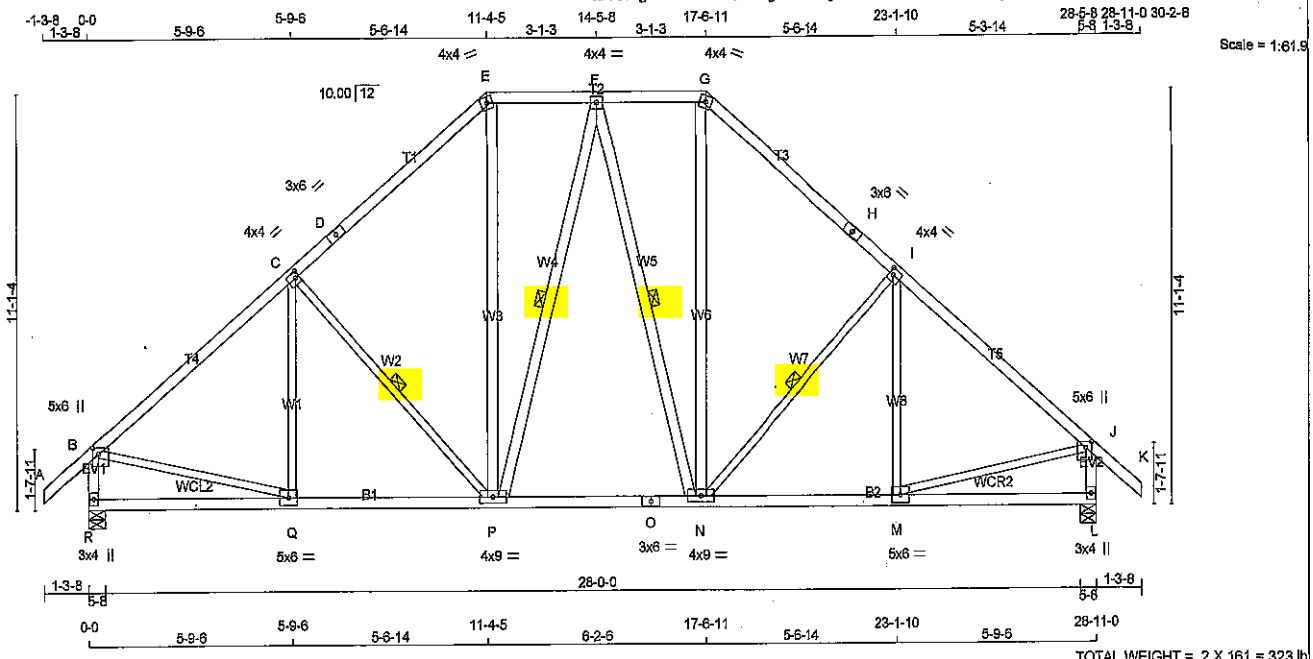

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2060	0	2060	0	0	5-8	5-8
L	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1943 / 0	-104.9	-104.9	0.54 (1)		4.27
C- D	-1645 / 0	-104.9	-104.9	0.50 (1)		4.60
D- E	-1645 / 0	-104.9	-104.9	0.50 (1)		4.60
E- F	-1234 / 0	-104.9	-104.9	0.12 (1)		5.66
F- G	-1234 / 0	-104.9	-104.9	0.12 (1)		5.66
G- H	-1645 / 0	-104.9	-104.9	0.50 (1)		4.60
H- I	-1645 / 0	-104.9	-104.9	0.50 (1)		4.60
I- J	-1943 / 0	-104.9	-104.9	0.54 (1)		4.27
J- K	0 / 47	-104.9	-104.9	0.14 (1)		10.00
R- B	-1994 / 0	0.0	0.0	0.21 (1)		5.97
L- J	-1994 / 0	0.0	0.0	0.21 (1)		5.97
R- Q	0 / 0	-27.5	-27.5	0.22 (3)		10.00
Q- P	0 / 1528	-27.5	-27.5	0.37 (2)		10.00
P- O	0 / 1285	-27.5	-27.5	0.31 (2)		10.00
O- N	0 / 1285	-27.5	-27.5	0.31 (2)		10.00
N- M	0 / 1528	-27.5	-27.5	0.37 (2)		10.00
M- L	0 / 0	-27.5	-27.5	0.22 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
Q- C	-171 / 163	0.12 (1)
C- P	-443 / 0	0.21 (1)
P- E	0 / 669	0.11 (1)
P- F	-194 / 0	0.13 (1)
F- N	-194 / 0	0.13 (1)
N- G	0 / 669	0.11 (1)
N- I	-443 / 0	0.21 (1)
M- I	-171 / 163	0.12 (1)
B- Q	0 / 1566	0.35 (1)
M- J	0 / 1566	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.54/1.00 (I-J:1) , BC=0.37/1.00 (M-N:2) , WB=0.35/1.00 (J-M:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.43 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3018
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13267-1K
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T09	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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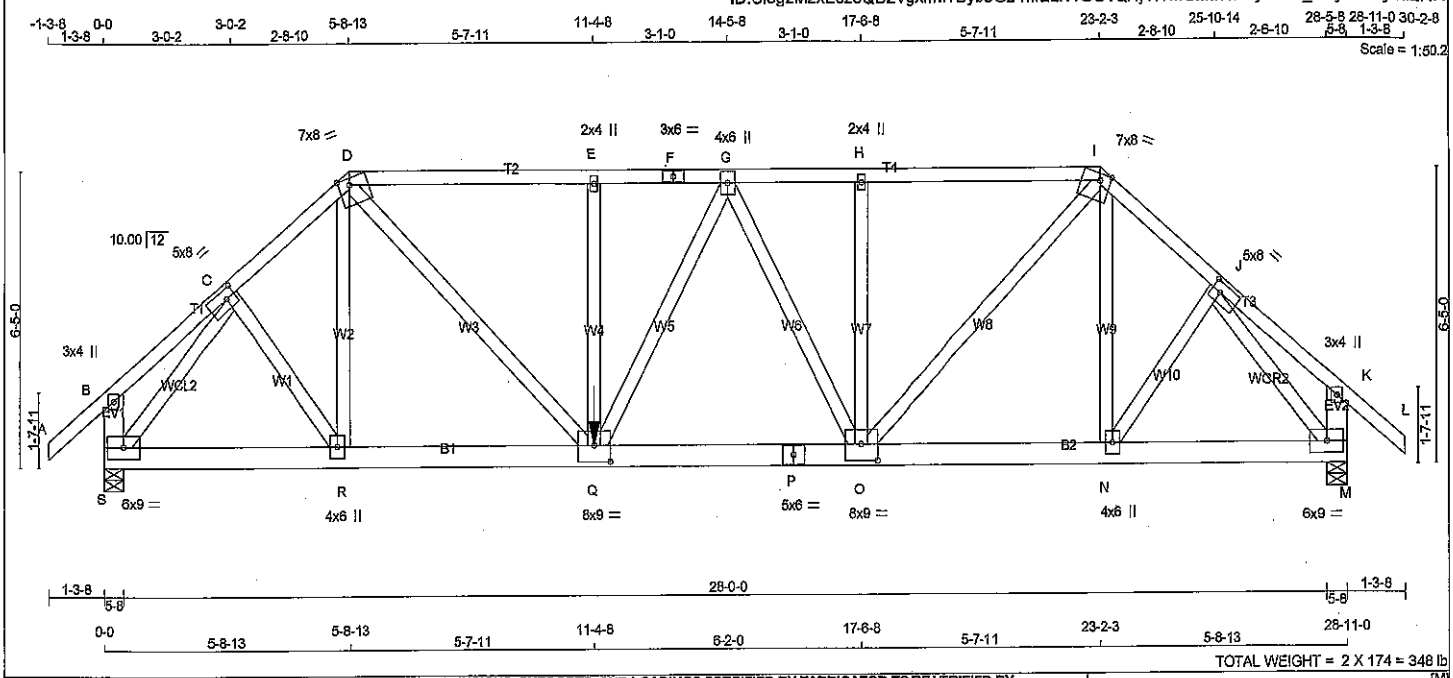


TOTAL WEIGHT = 2 X 174 = 348 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	TOP
D- F 1	12	TOP
F- I 1	12	TOP
I- L 1	12	TOP
S- B 2	12	TOP
M- K 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
S- P 2	12	SIDE(0.0)
P- M 2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.50
D	TTWW-m	MT20	7.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	7.0	8.0	Edge	3.00
J	TMWW-t	MT20	5.0	8.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWWW-t	MT20	8.0	9.0	4.25	4.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	8.0	9.0	4.25	4.50
R	BMWW+t	MT20	4.0	6.0		
S	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	5418	0	5418	0	0	5-8	5-8
M	4237	0	4237	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	4246	2709 / 0	757 / 0	0 / 0	0 / 0	781 / 0	0 / 0
M	3322	2118 / 0	592 / 0	0 / 0	0 / 0	611 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	C- R	0 / 1037	0.09 (1)
B- C	-3 / 12	-104.9	-104.9	0.06 (1)	10.00	R- D	-607 / 0	0.13 (1)
C- D	-6102 / 0	-104.9	-104.9	0.20 (1)	3.83	D- Q	0 / 5630	0.50 (1)
D- E	-8525 / 0	-104.9	-104.9	0.66 (1)	2.94	Q- E	-551 / 0	0.13 (1)
E- F	-8525 / 0	-104.9	-104.9	0.45 (1)	3.01	Q- G	0 / 2324	0.21 (1)
F- G	-8525 / 0	-104.9	-104.9	0.45 (1)	3.01	G- O	-2482 / 0	0.75 (1)
G- H	-6341 / 0	-104.9	-104.9	0.37 (1)	3.56	O- H	-562 / 0	0.13 (1)
H- I	-6341 / 0	-104.9	-104.9	0.46 (1)	3.55	O- I	0 / 4088	0.36 (1)
I- J	-4634 / 0	-104.9	-104.9	0.14 (1)	4.37	N- I	-408 / 0	0.09 (1)
J- K	-2 / 15	-104.9	-104.9	0.06 (1)	10.00	N- J	0 / 729	0.06 (1)
K- L	0 / 47	-104.9	-104.9	0.08 (1)	10.00	S- C	-6491 / 0	0.76 (1)
S- B	-277 / 0	0.0	0.0	0.01 (1)	7.81	J- M	-4980 / 0	0.58 (1)
M- K	-273 / 0	0.0	0.0	0.01 (1)	7.81			
S- R	0 / 4035	-27.5	-27.5	0.30 (1)	10.00			
R- Q	0 / 4676	-27.5	-27.5	0.36 (1)	10.00			
Q- P	0 / 7469	-27.5	-27.5	0.56 (1)	10.00			
P- O	0 / 7469	-27.5	-27.5	0.58 (1)	10.00			
O- N	0 / 3545	-27.5	-27.5	0.26 (1)	10.00			
N- M	0 / 3096	-27.5	-27.5	0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. TYPE	TOTAL
Q	11-4-8	-5535	-5535	---	BACK	VERT	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.66/1.00 (D-E:1) , BC=0.56/1.00 (O-Q:1) , WB=0.76/1.00 (C-S:1) , SSI=0.15/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

DWG NO. TAM 13268-B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

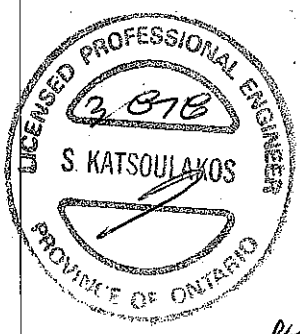
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T09	1	2	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5534.8 lbs FACTORED DOWN AT 11-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM13266-B

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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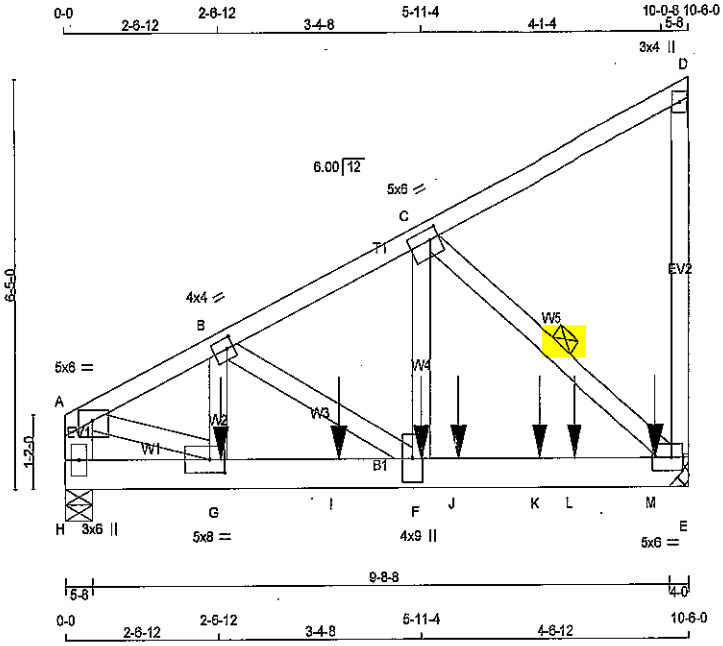

TOTAL WEIGHT = 2 X 58 = 117 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	2	12	TOP
A- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES *(table is in inches)*

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.00	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	5.0	6.0	2.50	2.25
F	BMWW+t	MT20	4.0	9.0		
G	BMWW-t	MT20	5.0	6.0	2.50	3.00
H	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2309.2 lbs FACTORED DOWN AT 2-7-4, 1253.0 lbs FACTORED DOWN AT 4-6-12, 1278.5 lbs FACTORED DOWN AT 5-11-4, 1253.0 lbs FACTORED DOWN AT 6-6-12, 717.4 lbs FACTORED DOWN AT 7-11-4, AND 1258.2 lbs FACTORED DOWN AT 8-6-12, AND 723.4 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	4621	0	4621	0	0	5-8	5-8
E	5562	0	5562	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	3657	2271 / 0	690 / 0	0 / 0	0 / 0	695 / 0	0 / 0
E	4370	2769 / 0	791 / 0	0 / 0	0 / 0	810 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-4414 / 0	0.0	0.0	0.16 (1)	6.84	A- G	0 / 5560	0.49 (1)
A- B	-5955 / 0	-104.9	-104.9	0.19 (1)	3.86	G- B	0 / 532	0.05 (1)
B- C	-5146 / 0	-104.9	-104.9	0.25 (1)	4.08	B- F	-812 / 0	0.08 (1)
C- D	-19 / 0	-104.9	-104.9	0.14 (1)	6.25	F- C	0 / 5589	0.49 (1)
E- D	-197 / 0	0.0	0.0	0.06 (1)	7.81	C- E	-5987 / 0	0.54 (1)
H- G	0 / 0	-27.5	-27.5	0.09 (1)	10.00			
G- I	0 / 5322	-27.5	-27.5	0.45 (1)	10.00			
I- F	0 / 5322	-27.5	-27.5	0.45 (1)	10.00			
F- J	0 / 4629	-27.5	-27.5	0.52 (1)	10.00			
J- K	0 / 4629	-27.5	-27.5	0.52 (1)	10.00			
K- L	0 / 4629	-27.5	-27.5	0.52 (1)	10.00			
L- M	0 / 4629	-27.5	-27.5	0.52 (1)	10.00			
M- E	0 / 4629	-27.5	-27.5	0.52 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	5-11-4	-1278	-1278	---	FRONT	VERT	TOTAL
G	2-7-4	-2309	-2309	---	BACK	VERT	TOTAL
I	4-6-12	-1253	-1253	---	BACK	VERT	TOTAL
J	6-6-12	-1253	-1253	---	BACK	VERT	TOTAL
K	7-11-4	-717	-717	---	FRONT	VERT	TOTAL
L	8-6-12	-1258	-1258	---	BACK	VERT	TOTAL
M	9-11-4	-723	-723	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.52/1.00 (E-F:1) , WB=0.54/1.00 (C-E:1) , SSI=0.83/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-8176
KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13269-R6
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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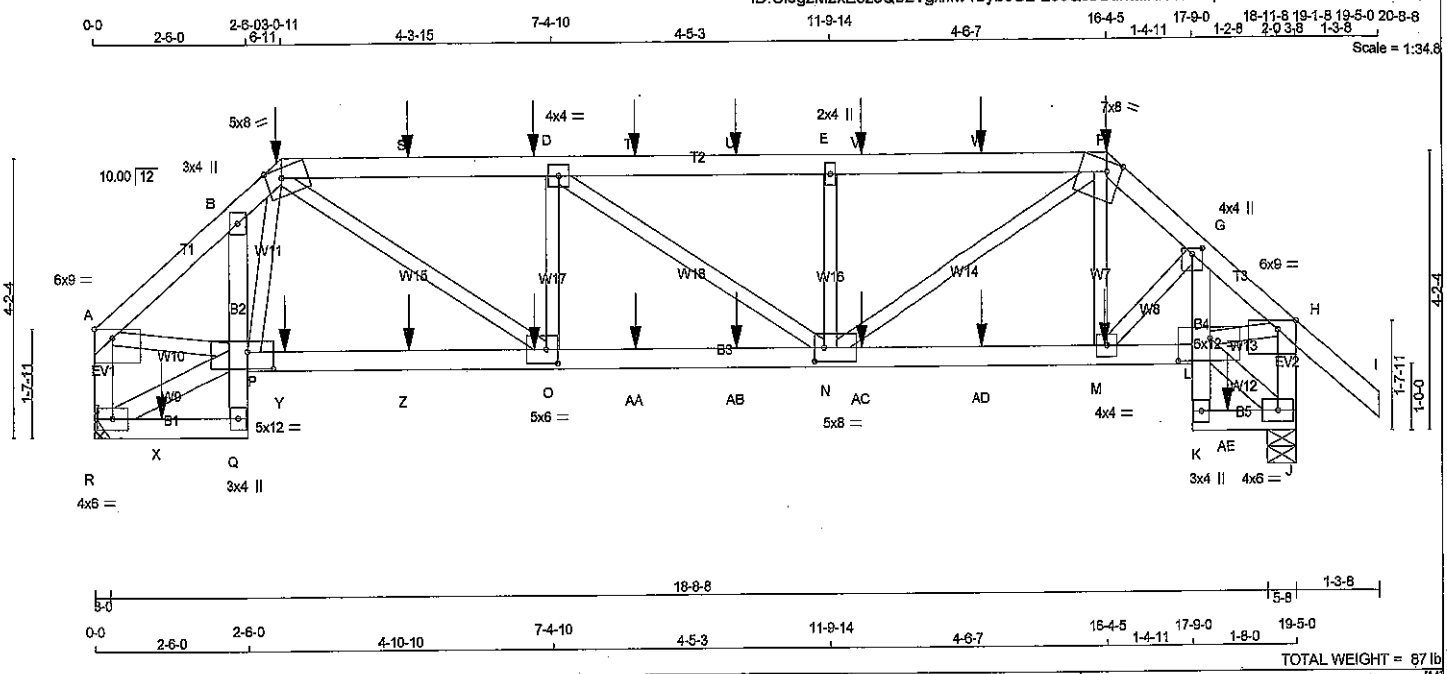

TOTAL WEIGHT = 87 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	1650F 1.5E	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	1.75	Edge
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	7.0	8.0	Edge	2.75
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMVW-p	MT20	6.0	9.0	Edge	3.50
J	BMVW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-I	MT20	6.0	12.0	4.00	6.25
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	1.75
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BVMWWW-I	MT20	5.0	12.0	3.00	5.00
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 137.3 lbs FACTORED DOWN AT 3-0-11, 133.7 lbs FACTORED DOWN AT 5-1-2, 133.7 lbs FACTORED DOWN AT 7-1-2, 133.7 lbs FACTORED DOWN AT 8-8-8, 133.7 lbs FACTORED DOWN AT 10-3-14, 133.7 lbs FACTORED DOWN AT 12-3-14, AND 133.7 lbs FACTORED DOWN AT 14-3-14, AND 137.3 lbs FACTORED DOWN AT 16-4-5 ON TOP CHORD, AND 70.8 lbs FACTORED DOWN AT 1-1-2, 121.5 lbs FACTORED DOWN AT 3-1-2, 118.0 lbs FACTORED DOWN AT 5-1-2, 118.0 lbs FACTORED DOWN AT 7-1-2, 118.0 lbs FACTORED DOWN AT 8-8-8, 118.0 lbs FACTORED DOWN AT 10-3-14, 118.0 lbs FACTORED DOWN AT 12-3-14, 118.0 lbs FACTORED DOWN AT 14-3-14, AND 118.0 lbs FACTORED DOWN AT 16-3-14, AND 84.5 lbs FACTORED DOWN AT 18-3-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2337	0	2337	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
J	2494	0	2494	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1880	1114 / 0	368 / 0	0 / 0	0 / 0	378 / 0	0 / 0
J	1988	1208 / 0	391 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2889 / 0	-104.9	-104.9	0.22 (1)	3.88
B- C	-2939 / 0	-104.9	-104.9	0.13 (1)	3.94
C- S	-4195 / 0	-104.9	-104.9	0.74 (1)	2.65
S- D	-4195 / 0	-104.9	-104.9	0.74 (1)	2.65
D- T	-4184 / 0	-104.9	-104.9	0.83 (1)	2.48
T- U	-4184 / 0	-104.9	-104.9	0.83 (1)	2.48
U- E	-4184 / 0	-104.9	-104.9	0.83 (1)	2.48
E- V	-4184 / 0	-104.9	-104.9	0.84 (1)	2.48
V- W	-4184 / 0	-104.9	-104.9	0.84 (1)	2.48
W- F	-4184 / 0	-104.9	-104.9	0.84 (1)	2.48
F- G	-2937 / 0	-104.9	-104.9	0.21 (1)	3.86
G- H	-2984 / 0	-104.9	-104.9	0.26 (1)	3.76
H- I	0 / 47	-104.9	-104.9	0.16 (1)	10.00
R- A	-2257 / 0	0.0	0.0	0.26 (1)	5.62
J- H	-2401 / 0	0.0	0.0	0.28 (1)	5.46
R- X	0 / 60	-27.5	-27.5	0.12 (2)	10.00
X- Q	0 / 60	-27.5	-27.5	0.12 (2)	10.00
Q- P	0 / 89	0.0	0.0	0.10 (1)	10.00
P- B	-215 / 0	0.0	0.0	0.09 (1)	7.81
P- Y	0 / 2199	-27.5	-27.5	0.51 (1)	10.00
Y- Z	0 / 2199	-27.5	-27.5	0.51 (1)	10.00
Z- O	0 / 2199	-27.5	-27.5	0.51 (1)	10.00
O-AA	0 / 4195	-27.5	-27.5	0.76 (1)	10.00
AA-AB	0 / 4195	-27.5	-27.5	0.76 (1)	10.00
AB- N	0 / 4195	-27.5	-27.5	0.76 (1)	10.00
N-AC	0 / 2265	-27.5	-27.5	0.44 (1)	10.00
AC-AD	0 / 2265	-27.5	-27.5	0.44 (1)	10.00
AD- M	0 / 2265	-27.5	-27.5	0.44 (1)	10.00
M- L	0 / 2308	-27.5	-27.5	0.44 (1)	10.00
K- L	0 / 97	0.0	0.0	0.14 (1)	10.00
L- G	-24 / 56	0.0	0.0	0.13 (1)	7.81
K-AE	0 / 76	-27.5	-27.5	0.06 (2)	10.00
AE- J	0 / 76	-27.5	-27.5	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- F	0 / 271	0.07 (2)
M- G	-62 / 0	0.01 (1)
R- P	-65 / 0	0.01 (1)
A- P	0 / 2235	0.55 (1)
P- C	0 / 173	0.04 (2)
L- J	-95 / 0	0.01 (1)
L- H	0 / 2250	0.56 (1)
N- F	0 / 2272	0.58 (1)
C- O	0 / 2396	0.59 (1)
N- E	-840 / 0	0.17 (1)
O- D	-819 / 0	0.16 (1)
D- N	-23 / 0	0.01 (3)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-0-11	-137	-137	—	BACK	VERT	TOTAL
D	7-1-2	-134	-134	—	BACK	VERT	TOTAL
F	16-4-5	-137	-137	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 866 (0.27")

CSI: TC=0.84/1.00 (E-F:1) , BC=0.76/1.00 (N-O:1) , WB=0.59/1.00 (C-O:1) , SSI=0.42/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (0) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3385 S. KATSOULAKOS PROVINCE OF ONTARIO

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DWG NO. TAM 13270-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:ClogzM2xEszoQB2Vgxmw1Byb8G2-EsoQssGurktuKEaUFztp2zdiruvLtTJkBzWU6OzdLKV

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	16-3-14	-118	-118	—	BACK	VERT	TOTAL
O	7-1-2	-118	-118	—	BACK	VERT	TOTAL
S	5-1-2	-134	-134	—	BACK	VERT	TOTAL
T	8-8-8	-134	-134	—	BACK	VERT	TOTAL
U	10-3-14	-134	-134	—	BACK	VERT	TOTAL
V	12-3-14	-134	-134	—	BACK	VERT	TOTAL
W	14-3-14	-134	-134	—	BACK	VERT	TOTAL
X	1-1-2	-41	-71	—	BACK	VERT	TOTAL
Y	3-1-2	-122	-122	—	BACK	VERT	TOTAL
Z	5-1-2	-118	-118	—	BACK	VERT	TOTAL
AA	8-8-8	-118	-118	—	BACK	VERT	TOTAL
AB	10-3-14	-118	-118	—	BACK	VERT	TOTAL
AC	12-3-14	-118	-118	—	BACK	VERT	TOTAL
AD	14-3-14	-118	-118	—	BACK	VERT	TOTAL
AE	18-3-14	-49	-84	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3678
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13210 PGE
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288690	T11A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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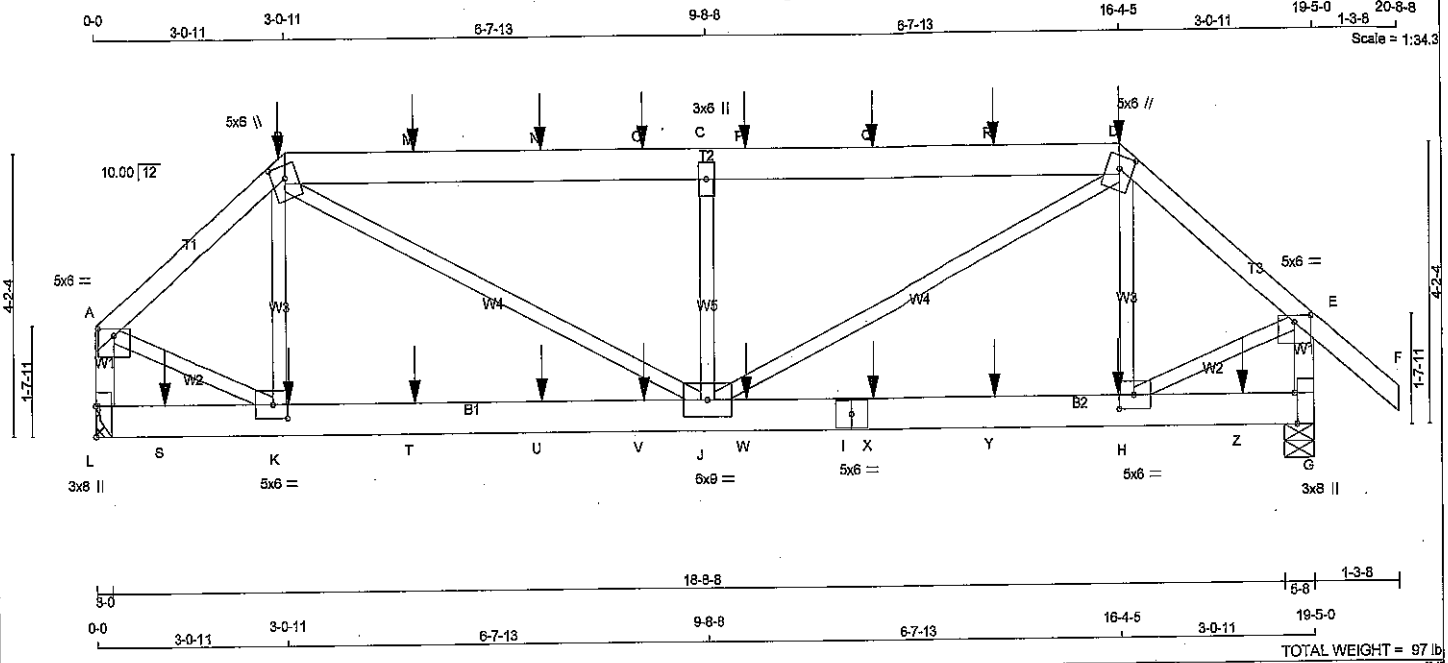

TOTAL WEIGHT = 97 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25	3.00
B	TTWW+m	MT20	5.0	6.0	2.25	2.50
C	TMW+w	MT20	3.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	2.50
E	TMVW-p	MT20	5.0	6.0	1.25	3.00
G	BMV1+t	MT20	3.0	8.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0	2.50	2.75
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	6.0	9.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+t	MT20	3.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 189.3 lbs FACTORED DOWN AT 3-0-11, 189.3 lbs FACTORED DOWN AT 16-4-5, 136.5 lbs FACTORED DOWN AT 3-0-11, 132.9 lbs FACTORED DOWN AT 5-1-2, 132.9 lbs FACTORED DOWN AT 7-1-2, 132.9 lbs FACTORED DOWN AT 8-8-8, 132.9 lbs FACTORED DOWN AT 10-3-14, 132.9 lbs FACTORED DOWN AT 12-3-14, AND 132.9 lbs FACTORED DOWN AT 14-3-14, AND 136.5 lbs FACTORED DOWN AT 16-4-5 ON TOP CHORD, AND 70.8 lbs FACTORED DOWN AT 1-1-2, 70.8 lbs FACTORED DOWN AT 3-1-2, 70.8 lbs FACTORED DOWN AT 5-1-2, 70.8 lbs FACTORED DOWN AT 7-1-2, 70.8 lbs FACTORED DOWN AT 8-8-8, 70.8 lbs FACTORED DOWN AT 10-3-14, 70.8 lbs FACTORED DOWN AT 12-3-14, 70.8 lbs FACTORED DOWN AT 14-3-14, AND 70.8 lbs FACTORED DOWN AT 16-3-14, AND 70.8 lbs FACTORED DOWN AT 18-3-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2218	0	2218	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
G	2357	0	2357	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1767	1054 / 0	372 / 0	0 / 0	0 / 0	361 / 0	0 / 0
G	1880	1141 / 0	371 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2142 / 0	-104.9	-104.9	0.26 (1)	4.36
B- M	-3211 / 0	-104.9	-104.9	0.72 (1)	3.72
M- N	-3211 / 0	-104.9	-104.9	0.72 (1)	3.72
N- O	-3211 / 0	-104.9	-104.9	0.72 (1)	3.72
O- C	-3211 / 0	-104.9	-104.9	0.72 (1)	3.72
C- P	-3212 / 0	-104.9	-104.9	0.72 (1)	3.72
P- Q	-3212 / 0	-104.9	-104.9	0.72 (1)	3.72
Q- R	-3212 / 0	-104.9	-104.9	0.72 (1)	3.72
R- D	-3212 / 0	-104.9	-104.9	0.72 (1)	3.72
D- E	-2133 / 0	-104.9	-104.9	0.26 (1)	4.38
E- F	0 / 47	-104.9	-104.9	0.16 (1)	10.00
L- A	-2207 / 0	0.0	0.0	0.25 (1)	5.67
G- E	-2344 / 0	0.0	0.0	0.27 (1)	5.52
L- S	0 / 0	-27.5	-27.5	0.17 (2)	10.00
S- K	0 / 0	-27.5	-27.5	0.17 (2)	10.00
K- T	0 / 1661	-27.5	-27.5	0.36 (2)	10.00
T- U	0 / 1661	-27.5	-27.5	0.36 (2)	10.00
U- V	0 / 1661	-27.5	-27.5	0.36 (2)	10.00
V- J	0 / 1661	-27.5	-27.5	0.36 (2)	10.00
J- W	0 / 1654	-27.5	-27.5	0.36 (2)	10.00
W- I	0 / 1654	-27.5	-27.5	0.36 (2)	10.00
I- X	0 / 1654	-27.5	-27.5	0.36 (2)	10.00
X- Y	0 / 1654	-27.5	-27.5	0.36 (2)	10.00
Y- H	0 / 1654	-27.5	-27.5	0.36 (2)	10.00
H- Z	0 / 0	-27.5	-27.5	0.17 (2)	10.00
Z- G	0 / 0	-27.5	-27.5	0.17 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-374 / 157	0.09 (1)
B- J	0 / 1779	0.44 (1)
J- C	-1442 / 0	0.36 (1)
J- D	0 / 1787	0.44 (1)
H- D	-376 / 151	0.10 (1)
A- K	0 / 1786	0.44 (1)
H- E	0 / 1779	0.44 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
B	3-0-11	-189	-189	---	FRONT	VERT	TOTAL
B	3-0-11	-136	-136	---	TOP	VERT	TOTAL
D	16-4-5	-189	-189	---	FRONT	VERT	TOTAL
D	16-4-5	-136	-136	---	TOP	VERT	TOTAL
H	16-3-14	-41	-71	---	BACK	VERT	TOTAL
K	3-1-2	-41	-71	---	BACK	VERT	TOTAL
M	5-1-2	-133	-133	---	TOP	VERT	TOTAL
N	7-1-2	-133	-133	---	TOP	VERT	TOTAL
O	8-8-8	-133	-133	---	TOP	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.72/1.00 (C-D:1) , BC=0.36/1.00 (J-K:2) , WB=0.44/1.00 (D-J:1) , SSI=0.47/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 132B5-18
STRUCTURAL COMPONENT ONLY

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288690	T11A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:ClogzM2xEszoQB2Vgxmw1Byb8G2-j2Ev_LgrSg0SolRTolmA57au54qLX5QctxJCbUzdL4U

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	10-3-14	-133	-133	--	TOP	VERT	TOTAL
Q	12-3-14	-133	-133	--	TOP	VERT	TOTAL
R	14-3-14	-133	-133	--	TOP	VERT	TOTAL
S	1-1-2	-41	-71	--	BACK	VERT	TOTAL
T	5-1-2	-41	-71	--	BACK	VERT	TOTAL
U	7-1-2	-41	-71	--	BACK	VERT	TOTAL
V	8-8-8	-41	-71	--	BACK	VERT	TOTAL
W	10-3-14	-41	-71	--	BACK	VERT	TOTAL
X	12-3-14	-41	-71	--	BACK	VERT	TOTAL
Y	14-3-14	-41	-71	--	BACK	VERT	TOTAL
Z	18-3-14	-41	-71	--	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3876
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13285-B
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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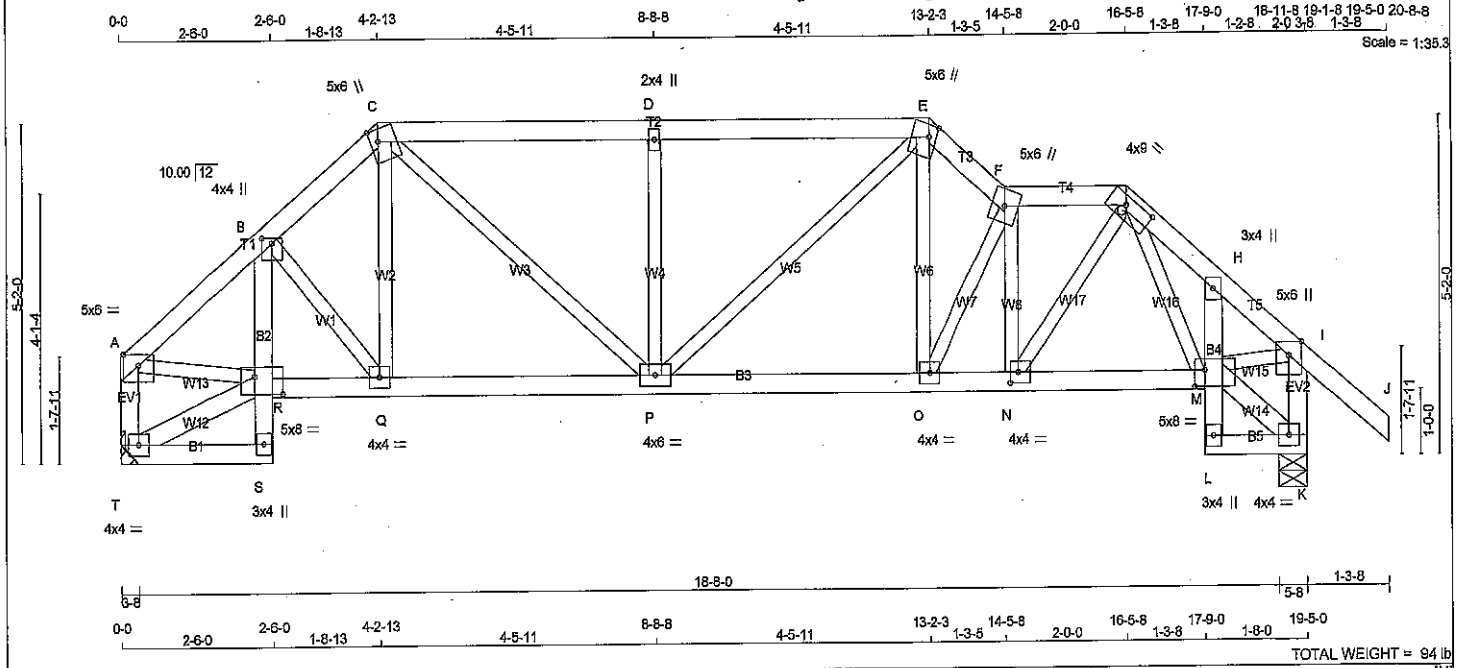


TOTAL WEIGHT = 94 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTWW+m	MT20	5.0	6.0		
G	TTWW-h	MT20	4.0	9.0	1.50	5.50
H	TMV+p	MT20	3.0	4.0		
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	5.0	8.0	3.00	2.00
N	BMWW-t	MT20	4.0	4.0	2.00	1.50
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BVMWW-I	MT20	5.0	8.0	3.25	5.50
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1280	0	1280	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
K	1436	0	1436	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
K	1121	724 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

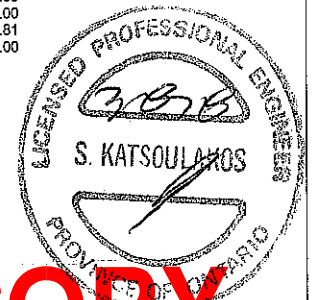
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1569 / 0	-104.9	-104.9	0.11 (1)	5.17
B- C	-1458 / 0	-104.9	-104.9	0.07 (1)	5.36
C- D	-1654 / 0	-104.9	-104.9	0.39 (1)	4.69
D- E	-1654 / 0	-104.9	-104.9	0.39 (1)	4.69
E- F	-1801 / 0	-104.9	-104.9	0.06 (1)	4.95
F- G	-1848 / 0	-104.9	-104.9	0.10 (1)	5.08
G- H	-1386 / 0	-104.9	-104.9	0.08 (1)	5.45
H- I	-1418 / 0	-104.9	-104.9	0.16 (1)	5.30
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
T- A	-1234 / 0	0.0	0.0	0.13 (1)	7.23
K- I	-1389 / 0	0.0	0.0	0.15 (1)	6.90
T- S	0 / 31	-27.5	-27.5	0.05 (2)	10.00
S- R	0 / 56	0.0	0.0	0.05 (1)	10.00
R- B	-37 / 43	0.0	0.0	0.05 (1)	7.81
R- Q	0 / 1228	-27.5	-27.5	0.24 (1)	10.00
Q- P	0 / 1110	-27.5	-27.5	0.23 (1)	10.00
P- O	0 / 1402	-27.5	-27.5	0.28 (1)	10.00
O- N	0 / 1676	-27.5	-27.5	0.30 (1)	10.00
N- M	0 / 1112	-27.5	-27.5	0.23 (1)	10.00
L- M	0 / 36	0.0	0.0	0.06 (1)	10.00
M- H	-80 / 0	0.0	0.0	0.05 (1)	7.81
L- K	0 / 40	-27.5	-27.5	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- Q	-187 / 0	0.04 (1)
Q- C	0 / 216	0.05 (2)
C- P	0 / 719	0.16 (1)
P- D	-562 / 0	0.15 (1)
P- E	0 / 333	0.07 (1)
O- E	0 / 680	0.15 (1)
O- F	-660 / 0	0.13 (1)
N- F	-734 / 0	0.13 (1)
T- R	-35 / 0	0.00 (1)
A- R	0 / 1218	0.27 (1)
M- K	-47 / 0	0.01 (1)
M- I	0 / 1086	0.24 (1)
G- M	-98 / 7	0.02 (1)
N- G	0 / 954	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

.

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.30/1.00 (N-O:1) , WB=0.27/1.00 (A-R:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)

DWG NO. TAM 13271-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288690	T12A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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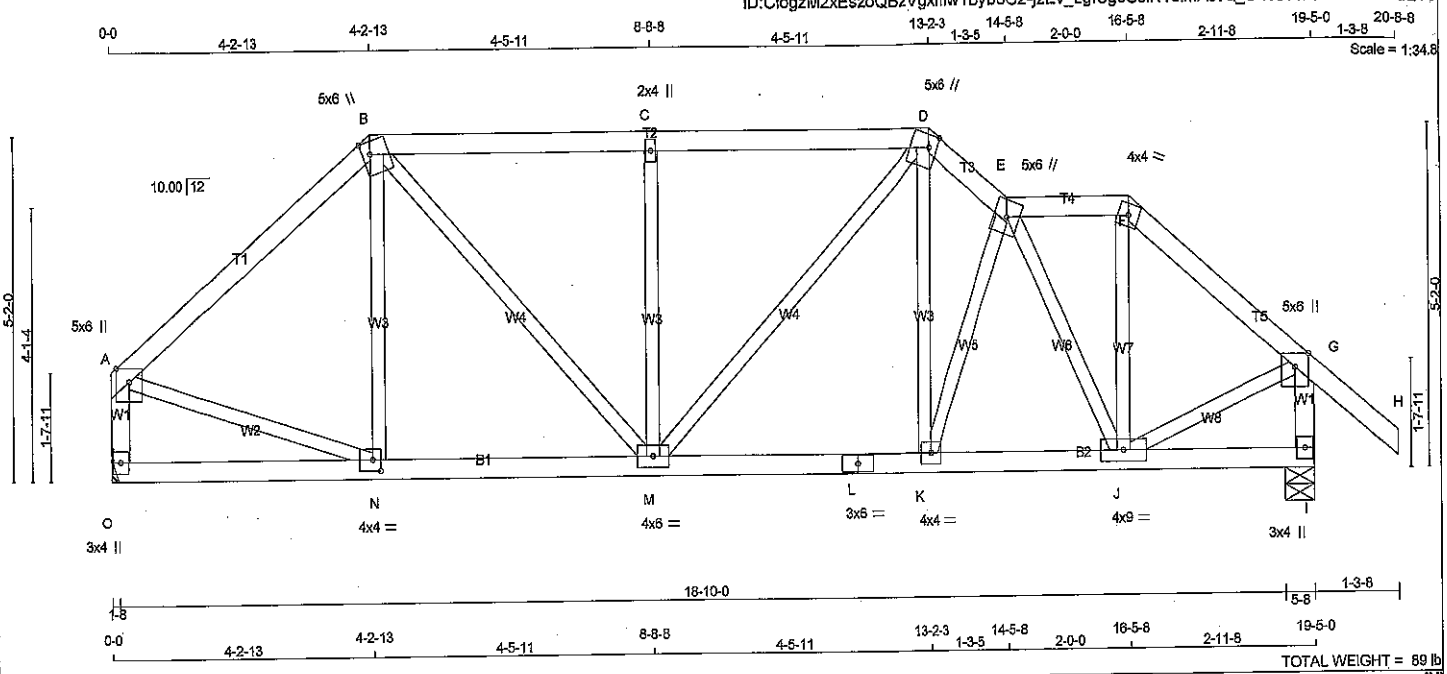

TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTWW+m	MT20	5.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG iN-SX	REQRD BRG IN-SX
O	1266	0	1266	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1431	0	1431	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1019	631 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	1117	721 / 0	194 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		(PLF)			
A- B	-1184 / 0	-104.9	-104.9	0.36 (1)	5.44
B- C	-1324 / 0	-104.9	-104.9	0.38 (1)	5.14
C- D	-1324 / 0	-104.9	-104.9	0.38 (1)	5.14
D- E	-1456 / 0	-104.9	-104.9	0.05 (1)	5.39
E- F	-676 / 0	-104.9	-104.9	0.08 (1)	6.25
F- G	-1128 / 0	-104.9	-104.9	0.18 (1)	5.77
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00
O- A	-1238 / 0	0.0	0.0	0.13 (1)	7.22
I- G	-1394 / 0	0.0	0.0	0.15 (1)	6.90
O- N	0 / 0	-27.5	-27.5	0.12 (3)	10.00
N- M	0 / 889	-27.5	-27.5	0.22 (2)	10.00
M- L	0 / 1128	-27.5	-27.5	0.23 (1)	10.00
L- K	0 / 1128	-27.5	-27.5	0.23 (1)	10.00
K- J	0 / 1249	-27.5	-27.5	0.24 (1)	10.00
J- I	0 / 0	-27.5	-27.5	0.05 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- B	-132 / 117	0.05 (1)
B- M	0 / 642	0.14 (1)
M- C	-567 / 0	0.22 (1)
M- D	0 / 289	0.06 (1)
K- D	0 / 467	0.11 (1)
K- E	-361 / 0	0.11 (1)
E- J	-633 / 0	0.25 (1)
J- F	0 / 466	0.10 (1)
A- N	0 / 931	0.21 (1)
J- G	0 / 938	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.24/1.00 (J-K:1) , WB=0.25/1.00 (E-J:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.33 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1328638
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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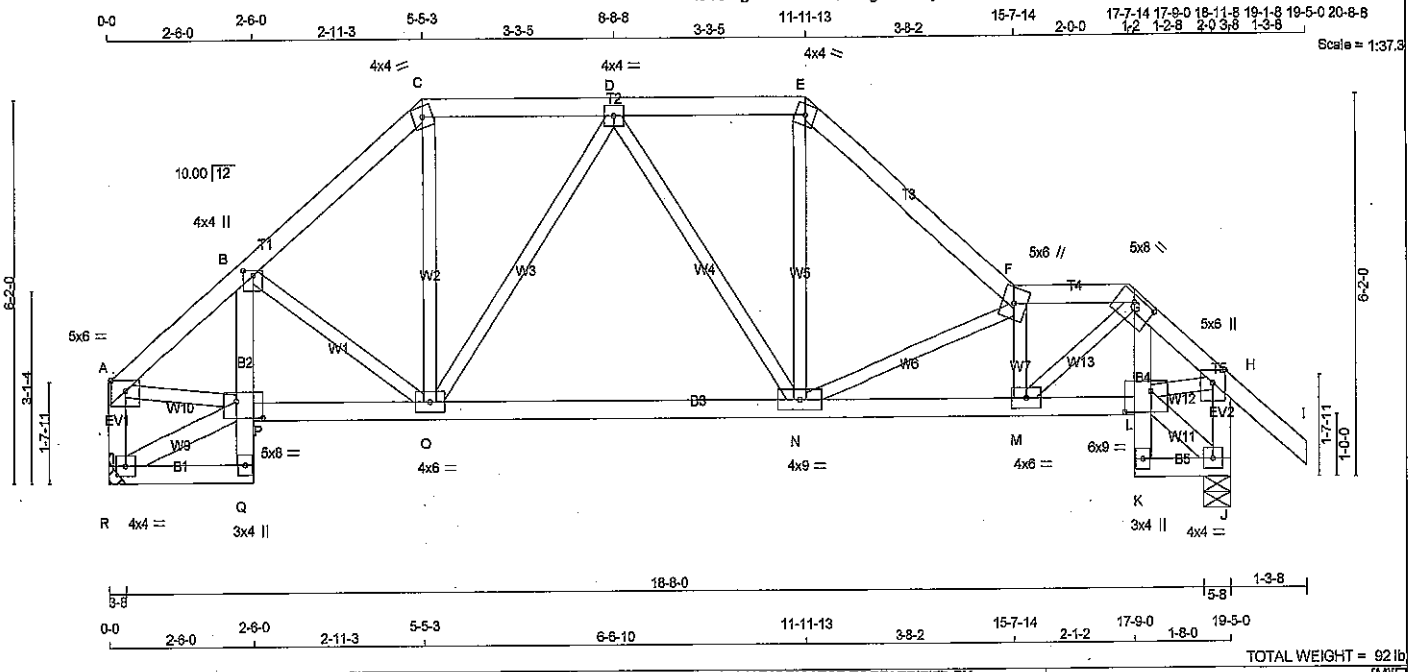

TOTAL WEIGHT = 92 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTVW-h	MT20	5.0	8.0	1.25	4.50
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BVMWW-l	MT20	5.0	8.0	3.25	5.50
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1280	0	1280	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1436	0	1436	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	1121	724 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1582 / 0	-104.9	-104.9	0.11 (1)	5.16		B- O	-252 / 0	0.07 (1)
B- C	-1376 / 0	-104.9	-104.9	0.14 (1)	5.41		O- C	0 / 597	0.13 (1)
C- D	-1054 / 0	-104.9	-104.9	0.14 (1)	5.98		O- D	-395 / 0	0.23 (1)
D- E	-1223 / 0	-104.9	-104.9	0.14 (1)	5.65		D- N	-86 / 55	0.05 (1)
E- F	-1573 / 0	-104.9	-104.9	0.21 (1)	5.08		N- E	0 / 672	0.15 (1)
F- G	-2057 / 0	-104.9	-104.9	0.11 (1)	4.65		N- F	-998 / 0	0.30 (1)
G- H	-1489 / 0	-104.9	-104.9	0.13 (1)	5.26		M- F	-910 / 0	0.14 (1)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00		R- P	-35 / 0	0.00 (1)
R- A	-1234 / 0	0.0	0.0	0.13 (1)	7.23		A- P	0 / 1237	0.28 (1)
J- H	-1389 / 0	0.0	0.0	0.15 (1)	6.90		L- J	-43 / 0	0.00 (1)
							L- H	0 / 1045	0.24 (1)
R- Q	0 / 32	-27.5	-27.5	0.05 (2)	10.00		M- G	0 / 1316	0.30 (1)
Q- P	0 / 56	0.0	0.0	0.05 (1)	10.00				
P- B	-46 / 50	0.0	0.0	0.04 (1)	7.81				
P- O	0 / 1247	-27.5	-27.5	0.31 (2)	10.00				
O- N	0 / 1270	-27.5	-27.5	0.32 (2)	10.00				
N- M	0 / 2110	-27.5	-27.5	0.45 (1)	10.00				
M- L	0 / 1068	-27.5	-27.5	0.22 (1)	10.00				
K- L	0 / 38	0.0	0.0	0.06 (1)	10.00				
L- G	0 / 68	0.0	0.0	0.06 (1)	10.00				
K- J	0 / 34	-27.5	-27.5	0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.45/1.00 (M-N:1) , WB=0.30/1.00 (F-N:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (P) (INPUT = 0.90)
JSI METAL= 0.39 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13272-07
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288690	T13A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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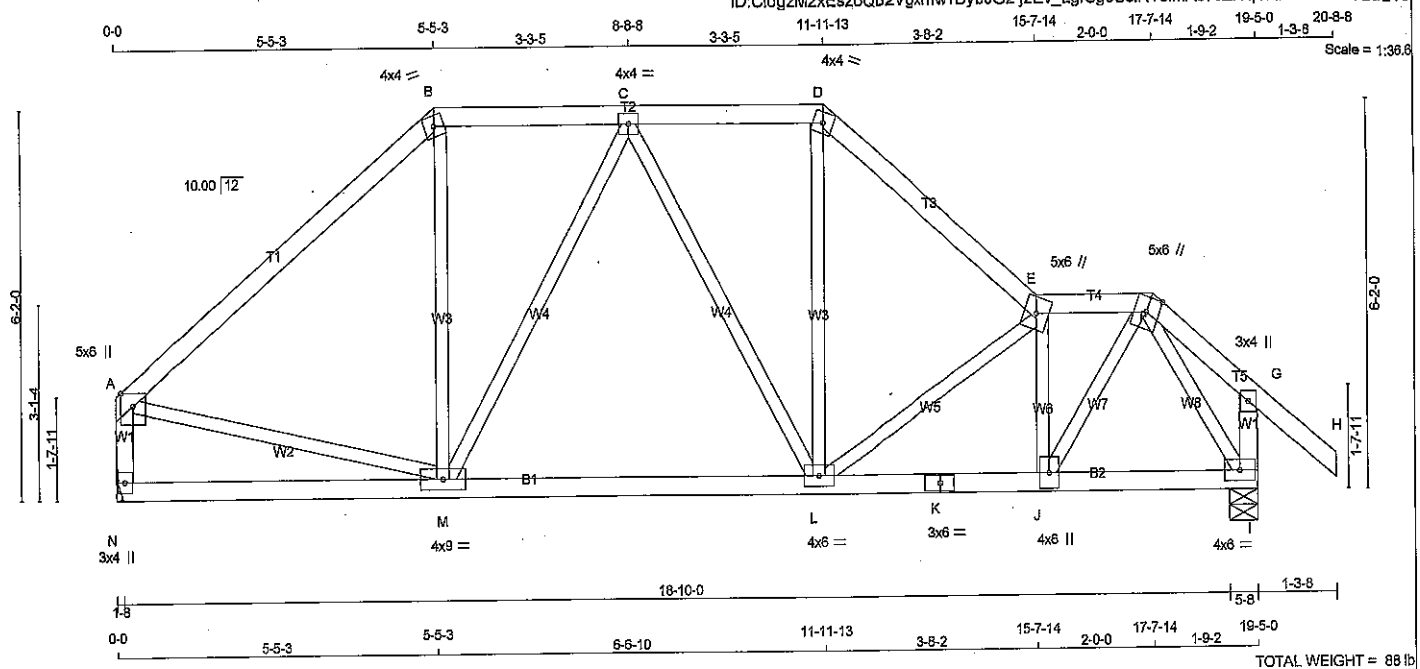

TOTAL WEIGHT = 88 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TTWW+m	MT20	5.0	6.0	Edge	
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1280	0	1280	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1436	0	1436	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	1121	724 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-1141 / 0	-104.9	-104.9	0.43 (1)	5.41	M- B	0 / 334	0.08 (2)
B- C	-883 / 0	-104.9	-104.9	0.14 (1)	6.25	M- C	-368 / 0	0.30 (1)
C- D	-1011 / 0	-104.9	-104.9	0.14 (1)	6.07	C- L	-94 / 37	0.08 (1)
D- E	-1304 / 0	-104.9	-104.9	0.20 (1)	5.44	L- D	0 / 502	0.11 (1)
E- F	-1338 / 0	-104.9	-104.9	0.07 (1)	5.55	L- E	-458 / 0	0.17 (1)
F- G	-36 / 0	-104.9	-104.9	0.11 (1)	6.25	J- E	-874 / 0	0.16 (1)
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	A- M	0 / 898	0.20 (1)
N- A	-1221 / 0	0.0	0.0	0.13 (1)	7.25	F- I	-1300 / 0	0.27 (1)
I- G	-296 / 0	0.0	0.0	0.03 (1)	7.81	J- F	0 / 1156	0.26 (1)
N- M	0 / 0	-27.5	-27.5	0.24 (3)	10.00			
M- L	0 / 1055	-27.5	-27.5	0.35 (2)	10.00			
L- K	0 / 1370	-27.5	-27.5	0.31 (1)	10.00			
K- J	0 / 1370	-27.5	-27.5	0.31 (1)	10.00			
J- I	0 / 704	-27.5	-27.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.0	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.35/1.00 (L-M:2) , WB=0.30/1.00 (C-M:1) , SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13287-19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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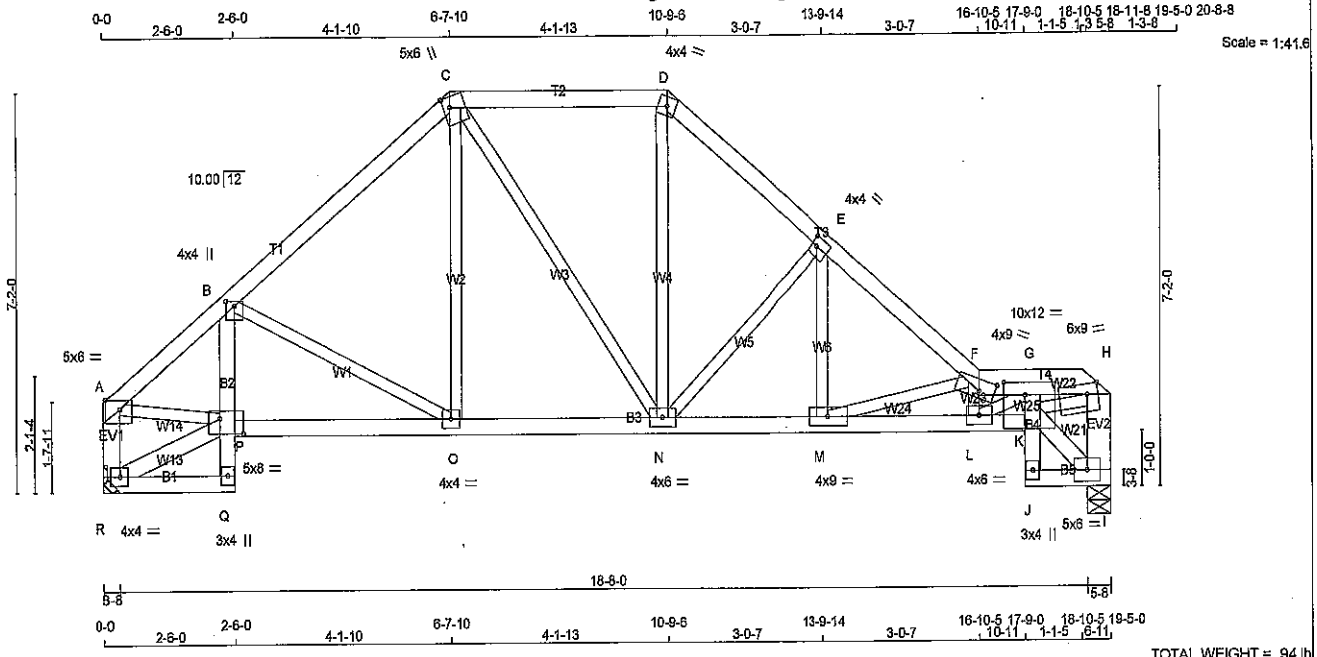

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTWW-m	MT20	4.0	9.0	2.50	3.50
G						
G	TMBWWWW*-I	MT20	10.0	12.0	2.75	5.00
H	TMVW+w	MT20	6.0	9.0	2.50	2.25
I	BMVW1-t	MT20	5.0	6.0		
J	BMV+p	MT20	3.0	4.0		
K						
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	9.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWW-I	MT20	5.0	8.0	3.25	5.50
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1286	0	1286	0	0	IN-SX	IN-SX
R	1286	0	1286	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1019	631 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
R	1019	631 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1615 / 0	-104.9	-104.9	0.19 (1)	5.02	
B- C	-1282 / 0	-104.9	-104.9	0.25 (1)	5.43	
C- D	-1055 / 0	-104.9	-104.9	0.24 (1)	5.84	
D- E	-1389 / 0	-104.9	-104.9	0.14 (1)	5.40	
E- F	-2066 / 0	-104.9	-104.9	0.16 (1)	4.59	
F- G	-3765 / 0	-104.9	-104.9	0.27 (1)	4.19	
G- H	-2288 / 0	-104.9	-104.9	0.23 (1)	5.18	
R- A	-1238 / 0	0.0	0.0	0.13 (1)	7.22	
I- H	-1140 / 0	0.0	0.0	0.08 (1)	7.81	
R- Q	0 / 33	-27.5	-27.5	0.05 (2)	10.00	
Q- P	0 / 56	0.0	0.0	0.05 (1)	10.00	
P- B	-31 / 95	0.0	0.0	0.04 (2)	7.81	
P- O	0 / 1290	-27.5	-27.5	0.28 (1)	10.00	
O- N	0 / 969	-27.5	-27.5	0.23 (2)	10.00	
N- M	0 / 1599	-27.5	-27.5	0.32 (1)	10.00	
M- L	0 / 3582	-27.5	-27.5	0.81 (1)	10.00	
L- K	0 / 2720	-27.5	-27.5	0.67 (1)	10.00	
J- K	0 / 36	0.0	0.0	0.26 (1)	10.00	
K- G	-250 / 0	0.0	0.0	0.20 (1)	7.81	
J- I	0 / 158	-27.5	-27.5	0.04 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- O	-367 / 0	0.15 (1)
O- C	0 / 310	0.07 (2)
C- N	0 / 152	0.03 (1)
N- D	0 / 537	0.12 (1)
N- E	-803 / 0	0.27 (1)
M- E	0 / 656	0.15 (1)
R- P	-36 / 0	0.00 (1)
A- P	0 / 1280	0.29 (1)
K- I	-201 / 0	0.02 (1)
K- H	0 / 2420	0.54 (1)
L- F	-1373 / 0	0.19 (1)
M- F	-2042 / 0	0.40 (1)
L- G	0 / 1330	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.27/1.00 (F-G:1) , BC=0.81/1.00 (L-M:1) , WB=0.54/1.00 (H-K:1) , SSI=0.28/1.00 (G-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.51 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3826
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13223 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288690	T14A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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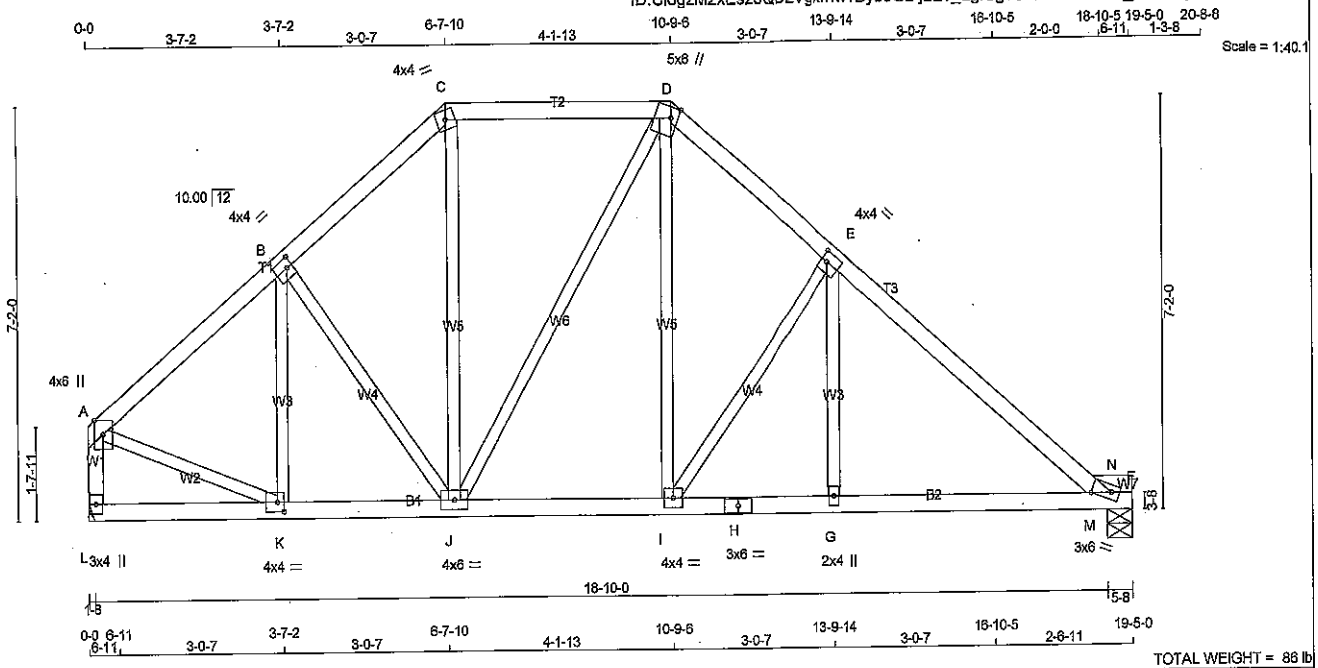

TOTAL WEIGHT = 86 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TBMW1-m	MT20	3.0	6.0	Edge	4.25
G	BMW+w	MT20	2.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1270	0	1270	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
O(F)	1250	0	1250	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1007	623 / 0	192 / 0	0 / 0	0 / 0	192 / 0	0 / 0
O(F)	991	613 / 0	189 / 0	0 / 0	0 / 0	189 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O(F)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1146 / 0	-104.9	-104.9	0.16 (1)	5.77
B- C	-1084 / 0	-104.9	-104.9	0.15 (1)	5.90
C- D	-813 / 0	-104.9	-104.9	0.24 (1)	6.25
D- E	-1183 / 0	-104.9	-104.9	0.26 (1)	5.57
E- N	-1566 / 0	-104.9	-104.9	0.34 (1)	4.91
N- F	-1543 / 0	-104.9	-104.9	0.09 (3)	5.27
L- A	-1226 / 0	0.0	0.0	0.13 (1)	7.24
L- K	0 / 0	-27.5	-27.5	0.08 (2)	10.00
K- J	0 / 899	-27.5	-27.5	0.20 (1)	10.00
J- I	0 / 876	-27.5	-27.5	0.20 (1)	10.00
I- H	0 / 1229	-27.5	-27.5	0.39 (1)	10.00
H- G	0 / 1229	-27.5	-27.5	0.39 (1)	10.00
G- M	0 / 1229	-27.5	-27.5	0.60 (1)	10.00
M- F	0 / 1229	-27.5	-27.5	0.64 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-236 / 36	0.07 (1)
B- J	-154 / 0	0.07 (1)
J- C	0 / 348	0.08 (1)
J- D	-123 / 0	0.16 (1)
I- D	0 / 535	0.12 (1)
I- E	-613 / 0	0.29 (1)
G- E	0 / 281	0.06 (2)
A- K	0 / 956	0.22 (1)
M- N	-311 / 184	0.00 (1)
M- O	0 / 0	0.84 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.34/1.00 (E-N:1) , BC=0.64/1.00 (F-M:1) , WB=0.84/1.00 (M-O:1) , SSI=0.98/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 13288-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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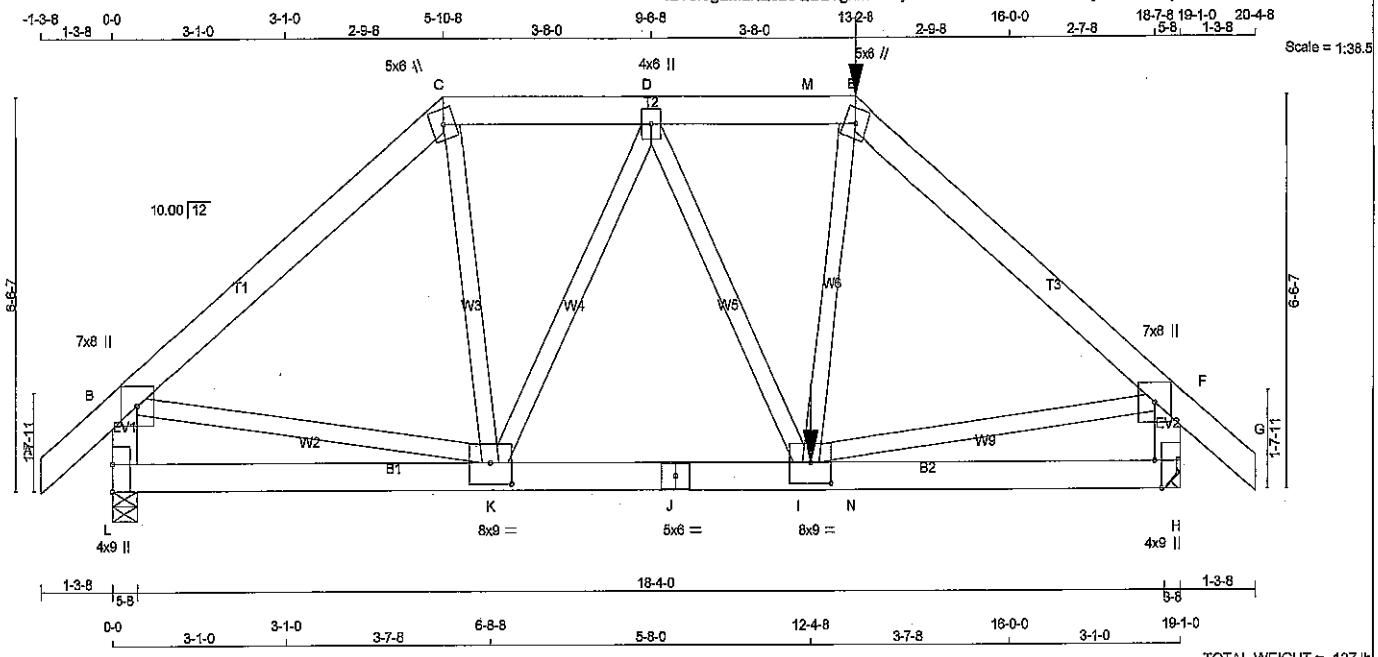

TOTAL WEIGHT = 127 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	7.0	6.0		
C	TTW+m	MT20	5.0	6.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW+m	MT20	5.0	6.0		
F	TMVW+p	MT20	7.0	8.0		
H	BMV1+t	MT20	4.0	9.0	Edge	1.50
I	BMWWW-t	MT20	8.0	9.0	4.25	4.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	8.0	9.0	4.25	4.50
L	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 449.5 lbs FACTORED DOWN AT 13-2-8 ON TOP CHORD, AND 2016.9 lbs FACTORED DOWN AT 12-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2331	0	2331	0	0	5-8	5-8
H	3232	0	3232	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1825	1168 / 0	323 / 0	0 / 0	0 / 0	334 / 0	0 / 0
H	2567	1579 / 0	495 / 0	0 / 0	0 / 0	493 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 48	-104.9	-104.9	0.09 (1)	10.00
B- C	-2331 / 0	-104.9	-104.9	0.40 (1)	4.91
C- D	-1938 / 0	-104.9	-104.9	0.11 (1)	5.64
D- M	-2775 / 0	-104.9	-104.9	0.15 (1)	4.86
M- E	-2775 / 0	-206.6	-206.6	0.15 (1)	4.86
E- F	-3393 / 0	-104.9	-104.9	0.45 (1)	4.16
F- G	0 / 48	-104.9	-104.9	0.09 (1)	10.00
L- B	-2250 / 0	0.0	0.0	0.16 (1)	6.78
H- F	-3066 / 0	0.0	0.0	0.22 (1)	5.98
L- K	0 / 0	-27.5	-27.5	0.15 (2)	10.00
K- J	0 / 2450	-27.5	-27.5	0.42 (2)	10.00
J- I	0 / 2450	-27.5	-27.5	0.42 (2)	10.00
I- N	0 / 0	-54.1	-54.1	0.30 (2)	10.00
N- H	0 / 0	-54.1	-54.1	0.30 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	0 / 1809	0.32 (1)
C- K	0 / 1022	0.18 (1)
K- D	-1240 / 0	0.70 (1)
D- I	0 / 788	0.14 (1)
I- E	0 / 1174	0.21 (1)
I- F	0 / 2634	0.47 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	13-2-8	-450	-450	---	FRONT	VERT	TOTAL
I	12-4-8	-2019	-2019	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 6-8-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.45/1.00 (E-F:1) , BC=0.42/1.00 (I-K:2) , WB=0.70/1.00 (D-K:1) , SSI=0.24/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13274-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

DWG NO. TAM 13224-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T16	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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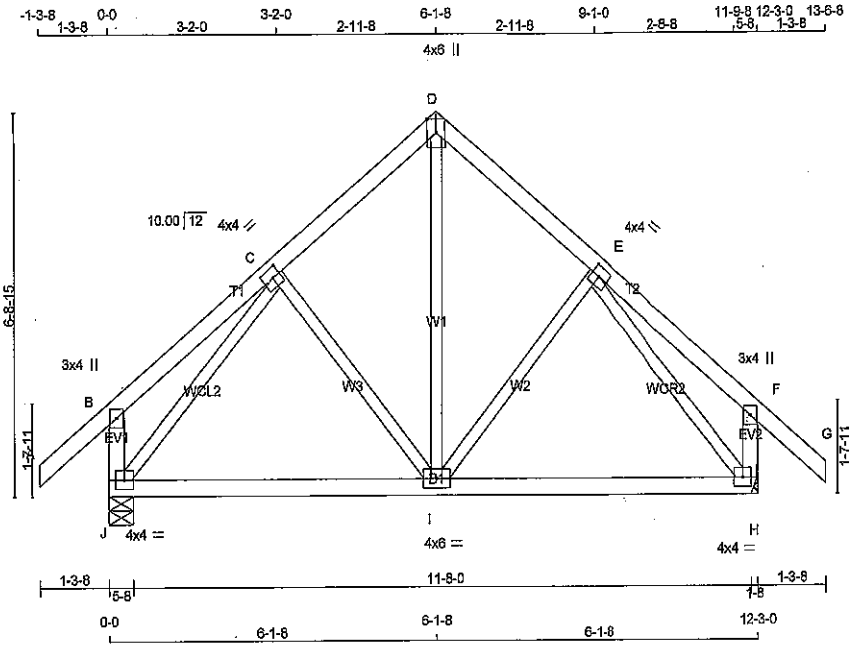

TOTAL WEIGHT = 3 X 59 = 177 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00	
C- D	-552 / 0	-104.9	-104.9	0.12 (1)	6.25	
D- E	-552 / 0	-104.9	-104.9	0.12 (1)	6.25	
E- F	0 / 23	-104.9	-104.9	0.15 (1)	10.00	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- B	-268 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-268 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 494	-27.5	-27.5	0.34 (2)	10.00	
I- H	0 / 494	-27.5	-27.5	0.34 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 424	0.10 (1)
I- E	-139 / 37	0.06 (1)
C- I	-139 / 37	0.06 (1)
J- C	-795 / 0	0.30 (1)
E- H	-795 / 0	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.34/1.00 (I-J:2) , WB=0.30/1.00 (E-H:1) , SSI=0.14/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 13275-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T17	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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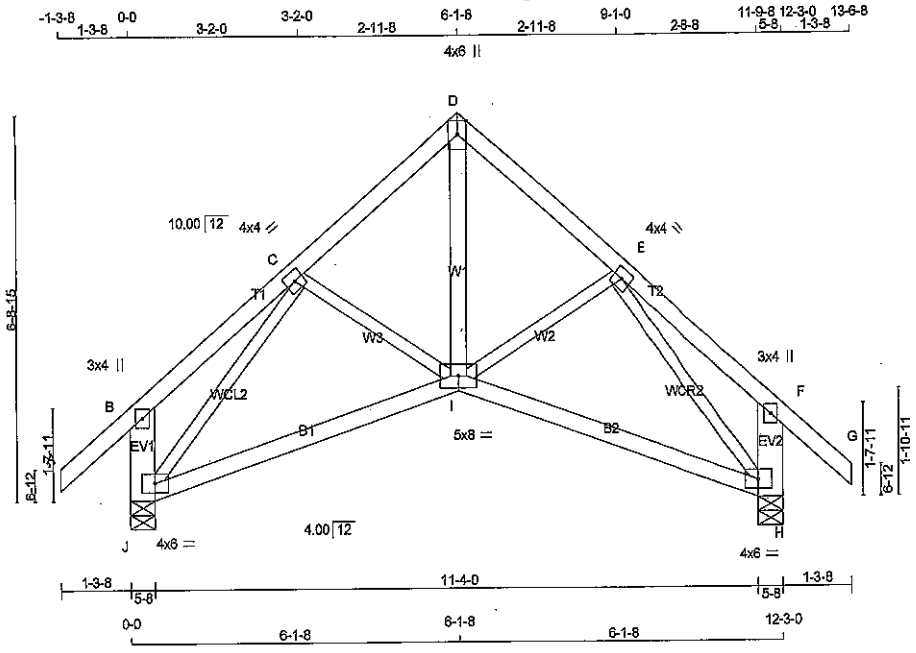

TOTAL WEIGHT = 2 X 60 = 119 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	4.0	6.0		
I	BBWWW-p	MT20	5.0	8.0		
J	BVMW1-p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	468 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	468 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00	I-D	0 / 663	0.11 (1)
B-C	0 / 22	-104.9	-104.9	1	0.15 (1)	10.00	I-E	-78 / 51	0.02 (1)
C-D	-739 / 0	-104.9	-104.9	1	0.12 (1)	6.25	C-I	-78 / 51	0.02 (1)
D-E	-739 / 0	-104.9	-104.9	1	0.12 (1)	6.25	J-C	-1015 / 0	0.37 (1)
E-F	0 / 22	-104.9	-104.9	1	0.15 (1)	10.00	E-H	-1015 / 0	0.37 (1)
F-G	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00			
J-B	-271 / 0	0.0	0.0	1	0.02 (1)	7.81			
H-F	-271 / 0	0.0	0.0	1	0.02 (1)	7.81			
J-I	0 / 650	-27.5	-27.5	2	0.36 (2)	10.00			
I-H	0 / 650	-27.5	-27.5	2	0.36 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 883 (0.17")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.36/1.00 (H-I:2) , WB=0.37/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 1327a18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T18	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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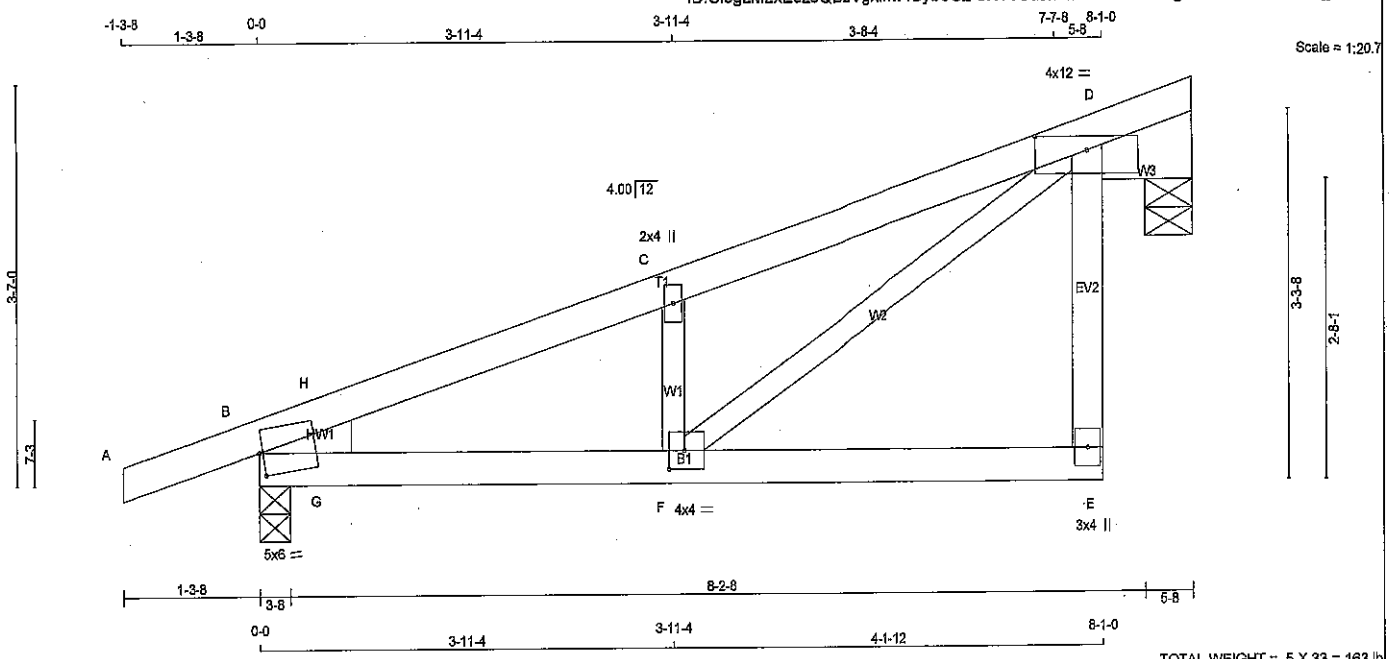

TOTAL WEIGHT = 5 X 33 = 163 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMH1-m	MT20	5.0	6.0	2.50	0.50
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	12.0	1.50	6.00
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	535	0	535	0	0	5-8	5-8	
B	676	0	676	0	0	3-8	3-8	2x4 L

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	424	263 / 0	81 / 0	0 / 0	0 / 0	81 / 0	0 / 0
B	519	349 / 0	81 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A-B	0 / 8	-104.9	-104.9	0.13 (1)	10.00	F-C	-448 / 0	0.07 (1)
B-H	-933 / 0	-104.9	-104.9	0.07 (1)	6.25	F-D	0 / 938	0.21 (1)
H-C	-806 / 0	-104.9	-104.9	0.16 (1)	6.25	G-H	0 / 125	0.00 (1)
C-D	-812 / 0	-104.9	-104.9	0.22 (1)	6.25			
E-D	0 / 85	0.0	0.0	0.01 (2)	10.00			
B-G	0 / 773	-27.5	-27.5	0.16 (1)	10.00			
G-F	0 / 773	-27.5	-27.5	0.16 (2)	10.00			
F-E	0 / 0	-27.5	-27.5	0.10 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.16/1.00 (F-G:2) , WB=0.21/1.00 (D-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (F) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13277-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T19	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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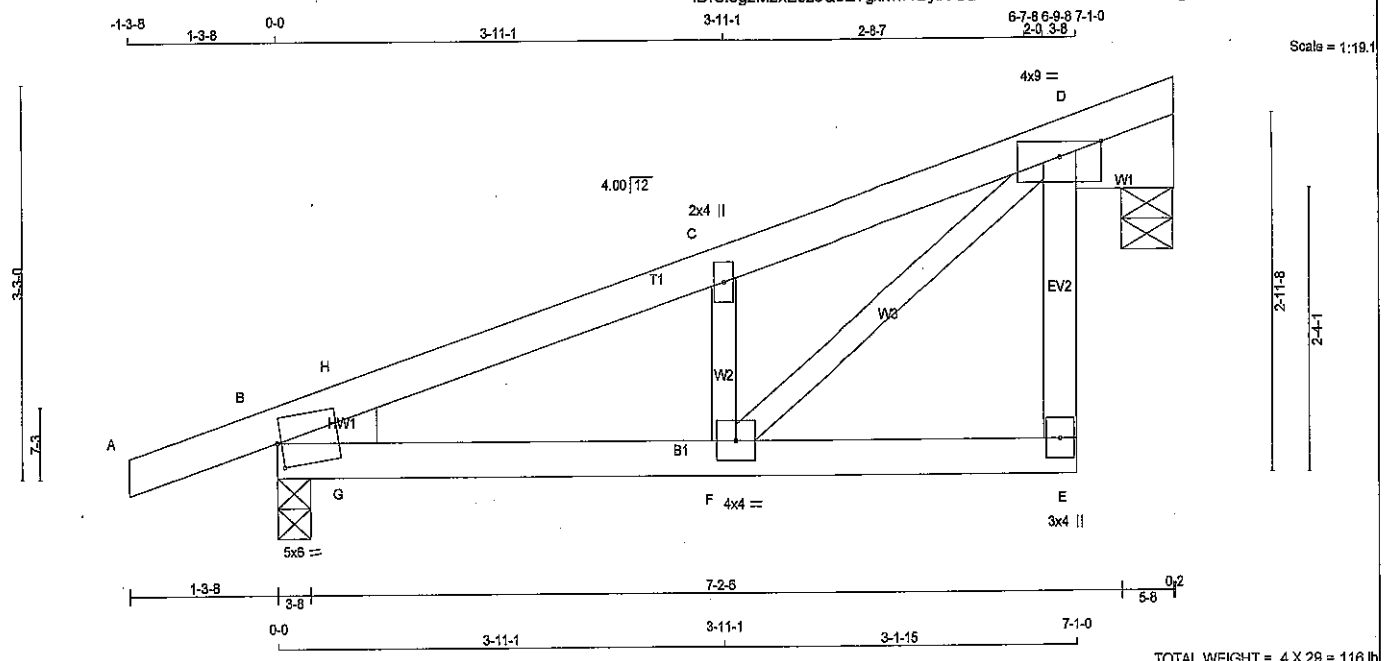

TOTAL WEIGHT = 4 X 29 = 116 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMH1-m	MT20	5.0	6.0	2.50	0.50
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	8.0	Edge	
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	469	0	469	0	0	5-8	5-8	
B	609	0	609	0	0	3-8	3-8	2x4 L

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	372	230 / 0	71 / 0	0 / 0	0 / 0	71 / 0	0 / 0
B	467	317 / 0	71 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 6	-104.9	-104.9	0.13 (1)	10.00	F- C	-387 / 0	0.06 (1)
B- H	-718 / 0	-104.9	-104.9	0.05 (1)	6.25	F- D	0 / 760	0.17 (1)
H- C	-618 / 0	-104.9	-104.9	0.11 (1)	6.25	G- H	0 / 110	0.00 (1)
C- D	-623 / 0	-104.9	-104.9	0.11 (1)	6.25			
E- D	0 / 61	0.0	0.0	0.01 (2)	10.00			
B- G	0 / 593	-27.5	-27.5	0.11 (1)	10.00			
G- F	0 / 593	-27.5	-27.5	0.14 (1)	10.00			
F- E	0 / 0	-27.5	-27.5	0.07 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.14/1.00 (F-G:1) , WB=0.17/1.00 (D-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-875
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1327B-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T20	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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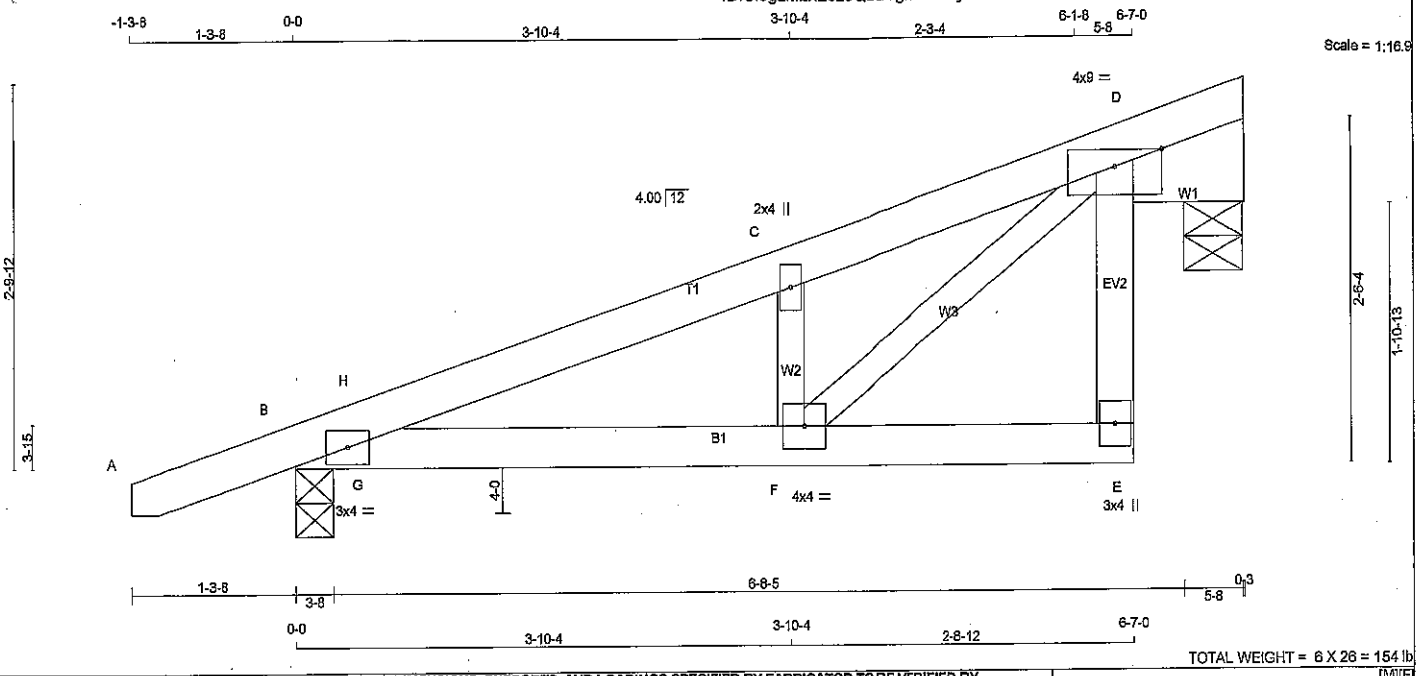

TOTAL WEIGHT = 6 X 26 = 154 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	9.0	Edge	
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	436	0	436	0	0	5-8	5-8
B	574	0	574	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	345	214 / 0	66 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B	439	299 / 0	66 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- H	-691 / 0	-104.9	-104.9	0.04 (2)	6.25
H- C	-659 / 0	-104.9	-104.9	0.13 (1)	6.25
C- D	-667 / 0	-104.9	-104.9	0.11 (1)	6.25
E- D	0 / 48	0.0	0.0	0.01 (3)	10.00
B- G	0 / 836	-27.5	-27.5	0.21 (1)	10.00
G- F	0 / 836	-27.5	-27.5	0.21 (1)	10.00
F- E	0 / 0	-27.5	-27.5	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-382 / 0	0.05 (1)
F- D	0 / 803	0.18 (1)
G- H	-104 / 65	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.21/1.00 (F-G:1) , WB=0.18/1.00 (D-F:1) , SSI=0.15/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13279 -T

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T29	1	2	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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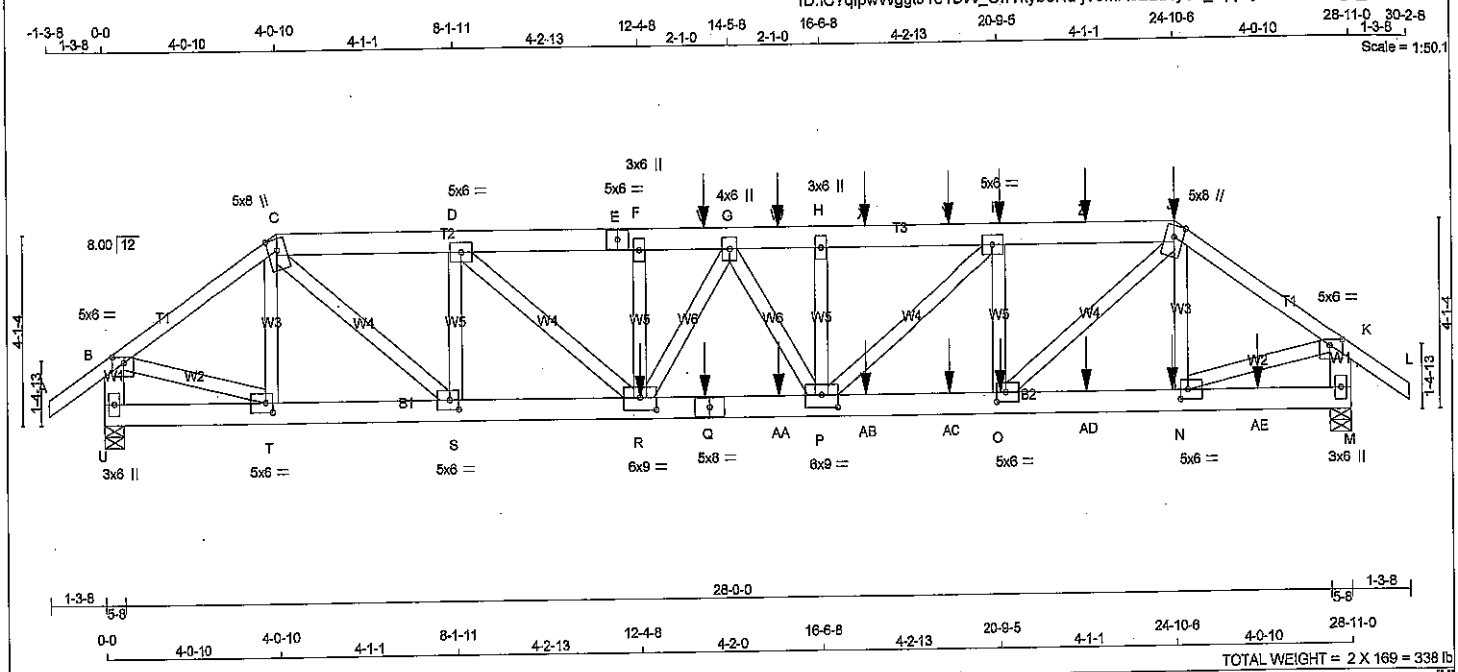

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
J- L	1	12	SIDE(0.0)
C- E	2	12	TOP
E- J	2	12	SIDE(0.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(0.0)
Q- M	2	12	SIDE(23.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	5.0	6.0	1.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMWWW-t	MT20	6.0	9.0	3.25	4.50
Q	BS-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	6.0	9.0	3.25	4.50
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	4065	0	4065	0	0	5-8	5-8
M	4503	0	4503	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3200	2018 / 0	585 / 0	0 / 0	0 / 0	597 / 0	0 / 0
M	3583	2191 / 0	699 / 0	0 / 0	0 / 0	694 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 40	-104.9	-104.9	0.08 (1)		10.00
B- C	-4789 / 0	-104.9	-104.9	0.27 (1)		4.17
C- D	-7576 / 0	-104.9	-104.9	0.16 (1)		4.29
D- E	-10482 / 0	-104.9	-104.9	0.28 (1)		3.62
E- F	-10482 / 0	-104.9	-104.9	0.28 (1)		3.62
F- V	-10482 / 0	-104.9	-104.9	0.23 (1)		3.66
V- G	-10462 / 0	-104.9	-104.9	0.23 (1)		3.66
G- W	-9854 / 0	-104.9	-104.9	0.19 (1)		3.81
W- H	-9854 / 0	-104.9	-104.9	0.19 (1)		3.81
H- X	-9854 / 0	-104.9	-104.9	0.25 (1)		3.75
X- Y	-9854 / 0	-104.9	-104.9	0.25 (1)		3.75
Y- I	-9854 / 0	-104.9	-104.9	0.25 (1)		3.75
I- Z	-7704 / 0	-104.9	-104.9	0.19 (1)		4.22
Z- J	-7704 / 0	-104.9	-104.9	0.19 (1)		4.22
J- K	-5278 / 0	-104.9	-104.9	0.29 (1)		3.99
K- L	0 / 40	-104.9	-104.9	0.08 (1)		10.00
U- B	-4014 / 0	0.0	0.0	0.14 (1)		7.10
M- K	-4388 / 0	0.0	0.0	0.16 (1)		6.85
U- T	0 / 0	-27.5	-27.5	0.02 (2)		10.00
T- S	0 / 4032	-27.5	-27.5	0.29 (1)		10.00
S- R	0 / 7576	-27.5	-27.5	0.53 (1)		10.00
R- Q	0 / 10253	-27.5	-27.5	0.82 (1)		10.00
Q-AA	0 / 10253	-27.5	-27.5	0.82 (1)		10.00
AA- P	0 / 10253	-27.5	-27.5	0.82 (1)		10.00
P-AB	0 / 7704	-27.5	-27.5	0.56 (1)		10.00
AB-AC	0 / 7704	-27.5	-27.5	0.56 (1)		10.00
AC- O	0 / 7704	-27.5	-27.5	0.56 (1)		10.00
O-AD	0 / 4439	-27.5	-27.5	0.32 (1)		10.00
AD- N	0 / 4439	-27.5	-27.5	0.32 (1)		10.00
N-AE	0 / 0	-54.1	-54.1	0.07 (2)		10.00
AE- M	0 / 0	-54.1	-54.1	0.07 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- C	-895 / 0	0.08 (1)
C- S	0 / 4763	0.42 (1)
S- D	-3067 / 0	0.26 (1)
D- R	0 / 3834	0.34 (1)
R- F	-246 / 20	0.02 (1)
P- H	-389 / 0	0.03 (1)
P- I	0 / 2837	0.25 (1)
O- I	-2713 / 0	0.23 (1)
O- J	0 / 4387	0.39 (1)
N- J	-861 / 0	0.06 (1)
B- T	0 / 4125	0.36 (1)
N- K	0 / 4546	0.40 (1)
R- G	0 / 462	0.04 (1)
G- P	-803 / 0	0.08 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	20-9-10	-126	-126	---	FRONT	VERT	TOTAL
J	24-10-6	-440	-440	---	FRONT	VERT	TOTAL
N	24-9-10	-68	-68	---	FRONT	VERT	TOTAL
O	20-9-10	-68	-68	---	FRONT	VERT	TOTAL
Q	13-10-8	-283	-283	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 4-0-10
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: JACK
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-0-10 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.29/1.00 (J-K:1) , BC=0.82/1.00 (P-R:1) , WB=0.42/1.00 (C-S:1) , SSI=0.15/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER 3-618 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13297-19
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T29	1	2	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

OPT.COFF Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed Mar 7 11:38:51 2018 Page 2
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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

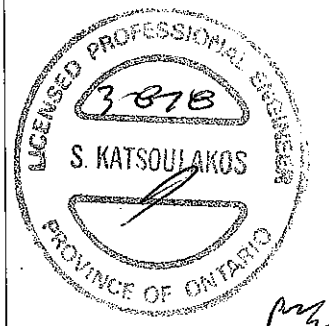
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 40.0 lbs FACTORED DOWN AND 5.1 lbs FACTORED UP AT 13-10-8, 40.0 lbs FACTORED DOWN AND 5.1 lbs FACTORED UP AT 15-6-12, 40.0 lbs FACTORED DOWN AND 5.1 lbs FACTORED UP AT 17-6-12, 126.3 lbs FACTORED DOWN AT 19-6-12, 126.3 lbs FACTORED DOWN AT 20-9-10, AND 126.3 lbs FACTORED DOWN AT 22-9-10, AND 439.8 lbs FACTORED DOWN AT 24-10-6 ON TOP CHORD, AND 2363.2 lbs FACTORED DOWN AT 12-4-8, 282.8 lbs FACTORED DOWN AT 13-10-8, 282.8 lbs FACTORED DOWN AT 15-6-12, 282.8 lbs FACTORED DOWN AT 17-6-12, 67.8 lbs FACTORED DOWN AT 19-6-12, 67.8 lbs FACTORED DOWN AT 20-9-10, 67.8 lbs FACTORED DOWN AT 22-9-10, AND 67.8 lbs FACTORED DOWN AT 24-9-10, AND 67.8 lbs FACTORED DOWN AT 26-9-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	12-4-8	-2363	-2363	—	FRONT	VERT	TOTAL
V	13-10-8	-40	-40	5	FRONT	VERT	TOTAL
W	15-6-12	-40	-40	5	FRONT	VERT	TOTAL
X	17-6-12	-40	-40	5	FRONT	VERT	TOTAL
Y	19-6-12	-126	-126	—	FRONT	VERT	TOTAL
Z	22-9-10	-126	-126	—	FRONT	VERT	TOTAL
AA	15-6-12	-283	-283	—	FRONT	VERT	TOTAL
AB	17-6-12	-283	-283	—	FRONT	VERT	TOTAL
AC	19-6-12	-39	-68	—	FRONT	VERT	TOTAL
AD	22-9-10	-39	-68	—	FRONT	VERT	TOTAL
AE	26-9-10	-39	-68	—	FRONT	VERT	TOTAL

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.93 (Q) (INPUT = 1.00)



DWG NO. TAM 13297

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T30	1	1	TRUSS DESC.	Wellington - S36-17 / C	

Tamarack Roof Truss, Burlington

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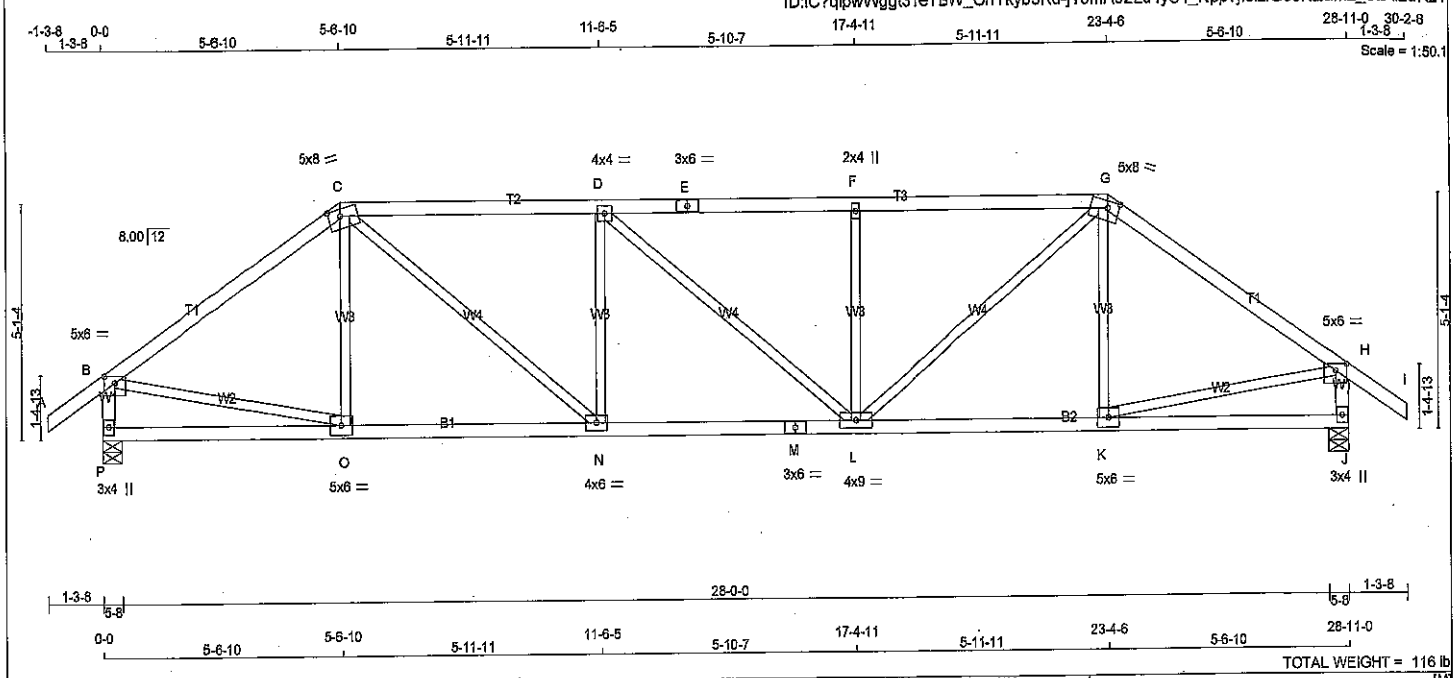

TOTAL WEIGHT = 116 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2183 / 0	-104.9	-104.9	0.77 (1)		3.72
C- D	-2844 / 0	-104.9	-104.9	0.74 (1)		3.38
D- E	-2843 / 0	-104.9	-104.9	0.73 (1)		3.38
E- F	-2843 / 0	-104.9	-104.9	0.73 (1)		3.38
F- G	-2843 / 0	-104.9	-104.9	0.73 (1)		3.39
G- H	-2183 / 0	-104.9	-104.9	0.77 (1)		3.72
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
P- B	-1997 / 0	0.0	0.0	0.21 (1)		5.97
J- H	-1997 / 0	0.0	0.0	0.21 (1)		5.97
P- O	0 / 0	-27.5	-27.5	0.22 (3)		10.00
O- N	0 / 1811	-27.5	-27.5	0.41 (2)		10.00
N- M	0 / 2844	-27.5	-27.5	0.54 (1)		10.00
M- L	0 / 2844	-27.5	-27.5	0.54 (1)		10.00
L- K	0 / 1811	-27.5	-27.5	0.41 (1)		10.00
K- J	0 / 0	-27.5	-27.5	0.22 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-168 / 159	0.06 (1)
C- N	0 / 1326	0.30 (1)
N- D	-673 / 0	0.28 (1)
D- L	-2 / 0	0.00 (1)
L- F	-672 / 0	0.28 (1)
L- G	0 / 1324	0.30 (1)
K- G	-167 / 159	0.06 (1)
B- O	0 / 1847	0.42 (1)
K- H	0 / 1847	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.77/1.00 (G-H:1) , BC=0.54/1.00 (L-N:1) , WB=0.42/1.00 (H-K:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.76 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3076 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 132988
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T31	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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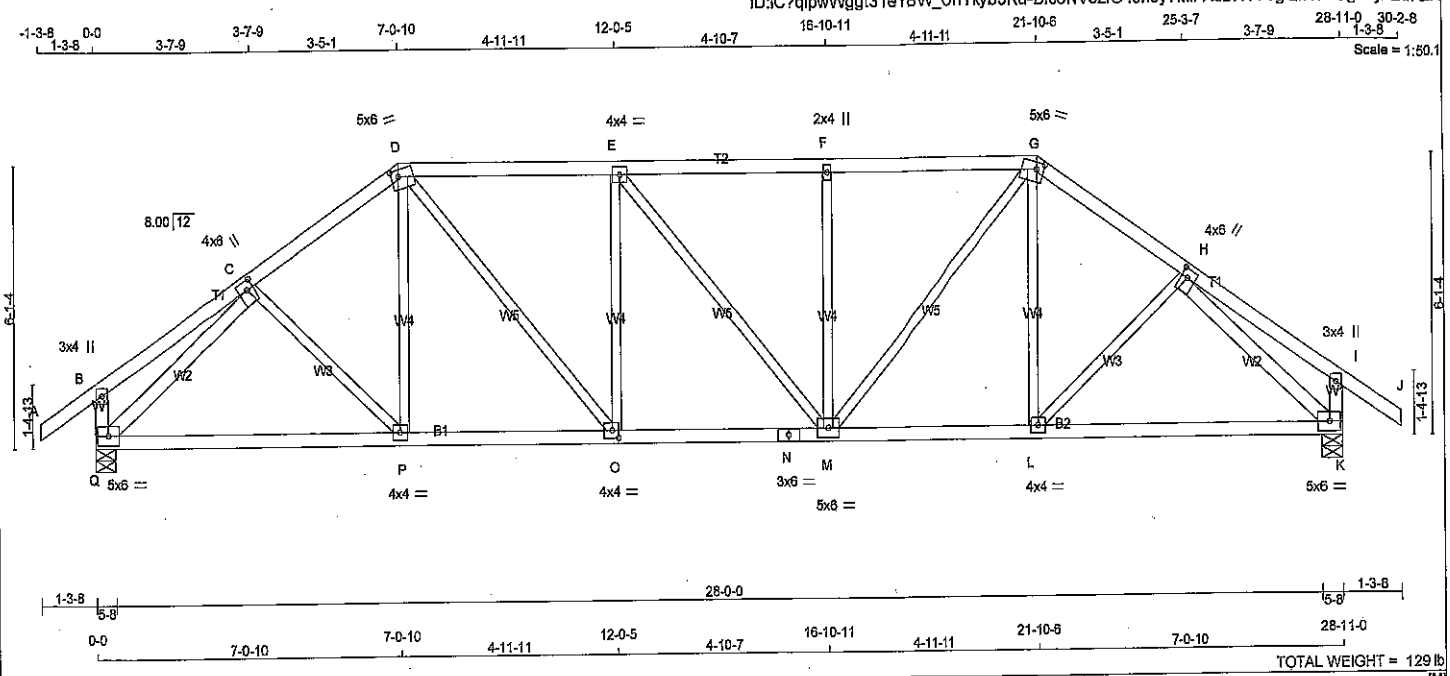

TOTAL WEIGHT = 129 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	6.0	2.25	1.75
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW+t	MT20	4.0	6.0	2.25	1.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
K	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-104.9	-104.9	1	0.14 (1)	10.00	C-P	0 / 147	0.03 (3)
B-C	0 / 22	-104.9	-104.9	1	0.19 (1)	10.00	P-D	0 / 251	0.06 (2)
C-D	-2166 / 0	-104.9	-104.9	1	0.26 (1)	4.38	D-O	0 / 888	0.20 (1)
D-E	-2361 / 0	-104.9	-104.9	1	0.45 (1)	4.00	O-E	-557 / 0	0.33 (1)
E-F	-2360 / 0	-104.9	-104.9	1	0.45 (1)	4.00	E-M	-3 / 0	0.00 (1)
F-G	-2360 / 0	-104.9	-104.9	1	0.44 (1)	4.02	M-F	-556 / 0	0.33 (1)
G-H	-2166 / 0	-104.9	-104.9	1	0.26 (1)	4.38	M-G	0 / 885	0.20 (1)
H-I	0 / 22	-104.9	-104.9	1	0.19 (1)	10.00	L-G	0 / 252	0.06 (2)
I-J	0 / 40	-104.9	-104.9	1	0.14 (1)	10.00	L-H	0 / 147	0.03 (3)
Q-B	-289 / 0	0.0	0.0	1	0.03 (1)	7.81	Q-C	-2419 / 0	0.67 (1)
K-I	-289 / 0	0.0	0.0	1	0.03 (1)	7.81	H-K	-2419 / 0	0.67 (1)
Q-P	0 / 1733	-27.5	-27.5	2	0.47 (2)	10.00			
P-O	0 / 1783	-27.5	-27.5	2	0.49 (2)	10.00			
O-N	0 / 2362	-27.5	-27.5	1	0.45 (1)	10.00			
N-M	0 / 2362	-27.5	-27.5	1	0.45 (1)	10.00			
M-L	0 / 1784	-27.5	-27.5	2	0.49 (2)	10.00			
L-K	0 / 1733	-27.5	-27.5	2	0.47 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.0	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.49/1.00 (L-M:2) , WB=0.67/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.82 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3675
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13299-S8
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T32	2	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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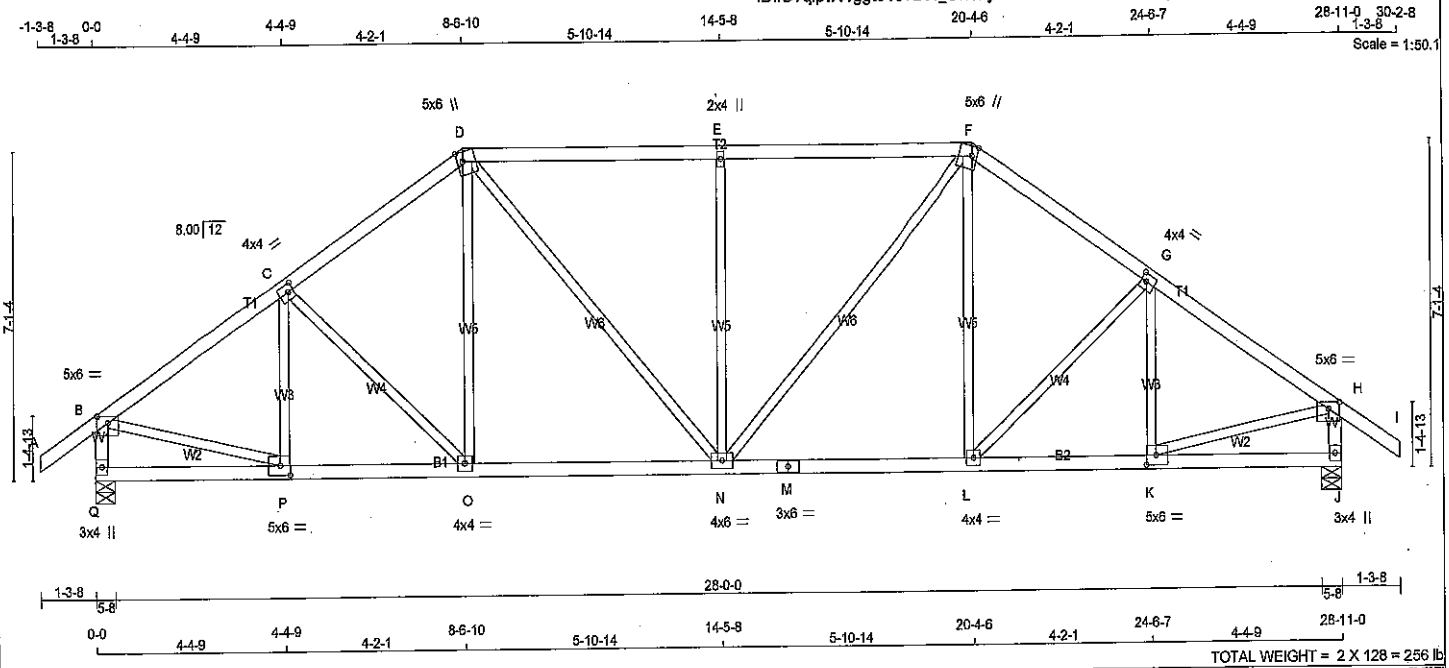

TOTAL WEIGHT = 2 X 128 = 256 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2184 / 0	-104.9	-104.9	0.30 (1)		4.35
C- D	-2079 / 0	-104.9	-104.9	0.29 (1)		4.44
D- E	-2107 / 0	-104.9	-104.9	0.54 (1)		4.08
E- F	-2107 / 0	-104.9	-104.9	0.54 (1)		4.08
F- G	-2079 / 0	-104.9	-104.9	0.29 (1)		4.44
G- H	-2184 / 0	-104.9	-104.9	0.30 (1)		4.35
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
Q- B	-2005 / 0	0.0	0.0	0.21 (1)		5.96
J- H	-2005 / 0	0.0	0.0	0.21 (1)		5.96
Q- P	0 / 0	-27.5	-27.5	0.11 (2)		10.00
P- O	0 / 1840	-27.5	-27.5	0.38 (1)		10.00
O- N	0 / 1707	-27.5	-27.5	0.38 (2)		10.00
N- M	0 / 1707	-27.5	-27.5	0.38 (2)		10.00
M- L	0 / 1707	-27.5	-27.5	0.38 (2)		10.00
L- K	0 / 1840	-27.5	-27.5	0.38 (1)		10.00
K- J	0 / 0	-27.5	-27.5	0.11 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-337 / 30	0.09 (1)
C- O	-189 / 0	0.11 (1)
O- D	0 / 313	0.07 (2)
D- N	0 / 611	0.14 (1)
N- E	-758 / 0	0.66 (1)
N- F	0 / 611	0.14 (1)
L- F	0 / 313	0.07 (2)
L- G	-189 / 0	0.11 (1)
K- G	-337 / 30	0.09 (1)
B- P	0 / 1896	0.43 (1)
K- H	0 / 1896	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.38/1.00 (K-L:1) , WB=0.66/1.00 (E-N:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13300-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T33	2	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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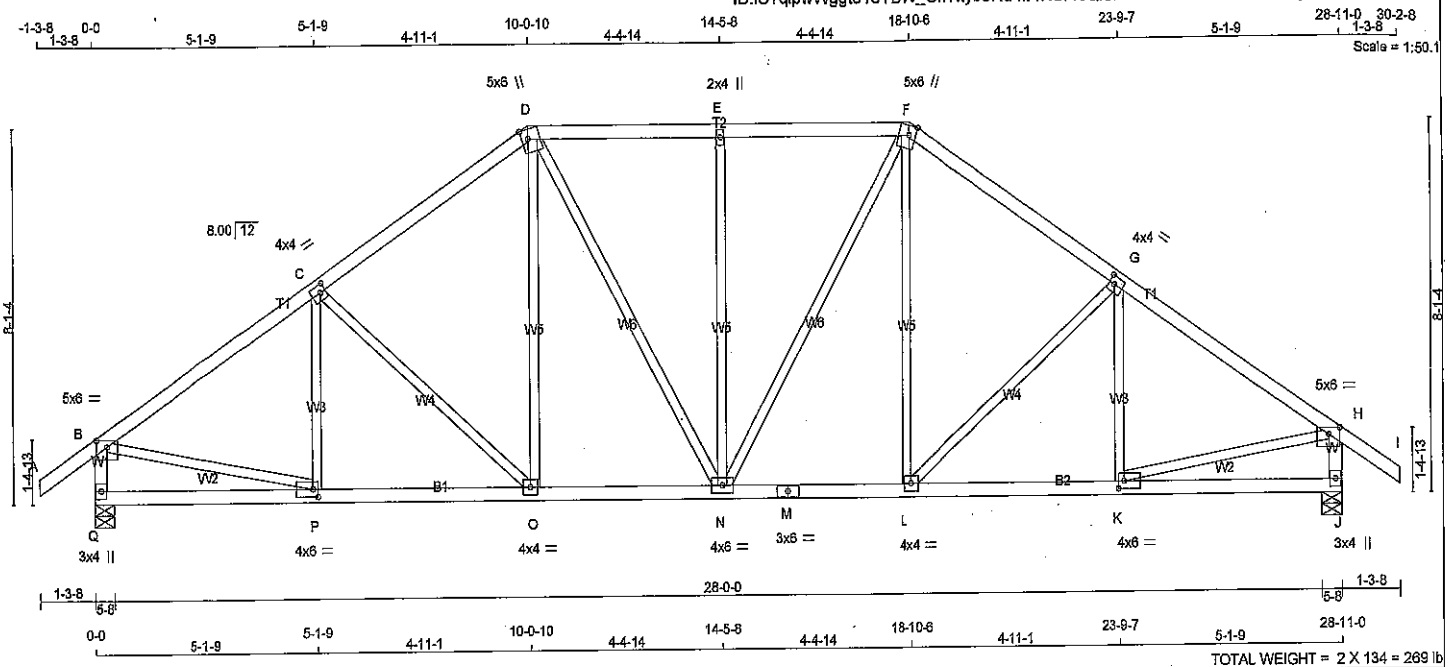


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2222 / 0	-104.9	-104.9	0.42 (1)	4.19	
C- D	-1976 / 0	-104.9	-104.9	0.39 (1)	4.41	
D- E	-1806 / 0	-104.9	-104.9	0.29 (1)	4.69	
E- F	-1806 / 0	-104.9	-104.9	0.29 (1)	4.69	
F- G	-1976 / 0	-104.9	-104.9	0.39 (1)	4.41	
G- H	-2222 / 0	-104.9	-104.9	0.42 (1)	4.19	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-1999 / 0	0.0	0.0	0.21 (1)	5.97	
J- H	-1999 / 0	0.0	0.0	0.21 (1)	5.97	
Q- P	0 / 0	-27.5	-27.5	0.17 (3)	10.00	
P- O	0 / 1877	-27.5	-27.5	0.38 (1)	10.00	
O- N	0 / 1617	-27.5	-27.5	0.33 (1)	10.00	
N- M	0 / 1617	-27.5	-27.5	0.33 (1)	10.00	
M- L	0 / 1617	-27.5	-27.5	0.33 (1)	10.00	
L- K	0 / 1877	-27.5	-27.5	0.38 (1)	10.00	
K- J	0 / 0	-27.5	-27.5	0.17 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-243 / 111	0.08 (1)
C- O	-355 / 0	0.30 (1)
O- D	0 / 367	0.08 (1)
D- N	0 / 384	0.09 (1)
N- E	-558 / 0	0.71 (1)
N- F	0 / 384	0.09 (1)
L- F	0 / 367	0.08 (1)
L- G	-355 / 0	0.30 (1)
K- G	-243 / 111	0.08 (1)
B- P	0 / 1918	0.43 (1)
K- H	0 / 1918	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.38/1.00 (K-L:1) , WB=0.71/1.00 (E-N:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

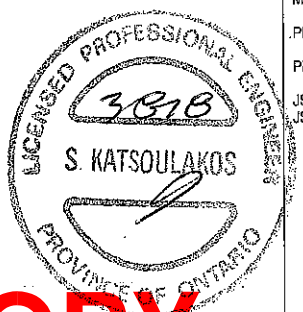
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 13307-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T34	2	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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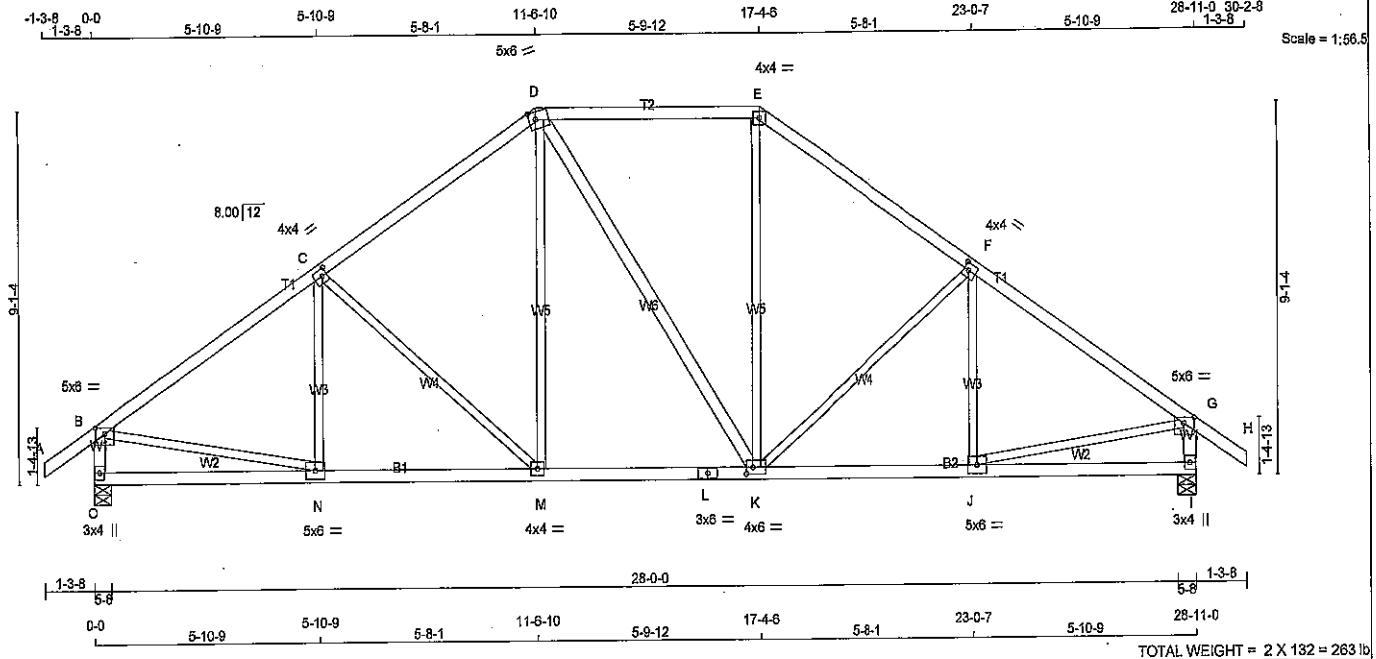

TOTAL WEIGHT = 2 X 132 = 263 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-I	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	1.75	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2059	0	2059	0	0	5-8	5-8
I	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
I	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00		N- C	-172 / 169	0.07 (1)
B- C	-2236 / 0	-104.9	-104.9	0.56 (1)	3.99		C- M	-504 / 0	0.59 (1)
C- D	-1861 / 0	-104.9	-104.9	0.51 (1)	4.35		M- D	0 / 483	0.11 (1)
D- E	-1519 / 0	-104.9	-104.9	0.49 (1)	4.73		D- K	0 / 1	0.00 (1)
E- F	-1862 / 0	-104.9	-104.9	0.52 (1)	4.35		K- E	0 / 484	0.11 (1)
F- G	-2236 / 0	-104.9	-104.9	0.56 (1)	3.99		K- F	-503 / 0	0.59 (1)
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00		J- F	-173 / 168	0.07 (1)
O- B	-1992 / 0	0.0	0.0	0.21 (1)	5.97		B- N	0 / 1925	0.43 (1)
I- G	-1991 / 0	0.0	0.0	0.21 (1)	5.97		J- G	0 / 1925	0.43 (1)
O- N	0 / 0	-27.5	-27.5	0.23 (3)	10.00				
N- M	0 / 1893	-27.5	-27.5	0.42 (2)	10.00				
M- L	0 / 1518	-27.5	-27.5	0.34 (1)	10.00				
L- K	0 / 1518	-27.5	-27.5	0.34 (1)	10.00				
K- J	0 / 1893	-27.5	-27.5	0.42 (2)	10.00				
J- I	0 / 0	-27.5	-27.5	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.42/1.00 (M-N:2) , WB=0.59/1.00 (C-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 1330298
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T35	2	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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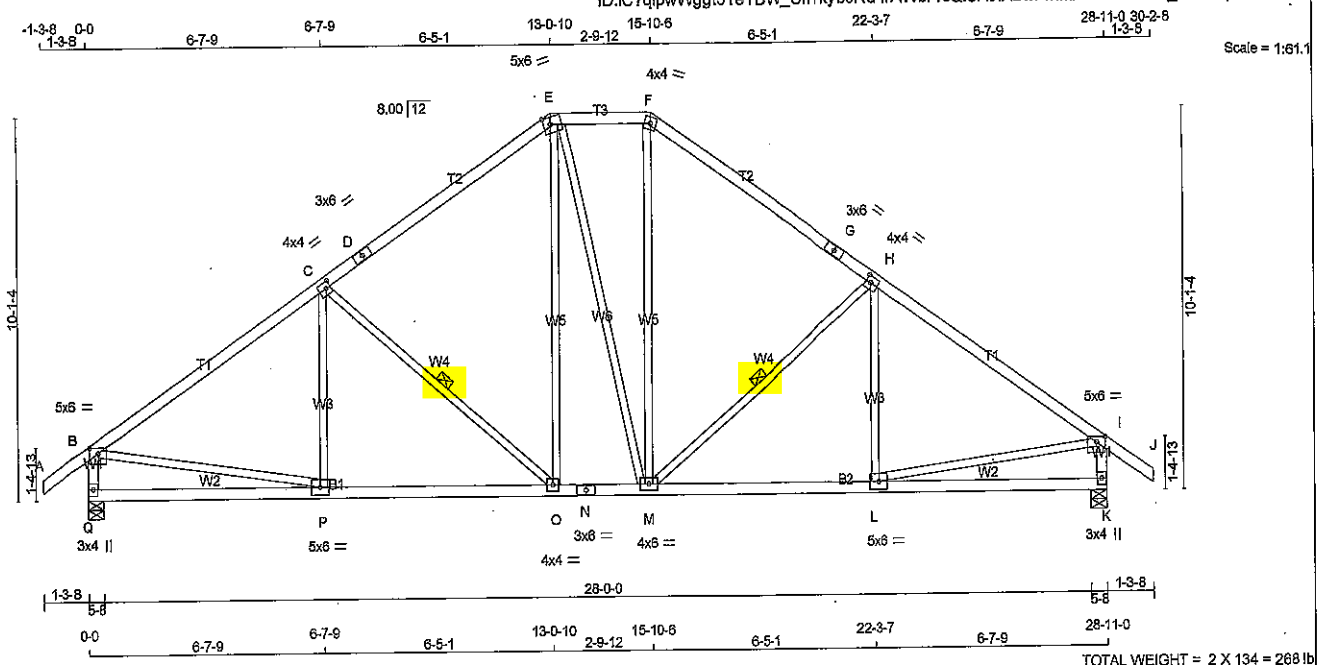

TOTAL WEIGHT = 2 X 134 = 268 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
K	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2236 / 0	-104.9	-104.9	0.74 (1)		3.72
C- D	-1730 / 0	-104.9	-104.9	0.66 (1)		4.24
D- E	-1730 / 0	-104.9	-104.9	0.66 (1)		4.24
E- F	-1407 / 0	-104.9	-104.9	0.13 (1)		5.38
F- G	-1732 / 0	-104.9	-104.9	0.66 (1)		4.23
G- H	-1732 / 0	-104.9	-104.9	0.66 (1)		4.23
H- I	-2236 / 0	-104.9	-104.9	0.74 (1)		3.72
I- J	0 / 40	-104.9	-104.9	0.14 (1)		10.00
Q- B	-1985 / 0	0.0	0.0	0.20 (1)		5.98
K- I	-1985 / 0	0.0	0.0	0.20 (1)		5.98
Q- P	0 / 0	-27.5	-27.5	0.31 (3)		10.00
P- O	0 / 1899	-27.5	-27.5	0.50 (2)		10.00
O- N	0 / 1405	-27.5	-27.5	0.32 (1)		10.00
N- M	0 / 1405	-27.5	-27.5	0.32 (1)		10.00
M- L	0 / 1898	-27.5	-27.5	0.50 (2)		10.00
L- K	0 / 0	-27.5	-27.5	0.31 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-101 / 239	0.05 (3)
C- O	-653 / 0	0.33 (1)
O- E	0 / 530	0.12 (1)
E- M	0 / 7	0.00 (1)
M- F	0 / 537	0.12 (1)
M- H	-650 / 0	0.32 (1)
L- H	-104 / 236	0.05 (1)
B- P	0 / 1924	0.43 (1)
L- I	0 / 1923	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.74/1.00 (B-C:1) , BC=0.50/1.00 (O-P:2) , WB=0.43/1.00 (B-P:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

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3078
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DWG NO. TAM 1330318
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T36	2	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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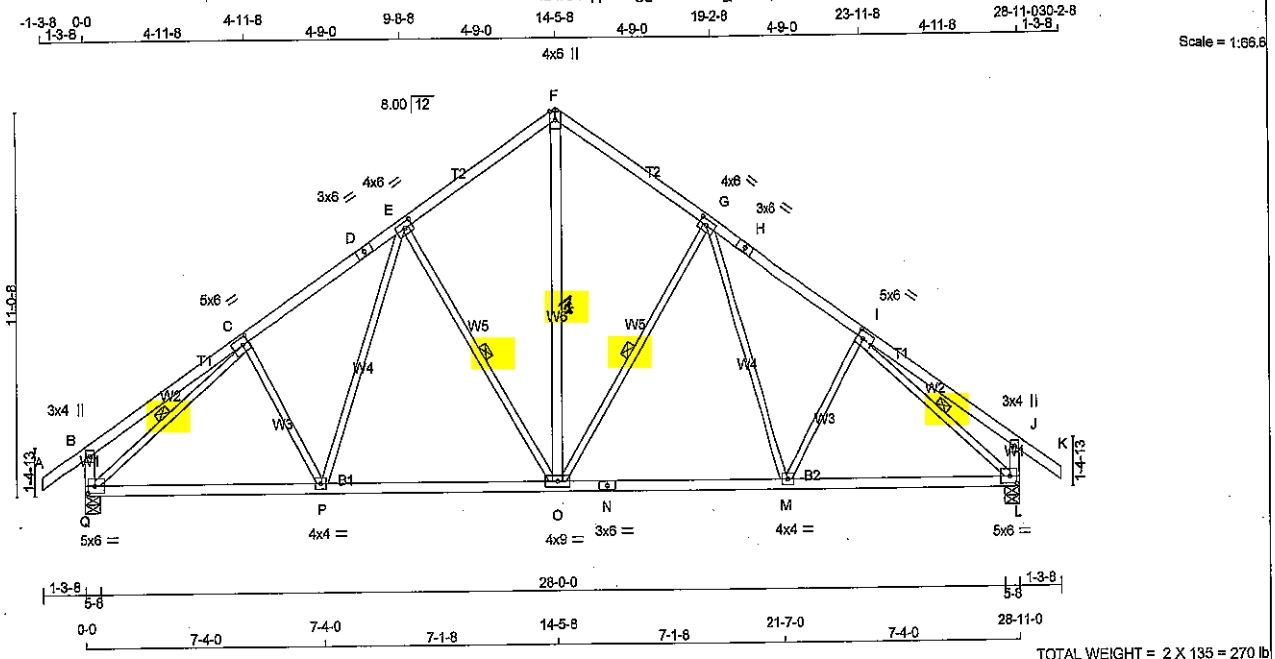

TOTAL WEIGHT = 2 X 135 = 270 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	2.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
L	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-Q, I-L. F.O.I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.32 (1)	10.00
C- D	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
D- E	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
E- F	-1591 / 0	-104.9	-104.9	0.30 (1)	4.94
F- G	-1591 / 0	-104.9	-104.9	0.30 (1)	4.94
G- H	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
H- I	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
I- J	0 / 26	-104.9	-104.9	0.32 (1)	10.00
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-350 / 0	0.0	0.0	0.04 (1)	7.81
L- J	-350 / 0	0.0	0.0	0.04 (1)	7.81
Q- P	0 / 1827	-27.5	-27.5	0.53 (2)	10.00
P- O	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
O- N	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
N- M	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
M- L	0 / 1827	-27.5	-27.5	0.53 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1344	0.22 (1)
O- G	-678 / 0	0.37 (1)
G- M	0 / 370	0.08 (2)
M- I	-128 / 103	0.05 (1)
E- O	-678 / 0	0.37 (1)
P- E	0 / 370	0.08 (2)
C- P	-126 / 103	0.05 (1)
Q- C	-2447 / 0	0.70 (1)
I- L	-2447 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.32/1.00 (G-I:1) , BC=0.53/1.00 (P-Q:2) , WB=0.70/1.00 (C-Q:1) , SSI=0.20/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

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DWG NO. TAM 13304-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T37	1	2	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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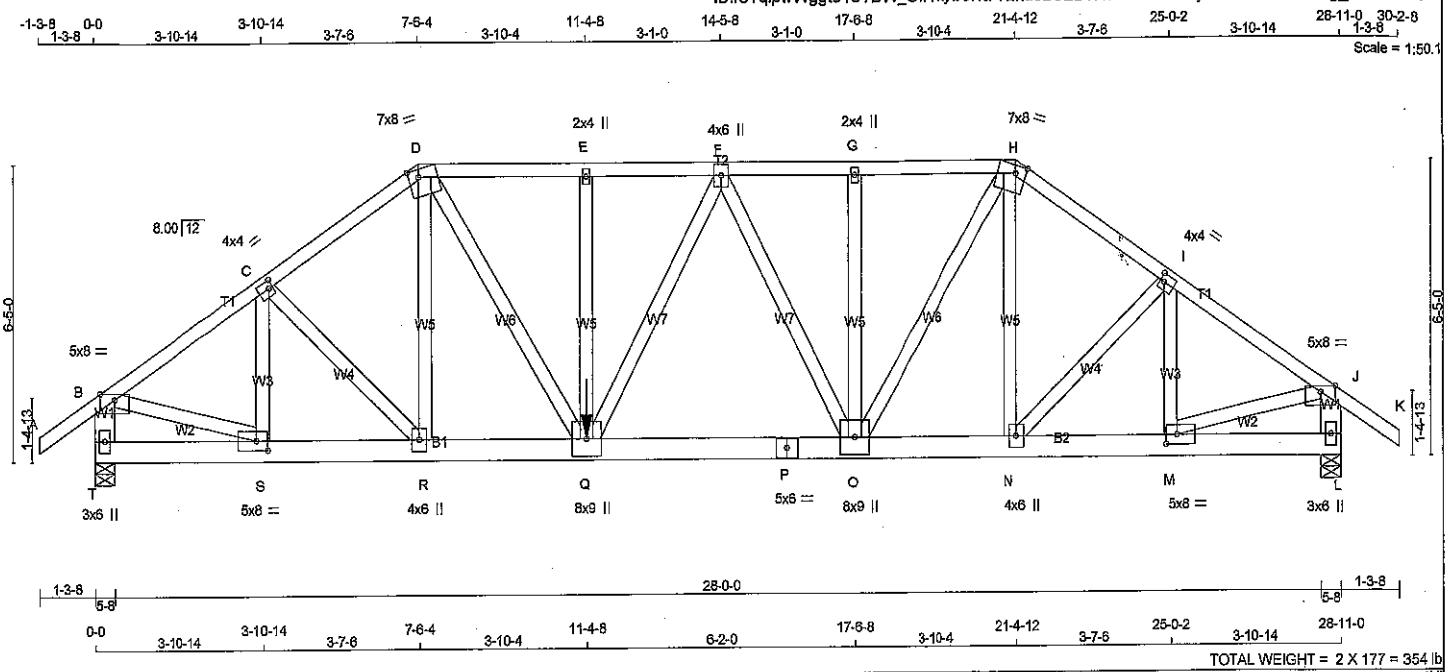

TOTAL WEIGHT = 2 X 177 = 354 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- H	1	12	TOP
H- K	1	12	TOP
T- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- P	2	12	SIDE(0.0)
P- L	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	7.0	8.0	2.00	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	7.0	8.0	2.00	2.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.00
N	BMWW+t	MT20	4.0	6.0		
O	BMWWW+t	MT20	8.0	9.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW+t	MT20	8.0	9.0		
R	BMWW+t	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	8.0	2.50	3.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	5510	0	5510	0	0	5-8	5-8
L	4296	0	4296	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	4315	2759 / 0	766 / 0	0 / 0	0 / 0	791 / 0	0 / 0
L	3366	2150 / 0	598 / 0	0 / 0	0 / 0	617 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)		10.00
B- C	-6624 / 0	-104.9	-104.9	0.29 (1)		3.61
C- D	-7377 / 0	-104.9	-104.9	0.32 (1)		3.40
D- E	-8663 / 0	-104.9	-104.9	0.40 (1)		3.05
E- F	-8663 / 0	-104.9	-104.9	0.30 (1)		3.15
F- G	-6428 / 0	-104.9	-104.9	0.23 (1)		3.69
G- H	-6428 / 0	-104.9	-104.9	0.26 (1)		3.67
H- I	-5534 / 0	-104.9	-104.9	0.24 (1)		3.96
I- J	-5065 / 0	-104.9	-104.9	0.23 (1)		4.13
J- K	0 / 40	-104.9	-104.9	0.08 (1)		10.00
T- B	-5411 / 0	0.0	0.0	0.19 (1)		6.30
L- J	-4214 / 0	0.0	0.0	0.15 (1)		6.96
T- S	0 / 0	-27.5	-27.5	0.06 (1)		10.00
S- R	0 / 5528	-27.5	-27.5	0.42 (1)		10.00
R- Q	0 / 6101	-27.5	-27.5	0.47 (1)		10.00
Q- P	0 / 7623	-27.5	-27.5	0.58 (1)		10.00
P- O	0 / 7623	-27.5	-27.5	0.58 (1)		10.00
O- N	0 / 4573	-27.5	-27.5	0.32 (1)		10.00
N- M	0 / 4232	-27.5	-27.5	0.32 (1)		10.00
M- L	0 / 0	-27.5	-27.5	0.05 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-1436 / 0	0.13 (1)
C- R	0 / 803	0.07 (1)
R- D	-278 / 0	0.06 (1)
D- Q	0 / 4755	0.42 (1)
Q- E	-349 / 2	0.08 (1)
Q- F	0 / 2288	0.20 (1)
F- O	-2630 / 0	0.79 (1)
O- G	-389 / 0	0.09 (1)
O- H	0 / 3443	0.30 (1)
N- H	-174 / 39	0.04 (1)
N- I	0 / 477	0.04 (1)
M- I	-1054 / 0	0.09 (1)
B- S	0 / 5706	0.50 (1)
M- J	0 / 4368	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
Q	11-4-8	-5689	-5689	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.58/1.00 (O-Q:1) , WB=0.79/1.00 (F-O:1) , SSI=0.10/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.66 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

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P6/6

DWG NO. TAM 13305-R5
STRUCTURAL COMPONENT

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T37	1	2	TRUSS DESC.	Wellington - S38-17 / C	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5688.6 lbs FACTORED DOWN AT 11-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

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LICENSED PROFESSIONAL ENGINEER
3878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM13305
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T38	1	2	Tamarack - Bayview	
				TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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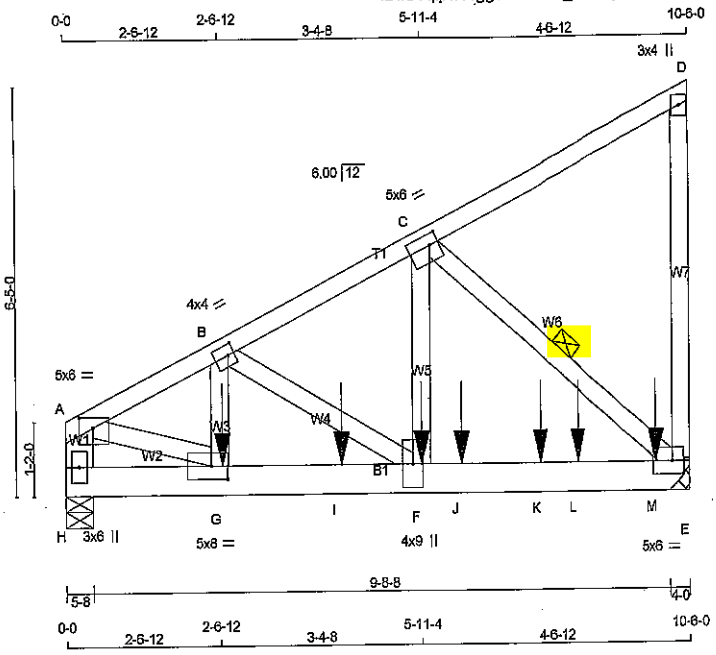


TOTAL WEIGHT = 2 X 58 = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	2	12	TOP
A- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	2.00	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	5.0	6.0	2.50	2.25
F	BMWW+t	MT20	4.0	9.0		
G	BMWW-t	MT20	5.0	8.0	2.50	3.25
H	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2193.3 lbs FACTORED DOWN AT 2-7-4, 1253.1 lbs FACTORED DOWN AT 4-7-4, 1239.2 lbs FACTORED DOWN AT 5-11-4, 1253.1 lbs FACTORED DOWN AT 6-7-4, 717.4 lbs FACTORED DOWN AT 7-11-4, AND 1253.1 lbs FACTORED DOWN AT 8-7-4, AND 723.4 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	4500	0	4500	0	0	IN-SX	IN-SX
E	5523	0	5523	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	3562	2211 / 0	673 / 0	0 / 0	0 / 0	678 / 0	0 / 0
E	4340	2749 / 0	786 / 0	0 / 0	0 / 0	805 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
H- A	-4295 / 0	0.0	0.0	15 (1)	6.91	A- G	0 / 5407	0.48 (1)	
A- B	-5791 / 0	-104.9	-104.9	18 (1)	3.92	G- B	0 / 445	0.04 (1)	
B- C	-5068 / 0	-104.9	-104.9	24 (1)	4.09	B- F	-723 / 0	0.07 (1)	
C- D	-19 / 0	-104.9	-104.9	14 (1)	6.25	F- C	0 / 5497	0.49 (1)	
E- D	-197 / 0	0.0	0.0	08 (1)	7.81	C- E	-5897 / 0	0.53 (1)	
H- G	0 / 0	-27.5	-27.5	09 (1)	10.00				
G- I	0 / 5176	-27.5	-27.5	45 (1)	10.00				
I- F	0 / 5176	-27.5	-27.5	45 (1)	10.00				
F- J	0 / 4559	-27.5	-27.5	52 (1)	10.00				
J- K	0 / 4559	-27.5	-27.5	52 (1)	10.00				
K- L	0 / 4559	-27.5	-27.5	52 (1)	10.00				
L- M	0 / 4559	-27.5	-27.5	52 (1)	10.00				
M- E	0 / 4559	-27.5	-27.5	52 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
F	5-11-4	-1239	-1239	---	FRONT	VERT	TOTAL
G	2-7-4	-2193	-2193	---	BACK	VERT	TOTAL
I	4-7-4	-1253	-1253	---	BACK	VERT	TOTAL
J	6-7-4	-1253	-1253	---	BACK	VERT	TOTAL
K	7-11-4	-717	-717	---	FRONT	VERT	TOTAL
L	8-7-4	-1253	-1253	---	BACK	VERT	TOTAL
M	9-11-4	-723	-723	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.52/1.00 (E-F:1) , WB=0.53/1.00 (C-E:1) , SSI=0.82/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

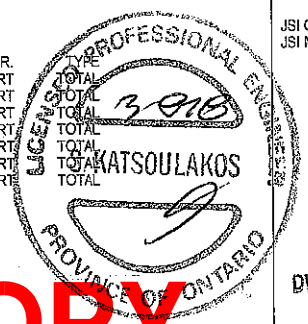
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.61 (C) (INPUT = 1.00)



DWG NO. TAM-133064

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T39	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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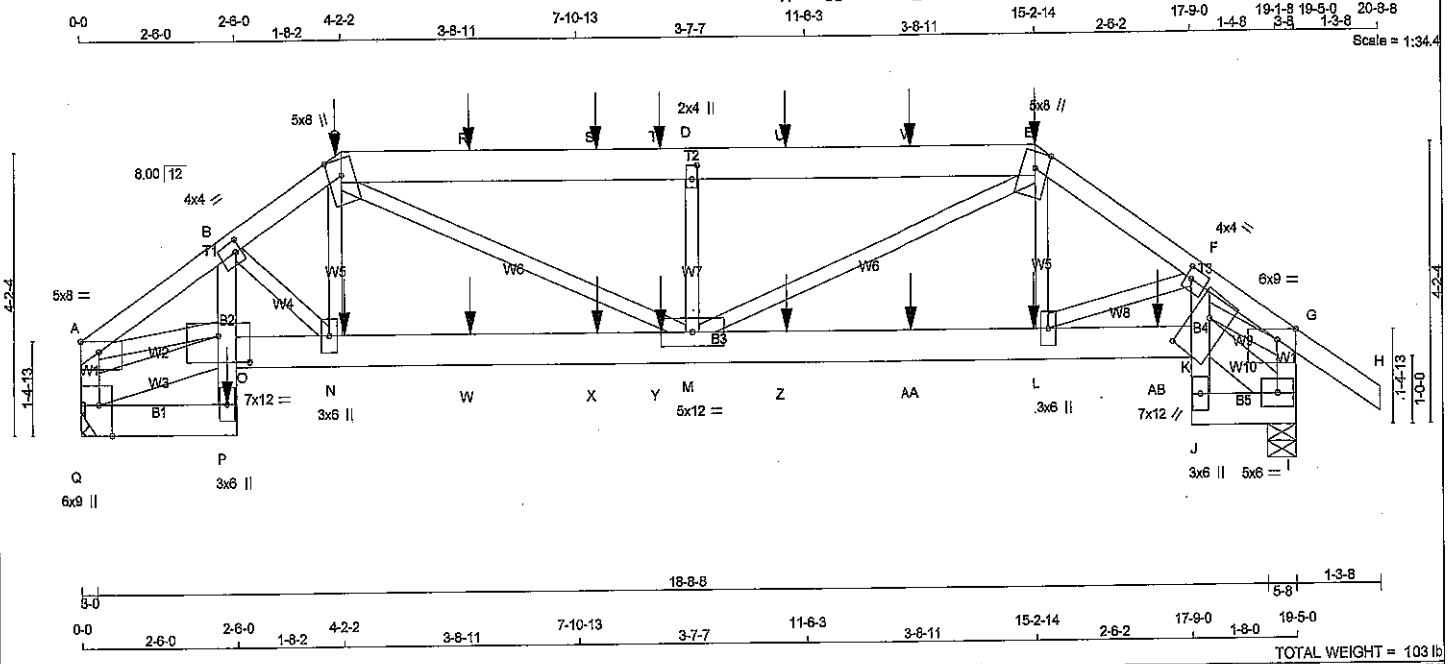

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.75	Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWW+m	MT20	5.0	8.0	Edge	
F	TMVW-t	MT20	4.0	4.0	2.00	1.00
G	TMVW-p	MT20	6.0	9.0	Edge	3.50
I	BMVW1-t	MT20	5.0	6.0		
J	BMV+p	MT20	3.0	6.0		
K	BVMWW-w	MT20	7.0	12.0	3.25	7.50
L	BMWW+t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	12.0		
N	BMWW+t	MT20	3.0	6.0		
O	BVMWW-I	MT20	7.0	12.0	4.75	6.00
P	BMV+p	MT20	3.0	6.0		
Q	BMVW1+t	MT20	6.0	9.0	Edge	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 137.9 lbs FACTORED DOWN AT 4-2-2, 134.3 lbs FACTORED DOWN AT 6-2-8, 134.3 lbs FACTORED DOWN AT 8-2-8, 134.3 lbs FACTORED DOWN AT 9-2-8, 134.3 lbs FACTORED DOWN AT 11-2-8, AND 134.3 lbs FACTORED DOWN AT 13-2-8, AND 137.9 lbs FACTORED DOWN AT 15-2-14 ON TOP CHORD, AND 90.4 lbs FACTORED DOWN AT 2-4-4, 114.5 lbs FACTORED DOWN AT 4-2-8, 114.5 lbs FACTORED DOWN AT 6-2-8, 114.5 lbs FACTORED DOWN AT 8-2-8, 114.5 lbs FACTORED DOWN AT 9-2-8, 114.5 lbs FACTORED DOWN AT 11-2-8, 114.5 lbs FACTORED DOWN AT 13-2-8, AND 114.5 lbs FACTORED DOWN AT 15-2-8, AND 116.6 lbs FACTORED DOWN AT 17-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2221	0	2221	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	
I	2414	0	2414	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1789	1055 / 0	372 / 0	0 / 0	0 / 0	361 / 0	0 / 0
I	1916	1179 / 0	387 / 0	0 / 0	0 / 0	369 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3625 / 0	-104.9	-104.9	0.30 (1)		3.42
B- C	-3577 / 0	-104.9	-104.9	0.24 (1)		3.50
C- R	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
R- S	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
S- T	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
T- D	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
D- U	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
U- V	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
V- E	-4654 / 0	-104.9	-104.9	0.50 (1)		3.52
E- F	-3550 / 0	-104.9	-104.9	0.34 (1)		3.40
F- G	-3617 / 0	-104.9	-104.9	0.31 (1)		3.39
G- H	0 / 40	-104.9	-104.9	0.15 (1)		10.00
Q- A	-2137 / 0	0.0	0.0	0.24 (1)		5.75
I- G	-2288 / 0	0.0	0.0	0.26 (1)		5.58
Q- P	0 / 121	-27.5	-27.5	0.04 (2)		10.00
P- O	0 / 146	0.0	0.0	0.20 (1)		10.00
O- B	-69 / 60	0.0	0.0	0.18 (1)		7.81
O- N	0 / 3090	-27.5	-27.5	0.56 (1)		10.00
N- W	0 / 2979	-27.5	-27.5	0.55 (1)		10.00
W- X	0 / 2979	-27.5	-27.5	0.55 (1)		10.00
X- Y	0 / 2979	-27.5	-27.5	0.55 (1)		10.00
Y- M	0 / 2979	-27.5	-27.5	0.55 (1)		10.00
M- Z	0 / 2955	-27.5	-27.5	0.53 (1)		10.00
Z-AA	0 / 2955	-27.5	-27.5	0.53 (1)		10.00
AA- L	0 / 2955	-27.5	-27.5	0.53 (1)		10.00
L-AB	0 / 3126	-27.5	-27.5	0.54 (1)		10.00
AB- K	0 / 3126	-27.5	-27.5	0.54 (1)		10.00
J- K	0 / 36	0.0	0.0	0.24 (1)		10.00
K- F	-30 / 51	0.0	0.0	0.24 (1)		7.81
J- I	0 / 160	-27.5	-27.5	0.03 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- N	-122 / 0	0.02 (1)
N- C	0 / 564	0.14 (1)
C- M	0 / 1871	0.46 (1)
M- D	-1213 / 0	0.22 (1)
M- E	0 / 1898	0.47 (1)
L- E	0 / 525	0.13 (2)
L- F	-164 / 0	0.03 (1)
Q- O	-132 / 0	0.01 (1)
A- O	0 / 3013	0.75 (1)
K- I	-192 / 0	0.01 (1)
K- G	0 / 2968	0.73 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	4-2-2	-138	-138	---	BACK	VERT	TOTAL
E	15-2-14	-138	-138	---	BACK	VERT	TOTAL
L	15-2-8	-114	-114	---	BACK	VERT	TOTAL
N	4-2-8	-114	-114	---	BACK	VERT	TOTAL
P	2-4-4	-52	-90	---	BACK	VERT	TOTAL
R	6-2-8	-134	-134	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 962 (0.24")

CSI: TC=0.50/1.00 (C-D:1) , BC=0.56/1.00 (N-O:1) , WB=0.75/1.00 (A-O:1) , SSI=0.43/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.58 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/8/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13307
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T39	1	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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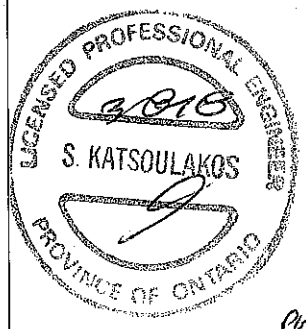
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	6-2-8	-134	-134	---	BACK	VERT	TOTAL
T	9-2-8	-134	-134	---	BACK	VERT	TOTAL
U	11-2-8	-134	-134	---	BACK	VERT	TOTAL
V	13-2-8	-134	-134	---	BACK	VERT	TOTAL
W	6-2-8	-114	-114	---	BACK	VERT	TOTAL
X	8-2-8	-114	-114	---	BACK	VERT	TOTAL
Y	9-2-8	-114	-114	---	BACK	VERT	TOTAL
Z	11-2-8	-114	-114	---	BACK	VERT	TOTAL
AA	13-2-8	-114	-114	---	BACK	VERT	TOTAL
AB	17-2-8	-119	-119	---	BACK	VERT	TOTAL



DWG NO. TAM 133078
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288711	T39A	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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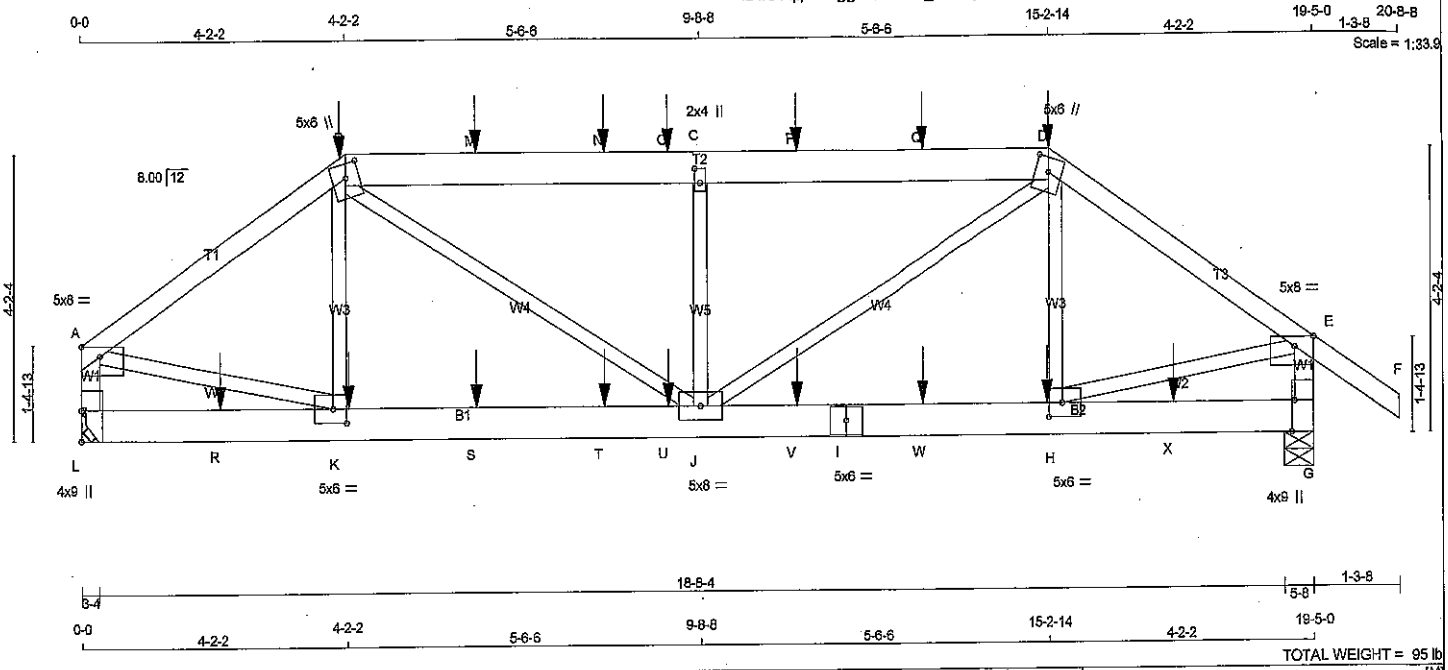


TOTAL WEIGHT = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.75	Edge
B	TTWW+m	MT20	5.0	6.0	2.50	2.50
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TTWW+m	MT20	5.0	6.0	2.50	2.50
E	TMVW-p	MT20	5.0	8.0	1.75	3.50
G	BMV1+t	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0	2.50	2.50
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 263.7 lbs FACTORED DOWN AT 4-2-2, 263.7 lbs FACTORED DOWN AT 15-2-14, 134.8 lbs FACTORED DOWN AT 4-2-2, 131.2 lbs FACTORED DOWN AT 6-2-8, 131.2 lbs FACTORED DOWN AT 8-2-8, 131.2 lbs FACTORED DOWN AT 9-2-8, 131.2 lbs FACTORED DOWN AT 11-2-8, AND 131.2 lbs FACTORED DOWN AT 13-2-8, AND 134.8 lbs FACTORED DOWN AT 15-2-14 ON TOP CHORD, AND 70.1 lbs FACTORED DOWN AT 2-2-8, 70.1 lbs FACTORED DOWN AT 4-2-8, 70.1 lbs FACTORED DOWN AT 6-2-8, 70.1 lbs FACTORED DOWN AT 8-2-8, 70.1 lbs FACTORED DOWN AT 9-2-8, 70.1 lbs FACTORED DOWN AT 11-2-8, 70.1 lbs FACTORED DOWN AT 13-2-8, AND 70.1 lbs FACTORED DOWN AT 15-2-8, AND 70.1 lbs FACTORED DOWN AT 17-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2199	0	2199	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-4	
G	2334	0	2334	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1771	1046 / 0	368 / 0	0 / 0	0 / 0	357 / 0	0 / 0
G	1881	1131 / 0	366 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	-2550 / 0	-104.9	-104.9	0.50 (1)	3.78
B- M	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
M- N	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
N- O	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
O- C	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
C- P	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
P- Q	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
Q- D	-3224 / 0	-104.9	-104.9	0.48 (1)	4.15
D- E	-2536 / 0	-104.9	-104.9	0.49 (1)	3.60
E- F	0 / 40	-104.9	-104.9	0.15 (1)	10.00
L- A	-2145 / 0	0.0	0.0	0.24 (1)	5.74
G- E	-2279 / 0	0.0	0.0	0.25 (1)	5.58
L- R	0 / 0	-27.5	-27.5	0.12 (3)	10.00
R- K	0 / 0	-27.5	-27.5	0.12 (3)	10.00
K- S	0 / 2135	-27.5	-27.5	0.37 (1)	10.00
S- T	0 / 2135	-27.5	-27.5	0.37 (1)	10.00
T- U	0 / 2135	-27.5	-27.5	0.37 (1)	10.00
U- J	0 / 2135	-27.5	-27.5	0.37 (1)	10.00
J- V	0 / 2124	-27.5	-27.5	0.36 (1)	10.00
V- I	0 / 2124	-27.5	-27.5	0.36 (1)	10.00
I- W	0 / 2124	-27.5	-27.5	0.36 (1)	10.00
W- H	0 / 2124	-27.5	-27.5	0.36 (1)	10.00
H- X	0 / 0	-27.5	-27.5	0.11 (3)	10.00
X- G	0 / 0	-27.5	-27.5	0.11 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K- B	-258 / 196	0.07 (1)
B- J	0 / 1316	0.33 (1)
J- C	-1216 / 0	0.30 (1)
J- D	0 / 1329	0.33 (1)
H- D	-262 / 168	0.07 (1)
A- K	0 / 2191	0.54 (1)
H- E	0 / 2179	0.54 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	4-2-2	-264	-264	---	FRONT	VERT	TOTAL
B	4-2-2	-135	-135	---	BACK	VERT	TOTAL
D	15-2-14	-264	-264	---	FRONT	VERT	TOTAL
D	15-2-14	-135	-135	---	BACK	VERT	TOTAL
H	15-2-8	-41	-70	---	BACK	VERT	TOTAL
K	4-2-8	-41	-70	---	BACK	VERT	TOTAL
M	6-2-8	-131	-131	---	BACK	VERT	TOTAL
N	8-2-8	-131	-131	---	BACK	VERT	TOTAL
O	9-2-8	-131	-131	---	BACK	VERT	TOTAL
P	11-2-8	-131	-131	---	BACK	VERT	TOTAL
Q	13-2-8	-131	-131	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.50/1.00 (A-B:1) , BC=0.37/1.00 (J-K:1) , WB=0.54/1.00 (A-K:1) , SSI=0.43/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

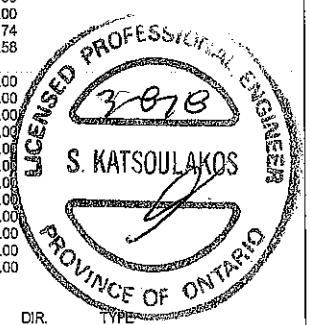
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 1332-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288711	T39A	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	2-2-8	-41	-70	—	BACK	VERT	TOTAL
S	6-2-8	-41	-70	—	BACK	VERT	TOTAL
T	8-2-8	-41	-70	—	BACK	VERT	TOTAL
U	9-2-8	-41	-70	—	BACK	VERT	TOTAL
V	11-2-8	-41	-70	—	BACK	VERT	TOTAL
W	13-2-8	-41	-70	—	BACK	VERT	TOTAL
X	17-2-8	-41	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13321-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T40	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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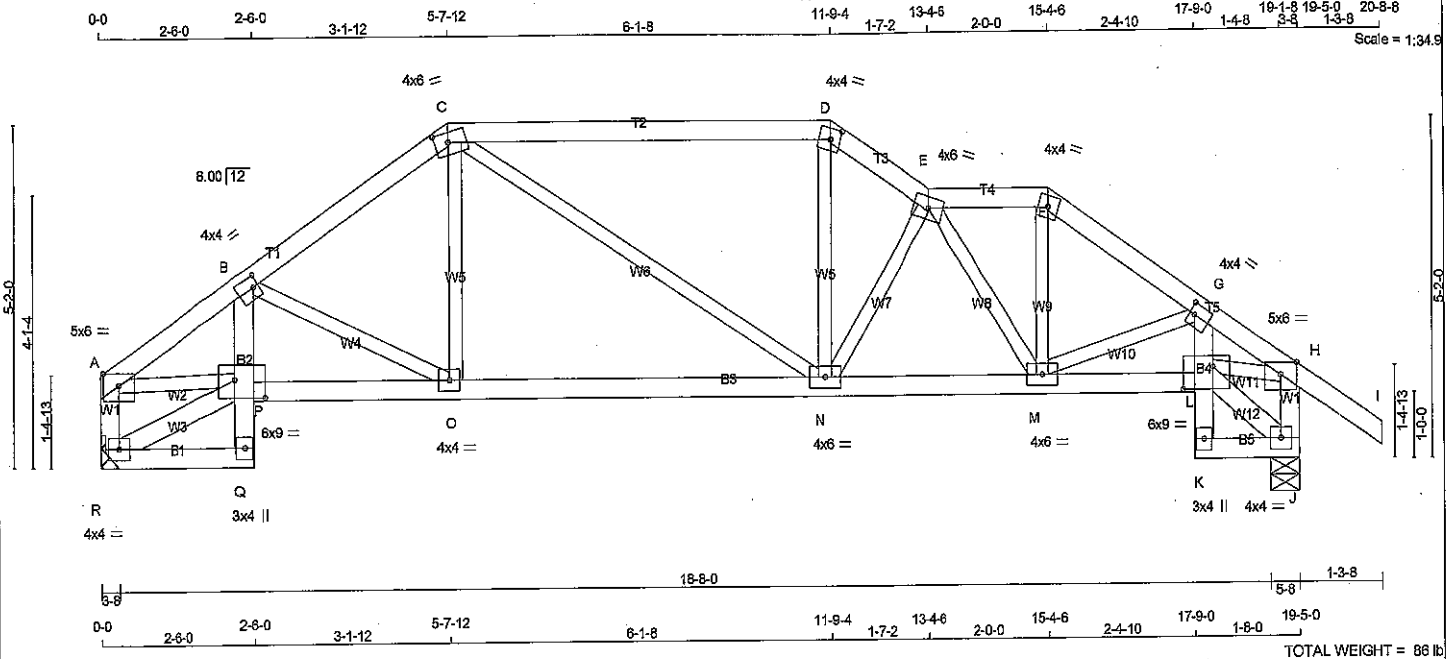

TOTAL WEIGHT = 86 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.00
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-t	MT20	6.0	9.0	4.00	5.75
M	BMWWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWW-t	MT20	6.0	9.0	3.25	5.75
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1281	0	1281	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1435	0	1435	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	1120	723 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2013 / 0	-104.9	-104.9	0.12 (1)		4.66
B- C	-1623 / 0	-104.9	-104.9	0.17 (1)		5.05
C- D	-1541 / 0	-104.9	-104.9	0.54 (1)		4.61
D- E	-1833 / 0	-104.9	-104.9	0.07 (1)		4.91
E- F	-1466 / 0	-104.9	-104.9	0.08 (1)		5.32
F- G	-1733 / 0	-104.9	-104.9	0.15 (1)		4.93
G- H	-2093 / 0	-104.9	-104.9	0.15 (1)		4.56
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
R- A	-1220 / 0	0.0	0.0	0.13 (1)		7.26
J- H	-1350 / 0	0.0	0.0	0.14 (1)		6.98
R- Q	0 / 61	-27.5	-27.5	0.05 (2)		10.00
Q- P	0 / 56	0.0	0.0	0.09 (1)		10.00
P- B	0 / 110	0.0	0.0	0.10 (1)		10.00
P- O	0 / 1731	-27.5	-27.5	0.37 (1)		10.00
O- N	0 / 1346	-27.5	-27.5	0.31 (1)		10.00
N- M	0 / 1886	-27.5	-27.5	0.38 (1)		10.00
M- L	0 / 1811	-27.5	-27.5	0.32 (1)		10.00
K- L	0 / 36	0.0	0.0	0.14 (1)		10.00
L- G	0 / 195	0.0	0.0	0.16 (1)		10.00
K- J	0 / 84	-27.5	-27.5	0.03 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- O	-437 / 0	0.11 (1)
O- C	0 / 339	0.08 (2)
C- N	0 / 232	0.05 (1)
N- D	0 / 612	0.14 (1)
N- E	-696 / 0	0.14 (1)
M- F	0 / 723	0.16 (1)
M- G	-394 / 0	0.07 (1)
R- P	-67 / 0	0.01 (1)
A- P	0 / 1688	0.38 (1)
L- J	-106 / 0	0.01 (1)
L- H	0 / 1706	0.38 (1)
E- M	-730 / 0	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.38/1.00 (M-N:1) , WB=0.38/1.00 (H-L:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.39 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 133088
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288711	T40A	1	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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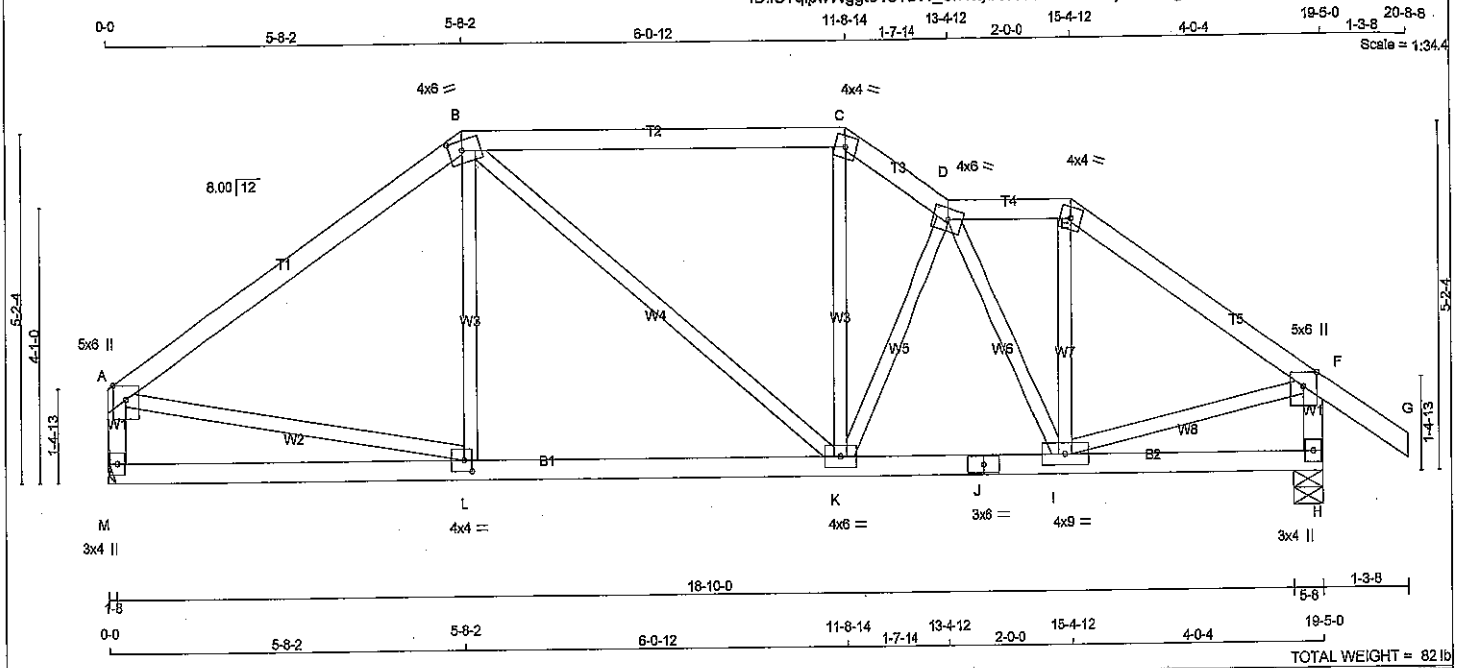


TOTAL WEIGHT = 82 lb [MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1286	0	1286	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	1430	0	1430	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1019	631 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
H	1116	720 / 0	194 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	-1300 / 0	-104.9	-104.9	0.47 (1)	5.08	L- B	-17 / 233	0.05 (3)
B- C	-1231 / 0	-104.9	-104.9	0.52 (1)	5.08	B- K	0 / 195	0.04 (1)
C- D	-1469 / 0	-104.9	-104.9	0.06 (1)	5.36	K- C	0 / 410	0.09 (1)
D- E	-1120 / 0	-104.9	-104.9	0.07 (1)	5.93	K- D	-450 / 0	0.13 (1)
E- F	-1327 / 0	-104.9	-104.9	0.24 (1)	5.36	I- E	0 / 423	0.10 (1)
F- G	0 / 40	-104.9	-104.9	0.14 (1)	10.00	A- L	0 / 1099	0.25 (1)
M- A	-1223 / 0	0.0	0.0	0.13 (1)	7.25	I- F	0 / 1138	0.26 (1)
H- F	-1379 / 0	0.0	0.0	0.14 (1)	6.93	D- I	-666 / 0	0.20 (1)
M- L	0 / 0	-27.5	-27.5	0.24 (3)	10.00			
L- K	0 / 1080	-27.5	-27.5	0.35 (2)	10.00			
K- J	0 / 1412	-27.5	-27.5	0.29 (1)	10.00			
J- I	0 / 1412	-27.5	-27.5	0.29 (1)	10.00			
I- H	0 / 0	-27.5	-27.5	0.10 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.35/1.00 (K-L:2) , WB=0.26/1.00 (F-I:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

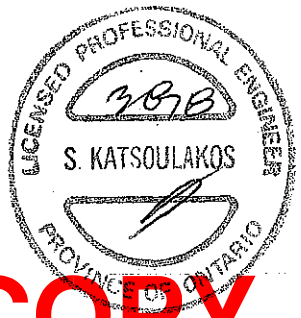
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.40 (L) (INPUT = 1.00)



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DWG NO. TAM 1332-08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T41	1	1	TRUSS DESC.	Wellington - S38-17 / C	

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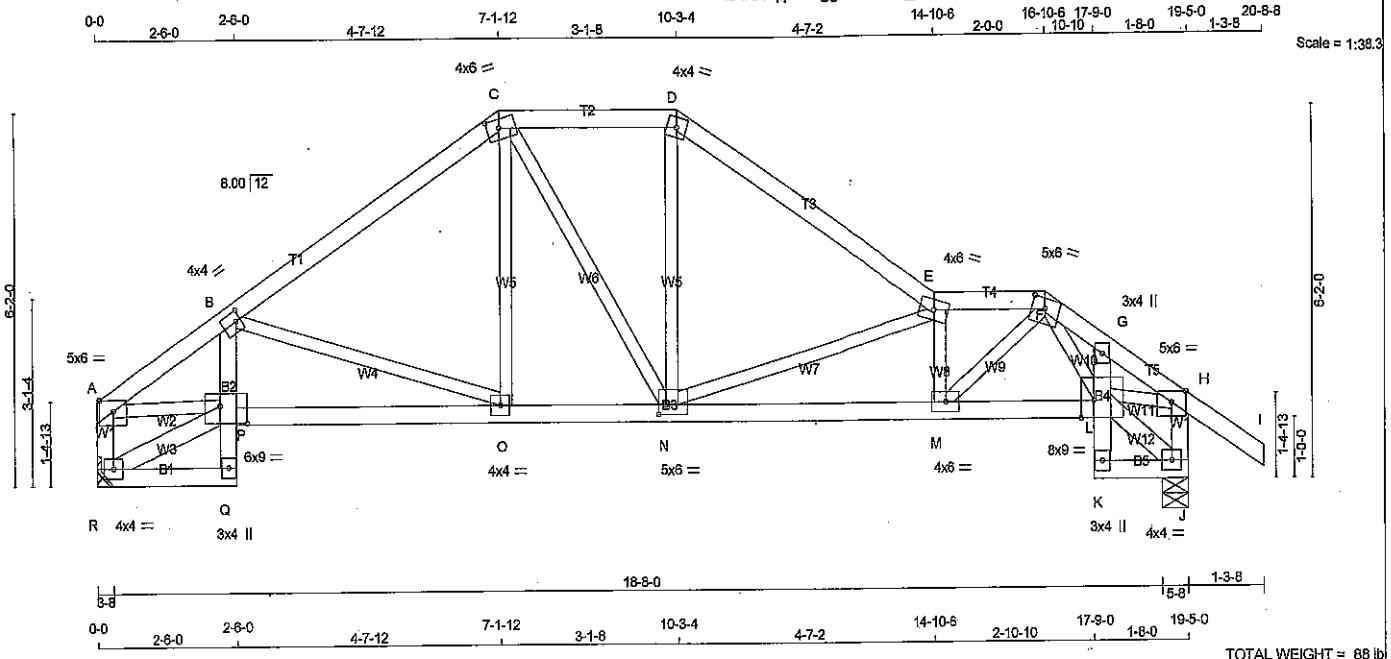

TOTAL WEIGHT = 88 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.75
G	TMV+p	MT20	3.0	4.0		
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWWW-I	MT20	8.0	9.0	Edge	2.75
M	BMWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	2.00	1.50
O	BMWW-t	MT20	4.0	4.0		
P	BVMWW-I	MT20	6.0	8.0	3.50	5.75
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1281	0	1281	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
J	1435	0	1435	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	1120	723 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2071 / 0	-104.9	-104.9	0.24 (1)		4.50
B- C	-1455 / 0	-104.9	-104.9	0.32 (1)		5.07
C- D	-1276 / 0	-104.9	-104.9	0.15 (1)		5.55
D- E	-1516 / 0	-104.9	-104.9	0.32 (1)		5.00
E- F	-2404 / 0	-104.9	-104.9	0.11 (1)		4.36
F- G	-1781 / 0	-104.9	-104.9	0.10 (1)		4.93
G- H	-1787 / 0	-104.9	-104.9	0.14 (1)		4.89
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
R- A	-1219 / 0	0.0	0.0	0.13 (1)		7.26
J- H	-1367 / 0	0.0	0.0	0.14 (1)		6.94
R- Q	0 / 63	-27.5	-27.5	0.05 (2)		10.00
Q- P	0 / 56	0.0	0.0	0.10 (1)		10.00
P- B	0 / 153	0.0	0.0	0.11 (1)		10.00
P- O	0 / 1803	-27.5	-27.5	0.37 (1)		10.00
O- N	0 / 1198	-27.5	-27.5	0.26 (1)		10.00
N- M	0 / 2447	-27.5	-27.5	0.47 (1)		10.00
M- L	0 / 1523	-27.5	-27.5	0.28 (1)		10.00
K- L	0 / 36	0.0	0.0	0.10 (1)		10.00
L- G	-19 / 14	0.0	0.0	0.07 (1)		7.81
K- J	0 / 71	-27.5	-27.5	0.03 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- O	-645 / 0	0.29 (1)
O- C	0 / 309	0.07 (2)
C- N	0 / 150	0.03 (1)
N- D	0 / 436	0.10 (1)
N- E	-1267 / 0	0.55 (1)
M- E	-740 / 0	0.11 (1)
M- F	0 / 1215	0.27 (1)
F- L	-45 / 22	0.01 (1)
R- P	-69 / 0	0.01 (1)
A- P	0 / 1760	0.40 (1)
L- J	-85 / 0	0.01 (1)
L- H	0 / 1436	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.47/1.00 (M-N:1) , WB=0.55/1.00 (E-N:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.38 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 133098
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288711	T41A	1	1	Tamarack - Bayview	
				TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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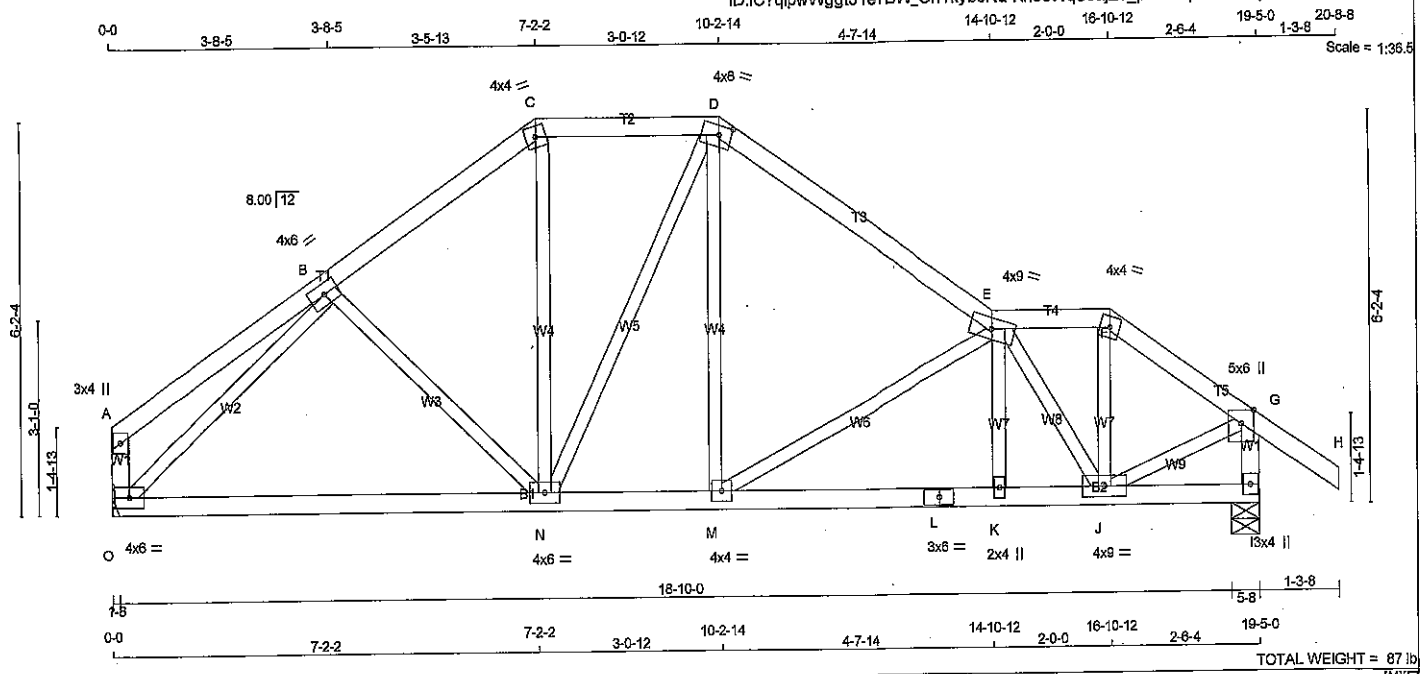

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTWWW-m	MT20	4.0	9.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMW+w	MT20	2.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1286	0	1286	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1430	0	1430	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1019	631 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	1116	720 / 0	194 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-104.9	-104.9	0.21 (1)	10.00	
B- C	-1236 / 0	-104.9	-104.9	0.17 (1)	5.60	
C- D	-1014 / 0	-104.9	-104.9	0.14 (1)	6.07	
D- E	-1266 / 0	-104.9	-104.9	0.32 (1)	5.36	
E- F	-1050 / 0	-104.9	-104.9	0.06 (1)	6.08	
F- G	-1244 / 0	-104.9	-104.9	0.10 (1)	5.56	
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
O- A	-146 / 0	0.0	0.0	0.02 (1)	7.81	
I- G	-1396 / 0	0.0	0.0	0.14 (1)	6.89	
O- N	0 / 1091	-27.5	-27.5	0.42 (2)	10.00	
N- M	0 / 1082	-27.5	-27.5	0.43 (2)	10.00	
M- L	0 / 1612	-27.5	-27.5	0.34 (1)	10.00	
L- K	0 / 1612	-27.5	-27.5	0.34 (1)	10.00	
K- J	0 / 1615	-27.5	-27.5	0.29 (1)	10.00	
J- I	0 / 0	-27.5	-27.5	0.05 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- N	-114 / 75	0.05 (1)
N- C	0 / 367	0.08 (1)
N- D	-107 / 15	0.08 (1)
M- D	0 / 394	0.09 (1)
M- E	-645 / 0	0.34 (1)
K- E	0 / 202	0.05 (3)
E- J	-1004 / 0	0.21 (1)
J- F	0 / 457	0.10 (1)
O- B	-1519 / 0	0.60 (1)
J- G	0 / 1117	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.43/1.00 (M-N:2) , WB=0.60/1.00 (B-O:1) , SSI=0.16/1.00 (N-O:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)

[M][F]

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13323-S1
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T42	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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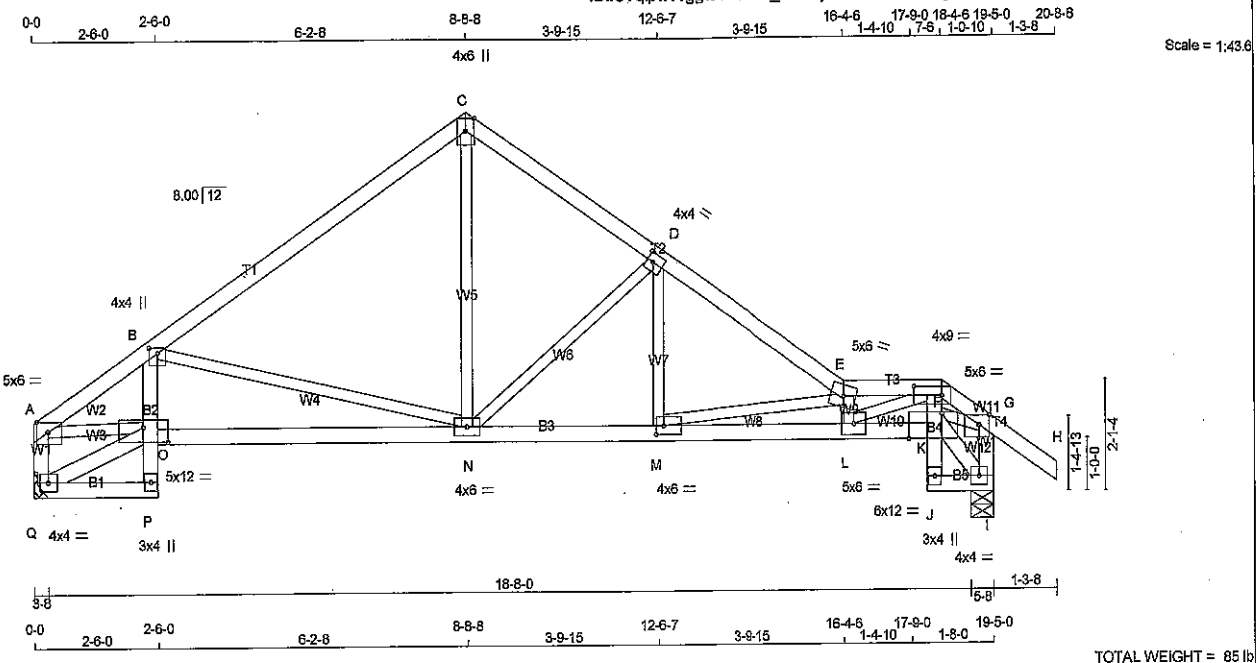

TOTAL WEIGHT = 85 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	2.25	2.75
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	5.0	6.0		
F	TTWW-l	MT20	4.0	9.0	2.00	6.75
G	TMWW-p	MT20	5.0	6.0	2.25	2.50
I	BMWW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-l	MT20	6.0	12.0	Edge	8.00
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.75
N	BMWWW-t	MT20	4.0	6.0		
O	BVMWW-l	MT20	5.0	12.0	3.25	6.00
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1281	0	1281	0	0	HANGER BY OTHERS MIN. SEAT.SIZE: 3-8	
I	1435	0	1435	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	1120	723 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2162 / 0	-104.9	-104.9	0.43 (1)	4.17	
B- C	-1298 / 0	-104.9	-104.9	0.56 (1)	4.93	
C- D	-1308 / 0	-104.9	-104.9	0.21 (1)	5.43	
D- E	-2087 / 0	-104.9	-104.9	0.24 (1)	4.49	
E- F	-3720 / 0	-104.9	-104.9	0.19 (1)	3.53	
F- G	-2368 / 0	-104.9	-104.9	0.16 (1)	4.34	
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q- A	-1218 / 0	0.0	0.0	0.13 (1)	7.26	
I- G	-1301 / 0	0.0	0.0	0.13 (1)	7.08	
Q- P	0 / 66	-27.5	-27.5	0.05 (2)	10.00	
P- O	0 / 56	0.0	0.0	0.10 (1)	10.00	
O- B	0 / 178	0.0	0.0	0.11 (1)	10.00	
O- N	0 / 1915	-27.5	-27.5	0.44 (1)	10.00	
N- M	0 / 1761	-27.5	-27.5	0.42 (2)	10.00	
M- L	0 / 3849	-27.5	-27.5	0.83 (1)	10.00	
L- K	0 / 2402	-27.5	-27.5	0.59 (1)	10.00	
J- K	0 / 28	0.0	0.0	0.19 (1)	10.00	
K- F	0 / 759	0.0	0.0	0.31 (1)	10.00	
J- I	0 / 111	-27.5	-27.5	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- N	-889 / 0	0.70 (1)
N- C	0 / 983	0.22 (1)
N- D	-929 / 0	0.39 (1)
M- D	0 / 579	0.13 (1)
M- E	-2133 / 0	0.60 (1)
L- E	-1000 / 0	0.14 (1)
L- F	0 / 1474	0.33 (1)
Q- O	-73 / 0	0.01 (1)
A- O	0 / 1870	0.42 (1)
K- I	-162 / 0	0.02 (1)
K- G	0 / 1938	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 879 (0.27")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.83/1.00 (L-M:1) , WB=0.70/1.00 (B-N:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.58 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3818
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 133/0-38
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288711	T42A	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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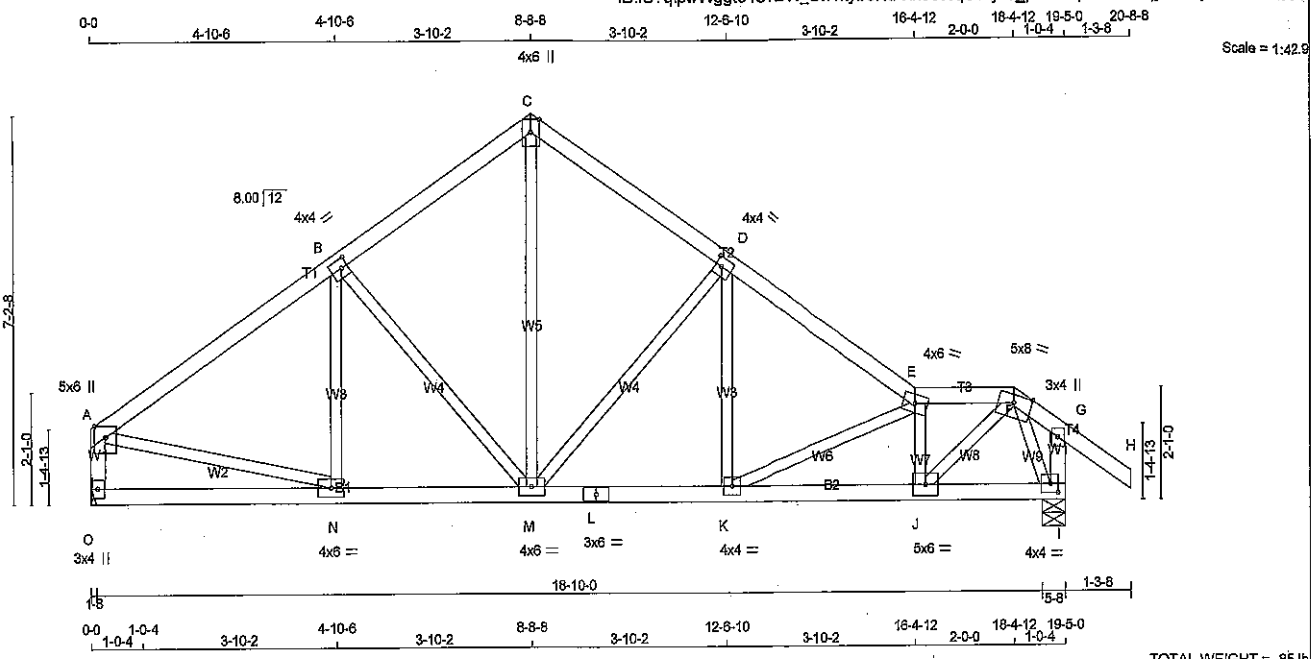

TOTAL WEIGHT = 85 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	8.0	Edge	4.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	2.00	1.75
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1281	0	1281	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1435	0	1435	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1015	627 / 0	194 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	1120	723 / 0	194 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1348 / 0	-104.9	-104.9	0.31 (1)	5.25
B- C	-1110 / 0	-104.9	-104.9	0.27 (1)	5.89
C- D	-1101 / 0	-104.9	-104.9	0.20 (1)	5.80
D- E	-1597 / 0	-104.9	-104.9	0.22 (1)	5.02
E- F	-1740 / 0	-104.9	-104.9	0.08 (1)	5.00
F- G	-55 / 0	-104.9	-104.9	0.10 (1)	6.25
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- A	-1223 / 0	0.0	0.0	0.13 (1)	7.25
I- G	-294 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 0	-27.5	-27.5	0.14 (3)	10.00
N- M	0 / 1145	-27.5	-27.5	0.25 (2)	10.00
M- L	0 / 1353	-27.5	-27.5	0.26 (1)	10.00
L- K	0 / 1353	-27.5	-27.5	0.26 (1)	10.00
K- J	0 / 1795	-27.5	-27.5	0.34 (1)	10.00
J- I	0 / 632	-27.5	-27.5	0.14 (1)	10.00

WEBS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	MAX CSI (LC)
N- B	-121 / 135	0.04 (1)
B- M	-377 / 0	0.22 (1)
M- C	0 / 899	0.20 (1)
M- D	-690 / 0	0.39 (1)
K- D	0 / 343	0.08 (2)
K- E	-488 / 0	0.15 (1)
J- E	-953 / 0	0.14 (1)
A- N	0 / 1173	0.26 (1)
F- I	-1269 / 0	0.19 (1)
J- F	0 / 1518	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.31/1.00 (A-B:1) , BC=0.34/1.00 (J-K:1) , WB=0.39/1.00 (D-M:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.44 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
3878
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13324-S8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T43	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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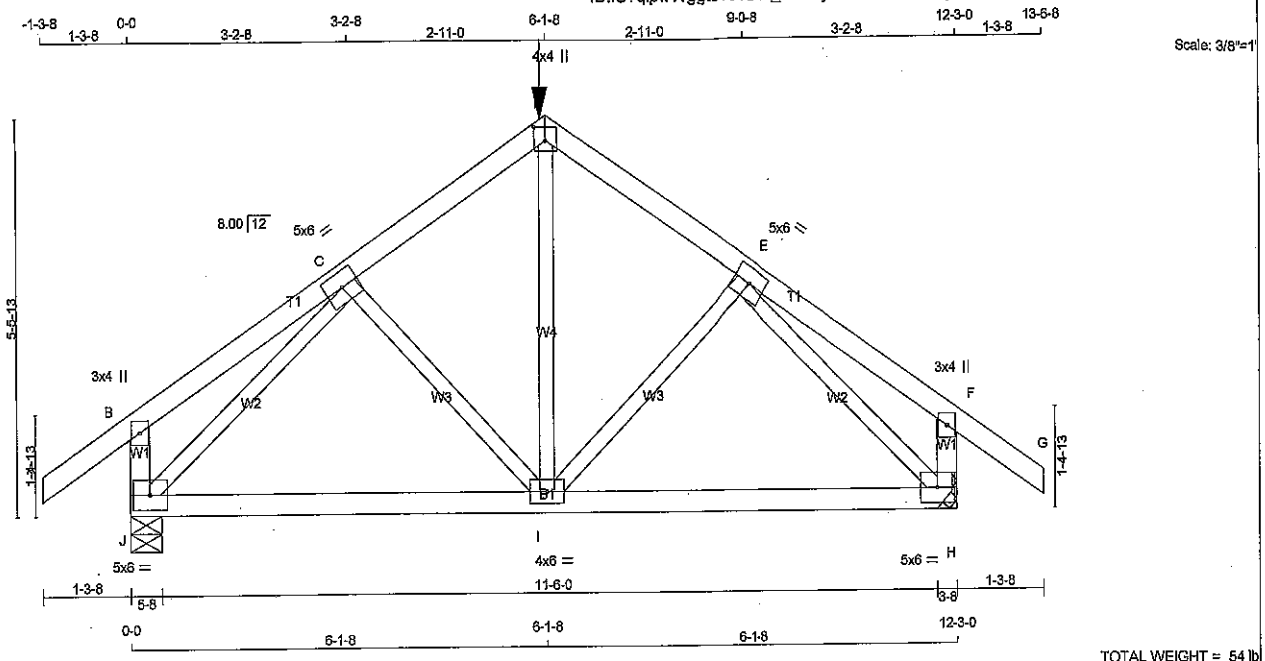

TOTAL WEIGHT = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 990.8 lbs FACTORED DOWN AT 6-1-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1624	0	1624	0	0	5-8	5-8
H	1624	0	1624	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1290	794 / 0	249 / 0	0 / 0	0 / 0	247 / 0	0 / 0
H	1290	794 / 0	249 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	I- D	0 / 593	0.15 (2)
B- C	0 / 17	-104.9	-104.9	0.15 (1)	10.00	I- E	0 / 109	0.03 (3)
C- D	-1482 / 0	-104.9	-104.9	0.18 (1)	5.16	C- I	0 / 109	0.03 (3)
D- E	-1482 / 0	-104.9	-104.9	0.18 (1)	5.16	J- C	-1709 / 0	0.55 (1)
E- F	0 / 17	-104.9	-104.9	0.15 (1)	10.00	E- H	-1709 / 0	0.55 (1)
F- G	0 / 40	-104.9	-104.9	0.15 (1)	10.00			
J- B	-275 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-275 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 1199	-55.9	-55.9	0.79 (2)	10.00			
I- H	0 / 1199	-55.9	-55.9	0.79 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-1-8	-991	-991	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-1-8
END SETBACK = 6-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: PARTIAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.79/1.00 (H-I:2) , WB=0.55/1.00 (E-H:1) , SSI=0.32/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13311-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T44	1	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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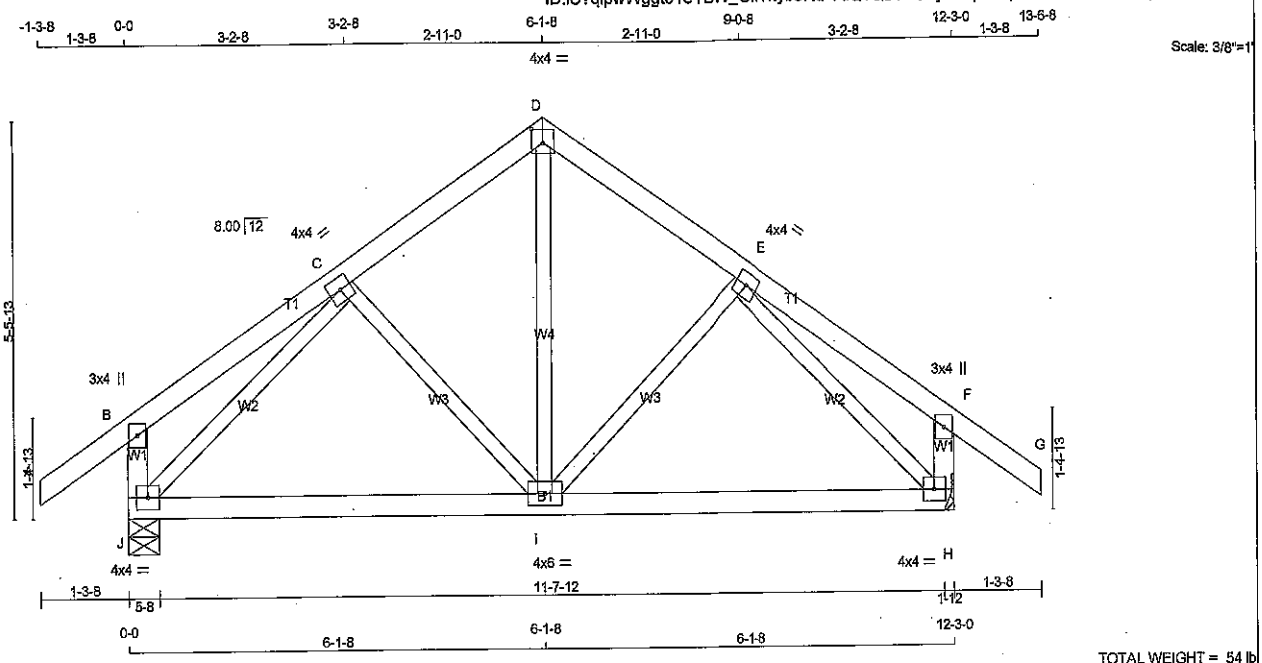

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	955	0	955	0	0	5-8	5-8
H	955	0	955	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-12	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	740	487 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	740	487 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 19	-104.9	-104.9	0.15 (1)	10.00	
C- D	-626 / 0	-104.9	-104.9	0.12 (1)	6.25	
D- E	-626 / 0	-104.9	-104.9	0.12 (1)	6.25	
E- F	0 / 19	-104.9	-104.9	0.15 (1)	10.00	
F- G	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
J- B	-272 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-272 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 609	-27.5	-27.5	0.35 (2)	10.00	
I- H	0 / 609	-27.5	-27.5	0.35 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 425	0.10 (1)
I- E	-151 / 42	0.05 (1)
C- I	-151 / 42	0.05 (1)
J- C	-868 / 0	0.27 (1)
E- H	-868 / 0	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 998 (0.07")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.35/1.00 (H-I:2) , WB=0.27/1.00 (E-H:1) , SSI=0.14/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

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PROVINCE OF ONTARIO

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DWG NO. TAM 133/2-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T45	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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-1-3-8 0-0 6-9-14 12-10-0 18-10-2 25-8-0 26-11-8
1-3-8 6-9-14 6-0-2 6-0-2 6-9-14 1-3-8

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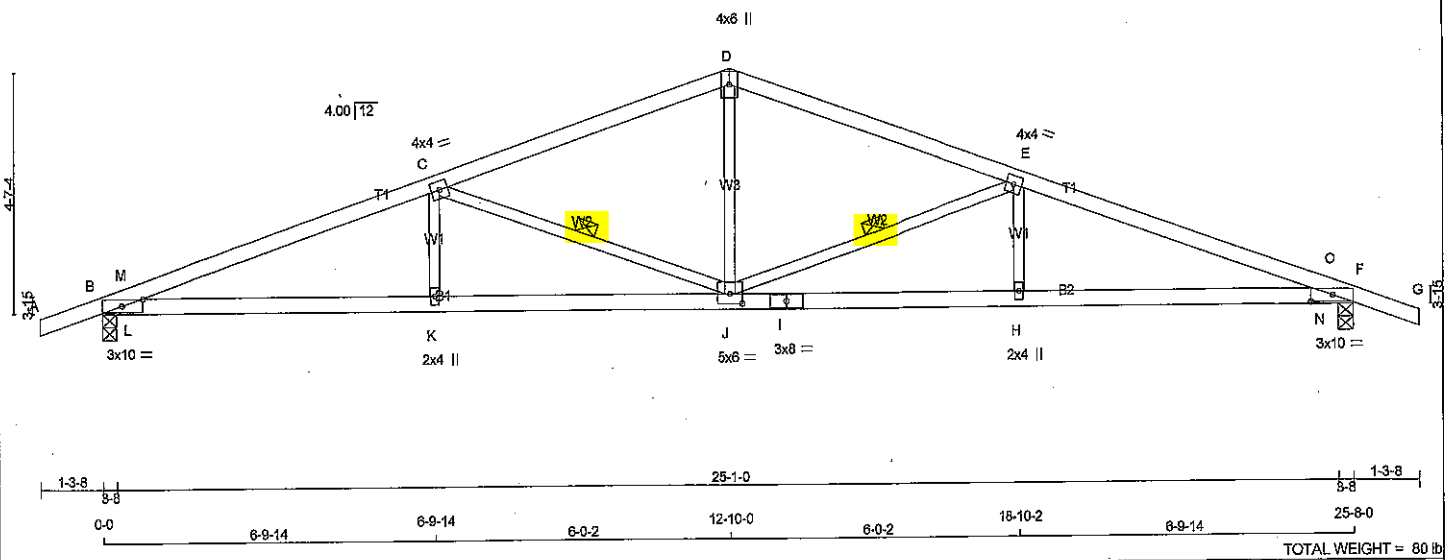

1-3-8 25-1-0 1-3-8
8-8 8-8

0-0 6-9-14 6-9-14 12-10-0 18-10-2 25-8-0
6-0-2 6-0-2 6-9-14

TOTAL WEIGHT = 80 lb

[M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	10.0	1.50	5.00
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	10.0	1.50	5.00
H	BMW+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	5.0	6.0	2.25	3.00
K	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1712	0	1712	0	0	3-8	3-8
F	1712	0	1712	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1185	920 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
F	1185	920 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J, C-J.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- M	-3836 / 0	-104.9	-104.9	0.22 (1)	3.45
M- C	-3802 / 0	-104.9	-104.9	0.74 (1)	2.97
C- D	-2501 / 0	-104.9	-104.9	0.57 (1)	3.78
D- E	-2501 / 0	-104.9	-104.9	0.57 (1)	3.78
E- O	-3802 / 0	-104.9	-104.9	0.74 (1)	2.97
O- F	-3838 / 0	-104.9	-104.9	0.22 (1)	3.45
F- G	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- L	0 / 3624	-17.5	-17.5	0.80 (1)	10.00
L- K	0 / 3624	-17.5	-17.5	0.82 (1)	10.00
K- J	0 / 3624	-17.5	-17.5	0.66 (1)	10.00
J- I	0 / 3624	-17.5	-17.5	0.66 (1)	10.00
I- H	0 / 3624	-17.5	-17.5	0.66 (1)	10.00
H- N	0 / 3624	-17.5	-17.5	0.82 (1)	10.00
N- F	0 / 3624	-17.5	-17.5	0.80 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 1077	0.24 (1)
J- E	-1363 / 0	0.40 (1)
H- E	0 / 144	0.05 (4)
C- J	-1363 / 0	0.40 (1)
K- C	0 / 144	0.05 (4)
L- M	-242 / 66	0.00 (1)
N- O	-242 / 66	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 764 (0.40")

CSI: TC=0.74/1.00 (E-O:1) , BC=0.82/1.00 (H-N:1) , WB=0.40/1.00 (E-J:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.89 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3078
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13313-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T45G	1	1	Tamarack - Bayview TRUSS DESC. Wellington - S36-17 / C	

Tamarack Roof Truss, Burlington

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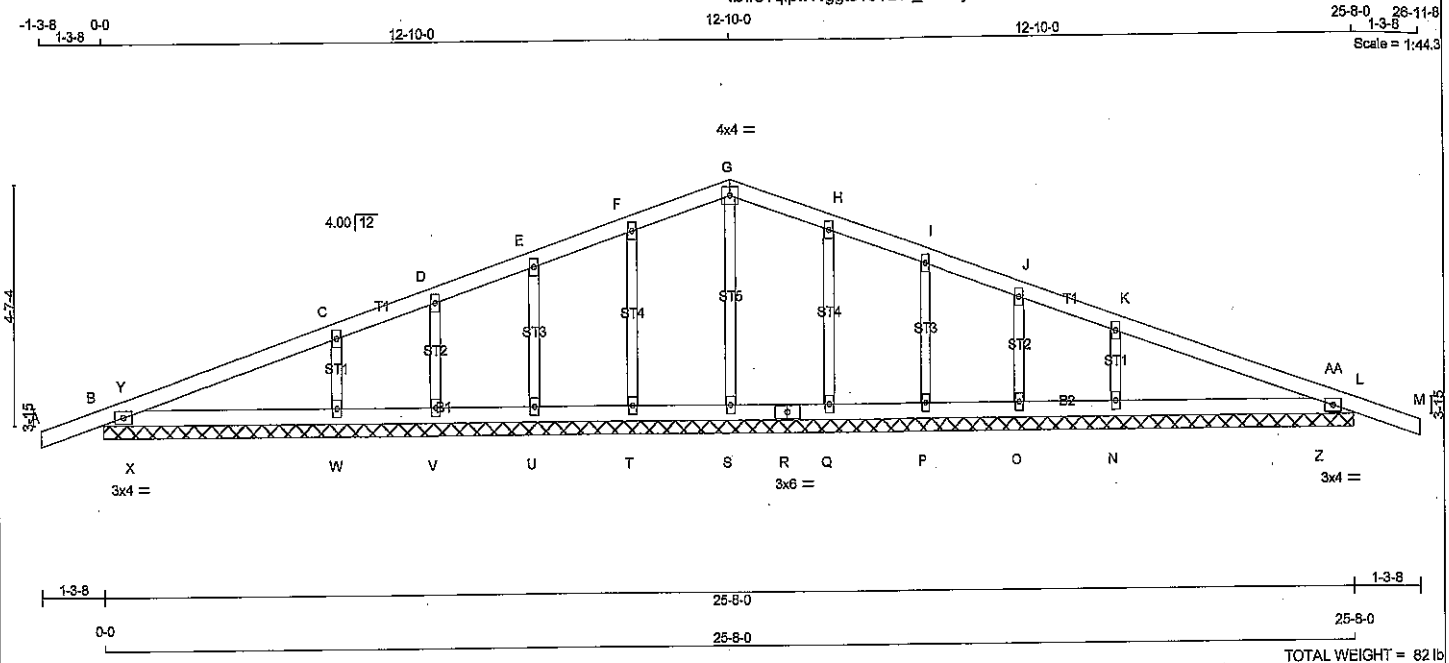

TOTAL WEIGHT = 82 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMB1-I	MT20	3.0	4.0		
N, O, P, Q, S, T, U, V, W						
N	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- Y	-56 / 0	-104.9	-104.9	0.08 (3)	6.25
Y- C	-37 / 0	-104.9	-104.9	0.25 (1)	6.25
C- D	-67 / 0	-104.9	-104.9	0.25 (1)	6.25
D- E	-29 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-32 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-38 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-36 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-32 / 0	-104.9	-104.9	0.06 (1)	6.25
I- J	-29 / 0	-104.9	-104.9	0.06 (1)	6.25
J- K	-67 / 0	-104.9	-104.9	0.25 (1)	6.25
K-AA	-37 / 0	-104.9	-104.9	0.25 (1)	6.25
AA- L	-56 / 0	-104.9	-104.9	0.08 (3)	6.25
L- M	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- X	0 / 49	-27.5	-27.5	0.18 (1)	10.00
X- W	0 / 49	-27.5	-27.5	0.18 (1)	10.00
W- V	0 / 36	-27.5	-27.5	0.15 (1)	10.00
V- U	0 / 34	-27.5	-27.5	0.03 (2)	10.00
U- T	0 / 30	-27.5	-27.5	0.03 (2)	10.00
T- S	0 / 28	-27.5	-27.5	0.03 (2)	10.00
S- R	0 / 28	-27.5	-27.5	0.03 (2)	10.00
R- Q	0 / 28	-27.5	-27.5	0.03 (2)	10.00
Q- P	0 / 30	-27.5	-27.5	0.03 (2)	10.00
P- O	0 / 34	-27.5	-27.5	0.03 (2)	10.00
O- N	0 / 36	-27.5	-27.5	0.15 (1)	10.00
N- Z	0 / 49	-27.5	-27.5	0.18 (1)	10.00
Z- L	0 / 49	-27.5	-27.5	0.18 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- G	-155 / 0	0.05 (1)
T- F	-233 / 0	0.06 (1)
U- E	-229 / 0	0.04 (1)
V- D	-96 / 11	0.02 (1)
W- C	-458 / 0	0.07 (1)
Q- H	-233 / 0	0.06 (1)
P- I	-229 / 0	0.04 (1)
O- J	-96 / 11	0.02 (1)
N- K	-458 / 0	0.07 (1)
X- Y	-192 / 94	0.00 (1)
Z-AA	-192 / 94	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (K-AA:1) , BC=0.18/1.00 (N-Z:1) , WB=0.07/1.00 (K-N:1) , SSI=0.20/1.00 (K-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (G) (INPUT = 0.90)
JSI METAL= 0.08 (N) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 13314-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T70	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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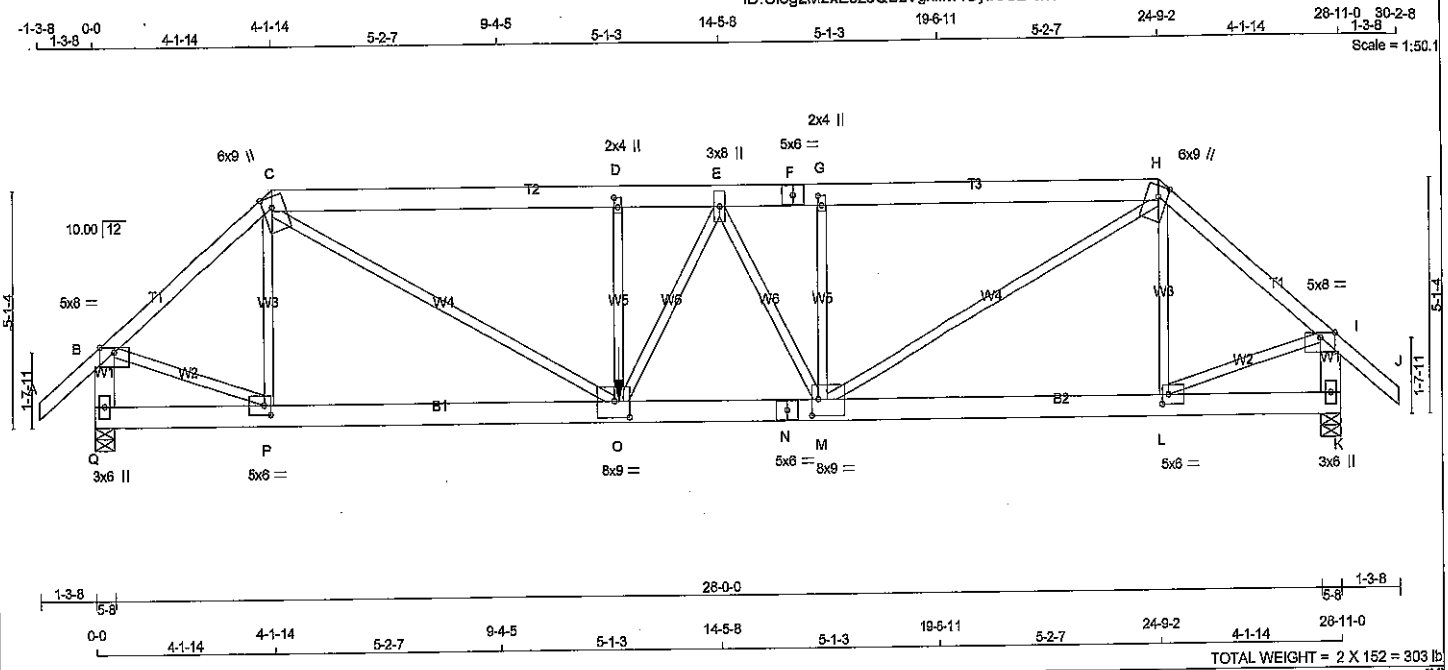

TOTAL WEIGHT = 2 X 152 = 303 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	TOP
C- F	2	12	TOP
F- H	2	12	TOP
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(0.0)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	2.75	2.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWW+t	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	6.0	9.0	2.75	2.50
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.75
M	BMWWW-t	MT20	8.0	9.0	4.25	1.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	8.0	9.0	4.25	4.25
P	BMWW-t	MT20	5.0	6.0	2.50	1.75
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	4952	0	4952	0	0	5-8	5-8
K	4148	0	4148	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	3877	2481 / 0	686 / 0	0 / 0	0 / 0	710 / 0	0 / 0
K	3249	2078 / 0	576 / 0	0 / 0	0 / 0	595 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-5293 / 0	-104.9	-104.9	0.32 (1)	3.97	
C- D	-10457 / 0	-104.9	-104.9	0.53 (1)	3.42	
D- E	-10457 / 0	-104.9	-104.9	0.27 (1)	3.62	
E- F	-8446 / 0	-104.9	-104.9	0.28 (1)	3.96	
F- G	-8446 / 0	-104.9	-104.9	0.28 (1)	3.96	
G- H	-8446 / 0	-104.9	-104.9	0.41 (1)	3.88	
H- I	-4346 / 0	-104.9	-104.9	0.28 (1)	4.36	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
Q- B	-4987 / 0	0.0	0.0	0.18 (1)	6.52	
K- I	-4143 / 0	0.0	0.0	0.15 (1)	7.00	
Q- P	0 / 0	-27.5	-27.5	0.12 (2)	10.00	
P- O	0 / 4097	-27.5	-27.5	0.37 (1)	10.00	
O- N	0 / 9406	-27.5	-27.5	0.70 (1)	10.00	
N- M	0 / 9406	-27.5	-27.5	0.70 (1)	10.00	
M- L	0 / 3363	-27.5	-27.5	0.29 (1)	10.00	
L- K	0 / 0	-27.5	-27.5	0.10 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-923 / 0	0.17 (1)
C- O	0 / 7371	0.91 (1)
O- D	-774 / 0	0.13 (1)
O- E	0 / 2341	0.29 (1)
E- M	-2140 / 0	0.47 (1)
M- G	-871 / 0	0.15 (1)
M- H	0 / 5890	0.73 (1)
L- H	-761 / 0	0.14 (1)
B- P	0 / 4257	0.53 (1)
L- I	0 / 3494	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. TYPE
O	12-1-8	-4980	-4980	—	FRONT	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.70/1.00 (M-O:1) , WB=0.91/1.00 (C-O:1) , SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.80 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3816 S. KATSOULAROS PROVINCE OF ONTARIO

DWG NO. TAM13289-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T70	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4980.1 lbs FACTORED DOWN AT 12-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SITE COPY

LICENSED PROFESSIONAL ENGINEER
2876
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13289-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T71	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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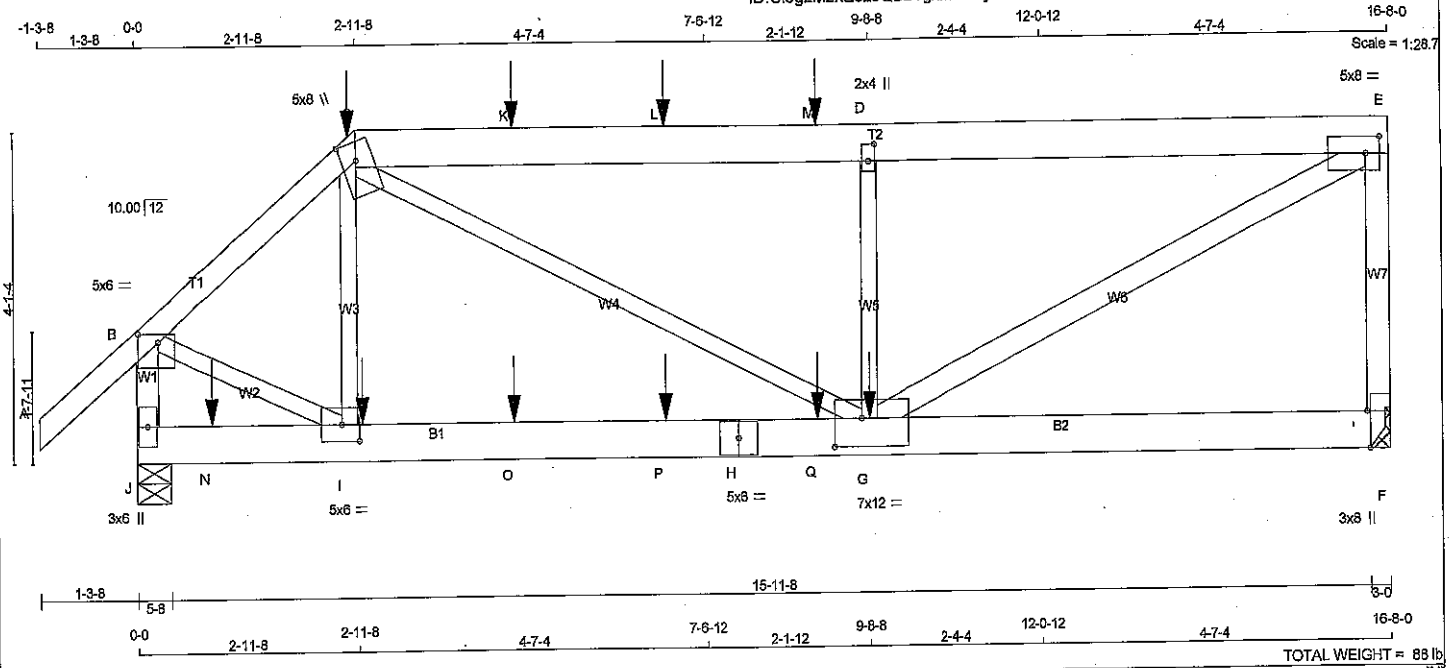


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.25
C	TTWW+m	MT20	5.0	8.0	2.75	2.25
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMVW-t	MT20	5.0	8.0	2.50	2.25
F	BMV1+t	MT20	3.0	8.0	Edge	0.50
G	BMWWW-t	MT20	7.0	12.0	4.25	4.25
H	BS-t	MT20	5.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 129.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 5-0-4, AND 126.3 lbs FACTORED DOWN AT 7-0-4, AND 126.3 lbs FACTORED DOWN AT 9-0-4 ON TOP CHORD, AND 67.8 lbs FACTORED DOWN AT 1-0-4, 67.8 lbs FACTORED DOWN AT 3-0-4, 67.8 lbs FACTORED DOWN AT 5-0-4, 67.8 lbs FACTORED DOWN AT 7-0-4, AND 67.8 lbs FACTORED DOWN AT 9-0-4, AND 1602.1 lbs FACTORED DOWN AT 9-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	2395	0	2395	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
J	2465	0	2465	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1898	1177 / 0	359 / 0	0 / 0	0 / 0	380 / 0	0 / 0
J	1955	1206 / 0	374 / 0	0 / 0	0 / 0	375 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00		I- C	-405 / 156	0.10 (1)
B- C	-2247 / 0	-104.9	-104.9	0.25 (1)	4.28		C- G	0 / 2406	0.60 (1)
C- K	-3858 / 0	-104.9	-104.9	0.63 (1)	3.65		B- I	0 / 1883	0.47 (1)
K- L	-3858 / 0	-104.9	-104.9	0.63 (1)	3.65		G- D	-1138 / 0	0.27 (1)
L- M	-3858 / 0	-104.9	-104.9	0.63 (1)	3.65		G- E	0 / 4355	0.77 (1)
M- D	-3858 / 0	-104.9	-104.9	0.63 (1)	3.65				
D- E	-3858 / 0	-104.9	-104.9	0.80 (1)	3.71				
F- E	-2298 / 0	0.0	0.0	0.53 (1)	5.57				
J- B	-2472 / 0	0.0	0.0	0.28 (1)	5.39				
J- N	0 / 0	-27.5	-27.5	0.20 (2)	10.00				
N- I	0 / 0	-27.5	-27.5	0.20 (2)	10.00				
I- O	0 / 1743	-27.5	-27.5	0.38 (1)	10.00				
O- P	0 / 1743	-27.5	-27.5	0.38 (1)	10.00				
P- H	0 / 1743	-27.5	-27.5	0.38 (1)	10.00				
H- Q	0 / 1743	-27.5	-27.5	0.38 (1)	10.00				
Q- G	0 / 1743	-27.5	-27.5	0.38 (1)	10.00				
G- F	0 / 0	-27.5	-27.5	0.18 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-130	-130	---	BACK	VERT	TOTAL
G	9-8-8	-1802	-1802	---	BACK	VERT	TOTAL
I	3-0-4	-39	-68	---	BACK	VERT	TOTAL
K	5-0-4	-126	-126	---	BACK	VERT	TOTAL
L	7-0-4	-126	-126	---	BACK	VERT	TOTAL
M	9-0-4	-126	-126	---	BACK	VERT	TOTAL
N	1-0-4	-39	-68	---	BACK	VERT	TOTAL
O	5-0-4	-39	-68	---	BACK	VERT	TOTAL
P	7-0-4	-39	-68	---	BACK	VERT	TOTAL
Q	9-0-4	-39	-68	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.63/1.00 (C-D:1) , BC=0.38/1.00 (G-I:1) , WB=0.77/1.00 (E-G:1) , SSI=0.45/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

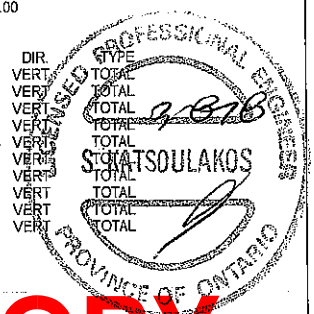
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.72 (E) (INPUT = 1.00)



DWG NO. TAM 13290 -T8
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T72	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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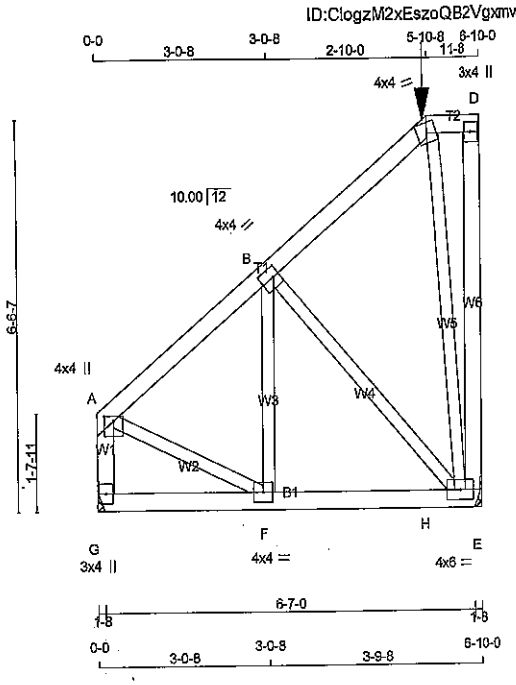


Scale = 1:38.6

TOTAL WEIGHT = 40 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	4.0	6.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 449.5 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1015	0	1015	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	610	0	610	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	788	515 / 0	132 / 0	0 / 0	0 / 0	140 / 0	0 / 0
G	515	265 / 0	132 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-420 / 0	-104.9	-104.9	0.17 (1)	6.25	F- B	0 / 311	0.08 (3)
B- C	-142 / 0	-104.9	-104.9	0.17 (1)	6.25	B- E	-358 / 0	0.16 (1)
C- D	0 / 0	-203.3	-203.3	0.03 (1)	10.00	C- E	-588 / 0	0.42 (1)
E- D	-97 / 0	0.0	0.0	0.09 (1)	7.81	A- F	0 / 371	0.09 (1)
G- A	-553 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-53.3	-53.3	0.20 (3)	10.00			
F- H	0 / 341	-53.3	-53.3	0.24 (2)	10.00			
H- E	0 / 341	-53.3	-53.3	0.24 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-450	-450	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-10-8
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.24/1.00 (E-F:2) , WB=0.42/1.00 (C-E:1) , SSI=0.18/1.00 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

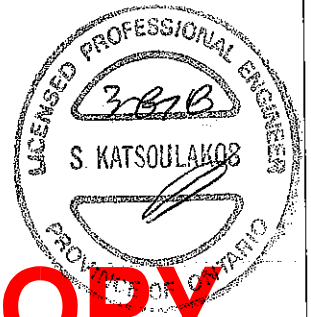
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (A) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

DWG NO. TAM 7329-18

STRUCTURAL COMPONENT ONLY



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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T73	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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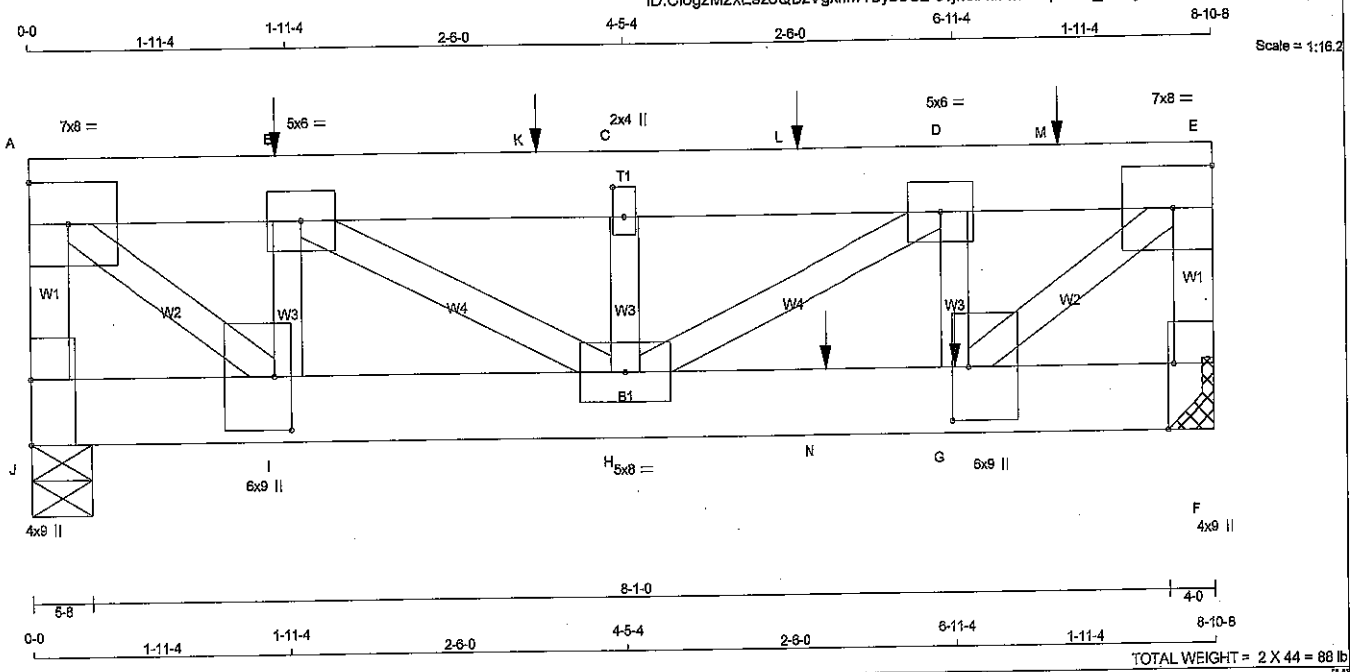

TOTAL WEIGHT = 2 X 44 = 88 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J- A	1	12	TOP
E- F	1	12	TOP
A- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	7.0	8.0		Edge
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	7.0	8.0		Edge
F	BMV1+t	MT20	4.0	9.0	Edge	0.50
G	BMWW+t	MT20	6.0	9.0	4.50	1.50
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW+t	MT20	6.0	9.0	4.50	1.50
J	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1163.5 lbs FACTORED DOWN AT 1-10-4, 851.5 lbs FACTORED DOWN AT 3-9-8, AND 851.5 lbs FACTORED DOWN AT 5-8-12, AND 851.5 lbs FACTORED DOWN AT 7-8-12 ON TOP CHORD, AND 987.2 lbs FACTORED DOWN AT 5-11-4, AND 2367.2 lbs FACTORED DOWN AT 6-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	3240	0	3240	0	0	5-8	5-8
F	5008	0	5008	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2547	1612 / 0	462 / 0	0 / 0	0 / 0	472 / 0	0 / 0
F	3928	2500 / 0	704 / 0	0 / 0	0 / 0	724 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
J- A	-3130 / 0	0.0	0.0	0.18 (1)	6.55	G- D	-1497 / 0	0.12 (1)
A- B	-3765 / 0	-104.9	-104.9	0.05 (1)	5.78	I- B	-3027 / 0	0.24 (1)
B- K	-6185 / 0	-104.9	-104.9	0.21 (1)	4.58	A- I	0 / 4812	0.60 (1)
K- C	-6185 / 0	-104.9	-104.9	0.21 (1)	4.58	G- E	0 / 8997	0.87 (1)
C- L	-6185 / 0	-104.9	-104.9	0.28 (1)	4.49	H- C	-1418 / 0	0.11 (1)
L- D	-6185 / 0	-104.9	-104.9	0.28 (1)	4.49	B- H	0 / 2843	0.35 (1)
D- M	-5475 / 0	-104.9	-104.9	0.22 (1)	4.79	H- D	0 / 834	0.10 (1)
M- E	-5475 / 0	-104.9	-104.9	0.22 (1)	4.79			
F- E	-4859 / 0	0.0	0.0	0.28 (1)	5.43			
J- I	0 / 0	-27.5	-27.5	0.06 (1)	10.00			
I- H	0 / 3765	-27.5	-27.5	0.31 (1)	10.00			
H- N	0 / 5475	-27.5	-27.5	0.65 (1)	10.00			
N- G	0 / 5475	-27.5	-27.5	0.65 (1)	10.00			
G- F	0 / 0	-27.5	-27.5	0.09 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	1-10-4	-1163	-1163	---	TOP	VERT	TOTAL
G	6-11-4	-2367	-2367	---	FRONT	VERT	TOTAL
K	3-9-8	-852	-852	---	TOP	VERT	TOTAL
L	5-8-12	-852	-852	---	TOP	VERT	TOTAL
M	7-8-12	-852	-852	---	TOP	VERT	TOTAL
N	5-11-4	-987	-987	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.65/1.00 (G-H:1) , WB=0.87/1.00 (E-G:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13292-S7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T74	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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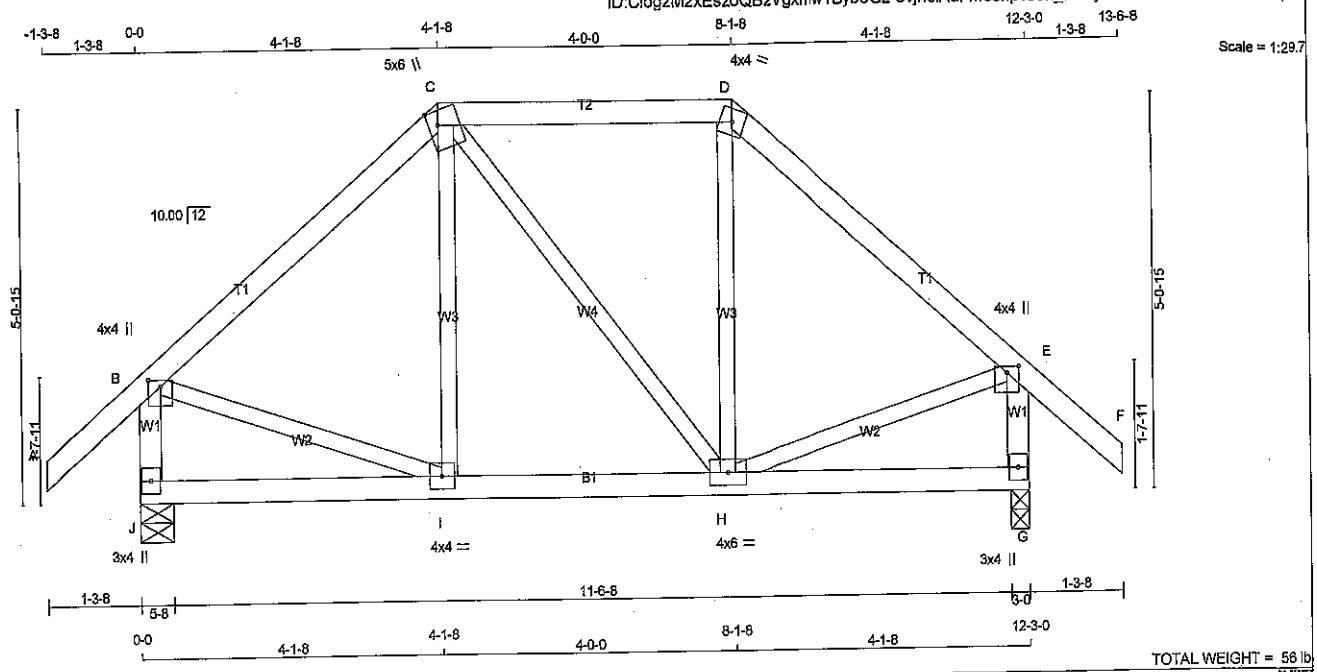

TOTAL WEIGHT = 56 lb

[M]IF

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	956	0	956	0	0	5-8	5-8
G	956	0	956	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
G	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-621 / 0	-104.9	-104.9	0.23 (1)	6.25	
C- D	-476 / 0	-104.9	-104.9	0.22 (1)	6.25	
D- E	-621 / 0	-104.9	-104.9	0.23 (1)	6.25	
E- F	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- B	-908 / 0	0.0	0.0	0.10 (1)	7.81	
G- E	-908 / 0	0.0	0.0	0.10 (1)	7.81	
J- I	0 / 0	-27.5	-27.5	0.10 (3)	10.00	
I- H	0 / 476	-27.5	-27.5	0.15 (2)	10.00	
H- G	0 / 0	-27.5	-27.5	0.11 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-27 / 143	0.03 (3)
C- H	0 / 0	0.00 (1)
H- D	-27 / 144	0.03 (3)
B- I	0 / 498	0.11 (1)
H- E	0 / 498	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.15/1.00 (H-I:2) , WB=0.11/1.00 (B-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (H) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13293-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T74Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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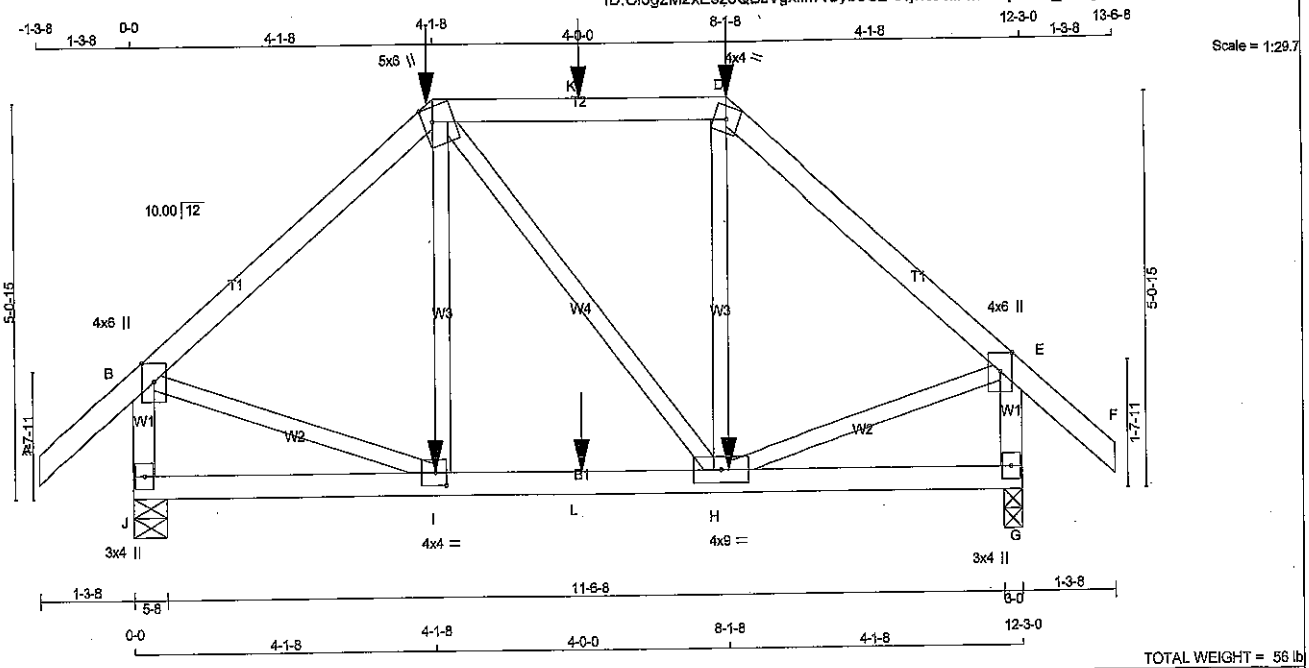

TOTAL WEIGHT = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 212.4 lbs FACTORED DOWN AT 4-1-8, AND 212.4 lbs FACTORED DOWN AT 8-1-8, AND 111.5 lbs FACTORED DOWN AT 6-1-8 ON TOP CHORD, AND 50.5 lbs FACTORED DOWN AT 4-1-8, AND 50.5 lbs FACTORED DOWN AT 6-1-8, AND 50.5 lbs FACTORED DOWN AT 8-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1269	0	1269	0	0	5-8	5-8
G	1268	0	1268	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	988	641 / 0	170 / 0	0 / 0	0 / 0	178 / 0	0 / 0
G	988	641 / 0	170 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-978 / 0	-104.9	-104.9	0.37 (1)	5.76
C- K	-748 / 0	-104.9	-104.9	0.50 (1)	6.05
K- D	-748 / 0	-104.9	-104.9	0.50 (1)	6.05
D- E	-978 / 0	-104.9	-104.9	0.37 (1)	5.76
E- F	0 / 47	-104.9	-104.9	0.16 (1)	10.00
J- B	-1221 / 0	0.0	0.0	0.14 (1)	7.22
G- E	-1221 / 0	0.0	0.0	0.14 (1)	7.22
J- I	0 / 0	-27.5	-27.5	0.13 (3)	10.00
I- L	0 / 748	-27.5	-27.5	0.22 (2)	10.00
L- H	0 / 748	-27.5	-27.5	0.22 (2)	10.00
H- G	0 / 0	-27.5	-27.5	0.13 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-66 / 186	0.05 (3)
C- H	0 / 0	0.00 (1)
H- D	-66 / 187	0.05 (3)
B- I	0 / 784	0.19 (1)
H- E	0 / 784	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-8	-212	-212	—	FRONT	VERT	TOTAL
D	8-1-8	-212	-212	—	FRONT	VERT	TOTAL
H	8-1-8	-29	-50	—	FRONT	VERT	TOTAL
I	4-1-8	-29	-50	—	FRONT	VERT	TOTAL
K	6-1-8	-111	-111	—	FRONT	VERT	TOTAL
L	6-1-8	-29	-50	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (C-D:1) , BC=0.22/1.00 (H-I:2) , WB=0.19/1.00 (B-I:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.28 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13294-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T75	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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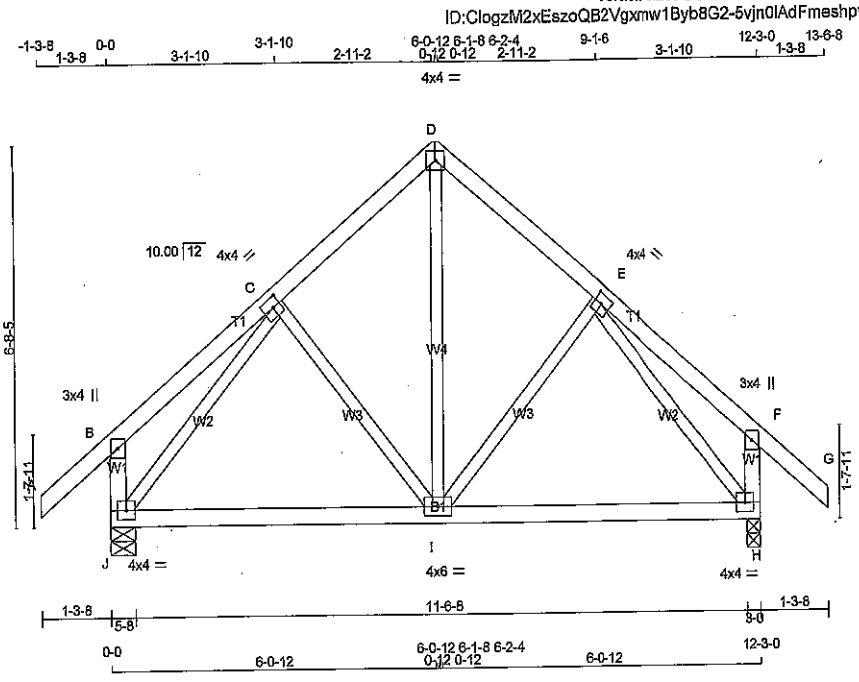

TOTAL WEIGHT = 59 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00	
C- D	-552 / 0	-104.9	-104.9	0.12 (1)	6.25	
D- E	-552 / 0	-104.9	-104.9	0.12 (1)	6.25	
E- F	0 / 23	-104.9	-104.9	0.15 (1)	10.00	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-266 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 493	-27.5	-27.5	0.34 (2)	10.00	
I- H	0 / 494	-27.5	-27.5	0.34 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- I	-137 / 38	0.05 (1)
I- E	-137 / 38	0.05 (1)
J- C	-797 / 0	0.30 (1)
E- H	-797 / 0	0.30 (1)
I- D	0 / 420	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.34/1.00 (H-I:2) , WB=0.30/1.00 (E-H:1) , SSI=0.14/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3820
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13295-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288660	T77	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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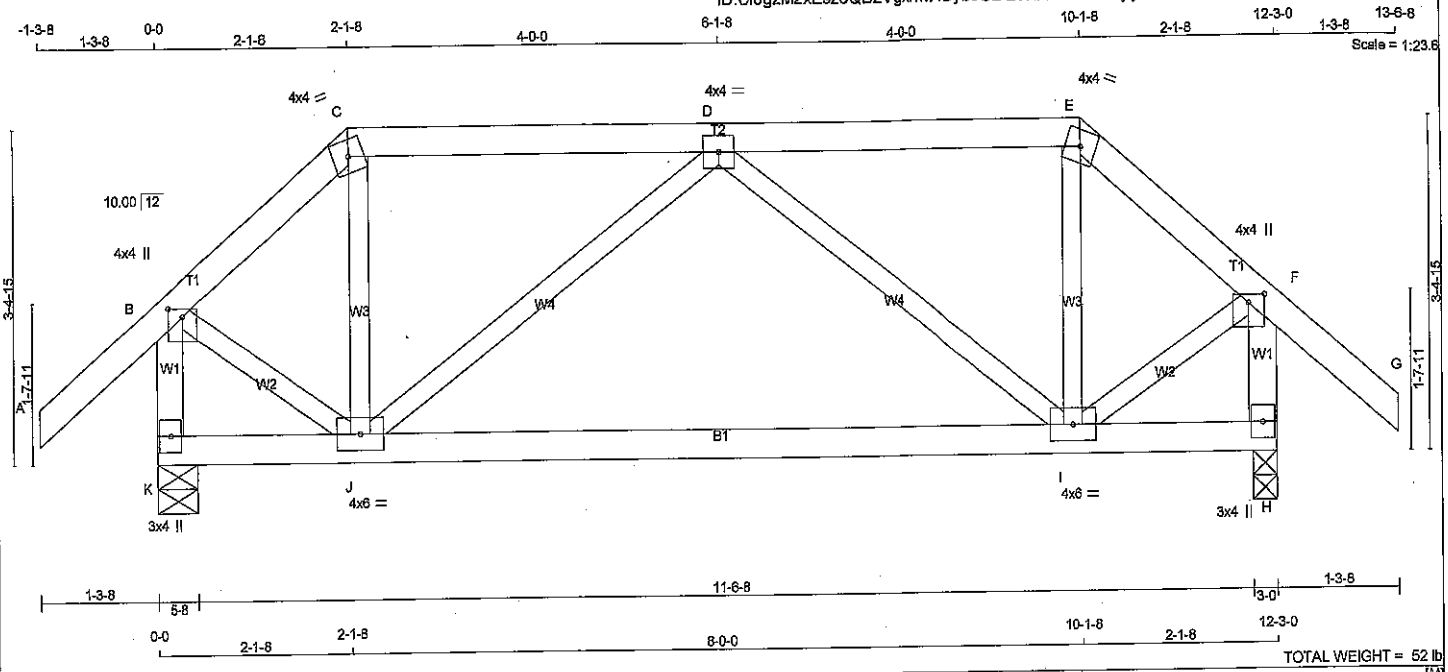


TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		J- C	0 / 207	0.05 (2)
B- C	-659 / 0	-104.9	-104.9	0.09 (1)	6.25		J- D	-411 / 0	0.17 (1)
C- D	-509 / 0	-104.9	-104.9	0.28 (1)	6.25		D- I	-410 / 0	0.17 (1)
D- E	-509 / 0	-104.9	-104.9	0.28 (1)	6.25		I- E	0 / 208	0.05 (2)
E- F	-659 / 0	-104.9	-104.9	0.09 (1)	6.25		B- J	0 / 584	0.13 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00		I- F	0 / 584	0.13 (1)
K- B	-977 / 0	0.0	0.0	0.10 (1)	7.81				
H- F	-977 / 0	0.0	0.0	0.10 (1)	7.81				
K- J	0 / 0	-27.5	-27.5	0.28 (3)	10.00				
J- I	0 / 829	-27.5	-27.5	0.36 (2)	10.00				
I- H	0 / 0	-27.5	-27.5	0.28 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.36/1.00 (I-J:2) , WB=0.17/1.00 (D-J:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

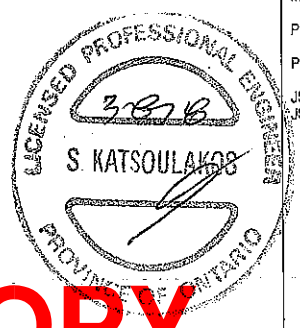
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 13296-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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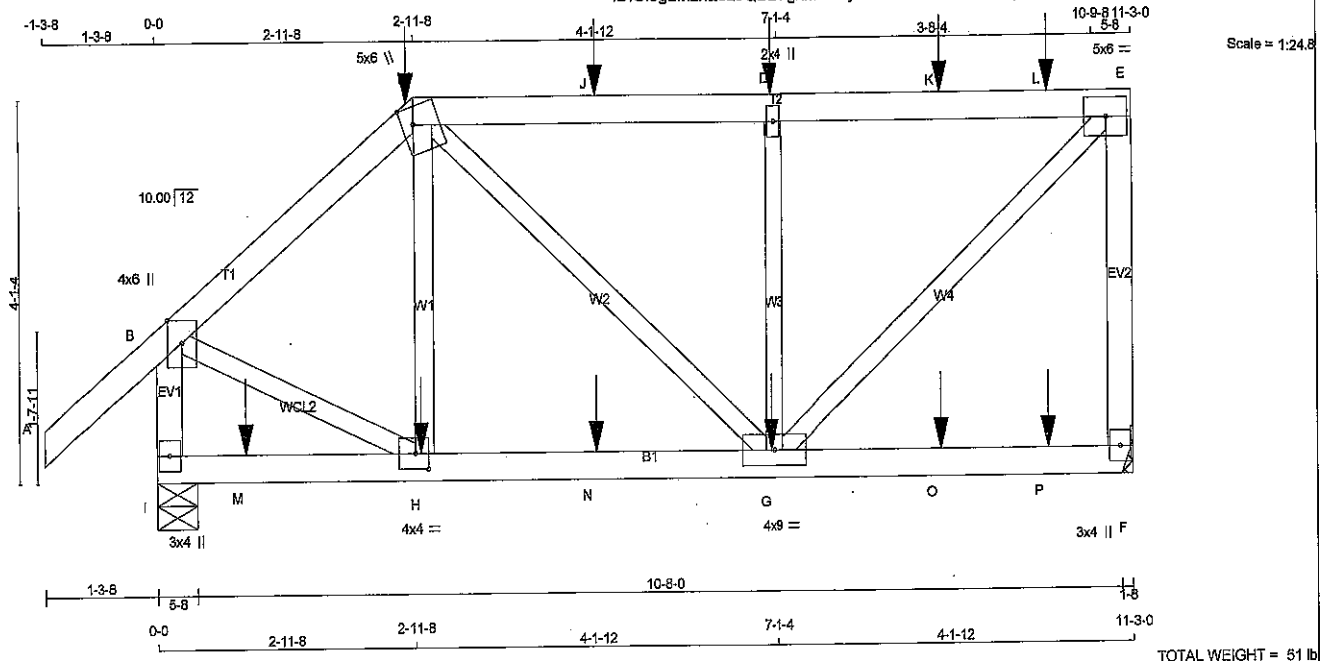

TOTAL WEIGHT = 51 lb

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 189.3 lbs FACTORED DOWN AT 2-11-8, 129.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 5-0-4, 126.3 lbs FACTORED DOWN AT 7-0-4, AND 126.3 lbs FACTORED DOWN AT 9-0-4, AND 127.4 lbs FACTORED DOWN AT 10-3-4 ON TOP CHORD, AND 67.8 lbs FACTORED DOWN AT 1-0-4, 67.8 lbs FACTORED DOWN AT 3-0-4, 67.8 lbs FACTORED DOWN AT 5-0-4, 67.8 lbs FACTORED DOWN AT 7-0-4, AND 67.8 lbs FACTORED DOWN AT 9-0-4, AND 68.3 lbs FACTORED DOWN AT 10-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1306	0	1306	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	1390	0	1390	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1045	629 / 0	210 / 0	0 / 0	0 / 0	206 / 0	0 / 0
I	1106	676 / 0	215 / 0	0 / 0	0 / 0	215 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-1068 / 0	-104.9	-104.9	0.19 (1)	5.84
C- J	-1008 / 0	-104.9	-104.9	0.53 (1)	5.36
J- D	-1008 / 0	-104.9	-104.9	0.53 (1)	5.36
D- K	-1009 / 0	-104.9	-104.9	0.53 (1)	5.35
K- L	-1009 / 0	-104.9	-104.9	0.53 (1)	5.35
L- E	-1009 / 0	-104.9	-104.9	0.53 (1)	5.35
F- E	-1216 / 0	0.0	0.0	0.33 (1)	7.23
I- B	-1336 / 0	0.0	0.0	0.16 (1)	6.97
I- M	0 / 0	-27.5	-27.5	0.11 (3)	10.00
M- H	0 / 0	-27.5	-27.5	0.11 (3)	10.00
H- N	0 / 813	-27.5	-27.5	0.31 (2)	10.00
N- G	0 / 813	-27.5	-27.5	0.31 (2)	10.00
G- O	0 / 0	-27.5	-27.5	0.21 (3)	10.00
O- P	0 / 0	-27.5	-27.5	0.21 (3)	10.00
P- F	0 / 0	-27.5	-27.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-171 / 140	0.05 (1)
C- G	0 / 266	0.07 (1)
G- D	-876 / 0	0.24 (1)
G- E	0 / 1364	0.34 (1)
B- H	0 / 889	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-189	-189	—	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	—	BACK	VERT	TOTAL
D	7-0-4	-126	-126	—	BACK	VERT	TOTAL
G	7-0-4	-39	-68	—	BACK	VERT	TOTAL
H	3-0-4	-39	-68	—	BACK	VERT	TOTAL
J	5-0-4	-126	-126	—	BACK	VERT	TOTAL
K	9-0-4	-126	-126	—	BACK	VERT	TOTAL
L	10-3-4	-127	-127	—	BACK	VERT	TOTAL
M	1-0-4	-39	-68	—	BACK	VERT	TOTAL
N	5-0-4	-39	-68	—	BACK	VERT	TOTAL
O	9-0-4	-39	-68	—	BACK	VERT	TOTAL
P	10-3-4	-40	-68	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.31/1.00 (G-H:2) , WB=0.34/1.00 (E-G:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
ARTSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13280-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288659	T101	1	1	44755 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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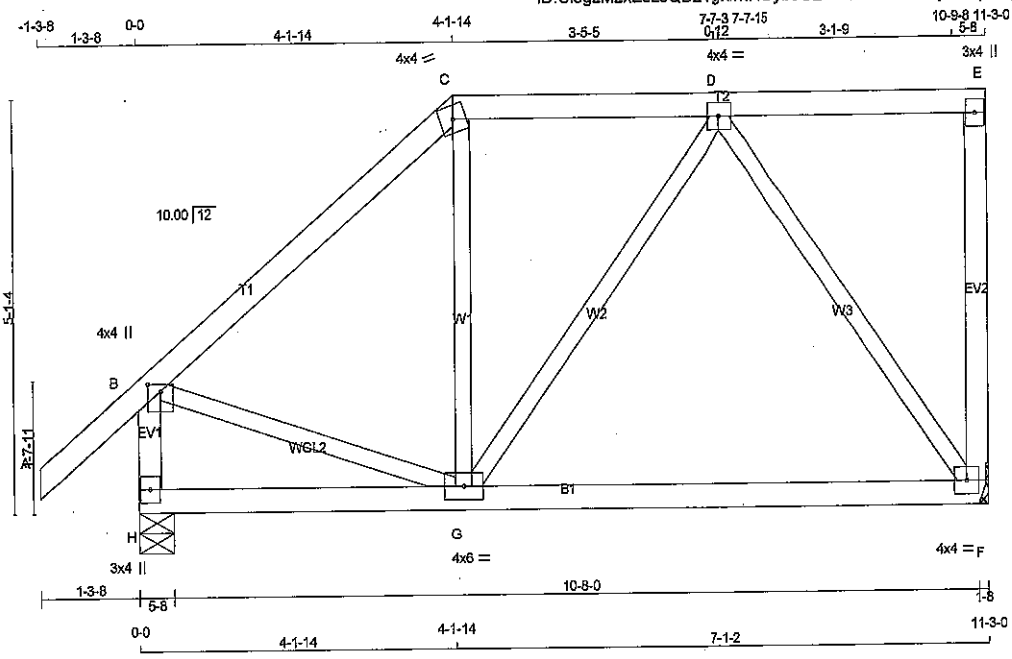

TOTAL WEIGHT = 52 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	745	0	745	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	890	0	890	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	590	365 / 0	113 / 0	0 / 0	0 / 0	113 / 0	0 / 0
H	689	455 / 0	113 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-566 / 0	-104.9	-104.9	0.32 (1)	6.25
C- D	-434 / 0	-104.9	-104.9	0.22 (1)	6.25
D- E	0 / 0	-104.9	-104.9	0.22 (1)	10.00
F- E	-144 / 0	0.0	0.0	0.06 (1)	7.81
H- B	-860 / 0	0.0	0.0	0.09 (1)	7.81
H- G	0 / 0	-27.5	-27.5	0.30 (3)	10.00
G- F	0 / 389	-27.5	-27.5	0.33 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 151	0.03 (3)
G- D	0 / 163	0.04 (3)
D- F	-651 / 0	0.39 (1)
B- G	0 / 454	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 941 (0.14")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.33/1.00 (F-G:2) , WB=0.39/1.00 (D-F:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1320 / -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T102	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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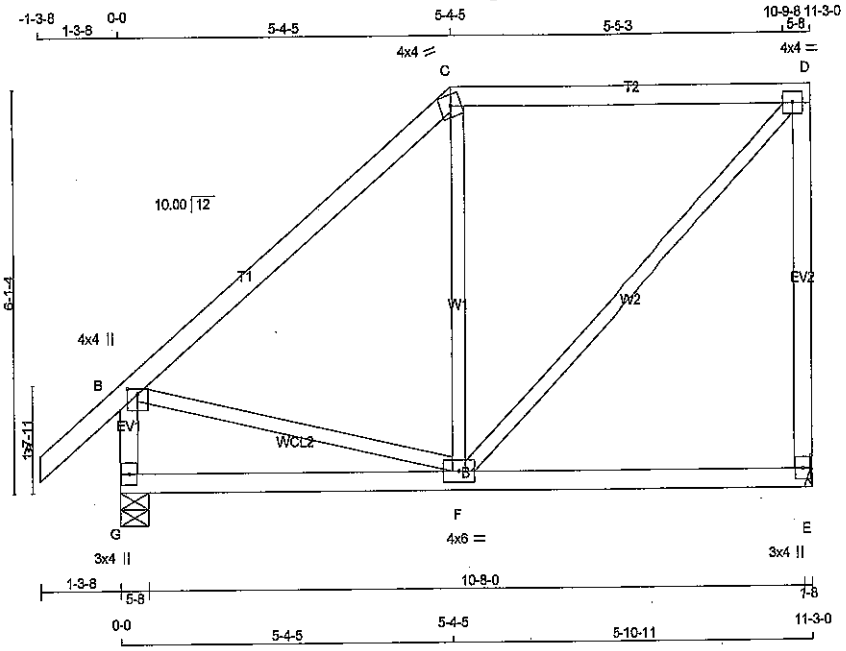

TOTAL WEIGHT = 52 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	745	0	745	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	890	0	890	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	590	365 / 0	113 / 0	0 / 0	0 / 0	113 / 0	0 / 0
G	689	455 / 0	113 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	F- C	-275 / 108	0.16 (1)
B- C	-494 / 0	-104.9	-104.9	0.54 (1)	6.25	F- D	0 / 528	0.12 (1)
C- D	-370 / 0	-104.9	-104.9	0.64 (1)	6.25	B- F	0 / 389	0.09 (1)
E- D	-684 / 0	0.0	0.0	0.49 (1)	7.81			
G- B	-833 / 0	0.0	0.0	0.09 (1)	7.81			
G- F	0 / 0	-27.5	-27.5	0.25 (3)	10.00			
F- E	0 / 0	-27.5	-27.5	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.64/1.00 (C-D:1) , BC=0.25/1.00 (F-G:3) , WB=0.16/1.00 (C-F:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1328248
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T103	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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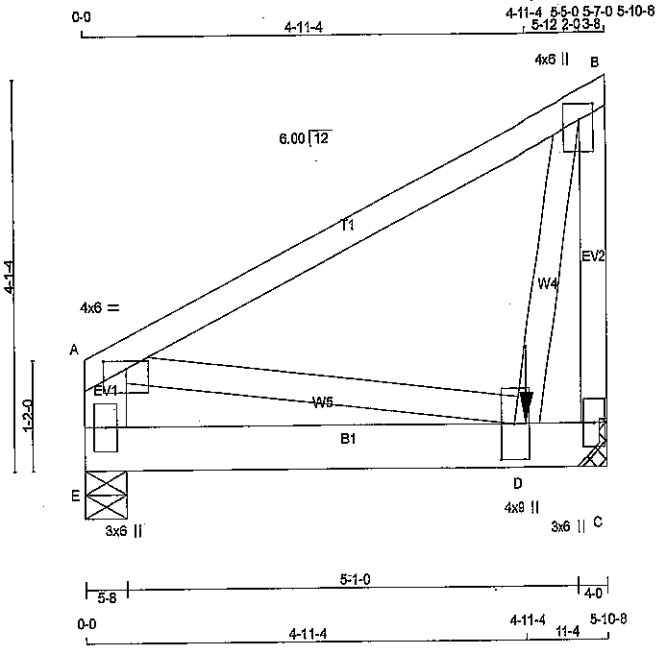

TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
E- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- C	2	12	SIDE(64.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW+p	MT20	4.0	6.0	2.00	1.75
C	BMV1+p	MT20	3.0	6.0		
D	BMWW+t	MT20	4.0	9.0	4.50	1.75
E	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3154.5 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1260	0	1260	0	0	5-8	5-8
C	3306	0	3306	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	998	618 / 0	190 / 0	0 / 0	0 / 0	190 / 0	0 / 0
C	2820	1822 / 0	499 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
E- A	-862 / 0	0.0	0.0	0.03 (1)	7.81	D- B	0 / 3444	0.30 (1)
A- B	-943 / 0	-104.9	-104.9	0.36 (1)	6.25	A- D	0 / 854	0.08 (1)
C- B	-3226 / 0	0.0	0.0	0.40 (1)	6.47			
E- D	0 / 0	-155.8	-155.8	0.19 (1)	10.00			
D- C	0 / 0	-27.5	-27.5	0.02 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-11-4	-3154	-3154	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 4-11-4
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

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THIS DESIGN COMPLIES WITH:
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- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.19/1.00 (D-E:1) , WB=0.30/1.00 (B-D:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
2 815
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13283-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	T103	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:ClogzM2xEszoQB2Vgxmw1Byb8G2-6d1xiEKPvzNKprtFUpxDpoUrWPCoMpK5bUhG9zdLKR

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.32 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13283-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T180	5	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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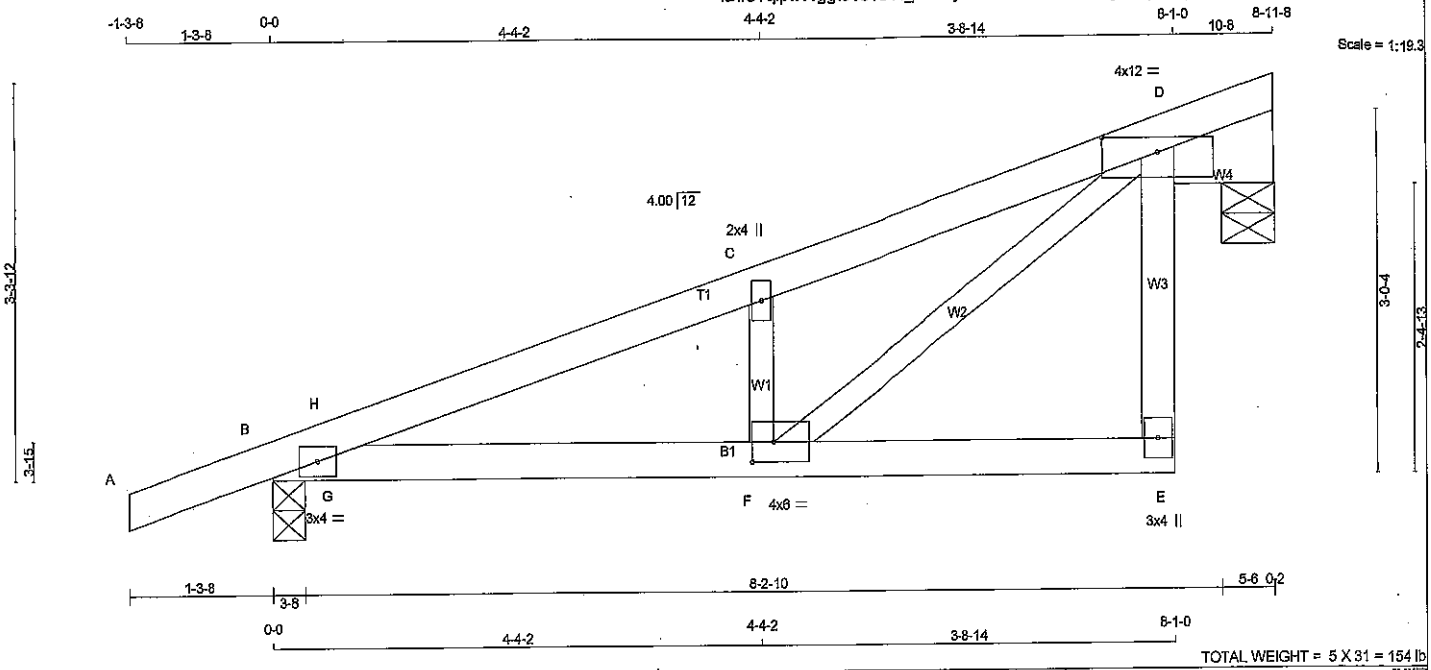

TOTAL WEIGHT = 5 X 31 = 154 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	12.0	1.50	6.00
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0	2.00	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	535	0	535	0	0	5-8 (5-6)	5-8
B	676	0	676	0	0	3-8	3-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	424	263 / 0	81 / 0	0 / 0	0 / 0	81 / 0	0 / 0
B	519	349 / 0	81 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)		10.00
B- H	-940 / 0	-104.9	-104.9	0.04 (2)		6.25
H- C	-902 / 0	-104.9	-104.9	0.18 (1)		6.25
C- D	-914 / 0	-104.9	-104.9	0.18 (1)		6.25
E- D	0 / 70	0.0	0.0	0.01 (3)		10.00
B- G	0 / 870	-27.5	-27.5	0.25 (1)		10.00
G- F	0 / 870	-27.5	-27.5	0.26 (1)		10.00
F- E	0 / 0	-27.5	-27.5	0.10 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-476 / 0	0.07 (1)
F- D	0 / 1057	0.24 (1)
G- H	-111 / 74	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (C-D:1) , BC=0.26/1.00 (F-G:1) , WB=0.24/1.00 (D-F:1) , SSI=0.18/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13315-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T181	3	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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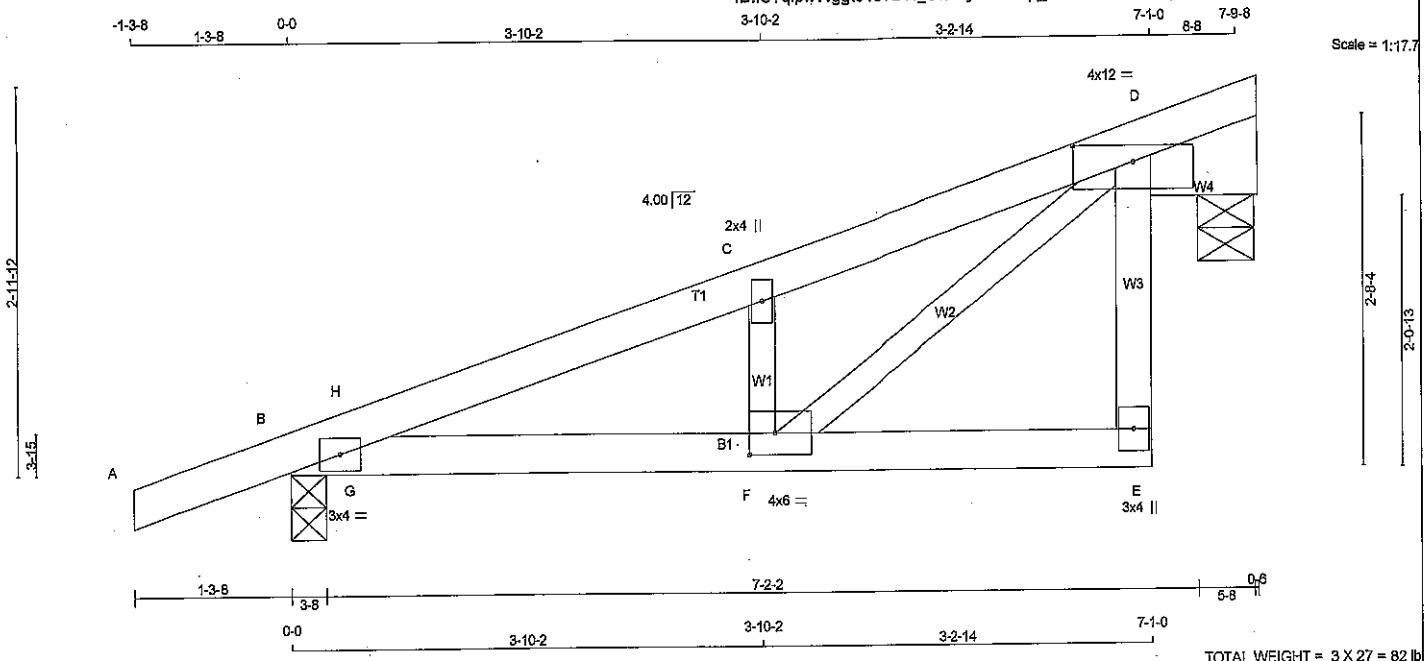

TOTAL WEIGHT = 3 X 27 = 82 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWW1-p	MT20	4.0	12.0	1.50	6.00
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	8.0	2.00	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	469	0	469	0	0	5-8	5-8
B	609	0	609	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	372	230 / 0	71 / 0	0 / 0	0 / 0	71 / 0	0 / 0
B	467	317 / 0	71 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00
B- H	-806 / 0	-104.9	-104.9	0.04 (2)	6.25
H- C	-769 / 0	-104.9	-104.9	0.13 (1)	6.25
C- D	-778 / 0	-104.9	-104.9	0.13 (1)	6.25
E- D	0 / 61	0.0	0.0	0.01 (3)	10.00
B- G	0 / 742	-27.5	-27.5	0.21 (1)	10.00
G- F	0 / 742	-27.5	-27.5	0.22 (1)	10.00
F- E	0 / 0	-27.5	-27.5	0.07 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-411 / 0	0.06 (1)
F- D	0 / 904	0.20 (1)
G- H	-90 / 61	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.22/1.00 (F-G:1) , WB=0.20/1.00 (D-F:1) , SSI=0.15/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	615	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 133168
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T183	1	2	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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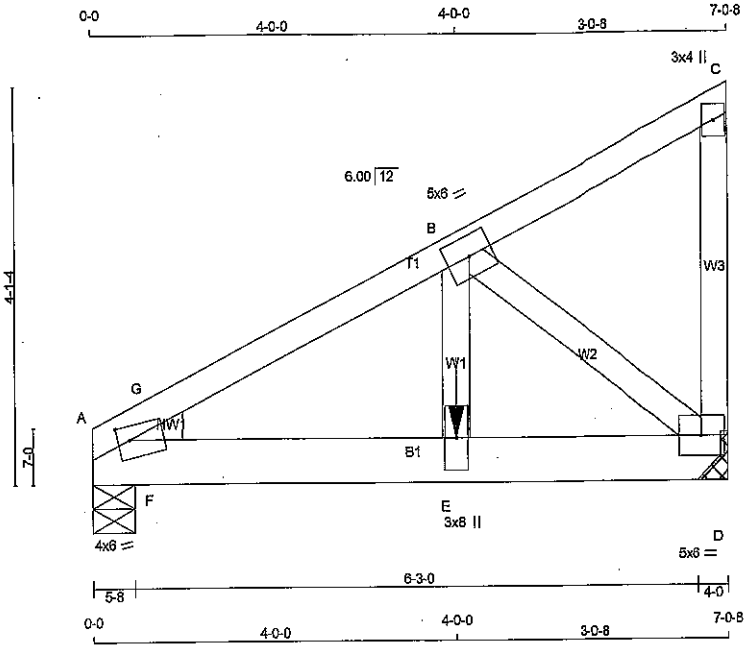

TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(169.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	6.0	1.75	1.50
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMW+w	MT20	3.0	8.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1640.3 lbs FACTORED DOWN AT 4-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	2391	0	2391	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0		
A	1880	0	1660	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1895	1173 / 0	361 / 0	0 / 0	0 / 0	361 / 0	0 / 0
A	1474	912 / 0	281 / 0	0 / 0	0 / 0	281 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- G	-2974 / 0	-104.9	-104.9	0.06 (1)	5.30	E- B	0 / 2560	0.23 (1)
G- B	-2676 / 0	-104.9	-104.9	0.09 (1)	5.50	B- D	-2955 / 0	0.25 (1)
B- C	-11 / 1	-104.9	-104.9	0.06 (1)	6.25	F- G	0 / 361	0.00 (1)
D- C	-134 / 0	0.0	0.0	0.02 (1)	7.81			
A- F	0 / 2398	-188.9	-188.9	0.16 (1)	10.00			
F- E	0 / 2398	-188.9	-188.9	0.23 (1)	10.00			
E- D	0 / 2398	-366.9	-366.9	0.30 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	4-0-0	-1640	-1640	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 6-10-8
END DISTANCE = 4-0-0
END SPAN CARRIED = 6-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 4-0-0
START SPAN CARRIED = 12-3-0
END DISTANCE = 7-0-8
END SPAN CARRIED = 12-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.09/1.00 (B-G:1) , BC=0.30/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER
3078
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 13317-A

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T183	1	2	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3080
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 13317-S8
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T190	1	1	Tamarack - Bayview Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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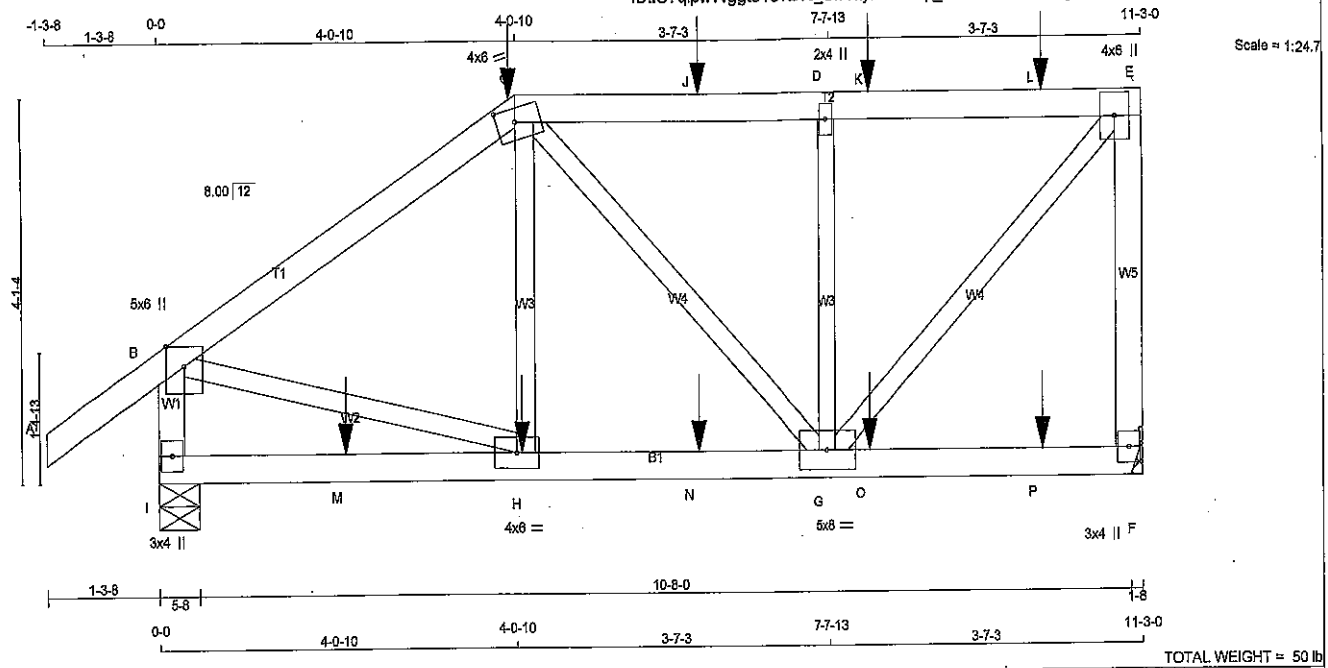

TOTAL WEIGHT = 50 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 263.7 lbs FACTORED DOWN AT 4-0-10, 129.8 lbs FACTORED DOWN AT 4-0-10, 126.3 lbs FACTORED DOWN AT 6-1-6, AND 126.3 lbs FACTORED DOWN AT 8-1-6, AND 126.3 lbs FACTORED DOWN AT 10-1-6 ON TOP CHORD, AND 67.8 lbs FACTORED DOWN AT 2-1-6, 67.8 lbs FACTORED DOWN AT 4-1-6, 67.8 lbs FACTORED DOWN AT 6-1-6, AND 67.8 lbs FACTORED DOWN AT 8-1-6, AND 67.8 lbs FACTORED DOWN AT 10-1-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1267	0	1267	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
I	1336	0	1336	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1014	609 / 0	205 / 0	0 / 0	0 / 0	200 / 0	0 / 0
I	1060	653 / 0	203 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00
B- C	-1185 / 0	-104.9	-104.9	0.36 (1)	5.36
C- J	-925 / 0	-104.9	-104.9	0.39 (1)	5.81
J- D	-925 / 0	-104.9	-104.9	0.39 (1)	5.81
D- K	-925 / 0	-104.9	-104.9	0.39 (1)	5.81
K- L	-925 / 0	-104.9	-104.9	0.39 (1)	5.81
L- E	-925 / 0	-104.9	-104.9	0.39 (1)	5.81
F- E	-1198 / 0	0.0	0.0	0.32 (1)	7.28
I- B	-1273 / 0	0.0	0.0	0.14 (1)	7.10
I- M	0 / 0	-27.5	-27.5	0.18 (2)	10.00
M- H	0 / 0	-27.5	-27.5	0.18 (2)	10.00
H- N	0 / 983	-27.5	-27.5	0.28 (2)	10.00
N- G	0 / 983	-27.5	-27.5	0.28 (2)	10.00
G- O	0 / 0	-27.5	-27.5	0.15 (2)	10.00
O- P	0 / 0	-27.5	-27.5	0.15 (2)	10.00
P- F	0 / 0	-27.5	-27.5	0.15 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-58 / 221	0.05 (3)
C- G	-100 / 0	0.04 (2)
G- D	-734 / 0	0.20 (1)
G- E	0 / 1339	0.33 (1)
B- H	0 / 1017	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	4-0-10	-264	-264	---	FRONT	VERT	TOTAL
C	4-0-10	-130	-130	---	BACK	VERT	TOTAL
H	4-1-6	-39	-68	---	BACK	VERT	TOTAL
J	6-1-6	-126	-126	---	BACK	VERT	TOTAL
K	8-1-6	-126	-126	---	BACK	VERT	TOTAL
L	10-1-6	-126	-126	---	BACK	VERT	TOTAL
M	2-1-6	-39	-68	---	BACK	VERT	TOTAL
N	6-1-6	-39	-68	---	BACK	VERT	TOTAL
O	8-1-6	-39	-68	---	BACK	VERT	TOTAL
P	10-1-6	-39	-68	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.28/1.00 (G-H:2) , WB=0.33/1.00 (E-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

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DWG NO. TAM 13318-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Tamarack - Bayview	DRWG NO.
288664	T191	1	1	TRUSS DESC.	Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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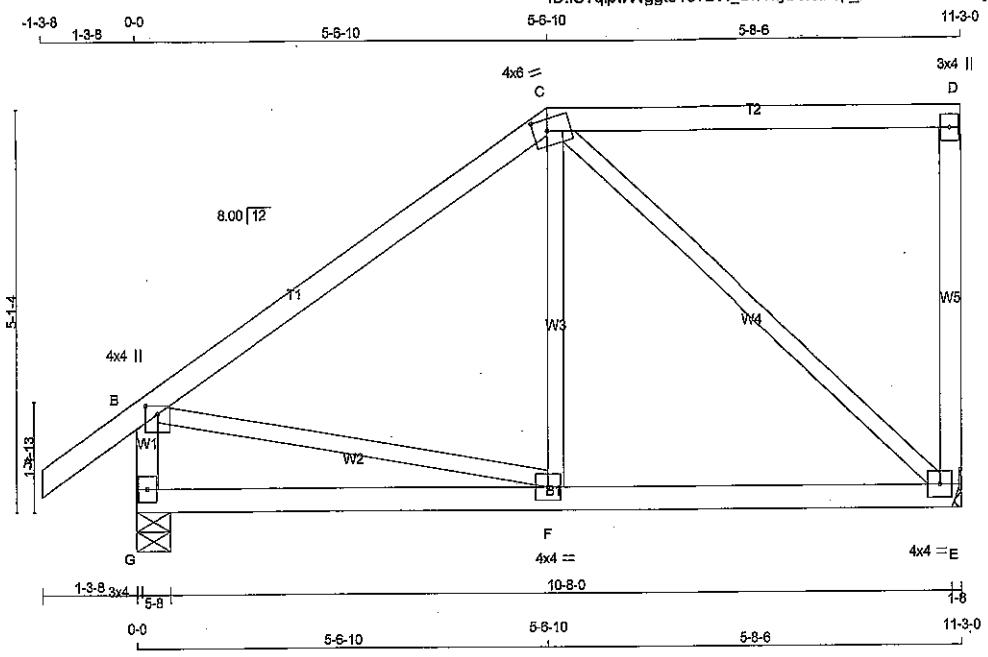

TOTAL WEIGHT = 49 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	745	0	745	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	889	0	889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	590	365 / 0	113 / 0	0 / 0	0 / 0	113 / 0	0 / 0
G	688	455 / 0	113 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00				
B- C	-550 / 0	-104.9	-104.9	0.58 (1)	6.25		F- C	0 / 291	0.07 (3)
C- D	0 / 0	-104.9	-104.9	0.58 (1)	10.00		C- E	-601 / 0	0.60 (1)
E- D	-299 / 0	0.0	0.0	0.13 (1)	7.81		B- F	0 / 466	0.10 (1)
G- B	-829 / 0	0.0	0.0	0.09 (1)	7.81				
G- F	0 / 0	-27.5	-27.5	0.25 (3)	10.00				
F- E	0 / 460	-27.5	-27.5	0.30 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.30/1.00 (E-F:2) , WB=0.60/1.00 (C-E:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3676
S KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13319-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
288664	T192	1	1	Tamarack - Bayview TRUSS DESC. Wellington - S38-17 / C	

Tamarack Roof Truss, Burlington

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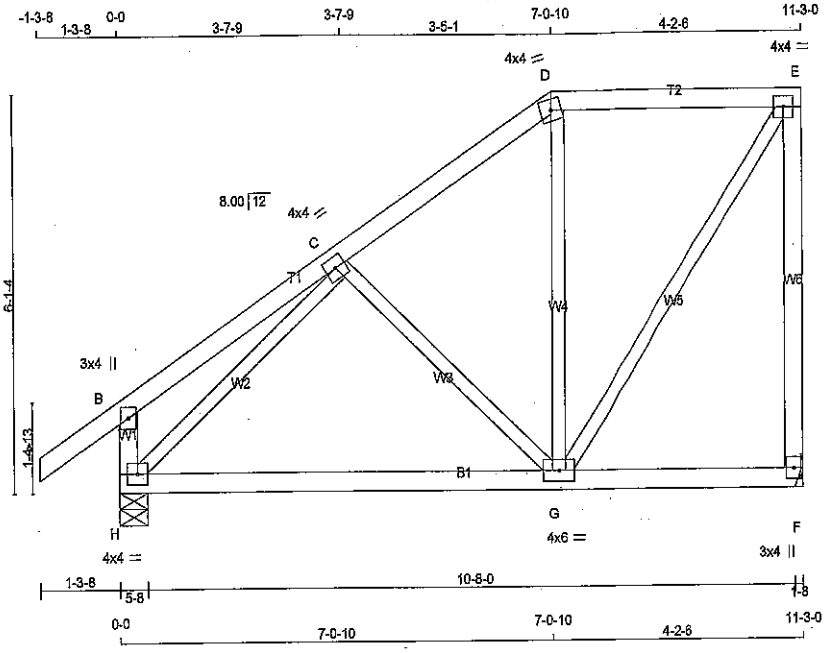

Scale = 1:35.4

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	745	0	745	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	889	0	889	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	590	365 / 0	113 / 0	0 / 0	0 / 0	113 / 0	0 / 0
H	688	455 / 0	113 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 24	-104.9	-104.9	0.21 (1)	10.00
C- D	-451 / 0	-104.9	-104.9	0.16 (1)	6.25
D- E	-352 / 0	-104.9	-104.9	0.24 (1)	6.25
F- E	-719 / 0	0.0	0.0	0.52 (1)	7.81
H- B	-285 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 537	-27.5	-27.5	0.35 (2)	10.00
G- F	0 / 0	-27.5	-27.5	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- G	-251 / 16	0.10 (1)
G- D	-128 / 91	0.07 (1)
G- E	0 / 611	0.14 (1)
H- C	-749 / 0	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 838 (0.16")

CSI: TC=0.52/1.00 (E-F:1) , BC=0.35/1.00 (G-H:2) , WB=0.29/1.00 (C-H:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

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3070
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DWG NO. TAM 13320-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288659	J02A	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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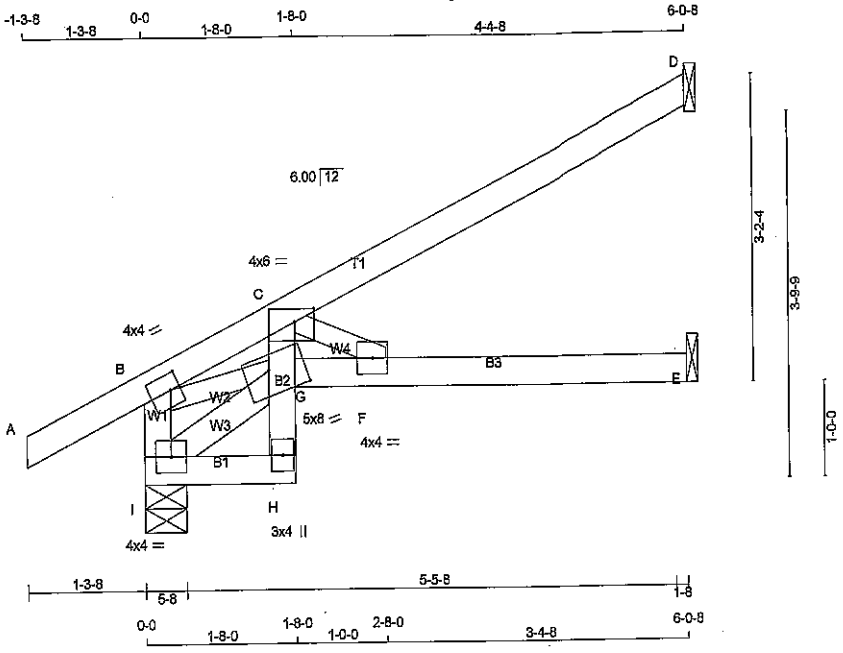

TOTAL WEIGHT = 8 X 22 = 178 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-p	MT20	4.0	6.0	1.75	2.50
F	BMW-t	MT20	4.0	4.0		
G	BVMWWm	MT20	5.0	8.0	3.00	4.75
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	558	0	558	0	0	5-8	5-8
D	240	0	240	0	0	1-8	1-8
E	144	0	149	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	424	294 / 0	60 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	170	141 / 0	10 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	136	46 / 0	51 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-508 / 0	0.0	0.0	0.05 (1)	7.81	I- G	-56 / 0	0.01 (1)
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00	B- G	0 / 484	0.11 (1)
B- C	-573 / 0	-104.9	-104.9	0.14 (3)	6.25	C- F	-648 / 0	0.09 (1)
C- D	0 / 10	-104.9	-104.9	0.37 (1)	10.00			
I- H	0 / 48	-27.5	-27.5	0.02 (2)	10.00			
H- G	0 / 36	0.0	0.0	0.06 (1)	10.00			
G- C	0 / 342	0.0	0.0	0.12 (1)	10.00			
G- F	0 / 535	-27.5	-27.5	0.54 (1)	10.00			
F- E	0 / 0	-27.5	-27.5	0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 621 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 383 (0.19")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.54/1.00 (F-G:1) , WB=0.11/1.00 (B-G:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

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TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

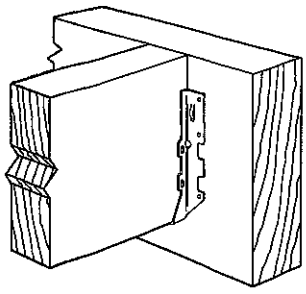
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

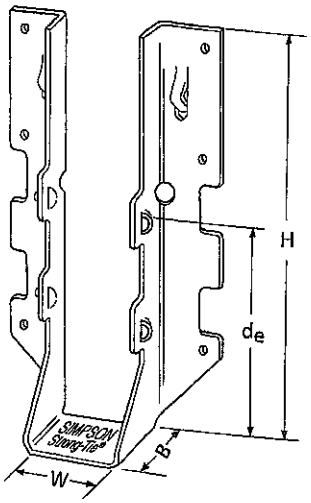
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



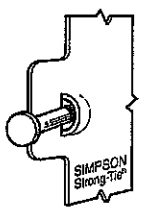
Typical LUS Installation



LUS28

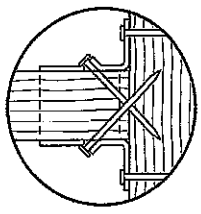
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

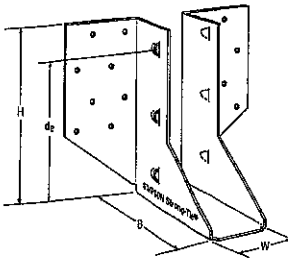
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

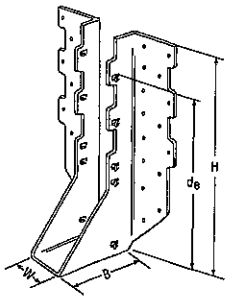
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

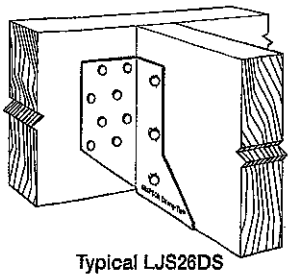
- See current catalogue for options



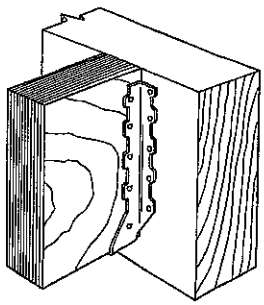
LJS26DS



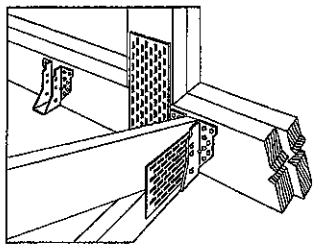
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



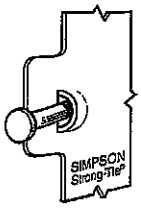
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

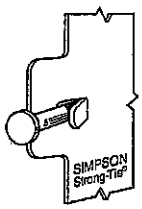
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

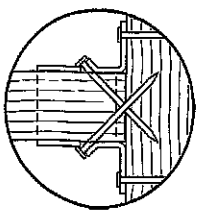


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

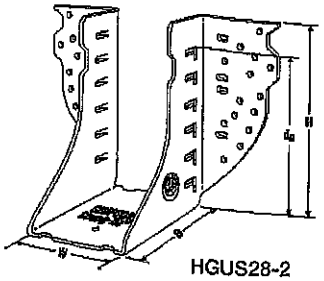
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

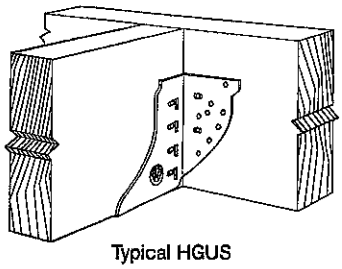
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

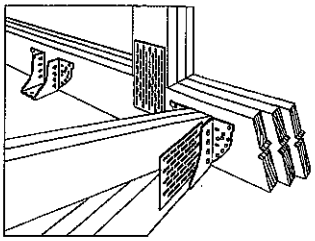
- See current catalogue for options



HGUS28-2



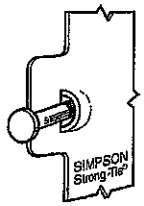
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

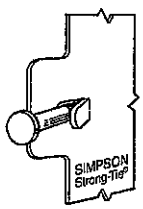
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

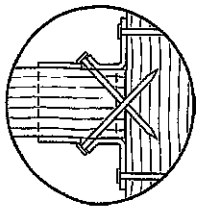


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

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ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

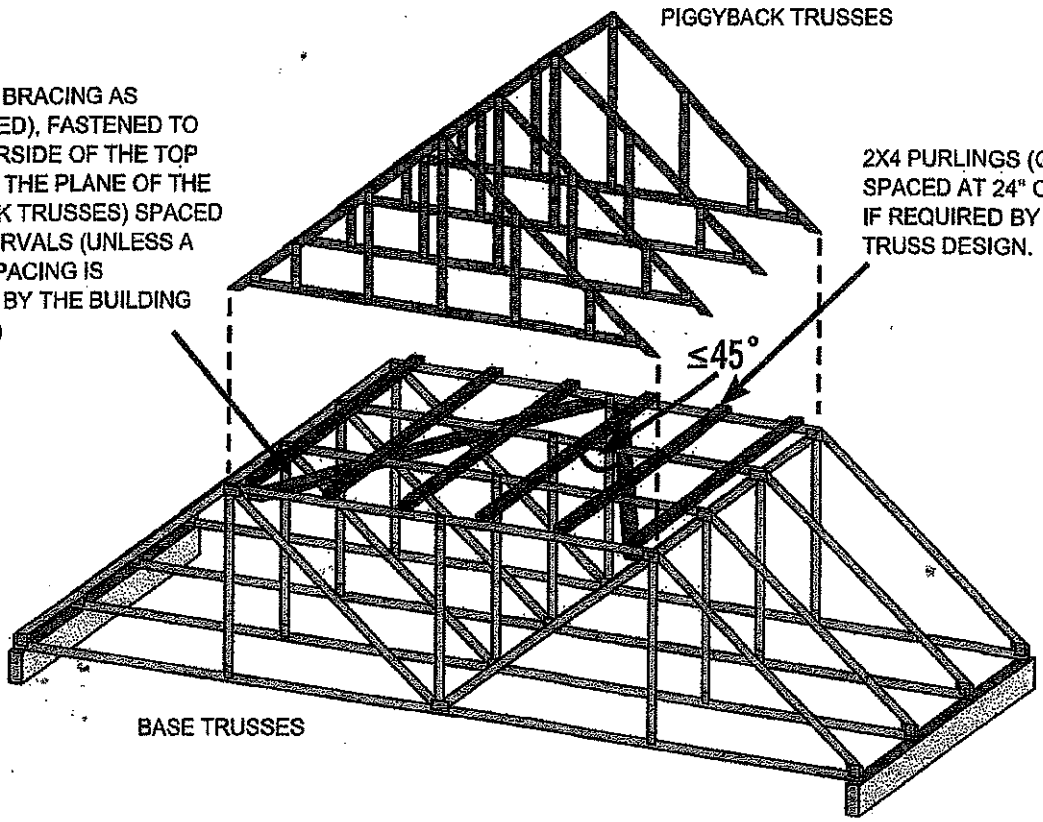
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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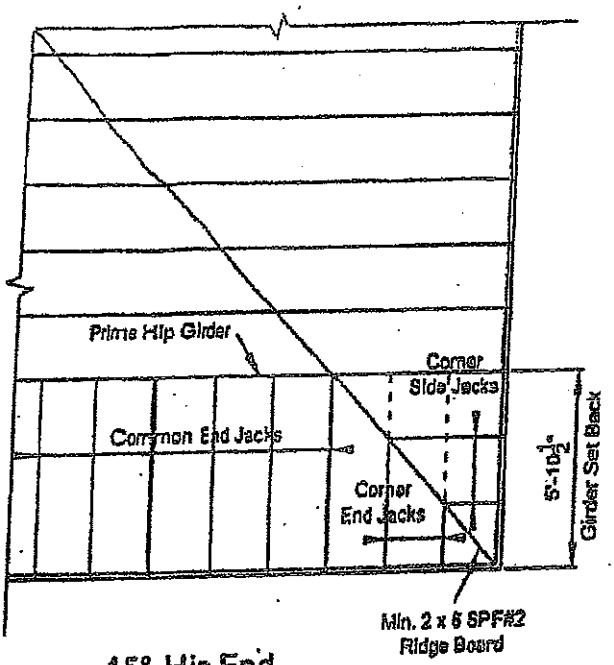

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

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1-2-24
S. KATSOULAKOS
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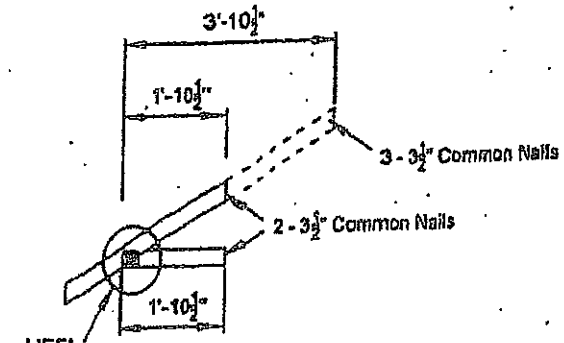
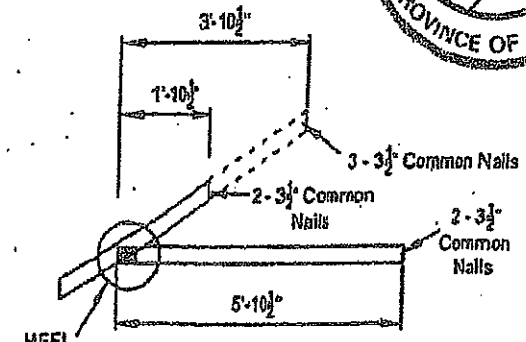
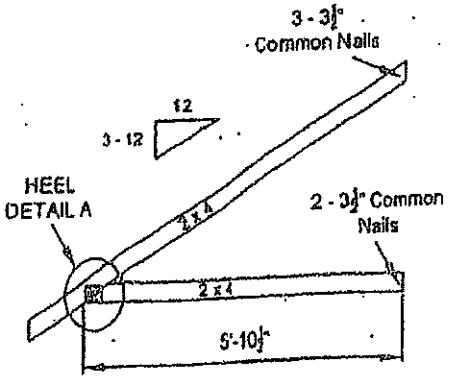


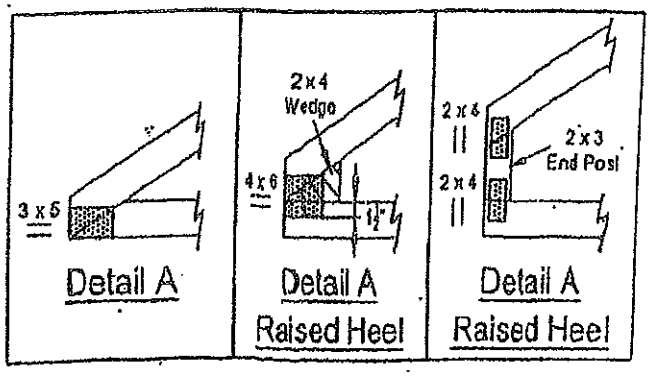
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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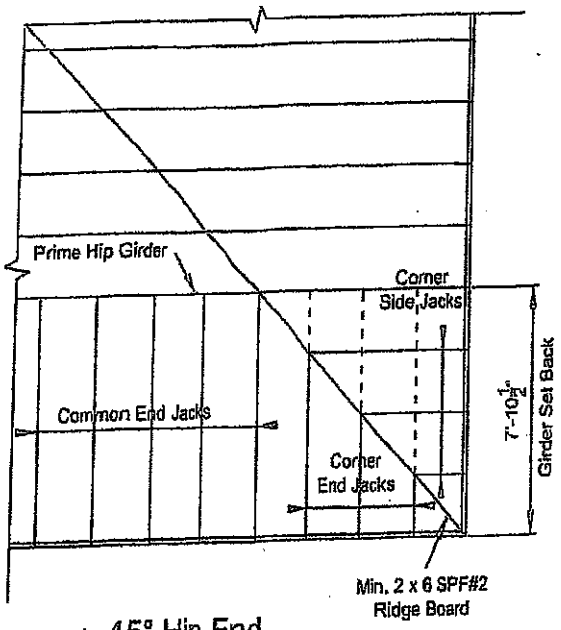
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

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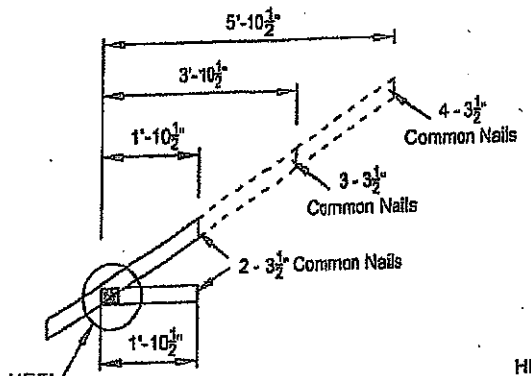
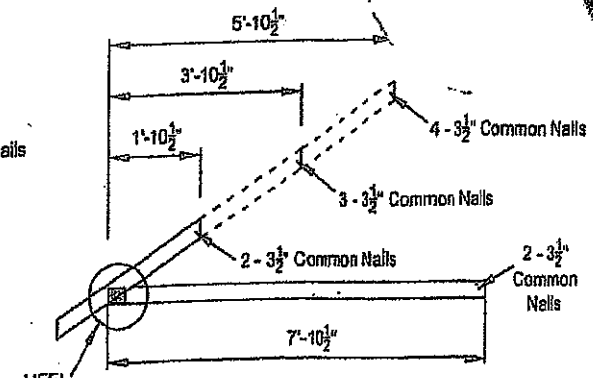
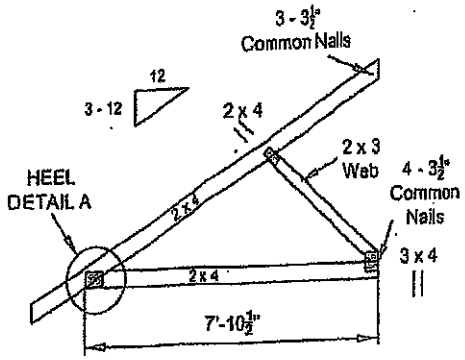


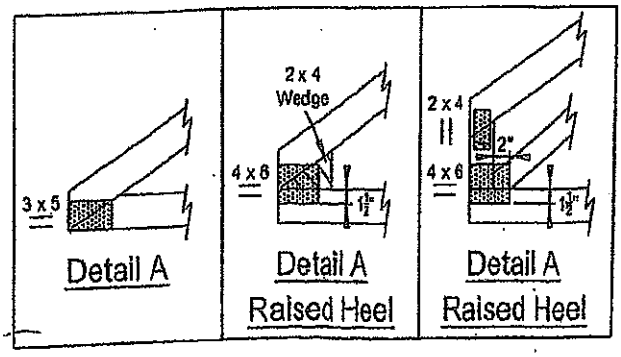
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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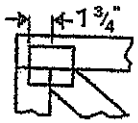
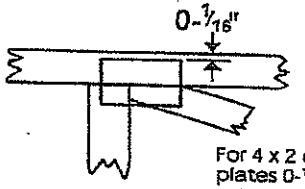
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

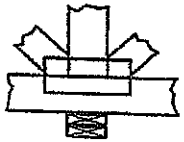
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

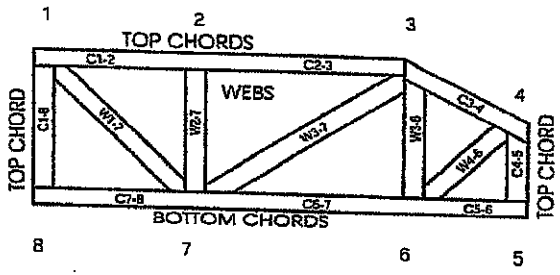
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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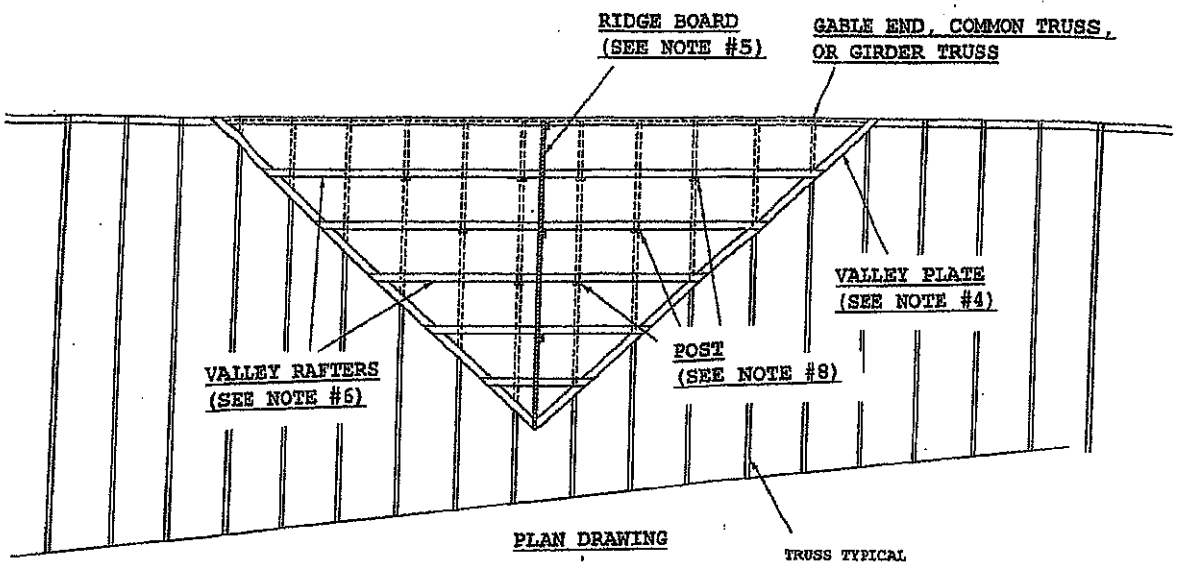
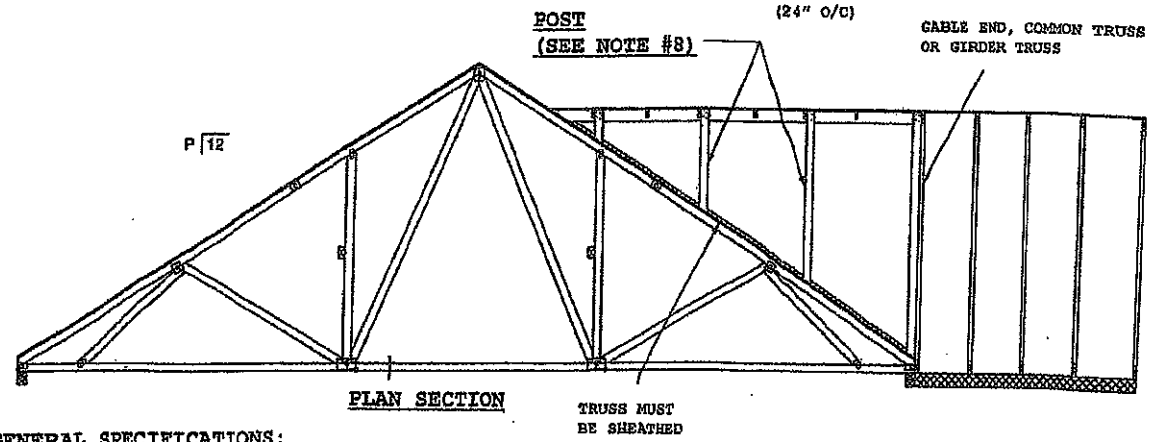
CONVENTIONAL VALLEY FRAMING DETAIL





GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

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Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

DWG NO [illegible] 4/14
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Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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