TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-11-14
INSPECTOR: BG

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

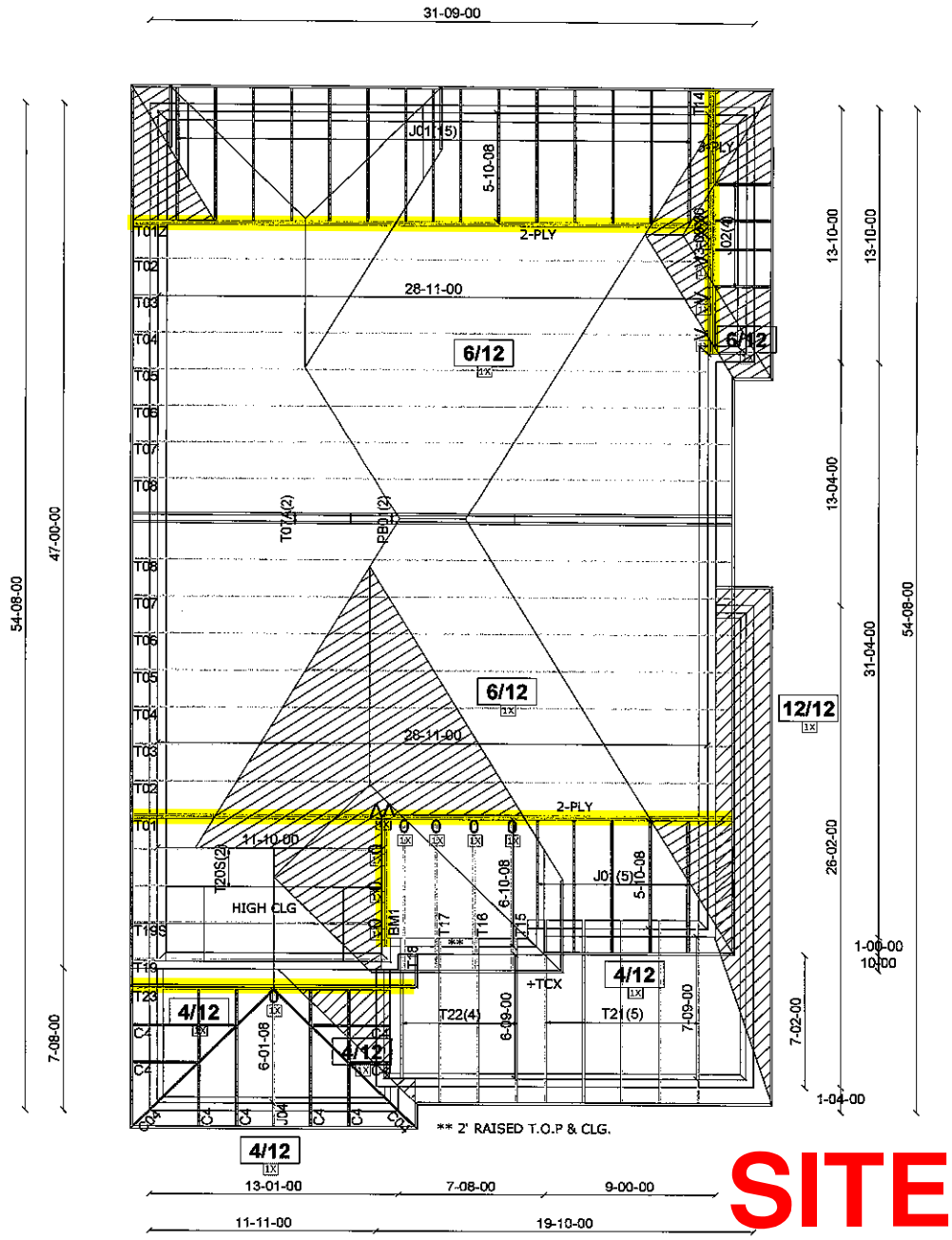
CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

**PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)

**COFFERED CL.
- RAISED 12 "
- INSET 14 "

BM1: 2-2X10



SITE COPY

M11251

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD		Model / Elevation: S38-15 / A -STD
Layout ID: 288643	Project: GREEN VALLEY EAST		
Plan Log: 96660	Date: 3/5/2018	Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

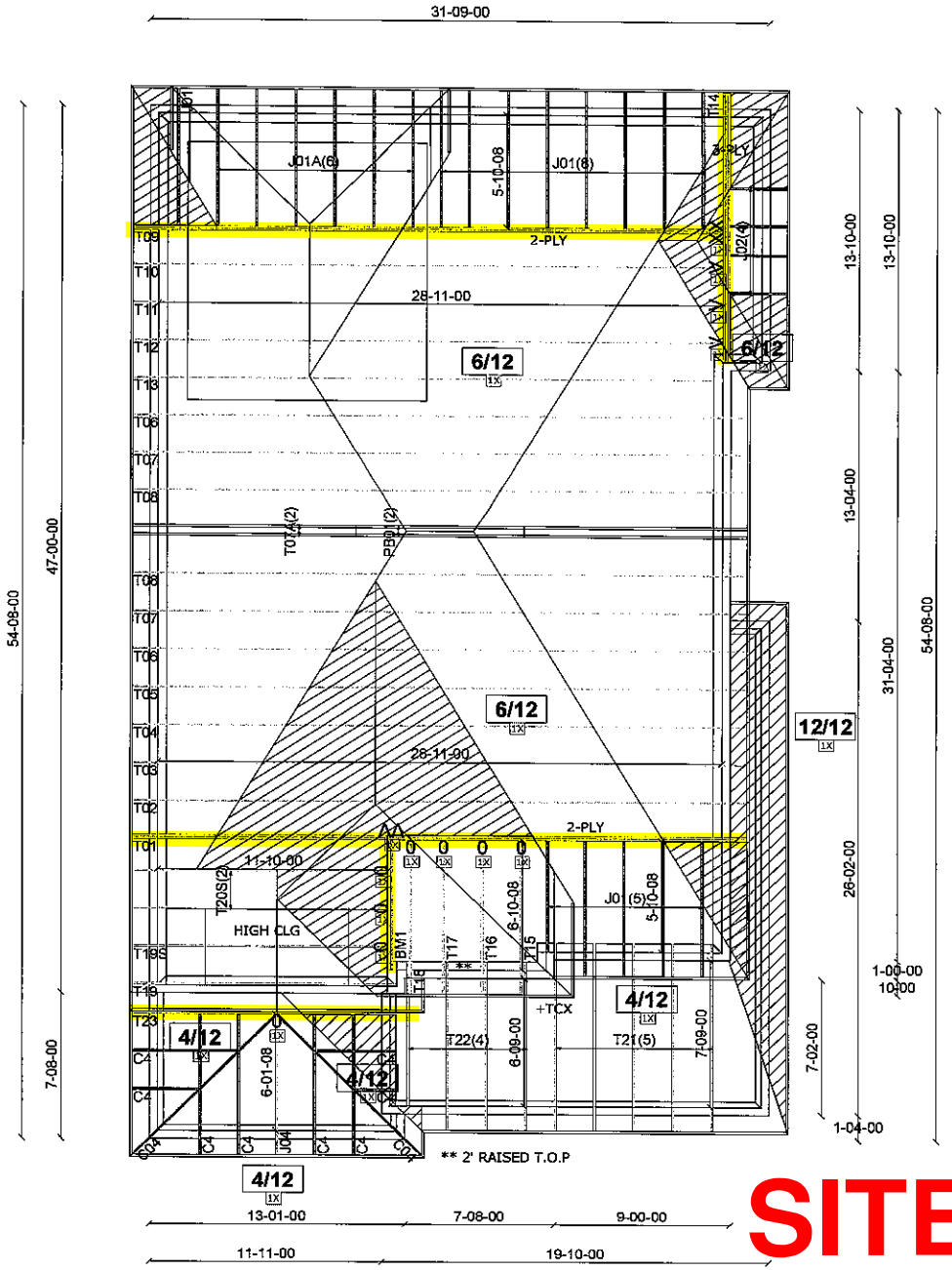
CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL.**
- RAISED 12 "
- INSET 14 "

BM1:2-2X10

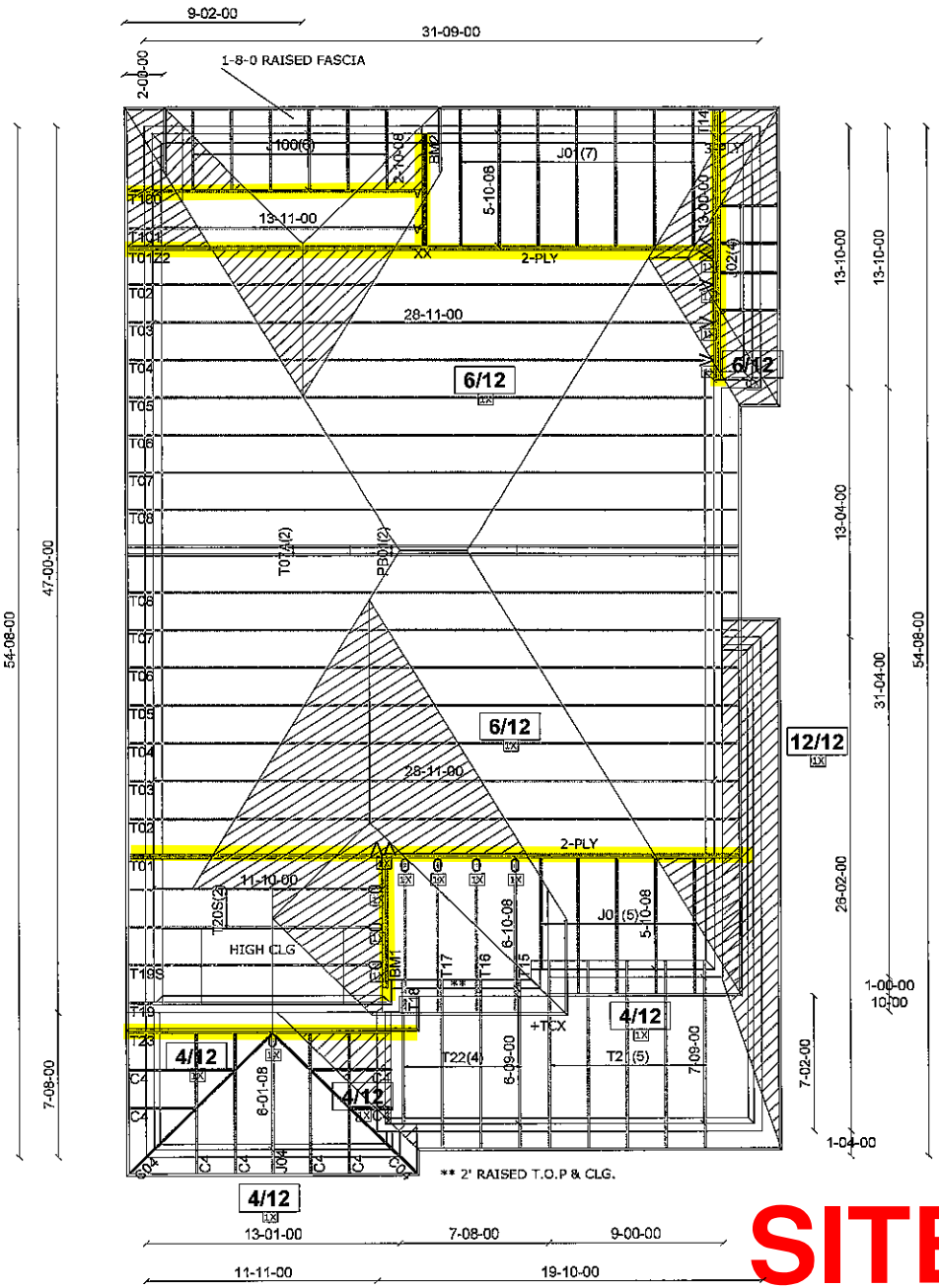


SITE COPY

M11,261

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-15 / A OPT.COFF
	Layout ID: 288629	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/5/2018 Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 8' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL.**
- RAISED 12 "
- INSET 14 "

BM1,2:2-2X10

SITE COPY

M11,251

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-15 / A -REAR UPGRADE**
	Layout ID: **288643**	Project: **GREEN VALLEY EAST**	
	Plan Log: **96660**	Date: **3/5/2018** Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

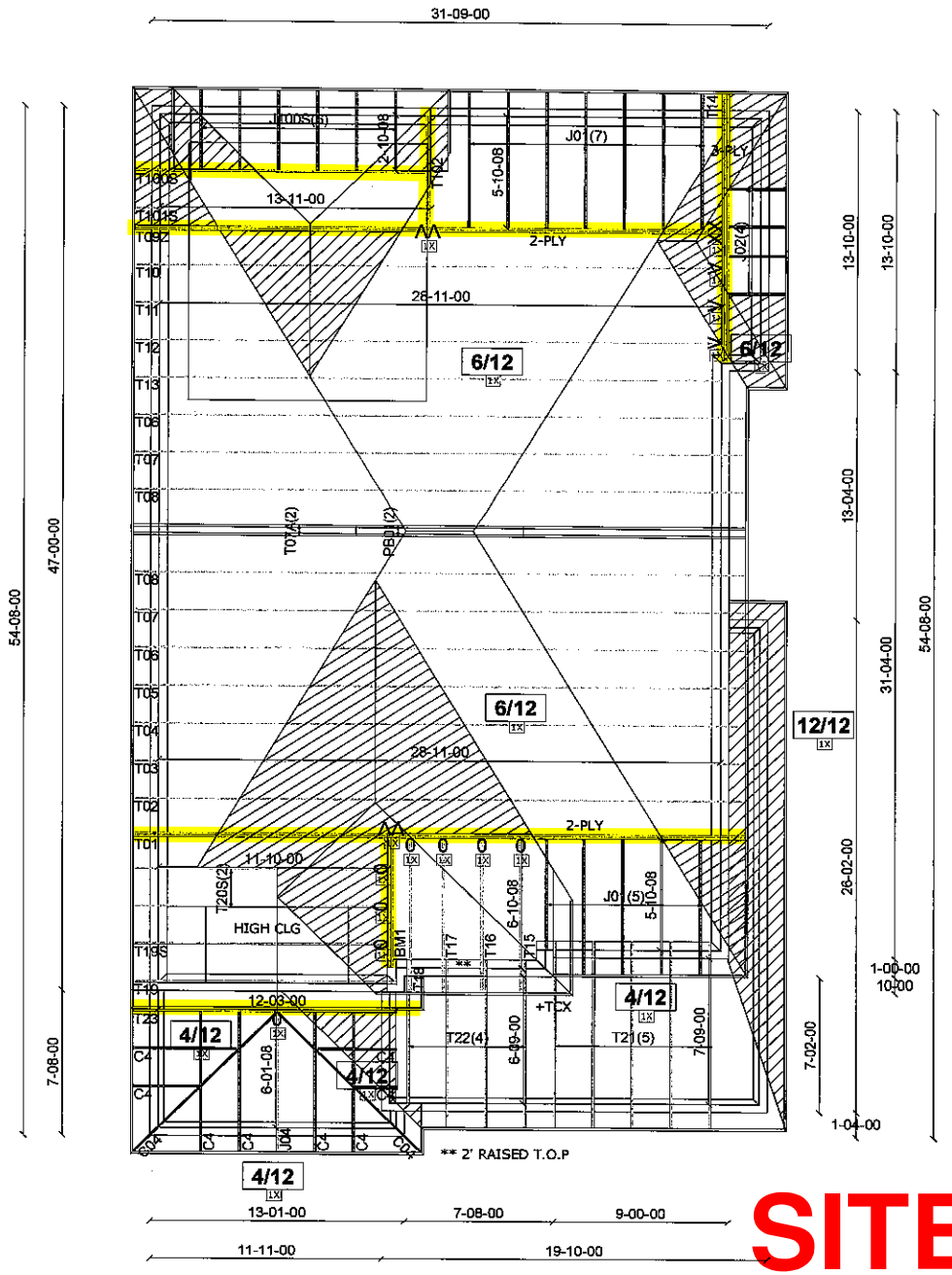
CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL**
- RAISED 12 "
- INSET 14 "

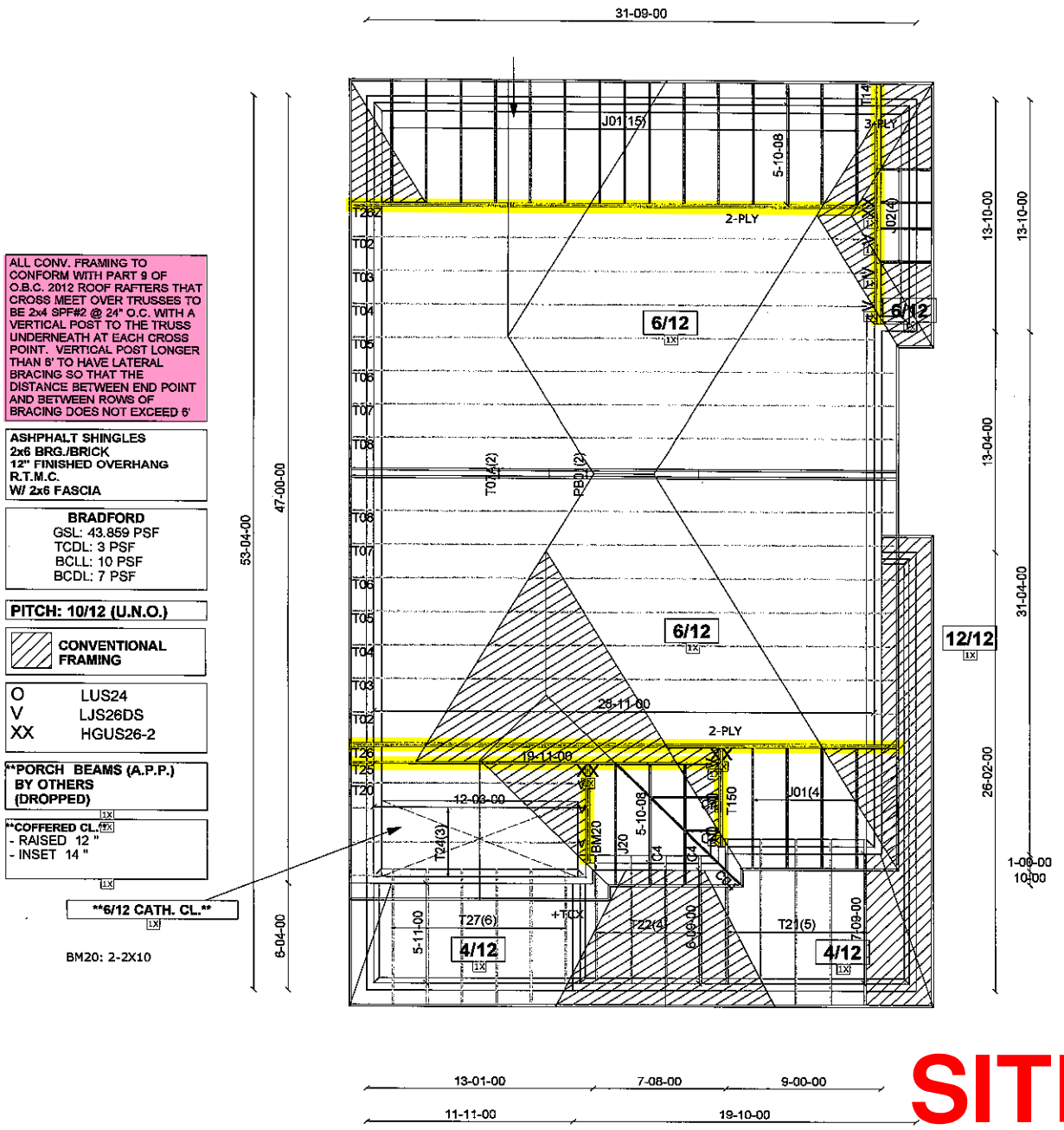
BM1:2-2X10



SITE COPY

m11251

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-15 / A OPT.COFF - REAR UPGRADE**
	Layout ID: **295462**	Project: **GREEN VALLEY EAST**	
	Plan Log: **96660**	Date: **3/5/2018** Designer: Peter Dahlberg/JG	



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

**PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)

COFFERED CL.
- RAISED 12 "
- INSET 14 "

6/12 CATH. CL.

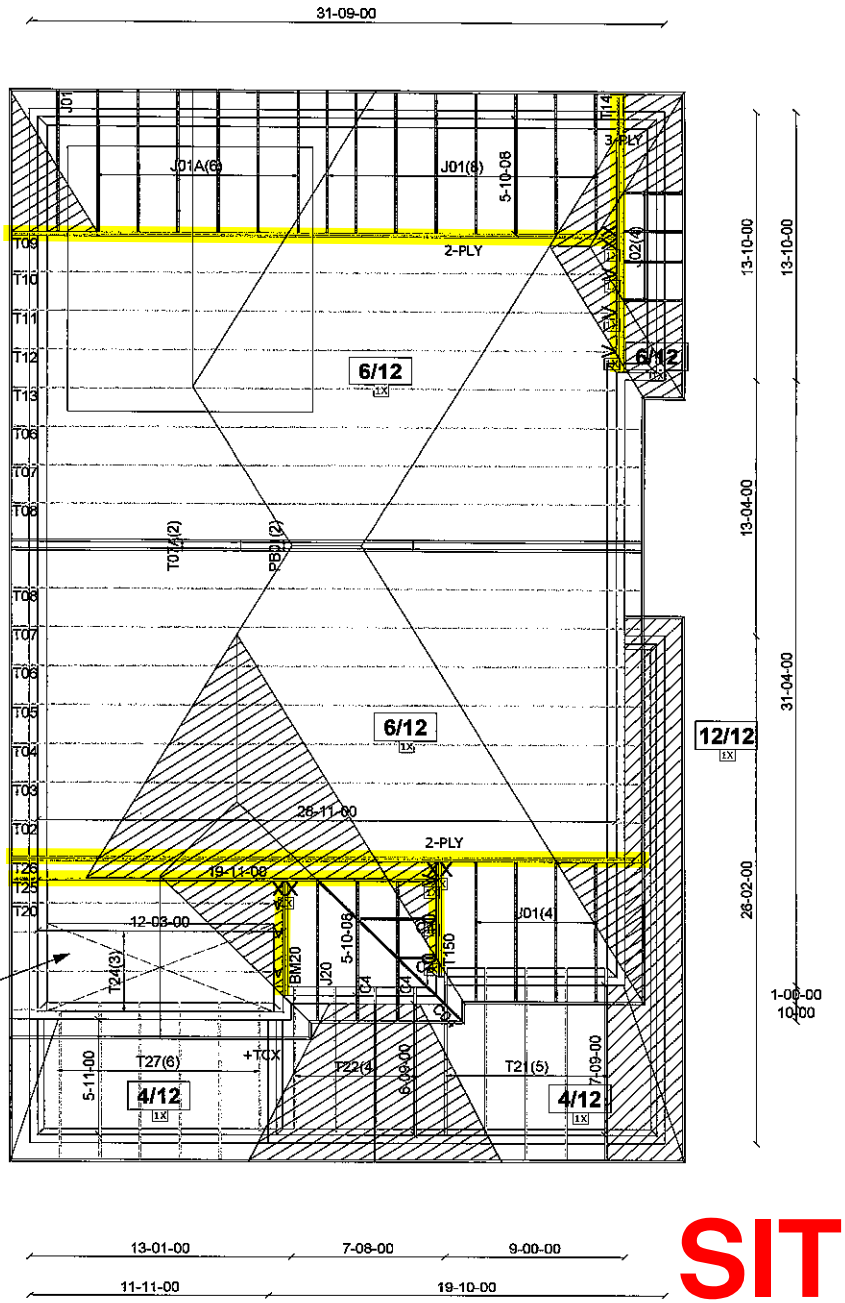
BM20: 2-2X10

SITE COPY

M11761

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-15 / B
	Layout ID: 288646	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/5/2018 Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)** 1X

****COFFERED CL.**** 1X
- RAISED 12 "
- INSET 14 "

1X

****6/12 CATH. CL.**** 1X

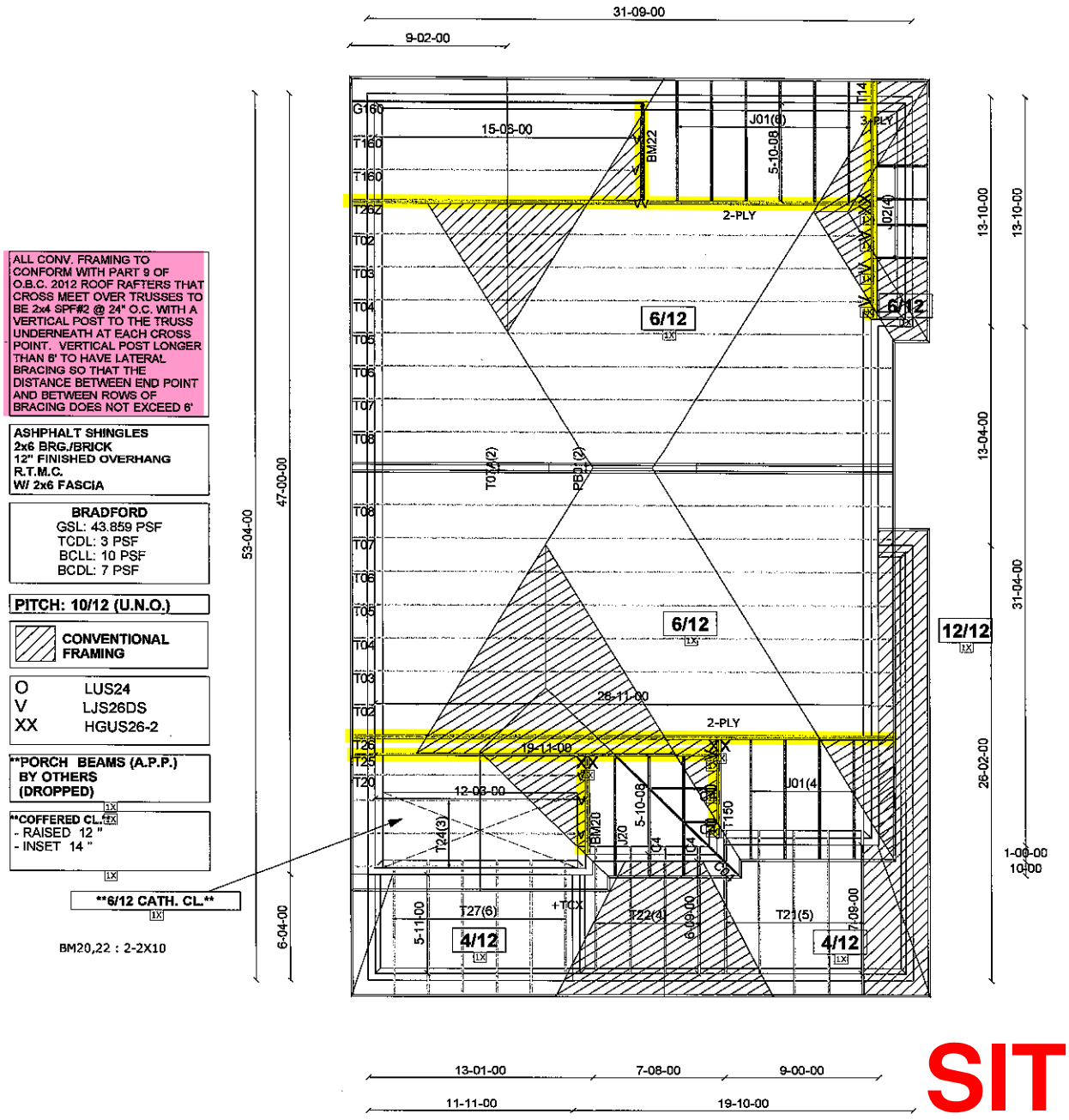
BM20: 2-2X10

SITE COPY

M11/201

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-15 / B - OPT.COFF**
	Layout ID: **288630**	Project: **GREEN VALLEY EAST**	
	Plan Log: **96660**	Date: **3/5/2018** Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL.****
- RAISED 12 "
- INSET 14 "

****6/12 CATH. CL.****

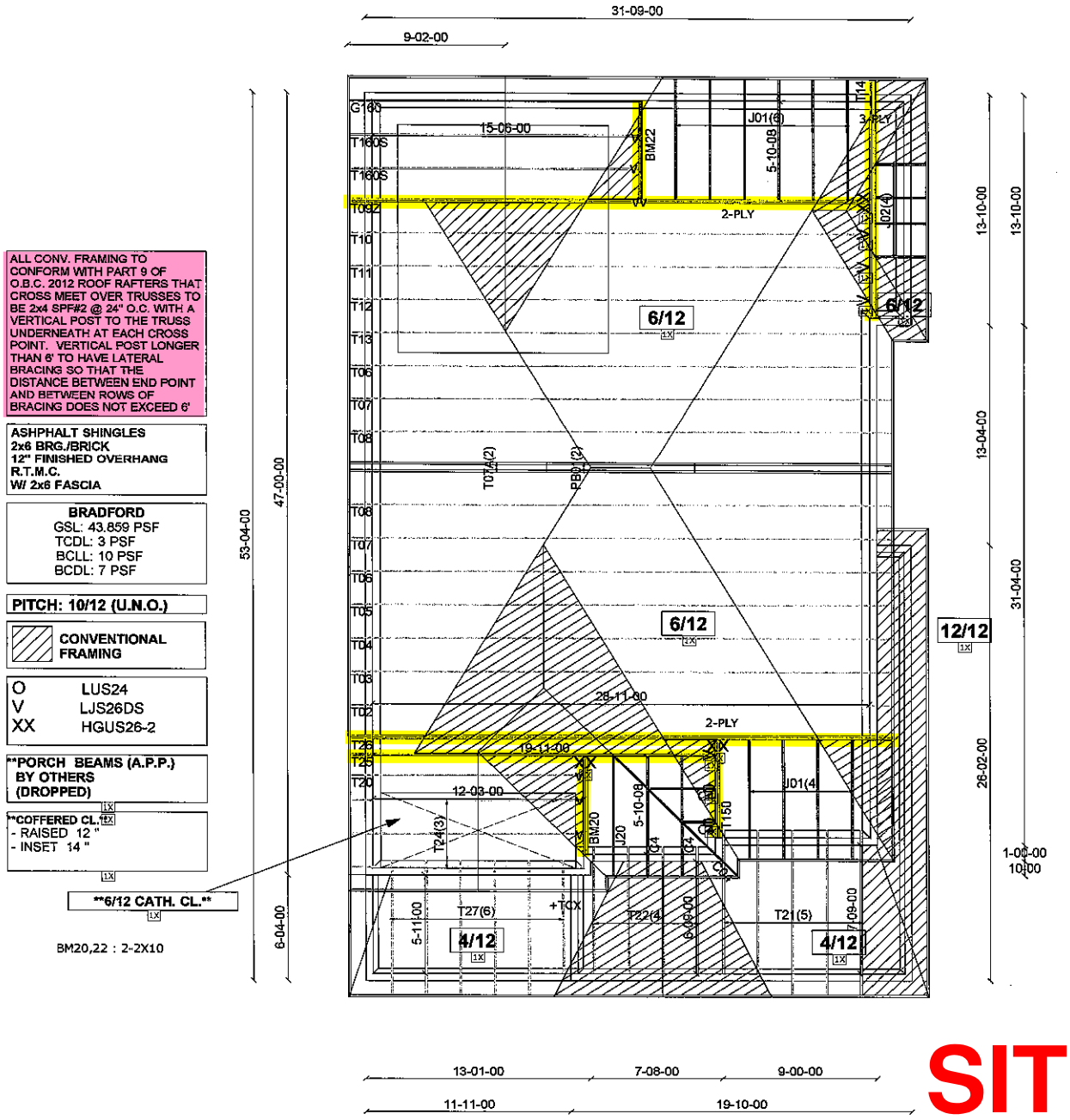
BM20,22 : 2-2X10

SITE COPY

M11,251

TAMARACK LUMBER INC.	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**	Model / Elevation: **S38-15 / B - REAR UPGRADE**
	Layout ID: **295493**	Project: **GREEN VALLEY EAST**	
	Plan Log: **96660**	Date: **3/5/2018** Designer: Peter Dahlberg/JG	

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

O	LUS24
V	LJS26DS
XX	HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL.****
- RAISED 12 "
- INSET 14 "

****6/12 CATH. CL.****

BM20,22 : 2-2X10

SITE COPY

M11,151

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: **44755**
Layout ID: **295492**
Plan Log: **96660**

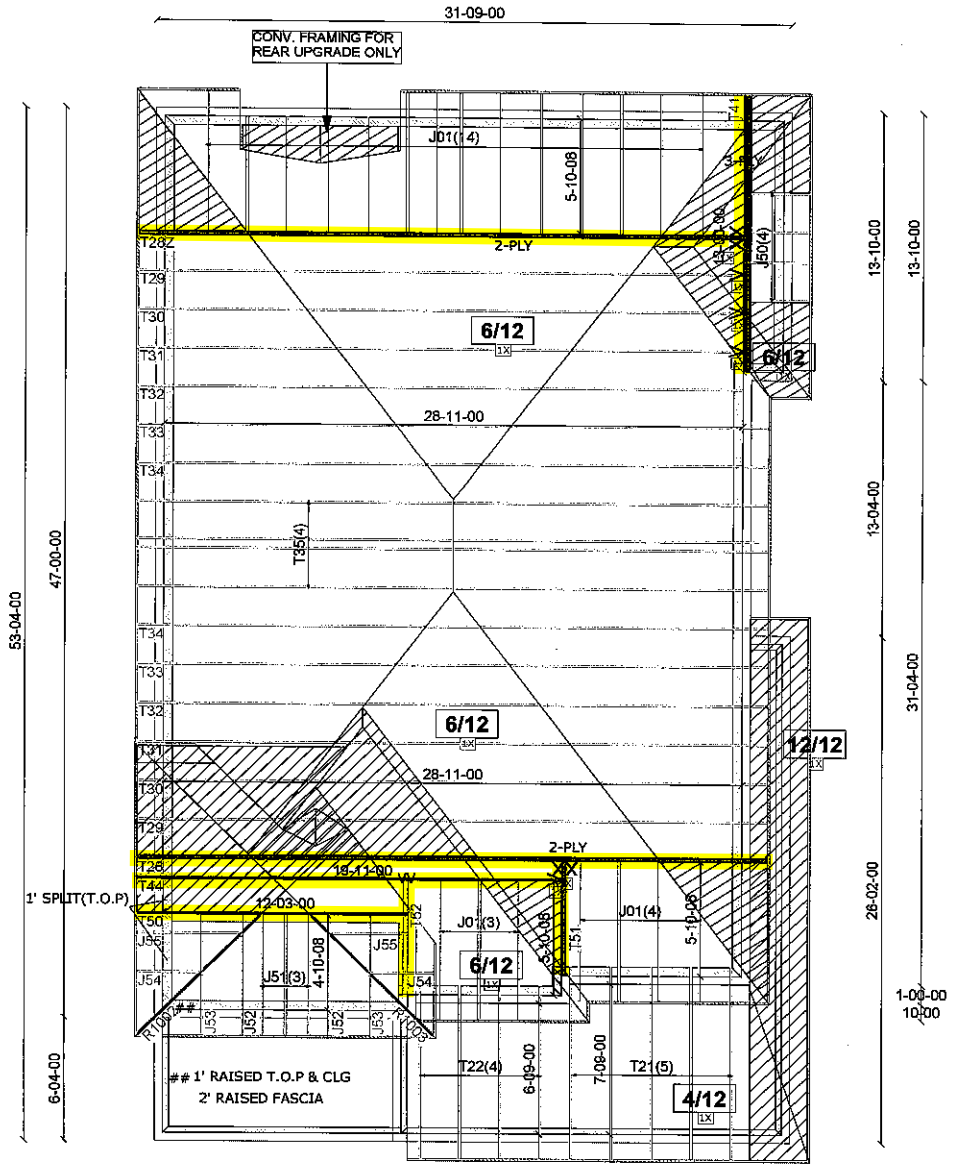
Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **3/5/2018** Designer: Peter Dahlberg/JG

Model / Elevation:
S38-15 / B - OPT.COFF . REAR UPGRADE

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

LUS26-2(VV)
V LJS26DS
XX HGUS26-2

****PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)**

****COFFERED CL.**
- RAISED 12 "
- INSET 14 "

SITE COPY

M11251

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	288647
Plan Log:	96660

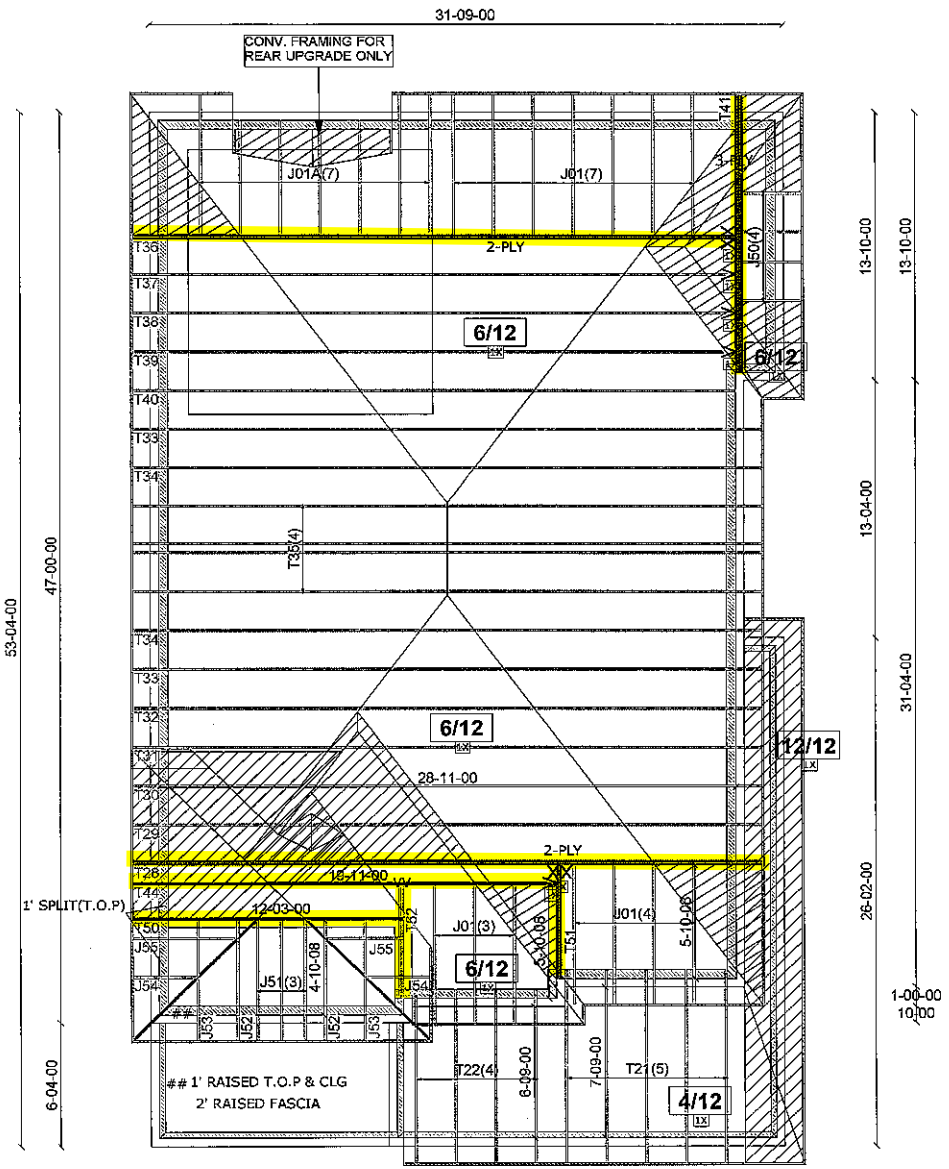
Builder / Location: **BAYVIEW WELLINGTON / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: **3/5/2018** Designer: Peter Dahlberg/JG

Model / Elevation: **S38-15 / C**

Mitek ver 7.6.4



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

LUS26-2(VV)
V LJS26DS
XX HGUS26-2

**PORCH BEAMS (A.P.P.) BY OTHERS (DROPPED)

**COFFERED CL.
- RAISED 12 "
- INSET 14 "

SITE COPY

M11,261

TAMARACK LUMBER INC	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON / BRADFORD	Model / Elevation: S38-15 / C OPT.COFF
	Layout ID: 288631	Project: GREEN VALLEY EAST	
	Plan Log: 93767	Date: 3/5/2018 Designer: Peter Dahlberg	

Mitek ver 7.6.4

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

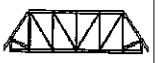
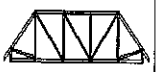
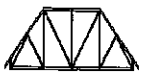
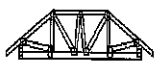
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44744 **LAYOUT ID:** 288643 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1 2 Ply	T01Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	2	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.34 156.34		
	2	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.60 153.34		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T15 HALF HIP	10.00 0.00	06-10-08	04-07-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-07-12	41.35 28.67		
	1	T16 HALF HIP	10.00 0.00	06-10-08	06-03-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-03-12	44.12 29.17		
	1	T17 HALF HIP	10.00 0.00	06-10-08	07-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-11-12	53.50 35.00		
	1	T18 MONOPITCH	10.00 0.00	06-10-08	09-04-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-04-07	50.82 32.67		
	1	T19 ROOF	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.76 40.67		
	1	T19S ROOF	10.00 6.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-10-05	55.77 38.17		
	2	T20S COMMON	10.00 0.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-11-13	112.54 72.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

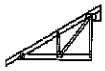
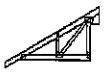
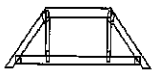
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44744 LAYOUT ID: 288643 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T21 ROOF	4.00 / 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-10-15	148.35 / 95.00		
	4	T22 ROOF	4.00 / 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-06-15	105.28 / 66.00		
	1	T23 COMMON	4.00 / 0.00	12-03-00	02-04-03	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	33.44 / 20.33		
	2	PB01 PIGGYBACK	10.00 / 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.78 / 30.66		
	2	C04 JACK-PARTIAL	4.00 / 0.00	06-01-08	04-09-10	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-08	119.76 / 77.34		
	20	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	335.80 / 213.40		
	4	J02 JACK-OPEN	10.00 / 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-15	31.36 / 24.00		
	1	J04 JACK-PARTIAL	4.00 / 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-04-07	18.32 / 11.50		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2703.93** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4251.20** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
6	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

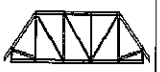
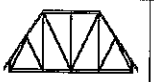
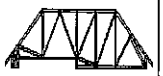
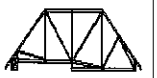
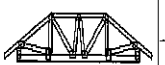
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288629 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	1	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.76 84.83		
	1	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.24 82.67		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	344.26 218.66		
	1	T10 HIPP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	130.82 85.17		
	1	T11 HIPP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.03 90.17		
	1	T12 HIPP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.83 94.33		
	1	T13 HIPP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.53 90.33		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T15 HALF HIP	10.00 0.00	06-10-08	04-07-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-07-12	41.35 28.67		
	1	T16 HALF HIP	10.00 0.00	06-10-08	06-03-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-03-12	44.12 29.17		
	1	T17 HALF HIP	10.00 0.00	06-10-08	07-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-11-12	53.50 35.00		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

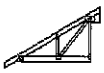
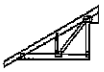
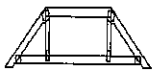
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288629 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T18 MONOPITCH	10.00 0.00	06-10-08	09-04-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-04-07	50.82 32.67		
	1	T19 ROOF	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.76 40.67		
	1	T19S ROOF	10.00 6.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-10-05	55.77 38.17		
	2	T20S COMMON	10.00 0.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-11-13	112.54 72.34		
	5	T21 ROOF	4.00 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-10-15	148.35 95.00		
	4	T22 ROOF	4.00 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-15	105.28 66.00		
	1	T23 COMMON	4.00 0.00	12-03-00	02-04-03	2 X 4	2 X 4	01-03-08 01-03-08	00-03-15 00-03-15	33.44 20.33		
	2	PB01 PIGGYBACK	10.00 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.78 30.66		
	14	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	6	J01A JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	123.42 91.98		
	4	J02 JACK-OPEN	10.00 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-11-15	31.36 24.00		
	1	J04 JACK-PARTIAL	4.00 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-07	18.32 11.50		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2740.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4274.98** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **9.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

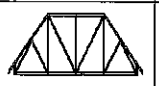
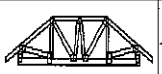
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295461 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: A-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T01 HIP GIRDER	10.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	269.14 / 170.00		
	1 / 2 Ply	T01Z2 HIP GIRDER	10.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	269.14 / 170.00		
	2	T02 HIP	10.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	245.34 / 156.34		
	2	T03 HIP	10.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	245.60 / 153.34		
	2	T04 HIP	10.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	269.52 / 169.66		
	2	T05 HIP	10.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	262.48 / 165.34		
	2	T06 HIP	10.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.84 / 177.00		
	2	T07 HIP	10.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	311.00 / 196.00		
	2	T07A PIGGYBACK	10.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	311.00 / 196.00		
	2	T08 HIP	10.00 / 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	322.74 / 199.34		
	1 / 3 Ply	T14 HIP GIRDER	6.00 / 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	207.45 / 135.99		
	1	T15 HALF HIP	10.00 / 0.00	06-10-08	04-07-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-07-12	41.35 / 28.67		
	1	T16 HALF HIP	10.00 / 0.00	06-10-08	06-03-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-03-12	44.12 / 29.17		
	1	T17 HALF HIP	10.00 / 0.00	06-10-08	07-11-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 07-11-12	53.50 / 35.00		
	1	T18 MONOPITCH	10.00 / 0.00	06-10-08	09-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 09-04-07	50.82 / 32.67		
	1	T19 ROOF	10.00 / 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	59.76 / 40.67		
	1	T19S ROOF	10.00 / 6.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-10-05	55.77 / 38.17		
	2	T20S COMMON	10.00 / 0.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-13	112.54 / 72.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

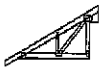
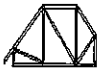
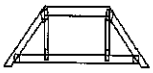
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295461 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** A-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T21 ROOF	4.00 / 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-10-15	148.35 / 95.00		
	4	T22 ROOF	4.00 / 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-06-15	105.28 / 66.00		
	1	T23 COMMON	4.00 / 0.00	12-03-00	02-04-03	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	33.44 / 20.33		
	1	T100 HIP GIRDER	10.00 / 0.00	13-11-00	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-08	62.70 / 40.50		
	1	T101 HIP	10.00 / 0.00	13-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-08	72.16 / 45.67		
	2	PB01 PIGGYBACK	10.00 / 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.78 / 30.66		
	12	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	201.48 / 128.04		
	4	J02 JACK-OPEN	10.00 / 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-15	31.36 / 24.00		
	1	J04 JACK-PARTIAL	4.00 / 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-04-07	18.32 / 11.50		
	6	J100 JACK-OPEN	10.00 / 0.00	02-10-08	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-03-11 / 05-08-07	93.78 / 60.00		
	2	C04 JACK-PARTIAL	4.00 / 0.00	06-01-08	04-09-10	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-08	119.76 / 77.34		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2764.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4345.52** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
7	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

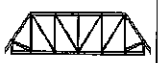
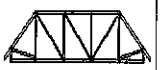
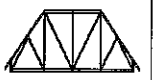
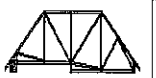
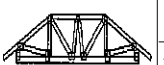
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295462 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: A-OPT.COFF.REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	1	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.76 84.83		
	1	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.24 82.67		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	344.26 218.66		
	1	T10 HIPP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	130.82 85.17		
	1	T11 HIPP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.03 90.17		
	1	T12 HIPP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.83 94.33		
	1	T13 HIPP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.53 90.33		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T15 HALF HIP	10.00 0.00	06-10-08	04-07-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-07-12	41.35 28.67		
	1	T16 HALF HIP	10.00 0.00	06-10-08	06-03-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-03-12	44.12 29.17		
	1	T17 HALF HIP	10.00 0.00	06-10-08	07-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-11-12	53.50 35.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

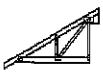
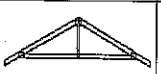
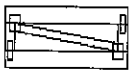
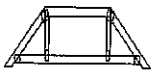
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295462 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: A-OPT.COFF.REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T18 MONOPITCH	10.00 / 0.00	06-10-08	09-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 09-04-07	50.82 / 32.67		
	1	T19 ROOF	10.00 / 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	59.76 / 40.67		
	1	T19S ROOF	10.00 / 6.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-10-05	55.77 / 38.17		
	2	T20S COMMON	10.00 / 0.00	11-10-00	06-08-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-13	112.54 / 72.34		
	5	T21 ROOF	4.00 / 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-10-15	148.35 / 95.00		
	4	T22 ROOF	4.00 / 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-06-15	105.28 / 66.00		
	1	T23 COMMON	4.00 / 0.00	12-03-00	02-04-03	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	33.44 / 20.33		
	1	T100S ROOF	10.00 / 0.00	13-11-00	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-08	62.93 / 42.33		
	1	T101S HIP	10.00 / 0.00	13-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-08	69.45 / 45.50		
	1 / 2 Ply	T102 ROOF	0.00 / 0.00	05-10-08	01-08-00	2 X 6	2 X 6	00-00-00 / 00-00-00	01-08-00 / 01-08-00	55.64 / 34.00		
	2	PB01 PIGGYBACK	10.00 / 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.78 / 30.66		
	12	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	201.48 / 128.04		
	4	J02 JACK-OPEN	10.00 / 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-15	31.36 / 24.00		
	1	J04 JACK-PARTIAL	4.00 / 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-04-07	18.32 / 11.50		
	6	J100S JACK-OPEN	10.00 / 0.00	02-10-08	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-03-11 / 04-08-07	124.56 / 85.02		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2834.40** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4430.56** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295462 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** A-OPT.COFF.REAR UPGRADE

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LUS24	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

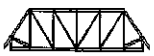
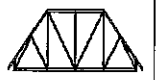
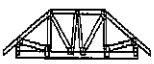
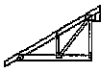
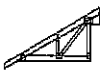
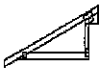
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288646 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.34 156.34		
	2	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.60 153.34		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T20 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.09 38.17		
	5	T21 ROOF	4.00 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-10-15	148.35 95.00		
	4	T22 ROOF	4.00 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-15	105.28 66.00		
	3	T24 SCISSORS	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.00 113.01		
	1	T25 HIP GIRDER	10.00 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	131.45 82.00		
	1 2 Ply	T26 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	326.92 204.00		
	1 2 Ply	T26Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	326.92 204.00		
	6	T27 MONOPITCH	4.00 0.00	05-11-00	02-07-02	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-03-10	123.48 76.02		
	1 2 Ply	T150 JACK-CLOSED	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-01-04	63.56 40.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

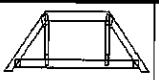
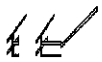
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288646 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB01 PIGGYBACK	10.00 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.78 30.66		
	19	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	319.01 202.73		
	4	J02 JACK-OPEN	10.00 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-11-15	36.84 28.00		
	1	J20 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	18.99 12.00		
	1	C04 JACK-OPEN	10.00 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	70.45 46.00		

TOTAL # TRUSS= **72.00** TOTAL BFT OF ALL TRUSSES= **2787.26** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4407.09** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
7	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

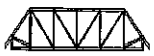
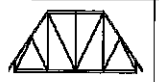
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288630 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	1	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.76 84.83		
	1	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.24 82.67		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	344.12 218.66		
	1	T10 HIPP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	130.82 85.17		
	1	T11 HIPP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.02 90.17		
	1	T12 HIPP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	146.26 93.50		
	1	T13 HIPP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.53 90.33		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T20 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.09 38.17		
	5	T21 ROOF	4.00 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-10-15	148.35 95.00		
	4	T22 ROOF	4.00 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-15	105.28 66.00		
	3	T24 SCISSORS	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.00 113.01		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

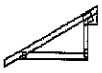
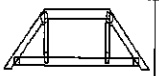
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288630 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T25 HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	131.45 / 82.00		
	1 / 2 Ply	T26 HIP GIRDER	10.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	326.92 / 204.00		
	6	T27 MONOPITCH	4.00 / 0.00	05-11-00	02-07-02	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-03-10	123.48 / 76.02		
	1 / 2 Ply	T150 JACK-CLOSED	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 04-01-04	63.56 / 40.66		
	2	PB01 PIGGYBACK	10.00 / 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.78 / 30.66		
	13	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	218.27 / 138.71		
	6	J01A JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	127.08 / 93.00		
	4	J02 JACK-OPEN	10.00 / 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-15	36.84 / 28.00		
	1	J20 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	18.99 / 12.00		
	1	C04 JACK-OPEN	10.00 / 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 00-03-08	70.45 / 46.00		

TOTAL # TRUSS= **72.00** TOTAL BFT OF ALL TRUSSES= **2867.73** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4494.79** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
7	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

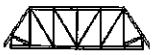
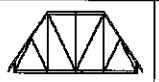
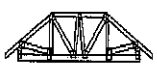
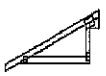
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295493 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** B-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.34 156.34		
	2	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.60 153.34		
	2	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.52 169.66		
	2	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.48 165.34		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T20 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.09 38.17		
	5	T21 ROOF	4.00 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-10-15	148.35 95.00		
	4	T22 ROOF	4.00 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-15	105.28 66.00		
	3	T24 SCISSORS	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.00 113.01		
	1	T25 HIP GIRDER	10.00 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	131.45 82.00		
	1 2 Ply	T26 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	326.92 204.00		
	1 2 Ply	T26Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	326.92 204.00		
	6	T27 MONOPITCH	4.00 0.00	05-11-00	02-07-02	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-03-10	123.48 76.02		
	1 2 Ply	T150 JACK-CLOSED	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-01-04	63.56 40.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

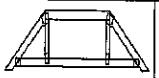
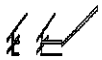
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295493 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: B-REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T160 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	144.20 93.34		
	1	G160 COMMON	10.00 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.25 50.67		
	2	PB01 PIGGYBACK	10.00 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.78 30.66		
	10	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	4	J02 JACK-OPEN	10.00 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-11-15	36.84 28.00		
	1	J20 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	18.99 12.00		
	1	C04 JACK-OPEN	10.00 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	70.45 46.00		

TOTAL # TRUSS= **66.00** TOTAL BFT OF ALL TRUSSES= **2835.24** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4477.43** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

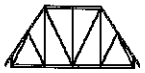
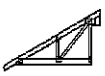
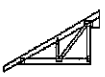
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295492 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: B-OPT.COFF.REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T02 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.67 78.17		
	1	T03 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.80 76.67		
	1	T04 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.76 84.83		
	1	T05 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.24 82.67		
	2	T06 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.84 177.00		
	2	T07 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T07A PIGGYBACK	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	311.00 196.00		
	2	T08 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	1 2 Ply	T09Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	344.12 218.66		
	1	T10 HIPP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	130.82 85.17		
	1	T11 HIPP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.02 90.17		
	1	T12 HIPP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	146.26 93.50		
	1	T13 HIPP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.53 90.33		
	1 3 Ply	T14 HIP GIRDER	6.00 0.00	13-00-00	02-11-15	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	207.45 135.99		
	1	T20 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	59.09 38.17		
	5	T21 ROOF	4.00 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-10-15	148.35 95.00		
	4	T22 ROOF	4.00 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-15	105.28 66.00		
	3	T24 SCISSORS	10.00 6.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.00 113.01		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

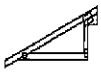
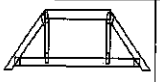
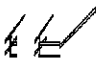
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295492 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** B-OPT.COFF.REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T25 HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	131.45 / 82.00		
	1 / 2 Ply	T26 HIP GIRDER	10.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	326.92 / 204.00		
	6	T27 MONOPITCH	4.00 / 0.00	05-11-00	02-07-02	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-03-10	123.48 / 76.02		
	1 / 2 Ply	T150 JACK-CLOSED	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 04-01-04	63.56 / 40.66		
	2	T160S ROOF	10.00 / 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	147.10 / 100.00		
	1	G160 COMMON	10.00 / 0.00	15-06-00	08-01-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	77.25 / 50.67		
	2	PB01 PIGGYBACK	10.00 / 0.00	07-04-01	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.78 / 30.66		
	10	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	167.90 / 106.70		
	4	J02 JACK-OPEN	10.00 / 0.00	01-07-08	02-11-15	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-11-15	36.84 / 28.00		
	1	J20 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	18.99 / 12.00		
	1	C04 JACK-OPEN	10.00 / 0.00	05-10-08	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 00-03-08	70.45 / 46.00		

TOTAL # TRUSS= **66.00** TOTAL BFT OF ALL TRUSSES= **2893.39** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4541.69** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

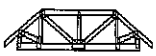
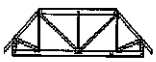
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288647 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: C -STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T21 ROOF	4.00 / 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-10-15	148.35 / 95.00		
	4	T22 ROOF	4.00 / 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-06-15	105.28 / 66.00		
	1 / 2 Ply	T28 HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	321.30 / 196.68		
	1 / 2 Ply	T28Z HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	321.30 / 196.68		
	2	T29 HIP	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	231.60 / 145.34		
	2	T30 HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	252.02 / 158.66		
	2	T31 HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	255.52 / 161.00		
	2	T32 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	268.64 / 166.66		
	2	T33 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	263.06 / 162.68		
	2	T34 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	267.64 / 167.34		
	4	T35 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	539.16 / 334.68		
	1 / 3 Ply	T41 HIP GIRDER	6.00 / 0.00	13-00-00	02-05-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	193.11 / 123.99		
	1	T44 HIP GIRDER	8.00 / 0.00	19-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	99.62 / 62.33		
	1	T50 HIP GIRDER	8.00 / 0.00	12-03-00	05-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-04-13 / 02-04-13	59.45 / 38.00		
	1 / 2 Ply	T51 JACK-CLOSED	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 04-01-04	57.90 / 37.00		
	1 / 2 Ply	T52 FLAT	0.00 / 0.00	05-10-08	01-00-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-00-00 / 01-00-00	37.74 / 25.66		
	21	J01 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	352.59 / 224.07		
	4	J50 JACK-OPEN	8.00 / 0.00	01-07-08	02-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-05-13	28.64 / 18.68		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 288647 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL **SUB-BUILDER:**
MODEL: S38-15 **ELEVATION:** C -STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	3	J51 JACK-OPEN	8.00 0.00	04-10-08	05-07-13	2 X 4	2 X 4	01-03-08 00-00-00	02-04-13 05-07-13	55.95 34.50		
	2	J52 JACK-OPEN	8.00 0.00	03-09-07	04-11-02	2 X 4	2 X 4	01-03-08 01-01-01	02-04-13 00-03-08	34.32 21.66		
	2	J53 JACK-OPEN	8.00 0.00	01-09-07	03-07-02	2 X 4	2 X 4	01-03-08 03-01-01	02-04-13 00-03-08	28.16 17.66		
	2	J54 JACK-OPEN	8.00 0.00	01-09-07	03-07-02	2 X 4	2 X 4	01-03-08 00-01-01	02-04-13 00-03-08	17.62 10.66		
	2	J55 JACK-OPEN	8.00 0.00	01-10-08	04-11-02	2 X 4	2 X 4	01-03-08 01-10-15	02-04-13 00-05-03	26.88 17.66		

TOTAL # TRUSS= **74.00** TOTAL BFT OF ALL TRUSSES= **2482.59** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3965.85** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

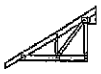
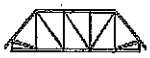
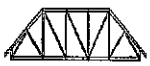
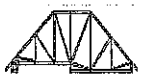
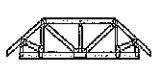
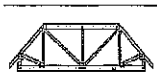
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288631 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T21 ROOF	4.00 / 0.00	07-09-00	03-02-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-10-15	148.35 / 95.00		
	4	T22 ROOF	4.00 / 0.00	06-09-00	02-10-07	2 X 4	2 X 4	01-03-08 / 00-10-08	00-03-15 / 02-06-15	105.28 / 66.00		
	1 / 2 Ply	T28 HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	321.30 / 196.68		
	1	T29 HIP	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	115.80 / 72.67		
	1	T30 HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	126.01 / 79.33		
	1	T31 HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	127.76 / 80.50		
	1	T32 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	134.32 / 83.33		
	2	T33 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	263.06 / 162.68		
	2	T34 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	267.64 / 167.34		
	4	T35 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	539.16 / 334.68		
	1 / 2 Ply	T36 HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	335.76 / 212.68		
	1	T37 HIPP	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	128.55 / 82.83		
	1	T38 HIPP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	134.39 / 85.50		
	1	T39 HIPP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	136.07 / 87.34		
	1	T40 HIPP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	142.71 / 90.00		
	1 / 3 Ply	T41 HIP GIRDER	6.00 / 0.00	13-00-00	02-05-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-02-00 / 01-02-00	193.11 / 123.99		
	1	T44 HIP GIRDER	8.00 / 0.00	19-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	99.62 / 62.33		
	1	T50 HIP GIRDER	8.00 / 0.00	12-03-00	05-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-04-13 / 02-04-13	59.45 / 38.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

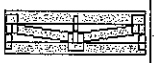
Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288631 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
MODEL: S38-15 ELEVATION: C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T51 JACK-CLOSED	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-01-04	57.90 37.00		
	1 2 Ply	T52 FLAT	0.00 0.00	05-10-08	01-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-00-00 01-00-00	37.74 25.66		
	14	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	7	J01A JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	148.26 108.50		
	4	J50 JACK-OPEN	8.00 0.00	01-07-08	02-05-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-05-13	28.64 18.68		
	3	J51 JACK-OPEN	8.00 0.00	04-10-08	05-07-13	2 X 4	2 X 4	01-03-08 00-00-00	02-04-13 05-07-13	55.95 34.50		
	2	J52 JACK-OPEN	8.00 0.00	03-09-07	04-11-02	2 X 4	2 X 4	01-03-08 01-01-01	02-04-13 00-03-08	34.32 21.66		
	2	J53 JACK-OPEN	8.00 0.00	01-09-07	03-07-02	2 X 4	2 X 4	01-03-08 03-01-01	02-04-13 00-03-08	28.16 17.66		
	2	J54 JACK-OPEN	8.00 0.00	01-09-07	03-07-02	2 X 4	2 X 4	01-03-08 00-01-01	02-04-13 00-03-08	17.62 10.66		
	2	J55 JACK-OPEN	8.00 0.00	01-10-08	04-11-02	2 X 4	2 X 4	01-03-08 01-10-15	02-04-13 00-05-03	26.88 17.66		

TOTAL # TRUSS= **74.00** TOTAL BFT OF ALL TRUSSES= **2562.24** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4048.87** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
1	Hangers	LUS26-2	

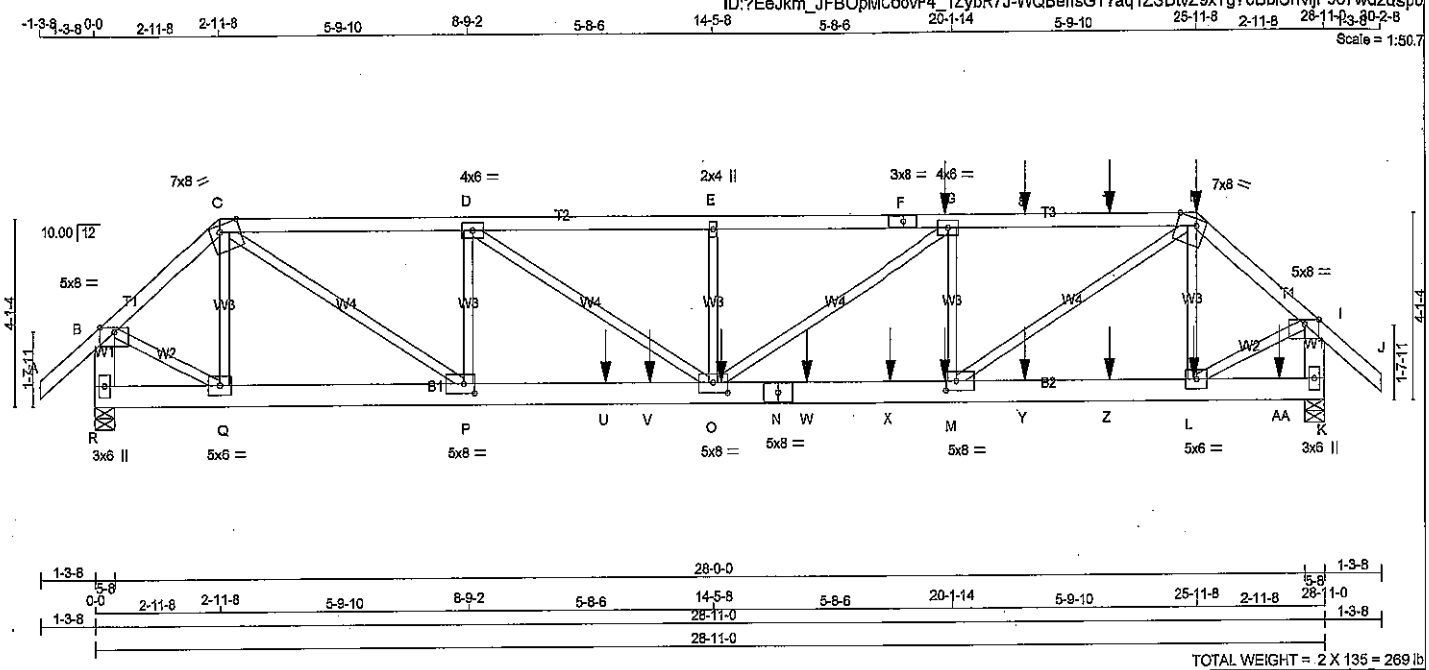
TOTAL # ITEMS= **7.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T01	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:01 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WQBensGY?aq1ZSDtvZ9xYg?oBblShvljF967wdzdspD



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-H	1	12	SIDE(0.0)
H-J	1	12	SIDE(0.0)
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-N	2	12	SIDE(0.0)
N-K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	5.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	5.50
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.00
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	8.0	2.75	4.00
P	BMWW-t	MT20	5.0	8.0	2.50	3.00
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3885	0	3885	0	0	5-8	5-8
K	4346	0	4346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3025	1964 / 0	518 / 0	0 / 0	0 / 0	544 / 0	0 / 0
K	3415	2162 / 0	618 / 0	0 / 0	0 / 0	635 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B		0 / 47	-104.9	-104.9	0.08 (1)	10.00
B-C		-3735 / 0	-104.9	-104.9	0.14 (1)	4.77
C-D		-7662 / 0	-104.9	-104.9	0.31 (1)	4.17
D-E		-9892 / 0	-104.9	-104.9	0.39 (1)	3.69
E-F		-9892 / 0	-104.9	-104.9	0.46 (1)	3.61
F-G		-9892 / 0	-104.9	-104.9	0.46 (1)	3.61
G-S		-7754 / 0	-104.9	-104.9	0.40 (1)	4.06
S-T		-7754 / 0	-104.9	-104.9	0.40 (1)	4.06
T-H		-7754 / 0	-104.9	-104.9	0.40 (1)	4.06
H-I		-4257 / 0	-104.9	-104.9	0.15 (1)	4.50
I-J		0 / 47	-104.9	-104.9	0.08 (1)	10.00
R-B		-3843 / 0	0.0	0.0	0.14 (1)	7.22
K-I		-4338 / 0	0.0	0.0	0.16 (1)	6.88
R-Q		0 / 0	-27.5	-27.5	0.02 (3)	10.00
Q-P		0 / 2828	-27.5	-27.5	0.19 (1)	10.00
P-U		0 / 7663	-27.5	-27.5	0.56 (1)	10.00
U-V		0 / 7663	-27.5	-27.5	0.56 (1)	10.00
V-O		0 / 7663	-27.5	-27.5	0.56 (1)	10.00
O-N		0 / 7754	-27.5	-27.5	0.40 (1)	10.00
N-W		0 / 7754	-27.5	-27.5	0.40 (1)	10.00
W-X		0 / 7754	-27.5	-27.5	0.40 (1)	10.00
X-M		0 / 7754	-27.5	-27.5	0.40 (1)	10.00
M-Y		0 / 3230	-27.5	-27.5	0.12 (1)	10.00
Y-Z		0 / 3230	-27.5	-27.5	0.12 (1)	10.00
Z-L		0 / 3230	-27.5	-27.5	0.12 (1)	10.00
L-AA		0 / 0	-27.5	-27.5	0.04 (2)	10.00
AA-K		0 / 0	-27.5	-27.5	0.04 (2)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C		-1116 / 0	0.14 (1)
C-P		0 / 5737	0.71 (1)
P-D		-2129 / 0	0.27 (1)
D-O		0 / 2665	0.33 (1)
O-E		-501 / 0	0.06 (1)
O-G		0 / 2555	0.32 (1)
M-G		-2372 / 0	0.30 (1)
M-H		0 / 5369	0.66 (1)
L-H		-1003 / 0	0.13 (1)
B-Q		0 / 3078	0.38 (1)
L-I		0 / 3507	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	19-11-12	-126	-126	---	FRONT	VERT	TOTAL
H	25-11-8	-323	-323	---	FRONT	VERT	TOTAL
L	25-10-12	-39	-68	---	FRONT	VERT	TOTAL
M	19-11-12	-39	-68	---	FRONT	VERT	TOTAL
O	14-7-12	-428	-428	---	FRONT	VERT	TOTAL
S	21-10-12	-126	-126	---	FRONT	VERT	TOTAL
T	23-10-12	-126	-126	---	FRONT	VERT	TOTAL
U	11-11-8	-1516	-1516	---	FRONT	VERT	TOTAL
V	12-11-12	-428	-428	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 852 (0.41")

CSI: TC=0.46/1.00 (E-G:1) , BC=0.56/1.00 (O-P:1) , WB=0.71/1.00 (C-P:1) , SSI=0.44/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.98 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12193-T01
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T01	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WQBensGY?aq1ZSDtvZ9xYg?oBbIShvjjF967wdzdsp0

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 322.9 lbs FACTORED DOWN AT 25-11-8, 126.3 lbs FACTORED DOWN AT 19-11-12, AND 126.3 lbs FACTORED DOWN AT 21-10-12, AND 126.3 lbs FACTORED DOWN AT 23-10-12 ON TOP CHORD, AND 1515.9 lbs FACTORED DOWN AT 11-11-8, 427.7 lbs FACTORED DOWN AT 12-11-12, 427.7 lbs FACTORED DOWN AT 14-7-12, 427.7 lbs FACTORED DOWN AT 16-7-12, 413.1 lbs FACTORED DOWN AT 18-7-12, 67.8 lbs FACTORED DOWN AT 19-11-12, 67.8 lbs FACTORED DOWN AT 21-10-12, 67.8 lbs FACTORED DOWN AT 23-10-12, AND 67.8 lbs FACTORED DOWN AT 25-10-12, AND 67.8 lbs FACTORED DOWN AT 27-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	16-7-12	-428	-428	—	FRONT	VERT	TOTAL
X	18-7-12	-413	-413	—	FRONT	VERT	TOTAL
Y	21-10-12	-39	-68	—	FRONT	VERT	TOTAL
Z	23-10-12	-39	-68	—	FRONT	VERT	TOTAL
AA	27-10-12	-39	-68	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3518
S. KATSOULAKOS
PROVINCE OF ONTARIO

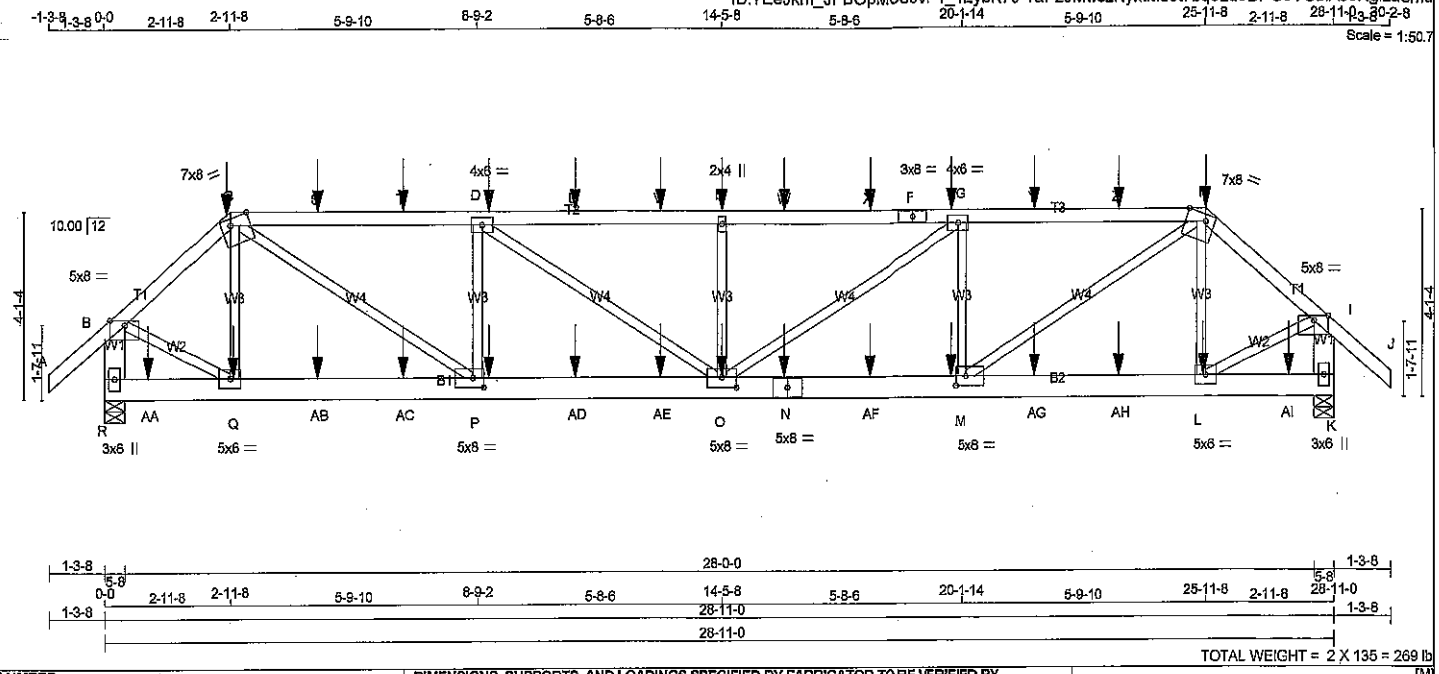
DWG NO. TAM 12193-17B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44744	DRWG NO.
288643	T01Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:11:17 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-YaP2JMwszNyktMd0t7bq3Ld3BPG9YUblXJ3Kglzdsmu



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(0.0)
C- F	1	12	SIDE(0.0)
F- H	1	12	SIDE(0.0)
H- J	1	12	SIDE(0.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	5.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	5.50
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.00
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	8.0	2.75	4.00
P	BMWW-t	MT20	5.0	8.0	2.50	3.00
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	3372	0	3372	0	0	5-8	5-8
K	3372	0	3372	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2690	1632 / 0	532 / 0	0 / 0	0 / 0	526 / 0	0 / 0
K	2691	1632 / 0	532 / 0	0 / 0	0 / 0	526 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

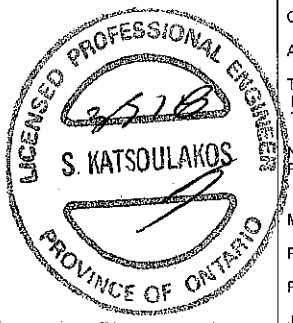
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-3224 / 0	-104.9	-104.9	0.13 (1)	5.07
C- S	-5470 / 0	-104.9	-104.9	0.35 (1)	4.69
S- T	-5470 / 0	-104.9	-104.9	0.35 (1)	4.69
T- D	-5470 / 0	-104.9	-104.9	0.35 (1)	4.69
D- U	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
U- V	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
V- E	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
E- W	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
W- X	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
X- F	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
F- G	-6401 / 0	-104.9	-104.9	0.38 (1)	4.39
G- Y	-5471 / 0	-104.9	-104.9	0.35 (1)	4.69
Y- Z	-5471 / 0	-104.9	-104.9	0.35 (1)	4.69
Z- H	-5471 / 0	-104.9	-104.9	0.35 (1)	4.69
H- I	-3224 / 0	-104.9	-104.9	0.13 (1)	5.07
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
R- B	-3358 / 0	0.0	0.0	0.12 (1)	7.60
K- I	-3359 / 0	0.0	0.0	0.12 (1)	7.60
R-AA	0 / 0	-27.5	-27.5	0.04 (2)	10.00
AA- Q	0 / 0	-27.5	-27.5	0.04 (2)	10.00
Q-AB	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
AB-AC	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
AC- P	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
P-AD	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
AD-AE	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
AE- O	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
O- N	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
N-AF	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
AF- M	0 / 5471	-27.5	-27.5	0.19 (1)	10.00
M-AG	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
AG-AH	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
AH- L	0 / 2449	-27.5	-27.5	0.10 (1)	10.00
L-AI	0 / 0	-27.5	-27.5	0.04 (2)	10.00
AI- K	0 / 0	-27.5	-27.5	0.04 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-686 / 0	0.09 (1)
C- P	0 / 3586	0.44 (1)
P- D	-1667 / 0	0.21 (1)
D- O	0 / 1112	0.14 (1)
O- E	-943 / 0	0.12 (1)
O- G	0 / 1111	0.14 (1)
M- G	-1667 / 0	0.21 (1)
M- H	0 / 3587	0.44 (1)
L- H	-686 / 0	0.09 (1)
B- Q	0 / 2656	0.33 (1)
L- I	0 / 2657	0.33 (1)



DWG NO. TAM12225-17B

STRUCTURAL COMPONENT ONLY

SITE COPY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.19/1.00 (M-O:1) , WB=0.44/1.00 (H-M:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)

[M]

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44744	DRWG NO.
288643	T01Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-0mzQXiwUkg5bUVCCRr63cZAEwpbOHxrumzotCBzdsmt

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 193.1 lbs FACTORED DOWN AT 2-11-8, 193.1 lbs FACTORED DOWN AT 25-11-8, 129.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 5-0-4, 126.3 lbs FACTORED DOWN AT 7-0-4, 126.3 lbs FACTORED DOWN AT 9-0-4, 126.3 lbs FACTORED DOWN AT 11-0-4, 126.3 lbs FACTORED DOWN AT 13-0-4, 126.3 lbs FACTORED DOWN AT 14-5-8, 126.3 lbs FACTORED DOWN AT 15-10-12, 126.3 lbs FACTORED DOWN AT 17-10-12, 126.3 lbs FACTORED DOWN AT 19-10-12, 126.3 lbs FACTORED DOWN AT 21-10-12, AND 126.3 lbs FACTORED DOWN AT 23-10-12, AND 129.8 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 67.8 lbs FACTORED DOWN AT 1-0-4, 67.8 lbs FACTORED DOWN AT 3-0-4, 67.8 lbs FACTORED DOWN AT 5-0-4, 67.8 lbs FACTORED DOWN AT 7-0-4, 67.8 lbs FACTORED DOWN AT 9-0-4, 67.8 lbs FACTORED DOWN AT 11-0-4, 67.8 lbs FACTORED DOWN AT 13-0-4, 67.8 lbs FACTORED DOWN AT 14-5-8, 67.8 lbs FACTORED DOWN AT 15-10-12, 67.8 lbs FACTORED DOWN AT 17-10-12, 67.8 lbs FACTORED DOWN AT 19-10-12, 67.8 lbs FACTORED DOWN AT 21-10-12, 67.8 lbs FACTORED DOWN AT 23-10-12, AND 67.8 lbs FACTORED DOWN AT 25-10-12, AND 67.8 lbs FACTORED DOWN AT 27-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-193	-193	—	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	—	BACK	VERT	TOTAL
D	9-0-4	-126	-126	—	BACK	VERT	TOTAL
E	14-5-8	-126	-126	—	BACK	VERT	TOTAL
G	19-10-12	-126	-126	—	BACK	VERT	TOTAL
H	25-11-8	-193	-193	—	FRONT	VERT	TOTAL
H	25-11-8	-130	-130	—	BACK	VERT	TOTAL
L	25-10-12	-39	-68	—	BACK	VERT	TOTAL
M	19-10-12	-39	-68	—	BACK	VERT	TOTAL
N	15-10-12	-39	-68	—	BACK	VERT	TOTAL
O	14-5-8	-39	-68	—	BACK	VERT	TOTAL
P	9-0-4	-39	-68	—	BACK	VERT	TOTAL
Q	3-0-4	-39	-68	—	BACK	VERT	TOTAL
S	5-0-4	-126	-126	—	BACK	VERT	TOTAL
T	7-0-4	-126	-126	—	BACK	VERT	TOTAL
U	11-0-4	-126	-126	—	BACK	VERT	TOTAL
V	13-0-4	-126	-126	—	BACK	VERT	TOTAL
W	15-10-12	-126	-126	—	BACK	VERT	TOTAL
X	17-10-12	-126	-126	—	BACK	VERT	TOTAL
Y	21-10-12	-126	-126	—	BACK	VERT	TOTAL
Z	23-10-12	-126	-126	—	BACK	VERT	TOTAL
AA	1-0-4	-39	-68	—	BACK	VERT	TOTAL
AB	5-0-4	-39	-68	—	BACK	VERT	TOTAL
AC	7-0-4	-39	-68	—	BACK	VERT	TOTAL
AD	11-0-4	-39	-68	—	BACK	VERT	TOTAL
AE	13-0-4	-39	-68	—	BACK	VERT	TOTAL
AF	17-10-12	-39	-68	—	BACK	VERT	TOTAL
AG	21-10-12	-39	-68	—	BACK	VERT	TOTAL
AH	23-10-12	-39	-68	—	BACK	VERT	TOTAL
AI	27-10-12	-39	-68	—	BACK	VERT	TOTAL

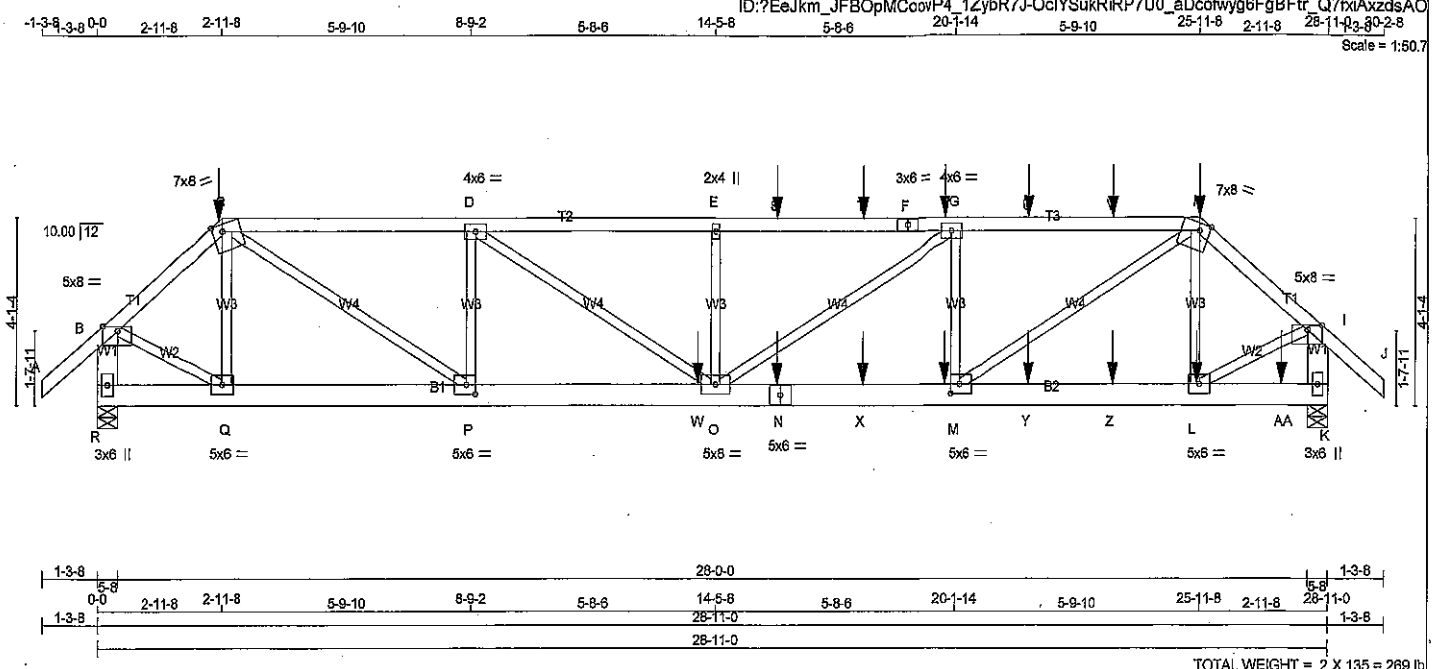
DWG NO. TAM 12225-1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295461	T01Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:52:21 2018 Page 1

ID:?EeJkm_JFBOpMCcovP4_1ZybR7J-OclYSukRiRP7U0_aDcofwyg6FgBFtr_Q7fxiAxzdsAO



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(0.0)
C- F	1	12	SIDE(0.0)
F- H	1	12	SIDE(0.0)
H- J	1	12	SIDE(0.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	2.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	3398	0	3398	0	0	5-8	5-8
K	3834	0	3834	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2672	1693 / 0	486 / 0	0 / 0	0 / 0	493 / 0	0 / 0
K	3041	1879 / 0	583 / 0	0 / 0	0 / 0	580 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-829 / 0	0.11 (1)	
B- C	-3261 / 0	-104.9	-104.9	0.13 (1)	5.05	C- P	0 / 4268	0.53 (1)	
C- D	-6088 / 0	-104.9	-104.9	0.25 (1)	4.63	P- D	-2043 / 0	0.26 (1)	
D- E	-8210 / 0	-104.9	-104.9	0.32 (1)	4.05	D- O	0 / 2538	0.31 (1)	
E- S	-8210 / 0	-104.9	-104.9	0.45 (1)	3.90	O- E	-671 / 0	0.09 (1)	
S- T	-8210 / 0	-104.9	-104.9	0.45 (1)	3.90	O- G	0 / 1989	0.25 (1)	
T- F	-8210 / 0	-104.9	-104.9	0.45 (1)	3.90	M- G	-2187 / 0	0.28 (1)	
F- G	-8210 / 0	-104.9	-104.9	0.45 (1)	3.90	M- H	0 / 4411	0.55 (1)	
G- U	-6545 / 0	-104.9	-104.9	0.40 (1)	4.33	L- H	-809 / 0	0.10 (1)	
U- V	-6545 / 0	-104.9	-104.9	0.40 (1)	4.33	B- Q	0 / 2687	0.33 (1)	
V- H	-6545 / 0	-104.9	-104.9	0.40 (1)	4.33	L- I	0 / 3068	0.38 (1)	
H- I	-3724 / 0	-104.9	-104.9	0.14 (1)	4.77				
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00				
R- B	-3393 / 0	0.0	0.0	0.13 (1)	7.57				
K- I	-3833 / 0	0.0	0.0	0.14 (1)	7.22				
R- Q	0 / 0	-27.5	-27.5	0.03 (2)	10.00				
Q- P	0 / 2473	-27.5	-27.5	0.09 (1)	10.00				
P- W	0 / 6086	-27.5	-27.5	0.27 (1)	10.00				
W- O	0 / 6086	-27.5	-27.5	0.27 (1)	10.00				
O- N	0 / 6545	-27.5	-27.5	0.21 (1)	10.00				
N- X	0 / 6545	-27.5	-27.5	0.21 (1)	10.00				
X- M	0 / 6545	-27.5	-27.5	0.21 (1)	10.00				
M- Y	0 / 2828	-27.5	-27.5	0.12 (1)	10.00				
Y- Z	0 / 2828	-27.5	-27.5	0.12 (1)	10.00				
Z- L	0 / 2828	-27.5	-27.5	0.12 (1)	10.00				
L- AA	0 / 0	-27.5	-27.5	0.04 (2)	10.00				
AA- K	0 / 0	-27.5	-27.5	0.04 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-193	-193	---	FRONT	VERT	TOTAL
G	19-10-12	-126	-126	---	BACK	VERT	TOTAL
H	25-11-8	-193	-193	---	FRONT	VERT	TOTAL
H	25-11-8	-130	-130	---	BACK	VERT	TOTAL
L	25-10-12	-39	-68	---	BACK	VERT	TOTAL
M	19-10-12	-39	-68	---	BACK	VERT	TOTAL
N	15-10-12	-39	-68	---	BACK	VERT	TOTAL
S	15-10-12	-126	-126	---	BACK	VERT	TOTAL
T	17-10-12	-126	-126	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.45/1.00 (E-G:1) , BC=0.27/1.00 (O-P:1) , WB=0.55/1.00 (H-M:1) , SSI=0.55/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.66 (N) (INPUT = 1.00)

PROFESSIONAL ENGINEER LICENSED 3518 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17226-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295461	T01Z2	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 193.1 lbs FACTORED DOWN AT 2-11-8, 193.1 lbs FACTORED DOWN AT 25-11-8, 126.3 lbs FACTORED DOWN AT 15-10-12, 126.3 lbs FACTORED DOWN AT 17-10-12, 126.3 lbs FACTORED DOWN AT 19-10-12, 126.3 lbs FACTORED DOWN AT 21-10-12, AND 126.3 lbs FACTORED DOWN AT 23-10-12, AND 129.8 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 1690.0 lbs FACTORED DOWN AT 14-0-8, 67.8 lbs FACTORED DOWN AT 15-10-12, 67.8 lbs FACTORED DOWN AT 17-10-12, 67.8 lbs FACTORED DOWN AT 19-10-12, 67.8 lbs FACTORED DOWN AT 21-10-12, 67.8 lbs FACTORED DOWN AT 23-10-12, AND 67.8 lbs FACTORED DOWN AT 25-10-12, AND 67.8 lbs FACTORED DOWN AT 27-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	21-10-12	-126	-126	—	BACK	VERT	TOTAL
V	23-10-12	-126	-126	—	BACK	VERT	TOTAL
W	14-0-8	-1690	-1690	—	BACK	VERT	TOTAL
X	17-10-12	-39	-68	—	BACK	VERT	TOTAL
Y	21-10-12	-39	-68	—	BACK	VERT	TOTAL
Z	23-10-12	-39	-68	—	BACK	VERT	TOTAL
AA	27-10-12	-39	-68	—	BACK	VERT	TOTAL

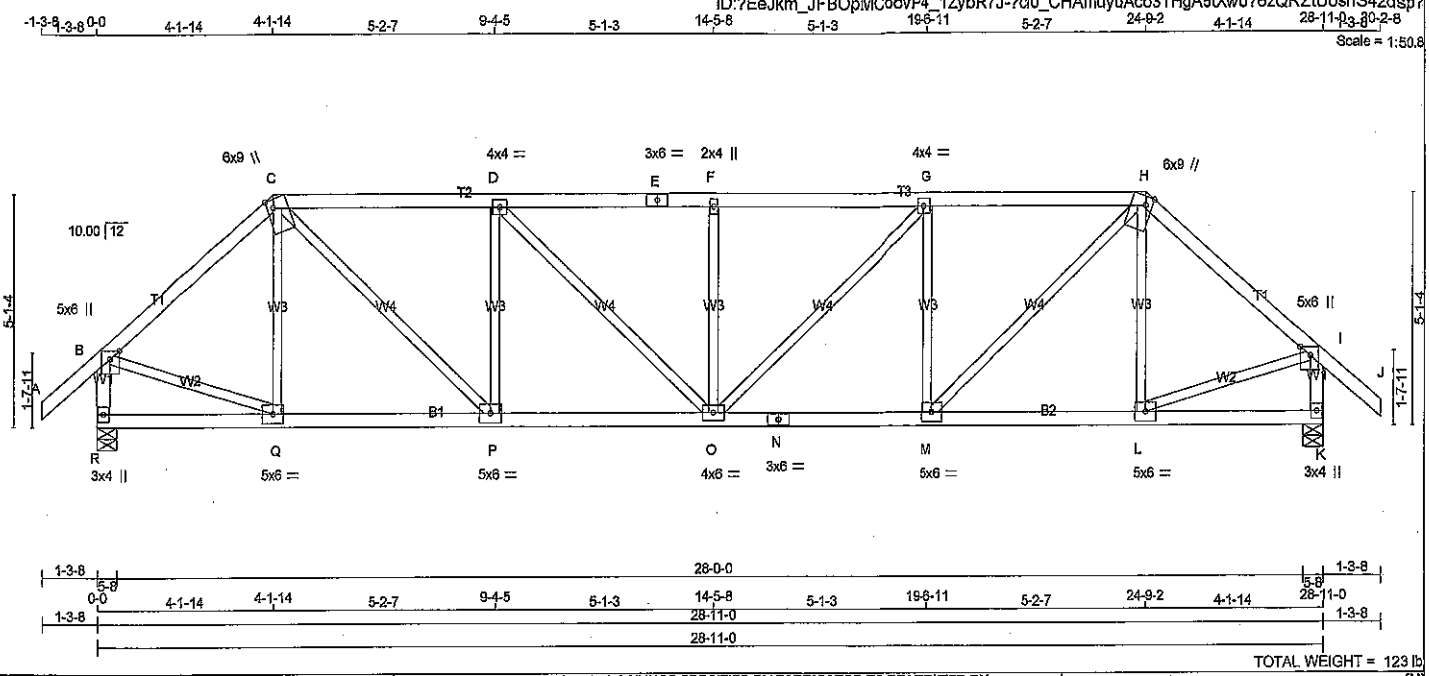
DWG NO. TAM 17216 -1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T02	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:02 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?cl0_CHAmuyuAco3THgA5tXw0?6zQRZtUoshS4zdsp?



TOTAL WEIGHT = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2060	0	2060	0	0	5-8	5-8
K	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	Q- C	-296 / 60	0.11 (1)
B- C	-1884 / 0	-104.9	-104.9	0.40 (1)	4.47	C- P	0 / 1564	0.35 (1)
C- D	-2588 / 0	-104.9	-104.9	0.55 (1)	3.74	P- D	-921 / 0	0.35 (1)
D- E	-2922 / 0	-104.9	-104.9	0.58 (1)	3.51	D- O	0 / 459	0.10 (1)
E- F	-2922 / 0	-104.9	-104.9	0.58 (1)	3.51	O- F	-493 / 0	0.19 (1)
F- G	-2922 / 0	-104.9	-104.9	0.58 (1)	3.51	O- G	0 / 459	0.10 (1)
G- H	-2588 / 0	-104.9	-104.9	0.55 (1)	3.74	M- G	-921 / 0	0.35 (1)
H- I	-1884 / 0	-104.9	-104.9	0.40 (1)	4.47	M- H	0 / 1564	0.35 (1)
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- H	-296 / 60	0.11 (1)
R- B	-2016 / 0	0.0	0.0	0.21 (1)	5.95	B- Q	0 / 1510	0.34 (1)
K- I	-2016 / 0	0.0	0.0	0.21 (1)	5.95	L- I	0 / 1510	0.34 (1)
R- Q	0 / 0	-27.5	-27.5	0.14 (3)	10.00			
Q- P	0 / 1437	-27.5	-27.5	0.31 (1)	10.00			
P- O	0 / 2588	-27.5	-27.5	0.48 (1)	10.00			
O- N	0 / 2588	-27.5	-27.5	0.48 (1)	10.00			
N- M	0 / 2588	-27.5	-27.5	0.48 (1)	10.00			
M- L	0 / 1437	-27.5	-27.5	0.31 (1)	10.00			
L- K	0 / 0	-27.5	-27.5	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.58/1.00 (F-G:1) , BC=0.48/1.00 (O-P:1) , WB=0.35/1.00 (G-M:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = .0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

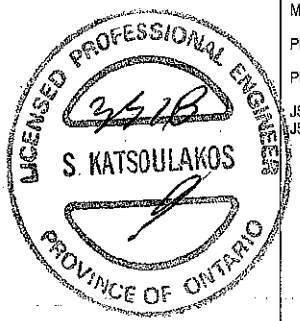
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)



DWG NO. TAM 12194-T2
STRUCTURAL COMPONENT ONLY

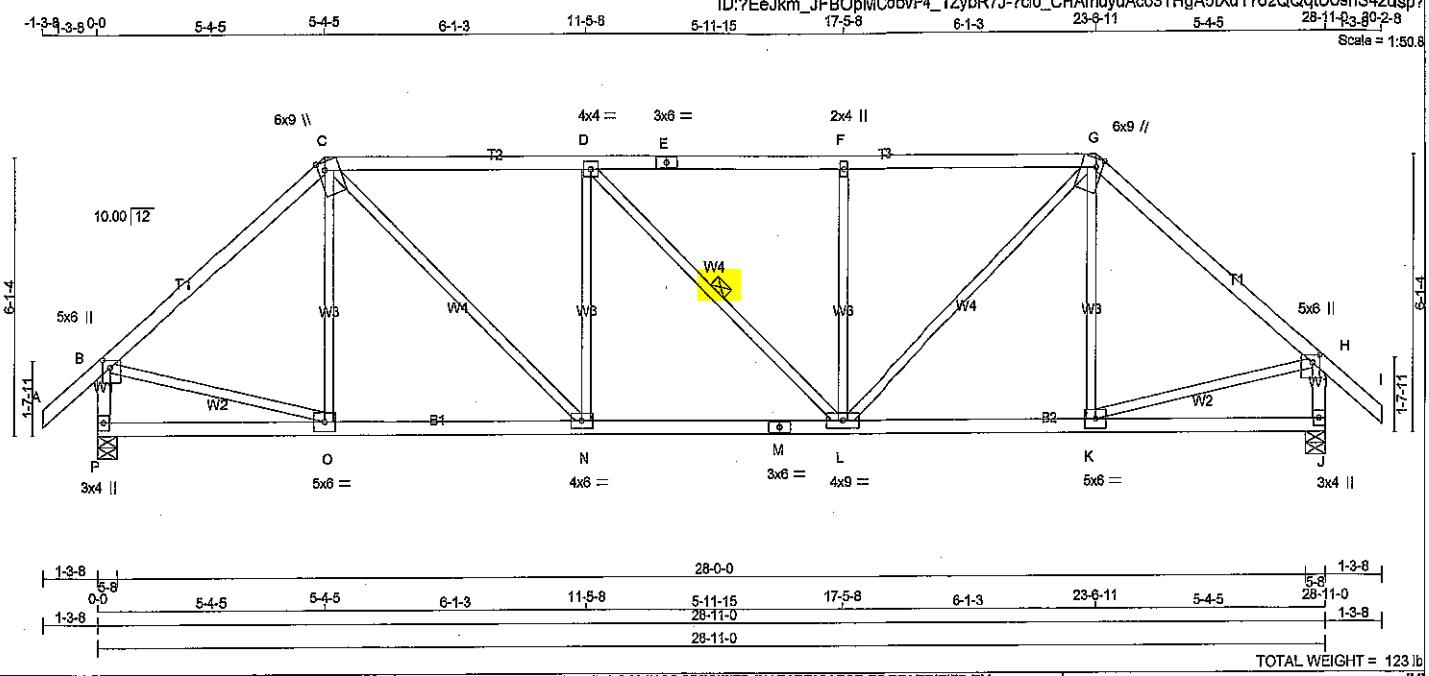
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T03	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:02 2018 Page 1

ID:?EeJkm_JFBOpMCovP4_1ZybR7J-?cl0_CHAmuyuAco3THgA5tXu1?62QQqtUoshS4zdsp?



TOTAL WEIGHT = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1909 / 0	-104.9	-104.9	0.70 (1)	4.06
C- D	-2355 / 0	-104.9	-104.9	0.71 (1)	3.68
D- E	-2354 / 0	-104.9	-104.9	0.70 (1)	3.68
E- F	-2354 / 0	-104.9	-104.9	0.70 (1)	3.68
F- G	-2354 / 0	-104.9	-104.9	0.70 (1)	3.69
G- H	-1909 / 0	-104.9	-104.9	0.70 (1)	4.06
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P- B	-2001 / 0	0.0	0.0	0.21 (1)	5.97
J- H	-2001 / 0	0.0	0.0	0.21 (1)	5.97
P- O	0 / 0	-27.5	-27.5	0.22 (3)	10.00
O- N	0 / 1460	-27.5	-27.5	0.37 (2)	10.00
N- M	0 / 2355	-27.5	-27.5	0.48 (1)	10.00
M- L	0 / 2355	-27.5	-27.5	0.48 (1)	10.00
L- K	0 / 1461	-27.5	-27.5	0.37 (2)	10.00
K- J	0 / 0	-27.5	-27.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-172 / 155	0.10 (1)
C- N	0 / 1234	0.28 (1)
N- D	-687 / 0	0.40 (1)
D- L	-2 / 0	0.00 (1)
L- F	-686 / 0	0.40 (1)
L- G	0 / 1231	0.28 (1)
K- G	-171 / 155	0.10 (1)
B- O	0 / 1505	0.34 (1)
K- H	0 / 1505	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.48/1.00 (L-N:1) , WB=0.40/1.00 (D-N:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.84 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12195-18
STRUCTURAL COMPONENT ONLY

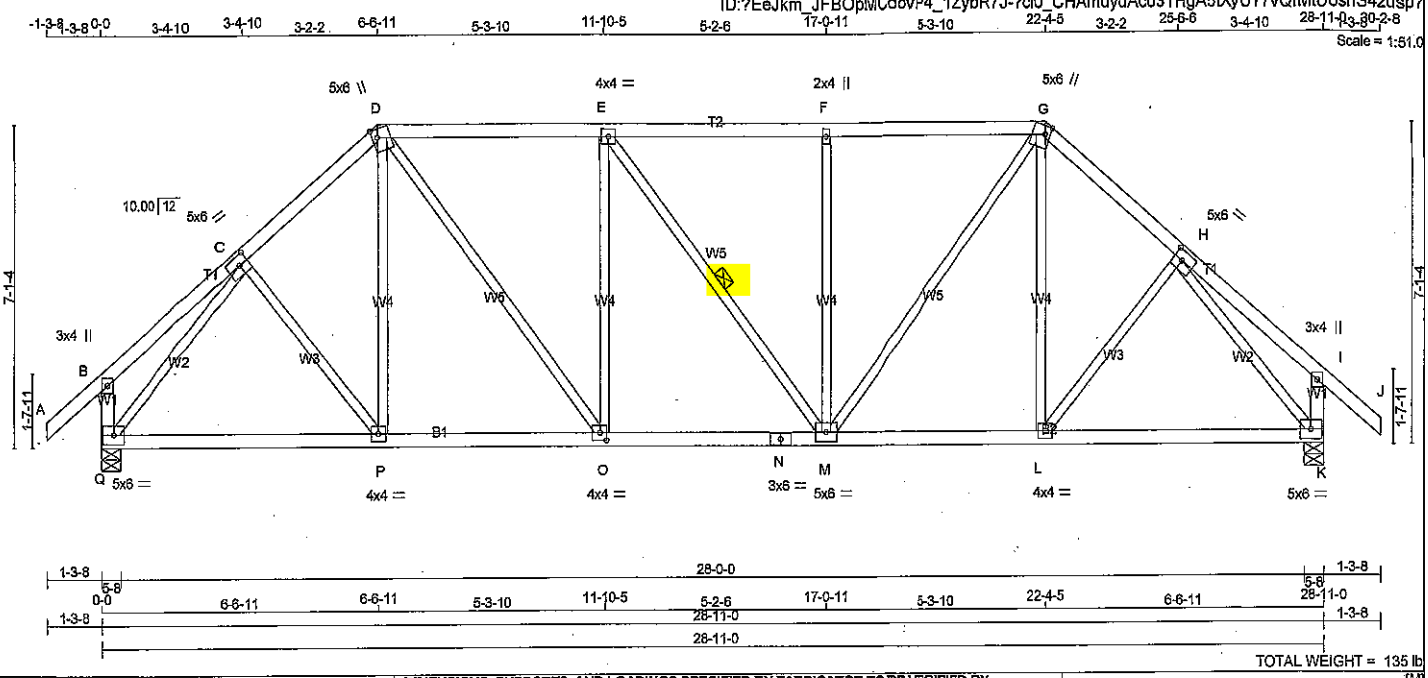
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T04	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:02 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?cl0_CHAmuyuAco3THgA5tXyU?7vQIMtUoshS4zdsp?



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2060	0	2060	0	0	5-8	5-8
K	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

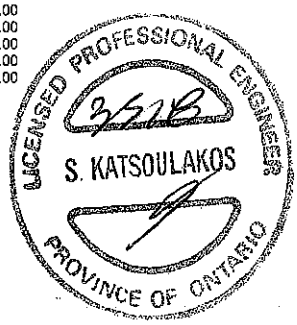
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00
C- D	-1910 / 0	-104.9	-104.9	0.21 (1)	4.67
D- E	-2014 / 0	-104.9	-104.9	0.49 (1)	4.22
E- F	-2013 / 0	-104.9	-104.9	0.49 (1)	4.22
F- G	-2012 / 0	-104.9	-104.9	0.48 (1)	4.24
G- H	-1910 / 0	-104.9	-104.9	0.21 (1)	4.67
H- I	0 / 23	-104.9	-104.9	0.16 (1)	10.00
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-280 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-280 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 1381	-27.5	-27.5	0.40 (2)	10.00
P- O	0 / 1447	-27.5	-27.5	0.42 (2)	10.00
O- N	0 / 2014	-27.5	-27.5	0.39 (1)	10.00
N- M	0 / 2014	-27.5	-27.5	0.39 (1)	10.00
M- L	0 / 1447	-27.5	-27.5	0.42 (2)	10.00
L- K	0 / 1381	-27.5	-27.5	0.40 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 147	0.03 (2)
P- D	0 / 224	0.05 (3)
D- O	0 / 922	0.21 (1)
O- E	-594 / 0	0.52 (1)
E- M	-3 / 0	0.00 (1)
M- F	-593 / 0	0.52 (1)
M- G	0 / 919	0.21 (1)
L- G	0 / 224	0.05 (3)
L- H	0 / 147	0.03 (2)
Q- C	-2194 / 0	0.94 (1)
H- K	-2194 / 0	0.94 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.42/1.00 (L-M:2) , WB=0.94/1.00 (H-K:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.54 (N) (INPUT = 1.00)

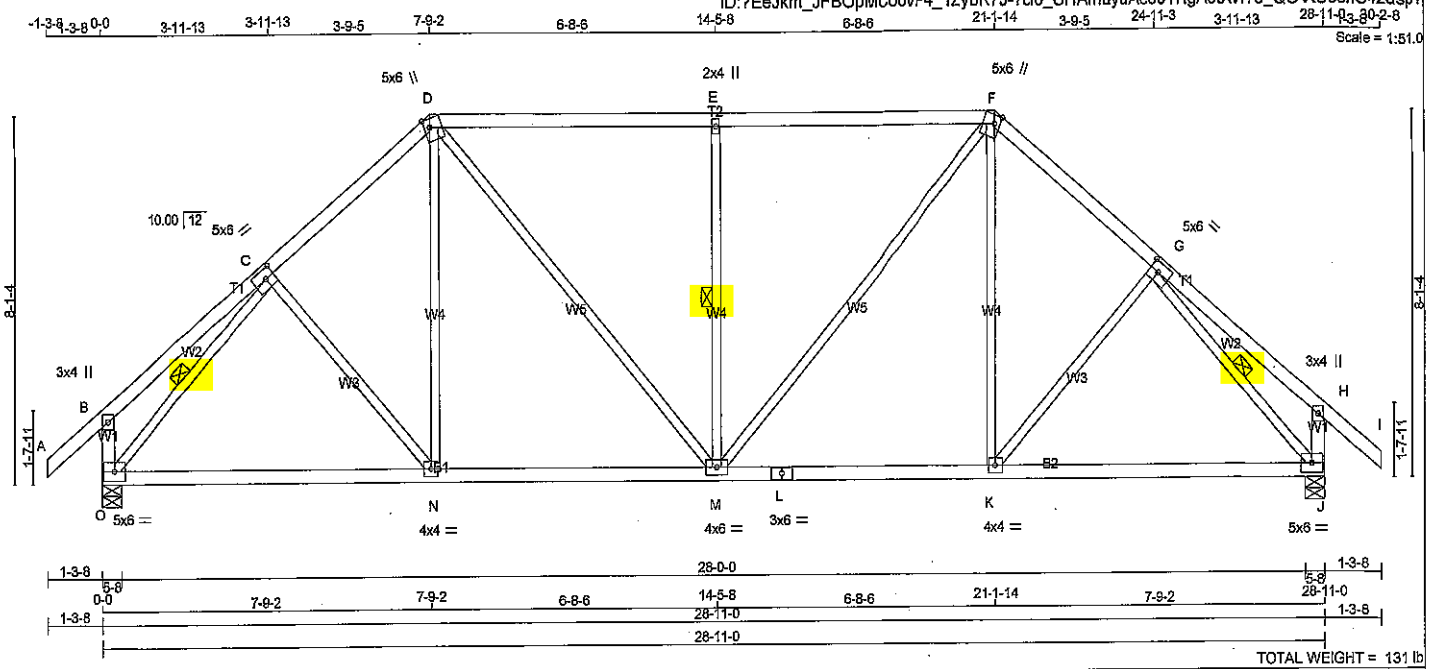
DWG NO. TAM 129618
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T05	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:02 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?cl0_CHAmuyuAco3THgA5tXvf?5_QOVtUoshS4zdsp?



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMWW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 29	-104.9	-104.9	0.24 (1)	10.00	
C- D	-1875 / 0	-104.9	-104.9	0.23 (1)	4.71	
D- E	-1852 / 0	-104.9	-104.9	0.67 (1)	4.04	
E- F	-1852 / 0	-104.9	-104.9	0.67 (1)	4.04	
F- G	-1875 / 0	-104.9	-104.9	0.23 (1)	4.71	
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
O- B	-302 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-302 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 1426	-27.5	-27.5	0.53 (2)	10.00	
N- M	0 / 1417	-27.5	-27.5	0.54 (2)	10.00	
M- L	0 / 1417	-27.5	-27.5	0.54 (2)	10.00	
L- K	0 / 1417	-27.5	-27.5	0.54 (2)	10.00	
K- J	0 / 1426	-27.5	-27.5	0.53 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-17 / 113	0.03 (3)
N- D	0 / 342	0.08 (2)
D- M	0 / 667	0.15 (1)
M- E	-862 / 0	0.38 (1)
M- F	0 / 667	0.15 (1)
K- F	0 / 342	0.08 (2)
K- G	-17 / 113	0.03 (3)
O- C	-2196 / 0	0.55 (1)
G- J	-2196 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.67/1.00 (E-F:1) , BC=0.54/1.00 (K-M:2) , WB=0.55/1.00 (G-J:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

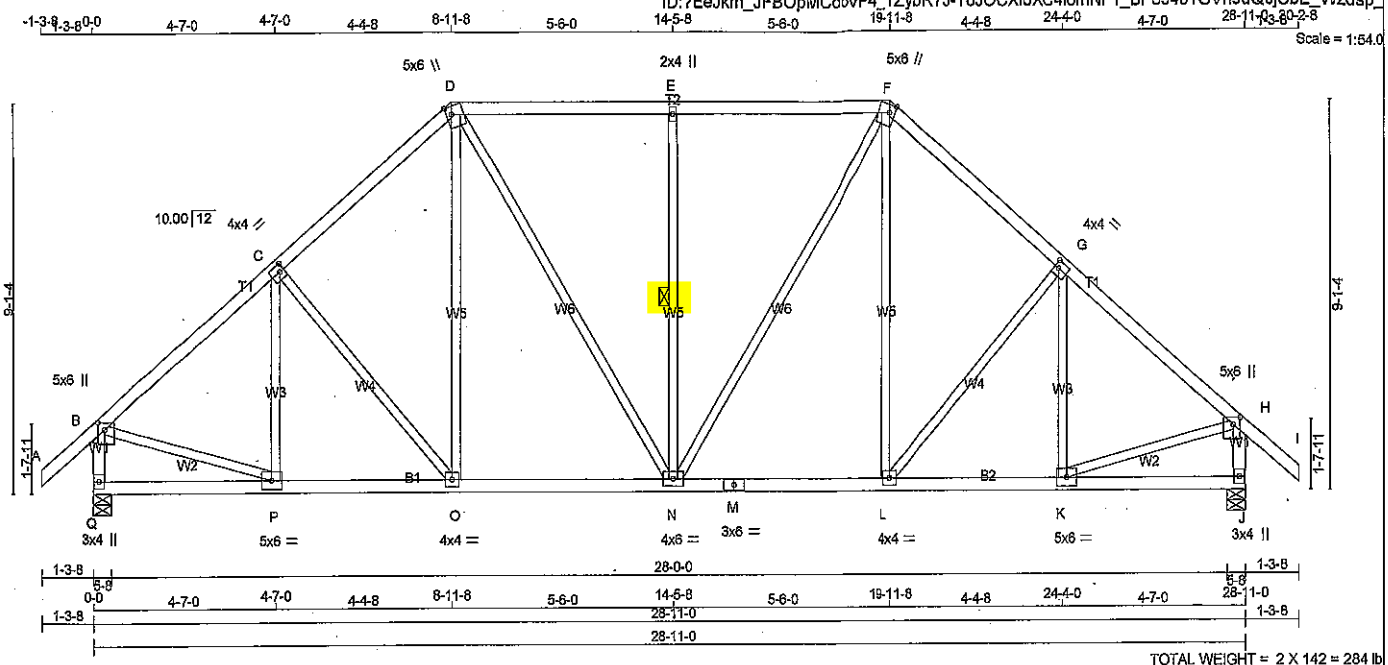
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM V2197-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T06	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:03 2018 Page 1
ID:?EeJkm_JFBOpMCovP4_1ZybR7J-ToJOCXIoXC4lomNF1_BPe5481OVn9uQ0jSbE_Wzdsp_



[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1916 / 0	-104.9	-104.9	0.32 (1)	4.57	
C- D	-1799 / 0	-104.9	-104.9	0.31 (1)	4.69	
D- E	-1621 / 0	-104.9	-104.9	0.44 (1)	4.69	
E- F	-1621 / 0	-104.9	-104.9	0.44 (1)	4.69	
F- G	-1799 / 0	-104.9	-104.9	0.31 (1)	4.69	
G- H	-1916 / 0	-104.9	-104.9	0.32 (1)	4.57	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-2006 / 0	0.0	0.0	0.21 (1)	5.96	
J- H	-2006 / 0	0.0	0.0	0.21 (1)	5.96	
Q- P	0 / 0	-27.5	-27.5	0.13 (3)	10.00	
P- O	0 / 1498	-27.5	-27.5	0.32 (1)	10.00	
O- N	0 / 1354	-27.5	-27.5	0.31 (2)	10.00	
N- M	0 / 1354	-27.5	-27.5	0.31 (2)	10.00	
M- L	0 / 1354	-27.5	-27.5	0.31 (2)	10.00	
L- K	0 / 1498	-27.5	-27.5	0.32 (1)	10.00	
K- J	0 / 0	-27.5	-27.5	0.13 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-295 / 60	0.13 (1)
C- O	-225 / 0	0.19 (1)
O- D	0 / 332	0.07 (2)
D- N	0 / 504	0.11 (1)
N- E	-703 / 0	0.38 (1)
N- F	0 / 504	0.11 (1)
L- F	0 / 332	0.07 (2)
L- G	-225 / 0	0.19 (1)
K- G	-295 / 60	0.13 (1)
B- P	0 / 1558	0.35 (1)
K- H	0 / 1558	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.32/1.00 (O-P:1) , WB=0.38/1.00 (E-N:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/5/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

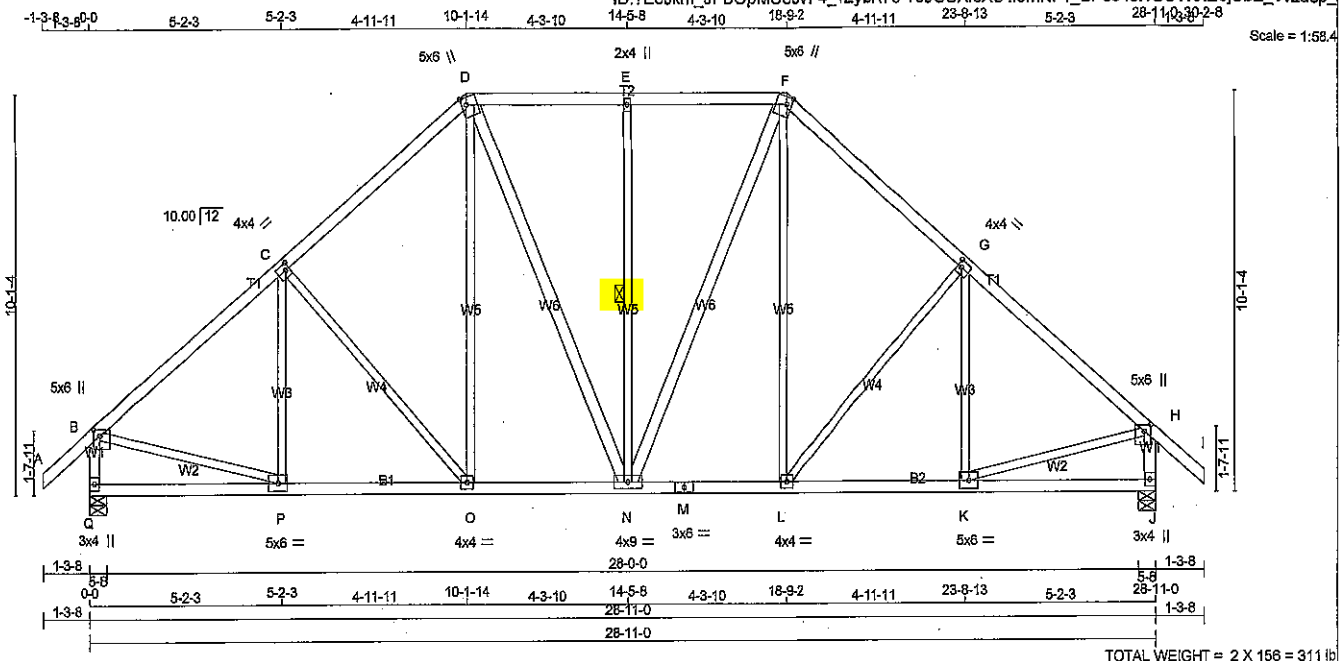
DWG NO. TAM N19617B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T07	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:03 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ToJOCXloXC4lomNF1_BPe548HOUW9tE0jSbE_Wzdsp_



TOTAL WEIGHT = 2 X 156 = 311 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2060	0	2060	0	0	5-8	5-8
J	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47		-104.9	-104.9	0.14 (1)	10.00
B- C	-1936 / 0		-104.9	-104.9	0.42 (1)	4.43
C- D	-1725 / 0		-104.9	-104.9	0.40 (1)	4.66
D- E	-1437 / 0		-104.9	-104.9	0.26 (1)	5.17
E- F	-1437 / 0		-104.9	-104.9	0.26 (1)	5.17
F- G	-1725 / 0		-104.9	-104.9	0.40 (1)	4.66
G- H	-1936 / 0		-104.9	-104.9	0.42 (1)	4.43
H- I	0 / 47		-104.9	-104.9	0.14 (1)	10.00
Q- B	-2001 / 0		0.0	0.0	0.21 (1)	5.97
J- H	-2001 / 0		0.0	0.0	0.21 (1)	5.97
Q- P	0 / 0		-27.5	-27.5	0.18 (3)	10.00
P- O	0 / 1518		-27.5	-27.5	0.33 (2)	10.00
O- N	0 / 1294		-27.5	-27.5	0.27 (1)	10.00
N- M	0 / 1294		-27.5	-27.5	0.27 (1)	10.00
M- L	0 / 1294		-27.5	-27.5	0.27 (1)	10.00
L- K	0 / 1518		-27.5	-27.5	0.33 (2)	10.00
K- J	0 / 0		-27.5	-27.5	0.18 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
P- C	-224 / 121		0.12 (1)
C- O	-340 / 0		0.39 (1)
O- D	0 / 378		0.09 (1)
D- N	0 / 357		0.08 (1)
N- E	-542 / 0		0.38 (1)
N- F	0 / 357		0.08 (1)
L- F	0 / 378		0.09 (1)
L- G	-340 / 0		0.39 (1)
K- G	-224 / 121		0.12 (1)
B- P	0 / 1565		0.35 (1)
K- H	0 / 1565		0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT. CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.33/1.00 (K-L:2) , WB=0.39/1.00 (G-L:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

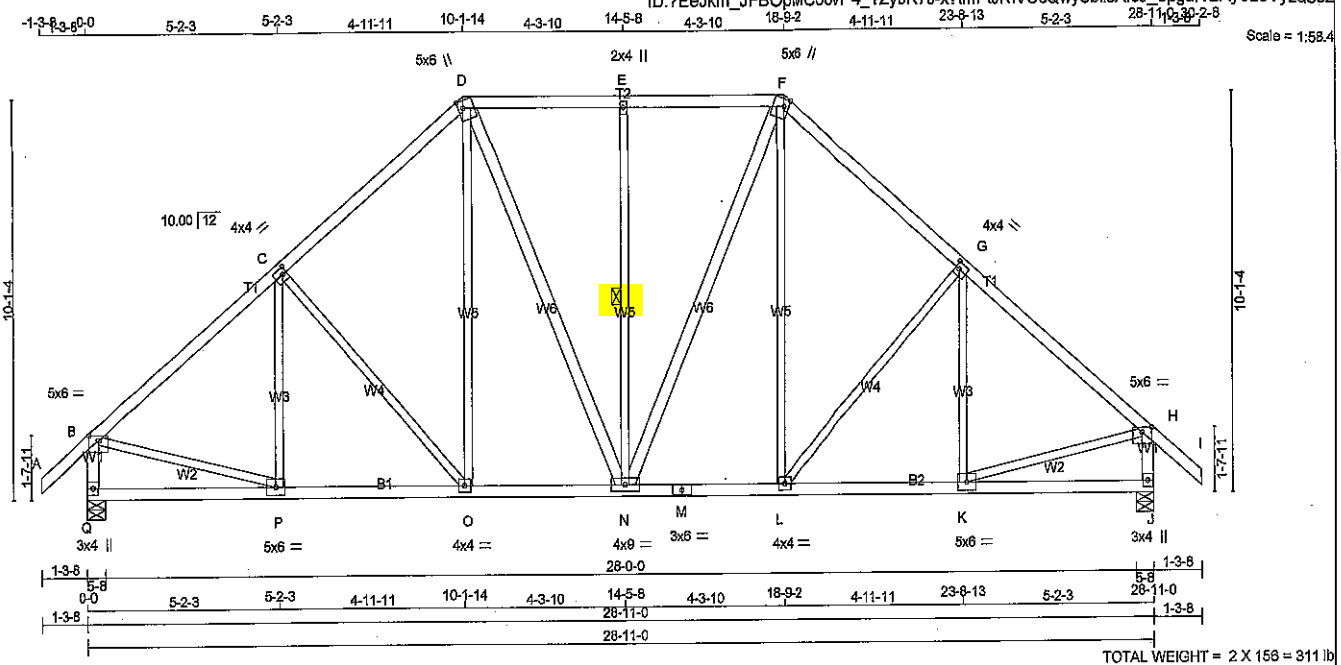
JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO. TAM 12199-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T07A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:04 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-x?tmPtJRIVCcQwySbieAlcJ_opguKGAy6LoVyzdsoz



[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2092	0	2092	0	0	5-8	5-8
J	2092	0	2092	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1642	1029 / 0	289 / 0	0 / 0	0 / 0	323 / 0	0 / 0
J	1642	1029 / 0	289 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1974 / 0	-104.9	-104.9	0.43 (1)	4.39
C- D	-1768 / 0	-104.9	-104.9	0.40 (1)	4.61
D- E	-1479 / 0	-112.4	-112.4	0.28 (1)	2.00
E- F	-1479 / 0	-112.4	-112.4	0.28 (1)	2.00
F- G	-1768 / 0	-104.9	-104.9	0.40 (1)	4.61
G- H	-1974 / 0	-104.9	-104.9	0.43 (1)	4.39
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-2033 / 0	0.0	0.0	0.22 (1)	5.92
J- H	-2033 / 0	0.0	0.0	0.22 (1)	5.92
Q- P	0 / 0	-27.5	-27.5	0.18 (3)	10.00
P- O	0 / 1548	-27.5	-27.5	0.34 (2)	10.00
O- N	0 / 1327	-27.5	-27.5	0.27 (1)	10.00
N- M	0 / 1327	-27.5	-27.5	0.27 (1)	10.00
M- L	0 / 1327	-27.5	-27.5	0.27 (1)	10.00
L- K	0 / 1548	-27.5	-27.5	0.34 (2)	10.00
K- J	0 / 0	-27.5	-27.5	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-232 / 114	0.13 (1)
C- O	-334 / 0	0.38 (1)
O- D	0 / 374	0.08 (1)
D- N	0 / 378	0.08 (1)
N- E	-581 / 0	0.41 (1)
N- F	0 / 378	0.08 (1)
L- F	0 / 374	0.08 (1)
L- G	-334 / 0	0.38 (1)
K- G	-232 / 114	0.13 (1)
B- P	0 / 1596	0.36 (1)
K- H	0 / 1596	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 10.00/12 AND -10.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.34/1.00 (O-P:2) , WB=0.41/1.00 (E-N:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

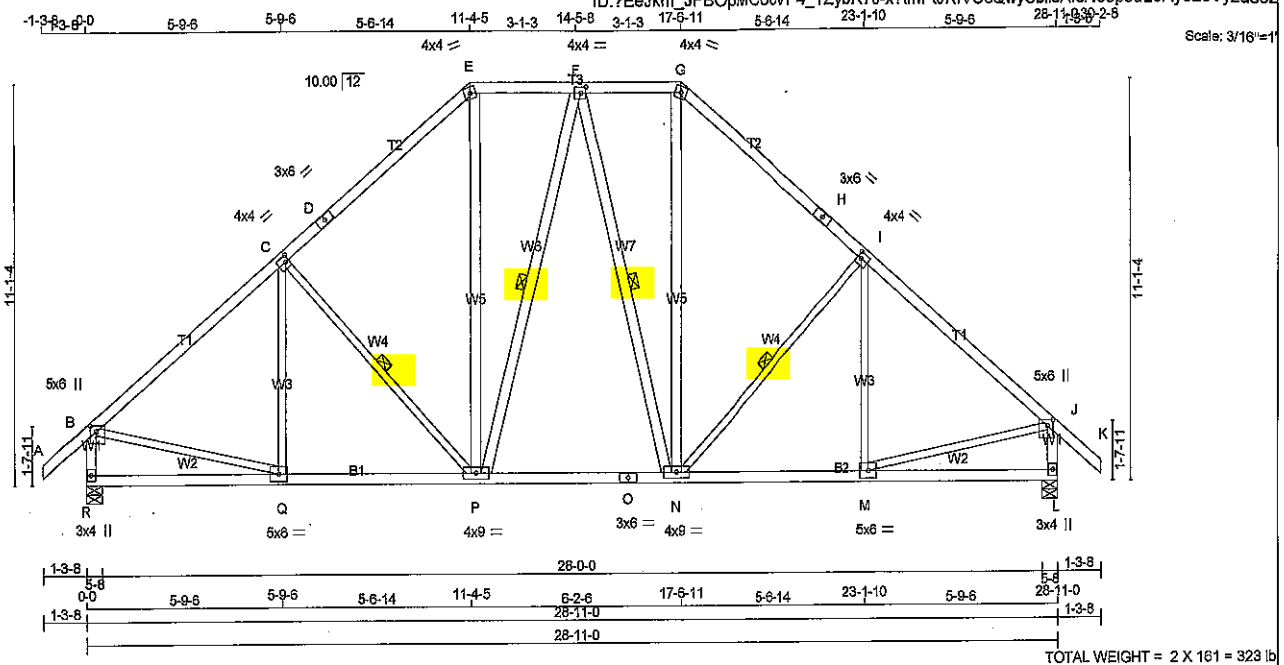
DWG NO. TAM 12200-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T08	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:04 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-x?tmPtJRIVCcQwySbileAlcH8op8uL5Ay6LoVyzdsoz



[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TTW-m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMWW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2060	0	2060	0	0	5-8	5-8
L	2060	0	2060	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1616	1029 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1943 / 0	-104.9	-104.9	0.54 (1)	4.27
C- D	-1645 / 0	-104.9	-104.9	0.50 (1)	4.60
D- E	-1645 / 0	-104.9	-104.9	0.50 (1)	4.60
E- F	-1234 / 0	-104.9	-104.9	0.12 (1)	5.66
F- G	-1234 / 0	-104.9	-104.9	0.12 (1)	5.66
G- H	-1645 / 0	-104.9	-104.9	0.50 (1)	4.60
H- I	-1645 / 0	-104.9	-104.9	0.50 (1)	4.60
I- J	-1943 / 0	-104.9	-104.9	0.54 (1)	4.27
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
R- B	-1994 / 0	0.0	0.0	0.21 (1)	5.97
L- J	-1994 / 0	0.0	0.0	0.21 (1)	5.97
R- Q	0 / 0	-27.5	-27.5	0.22 (3)	10.00
Q- P	0 / 1528	-27.5	-27.5	0.37 (2)	10.00
P- O	0 / 1285	-27.5	-27.5	0.31 (2)	10.00
O- N	0 / 1285	-27.5	-27.5	0.31 (2)	10.00
N- M	0 / 1528	-27.5	-27.5	0.37 (2)	10.00
M- L	0 / 0	-27.5	-27.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-171 / 163	0.12 (1)
C- P	-443 / 0	0.21 (1)
P- E	0 / 667	0.11 (1)
P- F	-192 / 0	0.13 (1)
F- N	-196 / 0	0.14 (1)
N- G	0 / 672	0.11 (1)
N- I	-443 / 0	0.21 (1)
M- I	-171 / 163	0.12 (1)
B- Q	0 / 1586	0.35 (1)
M- J	0 / 1586	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.37/1.00 (P-Q:2) , WB=0.35/1.00 (B-Q:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO
2/5/18

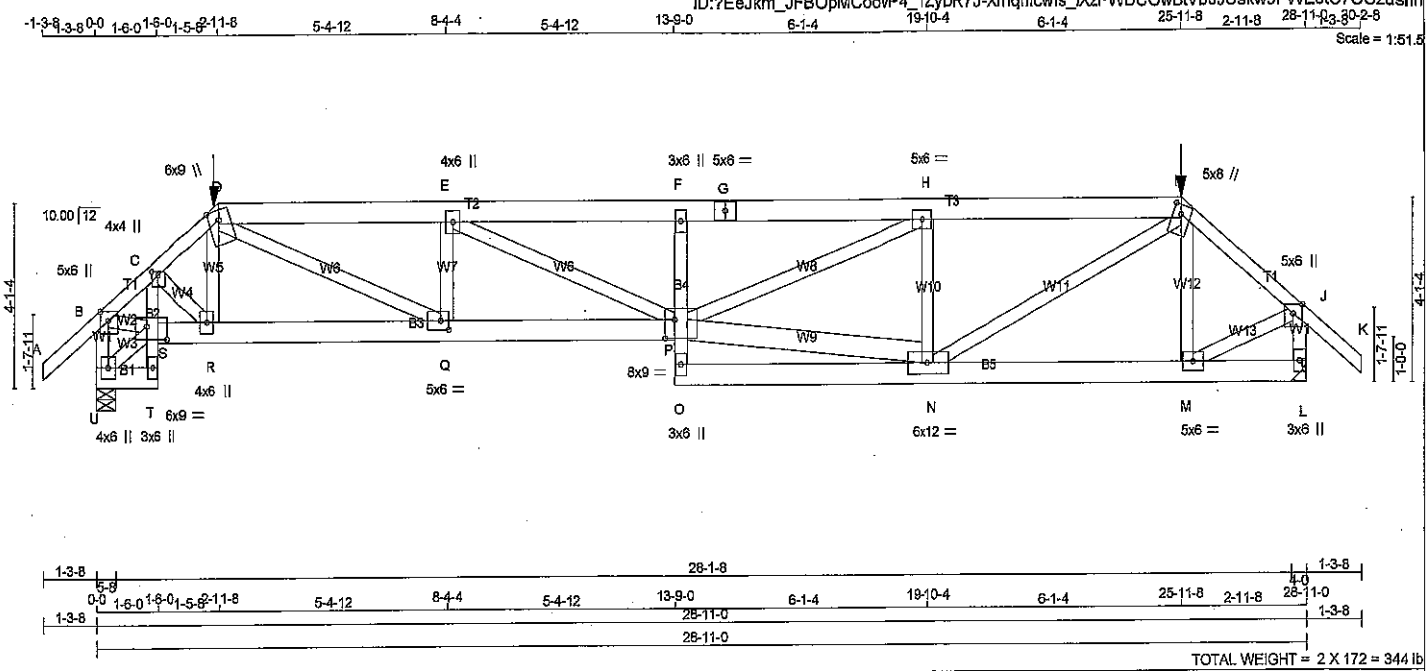
DWG NO. TAM 1220-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288629	T09	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:10:26 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-XmqmcwIs_iXzFWDCOwBtVb0JCskw5PWEotC7OCzdsnh



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x6	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	SIDE(0.0)
I- K 1	12	SIDE(0.0)
U- B 1	12	TOP
L- J 1	12	TOP
D- G 2	12	SIDE(50.8)
G- I 2	12	SIDE(50.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
U- T 2	12	SIDE(23.0)
S- P 2	12	SIDE(23.0)
O- L 2	12	SIDE(23.0)
T- C 1	12	TOP
F- O 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	
E	TMWW+t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	5.0	8.0	2.50	2.25
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	3837	0	3837	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
U	3844	0	3844	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3033	1891 / 0	569 / 0	0 / 0	0 / 0	573 / 0	0 / 0
U	3036	1895 / 0	569 / 0	0 / 0	0 / 0	574 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

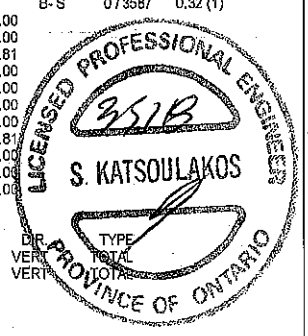
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 47	-104.9	-104.9	0.08 (1)		10.00
B- C	-4727 / 0	-104.9	-104.9	0.14 (1)		4.31
C- D	-4990 / 0	-104.9	-104.9	0.14 (1)		4.22
D- E	-8674 / 0	-206.6	-206.6	0.34 (1)		3.87
E- F	-10433 / 0	-206.6	-206.6	0.38 (1)		3.53
F- G	-10318 / 0	-206.6	-206.6	0.50 (1)		3.42
G- H	-10318 / 0	-206.6	-206.6	0.50 (1)		3.42
H- I	-6834 / 0	-206.6	-206.6	0.38 (1)		4.28
I- J	-3613 / 0	-104.9	-104.9	0.14 (1)		4.83
J- K	0 / 47	-104.9	-104.9	0.08 (1)		10.00
U- B	-3707 / 0	0.0	0.0	0.21 (1)		6.11
L- J	-3810 / 0	0.0	0.0	0.22 (1)		6.04
U- T	0 / 126	-54.1	-54.1	0.01 (2)		10.00
T- S	0 / 63	0.0	0.0	0.10 (1)		10.00
S- C	-381 / 0	0.0	0.0	0.10 (1)		7.81
S- R	0 / 3669	-54.1	-54.1	0.32 (1)		10.00
R- Q	0 / 3879	-54.1	-54.1	0.33 (1)		10.00
Q- P	0 / 8674	-54.1	-54.1	0.65 (1)		10.00
O- P	0 / 235	0.0	0.0	0.24 (1)		10.00
P- F	-1051 / 0	0.0	0.0	0.24 (1)		7.81
O- N	0 / 304	-54.1	-54.1	0.13 (3)		10.00
N- M	0 / 2821	-54.1	-54.1	0.24 (2)		10.00
M- L	0 / 0	-54.1	-54.1	0.08 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- R	0 / 283	0.02 (1)
R- D	0 / 305	0.03 (3)
D- Q	0 / 5348	0.47 (1)
Q- E	-2135 / 0	0.14 (1)
E- P	0 / 1949	0.17 (1)
P- N	0 / 6545	0.43 (1)
P- H	0 / 3890	0.34 (1)
N- H	-3064 / 0	0.26 (1)
N- I	0 / 4478	0.40 (1)
M- I	-914 / 0	0.08 (1)
M- J	0 / 3035	0.27 (1)
U- S	-161 / 0	0.01 (1)
B- S	0 / 3587	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR	TYPE
D	2-11-8	-226	-226	—	FRONT	VERT	TOTAL
I	25-11-8	-226	-226	—	FRONT	VERT	TOTAL



DWG NO. TAM12223-18
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 862 (0.40")

CSI: TC=0.50/1.00 (F-H:1) , BC=0.65/1.00 (P-Q:1) , WB=0.47/1.00 (D-Q:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288629	T09	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWWW-t	MT20	6.0	12.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWWW-I	MT20	8.0	9.0	5.00	2.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWW+t	MT20	4.0	6.0		
S	BVMWW-I	MT20	6.0	9.0	3.50	6.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8, AND 226.3 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.60 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3528
S. KATSOULAKOS
PROVINCE OF ONTARIO

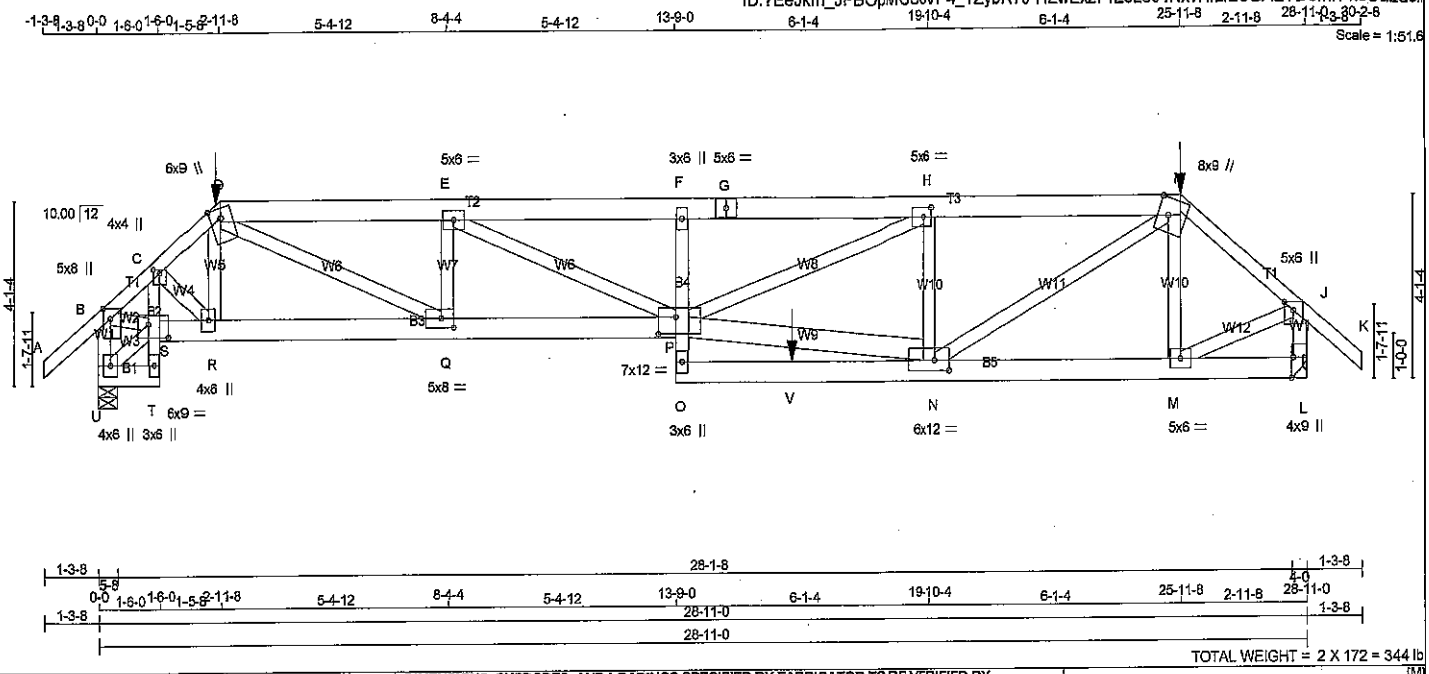
DWG NO. TAM 1222318
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295492	T09Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:15:42 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-HZwExz74ZoL894NxvHrzIE5GAEYD8IhIT1IsUuzdsi



TOTAL WEIGHT = 2 X 172 = 344 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x6	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(0.0)
I- K	1	12	SIDE(0.0)
U- B	1	12	TOP
L- J	1	12	TOP
D- G	2	12	SIDE(50.8)
G- I	2	12	SIDE(50.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- T	2	12	SIDE(23.0)
S- P	2	12	SIDE(23.0)
O- L	2	12	SIDE(23.0)
T- C	1	12	TOP
F- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.75	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TTWW+m	MT20	8.0	9.0	4.50	3.00
J	TMVW+p	MT20	5.0	6.0	2.25	2.50
L	BMV1+t	MT20	4.0	9.0	Edge	0.50
M	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	4725	0	4725	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 4-0	IN-SX
U	4512	0	4512	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3737	2326 / 0	703 / 0	0 / 0	0 / 0	707 / 0	0 / 0
U	3567	2223 / 0	670 / 0	0 / 0	0 / 0	674 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C		-5616 / 0	-104.9	-104.9	0.17 (1)	3.97	
C- D		-5957 / 0	-104.9	-104.9	0.18 (1)	3.86	
D- E		-10760 / 0	-206.6	-206.6	0.41 (1)	3.45	
E- F		-13901 / 0	-206.6	-206.6	0.55 (1)	2.94	
F- G		-13748 / 0	-206.6	-206.6	0.64 (1)	2.84	
G- H		-13748 / 0	-206.6	-206.6	0.64 (1)	2.84	
H- I		-9032 / 0	-206.6	-206.6	0.44 (1)	3.69	
I- J		-4575 / 0	-104.9	-104.9	0.16 (1)	4.36	
J- K		0 / 47	-104.9	-104.9	0.08 (1)	10.00	
U- B		-4357 / 0	0.0	0.0	0.25 (1)	5.70	
L- J		-4602 / 0	0.0	0.0	0.26 (1)	5.57	
U- T		0 / 150	-54.1	-54.1	0.01 (2)	10.00	
T- S		0 / 63	0.0	0.0	0.12 (1)	10.00	
S- C		-463 / 0	0.0	0.0	0.12 (1)	7.81	
S- R		0 / 4386	-54.1	-54.1	0.38 (1)	10.00	
R- Q		0 / 4633	-54.1	-54.1	0.40 (1)	10.00	
Q- P		0 / 10760	-54.1	-54.1	0.60 (1)	10.00	
O- P		0 / 901	0.0	0.0	0.38 (1)	10.00	
P- F		-1021 / 0	0.0	0.0	0.31 (1)	7.81	
O- V		0 / 404	-54.1	-54.1	0.82 (1)	10.00	
V- N		0 / 404	-54.1	-54.1	0.82 (1)	10.00	
N- M		0 / 3472	-54.1	-54.1	0.58 (1)	10.00	
M- L		0 / 0	-54.1	-54.1	0.05 (1)	10.00	

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R		0 / 357	0.03 (1)
R- D		-25 / 286	0.03 (3)
D- Q		0 / 6833	0.60 (1)
Q- E		-2816 / 0	0.18 (1)
E- P		0 / 3480	0.31 (1)
P- N		0 / 8888	0.58 (1)
P- H		0 / 5004	0.44 (1)
N- H		-3533 / 0	0.30 (1)
N- I		0 / 6528	0.58 (1)
M- I		-1370 / 0	0.12 (1)
M- J		0 / 3789	0.33 (1)
U- S		-191 / 0	0.01 (1)
B- S		0 / 4269	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-11-8	-226	-226	---	FRONT	VERT	TOTAL
I	25-11-8	-226	-226	---	FRONT	VERT	TOTAL
V	16-6-0	-1556	-1556	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 871 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 557 (0.62")

CSI: TC=0.64/1.00 (F-H:1) , BC=0.82/1.00 (N-O:1) , WB=0.60/1.00 (D-Q:1) , SSI=0.33/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12236-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295492	T09Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-HZwExz74ZoL894NxvHrzIE5GAEYD8IhIT1lsUuzdsil

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWWW-t	MT20	6.0	12.0	2.75	4.00
O	BMV+p	MT20	3.0	6.0		
P	BVMWWW-I	MT20	7.0	12.0	4.50	5.00
Q	BMWW-t	MT20	5.0	8.0	2.50	3.50
R	BMWW+t	MT20	4.0	6.0		
S	BVMWW-I	MT20	6.0	9.0	3.50	6.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8, AND 226.3 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD, AND 1555.8 lbs FACTORED DOWN AT 16-6-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.74 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2516
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12236-18
STRUCTURAL
COMPONENT ONLY

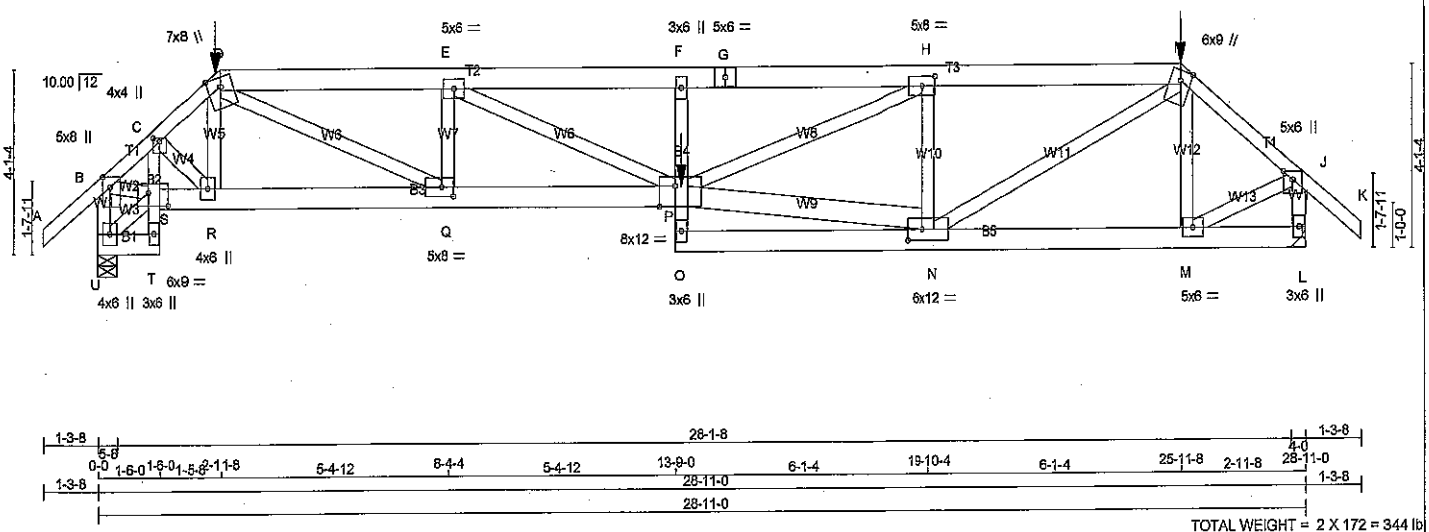
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T09Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Mon Mar 5 21:09:04 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-x?tmPtJRIVCcQwySbIieAlcEdohvuHfAy6LoVyzdsoz

Scale = 1:51.6



TOTAL WEIGHT = 2 X 172 = 344 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x6	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(0.0)
I- K	1	12	SIDE(0.0)
U- B	1	12	TOP
L- J	1	12	TOP
D- G	2	12	SIDE(50.8)
G- I	2	12	SIDE(50.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- T	2	12	SIDE(23.0)
S- P	2	12	SIDE(23.0)
O- L	2	12	SIDE(23.0)
T- C	1	12	TOP
F- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.75	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	7.0	8.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	8.0	2.50	3.75
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMVW+p	MT20	5.0	6.0	2.25	2.50
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4605	0	4605	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
U	4674	0	4674	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3642	2267 / 0	685 / 0	0 / 0	0 / 0	689 / 0	0 / 0
U	3696	2302 / 0	695 / 0	0 / 0	0 / 0	699 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-5832 / 0	-104.9	-104.9	0.16 (1)	3.90	
C- D	-6192 / 0	-104.9	-104.9	0.19 (1)	3.78	
D- E	-11271 / 0	-206.6	-206.6	0.43 (1)	3.36	
E- F	-14729 / 0	-206.6	-206.6	0.60 (1)	2.80	
F- G	-14570 / 0	-206.6	-206.6	0.71 (1)	2.69	
G- H	-14570 / 0	-206.6	-206.6	0.71 (1)	2.69	
H- I	-8511 / 0	-206.6	-206.6	0.44 (1)	3.78	
I- J	-4409 / 0	-104.9	-104.9	0.16 (1)	4.44	
J- K	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
U- B	-4515 / 0	0.0	0.0	0.26 (1)	5.62	
L- J	-4583 / 0	0.0	0.0	0.26 (1)	5.58	
U- T	0 / 155	-54.1	-54.1	0.01 (1)	10.00	
T- S	0 / 63	0.0	0.0	0.13 (1)	10.00	
S- C	-484 / 0	0.0	0.0	0.13 (1)	7.81	
S- R	0 / 4536	-54.1	-54.1	0.39 (1)	10.00	
R- Q	0 / 4817	-54.1	-54.1	0.41 (1)	10.00	
Q- P	0 / 11270	-54.1	-54.1	0.83 (1)	10.00	
O- P	0 / 236	0.0	0.0	0.33 (1)	10.00	
P- F	-990 / 0	0.0	0.0	0.33 (1)	7.81	
O- N	0 / 423	-54.1	-54.1	0.13 (3)	10.00	
N- M	0 / 3444	-54.1	-54.1	0.28 (1)	10.00	
M- L	0 / 0	-54.1	-54.1	0.08 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 376	0.03 (1)
R- D	-35 / 262	0.02 (3)
D- Q	0 / 7198	0.64 (1)
Q- E	-2984 / 0	0.19 (1)
E- P	0 / 3833	0.34 (1)
P- N	0 / 8374	0.54 (1)
P- H	0 / 6437	0.57 (1)
N- H	-4141 / 0	0.35 (1)
N- I	0 / 5951	0.53 (1)
M- I	-1170 / 0	0.10 (1)
M- J	0 / 3704	0.33 (1)
U- S	-198 / 0	0.01 (1)
B- S	0 / 4435	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	2-11-8	-226	-226	---	FRONT	VERT	TOTAL
I	25-11-8	-226	-226	---	FRONT	VERT	TOTAL
P	13-10-12	-1599	-1599	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 122028

STRUCTURAL COMPONENT ONLY

SITE COPY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 967 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 623 (0.56")

CSI: TC=0.71/1.00 (F-H:1) , BC=0.83/1.00 (P-Q:1) , WB=0.64/1.00 (D-Q:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T09Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWWW-t	MT20	6.0	12.0	3.00	4.50
O	BMV+p	MT20	3.0	6.0		
P	BVMWWW-I	MT20	8.0	12.0	Edge	4.50
Q	BMWW-t	MT20	5.0	8.0	2.50	3.25
R	BMWW+t	MT20	4.0	6.0		
S	BVMWW-I	MT20	6.0	9.0	3.50	6.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8, AND 226.3 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD, AND 1598.7 lbs FACTORED DOWN AT 13-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.68 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

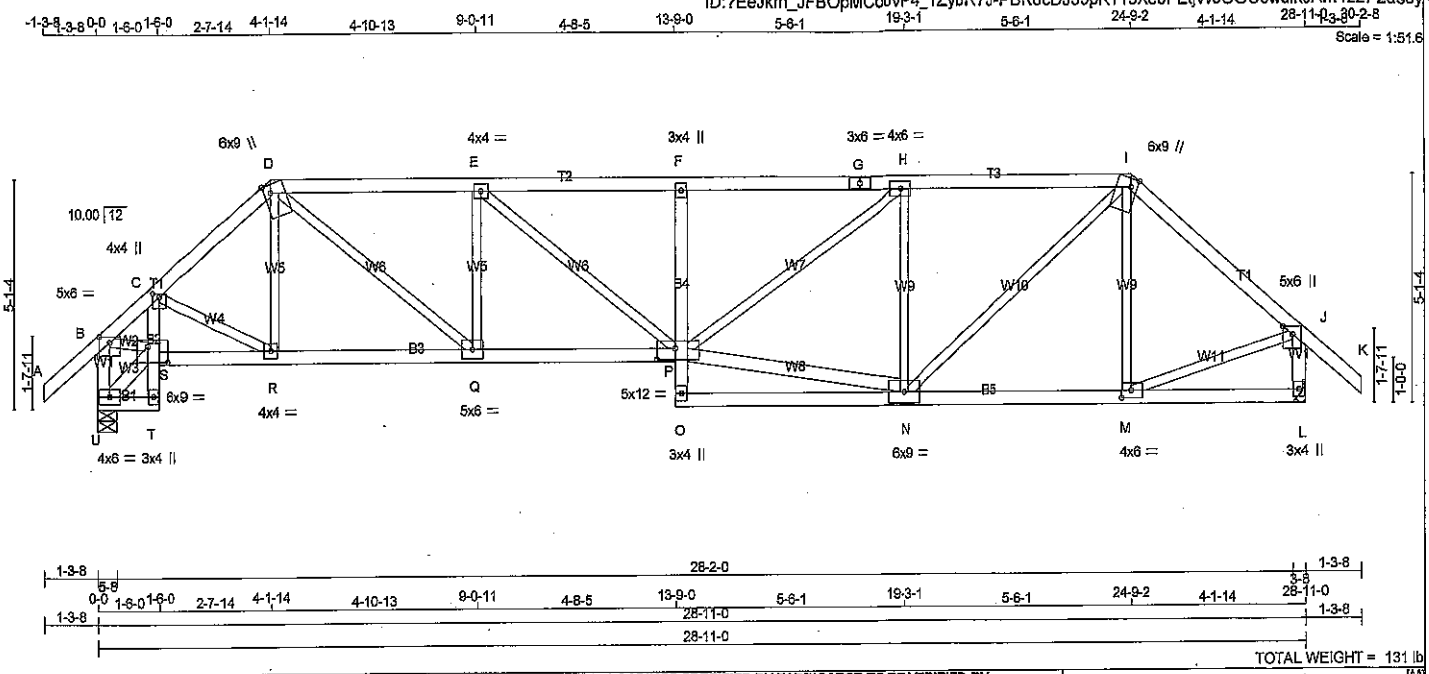
DWG NO. TAM 122028
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:05 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-PBR8cDJ33pKT13Xe8PEtjW9OOC5wdlkJAm4L2Pzdsoy



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	5.0	6.0	Edge	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.75
N	BMWWW-t	MT20	6.0	9.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	5.0	12.0	3.00	5.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BVMWW-I	MT20	6.0	9.0	4.25	6.00
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2064	0	2064	0	0	5-8	5-8
L	2057	0	2057	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1818	1031 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1613	1027 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-2490 / 0	-104.9	-104.9	0.20 (1)	4.18
C- D	-2339 / 0	-104.9	-104.9	0.20 (1)	4.31
D- E	-3181 / 0	-104.9	-104.9	0.54 (1)	3.44
E- F	-3655 / 0	-104.9	-104.9	0.59 (1)	3.17
F- G	-3627 / 0	-104.9	-104.9	0.77 (1)	2.98
G- H	-3627 / 0	-104.9	-104.9	0.77 (1)	2.98
H- I	-2620 / 0	-104.9	-104.9	0.64 (1)	3.61
I- J	-1880 / 0	-104.9	-104.9	0.40 (1)	4.48
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
U- B	-1985 / 0	0.0	0.0	0.21 (1)	5.96
L- J	-2012 / 0	0.0	0.0	0.21 (1)	5.95
U- T	0 / 67	-27.5	-27.5	0.02 (2)	10.00
T- S	0 / 32	0.0	0.0	0.11 (1)	10.00
S- C	-15 / 54	0.0	0.0	0.11 (1)	7.81
S- R	0 / 1950	-27.5	-27.5	0.38 (1)	10.00
R- Q	0 / 1790	-27.5	-27.5	0.35 (1)	10.00
Q- P	0 / 3182	-27.5	-27.5	0.59 (1)	10.00
O- P	0 / 104	0.0	0.0	0.16 (1)	10.00
P- F	-489 / 0	0.0	0.0	0.17 (1)	7.81
O- N	0 / 107	-27.5	-27.5	0.20 (3)	10.00
N- M	0 / 1434	-27.5	-27.5	0.34 (2)	10.00
M- L	0 / 0	-27.5	-27.5	0.14 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- R	-176 / 0	0.04 (1)
R- D	0 / 167	0.04 (3)
D- Q	0 / 1760	0.40 (1)
Q- E	-935 / 0	0.24 (1)
E- P	0 / 603	0.14 (1)
P- N	0 / 2577	0.41 (1)
P- H	0 / 1209	0.27 (1)
N- H	-1350 / 0	0.52 (1)
N- I	0 / 1582	0.36 (1)
M- I	-298 / 55	0.11 (1)
M- J	0 / 1506	0.34 (1)
U- S	-90 / 0	0.01 (1)
B- S	0 / 1885	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.77/1.00 (F-H:1) , BC=0.59/1.00 (P-Q:1) , WB=0.52/1.00 (H-N:1) , SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.54 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

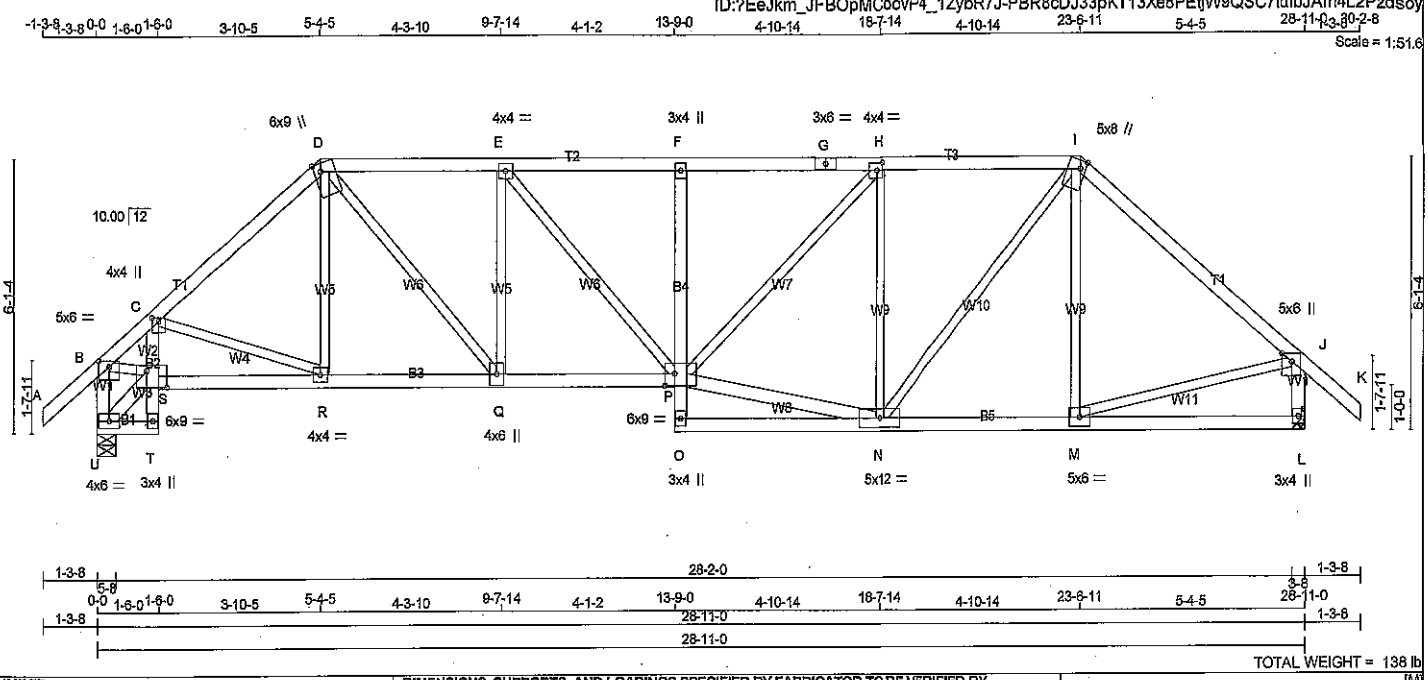
DWG NO. TAM 12203-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:05 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-PBR8cDJ33pKT13Xe8PEtjW9QSC7idibJAm4L2Pzdsoy



TOTAL WEIGHT = 138 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	6.0	8.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	5.0	8.0	2.25	1.50
J	TMVW+p	MT20	5.0	6.0	Edge	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	12.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	6.0	9.0	3.25	2.75
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BVMWW-I	MT20	6.0	9.0	4.50	6.00
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2064	0	2064	0	0	5-8	5-8
L	2057	0	2057	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1618	1031 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1613	1027 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2542 / 0	-104.9	-104.9	0.21 (1)	4.14	
C- D	-2283 / 0	-104.9	-104.9	0.33 (1)	4.24	
D- E	-2591 / 0	-104.9	-104.9	0.37 (1)	3.95	
E- F	-2888 / 0	-104.9	-104.9	0.39 (1)	3.74	
F- G	-2874 / 0	-104.9	-104.9	0.52 (1)	3.60	
G- H	-2874 / 0	-104.9	-104.9	0.52 (1)	3.60	
H- I	-2228 / 0	-104.9	-104.9	0.46 (1)	4.08	
I- J	-1901 / 0	-104.9	-104.9	0.70 (1)	4.08	
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
U- B	-1984 / 0	0.0	0.0	0.21 (1)	5.98	
L- J	-1995 / 0	0.0	0.0	0.21 (1)	5.97	
U- T	0 / 68	-27.5	-27.5	0.02 (2)	10.00	
T- S	0 / 32	0.0	0.0	0.12 (1)	10.00	
S- C	-37 / 69	0.0	0.0	0.11 (1)	7.81	
S- R	0 / 2022	-27.5	-27.5	0.39 (1)	10.00	
R- Q	0 / 1739	-27.5	-27.5	0.34 (1)	10.00	
Q- P	0 / 2592	-27.5	-27.5	0.46 (1)	10.00	
O- P	0 / 92	0.0	0.0	0.10 (1)	10.00	
P- F	-432 / 0	0.0	0.0	0.14 (1)	7.81	
O- N	0 / 67	-27.5	-27.5	0.14 (3)	10.00	
N- M	0 / 1454	-27.5	-27.5	0.33 (2)	10.00	
M- L	0 / 0	-27.5	-27.5	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-301 / 0	0.09 (1)
R- D	0 / 208	0.05 (2)
D- Q	0 / 1276	0.29 (1)
Q- E	-828 / 0	0.32 (1)
E- P	0 / 449	0.10 (1)
P- N	0 / 2209	0.36 (1)
P- H	0 / 919	0.21 (1)
N- H	-1290 / 0	0.72 (1)
N- I	0 / 1197	0.27 (1)
M- I	-189 / 126	0.11 (1)
M- J	0 / 1499	0.34 (1)
U- S	-91 / 0	0.01 (1)
B- S	0 / 1958	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.70/1.00 (I-J:1) , BC=0.48/1.00 (P-Q:1) , WB=0.72/1.00 (H-N:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

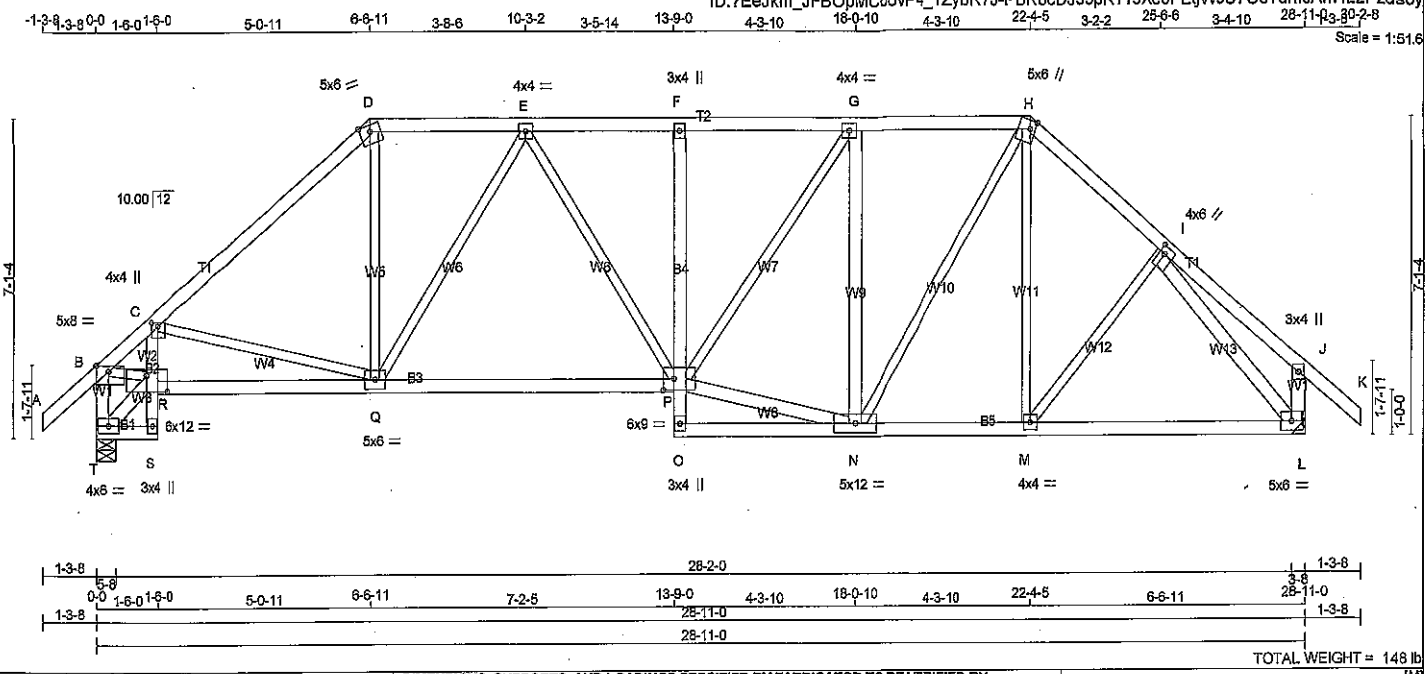
DWG NO. TAM 17204-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:05 2018 Page 1

ID:?EeJkm_JFBOpMCcovP4_1ZybR7J-PBR8cDJ33pKT13Xe8PEtjW9S7C6TdhfJAm4L2Pzdsoy



TOTAL WEIGHT = 148 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.50
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW+t	MT20	4.0	6.0	2.00	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	12.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-l	MT20	6.0	9.0	3.00	3.00
Q	BMWWW-t	MT20	5.0	6.0		
R	BVMWW-l	MT20	6.0	12.0	4.25	6.25
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2064	0	2064	0	0	5-8	5-8
L	2057	0	2057	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1618	1031 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1613	1027 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	0 / 47		-104.9	-104.9	0.14 (1)	10.00	
B- C	-2611 / 0		-104.9	-104.9	0.38 (1)	3.86	
C- D	-2223 / 0		-104.9	-104.9	0.53 (1)	4.10	
D- E	-1712 / 0		-104.9	-104.9	0.23 (1)	4.85	
E- F	-2365 / 0		-104.9	-104.9	0.26 (1)	4.20	
F- G	-2378 / 0		-104.9	-104.9	0.36 (1)	4.09	
G- H	-1934 / 0		-104.9	-104.9	0.34 (1)	4.49	
H- I	-1906 / 0		-104.9	-104.9	0.22 (1)	4.66	
I- J	0 / 24		-104.9	-104.9	0.17 (1)	10.00	
J- K	0 / 47		-104.9	-104.9	0.14 (1)	10.00	
T- B	-1982 / 0		0.0	0.0	0.21 (1)	5.99	
L- J	-278 / 0		0.0	0.0	0.03 (1)	7.81	
T- S	0 / 70		-27.5	-27.5	0.02 (2)	10.00	
S- R	0 / 32		0.0	0.0	0.12 (1)	10.00	
R- C	-62 / 83		0.0	0.0	0.12 (1)	7.81	
R- Q	0 / 2120		-27.5	-27.5	0.55 (2)	10.00	
Q- P	0 / 2174		-27.5	-27.5	0.56 (2)	10.00	
O- P	0 / 86		0.0	0.0	0.07 (1)	10.00	
P- F	-373 / 0		0.0	0.0	0.16 (1)	7.81	
O- N	0 / 44		-27.5	-27.5	0.11 (2)	10.00	
N- M	0 / 1443		-27.5	-27.5	0.41 (2)	10.00	
M- L	0 / 1378		-27.5	-27.5	0.39 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX. CSI
C- Q	-452 / 0		0.23 (1)
Q- D	0 / 1012		0.23 (1)
Q- E	-889 / 0		0.78 (1)
E- P	0 / 398		0.09 (1)
N- G	-1125 / 0		0.70 (1)
N- H	0 / 917		0.21 (1)
M- H	0 / 211		0.05 (3)
M- I	0 / 146		0.03 (2)
I- L	-2190 / 0		0.67 (1)
T- R	-94 / 0		0.01 (1)
B- R	0 / 2058		0.46 (1)
P- N	0 / 1944		0.31 (1)
P- G	0 / 762		0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.56/1.00 (P-Q:2) , WB=0.78/1.00 (E-Q:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.74 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

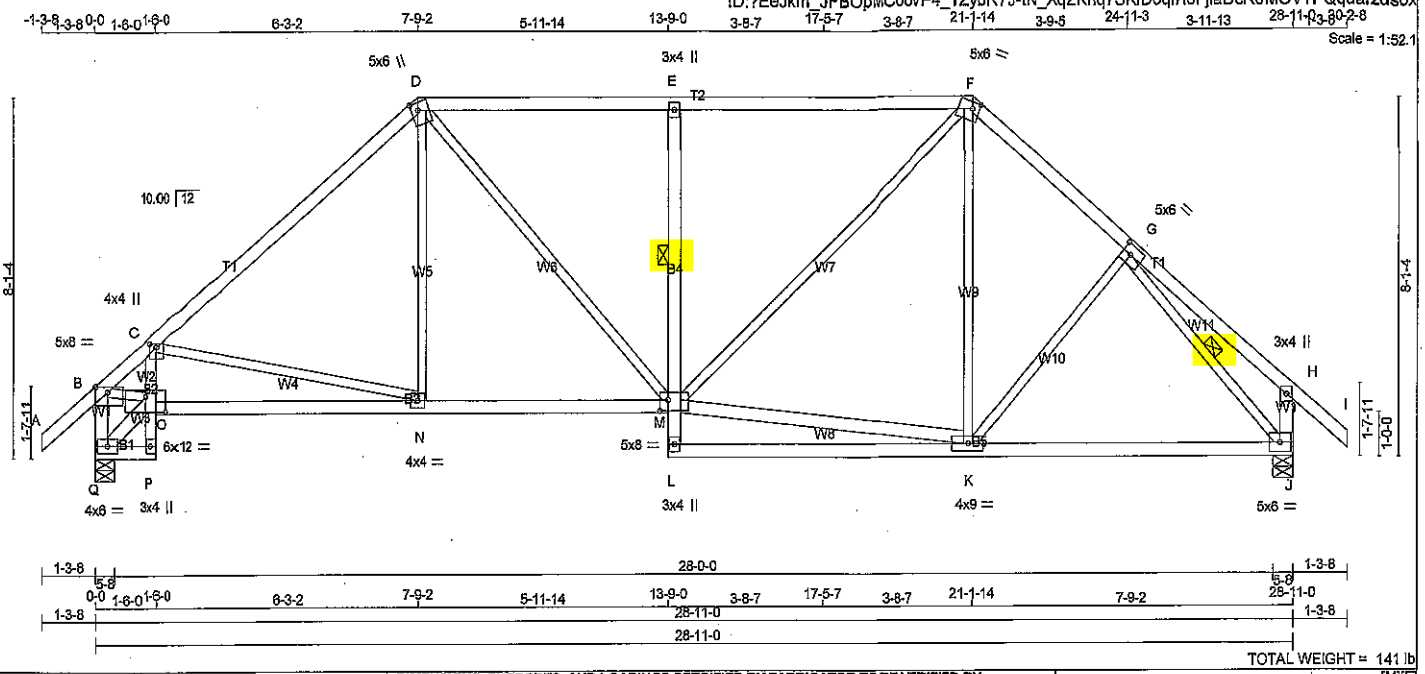
DWG NO. TAM12295-T12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:06 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-tN_XqZKhq7SKfD6qi7i6FjiaDcR6MCVTPQquarzdsox



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	2.00
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	5.0	8.0	3.00	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BVMWW-I	MT20	6.0	12.0	4.25	6.25
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2064	0	2064	0	0	5-8	5-8
J	2057	0	2057	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1618	1031 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
J	1613	1027 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	-661 / 0	0.55 (1)
B- C	-2704 / 0	-104.9	-104.9	0.48 (1)	3.70	N- D	0 / 399	0.09 (2)
C- D	-2133 / 0	-104.9	-104.9	0.67 (1)	3.96	D- M	0 / 740	0.17 (1)
D- E	-2107 / 0	-104.9	-104.9	0.69 (1)	3.77	M- K	0 / 1389	0.22 (1)
E- F	-2101 / 0	-104.9	-104.9	0.76 (1)	3.67	M- F	0 / 938	0.21 (1)
F- G	-1676 / 0	-104.9	-104.9	0.23 (1)	4.70	K- F	0 / 275	0.06 (3)
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	K- G	-17 / 115	0.03 (3)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G- J	-2198 / 0	0.55 (1)
Q- B	-1980 / 0	0.0	0.0	0.21 (1)	5.99	Q- O	-97 / 0	0.01 (1)
J- H	-302 / 0	0.0	0.0	0.03 (1)	7.81	B- O	0 / 2186	0.49 (1)
Q- P	0 / 73	-27.5	-27.5	0.02 (2)	10.00			
P- O	0 / 32	0.0	0.0	0.12 (1)	10.00			
O- C	-63 / 94	0.0	0.0	0.09 (1)	7.81			
O- N	0 / 2253	-27.5	-27.5	0.54 (1)	10.00			
N- M	0 / 1611	-27.5	-27.5	0.49 (2)	10.00			
L- M	0 / 126	0.0	0.0	0.07 (1)	10.00			
M- E	-865 / 0	0.0	0.0	0.10 (1)	6.25			
L- K	0 / 40	-27.5	-27.5	0.46 (3)	10.00			
K- J	0 / 1427	-27.5	-27.5	0.60 (2)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.76/1.00 (E-F:1) , BC=0.60/1.00 (J-K:2) , WB=0.55/1.00 (G-J:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.54 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3516 S. KATSOULAKOS PROVINCE OF ONTARIO

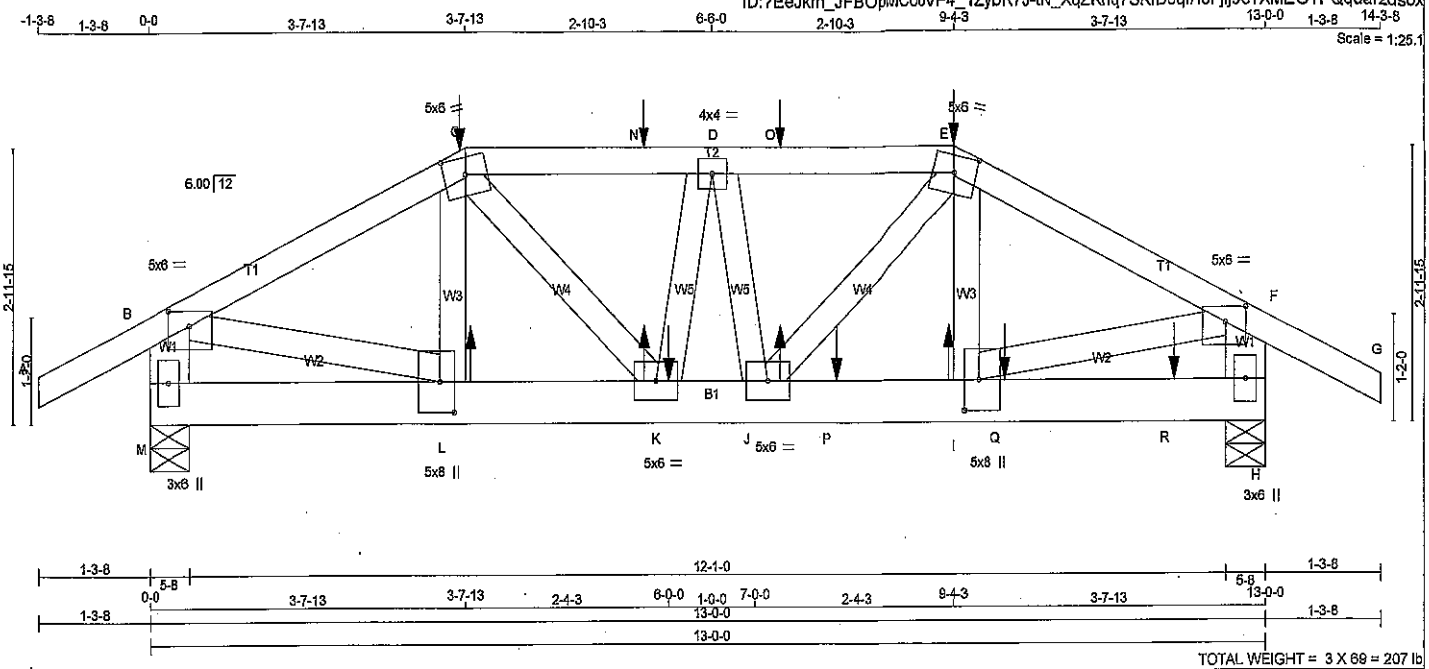
DWG NO. TAM 12206-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T14	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:06 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-tN_XqZKhq7SKfD6qi7l6Fjij9cTXMEOTPQquarzdsox



TOTAL WEIGHT = 3 X 69 = 207 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(4.7)
C- E	1	12	SIDE(4.4)
E- G	1	12	SIDE(0.6)
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- H	2	4	SIDE(529.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	6.0	2.25	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	3.00
F	TMVW-p	MT20	5.0	6.0	2.00	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.00	2.00
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	8.0	4.00	2.00
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	4659	0	4659	0	0	5-8	5-8
H	7462	0	7462	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	3645	2338 / 0	642 / 0	0 / 0	0 / 0	655 / 0	0 / 0
H	5823	3761 / 0	1008 / 0	0 / 0	0 / 0	1053 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 32	-104.9	-104.9	0.05 (1)	10.00	L- C	-980 / 0	0.04 (1)	
B- C	-6519 / 0	-104.9	-104.9	0.14 (1)	4.52	C- K	0 / 4643	0.25 (1)	
C- N	-8957 / 0	-104.9	-104.9	0.15 (1)	3.94	K- D	-290 / 0	0.01 (1)	
N- D	-8957 / 0	-104.9	-104.9	0.15 (1)	3.94	D- J	0 / 42	0.00 (2)	
D- O	-9034 / 0	-104.9	-104.9	0.14 (1)	3.93	J- E	0 / 1984	0.11 (1)	
O- E	-9034 / 0	-104.9	-104.9	0.14 (1)	3.93	I- E	0 / 2055	0.11 (1)	
E- F	-8766 / 0	-104.9	-104.9	0.19 (1)	3.95	B- L	0 / 5965	0.32 (1)	
F- G	0 / 32	-104.9	-104.9	0.05 (1)	10.00	I- F	0 / 8022	0.43 (1)	
M- B	-4547 / 0	0.0	0.0	0.10 (1)	7.81				
H- F	-5999 / 0	0.0	0.0	0.13 (1)	7.14				
M- L	0 / 0	-59.5	-59.5	0.02 (1)	10.00				
L- K	0 / 5878	-59.2	-59.2	0.33 (1)	10.00				
K- J	0 / 9028	-27.5	-27.5	0.46 (1)	10.00				
J- P	0 / 7718	-27.5	-27.5	0.50 (1)	10.00				
P- I	0 / 7718	-27.5	-27.5	0.50 (1)	10.00				
I- Q	0 / 0	-27.8	-27.8	0.34 (1)	10.00				
Q- R	0 / 0	-27.8	-27.8	0.34 (1)	10.00				
R- H	0 / 0	-27.8	-27.8	0.34 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	3-7-13	-35	-35	—	BACK	VERT	TOTAL
E	9-4-3	-0	-1	—	BACK	VERT	TOTAL
E	9-4-3	-35	-35	—	FRONT	VERT	TOTAL
I	9-3-7	11	1	11	BACK	VERT	TOTAL
J	7-3-7	11	1	11	BACK	VERT	TOTAL
K	5-8-9	11	1	11	BACK	VERT	TOTAL
K	6-0-0	-3810	-3810	—	FRONT	VERT	TOTAL
L	3-8-9	11	1	11	BACK	VERT	TOTAL
N	5-8-9	-0	-1	—	BACK	VERT	TOTAL
O	7-3-7	-0	-1	—	BACK	VERT	TOTAL
P	7-11-4	-2029	-2029	—	FRONT	VERT	TOTAL
Q	9-11-4	-2029	-2029	—	FRONT	VERT	TOTAL
R	11-11-4	-2029	-2029	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-7-13
RIGHT SETBACK = 0-0
END SETBACK = 2-0-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: PARTIAL
APPLIED TO BACK SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-7-13 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 3-7-13
END SETBACK = 2-0-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: JACK
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-7-13 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 2-11-8
END DISTANCE = 8-0-0
END SPAN CARRIED = 2-11-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.19/1.00 (E-F:1) , BC=0.50/1.00 (I-J:1) , WB=0.43/1.00 (F-I:1) , SSI=0.54/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 3/5/18 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 112207-18 STRUCTURAL COMPONENT ONLY

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 35.0 lbs FACTORED DOWN AT 3-7-13, 1.3 lbs FACTORED DOWN AT 5-8-9, 1.3 lbs FACTORED DOWN AT 7-3-7, AND 1.3 lbs FACTORED DOWN AT 9-4-3, AND 34.8 lbs FACTORED DOWN AT 9-4-3 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 10.8 lbs FACTORED UP AT 3-8-9, 1.0 lbs FACTORED DOWN AND 10.8 lbs FACTORED UP AT 5-8-9, 3809.6 lbs FACTORED DOWN AT 6-0-0, 1.0 lbs FACTORED DOWN AND 10.8 lbs FACTORED UP AT 7-3-7, 2029.1 lbs FACTORED DOWN AT 7-11-4, 1.0 lbs FACTORED DOWN AND 10.8 lbs FACTORED UP AT 9-3-7, AND 2029.1 lbs FACTORED DOWN AT 9-11-4, AND 2029.1 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

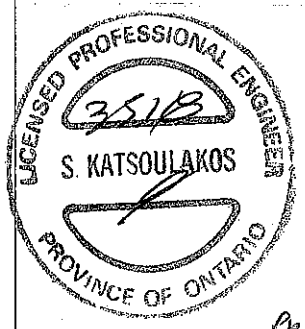
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.67 (L) (INPUT = 1.00)

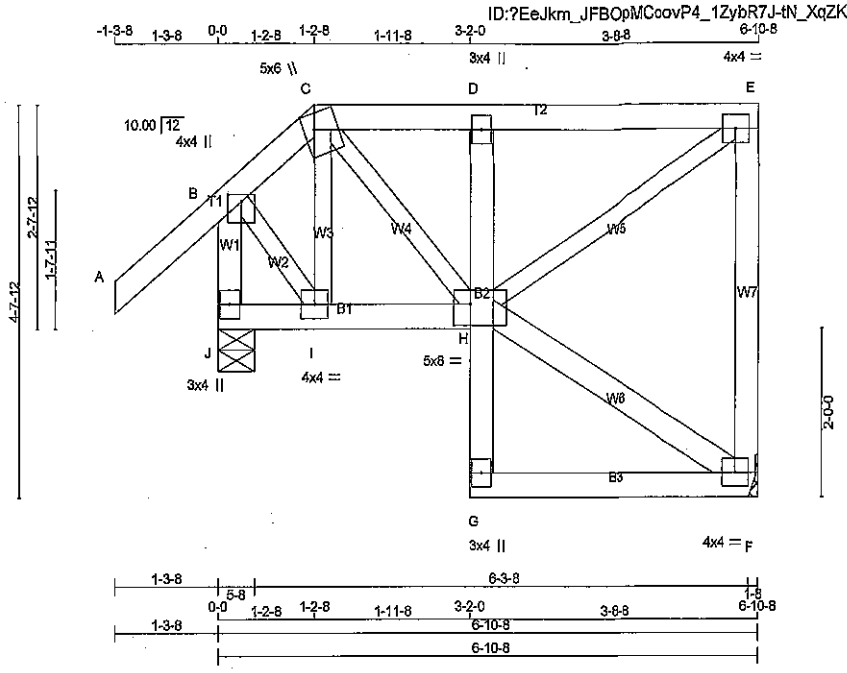


DWG NO. TAM 22207-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:06 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-tN_XqZKhq7SKfD8qi7l6FjijecZoMJcTPQquarzdsox



TOTAL WEIGHT = 41 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTVW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	441	0	441	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
J	615	0	615	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	351	214 / 0	69 / 0	0 / 0	0 / 0	68 / 0	0 / 0
J	469	322 / 0	69 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.61 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	I- C	-111 / 12	0.02 (1)
B- C	-276 / 0	-104.9	-104.9	0.14 (1)	6.25	C- H	0 / 288	0.06 (1)
C- D	-357 / 0	-104.9	-104.9	0.16 (1)	6.25	H- F	-6 / 0	0.00 (1)
D- E	-353 / 0	-104.9	-104.9	0.16 (1)	6.25	H- E	0 / 423	0.10 (1)
F- E	-388 / 0	0.0	0.0	0.14 (1)	7.81	B- I	0 / 239	0.05 (1)
J- B	-604 / 0	0.0	0.0	0.06 (1)	7.81			
J- I	0 / 0	-27.5	-27.5	0.02 (3)	10.00			
I- H	0 / 164	-27.5	-27.5	0.04 (2)	10.00			
G- H	0 / 85	0.0	0.0	0.02 (1)	10.00			
H- D	-379 / 0	0.0	0.0	0.02 (1)	7.81			
G- F	0 / 6	-27.5	-27.5	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (D-E:1) , BC=0.10/1.00 (F-G:3) , WB=0.10/1.00 (E-H:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (H) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1120G1B
STRUCTURAL COMPONENT ONLY

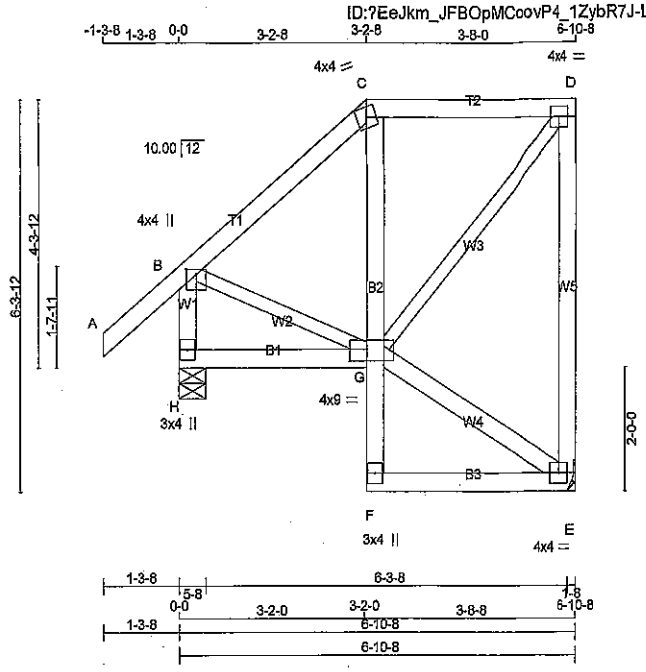
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:07 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-LZYv1vLJbQaBHNh1GqGLoxErr0v35nMce4ZS6Hzdsow

Scale = 1:37.3



TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTV-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	4.0	9.0	2.25	3.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	455	0	455	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	601	0	601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	361	223 / 0	69 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	459	313 / 0	69 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G- E	-3 / 0	0.00 (1)
B- C	-256 / 0	-104.9	-104.9	0.19 (1)	6.25	G- D	0 / 284	0.06 (1)
C- D	-187 / 0	-104.9	-104.9	0.24 (1)	6.25	B- G	0 / 210	0.05 (1)
E- D	-405 / 0	0.0	0.0	0.32 (1)	7.81			
H- B	-554 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-27.5	-27.5	0.09 (3)	10.00			
F- G	0 / 84	0.0	0.0	0.02 (2)	10.00			
G- C	-197 / 40	0.0	0.0	0.01 (1)	7.81			
F- E	0 / 3	-27.5	-27.5	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.10/1.00 (E-F:3) , WB=0.06/1.00 (D-G:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (G) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00) .

DWG NO. TAM 12209-1B
STRUCTURAL
COMPONENT ONLY

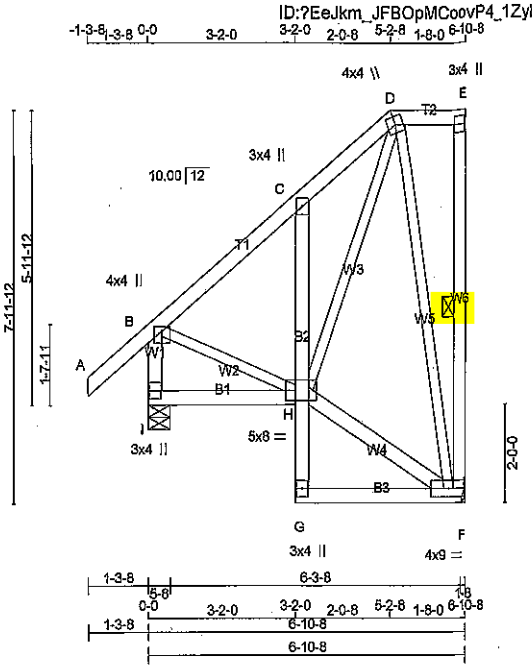
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 8 2018 MiTek Industries, Inc. Mon Mar 5 21:09:07 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-LZYv1vLJbQaBHNh1GqGLoxEud0v15g7ce4ZS6Hzdsow

Scale = 1:47.0



TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1-t	MT20	4.0	9.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	5.0	8.0	2.50	2.50
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	455	0	455	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	601	0	601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	361	223 / 0	69 / 0	0 / 0	0 / 0	69 / 0	0 / 0
I	459	313 / 0	69 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-267 / 0	-104.9	-104.9	0.12 (1)	6.25	
C- D	-299 / 0	-104.9	-104.9	0.10 (1)	6.25	
D- E	0 / 0	-104.9	-104.9	0.05 (1)	10.00	
F- E	-85 / 0	0.0	0.0	0.02 (1)	6.25	
I- B	-555 / 0	0.0	0.0	0.06 (1)	7.81	
I- H	0 / 0	-27.5	-27.5	0.09 (3)	10.00	
G- H	0 / 85	0.0	0.0	0.03 (1)	10.00	
H- C	-339 / 0	0.0	0.0	0.03 (1)	7.81	
G- F	0 / 7	-27.5	-27.5	0.10 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 235	0.05 (1)
D- F	-368 / 0	0.46 (1)
H- F	0 / 79	0.01 (1)
H- D	0 / 408	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.10/1.00 (F-G:3) , WB=0.46/1.00 (D-F:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.59 (D) (INPUT = 0.90)
MSI METAL= 0.13 (D) (INPUT = 1.00)

DWG NO. TAM 12210-1B
STRUCTURAL COMPONENT ONLY

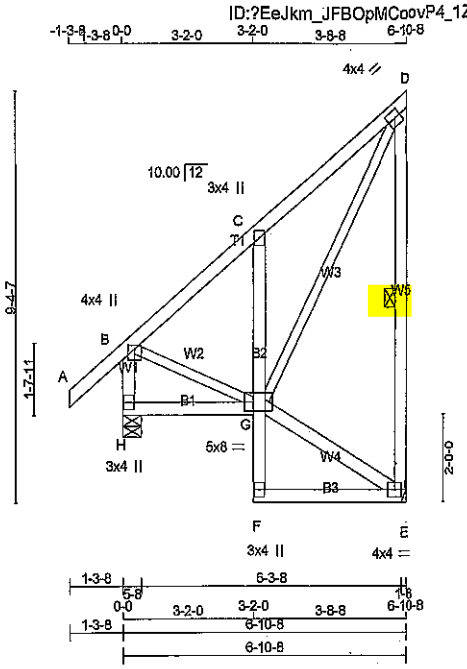
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:07 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-LZYv1vLJbQaBHNh1GqGLoxEuL0v15mfce4ZS6Hzdsow

Scale = 1:52.6



TOTAL WEIGHT = 51 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-t	MT20	5.0	8.0	2.50	2.50
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	455	0	455	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
H	601	0	601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	361	223 / 0	69 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	459	313 / 0	69 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G- E	-10 / 0	0.00 (1)
B- C	-274 / 0	-104.9	-104.9	0.15 (1)	6.25	G- D	0 / 484	0.11 (1)
C- D	-310 / 0	-104.9	-104.9	0.15 (1)	6.25	B- G	0 / 250	0.06 (1)
E- D	-402 / 0	0.0	0.0	0.16 (1)	6.25			
H- B	-555 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-27.5	-27.5	0.09 (3)	10.00			
F- G	0 / 85	0.0	0.0	0.03 (1)	10.00			
G- C	-434 / 0	0.0	0.0	0.04 (1)	7.81			
F- E	0 / 8	-27.5	-27.5	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (D-E:1) , BC=0.10/1.00 (E-F:3) , WB=0.11/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (D) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

DWG NO. TAM122111-T18
STRUCTURAL COMPONENT ONLY

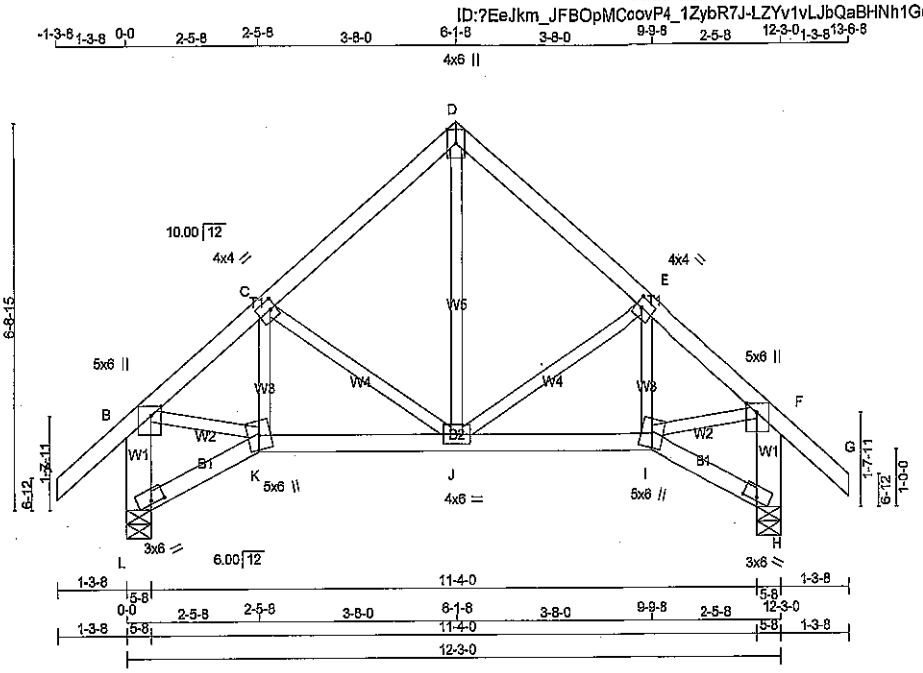
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:07 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-LZYv1vLJbQaBHNh1GqGLoxEuY0ut5loce4ZS6Hzdsow

Scale = 1:40.3



TOTAL WEIGHT = 60 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	5.0	6.0	2.00	2.25
H	BVM1-I	MT20	3.0	6.0	0.75	3.00
I	BBWW+m	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BBWW+m	MT20	5.0	6.0		
L	BVM1-I	MT20	3.0	6.0	0.75	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- D	0 / 471	0.11 (1)
B- C	-911 / 0	-104.9	-104.9	0.13 (1)	6.25	J- E	-311 / 0	0.10 (1)
C- D	-629 / 0	-104.9	-104.9	0.15 (1)	6.25	I- E	-12 / 91	0.02 (3)
D- E	-629 / 0	-104.9	-104.9	0.15 (1)	6.25	C- J	-311 / 0	0.10 (1)
E- F	-911 / 0	-104.9	-104.9	0.13 (1)	6.25	K- C	-12 / 91	0.02 (3)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	B- K	0 / 728	0.16 (1)
L- B	-923 / 0	0.0	0.0	0.07 (1)	7.81	I- F	0 / 728	0.16 (1)
H- F	-923 / 0	0.0	0.0	0.07 (1)	7.81			
L- K	0 / 0	-27.5	-27.5	0.05 (3)	10.00			
K- J	0 / 723	-27.5	-27.5	0.18 (2)	10.00			
J- I	0 / 723	-27.5	-27.5	0.18 (2)	10.00			
I- H	0 / 0	-27.5	-27.5	0.05 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.18/1.00 (I-J:2) , WB=0.16/1.00 (F-I:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

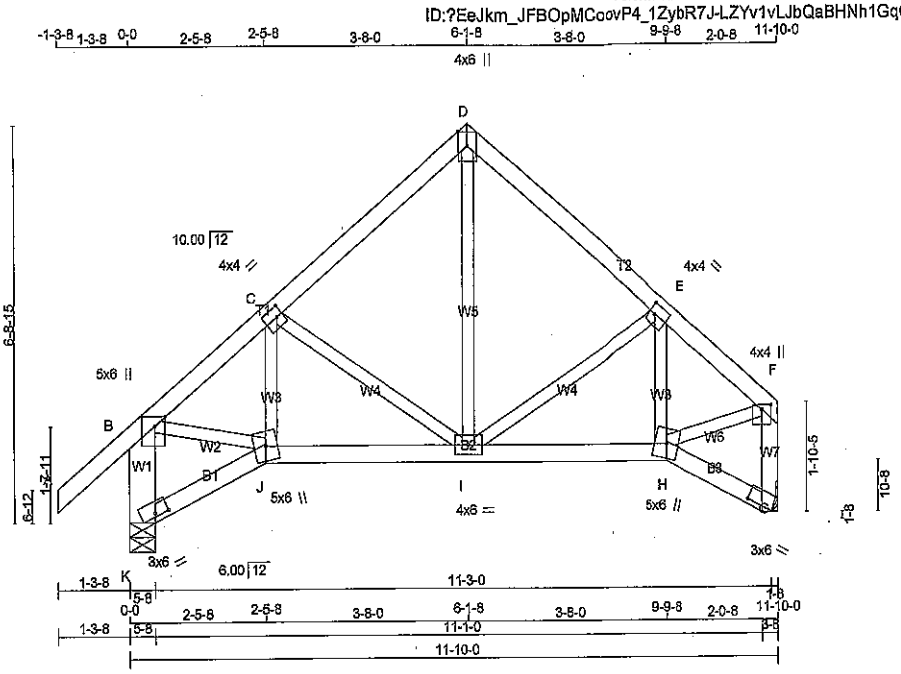
DWG NO. TAM 12212-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T19S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:07 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-LZYv1vLJbQaBHNh1GqGLoxEuW0uw5luce4ZS6Hzdsow

Scale = 1:39.3



TOTAL WEIGHT = 56 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	4.0	4.0	1.00	2.00
G	BVM1-I	MT20	3.0	6.0	1.00	4.00
H	BBWW+m	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BBWW+m	MT20	5.0	6.0		
K	BVM1-I	MT20	3.0	6.0	0.75	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	929	0	929	0	0	IN-SX	IN-SX
G	784	0	784	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	719	474 / 0	118 / 0	0 / 0	0 / 0	127 / 0	0 / 0
G	621	384 / 0	118 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- C	-9 / 93	0.02 (3)	
B- C	-874 / 0	-104.9	-104.9	0.13 (1)	6.25	C- I	-314 / 0	0.10 (1)	
C- D	-589 / 0	-104.9	-104.9	0.15 (1)	6.25	I- D	0 / 418	0.09 (1)	
D- E	-588 / 0	-104.9	-104.9	0.15 (1)	6.25	I- E	-213 / 0	0.07 (1)	
E- F	-755 / 0	-104.9	-104.9	0.13 (1)	6.25	H- E	-122 / 41	0.02 (1)	
K- B	-895 / 0	0.0	0.0	0.07 (1)	7.81	B- J	0 / 699	0.16 (1)	
G- F	-755 / 0	0.0	0.0	0.09 (1)	7.81	H- F	0 / 636	0.14 (1)	
K- J	0 / 0	-27.5	-27.5	0.05 (3)	10.00				
J- I	0 / 695	-27.5	-27.5	0.17 (2)	10.00				
I- H	0 / 612	-27.5	-27.5	0.17 (2)	10.00				
H- G	0 / 0	-27.5	-27.5	0.03 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (D-E:1) , BC=0.17/1.00 (I-J:2) , WB=0.16/1.00 (B-J:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12213-T19
STRUCTURAL COMPONENT ONLY

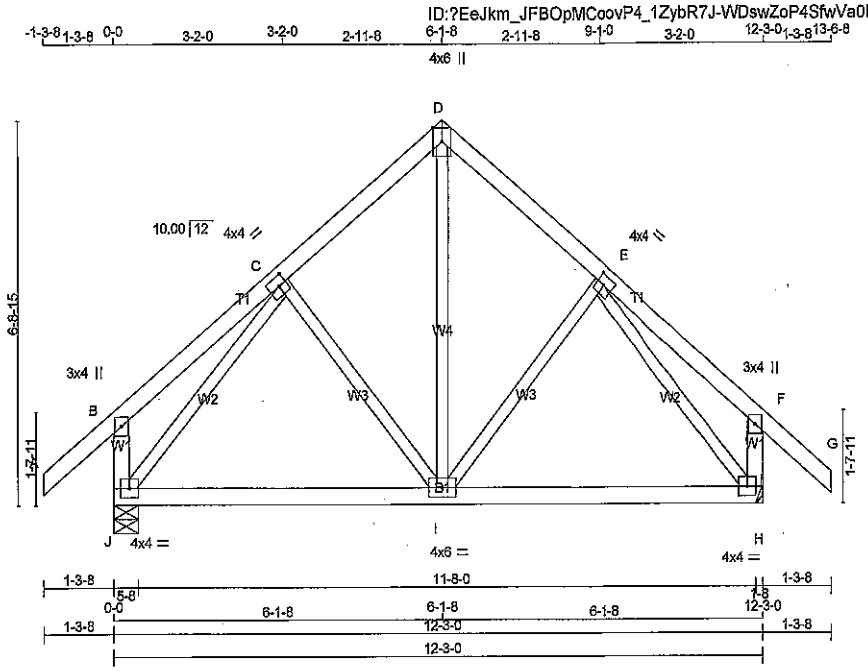
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:43 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WDswZoP4SfwVa0NbxOl8Po9JiSWuphm3vMyEvpzdsjg

Scale = 1:40.6



TOTAL WEIGHT = 59 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	956	0	956	0	0	5-8	5-8
H	956	0	956	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00
C- D	-552 / 0	-104.9	-104.9	0.12 (1)	6.25
D- E	-552 / 0	-104.9	-104.9	0.12 (1)	6.25
E- F	0 / 23	-104.9	-104.9	0.15 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- B	-268 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-268 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 494	-27.5	-27.5	0.34 (2)	10.00
I- H	0 / 494	-27.5	-27.5	0.34 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 424	0.10 (1)
I- E	-139 / 37	0.06 (1)
C- I	-139 / 37	0.06 (1)
J- C	-795 / 0	0.30 (1)
E- H	-795 / 0	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.34/1.00 (I-J:2) , WB=0.30/1.00 (C-J:1) , SSI=0.14/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

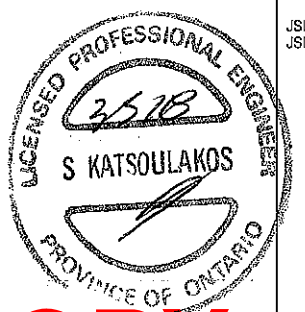
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



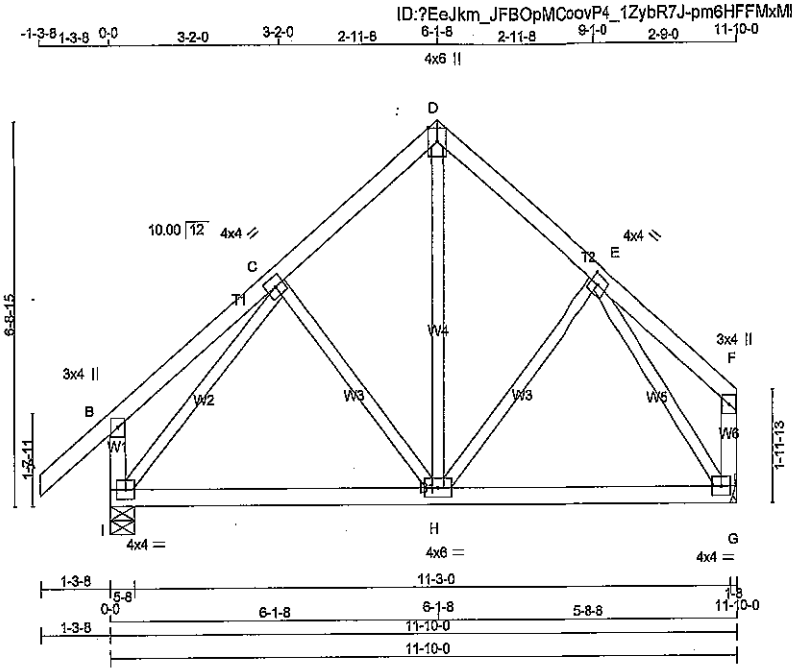
DWG NO. TAM 12229 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T20S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:08 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-pm6HFFMxMki2uXGDqYnaL8n3EPBsqA2lskJ?ekzdsov



TOTAL WEIGHT = 2 X 56 = 113 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	929	0	929	0	0	IN-SX 5-8	IN-SX 5-8
G	784	0	784	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	719	474 / 0	118 / 0	0 / 0	0 / 0	127 / 0	0 / 0
G	621	384 / 0	118 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		C- H	-147 / 34	0.06 (1)
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00		H- D	0 / 376	0.06 (1)
C- D	-516 / 0	-104.9	-104.9	0.12 (1)	6.25		H- E	-76 / 53	0.03 (1)
D- E	-513 / 0	-104.9	-104.9	0.10 (1)	6.25		I- C	-759 / 0	0.29 (1)
E- F	0 / 23	-104.9	-104.9	0.13 (1)	10.00		E- G	-754 / 0	0.26 (1)
I- B	-268 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	-104 / 0	0.0	0.0	0.01 (1)	7.81				
I- H	0 / 471	-27.5	-27.5	0.32 (2)	10.00				
H- G	0 / 427	-27.5	-27.5	0.32 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.0	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.32/1.00 (H-I:2) , WB=0.29/1.00 (C-I:1) , SSI=0.14/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

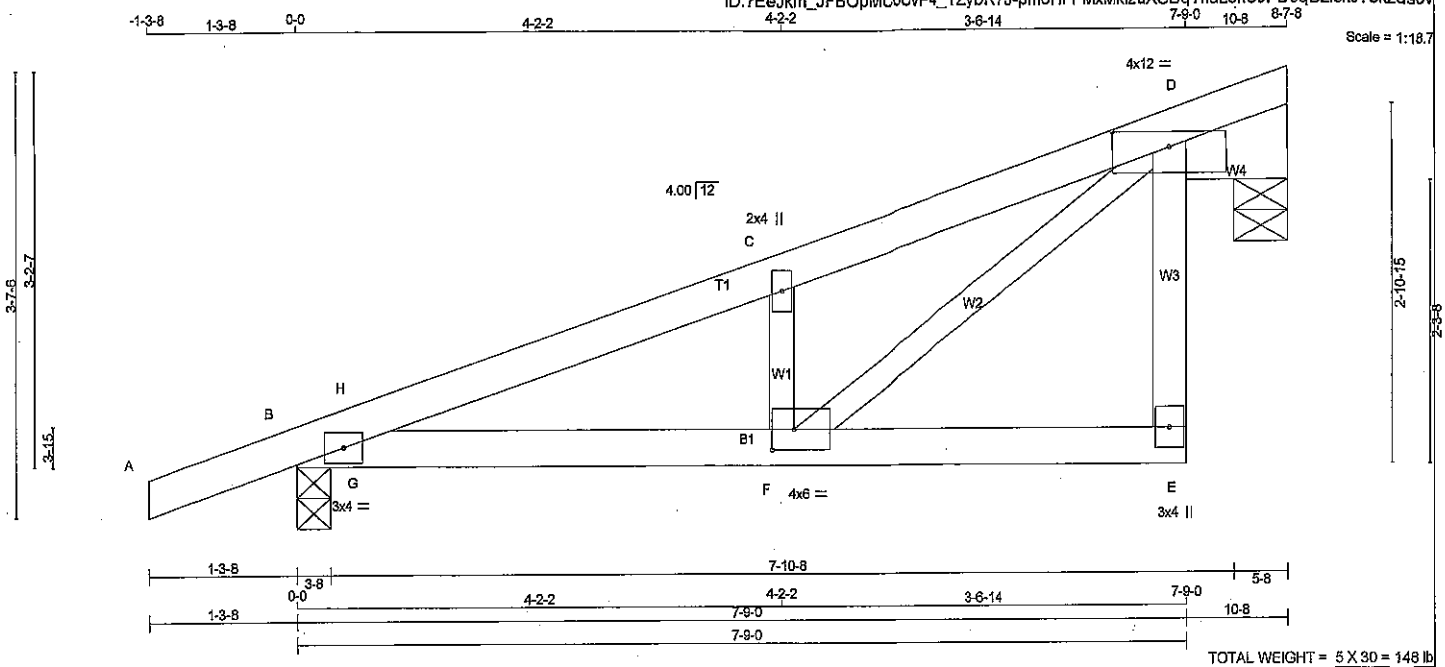
JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

DWG NO. TAM 12214-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T21	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:08 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-pm6HFFMxMki2uXGDqYnaL8n39PD0qB2lskJ?ekzdsov



TOTAL WEIGHT = 5 X 30 = 148 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	12.0	1.50	6.00
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0	2.00	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	513	0	513	0	0	5-8	5-8
B	654	0	654	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	407	252 / 0	77 / 0	0 / 0	0 / 0	77 / 0	0 / 0
B	502	339 / 0	77 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00	F- C	-455 / 0	0.07 (1)	
B- H	-895 / 0	-104.9	-104.9	0.04 (2)	6.25	F- D	0 / 1006	0.23 (1)	
H- C	-858 / 0	-104.9	-104.9	0.16 (1)	6.25	G- H	-104 / 70	0.00 (1)	
C- D	-869 / 0	-104.9	-104.9	0.16 (1)	6.25				
E- D	0 / 67	0.0	0.0	0.01 (3)	10.00				
B- G	0 / 827	-27.5	-27.5	0.24 (1)	10.00				
G- F	0 / 827	-27.5	-27.5	0.25 (1)	10.00				
F- E	0 / 0	-27.5	-27.5	0.09 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.25/1.00 (F-G:1) , WB=0.23/1.00 (D-F:1) , SSI=0.17/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

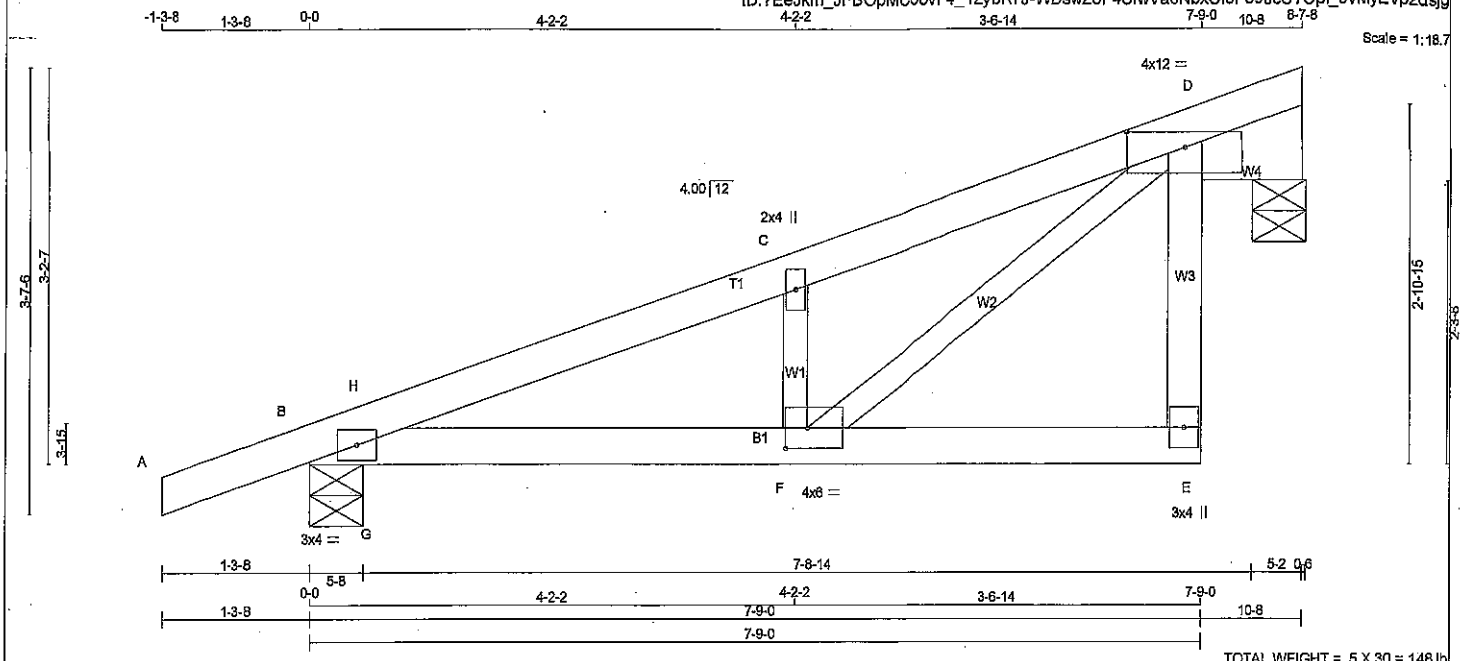
JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

DWG NO. TAM12215-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T21	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:43 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WDswZoP4SfwVa0NbxOl8Po9JcSYOpi_3vMyEvpzdsjg



TOTAL WEIGHT = 5 X 30 = 148 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWWW1-p	MT20	4.0	12.0	1.50	6.00
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0	2.00	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	513	0	513	0	0	IN-SX 5-8 (5-2)	IN-SX 5-8
B	654	0	654	0	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	407	252 / 0	77 / 0	0 / 0	0 / 0	77 / 0	0 / 0
B	502	339 / 0	77 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00		F- C	-455 / 0	0.07 (1)
B- H	-895 / 0	-104.9	-104.9	0.04 (2)	6.25		F- D	0 / 1006	0.23 (1)
H- C	-858 / 0	-104.9	-104.9	0.16 (1)	6.25		G- H	-104 / 70	0.00 (1)
C- D	-869 / 0	-104.9	-104.9	0.16 (1)	6.25				
E- D	0 / 67	0.0	0.0	0.01 (3)	10.00				
B- G	0 / 827	-27.5	-27.5	0.24 (1)	10.00				
G- F	0 / 827	-27.5	-27.5	0.25 (1)	10.00				
F- E	0 / 0	-27.5	-27.5	0.09 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.25/1.00 (F-G:1) , WB=0.23/1.00 (D-F:1) , SSI=0.17/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

DWG NO. TAM 12230-18
STRUCTURAL
COMPONENT ONLY

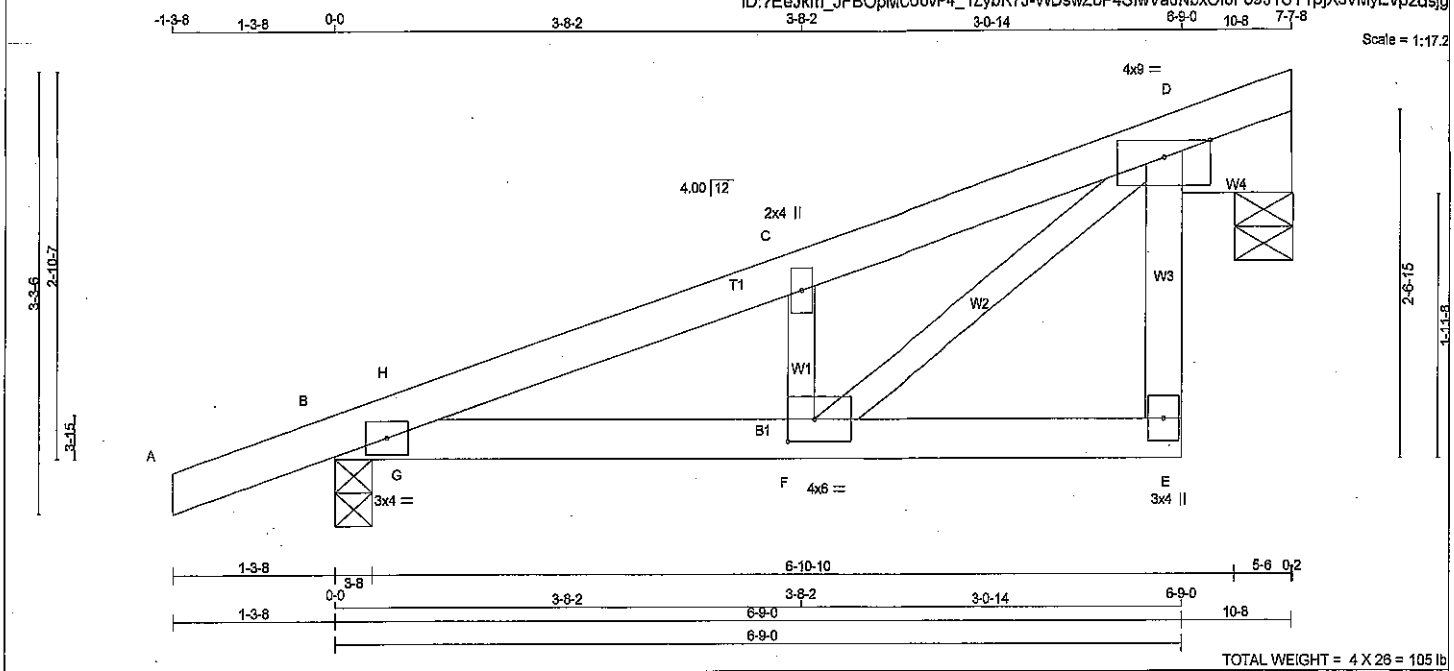
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T22	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:43 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WDswZoP4SfwVa0NbxOl8Po9J1SY1pjX3vMyEvpzdsjg



TOTAL WEIGHT = 4 X 26 = 105 lb

[M][F]

LUMBER

N. L. G. A. RULES

	SIZE		LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWWW1-p	MT20	4.0	9.0	Edge	
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	447	0	447	0	0	5-8 (5-6)	5-8
B	587	0	587	0	0	3-8	3-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	354	219 / 0	67 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	449	306 / 0	67 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00	
B- H	-761 / 0	-104.9	-104.9	0.03 (2)	6.25	
H- C	-725 / 0	-104.9	-104.9	0.11 (1)	6.25	
C- D	-733 / 0	-104.9	-104.9	0.11 (1)	6.25	
E- D	0 / 59	0.0	0.0	0.01 (2)	10.00	
B- G	0 / 700	-27.5	-27.5	0.20 (1)	10.00	
G- F	0 / 700	-27.5	-27.5	0.21 (1)	10.00	
F- E	0 / 0	-27.5	-27.5	0.06 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-390 / 0	0.06 (1)
F- D	0 / 853	0.19 (1)
G- H	-84 / 57	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.21/1.00 (F-G:1) , WB=0.19/1.00 (D-F:1) , SSI=0.15/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

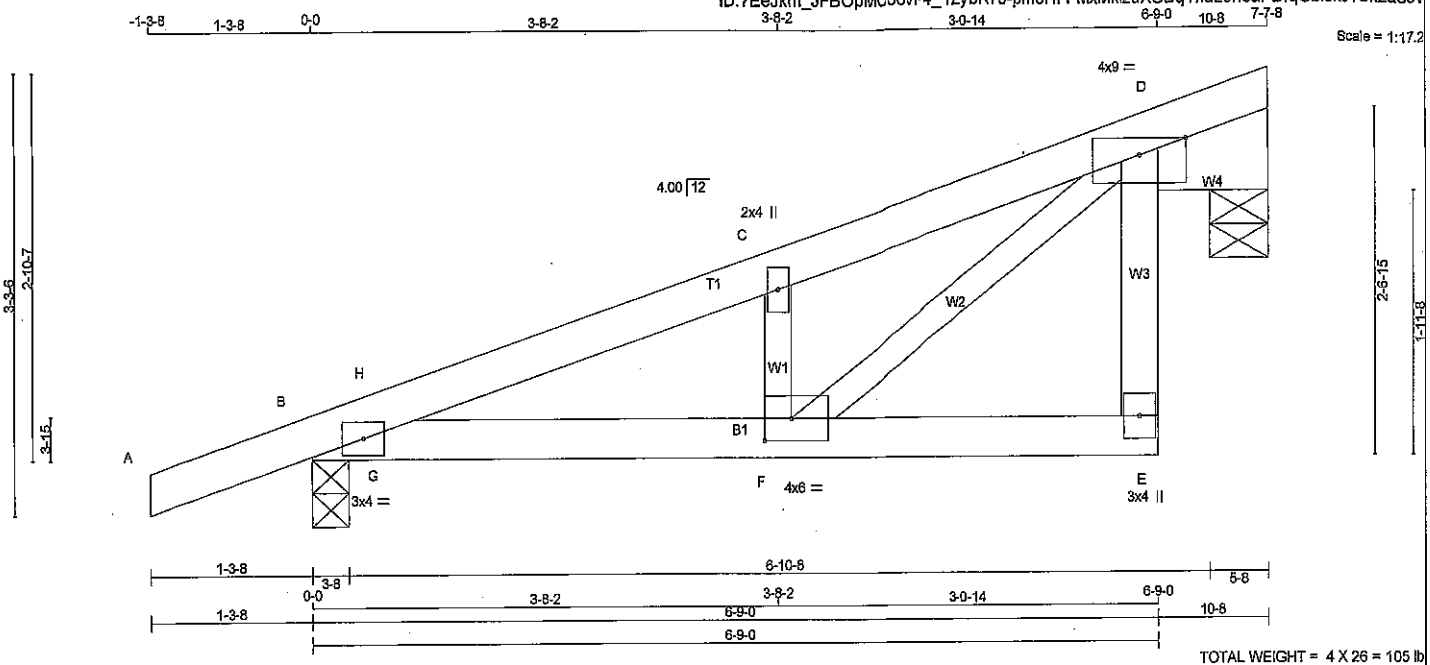
DWG NO. TAM12231-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T22	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:08 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-pm6HFFMxMki2uXGDqYnaL8n3aPDfqCbIskJ?ekzdsov



TOTAL WEIGHT = 4 X 26 = 105 lb

[MII]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	4.0	9.0	Edge	
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	447	0	447	0	0	5-8	5-8
B	587	0	587	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	354	219 / 0	67 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	449	306 / 0	67 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00	F- C	-390 / 0	0.08 (1)
B- H	-761 / 0	-104.9	-104.9	0.03 (2)	6.25	F- D	0 / 853	0.19 (1)
H- C	-725 / 0	-104.9	-104.9	0.11 (1)	6.25	G- H	-84 / 57	0.00 (1)
C- D	-733 / 0	-104.9	-104.9	0.11 (1)	6.25			
E- D	0 / 59	0.0	0.0	0.01 (2)	10.00			
B- G	0 / 700	-27.5	-27.5	0.20 (1)	10.00			
G- F	0 / 700	-27.5	-27.5	0.21 (1)	10.00			
F- E	0 / 0	-27.5	-27.5	0.06 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.21/1.00 (F-G:1) , WB=0.19/1.00 (D-F:1) , SSI=0.15/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

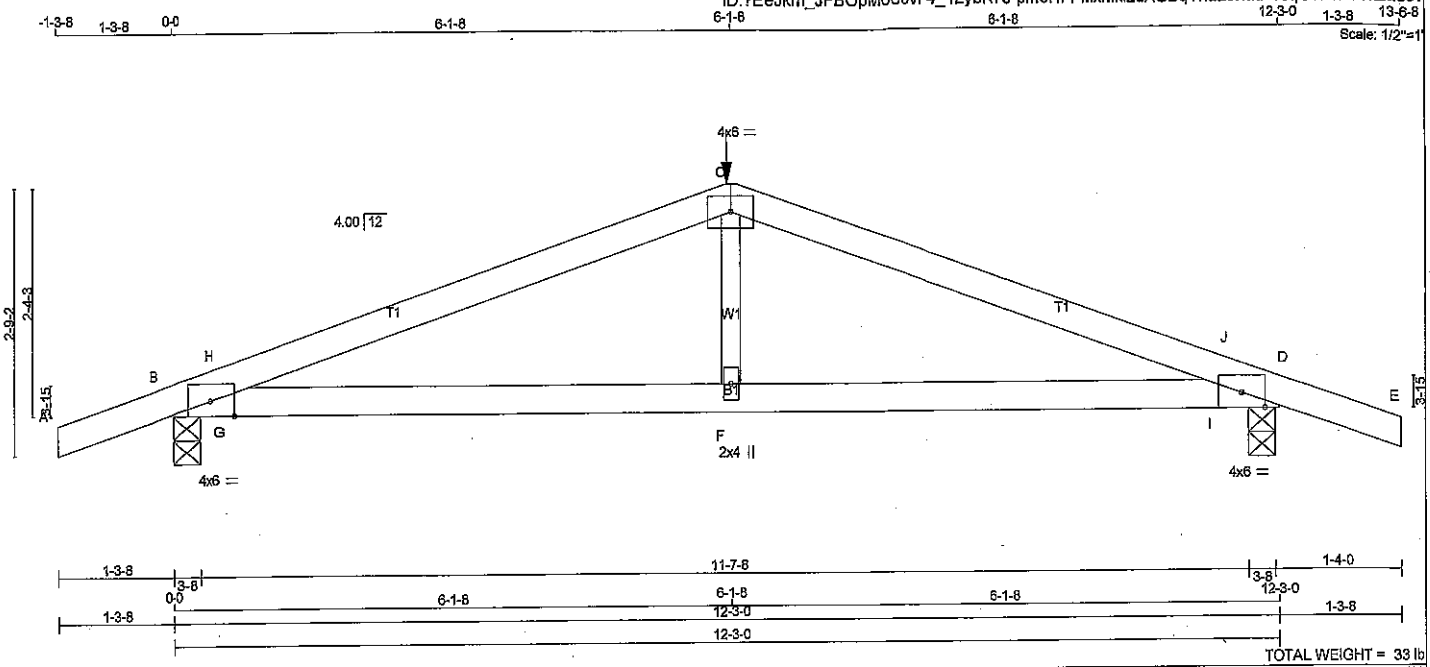
DWG NO. TAM 12216-TB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:08 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-pm6HFFMxMki2uXGDqYnaL8nutP1cqCvlskJ?ekzdsov



TOTAL WEIGHT = 33 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0	Edge	
C	TTW-p	MT20	4.0	6.0		
D	TMB1-I	MT20	4.0	6.0	Edge	
F	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 990.8 lbs FACTORED DOWN AT 6-1-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1621	0	1621	0	0	3-8	3-8
D	1621	0	1621	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1288	791 / 0	249 / 0	0 / 0	0 / 0	247 / 0	0 / 0
D	1288	791 / 0	249 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 21	-104.9	-104.9	0.14 (1)	10.00	F- C	0 / 892	0.17 (2)
B- H	-3233 / 0	-104.9	-104.9	0.24 (1)	3.65	G- H	-315 / 262	0.00 (1)
H- C	-3233 / 0	-104.9	-104.9	0.88 (1)	2.91	I- J	-315 / 262	0.00 (1)
C- J	-3233 / 0	-104.9	-104.9	0.86 (1)	2.91			
J- D	-3233 / 0	-104.9	-104.9	0.24 (1)	3.65			
D- E	0 / 21	-104.9	-104.9	0.14 (1)	10.00			
B- G	0 / 3063	-55.9	-55.9	0.90 (1)	10.00			
G- F	0 / 3063	-55.9	-55.9	0.98 (1)	10.00			
F- I	0 / 3063	-55.9	-55.9	0.98 (1)	10.00			
I- D	0 / 3063	-55.9	-55.9	0.90 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-1-8	-991	-991	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-1-8
END SETBACK = 6-1-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: PARTIAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 985 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 598 (0.25")

CSI: TC=0.88/1.00 (C-H:1) , BC=0.98/1.00 (F-I:1) , WB=0.17/1.00 (C-F:2) , SSI=0.35/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.88 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

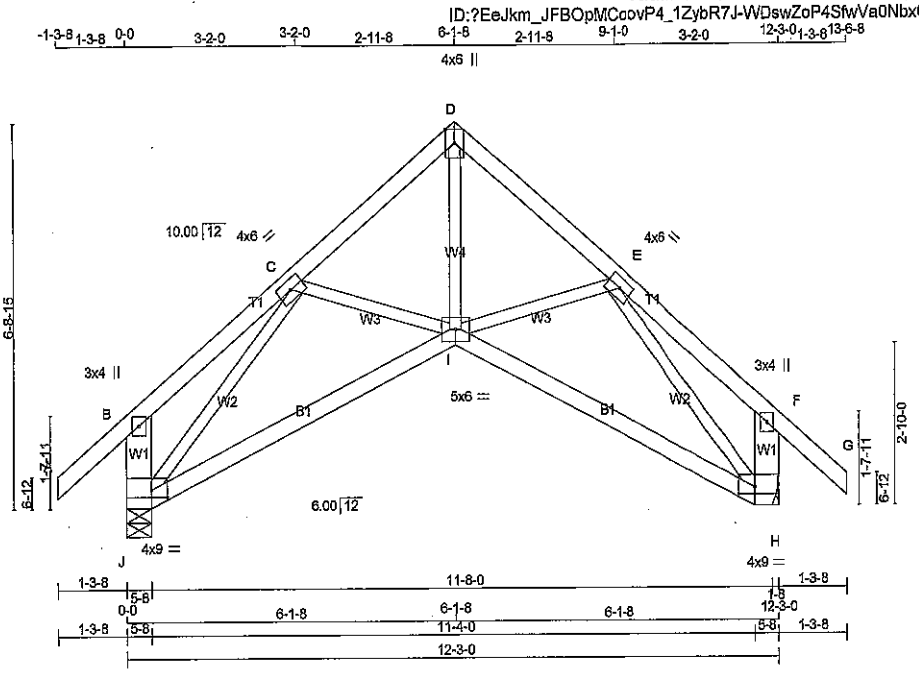
DWG NO. TAM 12217-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T24	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:43 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WDswZoP4SfwVa0NbxOl8Po9JqSWNpfY3vMyEvpzdsjg



TOTAL WEIGHT = 3 X 58 = 174 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	4.0	9.0	1.50	3.50
I	BBWWW-p	MT20	5.0	6.0		
J	BVMW1-p	MT20	4.0	9.0	1.50	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	956	0	956	0	0	IN-SX	IN-SX
H	956	0	956	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	741	488 / 0	123 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 20	-104.9	-104.9	0.13 (1)	10.00	
C- D	-929 / 0	-104.9	-104.9	0.11 (1)	6.25	
D- E	-929 / 0	-104.9	-104.9	0.11 (1)	6.25	
E- F	0 / 20	-104.9	-104.9	0.13 (1)	10.00	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- B	-275 / 0	0.0	0.0	0.02 (1)	7.81	
H- F	-275 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 807	-27.5	-27.5	0.38 (2)	10.00	
I- H	0 / 807	-27.5	-27.5	0.38 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 905	0.20 (1)
I- E	-25 / 77	0.02 (3)
C- I	-25 / 77	0.02 (3)
J- C	-1200 / 0	0.45 (1)
E- H	-1200 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 802 (0.18")

CSI: TC=0.14/1.00 (F-G:1) , BC=0.38/1.00 (I-J:2) , WB=0.45/1.00 (C-J:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

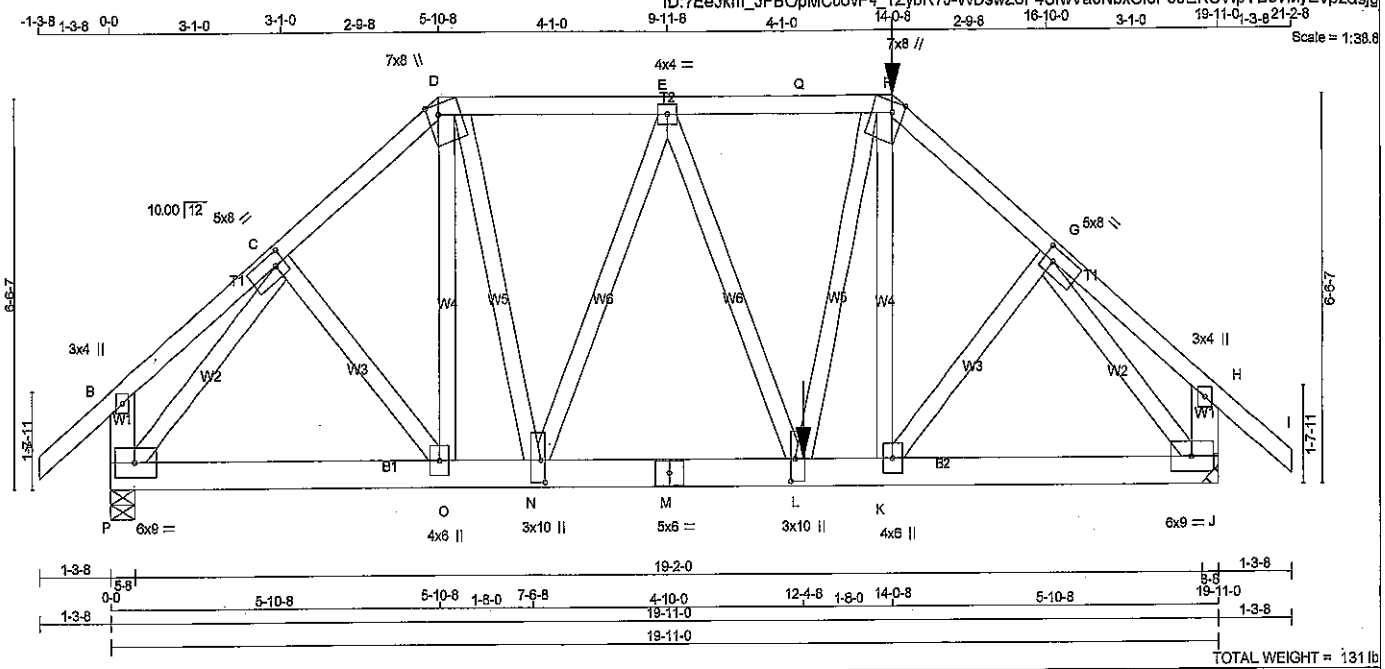
DWG NO. TAM12232-17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:43 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-WDswZoP4SfwVa0NbxOl8Po9ERSVlpYB3vMyEvpzdsjg



TOTAL WEIGHT = 131 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	7.0	8.0	Edge	2.25
G	TMWW-t	MT20	5.0	8.0	2.50	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	6.0	9.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	3.0	10.0	4.50	0.75
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	10.0	4.50	0.75
O	BMWW+t	MT20	4.0	6.0		
P	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 449.5 lbs FACTORED DOWN AT 14-0-8 ON TOP CHORD, AND 1766.5 lbs FACTORED DOWN AT 12-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	2366	0	2366	0	0	5-8	5-8
J	3150	0	3150	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1853	1184 / 0	329 / 0	0 / 0	0 / 0	340 / 0	0 / 0
J	2503	1539 / 0	483 / 0	0 / 0	0 / 0	480 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00		C- O	0 / 225	0.04 (1)
B- C	0 / 20	-104.9	-104.9	0.14 (1)	10.00		O- D	-8 / 131	0.02 (3)
C- D	-2332 / 0	-104.9	-104.9	0.22 (1)	4.26		D- N	0 / 1249	0.22 (1)
D- E	-2106 / 0	-104.9	-104.9	0.43 (1)	4.15		N- E	-1217 / 0	0.69 (1)
E- Q	-2768 / 0	-104.9	-104.9	0.49 (1)	3.61		E- L	0 / 628	0.11 (1)
Q- F	-2768 / 0	-206.6	-206.6	0.49 (1)	3.61		L- F	0 / 1136	0.20 (1)
F- G	-3232 / 0	-104.9	-104.9	0.28 (1)	3.64		K- F	0 / 395	0.07 (3)
G- H	-1 / 18	-104.9	-104.9	0.13 (1)	10.00		K- G	0 / 408	0.07 (1)
H- I	0 / 47	-104.9	-104.9	0.16 (1)	10.00		P- C	-2600 / 0	0.63 (1)
P- B	-269 / 0	0.0	0.0	0.02 (1)	7.81		G- J	-3515 / 0	0.85 (1)
J- H	-271 / 0	0.0	0.0	0.02 (1)	7.81				
P- O	0 / 1625	-27.5	-27.5	0.28 (1)	10.00				
O- N	0 / 1770	-27.5	-27.5	0.30 (1)	10.00				
N- M	0 / 2543	-27.5	-27.5	0.42 (1)	10.00				
M- L	0 / 2543	-27.5	-27.5	0.42 (1)	10.00				
L- K	0 / 2462	-54.1	-54.1	0.45 (1)	10.00				
K- J	0 / 2197	-54.1	-54.1	0.41 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
F	14-0-8	-450	-450	—	FRONT	VERT	TOTAL
L	12-4-8	-1767	-1767	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-6-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.45/1.00 (K-L:1) , WB=0.85/1.00 (G-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DWG NO. TAM 12233-18B pg 1/2

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
3516
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T25	1	1	TRUSS DESC.	

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)

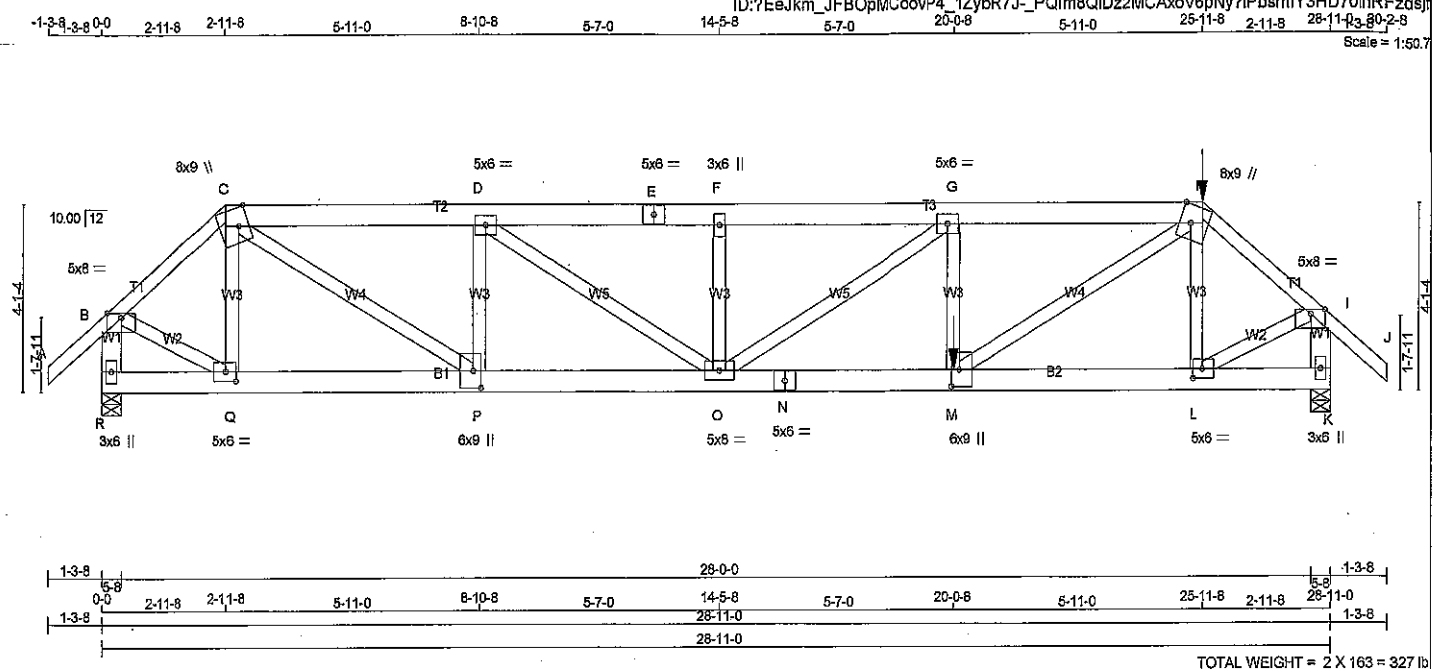
LICENSED PROFESSIONAL ENGINEER
3516
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12233-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T26	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:44 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-_PQIm8QiDz2MCAxoV6pNy?iPbsmIY3HD70inRFzdsjf



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	SIDE(0.0)
C- E	2	12	TOP
E- H	2	12	SIDE(50.8)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(23.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	3.00
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	3.00
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	6.0	9.0	4.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	6.0	9.0	4.50	2.25
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3249	0	3249	0	0	5-8	5-8
K	5216	0	5216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2557	1613 / 0	468 / 0	0 / 0	0 / 0	476 / 0	0 / 0
K	4126	2566 / 0	778 / 0	0 / 0	0 / 0	782 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-766 / 0		0.07 (1)
B- C	-3130 / 0	-104.9	-104.9	0.12 (1)	5.13	C- P	0 / 4486		0.40 (1)
C- D	-8196 / 0	-104.9	-104.9	0.20 (1)	4.63	P- D	-2238 / 0		0.19 (1)
D- E	-8542 / 0	-104.9	-104.9	0.24 (1)	4.01	D- O	0 / 2802		0.25 (1)
E- F	-8542 / 0	-104.9	-104.9	0.24 (1)	4.01	O- F	-504 / 0		0.04 (1)
F- G	-8542 / 0	-104.9	-104.9	0.27 (1)	3.98	O- G	-1526 / 0		0.42 (1)
G- H	-9819 / 0	-208.6	-208.6	0.46 (1)	3.53	M- G	-212 / 272		0.02 (3)
H- I	-5221 / 0	-104.9	-104.9	0.19 (1)	4.10	M- H	0 / 8873		0.61 (1)
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	L- H	-1193 / 0		0.10 (1)
R- B	-3242 / 0	0.0	0.0	0.12 (1)	7.69	B- Q	0 / 2592		0.23 (1)
K- I	-5208 / 0	0.0	0.0	0.19 (1)	6.39	L- I	0 / 4323		0.38 (1)
R- Q	0 / 0	-27.5	-27.5	0.05 (2)	10.00				
Q- P	0 / 2382	-27.5	-27.5	0.20 (1)	10.00				
P- O	0 / 6195	-27.5	-27.5	0.45 (1)	10.00				
O- N	0 / 9819	-27.5	-27.5	0.72 (1)	10.00				
N- M	0 / 9819	-27.5	-27.5	0.72 (1)	10.00				
M- L	0 / 3976	-54.1	-54.1	0.35 (1)	10.00				
L- K	0 / 0	-54.1	-54.1	0.11 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	25-11-8	-226	-226	---	FRONT	VERT	TOTAL
M	20-0-8	-3281	-3281	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 8-10-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.46/1.00 (G-H:1) , BC=0.72/1.00 (M-O:1) , WB=0.61/1.00 (H-M:1) , SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12234-18 P6/12
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T26	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 3280.7 lbs FACTORED DOWN AT 20-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9528
S. KATSOULAKOS
PROVINCE OF ONTARIO

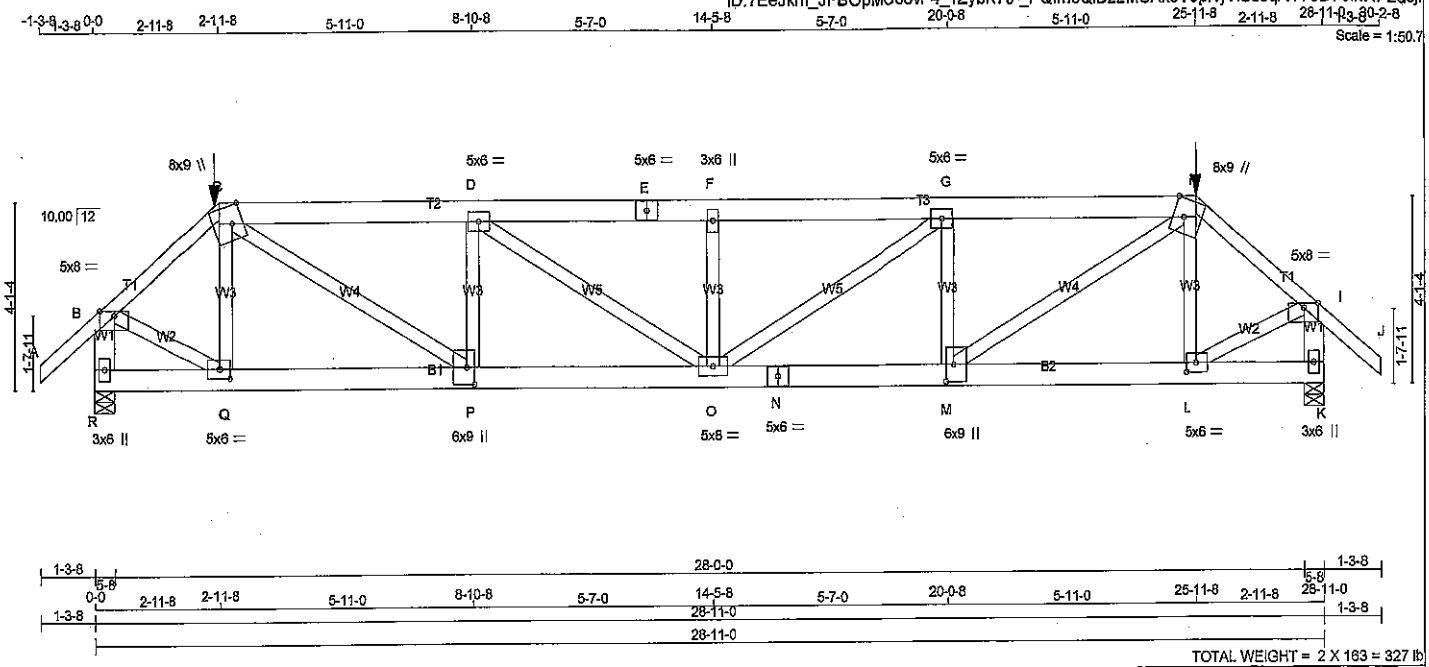
DWG NO. TAM 12234-1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T26Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:44 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-_PQIm8QiDz2MCAxoV6pNy?iSssqAY70D70inRFzdsjf



TOTAL WEIGHT = 2 X 183 = 327 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	SIDE(0.0)
H- J 1	12	SIDE(0.0)
C- E 2	12	SIDE(44.8)
E- H 2	12	SIDE(44.8)
R- B 2	12	TOP
K- I 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
R- N 2	12	SIDE(20.3)
N- K 2	12	SIDE(20.3)
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	3.00
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	3.00
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	6.0	9.0	4.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	6.0	9.0	4.50	2.25
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3657	0	3657	0	0	5-8	5-8
K	3657	0	3657	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2886	1807 / 0	536 / 0	0 / 0	0 / 0	542 / 0	0 / 0
K	2886	1807 / 0	536 / 0	0 / 0	0 / 0	542 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00		Q- C	-777 / 7	0.07 (1)
B- C	-3536 / 0	-104.9	-104.9	0.13 (1)	4.88		C- P	0 / 4163	0.37 (1)
C- D	-6234 / 0	-194.6	-194.6	0.30 (1)	4.49		P- D	-1924 / 0	0.16 (1)
D- E	-7209 / 0	-194.6	-194.6	0.32 (1)	4.21		D- O	0 / 1164	0.10 (1)
E- F	-7209 / 0	-194.6	-194.6	0.32 (1)	4.21		O- F	-988 / 0	0.08 (1)
F- G	-7209 / 0	-194.6	-194.6	0.32 (1)	4.21		O- G	0 / 1164	0.10 (1)
G- H	-6234 / 0	-194.6	-194.6	0.30 (1)	4.49		M- G	-1924 / 0	0.16 (1)
H- I	-3536 / 0	-104.9	-104.9	0.13 (1)	4.88		M- H	0 / 4163	0.37 (1)
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00		L- H	-777 / 7	0.07 (1)
R- B	-3625 / 0	0.0	0.0	0.13 (1)	7.38		B- Q	0 / 2929	0.26 (1)
K- I	-3625 / 0	0.0	0.0	0.13 (1)	7.38		L- I	0 / 2929	0.26 (1)
R- Q	0 / 0	-51.0	-51.0	0.08 (2)	10.00				
Q- P	0 / 2695	-51.0	-51.0	0.23 (1)	10.00				
P- O	0 / 6234	-51.0	-51.0	0.47 (1)	10.00				
O- N	0 / 6234	-51.0	-51.0	0.47 (1)	10.00				
N- M	0 / 6234	-51.0	-51.0	0.47 (1)	10.00				
M- L	0 / 2695	-51.0	-51.0	0.23 (1)	10.00				
L- K	0 / 0	-51.0	-51.0	0.08 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-226	-226	---	FRONT	VERT	TOTAL
H	25-11-8	-226	-226	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.32/1.00 (D-F:1) , BC=0.47/1.00 (M-O:1) , WB=0.37/1.00 (H-M:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 122351-18 P6 1/2

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T26Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 226.3 lbs FACTORED DOWN AT 25-11-8, AND 226.3 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

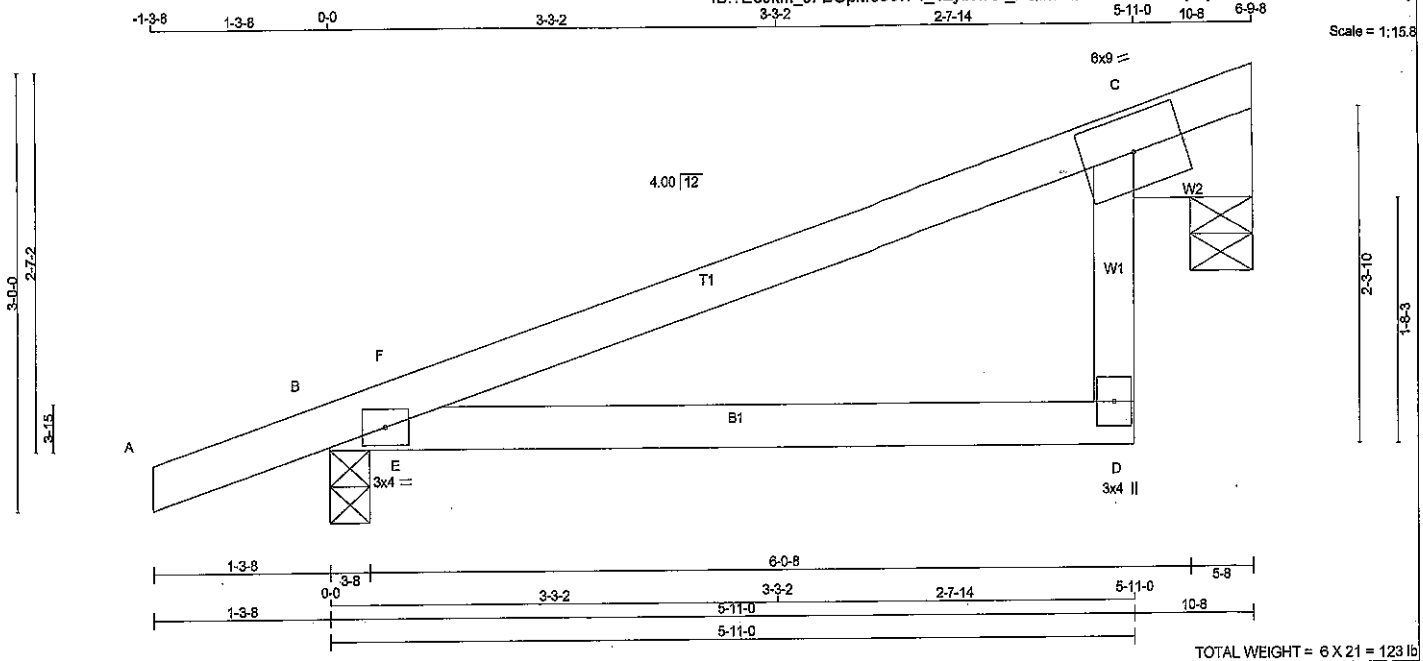
LICENSED PROFESSIONAL ENGINEER
3526
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12235-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T27	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:44 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-_PQIm8QiDz2MCAxoV6pNy?iPNss8YCmD70inRFzdsjf



TOTAL WEIGHT = 6 X 21 = 123 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMVW1-t	MT20	6.0	9.0	
D	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	392	0	392	0	0	5-8	5-8
B	532	0	532	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	310	192 / 0	59 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	405	279 / 0	59 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD (PLF)	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-104.9	-104.9	0.13	(1)	10.00	
B- F	-61 / 14	-104.9	-104.9	0.11	(3)	6.25	
F- C	-1 / 6	-104.9	-104.9	0.48	(1)	10.00	
D- C	0 / 138	0.0	0.0	0.02	(2)	10.00	
B- E	0 / 0	-27.5	-27.5	0.32	(1)	10.00	
E- D	0 / 0	-27.5	-27.5	0.34	(1)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
E- F	-353 / 168	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 617 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 379 (0.19")

CSI: TC=0.48/1.00 (C-F:1) , BC=0.34/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.31/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

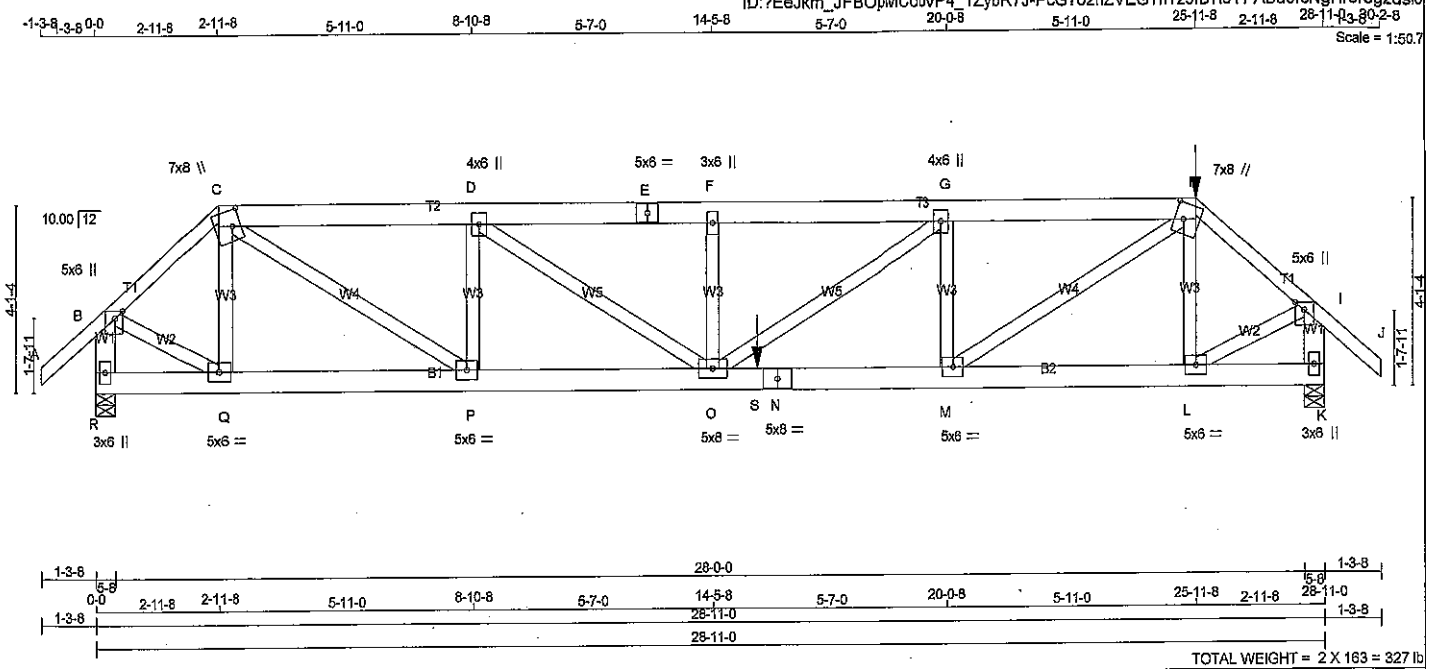
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 122361B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295493	T26Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:16:29 2018 Page 1
ID:?EeJkm_JFBOpMCovP4_1ZybR7J-PcG?o2hZVEGTIITz9fB7i8YPABd8fcNgHreregzdsl0



TOTAL WEIGHT = 2 X 163 = 327 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	SIDE(0.0)
C- E	2	12	TOP
E- H	2	12	SIDE(0.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	4.25	2.50
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	7.0	8.0	4.25	2.50
I	TMW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2804	0	2804	0	0	5-8	5-8
K	3065	0	3065	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2218	1389 / 0	420 / 0	0 / 0	0 / 0	408 / 0	0 / 0
K	2426	1517 / 0	461 / 0	0 / 0	0 / 0	448 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-591 / 0	0.05 (1)	
B- C	-2865 / 0	-104.9	-104.9	0.11 (1)	5.47	C- P	0 / 3592	0.32 (1)	
C- D	-5084 / 0	-104.9	-104.9	0.17 (1)	5.04	P- D	-1894 / 0	0.16 (1)	
D- E	-6895 / 0	-104.9	-104.9	0.20 (1)	4.42	D- O	0 / 2164	0.19 (1)	
E- F	-6895 / 0	-104.9	-104.9	0.20 (1)	4.42	O- F	-512 / 0	0.04 (1)	
F- G	-6895 / 0	-104.9	-104.9	0.20 (1)	4.43	O- G	0 / 1647	0.15 (1)	
G- H	-5517 / 0	-104.9	-104.9	0.18 (1)	4.86	M- G	-1607 / 0	0.14 (1)	
H- I	-2900 / 0	-104.9	-104.9	0.12 (1)	5.29	M- H	0 / 3897	0.34 (1)	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	L- H	-792 / 0	0.07 (1)	
R- B	-2806 / 0	0.0	0.0	0.10 (1)	7.81	B- Q	0 / 2207	0.20 (1)	
K- I	-3026 / 0	0.0	0.0	0.11 (1)	7.81	L- I	0 / 2401	0.21 (1)	
R- Q	0 / 0	-28.0	-28.0	0.06 (2)	10.00				
Q- P	0 / 2030	-28.0	-28.0	0.19 (1)	10.00				
P- O	0 / 5084	-28.0	-28.0	0.51 (1)	10.00				
O- S	0 / 5517	-28.0	-28.0	0.70 (1)	10.00				
S- N	0 / 5517	-28.0	-28.0	0.70 (1)	10.00				
N- M	0 / 5517	-28.0	-28.0	0.70 (1)	10.00				
M- L	0 / 2203	-28.0	-28.0	0.24 (1)	10.00				
L- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	25-11-8	-188	-188	---	FRONT	VERT	TOTAL
S	15-6-0	-1547	-1547	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.20/1.00 (D-F:1) , BC=0.70/1.00 (M-O:1) , WB=0.34/1.00 (H-M:1) , SSI=0.48/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

pGR

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 3528 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12241-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295493	T26Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:16:29 2018 Page 2
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-PcG?o2hZVEGTIITz9fB?i8YPABd8fcNgHreregzdsi0

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.2 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 1546.9 lbs FACTORED DOWN AT 15-6-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
35258
S. KATSOULAKOS
PROVINCE OF ONTARIO

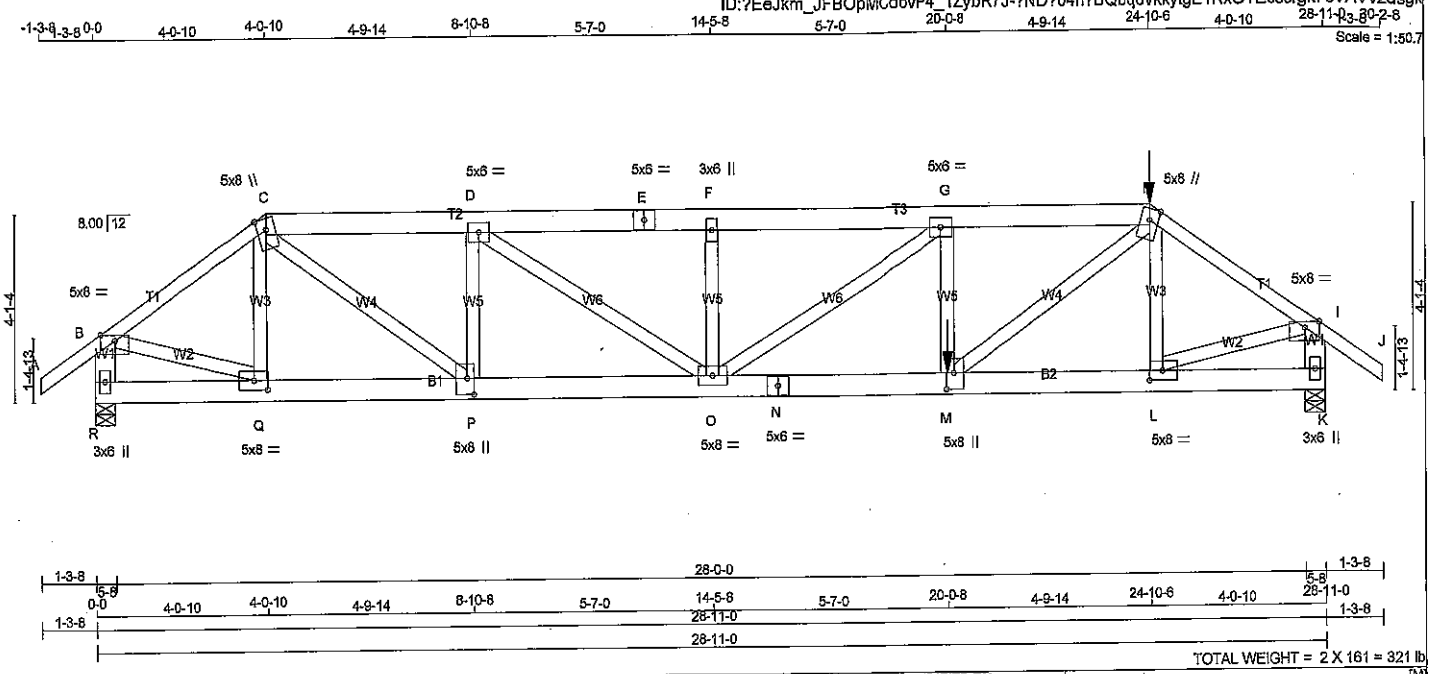
DWG NO. TAM 12241-TB
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T28	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:51 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?ND?04h?BQbq8vkkytgE1RxOTEcssfgk7cVAVvzdsgk



TOTAL WEIGHT = 2 X 161 = 321 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
H-J	1	12	SIDE(0.0)
C-E	2	12	TOP
E-H	2	12	SIDE(50.8)
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-N	2	12	TOP
N-K	2	12	SIDE(23.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMWW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.75
M	BMWW+t	MT20	5.0	8.0	4.25	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	8.0	4.25	2.00
Q	BMWW-t	MT20	5.0	8.0	2.50	3.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3139	0	3139	0	0	5-8	5-8
K	4918	0	4918	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2469	1560 / 0	450 / 0	0 / 0	0 / 0	459 / 0	0 / 0
K	3894	2415 / 0	739 / 0	0 / 0	0 / 0	740 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	Q-C	-628 / 0	0.06 (1)
B-C	-3581 / 0	-104.9	-104.9	0.23 (1)	4.75	C-P	0 / 3649	0.32 (1)
C-D	-5917 / 0	-104.9	-104.9	0.14 (1)	4.77	P-D	-2070 / 0	0.18 (1)
D-E	-8120 / 0	-104.9	-104.9	0.20 (1)	4.13	D-O	0 / 2631	0.23 (1)
E-F	-8120 / 0	-104.9	-104.9	0.20 (1)	4.13	O-F	-567 / 0	0.05 (1)
F-G	-8120 / 0	-104.9	-104.9	0.20 (1)	4.14	O-G	-1279 / 0	0.36 (1)
G-H	-9191 / 0	-206.6	-206.6	0.33 (1)	3.77	M-G	-197 / 222	0.02 (3)
H-I	-5863 / 0	-104.9	-104.9	0.32 (1)	3.78	M-H	0 / 5355	0.47 (1)
I-J	0 / 40	-104.9	-104.9	0.08 (1)	10.00	L-H	-940 / 0	0.08 (1)
R-B	-3093 / 0	0.0	0.0	0.11 (1)	7.81	B-Q	0 / 3085	0.27 (1)
K-I	-4837 / 0	0.0	0.0	0.17 (1)	6.59	L-I	0 / 5051	0.45 (1)
R-Q	0 / 0	-27.5	-27.5	0.03 (3)	10.00			
Q-P	0 / 3014	-27.5	-27.5	0.22 (1)	10.00			
P-O	0 / 5917	-27.5	-27.5	0.43 (1)	10.00			
O-N	0 / 9191	-27.5	-27.5	0.66 (1)	10.00			
N-M	0 / 9191	-27.5	-27.5	0.66 (1)	10.00			
M-L	0 / 4931	-54.1	-54.1	0.39 (1)	10.00			
L-K	0 / 0	-54.1	-54.1	0.07 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	24-10-6	-310	-310	---	FRONT	VERT	TOTAL
M	20-0-8	-2902	-2902	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 4-0-10
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 8-10-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.33/1.00 (G-H:1) , BC=0.66/1.00 (M-O:1) , WB=0.47/1.00 (H-M:1) , SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

LICENSED PROFESSIONAL ENGINEER 3578 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12293-1B p6/12

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T28	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 310.0 lbs FACTORED DOWN AT 24-10-6 ON TOP CHORD, AND 2902.1 lbs FACTORED DOWN AT 20-0-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.73 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

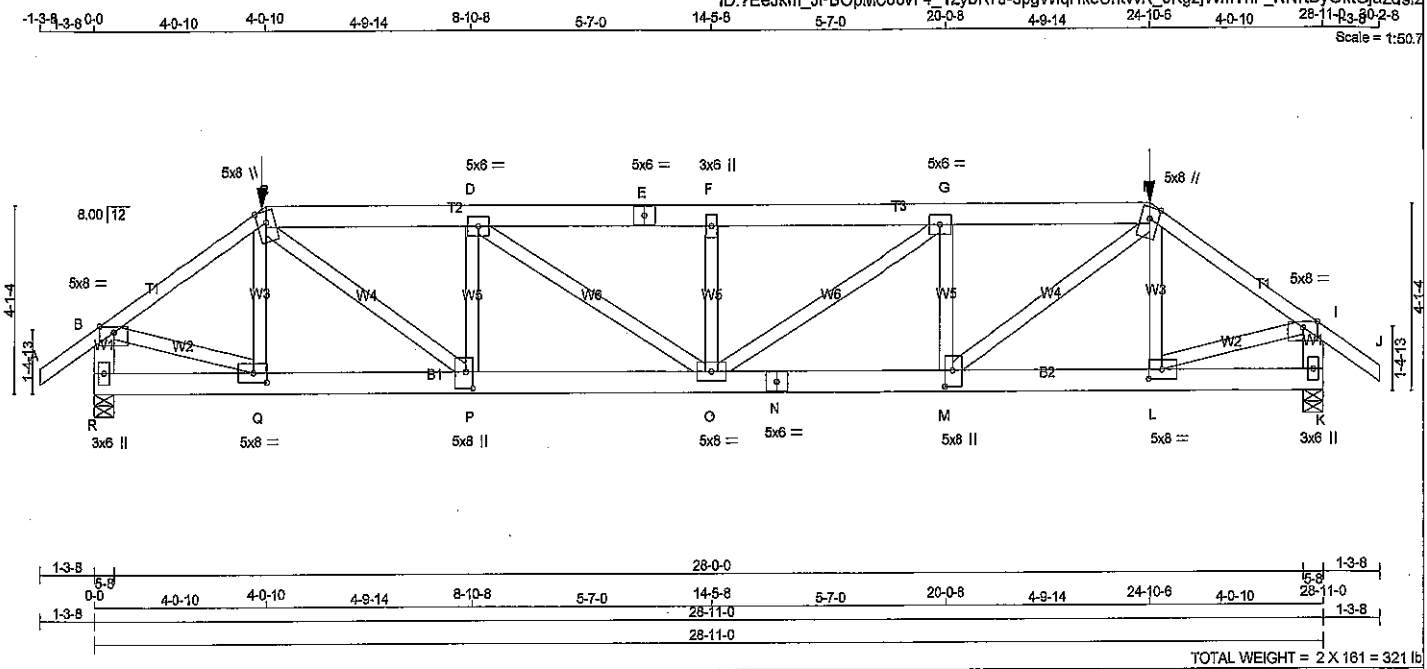
DWG NO. TAM 12243-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288647	T28Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:18:40 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-3pgWlqHkeUntWR_9Kg2jWmTnF_NNrtDyOktGjazdsfz



TOTAL WEIGHT = 2 X 161 = 321 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (in)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(0.0)
H- J	1	12	SIDE(0.0)
C- E	2	12	SIDE(44.6)
E- H	2	12	SIDE(44.6)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(20.3)
N- K	2	12	SIDE(20.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.75
M	BMWW+t	MT20	5.0	8.0	4.25	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	8.0	4.25	2.00
Q	BMWW-t	MT20	5.0	8.0	2.50	3.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3641	0	3641	0	0	5-8	5-8
K	3641	0	3641	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2875	1798 / 0	536 / 0	0 / 0	0 / 0	541 / 0	0 / 0
K	2875	1798 / 0	536 / 0	0 / 0	0 / 0	541 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	Q- C	-650 / 53	0.06 (1)	
B- C	-4187 / 0	-104.9	-104.9	0.25 (1)	4.44	C- P	0 / 3448	0.30 (1)	
C- D	-6264 / 0	-194.6	-194.6	0.23 (1)	4.56	P- D	-1817 / 0	0.16 (1)	
D- E	-7292 / 0	-194.6	-194.6	0.26 (1)	4.25	D- O	0 / 1229	0.11 (1)	
E- F	-7292 / 0	-194.6	-194.6	0.26 (1)	4.25	O- F	-1069 / 0	0.09 (1)	
F- G	-7292 / 0	-194.6	-194.6	0.26 (1)	4.25	O- G	0 / 1229	0.11 (1)	
G- H	-6264 / 0	-194.6	-194.6	0.23 (1)	4.56	M- G	-1817 / 0	0.16 (1)	
H- I	-4187 / 0	-104.9	-104.9	0.25 (1)	4.44	M- H	0 / 3448	0.30 (1)	
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00	L- H	-650 / 53	0.06 (1)	
R- B	-3556 / 0	0.0	0.0	0.13 (1)	7.43	B- Q	0 / 3607	0.32 (1)	
K- I	-3556 / 0	0.0	0.0	0.13 (1)	7.43	L- I	0 / 3607	0.32 (1)	
R- Q	0 / 0	-51.0	-51.0	0.05 (3)	10.00				
Q- P	0 / 3520	-51.0	-51.0	0.27 (1)	10.00				
P- O	0 / 6264	-51.0	-51.0	0.47 (1)	10.00				
O- N	0 / 6264	-51.0	-51.0	0.47 (1)	10.00				
N- M	0 / 6264	-51.0	-51.0	0.47 (1)	10.00				
M- L	0 / 3520	-51.0	-51.0	0.27 (1)	10.00				
L- K	0 / 0	-51.0	-51.0	0.05 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-10	-310	-310	—	FRONT	VERT	TOTAL
H	24-10-6	-310	-310	—	FRONT	VERT	TOTAL

DWG NO. TAM12261-18
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-0-10
END SETBACK = 5-10-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.26/1.00 (D-F:1) , BC=0.47/1.00 (O-P:1) , WB=0.32/1.00 (I-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (L) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288647	T28Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 310.0 lbs FACTORED DOWN AT 24-10-8, AND 310.0 lbs FACTORED DOWN AT 4-0-10 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
3378
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12261-1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T29	1	1	TRUSS DESC.	

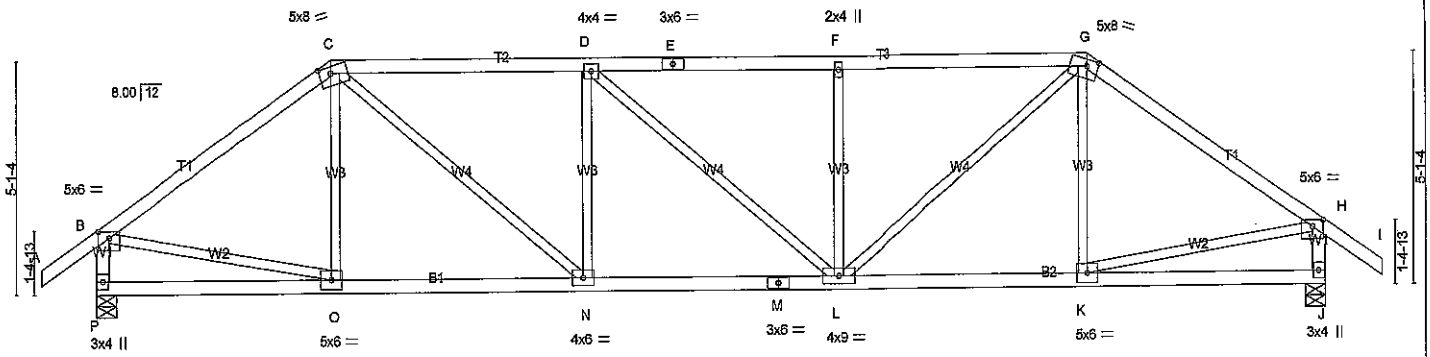
Tamarack Roof Truss, Burlington

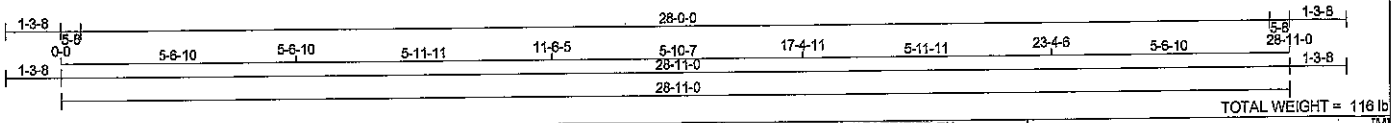
Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:51 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?ND?04h?BQbq8vkkytgE1RxHcEemsgak7cVAVvzdsgk

-1-3-8 1-3-8 0-0 5-6-10 5-6-10 5-11-11 11-6-5 5-10-7 17-4-11 5-11-11 23-4-6 5-6-10 28-11-0 30-2-8

Scale = 1:50.7





TOTAL WEIGHT = 116 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2183 / 0	-104.9	-104.9	0.77 (1)	3.72
C- D	-2844 / 0	-104.9	-104.9	0.74 (1)	3.38
D- E	-2843 / 0	-104.9	-104.9	0.73 (1)	3.38
E- F	-2843 / 0	-104.9	-104.9	0.73 (1)	3.38
F- G	-2843 / 0	-104.9	-104.9	0.73 (1)	3.39
G- H	-2183 / 0	-104.9	-104.9	0.77 (1)	3.72
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
P- B	-1997 / 0	0.0	0.0	0.21 (1)	5.97
J- H	-1997 / 0	0.0	0.0	0.21 (1)	5.97
P- O	0 / 0	-27.5	-27.5	0.22 (3)	10.00
O- N	0 / 1811	-27.5	-27.5	0.41 (2)	10.00
N- M	0 / 2844	-27.5	-27.5	0.54 (1)	10.00
M- L	0 / 2844	-27.5	-27.5	0.54 (1)	10.00
L- K	0 / 1811	-27.5	-27.5	0.41 (1)	10.00
K- J	0 / 0	-27.5	-27.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-166 / 159	0.06 (1)
C- N	0 / 1326	0.30 (1)
N- D	-673 / 0	0.26 (1)
D- L	-2 / 0	0.00 (1)
L- F	-672 / 0	0.26 (1)
L- G	0 / 1324	0.30 (1)
K- G	-167 / 159	0.06 (1)
B- O	0 / 1847	0.42 (1)
K- H	0 / 1847	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.77/1.00 (G-H:1) , BC=0.54/1.00 (L-N:1) , WB=0.42/1.00 (H-K:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.76 (M) (INPUT = 1.00)

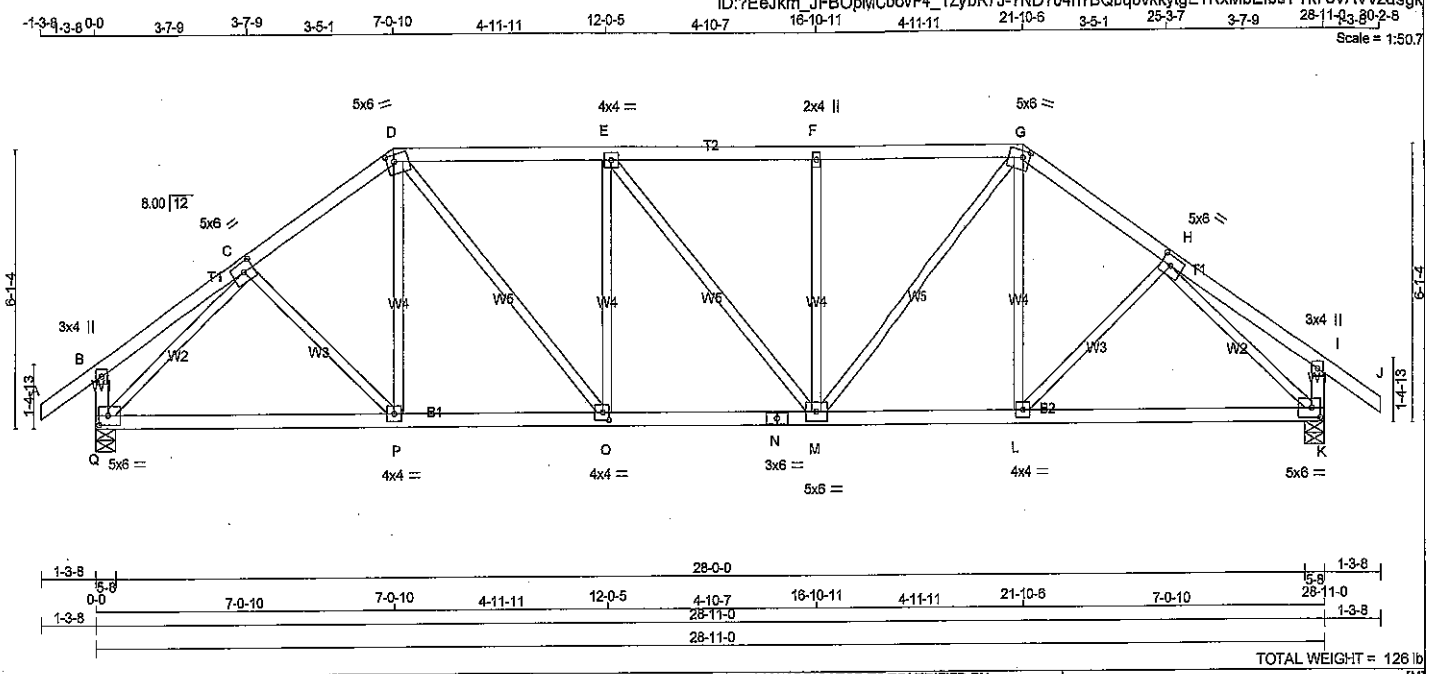
DWG NO. TAM 12274-1B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:51 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-?ND?04h?BQbq8vkkytgE1RxMbEfbsYYk7cVAVvzdsgk



TOTAL WEIGHT = 126 lb

[M]

LUMBER

N. L., G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2059	0	2059	0	0	5-8	5-8
K	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1026 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1615	1026 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- P	0 / 148	0.03 (3)
B- C	0 / 21	-104.9	-104.9	0.18 (1)	10.00	P- D	0 / 250	0.06 (2)
C- D	-2165 / 0	-104.9	-104.9	0.25 (1)	4.39	D- O	0 / 889	0.20 (1)
D- E	-2361 / 0	-104.9	-104.9	0.45 (1)	4.00	O- E	-557 / 0	0.33 (1)
E- F	-2360 / 0	-104.9	-104.9	0.45 (1)	4.00	E- M	-3 / 0	0.00 (1)
F- G	-2360 / 0	-104.9	-104.9	0.44 (1)	4.02	M- F	-556 / 0	0.33 (1)
G- H	-2165 / 0	-104.9	-104.9	0.25 (1)	4.39	M- G	0 / 865	0.20 (1)
H- I	0 / 21	-104.9	-104.9	0.18 (1)	10.00	L- G	0 / 250	0.06 (2)
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	L- H	0 / 148	0.03 (3)
Q- B	-291 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2417 / 0	0.93 (1)
K- I	-291 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2417 / 0	0.93 (1)
Q- P	0 / 1731	-27.5	-27.5	0.47 (2)	10.00			
P- O	0 / 1783	-27.5	-27.5	0.49 (2)	10.00			
O- N	0 / 2362	-27.5	-27.5	0.45 (1)	10.00			
N- M	0 / 2362	-27.5	-27.5	0.45 (1)	10.00			
M- L	0 / 1783	-27.5	-27.5	0.49 (2)	10.00			
L- K	0 / 1731	-27.5	-27.5	0.47 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.49/1.00 (L-M:2) , WB=0.93/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (K) (INPUT = 0.90)
USI METAL= 0.63 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3528 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12245-TB
STRUCTURAL COMPONENT ONLY

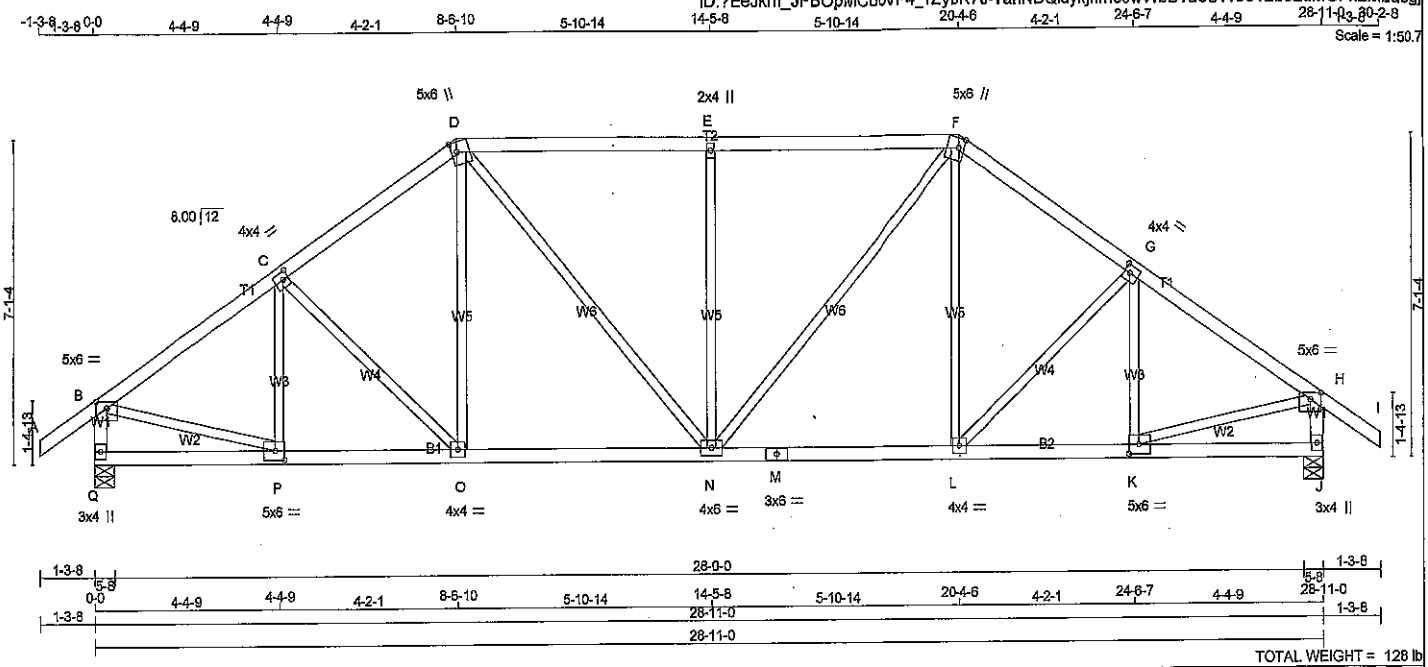
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:52 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-TanNDQidykjhm3JwWbBTaeUW0e1Zb3zuMGFk2Mzdsgj

Scale = 1:50.7



[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2184 / 0	-104.9	-104.9	0.30 (1)	4.35
C- D	-2079 / 0	-104.9	-104.9	0.29 (1)	4.44
D- E	-2107 / 0	-104.9	-104.9	0.54 (1)	4.08
E- F	-2107 / 0	-104.9	-104.9	0.54 (1)	4.08
F- G	-2079 / 0	-104.9	-104.9	0.29 (1)	4.44
G- H	-2184 / 0	-104.9	-104.9	0.30 (1)	4.35
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-2005 / 0	0.0	0.0	0.21 (1)	5.96
J- H	-2005 / 0	0.0	0.0	0.21 (1)	5.96
Q- P	0 / 0	-27.5	-27.5	0.11 (2)	10.00
P- O	0 / 1840	-27.5	-27.5	0.38 (1)	10.00
O- N	0 / 1707	-27.5	-27.5	0.38 (2)	10.00
N- M	0 / 1707	-27.5	-27.5	0.38 (2)	10.00
M- L	0 / 1707	-27.5	-27.5	0.38 (2)	10.00
L- K	0 / 1840	-27.5	-27.5	0.38 (1)	10.00
K- J	0 / 0	-27.5	-27.5	0.11 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-337 / 30	0.09 (1)
C- O	-189 / 0	0.11 (1)
O- D	0 / 313	0.07 (2)
D- N	0 / 611	0.14 (1)
N- E	-758 / 0	0.66 (1)
N- F	0 / 611	0.14 (1)
L- F	0 / 313	0.07 (2)
L- G	-189 / 0	0.11 (1)
K- G	-337 / 30	0.09 (1)
B- P	0 / 1898	0.43 (1)
K- H	0 / 1898	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.38/1.00 (K-L:1) , WB=0.66/1.00 (E-N:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

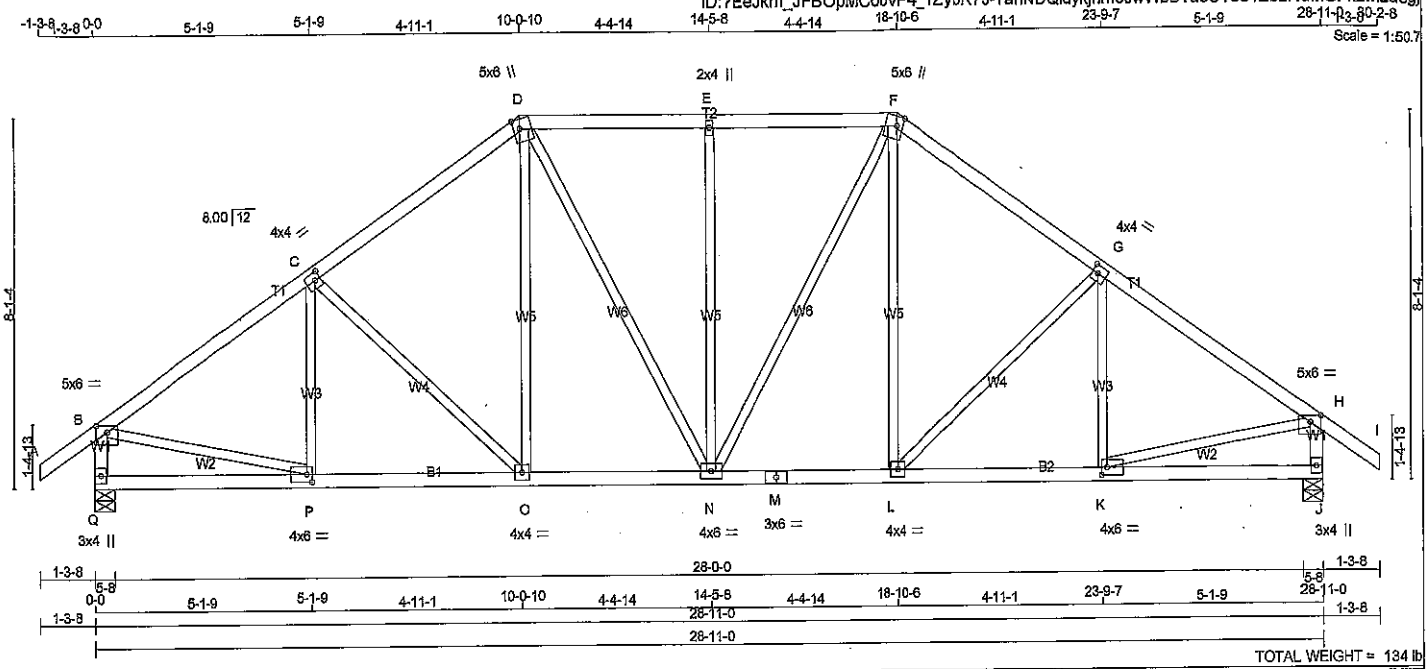
DWG NO. TAM 12246-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:52 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-TanNDQidykjhm3JwWbBTaeUYse1Zb2HuMGFk2Mzdsgj



TOTAL WEIGHT = 134 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2059	0	2059	0	0	5-8	5-8
J	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2222 / 0	-104.9	-104.9	0.42 (1)	4.19
C- D	-1976 / 0	-104.9	-104.9	0.39 (1)	4.41
D- E	-1806 / 0	-104.9	-104.9	0.29 (1)	4.69
E- F	-1806 / 0	-104.9	-104.9	0.29 (1)	4.69
F- G	-1976 / 0	-104.9	-104.9	0.39 (1)	4.41
G- H	-2222 / 0	-104.9	-104.9	0.42 (1)	4.19
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1999 / 0	0.0	0.0	0.21 (1)	5.97
J- H	-1999 / 0	0.0	0.0	0.21 (1)	5.97
Q- P	0 / 0	-27.5	-27.5	0.17 (3)	10.00
P- O	0 / 1877	-27.5	-27.5	0.36 (1)	10.00
O- N	0 / 1617	-27.5	-27.5	0.33 (1)	10.00
N- M	0 / 1617	-27.5	-27.5	0.33 (1)	10.00
M- L	0 / 1617	-27.5	-27.5	0.33 (1)	10.00
L- K	0 / 1877	-27.5	-27.5	0.38 (1)	10.00
K- J	0 / 0	-27.5	-27.5	0.17 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-243 / 111	0.08 (1)
C- O	-355 / 0	0.30 (1)
O- D	0 / 367	0.08 (1)
D- N	0 / 384	0.09 (1)
N- E	-558 / 0	0.71 (1)
N- F	0 / 384	0.09 (1)
L- F	0 / 367	0.08 (1)
L- G	-355 / 0	0.30 (1)
K- G	-243 / 111	0.08 (1)
B- P	0 / 1918	0.43 (1)
K- H	0 / 1918	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.38/1.00 (K-L:1) , WB=0.71/1.00 (E-N:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

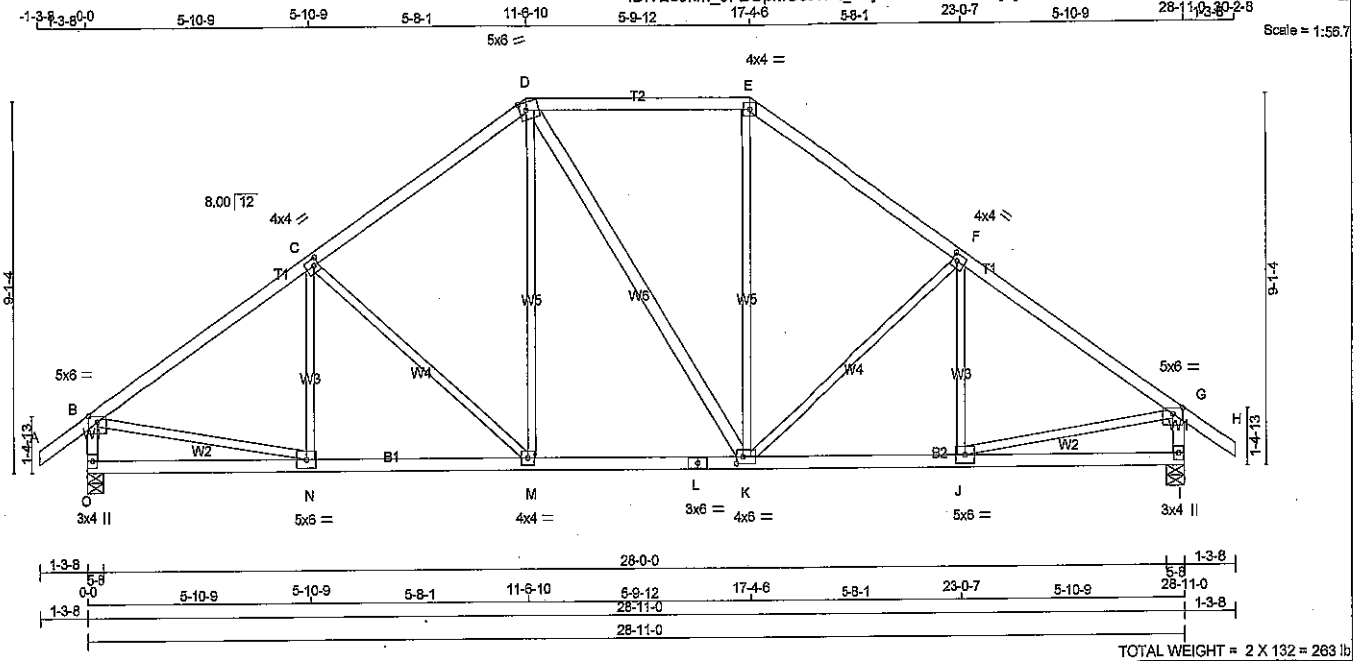
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 122477-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:52 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-TanNDQidykjhm3JwWbBTaeUWae0yb42uMGFk2Mzdsgj



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-I	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	1.75	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2059	0	2059	0	0	5-8	5-8
I	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
I	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	N- C	-172 / 169	0.07 (1)
B- C	-2236 / 0	-104.9	-104.9	0.56 (1)	3.99	C- M	-504 / 0	0.59 (1)
C- D	-1861 / 0	-104.9	-104.9	0.51 (1)	4.35	M- D	0 / 483	0.11 (1)
D- E	-1519 / 0	-104.9	-104.9	0.49 (1)	4.73	D- K	0 / 1	0.00 (1)
E- F	-1862 / 0	-104.9	-104.9	0.52 (1)	4.35	K- E	0 / 484	0.11 (1)
F- G	-2236 / 0	-104.9	-104.9	0.56 (1)	3.99	K- F	-503 / 0	0.59 (1)
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00	J- F	-173 / 168	0.07 (1)
O- B	-1992 / 0	0.0	0.0	0.21 (1)	5.97	B- N	0 / 1925	0.43 (1)
I- G	-1991 / 0	0.0	0.0	0.21 (1)	5.97	J- G	0 / 1925	0.43 (1)
O- N	0 / 0	-27.5	-27.5	0.23 (3)	10.00			
N- M	0 / 1893	-27.5	-27.5	0.42 (2)	10.00			
M- L	0 / 1518	-27.5	-27.5	0.34 (1)	10.00			
L- K	0 / 1518	-27.5	-27.5	0.34 (1)	10.00			
K- J	0 / 1893	-27.5	-27.5	0.42 (2)	10.00			
J- I	0 / 0	-27.5	-27.5	0.22 (3)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.42/1.00 (M-N:2) , WB=0.59/1.00 (C-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES -

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3518 S. KATSOULAKOS PROVINCE OF ONTARIO

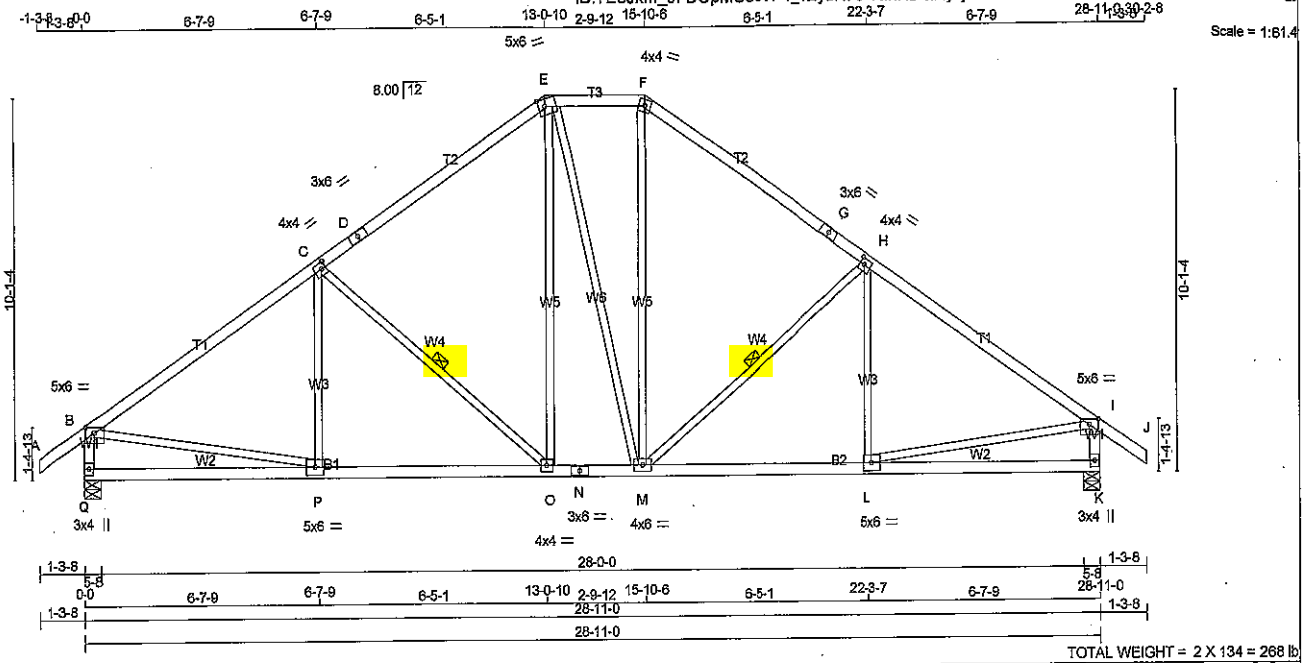
DWG NO. TAM-1224B.17B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T34	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:52 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-TanNDQidykjhm3JwWbBTaeUTme?db7ZuMGFk2Mzdsgj



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
K	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

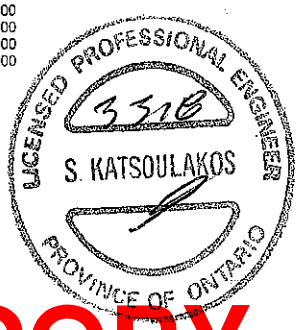
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2236 / 0	-104.9	-104.9	0.74 (1)	3.72
C- D	-1730 / 0	-104.9	-104.9	0.66 (1)	4.24
D- E	-1730 / 0	-104.9	-104.9	0.66 (1)	4.24
E- F	-1407 / 0	-104.9	-104.9	0.13 (1)	5.38
F- G	-1732 / 0	-104.9	-104.9	0.66 (1)	4.23
G- H	-1732 / 0	-104.9	-104.9	0.66 (1)	4.23
H- I	-2236 / 0	-104.9	-104.9	0.74 (1)	3.72
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1985 / 0	0.0	0.0	0.20 (1)	5.98
K- I	-1985 / 0	0.0	0.0	0.20 (1)	5.98
Q- P	0 / 0	-27.5	-27.5	0.31 (3)	10.00
P- O	0 / 1899	-27.5	-27.5	0.50 (2)	10.00
O- N	0 / 1405	-27.5	-27.5	0.32 (1)	10.00
N- M	0 / 1405	-27.5	-27.5	0.32 (1)	10.00
M- L	0 / 1898	-27.5	-27.5	0.50 (2)	10.00
L- K	0 / 0	-27.5	-27.5	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-101 / 239	0.05 (3)
C- O	-653 / 0	0.33 (1)
O- E	0 / 530	0.12 (1)
E- M	0 / 7	0.00 (1)
M- F	0 / 537	0.12 (1)
M- H	-650 / 0	0.32 (1)
L- H	-104 / 236	0.05 (1)
B- P	0 / 1924	0.43 (1)
L- I	0 / 1923	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.74/1.00 (B-C:1) , BC=0.50/1.00 (O-P:2) , WB=0.43/1.00 (B-P:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

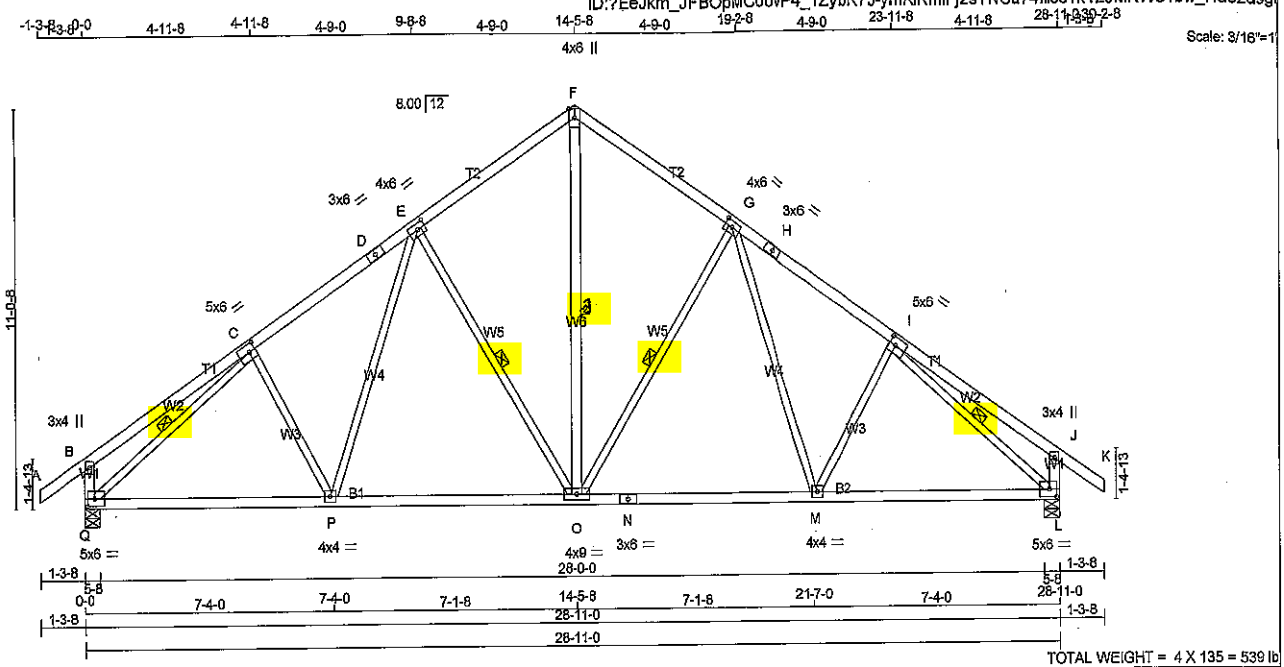
DWG NO. TAM 12249-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T35	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:53 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ymKlRmiFj2sYNCu74Iii6s1k42JMKWe1bw_Haozdsgi

Scale: 3/16"=1'



TOTAL WEIGHT = 4 X 135 = 539 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	2.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2059	0	2059	0	0	5-8	5-8
L	2059	0	2059	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1615	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, E-O, C-Q, I-L. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.32 (1)	10.00
C- D	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
D- E	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
E- F	-1591 / 0	-104.9	-104.9	0.30 (1)	4.94
F- G	-1591 / 0	-104.9	-104.9	0.30 (1)	4.94
G- H	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
H- I	-2121 / 0	-104.9	-104.9	0.32 (1)	4.39
I- J	0 / 26	-104.9	-104.9	0.32 (1)	10.00
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-350 / 0	0.0	0.0	0.04 (1)	7.81
L- J	-350 / 0	0.0	0.0	0.04 (1)	7.81
Q- P	0 / 1827	-27.5	-27.5	0.53 (2)	10.00
P- O	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
O- N	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
N- M	0 / 1656	-27.5	-27.5	0.52 (2)	10.00
M- L	0 / 1827	-27.5	-27.5	0.53 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1344	0.22 (1)
O- G	-678 / 0	0.37 (1)
G- M	0 / 370	0.08 (2)
M- I	-126 / 103	0.05 (1)
E- O	-678 / 0	0.37 (1)
P- E	0 / 370	0.08 (2)
C- P	-126 / 103	0.05 (1)
Q- C	-2447 / 0	0.70 (1)
I- L	-2447 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.32/1.00 (G-I:1) , BC=0.53/1.00 (P-Q:2) , WB=0.70/1.00 (C-Q:1) , SSI=0.20/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

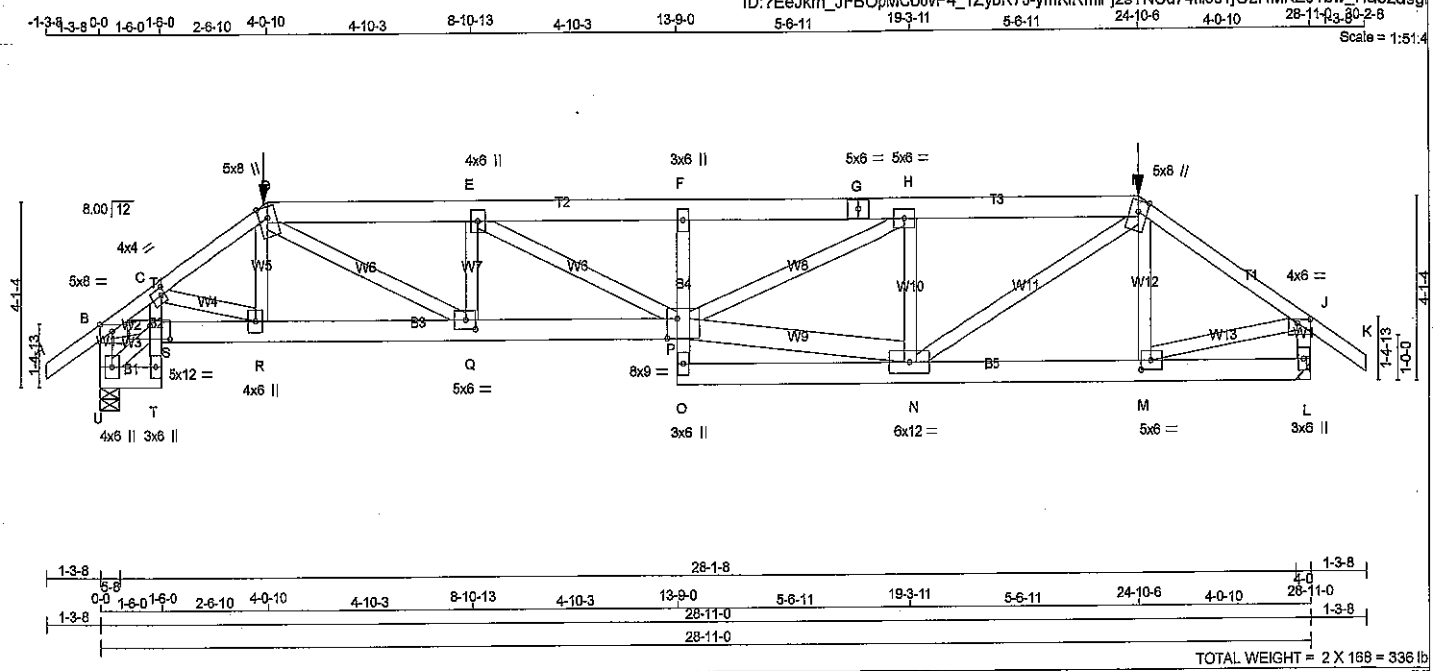
LICENSED PROFESSIONAL ENGINEER
3526
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM NLS 0-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T36	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:53 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ymKlRmiFj2sYNCu74lli6s1jU2HMKZJ1bw_Haozdsgi



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x6	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x8	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(0.0)
I- K	1	12	SIDE(0.0)
U- B	1	12	TOP
L- J	1	12	TOP
D- G	2	12	SIDE(50.8)
G- I	2	12	SIDE(50.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- T	2	12	SIDE(23.0)
S- P	2	12	SIDE(23.0)
O- L	2	12	SIDE(23.0)
T- C	1	12	TOP
F- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMWW+t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	5.0	8.0	Edge	
J	TMVW-p	MT20	4.0	6.0	1.00	3.50
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	3808	0	3808	0	0	IN-SX	IN-SX
						HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
U	3815	0	3815	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3014	1873 / 0	569 / 0	0 / 0	0 / 0	572 / 0	0 / 0
U	3018	1877 / 0	569 / 0	0 / 0	0 / 0	572 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C		-6373 / 0	-104.9	-104.9	0.21 (1)	3.72
C- D		-6064 / 0	-104.9	-104.9	0.25 (1)	3.77
D- E		-9015 / 0	-206.6	-206.6	0.31 (1)	3.84
E- F		-10475 / 0	-206.6	-206.6	0.34 (1)	3.56
F- G		-10367 / 0	-206.6	-206.6	0.43 (1)	3.48
G- H		-10367 / 0	-206.6	-206.6	0.43 (1)	3.48
H- I		-6857 / 0	-206.6	-206.6	0.32 (1)	4.29
I- J		-4405 / 0	-104.9	-104.9	0.26 (1)	4.34
J- K		0 / 40	-104.9	-104.9	0.08 (1)	10.00
U- B		-3567 / 0	0.0	0.0	0.20 (1)	6.21
L- J		-3722 / 0	0.0	0.0	0.21 (1)	6.10
U- T		0 / 267	-54.1	-54.1	0.02 (1)	10.00
T- S		0 / 63	0.0	0.0	0.22 (1)	10.00
S- C		0 / 300	0.0	0.0	0.24 (1)	10.00
S- R		0 / 5579	-54.1	-54.1	0.40 (1)	10.00
R- Q		0 / 5071	-54.1	-54.1	0.39 (1)	10.00
Q- P		0 / 9014	-54.1	-54.1	0.66 (1)	10.00
O- P		0 / 214	0.0	0.0	0.23 (1)	10.00
P- F		-942 / 0	0.0	0.0	0.22 (1)	7.81
O- N		0 / 285	-54.1	-54.1	0.11 (3)	10.00
N- M		0 / 3702	-54.1	-54.1	0.29 (1)	10.00
M- L		0 / 0	-54.1	-54.1	0.06 (3)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R		-520 / 0	0.03 (1)
R- D		0 / 438	0.04 (2)
D- Q		0 / 4503	0.40 (1)
Q- E		-1956 / 0	0.13 (1)
E- P		0 / 1653	0.15 (1)
P- N		0 / 6798	0.44 (1)
P- H		0 / 3775	0.33 (1)
N- H		-3028 / 0	0.26 (1)
N- I		0 / 3811	0.34 (1)
M- I		-666 / 77	0.06 (1)
M- J		0 / 3794	0.34 (1)
U- S		-340 / 0	0.02 (1)
B- S		0 / 5240	0.46 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-0-10	-310	-310	---	FRONT	VERT	TOTAL
I	24-10-6	-310	-310	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-0-10
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 874 (0.40")

CSI: TC=0.43/1.00 (F-H:1) , BC=0.66/1.00 (P-Q:1) , WB=0.46/1.00 (B-S:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

P6 1/2

CONTINUED ON PAGE 2

DWG NO. TAM 12251-T36
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T36	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWWW-t	MT20	6.0	12.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWWW-I	MT20	8.0	9.0	5.25	2.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMWW+t	MT20	4.0	6.0		
S	BVMWW-I	MT20	5.0	12.0	3.50	6.00
T	BMV+p	MT20	3.0	6.0		
U	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 310.0 lbs FACTORED DOWN AT 24-10-6, AND 310.0 lbs FACTORED DOWN AT 4-0-10 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.53 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3528
S. KATSOULAKOS
PROVINCE OF ONTARIO

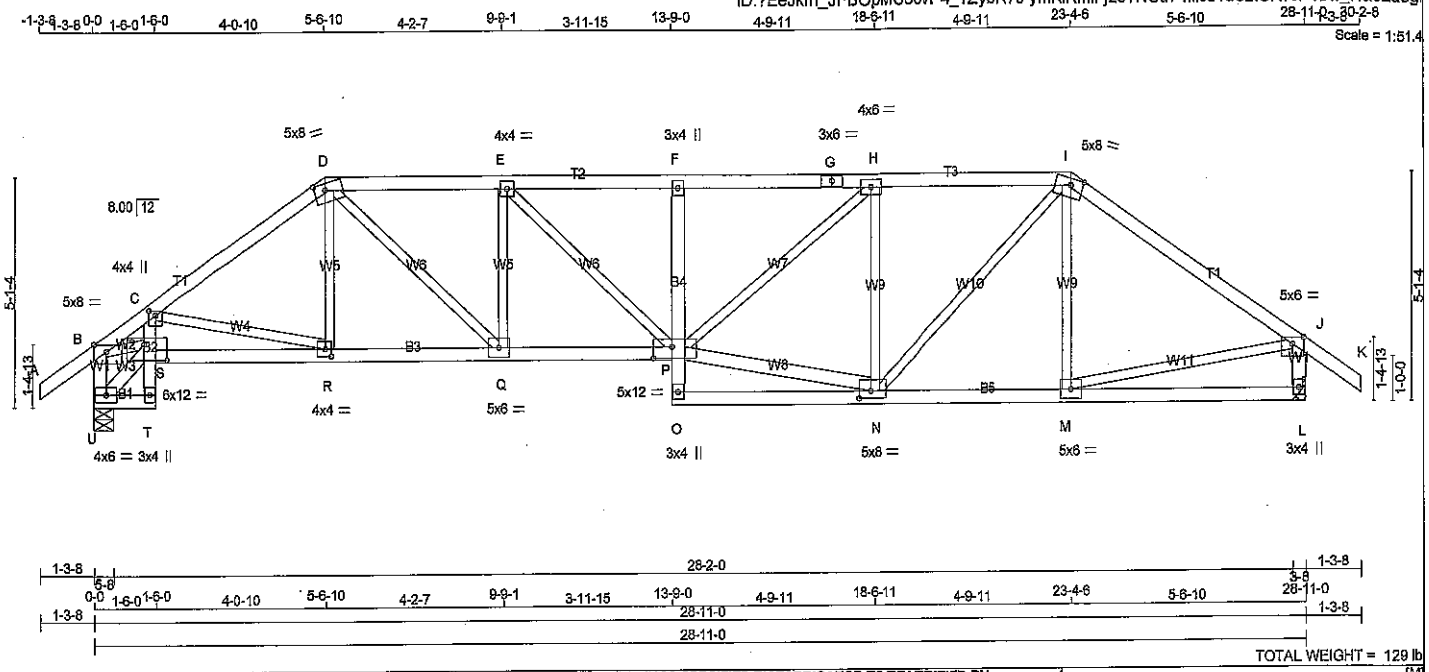
DWG NO. TAM 12151 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:53 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ymKlRmiFj2sYNCu74lii6s1d82ISKWP1bw_Haozdsgi



[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.50
J	TMVW-p	MT20	5.0	6.0	1.75	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.00	3.25
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	5.0	12.0	3.00	5.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.00	1.75
S	BVMWW-I	MT20	6.0	12.0	4.25	7.00
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2062	0	2062	0	0	5-8	5-8
L	2055	0	2055	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1617	1030 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1613	1026 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-3475 / 0	-104.9	-104.9	0.26 (1)	3.55
C- D	-2715 / 0	-104.9	-104.9	0.43 (1)	3.61
D- E	-3281 / 0	-104.9	-104.9	0.40 (1)	3.53
E- F	-3642 / 0	-104.9	-104.9	0.44 (1)	3.33
F- G	-3616 / 0	-104.9	-104.9	0.58 (1)	3.19
G- H	-3616 / 0	-104.9	-104.9	0.58 (1)	3.19
H- I	-2686 / 0	-104.9	-104.9	0.46 (1)	3.75
I- J	-2175 / 0	-104.9	-104.9	0.77 (1)	3.74
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
U- B	-1909 / 0	0.0	0.0	0.20 (1)	6.08
L- J	-1991 / 0	0.0	0.0	0.21 (1)	5.98
U- T	0 / 151	-27.5	-27.5	0.03 (1)	10.00
T- S	0 / 32	0.0	0.0	0.25 (1)	10.00
S- C	0 / 343	0.0	0.0	0.30 (1)	10.00
S- R	0 / 3109	-27.5	-27.5	0.57 (1)	10.00
R- Q	0 / 2256	-27.5	-27.5	0.43 (1)	10.00
Q- P	0 / 3281	-27.5	-27.5	0.59 (1)	10.00
O- P	0 / 93	0.0	0.0	0.14 (1)	10.00
P- F	-420 / 0	0.0	0.0	0.15 (1)	7.81
O- N	0 / 99	-27.5	-27.5	0.14 (2)	10.00
N- M	0 / 1803	-27.5	-27.5	0.39 (1)	10.00
M- L	0 / 0	-27.5	-27.5	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-877 / 0	0.29 (1)
R- D	0 / 247	0.06 (2)
D- Q	0 / 1381	0.31 (1)
Q- E	-814 / 0	0.21 (1)
E- P	0 / 491	0.11 (1)
P- N	0 / 2664	0.43 (1)
P- H	0 / 1176	0.26 (1)
N- H	-1313 / 0	0.50 (1)
N- I	0 / 1257	0.28 (1)
M- I	-185 / 130	0.07 (1)
M- J	0 / 1840	0.41 (1)
U- S	-202 / 0	0.02 (1)
B- S	0 / 2884	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.77/1.00 (I-J:1) , BC=0.59/1.00 (P-Q:1) , WB=0.65/1.00 (B-S:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12252-17B
STRUCTURAL COMPONENT ONLY

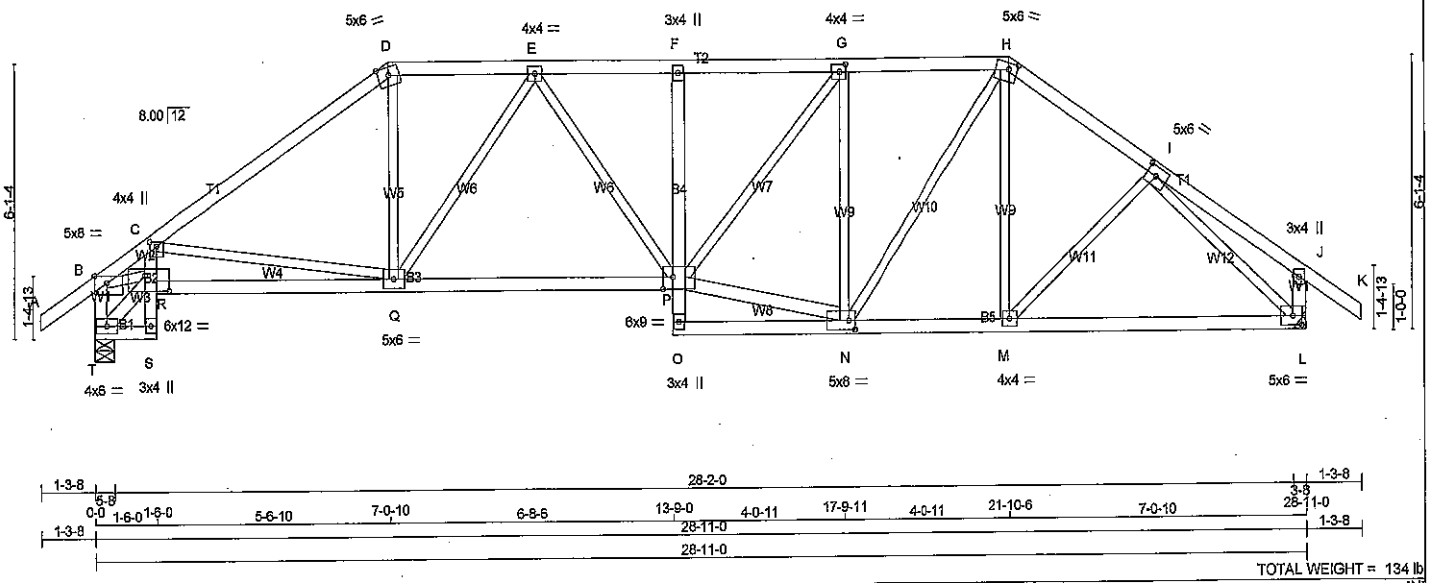
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T38	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:54 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-Qyu7e6jtUL_P?MTJe0Dxf3ZpWScu3vJApakr6Ezdsgh

Scale = 1:51.4



TOTAL WEIGHT = 134 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTWW-m	MT20	5.0	6.0	1.75	2.25
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	1.75
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-t	MT20	6.0	9.0	3.25	2.75
Q	BMWWW-t	MT20	5.0	6.0		
R	BVMWW-t	MT20	6.0	12.0	4.25	7.25
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2062	0	2062	0	0	5-8	5-8
L	2055	0	2055	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1617	1030 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1613	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

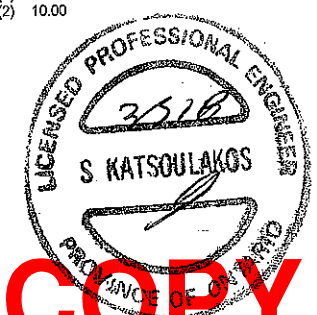
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-3642 / 0	-104.9	-104.9	0.41 (1)	3.27	
C- D	-2586 / 0	-104.9	-104.9	0.73 (1)	3.63	
D- E	-2168 / 0	-104.9	-104.9	0.22 (1)	4.43	
E- F	-2877 / 0	-104.9	-104.9	0.26 (1)	3.88	
F- G	-2865 / 0	-104.9	-104.9	0.36 (1)	3.78	
G- H	-2273 / 0	-104.9	-104.9	0.32 (1)	4.22	
H- I	-2159 / 0	-104.9	-104.9	0.25 (1)	4.40	
I- J	0 / 21	-104.9	-104.9	0.18 (1)	10.00	
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
T- B	-1902 / 0	0.0	0.0	0.20 (1)	6.09	
L- J	-291 / 0	0.0	0.0	0.03 (1)	7.81	
T- S	0 / 159	-27.5	-27.5	0.03 (1)	10.00	
S- R	0 / 32	0.0	0.0	0.26 (1)	10.00	
R- C	0 / 333	0.0	0.0	0.31 (1)	10.00	
R- Q	0 / 3312	-27.5	-27.5	0.71 (1)	10.00	
Q- P	0 / 2654	-27.5	-27.5	0.60 (1)	10.00	
O- P	0 / 85	0.0	0.0	0.09 (1)	10.00	
P- F	-348 / 0	0.0	0.0	0.12 (1)	7.81	
O- N	0 / 62	-27.5	-27.5	0.11 (2)	10.00	
N- M	0 / 1778	-27.5	-27.5	0.48 (2)	10.00	
M- L	0 / 1727	-27.5	-27.5	0.47 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-1194 / 0	0.72 (1)
Q- D	0 / 1007	0.23 (1)
P- N	0 / 2294	0.37 (1)
P- G	0 / 918	0.21 (1)
N- G	-1189 / 0	0.70 (1)
N- H	0 / 872	0.20 (1)
M- H	0 / 240	0.05 (2)
M- I	0 / 148	0.03 (3)
I- L	-2411 / 0	0.93 (1)
T- R	-212 / 0	0.02 (1)
B- R	0 / 3078	0.69 (1)
Q- E	-861 / 0	0.51 (1)
E- P	0 / 389	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.73/1.00 (C-D:1) , BC=0.71/1.00 (Q-R:1) , WB=0.93/1.00 (I-L:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

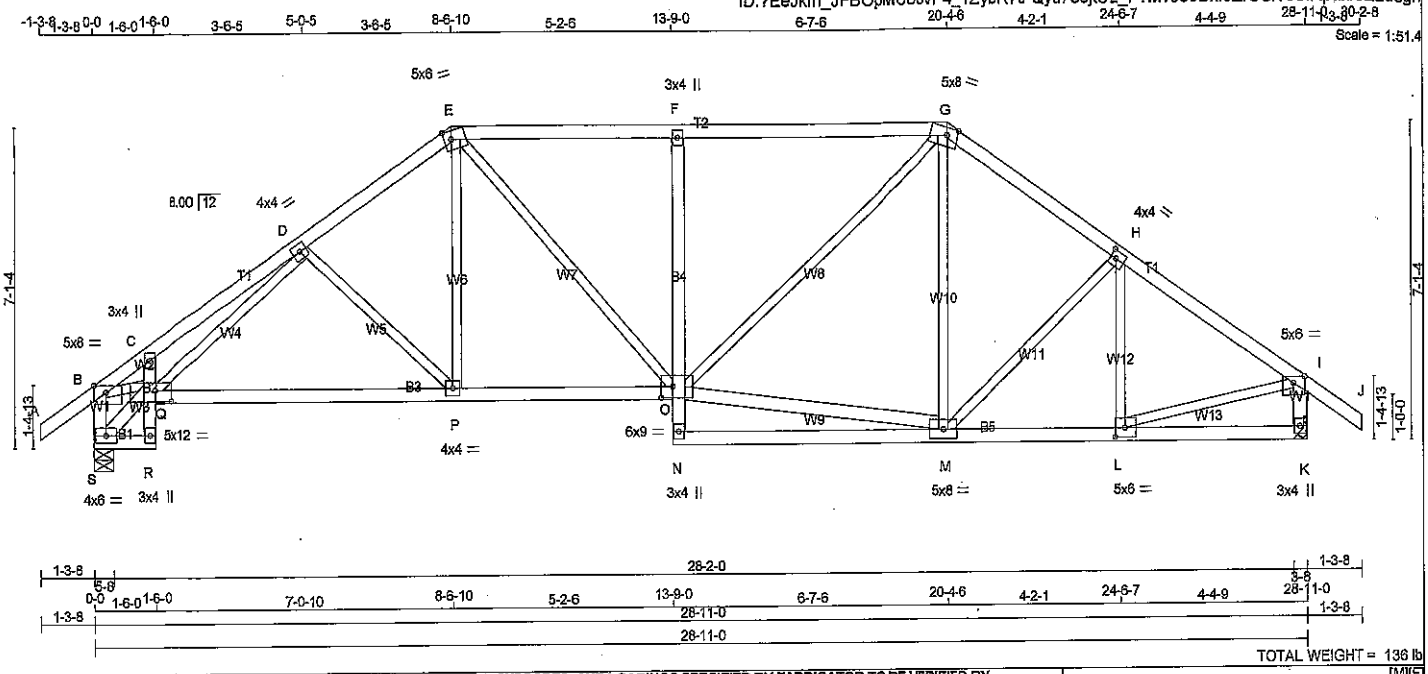
JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

DWG NO. TAM 12253-T8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:54 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-Qyu7e6jtUL_P?MTJe0Dxf3ZrCSf730rApakr6Ezdsgh



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	3.25
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	5.0	8.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-I	MT20	6.0	9.0	3.00	3.25
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-I	MT20	5.0	12.0	3.00	4.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2062	0	2062	0	0	5-8	5-8
K	2055	0	2055	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL.
S	1617	1030 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	1613	1028 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

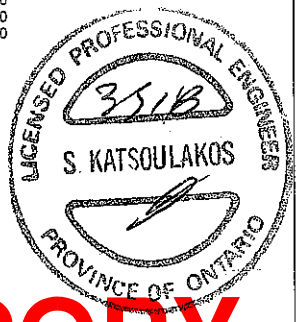
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B-C	-2775 / 0	-104.9	-104.9	0.18 (1)	4.03	
C-D	-2820 / 0	-104.9	-104.9	0.27 (1)	3.93	
D-E	-2425 / 0	-104.9	-104.9	0.25 (1)	4.22	
E-F	-2446 / 0	-104.9	-104.9	0.56 (1)	3.76	
F-G	-2438 / 0	-104.9	-104.9	0.62 (1)	3.73	
G-H	-2084 / 0	-104.9	-104.9	0.29 (1)	4.44	
H-I	-2176 / 0	-104.9	-104.9	0.30 (1)	4.35	
I-J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
S-B	-1958 / 0	0.0	0.0	0.20 (1)	6.02	
K-I	-1999 / 0	0.0	0.0	0.21 (1)	5.97	
S-R	0 / 116	-27.5	-27.5	0.03 (1)	10.00	
R-Q	0 / 32	0.0	0.0	0.16 (1)	10.00	
Q-C	-164 / 2	0.0	0.0	0.12 (1)	7.81	
Q-P	0 / 2316	-27.5	-27.5	0.57 (2)	10.00	
P-O	0 / 2002	-27.5	-27.5	0.55 (2)	10.00	
N-O	0 / 125	0.0	0.0	0.08 (1)	10.00	
O-F	-761 / 0	0.0	0.0	0.46 (1)	7.81	
N-M	0 / 50	-27.5	-27.5	0.25 (2)	10.00	
M-L	0 / 1834	-27.5	-27.5	0.44 (1)	10.00	
L-K	0 / 0	-27.5	-27.5	0.12 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-D	0 / 121	0.03 (2)
D-P	-433 / 0	0.18 (1)
P-E	0 / 507	0.11 (1)
E-O	0 / 855	0.15 (1)
O-M	0 / 1676	0.27 (1)
O-G	0 / 981	0.22 (1)
M-G	0 / 222	0.05 (3)
M-H	-176 / 0	0.10 (1)
L-H	-352 / 2	0.10 (1)
L-I	0 / 1889	0.43 (1)
S-Q	-145 / 0	0.02 (1)
B-Q	0 / 2265	0.51 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.62/1.00 (F-G:1) , BC=0.57/1.00 (P-Q:2) , WB=0.51/1.00 (B-Q:1) , SSI=0.32/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

DWG NO. TAM 12254-T39
STRUCTURAL COMPONENT ONLY

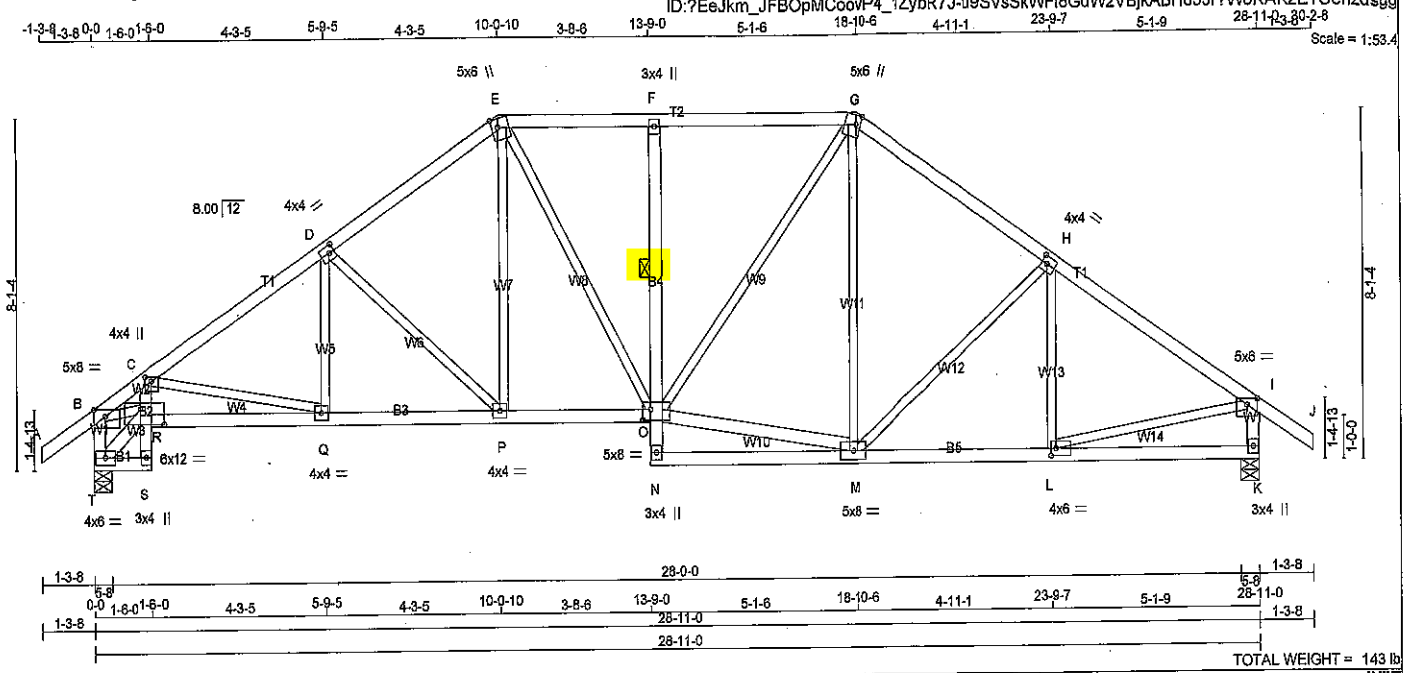
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:55 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-u9SVsSkWFf6GdW2VBjkABH635r?WoRAK2ETOehzdsgg

Scale = 1:53.4



TOTAL WEIGHT = 143 lb

[MII[F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	3.50
C	TMW+p	MT20	4.0	4.0	1.25	2.00
D	TMW-t	MT20	4.0	4.0	2.00	1.50
E	TTW+m	MT20	5.0	6.0	Edge	1.75
F	TMV+p	MT20	3.0	4.0		
G	TTW+m	MT20	5.0	6.0	2.50	1.50
H	TMW-t	MT20	4.0	4.0	2.00	1.50
I	TMW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMW-t	MT20	4.0	6.0	2.00	1.50
M	BMWW-t	MT20	5.0	8.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-t	MT20	5.0	8.0	3.00	2.25
P	BMW-t	MT20	4.0	4.0		
Q	BMW-t	MT20	4.0	4.0		
R	BVMW-t	MT20	6.0	12.0	4.25	7.00
S	BMV+p	MT20	3.0	4.0		
T	BMW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2062	0	2062	0	0	5-8	5-8
K	2055	0	2055	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1617	1030 / 0	289 / 0	0 / 0	0 / 0	298 / 0	0 / 0
K	1613	1026 / 0	289 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B-C	-3412 / 0	-104.9	-104.9	0.22 (1)	3.64
C-D	-2733 / 0	-104.9	-104.9	0.36 (1)	3.90
D-E	-2241 / 0	-104.9	-104.9	0.32 (1)	4.28
E-F	-2057 / 0	-104.9	-104.9	0.31 (1)	4.43
F-G	-2053 / 0	-104.9	-104.9	0.34 (1)	4.40
G-H	-1978 / 0	-104.9	-104.9	0.40 (1)	4.41
H-I	-2216 / 0	-104.9	-104.9	0.42 (1)	4.19
I-J	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T-B	-1912 / 0	0.0	0.0	0.20 (1)	6.08
K-I	-1994 / 0	0.0	0.0	0.21 (1)	5.97
T-S	0 / 148	-27.5	-27.5	0.03 (1)	10.00
S-R	0 / 32	0.0	0.0	0.25 (1)	10.00
R-C	0 / 336	0.0	0.0	0.30 (1)	10.00
R-Q	0 / 3031	-27.5	-27.5	0.55 (1)	10.00
Q-P	0 / 2300	-27.5	-27.5	0.43 (1)	10.00
P-O	0 / 1845	-27.5	-27.5	0.35 (1)	10.00
N-O	0 / 94	0.0	0.0	0.05 (1)	10.00
O-F	-583 / 0	0.0	0.0	0.06 (1)	6.25
N-M	0 / 32	-27.5	-27.5	0.16 (2)	10.00
M-L	0 / 1872	-27.5	-27.5	0.40 (1)	10.00
L-K	0 / 0	-27.5	-27.5	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-Q	-746 / 0	0.25 (1)
Q-D	0 / 243	0.05 (2)
D-P	-623 / 0	0.37 (1)
P-E	0 / 540	0.12 (1)
E-O	0 / 432	0.10 (1)
O-M	0 / 1614	0.26 (1)
O-G	0 / 744	0.17 (1)
M-G	0 / 206	0.05 (3)
M-H	-349 / 0	0.29 (1)
L-H	-250 / 98	0.09 (1)
L-I	0 / 1913	0.43 (1)
T-R	-198 / 0	0.02 (1)
B-R	0 / 2811	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.42/1.00 (H-I:1) , BC=0.55/1.00 (Q-R:1) , WB=0.63/1.00 (B-R:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.89 (O) (INPUT = 0.90)
SSI METAL= 0.55 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 3516 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12255-1B
STRUCTURAL COMPONENT ONLY

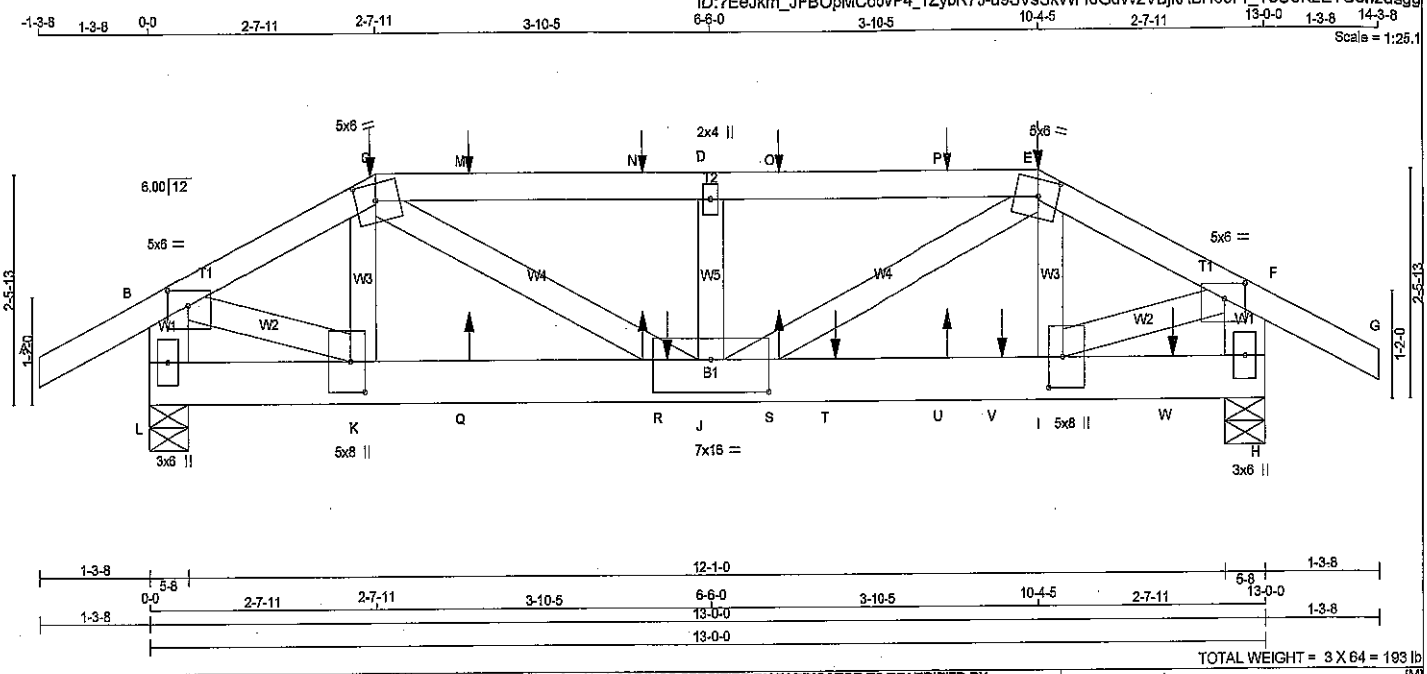
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T41	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:55 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-u9SVsSkWFf6GdW2VBjkABH65Fr_YoU5K2ETOehzdsgg



TOTAL WEIGHT = 3 X 64 = 193 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

*DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(3.4)
C- E	1	12	SIDE(3.2)
E- G	1	12	SIDE(3.4)
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	2	4	SIDE(674.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	6.0	2.25	2.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.75
F	TMVW-p	MT20	5.0	6.0	2.00	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.00	2.00
J	BMWWW-t	MT20	7.0	16.0	4.25	8.00
K	BMWW+t	MT20	5.0	8.0	4.00	2.00
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

P6 1/2

DWG NO. TAM122256-T41B

STRUCTURAL COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4791	0	4791	0	0	5-8	5-8
H	7498	0	7498	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3752	2399 / 0	666 / 0	0 / 0	0 / 0	688 / 0	0 / 0
H	5853	3775 / 0	1017 / 0	0 / 0	0 / 0	1061 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-104.9	-104.9	0.05 (1)	10.00	
B- C	-6227 / 0	-104.9	-104.9	0.09 (1)	4.65	
C- M	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
M- N	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
N- D	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
D- O	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
O- P	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
P- E	-11788 / 0	-104.9	-104.9	0.28 (1)	3.38	
E- F	-8891 / 0	-104.9	-104.9	0.14 (1)	3.97	
F- G	0 / 32	-104.9	-104.9	0.05 (1)	10.00	
L- B	-4808 / 0	0.0	0.0	0.10 (1)	7.76	
H- F	-6745 / 0	0.0	0.0	0.14 (1)	6.82	
L- K	0 / 0	-95.7	-95.7	0.08 (1)	10.00	
K- Q	0 / 5628	-95.4	-95.4	0.43 (1)	10.00	
Q- R	0 / 5628	-95.4	-95.4	0.43 (1)	10.00	
R- J	0 / 5628	-27.5	-27.5	0.43 (1)	10.00	
J- S	0 / 7841	-27.5	-27.5	0.62 (1)	10.00	
S- T	0 / 7841	-27.5	-27.5	0.62 (1)	10.00	
T- U	0 / 7841	-27.5	-27.5	0.62 (1)	10.00	
U- V	0 / 7841	-27.5	-27.5	0.62 (1)	10.00	
V- I	0 / 7841	-27.5	-27.5	0.62 (1)	10.00	
I- W	0 / 0	-27.8	-27.8	0.25 (1)	10.00	
W- H	0 / 0	-27.8	-27.8	0.25 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-949 / 0	0.03 (1)
C- J	0 / 7014	0.38 (1)
J- D	-354 / 20	0.01 (1)
J- E	0 / 4494	0.24 (1)
I- E	0 / 1459	0.08 (1)
B- K	0 / 5825	0.31 (1)
I- F	0 / 8317	0.45 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-7-11	-25	-25	---	BACK	VERT	TOTAL
E	10-4-5	-25	-25	---	BACK	VERT	TOTAL
M	3-8-9	-0	-1	---	BACK	VERT	TOTAL
N	5-8-9	-0	-1	---	BACK	VERT	TOTAL
O	7-3-7	-0	-1	---	BACK	VERT	TOTAL
P	9-3-7	-0	-1	---	BACK	VERT	TOTAL
Q	3-8-9	11	1	11	BACK	VERT	TOTAL
R	5-8-9	11	1	11	BACK	VERT	TOTAL
R	6-0-0	-3781	-3781	---	FRONT	VERT	TOTAL
S	7-3-7	11	1	11	BACK	VERT	TOTAL
T	7-11-4	-2028	-2028	---	FRONT	VERT	TOTAL
U	9-3-7	11	1	11	BACK	VERT	TOTAL
V	9-11-4	-2028	-2028	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 3528 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-7-11
RIGHT SETBACK = 0-0
END SETBACK = 2-0-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-7-11 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 2-7-11
END SETBACK = 2-0-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-7-11 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-10
END DISTANCE = 6-0-0
END SPAN CARRIED = 4-0-10
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.62/1.00 (I-J:1) , WB=0.45/1.00 (F-I:1) , SSI=0.72/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T41	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.4 lbs FACTORED DOWN AT 3-8-9, 1.4 lbs FACTORED DOWN AT 5-8-9, 1.4 lbs FACTORED DOWN AT 7-3-7, 1.4 lbs FACTORED DOWN AT 9-3-7, AND 25.0 lbs FACTORED DOWN AT 2-7-11, AND 25.0 lbs FACTORED DOWN AT 10-4-5 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 10.9 lbs FACTORED UP AT 3-8-9, 1.0 lbs FACTORED DOWN AND 10.9 lbs FACTORED UP AT 5-8-9, 3780.8 lbs FACTORED DOWN AT 6-0-0, 1.0 lbs FACTORED DOWN AND 10.9 lbs FACTORED UP AT 7-3-7, 2027.8 lbs FACTORED DOWN AT 7-11-4, 1.0 lbs FACTORED DOWN AND 10.9 lbs FACTORED UP AT 9-3-7, AND 2027.8 lbs FACTORED DOWN AT 9-11-4, AND 2027.8 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	11-11-4	-2028	-2028	—	FRONT	VERT	TOTAL

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.66 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3528
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12256-1B
STRUCTURAL
COMPONENT ONLY

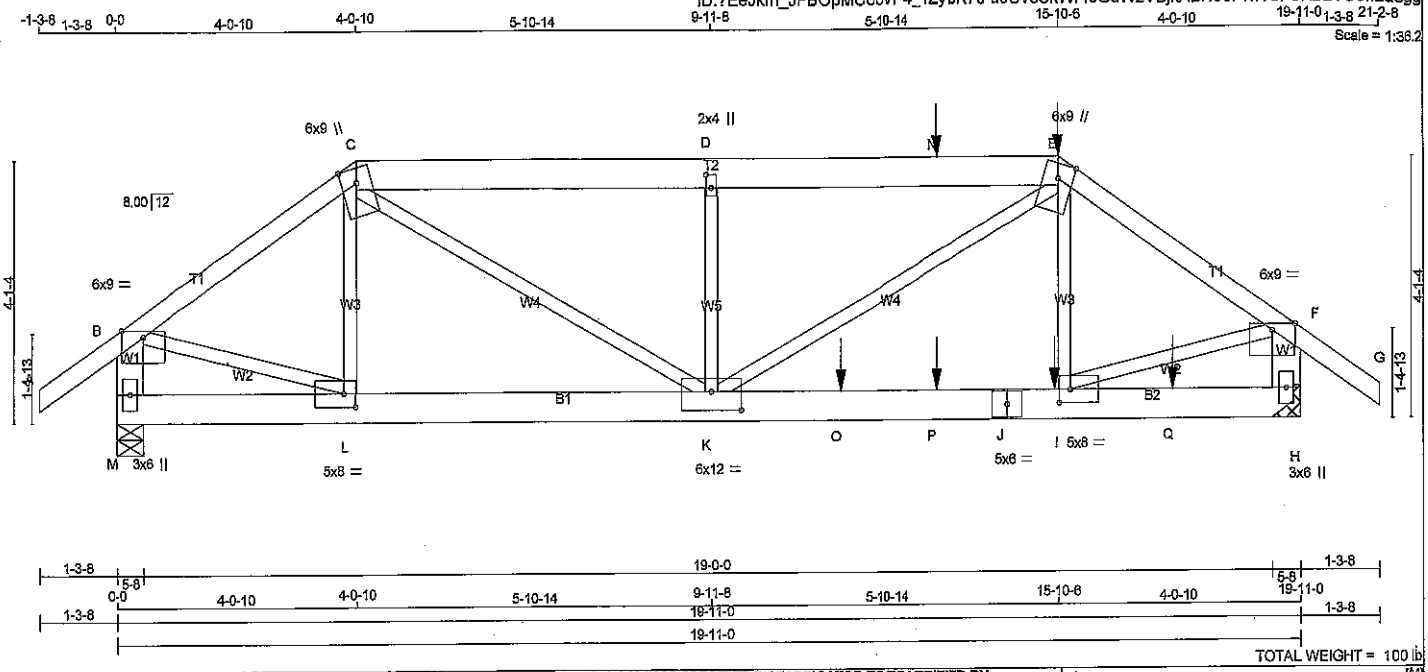
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T44	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:55 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-u9SVsSkWFf6GdW2VBjkABH60Prw?oP5K2ETOehzdsgg

Scale = 1:36.2



TOTAL WEIGHT = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWW+m	MT20	6.0	9.0	Edge	
F	TMVW-p	MT20	6.0	9.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	8.0	2.50	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	12.0	3.50	6.00
L	BMWW-t	MT20	5.0	8.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 393.5 lbs FACTORED DOWN AT 15-10-6, AND 126.3 lbs FACTORED DOWN AT 13-9-10 ON TOP CHORD, AND 1671.5 lbs FACTORED DOWN AT 12-1-8, 67.8 lbs FACTORED DOWN AT 13-9-10, AND 67.8 lbs FACTORED DOWN AT 15-9-10, AND 67.8 lbs FACTORED DOWN AT 17-9-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2260	0	2260	0	0	5-8	5-8
H	2975	0	2975	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1764	1137 / 0	306 / 0	0 / 0	0 / 0	321 / 0	0 / 0
H	2340	1477 / 0	426 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)		10.00
B- C	-2385 / 0	-104.9	-104.9	0.45 (1)		3.95
C- D	-4276 / 0	-104.9	-104.9	0.39 (1)		3.81
D- N	-4276 / 0	-104.9	-104.9	0.48 (1)		3.70
N- E	-4276 / 0	-104.9	-104.9	0.48 (1)		3.70
E- F	-3594 / 0	-104.9	-104.9	0.59 (1)		3.14
F- G	0 / 40	-104.9	-104.9	0.15 (1)		10.00
M- B	-2174 / 0	0.0	0.0	0.16 (1)		6.88
H- F	-3095 / 0	0.0	0.0	0.22 (1)		5.95
M- L	0 / 0	-27.5	-27.5	0.05 (1)		10.00
L- K	0 / 2006	-27.5	-27.5	0.40 (1)		10.00
K- O	0 / 2993	-27.5	-27.5	0.84 (1)		10.00
O- P	0 / 2993	-27.5	-27.5	0.84 (1)		10.00
P- J	0 / 2993	-27.5	-27.5	0.84 (1)		10.00
J- I	0 / 2993	-27.5	-27.5	0.84 (1)		10.00
I- Q	0 / 0	-27.5	-27.5	0.30 (1)		10.00
Q- H	0 / 0	-27.5	-27.5	0.30 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-541 / 0	0.13 (1)
C- K	0 / 2673	0.66 (1)
K- D	-822 / 0	0.20 (1)
K- E	0 / 1511	0.37 (1)
I- E	0 / 397	0.10 (2)
B- L	0 / 2053	0.51 (1)
I- F	0 / 3094	0.77 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	15-10-6	-394	-394	---	FRONT	VERT	TOTAL
I	15-9-10	-39	-68	---	FRONT	VERT	TOTAL
N	13-9-10	-126	-126	---	FRONT	VERT	TOTAL
O	12-1-8	-1672	-1672	---	FRONT	VERT	TOTAL
P	13-9-10	-39	-68	---	FRONT	VERT	TOTAL
Q	17-9-10	-39	-68	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 748 (0.32")

CSI: TC=0.59/1.00 (E-F:1) , BC=0.84/1.00 (I-K:1) , WB=0.77/1.00 (F-I:1) , SSI=0.77/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (I) (INPUT = 0.90)
SI METAL= 0.61 (I) (INPUT = 1.00)

DWG NO. TAM 12257-12
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER 3528 S. KATSOULAKOS PROVINCE OF ONTARIO

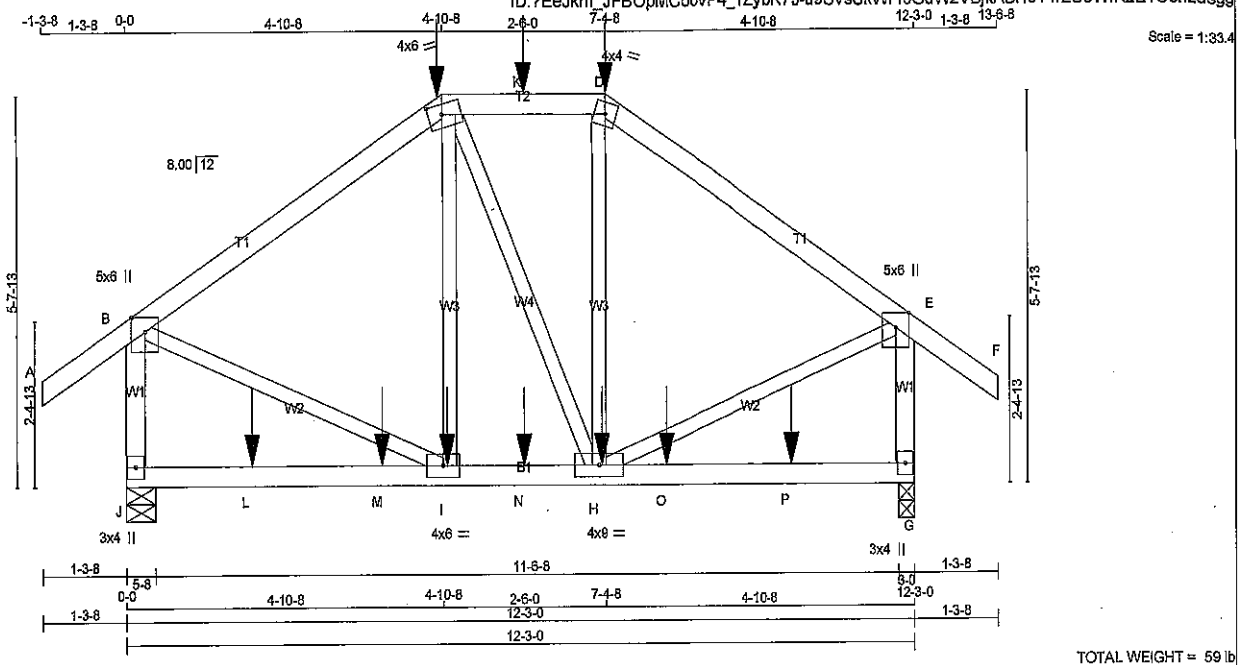
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T50	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:55 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-u9SVsSkWFf6GdW2VBjkABH614r2UoWIK2ETOehzdsgg



TOTAL WEIGHT = 59 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 550.0 lbs FACTORED DOWN AT 4-10-8, AND 150.6 lbs FACTORED DOWN AT 6-1-8, AND 550.0 lbs FACTORED DOWN AT 7-4-8 ON TOP CHORD, AND 68.3 lbs FACTORED DOWN AT 1-11-4, 68.3 lbs FACTORED DOWN AT 3-11-4, 68.3 lbs FACTORED DOWN AT 4-11-4, 68.3 lbs FACTORED DOWN AT 6-1-8, 68.3 lbs FACTORED DOWN AT 7-3-12, AND 68.3 lbs FACTORED DOWN AT 8-3-12, AND 68.3 lbs FACTORED DOWN AT 10-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1716	0	1716	0	0	5-8	5-8
G	1716	0	1716	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1333	870 / 0	224 / 0	0 / 0	0 / 0	240 / 0	0 / 0
G	1333	870 / 0	224 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	I- C	-250 / 207	0.12 (1)
B- C	-1367 / 0	-104.9	-104.9	0.55 (1)	4.79	C- H	-3 / 2	0.00 (1)
C- K	-1129 / 0	-104.9	-104.9	0.26 (1)	5.57	H- D	-253 / 210	0.13 (1)
K- D	-1129 / 0	-104.9	-104.9	0.26 (1)	5.57	B- I	0 / 1231	0.30 (1)
D- E	-1365 / 0	-104.9	-104.9	0.55 (1)	4.79	H- E	0 / 1229	0.30 (1)
E- F	0 / 40	-104.9	-104.9	0.15 (1)	10.00			
J- B	-1634 / 0	0.0	0.0	0.22 (1)	6.43			
G- E	-1633 / 0	0.0	0.0	0.22 (1)	6.44			
J- L	0 / 0	-27.5	-27.5	0.28 (3)	10.00			
L- M	0 / 0	-27.5	-27.5	0.28 (3)	10.00			
M- I	0 / 0	-27.5	-27.5	0.28 (3)	10.00			
I- N	0 / 1131	-27.5	-27.5	0.36 (2)	10.00			
N- H	0 / 1131	-27.5	-27.5	0.36 (2)	10.00			
H- O	0 / 0	-27.5	-27.5	0.28 (2)	10.00			
O- P	0 / 0	-27.5	-27.5	0.28 (2)	10.00			
P- G	0 / 0	-27.5	-27.5	0.28 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-550	-550	---	FRONT	VERT	TOTAL
D	7-4-8	-550	-550	---	FRONT	VERT	TOTAL
H	7-3-12	-40	-68	---	FRONT	VERT	TOTAL
I	4-11-4	-40	-68	---	FRONT	VERT	TOTAL
K	6-1-8	-151	-151	---	FRONT	VERT	TOTAL
L	1-11-4	-37	-68	---	FRONT	VERT	TOTAL
M	3-11-4	-40	-68	---	FRONT	VERT	TOTAL
N	6-1-8	-40	-68	---	FRONT	VERT	TOTAL
O	8-3-12	-40	-68	---	FRONT	VERT	TOTAL
P	10-3-12	-37	-68	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.36/1.00 (H-I:2) , WB=0.30/1.00 (B-I:1) , SSI=0.20/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL = 0.28 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S KATSOULAKOS PROVINCE OF ONTARIO

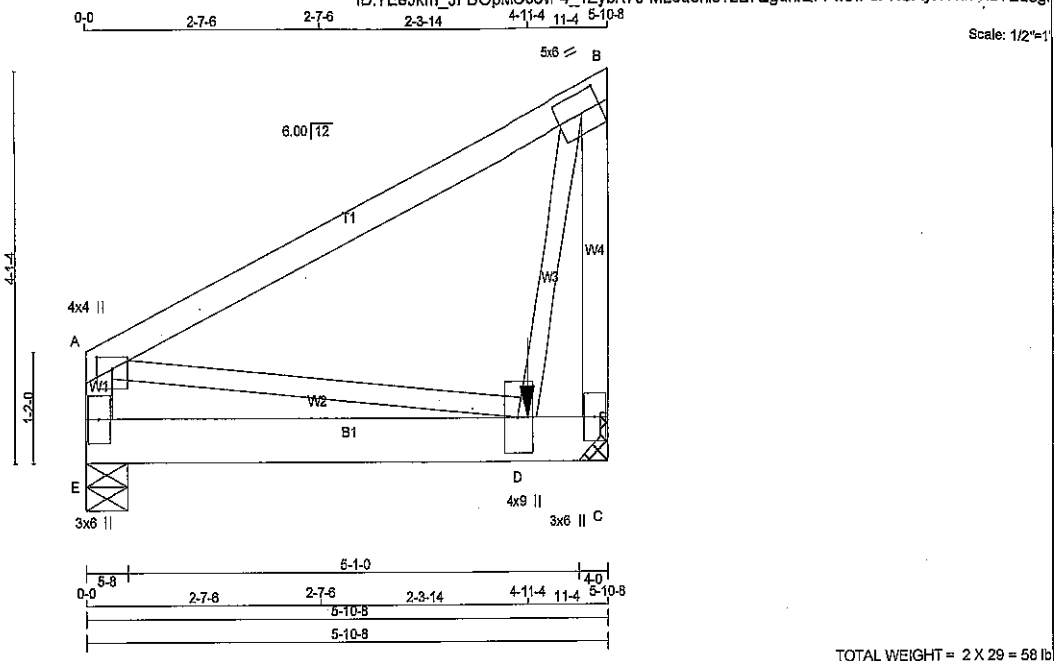
DWG NO. TAM 122588-T50
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T51	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:56 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ML0u3nl8?zE7EgdhlQFPkUfFsFRSXyfTHuDxB7zdsgf

Scale: 1/2"=1



TOTAL WEIGHT = 2 X 29 = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
E- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- C	2	12	SIDE(64.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	BMV1+p	MT20	3.0	6.0		
D	BMWW+t	MT20	4.0	9.0		
E	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2649.8 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1179	0	1179	0	0	5-8	5-8
C	2682	0	2682	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	934	578 / 0	178 / 0	0 / 0	0 / 0	178 / 0	0 / 0
C	2264	1414 / 0	435 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-778 / 0	0.0	0.0	0.04 (1)	7.81	A- D	0 / 724	0.09 (1)
A- B	-800 / 0	-104.9	-104.9	0.35 (1)	6.25	D- B	0 / 2921	0.36 (1)
C- B	-2782 / 0	0.0	0.0	0.34 (1)	6.87			
E- D	0 / 0	-155.8	-155.8	0.19 (1)	10.00			
D- C	0 / 0	-27.5	-27.5	0.03 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-11-4	-2650	-2650	---	FRONT	VERT.	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 4-11-4
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35/1.00 (A-B:1) , BC=0.19/1.00 (D-E:1) , WB=0.36/1.00 (B-D:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 3518 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12259-1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T51	1	2	TRUSS DESC.	

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)

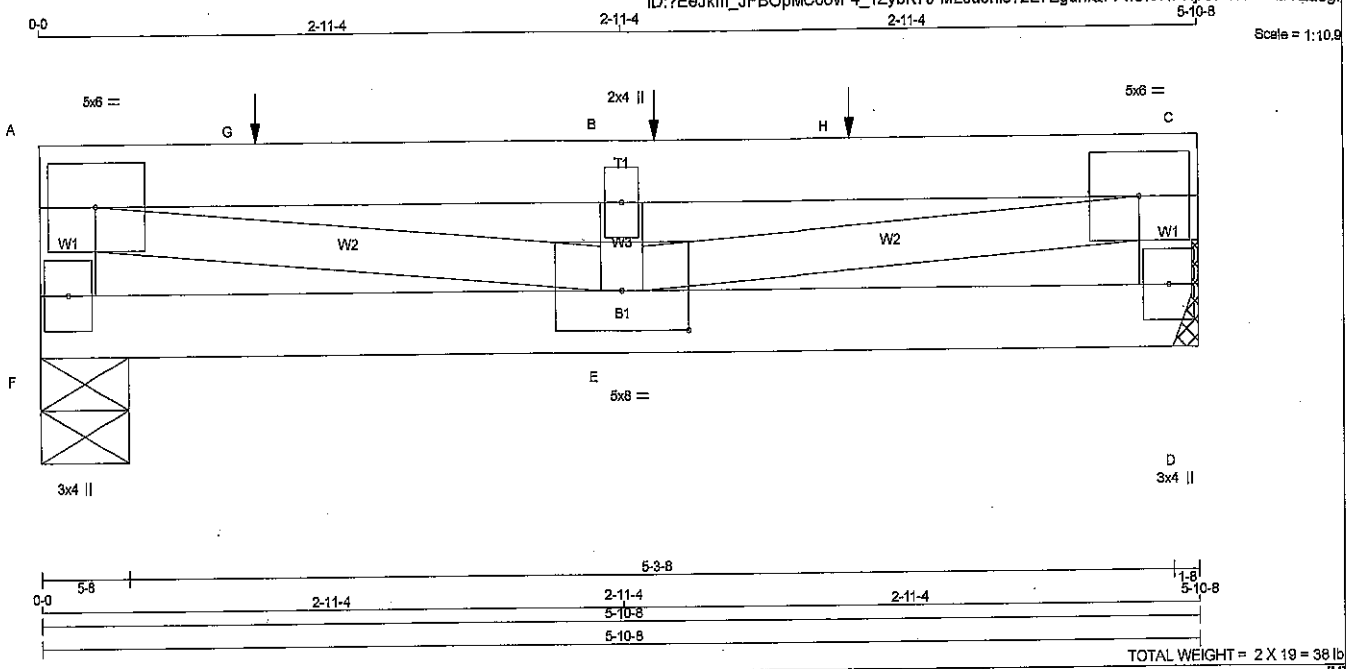
LICENSED PROFESSIONAL ENGINEER
3528
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12259-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288631	T52	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:17:56 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-ML0u3nl8?zE7EgdhlQFPkUf9RFRjXxcTHuDxB7zdsgf



TOTAL WEIGHT = 2 X 19 = 38 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(0.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	8.0	2.25	4.00
F	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 235.4 lbs FACTORED DOWN AT 1-1-4, AND 279.9 lbs FACTORED DOWN AT 3-1-4, AND 1611.6 lbs FACTORED DOWN AT 4-1-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1206	0	1206	0	0	5-8	5-8
D	1699	0	1699	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	932	618 / 0	151 / 0	0 / 0	0 / 0	163 / 0	0 / 0
D	1325	856 / 0	228 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1079 / 0	0.0	0.0	0.05 (1)	7.81		A- E	0 / 3811	0.43 (1)
A- G	-3704 / 0	-104.9	-104.9	0.27 (1)	4.63		E- B	-1878 / 0	0.13 (1)
G- B	-3704 / 0	-104.9	-104.9	0.27 (1)	4.63		E- C	0 / 3811	0.43 (1)
B- H	-3704 / 0	-104.9	-104.9	0.76 (1)	3.58				
H- C	-3704 / 0	-104.9	-104.9	0.76 (1)	3.58				
D- C	-1572 / 0	0.0	0.0	0.08 (1)	7.81				
F- E	0 / 0	-27.5	-27.5	0.17 (1)	10.00				
E- D	0 / 0	-27.5	-27.5	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-1-4	-280	-280	---	TOP	VERT	TOTAL
G	1-1-4	-235	-235	---	FRONT	VERT	TOTAL
H	4-1-4	-1612	-1612	---	TOP	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 841 (0.08")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.17/1.00 (E-F:1) , WB=0.43/1.00 (A-E:1) , SSI=0.48/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.48 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3528
S. KATSOULAKOS
PROVINCE OF ONTARIO

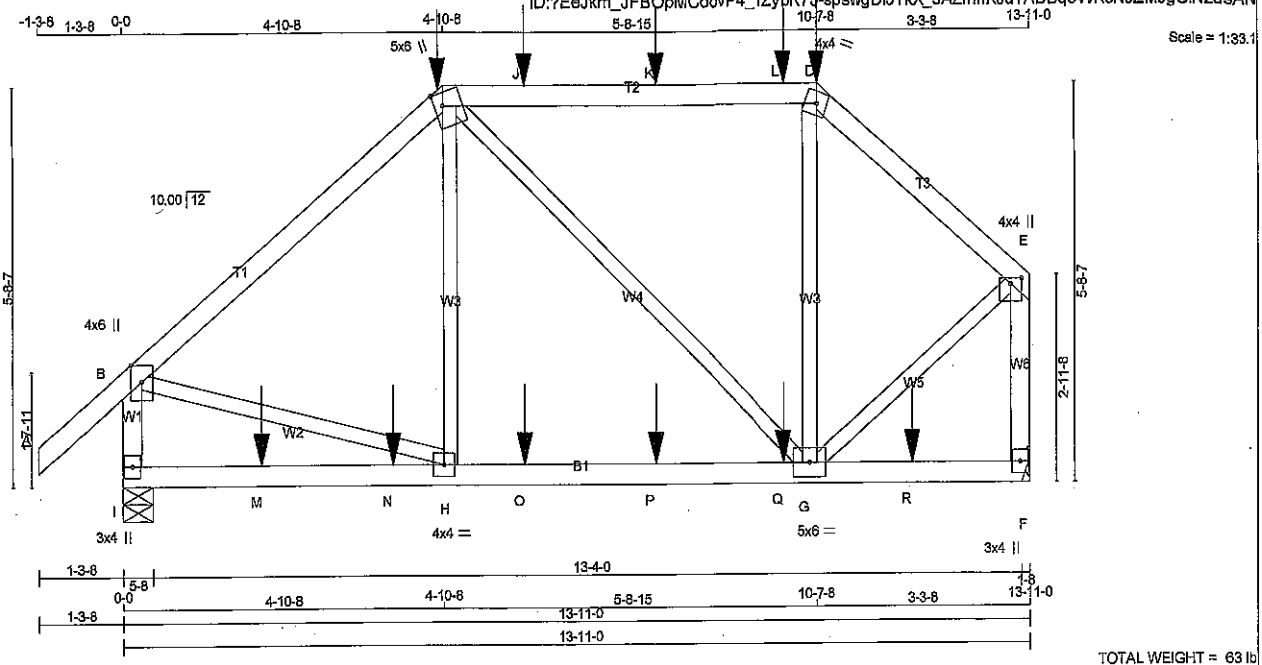
DWG NO. TAM 12260-TB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295461	T100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:52:22 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-spswgDl3TkX_5AZmnKJuTADBq3WKcNJZMJgGfNzdsAN



TOTAL WEIGHT = 63 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 100.4 lbs FACTORED DOWN AT 4-10-8, 100.4 lbs FACTORED DOWN AT 10-7-8, 45.9 lbs FACTORED DOWN AT 6-1-4, AND 45.9 lbs FACTORED DOWN AT 8-1-4, AND 71.2 lbs FACTORED DOWN AT 10-1-4 ON TOP CHORD, AND 21.4 lbs FACTORED DOWN AT 2-1-4, 21.4 lbs FACTORED DOWN AT 4-1-4, 21.4 lbs FACTORED DOWN AT 6-1-4, 21.4 lbs FACTORED DOWN AT 8-1-4, AND 21.4 lbs FACTORED DOWN AT 10-1-4, AND 21.4 lbs FACTORED DOWN AT 12-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG	REQRD BRG
I	1260	0	1260	0	0	5-8	5-8
F	1173	0	1173	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	995	625 / 0	187 / 0	0 / 0	0 / 0	183 / 0	0 / 0
F	940	568 / 0	192 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	
B- C	-947 / 0	-104.9	-104.9	0.52 (1)	5.55	
C- J	-596 / 0	-104.9	-104.9	0.84 (1)	5.32	
J- K	-596 / 0	-104.9	-104.9	0.84 (1)	5.32	
K- L	-596 / 0	-104.9	-104.9	0.84 (1)	5.32	
L- D	-596 / 0	-104.9	-104.9	0.84 (1)	5.32	
D- E	-782 / 0	-104.9	-104.9	0.23 (1)	6.25	
I- B	-1200 / 0	0.0	0.0	0.14 (1)	7.26	
F- E	-1138 / 0	0.0	0.0	0.19 (1)	7.42	
I- M	0 / 0	-28.0	-28.0	0.26 (3)	10.00	
M- N	0 / 0	-28.0	-28.0	0.26 (3)	10.00	
N- H	0 / 0	-28.0	-28.0	0.26 (3)	10.00	
H- O	0 / 726	-28.0	-28.0	0.35 (2)	10.00	
O- P	0 / 726	-28.0	-28.0	0.35 (2)	10.00	
P- Q	0 / 726	-28.0	-28.0	0.35 (2)	10.00	
Q- G	0 / 726	-28.0	-28.0	0.35 (2)	10.00	
G- R	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
R- F	0 / 0	-28.0	-28.0	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 267	0.07 (3)
C- G	-181 / 0	0.22 (1)
G- D	-172 / 147	0.09 (1)
B- H	0 / 751	0.19 (1)
G- E	0 / 757	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-100	-100	—	FRONT	VERT	TOTAL
D	10-7-8	-100	-100	—	FRONT	VERT	TOTAL
J	6-1-4	-46	-46	—	BACK	VERT	TOTAL
K	8-1-4	-46	-46	—	BACK	VERT	TOTAL
L	10-1-4	-71	-71	—	BACK	VERT	TOTAL
M	2-1-4	-12	-21	—	BACK	VERT	TOTAL
N	4-1-4	-12	-21	—	BACK	VERT	TOTAL
O	6-1-4	-12	-21	—	BACK	VERT	TOTAL
P	8-1-4	-12	-21	—	BACK	VERT	TOTAL
Q	10-1-4	-12	-21	—	BACK	VERT	TOTAL
R	12-1-4	-12	-21	—	BACK	VERT	TOTAL

DWG NO. TAM 12227-1B

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.84/1.00 (C-D:1) , BC=0.35/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

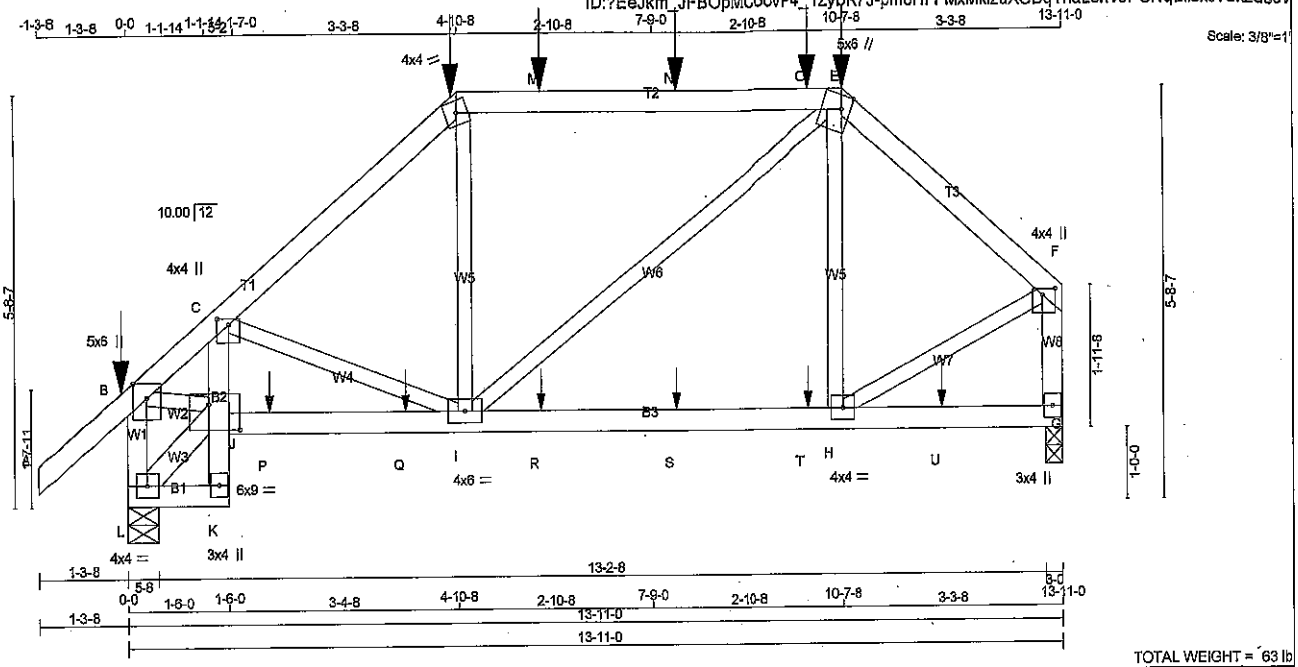
JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.27 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T100S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:08 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-pm6HFFMxMki2uXGDqYnaL8nv3PCNqBilskJ?ekzdsov



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BVMWW-I	MT20	6.0	9.0	4.25	5.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 75.3 lbs FACTORED DOWN AT 4-10-8, 75.3 lbs FACTORED DOWN AT 0-0, 75.3 lbs FACTORED DOWN AT 10-7-8, 37.3 lbs FACTORED DOWN AND 117.3 lbs FACTORED UP AT 6-1-4, AND 37.3 lbs FACTORED DOWN AND 117.3 lbs FACTORED UP AT 8-1-4, AND 55.1 lbs FACTORED DOWN AND 92.1 lbs FACTORED UP AT 10-1-4 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AT 2-1-4, 1.0 lbs FACTORED DOWN AT 4-1-4, 1.0 lbs FACTORED DOWN AT 6-1-4, 1.0 lbs FACTORED DOWN AT 8-1-4, AND 1.0 lbs FACTORED DOWN AT 10-1-4, AND 1.0 lbs FACTORED DOWN AT 12-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1264	0	1264	0	0	5-8	5-8
G	1072	0	1072	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	997	629 / 0	186 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	866	512 / 0	184 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	C- I	-275 / 0	0.08 (5)	
B- C	-1313 / 0	-104.9	-104.9	0.17 (1)	5.41	I- D	0 / 213	0.06 (7)	
C- D	-1043 / 0	-104.9	-104.9	0.21 (1)	5.87	I- E	0 / 169	0.04 (1)	
D- M	-789 / 0	-104.9	-104.9	0.80 (1)	5.02	H- E	-173 / 109	0.06 (1)	
M- N	-789 / 0	-104.9	-104.9	0.80 (1)	5.02	H- F	0 / 731	0.18 (1)	
N- O	-789 / 0	-104.9	-104.9	0.80 (1)	5.02	L- J	-49 / 0	0.01 (1)	
O- E	-789 / 0	-104.9	-104.9	0.80 (1)	5.02	B- J	0 / 1005	0.25 (1)	
E- F	-861 / 0	-104.9	-104.9	0.23 (1)	6.25				
L- B	-1212 / 0	0.0	0.0	0.14 (1)	7.24				
G- F	-1041 / 0	0.0	0.0	0.13 (1)	7.67				
L- K	0 / 37	-28.0	-28.0	0.02 (7)	10.00				
K- J	0 / 33	0.0	0.0	0.07 (1)	10.00				
J- C	0 / 72	0.0	0.0	0.07 (1)	10.00				
J- P	0 / 1039	-28.0	-28.0	0.29 (2)	10.00				
P- Q	0 / 1039	-28.0	-28.0	0.29 (2)	10.00				
Q- I	0 / 1039	-28.0	-28.0	0.29 (2)	10.00				
I- R	0 / 655	-28.0	-28.0	0.25 (2)	10.00				
R- S	0 / 655	-28.0	-28.0	0.25 (2)	10.00				
S- T	0 / 655	-28.0	-28.0	0.25 (2)	10.00				
T- H	0 / 655	-28.0	-28.0	0.25 (2)	10.00				
H- U	0 / 0	-28.0	-28.0	0.18 (7)	10.00				
U- G	0 / 0	-28.0	-28.0	0.18 (7)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
B	0-0	-75	-75	—	FRONT	VERT	TOTAL
D	4-10-8	-75	-75	—	FRONT	VERT	TOTAL
E	10-7-8	-75	-75	—	FRONT	VERT	TOTAL
M	6-1-4	-30	-37	117	BACK	VERT	TOTAL
N	8-1-4	-30	-37	117	BACK	VERT	TOTAL
O	10-1-4	-55	-55	92	BACK	VERT	TOTAL
P	2-1-4	1	1	—	BACK	VERT	TOTAL
Q	4-1-4	1	1	—	BACK	VERT	TOTAL
R	6-1-4	1	1	—	BACK	VERT	TOTAL
S	8-1-4	1	1	—	BACK	VERT	TOTAL
T	10-1-4	1	1	—	BACK	VERT	TOTAL
U	12-1-4	1	1	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.28/1.00 (I-J:2) , WB=0.25/1.00 (B-J:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 3516 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 12218-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T100S	1	1	TRUSS DESC.	

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3518
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2118-1B
STRUCTURAL
COMPONENT ONLY

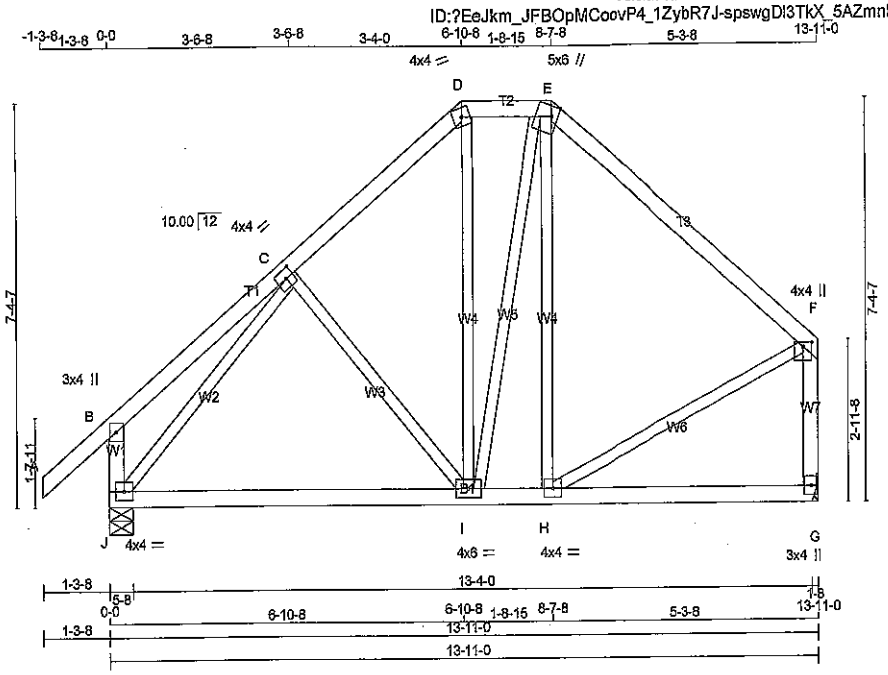
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295461	T101	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:52:22 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-spswgDl3TkX_5AZmnKJuTADJ13W?cK2ZMJgGiNzdsAN

Scale = 1:42.2



TOTAL WEIGHT = 72 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	925	0	925	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
J	1070	0	1070	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	737	452 / 0	146 / 0	0 / 0	0 / 0	139 / 0	0 / 0
J	836	542 / 0	146 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		C- I	-172 / 35	0.09 (1)
B- C	0 / 27	-104.9	-104.9	0.20 (1)	10.00		I- D	0 / 192	0.04 (2)
C- D	-640 / 0	-104.9	-104.9	0.16 (1)	6.25		I- E	0 / 148	0.03 (2)
D- E	-472 / 0	-104.9	-104.9	0.04 (1)	6.25		H- E	-141 / 19	0.14 (1)
E- F	-576 / 0	-104.9	-104.9	0.38 (1)	6.25		J- C	-917 / 0	0.43 (1)
J- B	-282 / 0	0.0	0.0	0.03 (1)	7.81		H- F	0 / 491	0.11 (1)
G- F	-862 / 0	0.0	0.0	0.14 (1)	7.81				
J- I	0 / 583	-28.0	-28.0	0.37 (2)	10.00				
I- H	0 / 439	-28.0	-28.0	0.36 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.38/1.00 (E-F:1) , BC=0.37/1.00 (I-J:2) , WB=0.43/1.00 (C-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM 12228-18
STRUCTURAL COMPONENT ONLY

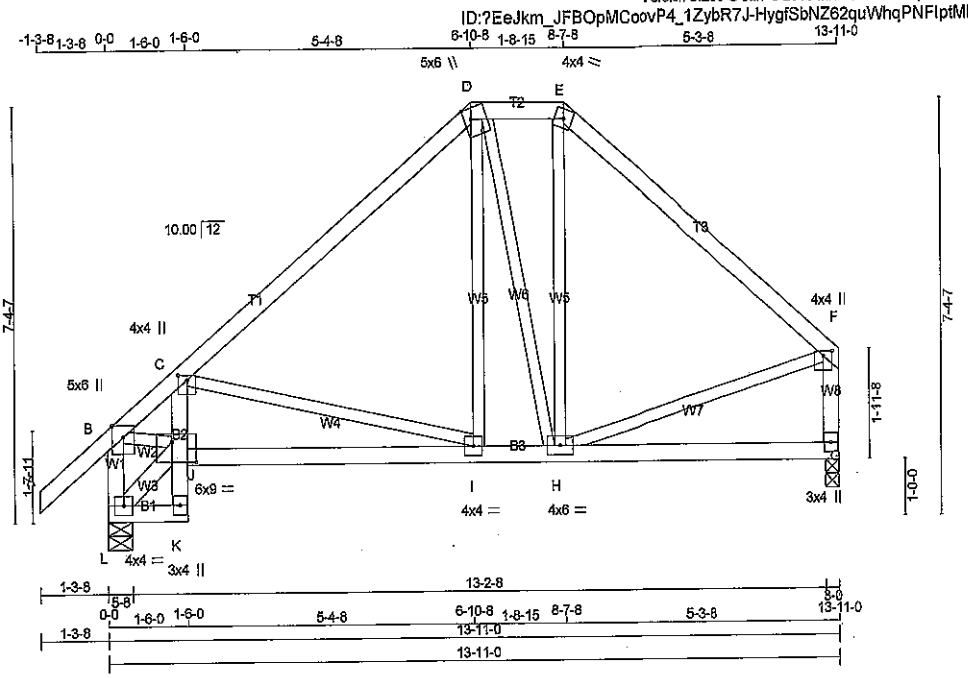
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T101S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:09 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-HygfSbNZ62quWhqPNFIptMKANpYaZcgv5O2ZBAzdsou

Scale = 1:41.0



TOTAL WEIGHT = 69 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	4.25	5.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1078	0	1078	0	0	5-8	5-8
G	918	0	918	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	840	546 / 0	146 / 0	0 / 0	0 / 0	148 / 0	0 / 0
G	732	447 / 0	146 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- I	-567 / 0		0.33 (1)
B- C	-1275 / 0	-104.9	-104.9	0.32 (1)	5.33	I- D	0 / 256		0.06 (2)
C- D	-727 / 0	-104.9	-104.9	0.38 (1)	6.25	D- H	-103 / 0		0.07 (1)
D- E	-509 / 0	-104.9	-104.9	0.04 (1)	6.25	H- E	0 / 166		0.04 (3)
E- F	-662 / 0	-104.9	-104.9	0.38 (1)	6.25	H- F	0 / 530		0.12 (1)
L- B	-1027 / 0	0.0	0.0	0.11 (1)	7.74	L- J	-47 / 0		0.01 (1)
G- F	-855 / 0	0.0	0.0	0.10 (1)	7.61	B- J	0 / 1049		0.24 (1)
L- K	0 / 35	-28.0	-28.0	0.02 (2)	10.00				
K- J	0 / 33	0.0	0.0	0.06 (1)	10.00				
J- C	0 / 122	0.0	0.0	0.06 (1)	10.00				
J- I	0 / 1082	-28.0	-28.0	0.29 (2)	10.00				
I- H	0 / 535	-28.0	-28.0	0.24 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.38/1.00 (E-F:1) , BC=0.29/1.00 (I-J:2) , WB=0.33/1.00 (C-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (H) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

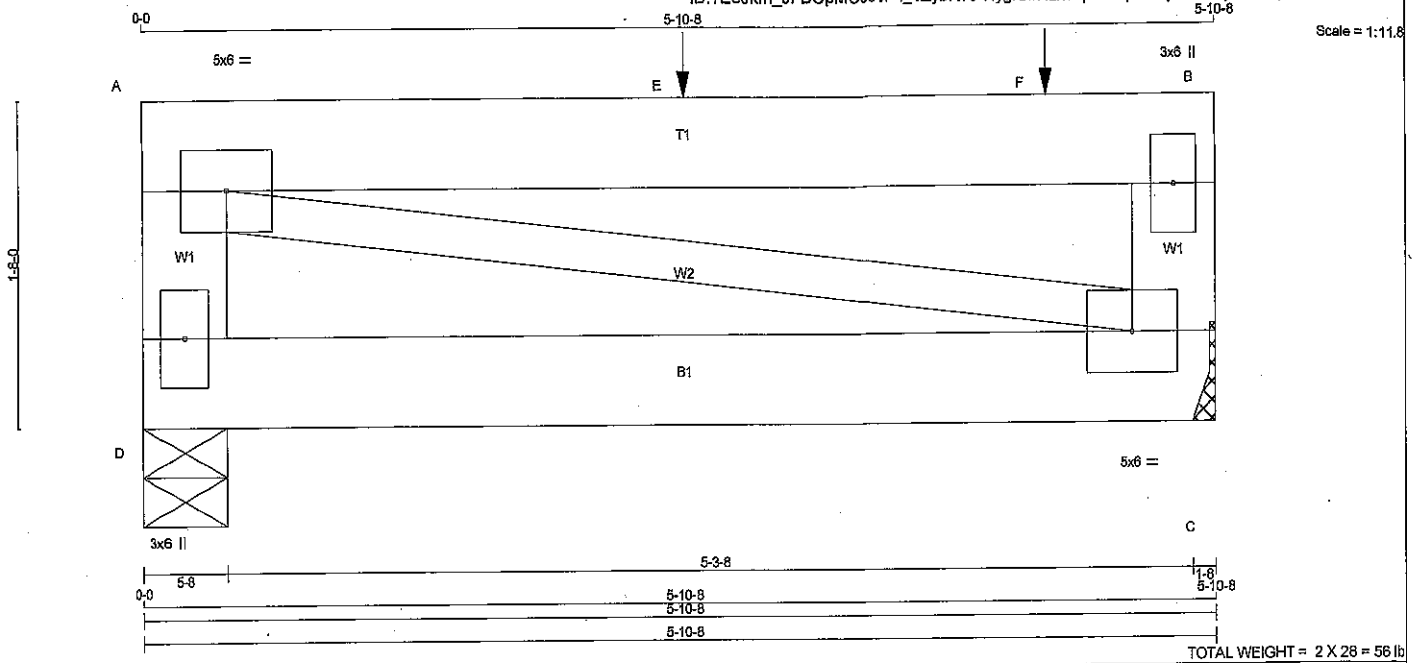
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12219-1B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	T102	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:09 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-HygfSbNZ62quWhqPNFlptMK5bpb0Zhqv5O2ZBAzdsou



TOTAL WEIGHT = 2 X 28 = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
C - B	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	2	12	TOP
B- C	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMV+p	MT20	3.0	6.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 967.1 lbs FACTORED DOWN AT 2-11-4, AND 816.1 lbs FACTORED DOWN AT 4-11-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1004	0	1004	0	0	IN-SX	IN-SX
C	1560	0	1560	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	814	475 / 0	177 / 0	0 / 0	0 / 0	162 / 0	0 / 0
C	1287	737 / 0	277 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
D- A	-922 / 0	0.0	0.0	0.03 (1)	7.81	A- C	0 / 0	0.00 (1)
A- E	0 / 0	-104.9	-104.9	0.75 (1)	10.00			
E- F	0 / 0	-104.9	-104.9	0.75 (1)	10.00			
F- B	0 / 0	-104.9	-104.9	0.75 (1)	10.00			
C- B	-1478 / 0	0.0	0.0	0.05 (1)	7.81			
D- C	0 / 0	-26.0	-28.0	0.07 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-11-4	-967	-967	—	TOP	VERT	TOTAL
F	4-11-4	-816	-816	—	TOP	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.75/1.00 (A-B:1) , BC=0.07/1.00 (C-D:2) , WB=0.00/1.00 (A-C:1) , SSI=0.45/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.19 (C) (INPUT = 0.90)
SSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1222-18
STRUCTURAL COMPONENT ONLY

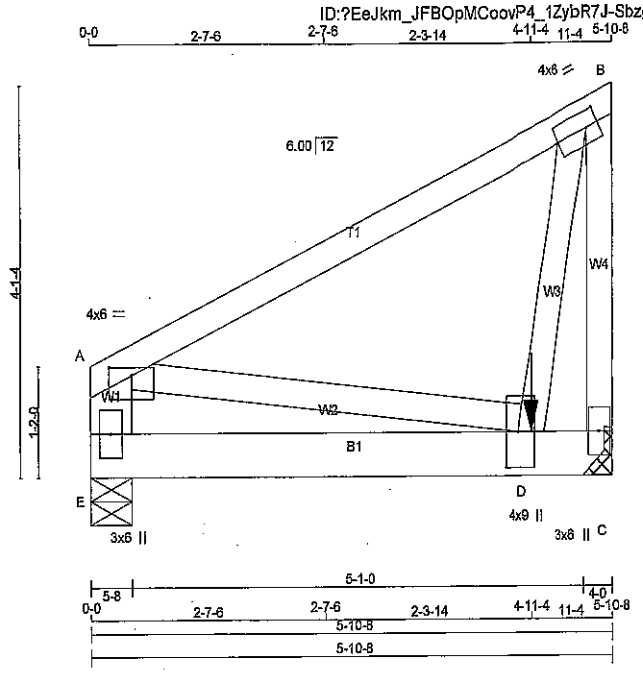
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T150	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:14:45 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-SbzgzURK_HADqJW_3pKcVDEbaGFsHbPMMgRL_hzdsje

Scale: 1/2"=1'



TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
E- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
E- C	2	12	SIDE(64.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0	2.00	1.75
C	BMV1+p	MT20	3.0	6.0		
D	BMWW+t	MT20	4.0	9.0	4.50	1.75
E	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3032.1 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	1240	0	1240	0	0	5-8	5-8
C	3203	0	3203	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	983	608 / 0	187 / 0	0 / 0	0 / 0	187 / 0	0 / 0
C	2539	1571 / 0	484 / 0	0 / 0	0 / 0	484 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-845 / 0	0.0	0.0	0.03 (1)	7.81	D- B	0 / 3340	0.30 (1)
A- B	-914 / 0	-104.9	-104.9	0.36 (1)	6.25	A- D	0 / 828	0.07 (1)
C- B	-3136 / 0	0.0	0.0	0.38 (1)	6.55			
E- D	0 / 0	-155.8	-155.8	0.18 (1)	10.00			
D- C	0 / 0	-27.5	-27.5	0.02 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-11-4	-3032	-3032	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 4-11-4
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.18/1.00 (D-E:1) , WB=0.30/1.00 (B-D:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
3516
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12237-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288646	T150	1	2	TRUSS DESC.	

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

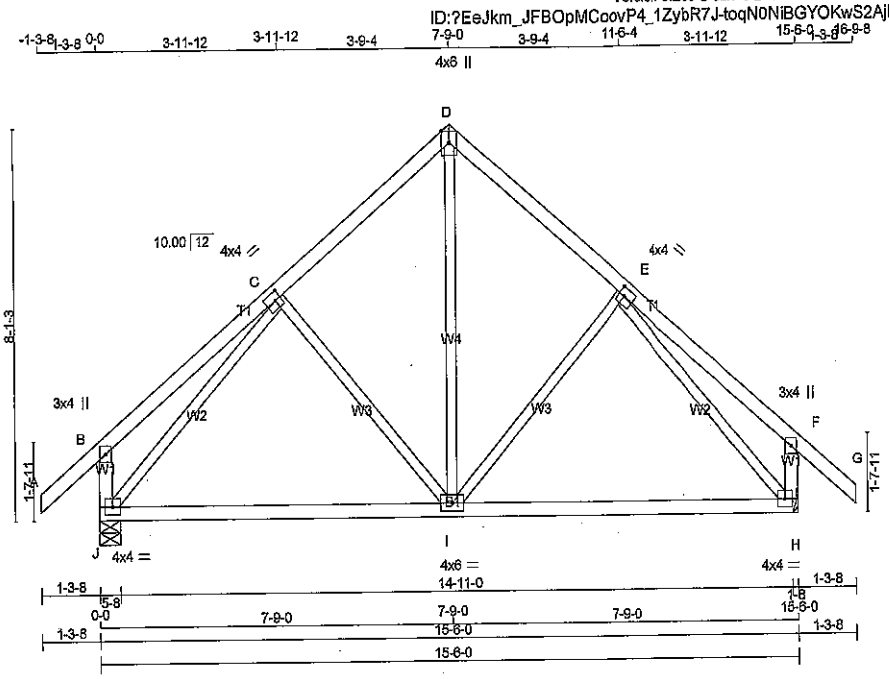
DWG NO. TAM 12237-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295493	T160	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:16:30 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-toqN0NiBGYOKwS2AjNjEFM5a7a_ZO?MpVVNOA6zdsi?

Scale: 1/4"=1'



TOTAL WEIGHT = 2 X 72 = 144 lb [MiF]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1176	0	1176	0	0	5-8	5-8
H	1176	0	1176	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	919	593 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	919	593 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 30	-104.9	-104.9	0.25 (1)	10.00	
C- D	-733 / 0	-104.9	-104.9	0.20 (1)	6.25	
D- E	-733 / 0	-104.9	-104.9	0.20 (1)	6.25	
E- F	0 / 30	-104.9	-104.9	0.25 (1)	10.00	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- B	-300 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-300 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 677	-28.0	-28.0	0.56 (2)	10.00	
I- H	0 / 677	-28.0	-28.0	0.56 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 584	0.13 (1)
I- E	-209 / 37	0.13 (1)
C- I	-209 / 37	0.13 (1)
J- C	-1043 / 0	0.62 (1)
E- H	-1043 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.56/1.00 (I-J:2) , WB=0.62/1.00 (E-H:1) , SSI=0.18/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.83 (C) (INPUT = 0.90)
MSI METAL= 0.38 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3516
S. KATSOULAKOS
PROVINCE OF ONTARIO

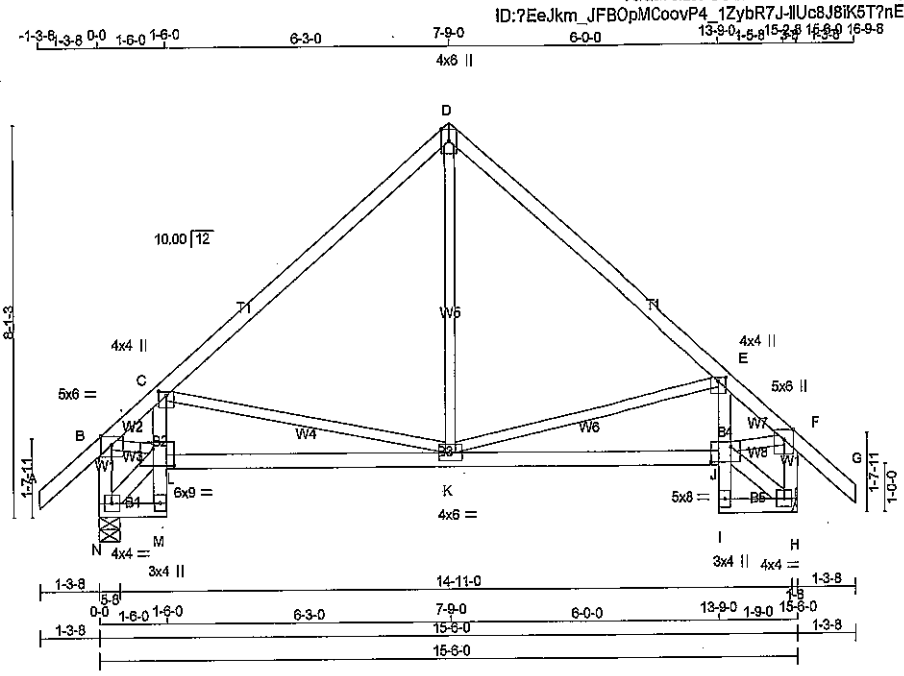
DWG NO. TAM 112142-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295492	T160S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:15:43 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-lIUc8J8iK5T?nEy7S?NCrReTgezHtfuihVP0KzdsiK

Scale: 1/4"=1'



TOTAL WEIGHT = 2 X 74 = 147 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	6.0	3.75	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	6.0	9.0	4.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1176	0	1176	0	0	IN-SX	IN-SX
H	1176	0	1176	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	919	593 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	919	593 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		C- K	-706 / 0	0.56 (1)	
	B- C	-1477 / 0	-104.9	-104.9	0.44 (1)	4.84		K- D	0 / 501	0.11 (2)	
	C- D	-815 / 0	-104.9	-104.9	0.53 (1)	5.92		K- E	-594 / 0	0.43 (1)	
	D- E	-813 / 0	-104.9	-104.9	0.49 (1)	6.02		N- L	-55 / 0	0.01 (1)	
	E- F	-1382 / 0	-104.9	-104.9	0.40 (1)	5.04		B- L	0 / 1248	0.28 (1)	
	F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00		J- H	-43 / 0	0.00 (1)	
	N- B	-1120 / 0	0.0	0.0	0.12 (1)	7.49		J- F	0 / 1147	0.26 (1)	
	H- F	-1128 / 0	0.0	0.0	0.12 (1)	7.48					
	N- M	0 / 41	-28.0	-28.0	0.02 (2)	10.00					
	M- L	0 / 33	0.0	0.0	0.07 (1)	10.00					
	L- C	0 / 122	0.0	0.0	0.07 (1)	10.00					
	L- K	0 / 1285	-28.0	-28.0	0.45 (2)	10.00					
	K- J	0 / 1169	-28.0	-28.0	0.44 (2)	10.00					
	I- J	0 / 39	0.0	0.0	0.06 (1)	10.00					
	J- E	-20 / 111	0.0	0.0	0.05 (2)	7.81					
	I- H	0 / 35	-28.0	-28.0	0.03 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.45/1.00 (K-L:2) , WB=0.56/1.00 (C-K:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3516
S KATSOULAKOS
PROVINCE OF ONTARIO

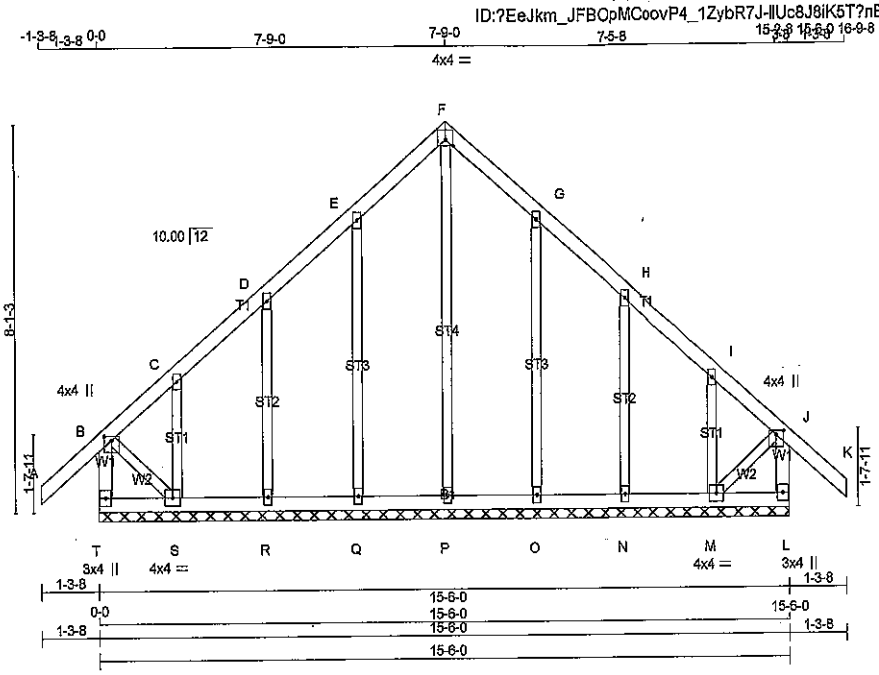
DWG NO. TAM 12239-1B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295492	G160	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:15:43 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-llUc8J8iK5T?nEy7S?NCrReZhe4vtrSuihVP0KzdsIk

Scale: 1/4"=1'



TOTAL WEIGHT = 77 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-328 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-69 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-23 / 0	-104.9	-104.9	0.05 (1)	6.25
D- E	-22 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-33 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-33 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-22 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-23 / 0	-104.9	-104.9	0.05 (1)	6.25
I- J	-69 / 0	-104.9	-104.9	0.14 (1)	6.25
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- J	-328 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 23	-28.0	-28.0	0.03 (2)	10.00
R- Q	0 / 19	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 15	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 15	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 19	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 23	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-153 / 0	0.19 (1)
Q- E	-236 / 0	0.16 (1)
R- D	-215 / 0	0.07 (1)
S- C	-124 / 0	0.02 (1)
O- G	-236 / 0	0.16 (1)
N- H	-215 / 0	0.07 (1)
M- I	-124 / 0	0.02 (1)
B- S	0 / 34	0.01 (1)
M- J	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (R-S:2) , WB=0.19/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)

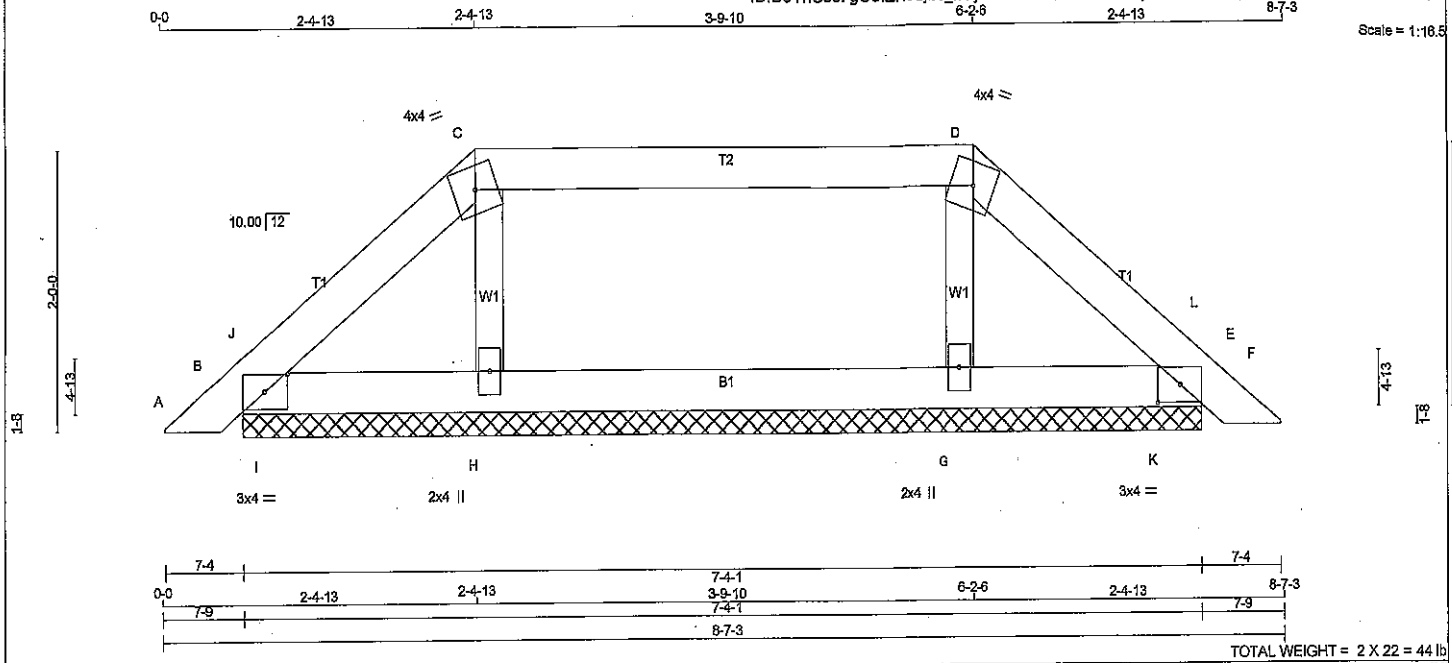
LICENSED PROFESSIONAL ENGINEER
3526
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12240-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	PB01	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:01 2018 Page 1
ID:B6YhSsc7gCoIZH36jbs_hTyW4u8-WQBensGY?aq1ZSDtvZ9xYg?rMbsEh4LjF967wdzdsp0



TOTAL WEIGHT = 2 X 22 = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	211	0	211	0	0	7-4-1	7-4-1
E	211	0	211	0	0	7-4-1	7-4-1
H	329	0	329	0	0	7-4-1	7-4-1
G	329	0	329	0	0	7-4-1	7-4-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	150	123 / 0	9 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	150	123 / 0	9 / 0	0 / 0	0 / 0	18 / 0	0 / 0
H	272	149 / 0	65 / 0	0 / 0	0 / 0	59 / 0	0 / 0
G	272	149 / 0	65 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	H- C	-224 / 0	0.03 (1)
B- J	-61 / 0	-104.9	-104.9	0.01 (1)	6.25	G- D	-224 / 0	0.03 (1)
J- C	-82 / 0	-104.9	-104.9	0.04 (1)	6.25	I- J	-121 / 0	0.00 (1)
C- D	-44 / 0	-104.9	-104.9	0.26 (1)	6.25	K- L	-121 / 0	0.00 (1)
D- L	-82 / 0	-104.9	-104.9	0.04 (1)	6.25			
L- E	-61 / 0	-104.9	-104.9	0.01 (1)	6.25			
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00			
B- I	0 / 59	-27.5	-27.5	0.05 (1)	10.00			
I- H	0 / 59	-27.5	-27.5	0.06 (2)	10.00			
H- G	0 / 44	-27.5	-27.5	0.06 (2)	10.00			
G- K	0 / 59	-27.5	-27.5	0.06 (2)	10.00			
K- E	0 / 59	-27.5	-27.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-D:1) , BC=0.06/1.00 (H-I:2) , WB=0.03/1.00 (C-H:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

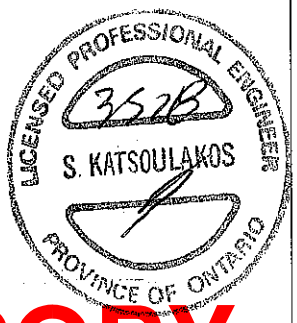
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



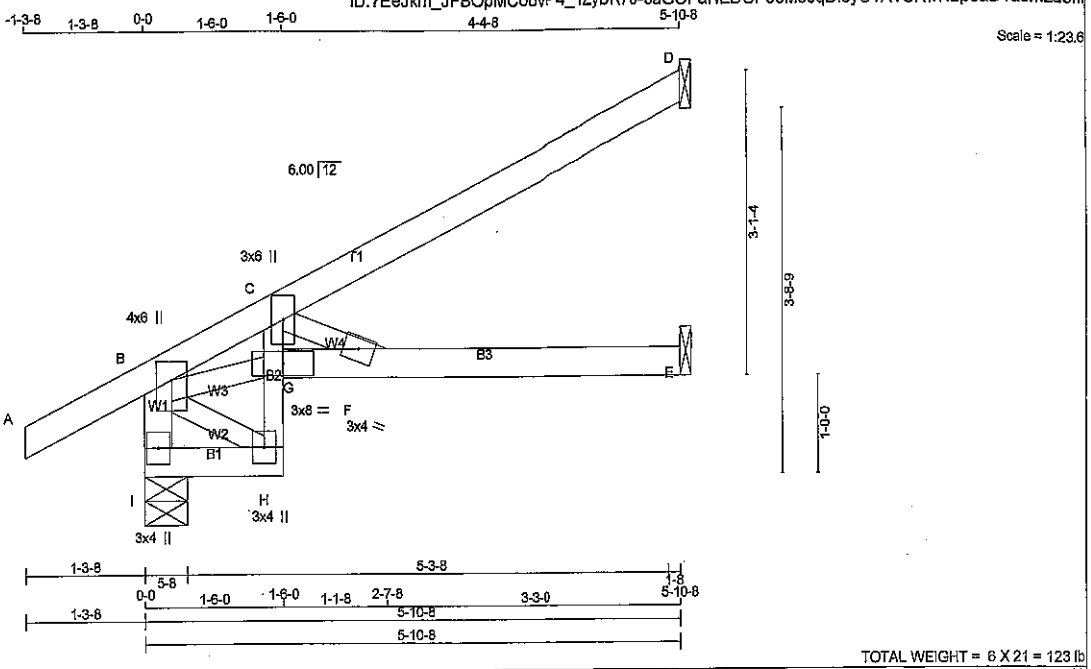
DWG NO. TAM 1222-1-183
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288629	J01A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:10:25 2018 Page 1

ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-3aGOPaHEDOP6eMe0qDfeyOTATSRwN2p5aDTasmzdsni



TOTAL WEIGHT = 6 X 21 = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	4.0	6.0	2.25	2.00
C	TMVW+p	MT20	3.0	6.0		
F	BMW+w	MT20	3.0	4.0		
G	BVMW-I	MT20	3.0	6.0		
H	BMVW+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	548	0	548	0	0	5-8	5-8
D	238	0	238	0	0	1-8	1-8
E	135	0	143	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	416	289 / 0	59 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	168	139 / 0	9 / 0	0 / 0	0 / 0	19 / 0	0 / 0
E	129	41 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-529 / 0	0.0	0.0	0.05 (1)		7.81
A- B	0 / 32	-104.9	-104.9	0.14 (1)		10.00
B- C	-644 / 0	-104.9	-104.9	0.14 (1)		6.25
C- D	0 / 10	-104.9	-104.9	0.37 (1)		10.00
I- H	0 / 0	-27.5	-27.5	0.02 (3)		10.00
H- G	0 / 29	0.0	0.0	0.07 (1)		10.00
G- C	0 / 400	0.0	0.0	0.16 (1)		10.00
G- F	0 / 591	-27.5	-27.5	0.51 (1)		10.00
F- E	0 / 0	-27.5	-27.5	0.42 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- F	-689 / 0	0.10 (1)
B- G	0 / 522	0.12 (1)
B- H	0 / 30	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 670 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 413 (0.17")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.51/1.00 (F-G:1) , WB=0.12/1.00 (B-G:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.28 (F) (INPUT = 1.00)

DWG NO. TAM 17224-18
STRUCTURAL COMPONENT ONLY

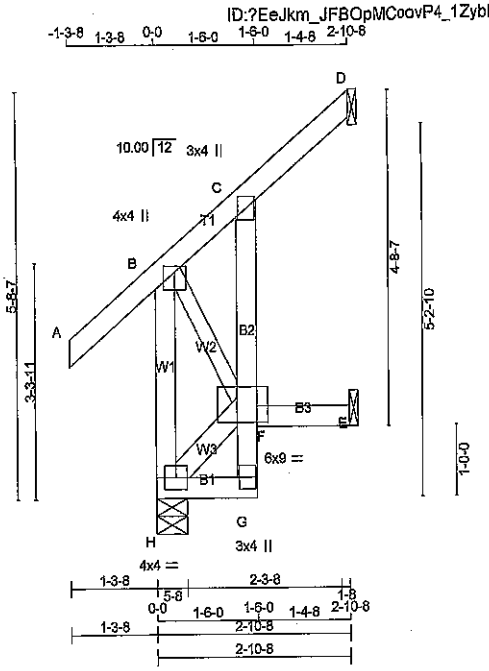
SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295462	J100S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 21:09:00 2018 Page 1
ID:?EeJkm_JFBOpMCoovP4_1ZybR7J-2DdFZWGwEHiAxIegMsei0SSiOBXiydca1VNaNBzdsp1

Scale = 1:32.4



TOTAL WEIGHT = 6 X 21 = 125 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
F	BVMWW-I	MT20	6.0	9.0	4.25	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	371	0	371	0	0	5-8	5-8
D	135	0	135	0	-19	1-8	1-8
E	21	0	37	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	274	205 / 0	30 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	102	72 / -22	14 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	27	0 / 0	16 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-349 / 0	0.0	0.0	0.07 (1)		7.81	H- F	-5 / 0	0.00 (1)
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00	B- F	-5 / 0	0.00 (2)
B- C	-88 / 0	-104.9	-104.9	0.14 (1)		6.25			
C- D	-15 / 32	-104.9	-104.9	0.12 (1)		6.25			
H- G	0 / 4	-28.0	-28.0	0.02 (3)		10.00			
G- F	0 / 33	0.0	0.0	0.01 (1)		10.00			
F- C	0 / 73	0.0	0.0	0.01 (2)		10.00			
F- E	0 / 0	-28.0	-28.0	0.02 (3)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (E-F:3) , WB=0.00/1.00 (B-F:2) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 122221B
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

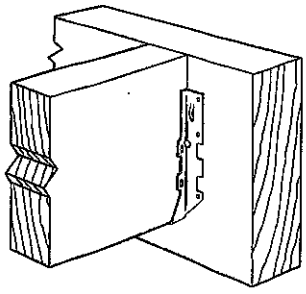
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

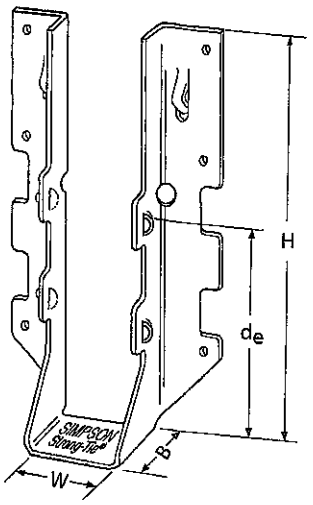
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



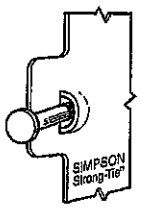
Typical LUS Installation



LUS28

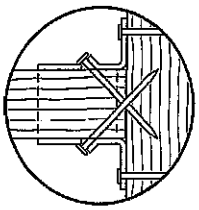
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

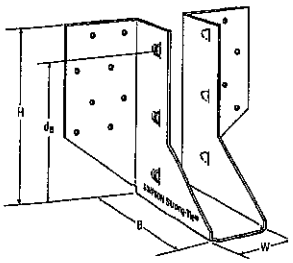
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

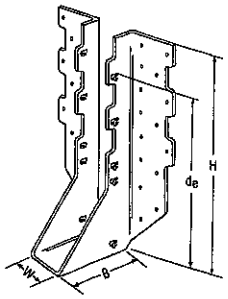
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

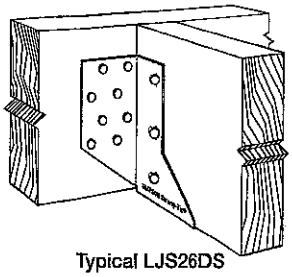
- See current catalogue for options



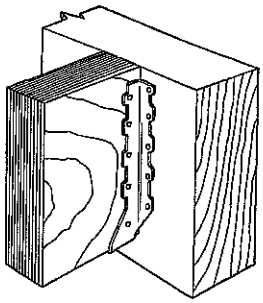
LJS26DS



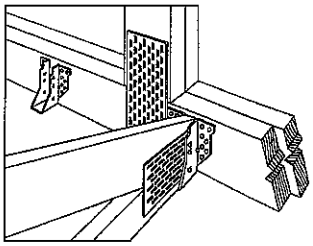
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



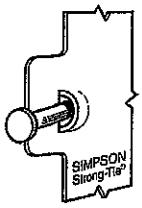
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

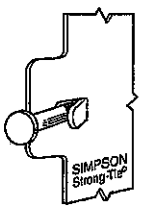
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

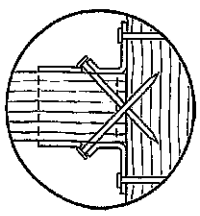


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

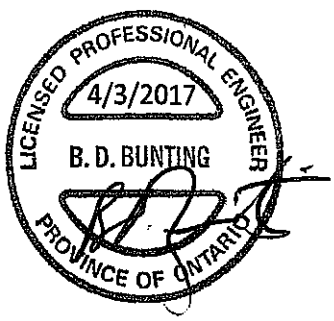
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

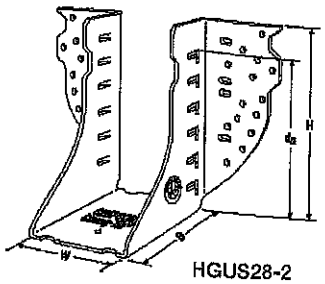
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

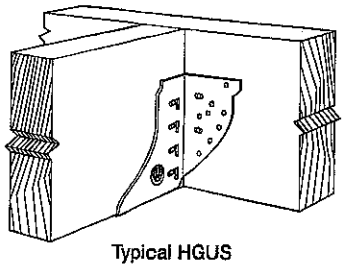
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

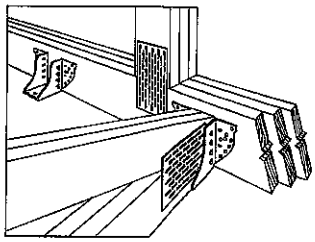
Options:

- See current catalogue for options



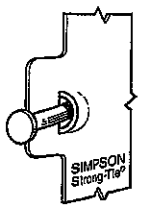
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

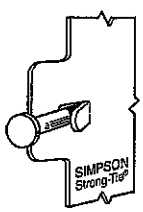
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

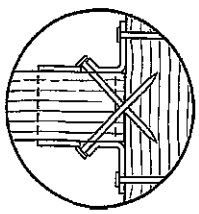


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

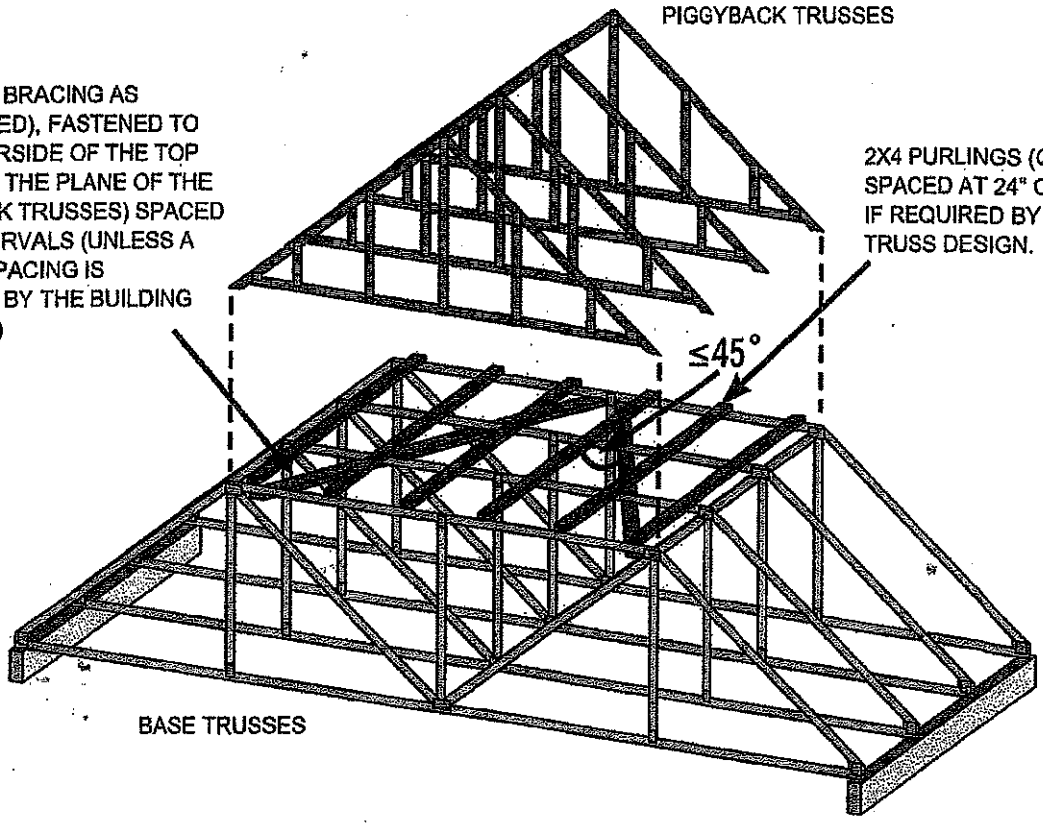
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

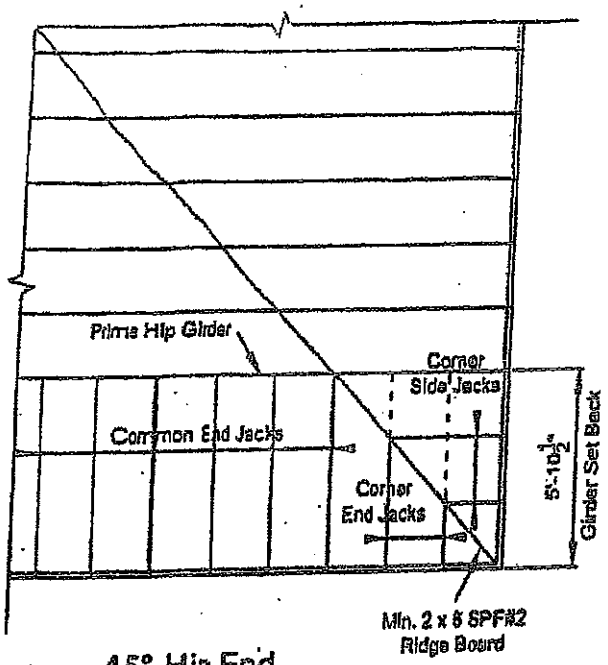
Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

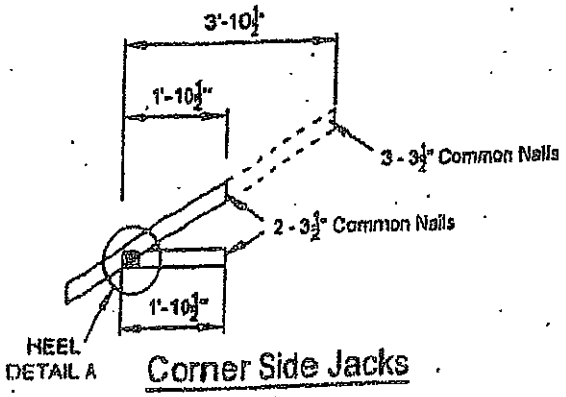
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

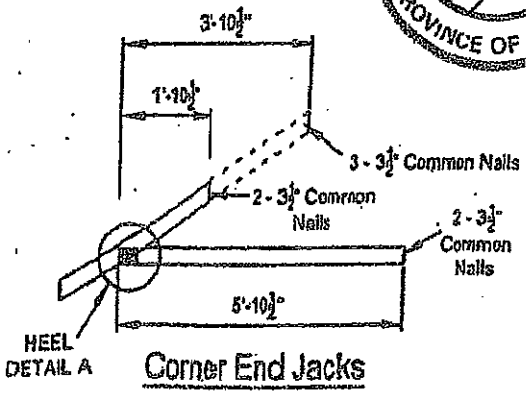
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495 14 R1
STRUCTURAL COMPONENT ONLY

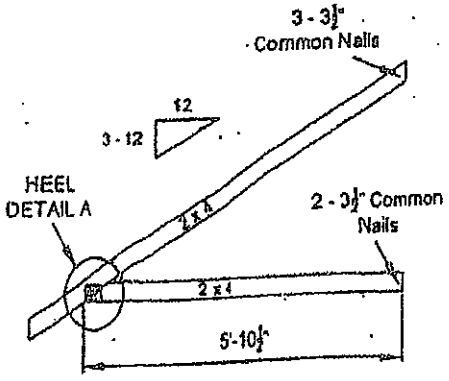
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



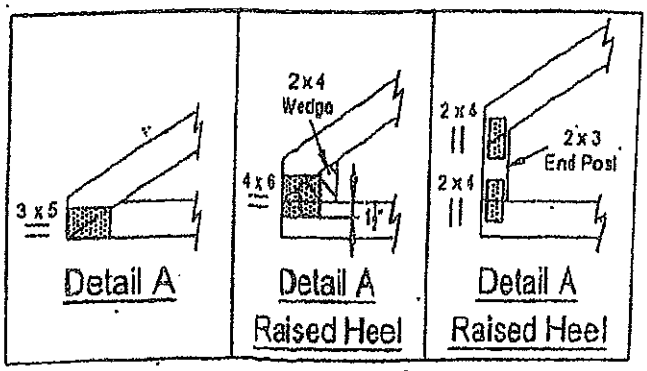
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

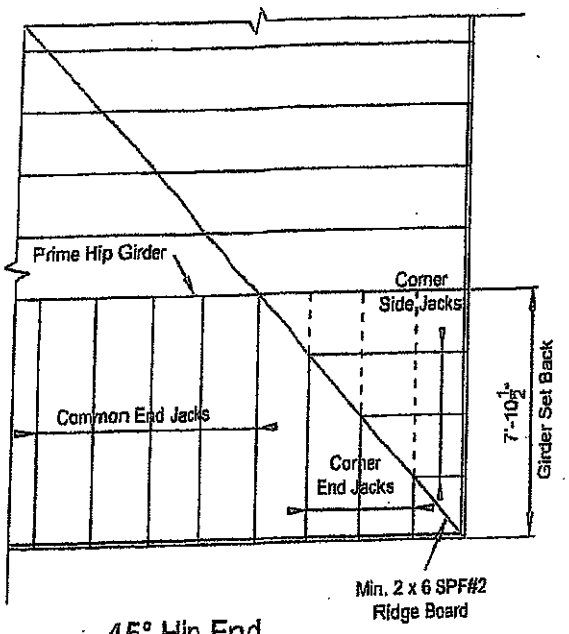
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



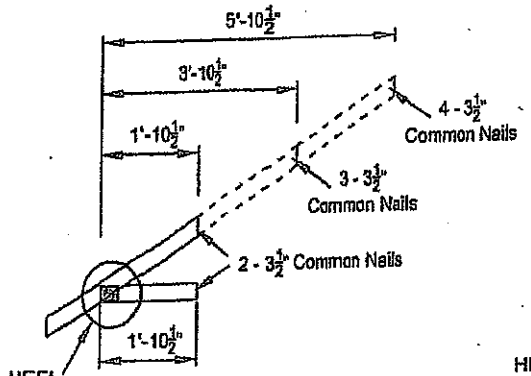
45° Hip End

DESIGN LOAD:

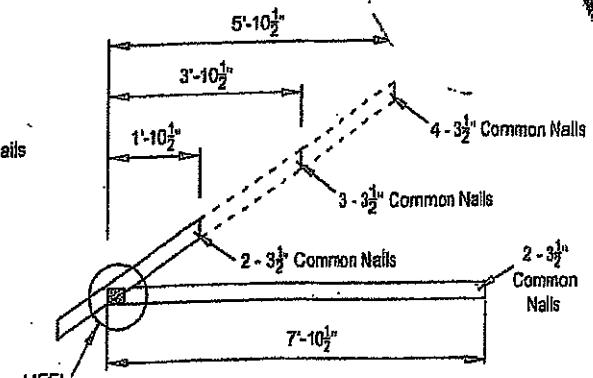
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY

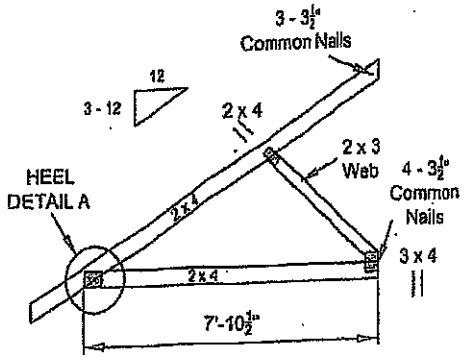
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



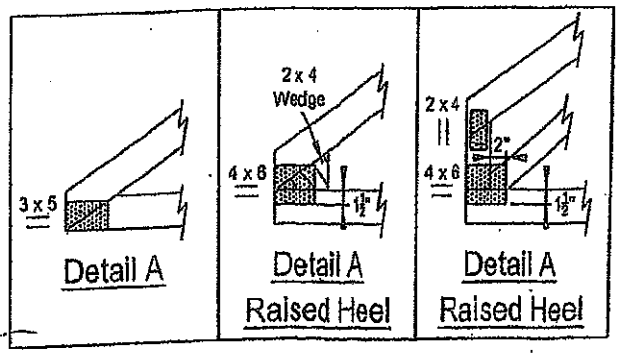
Corner Side Jacks



Corner End Jacks



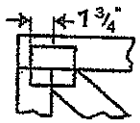
Common End Jacks



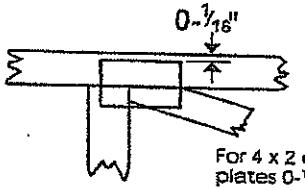
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

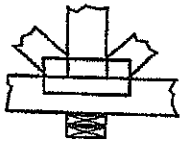
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

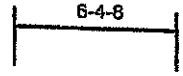
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

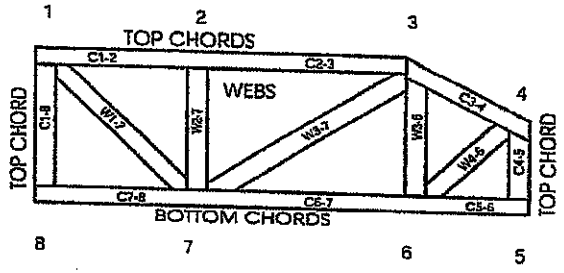
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.®

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

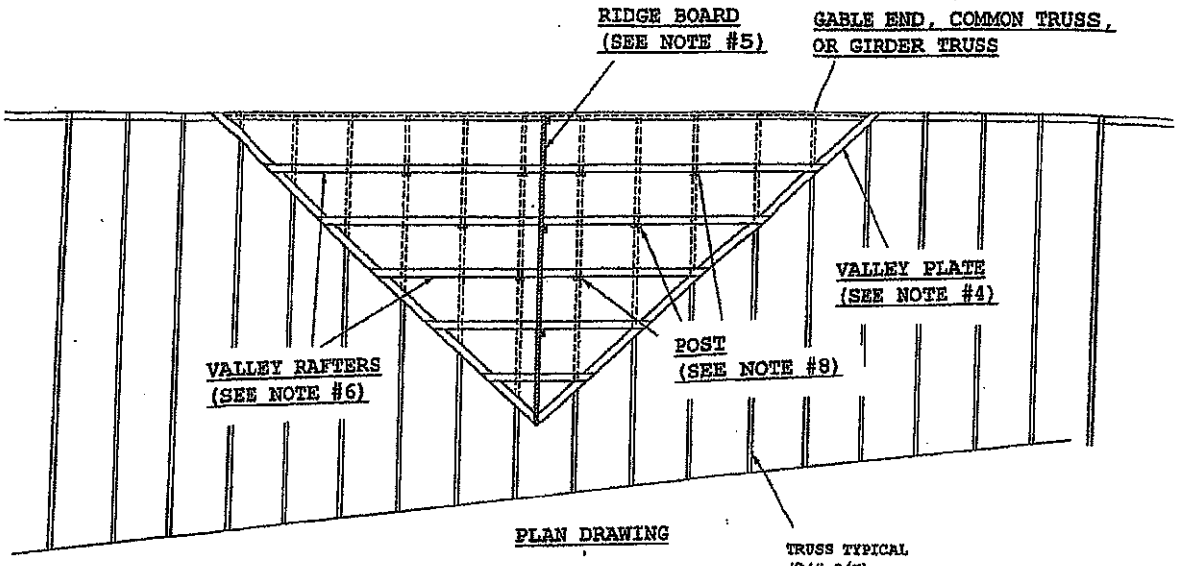
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY

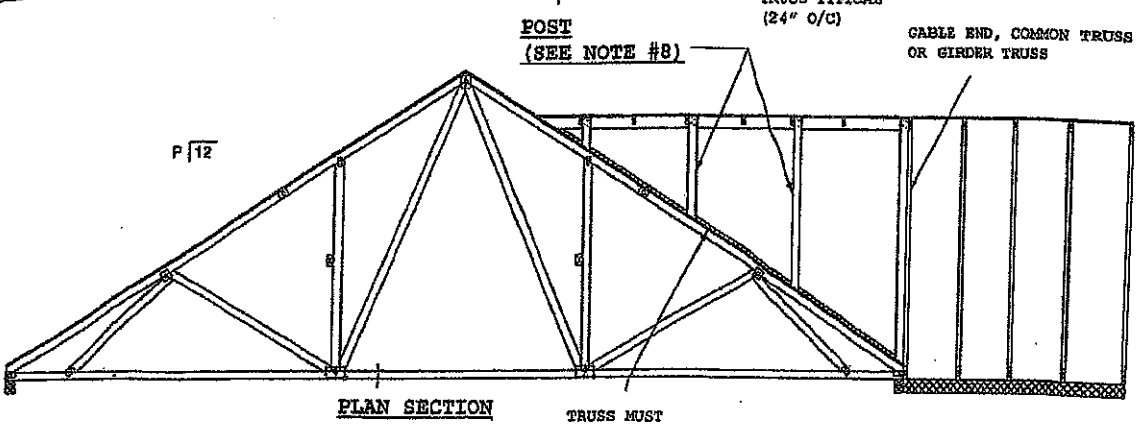
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOE CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.0 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2078
S. KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO [illegible] 7/24/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.