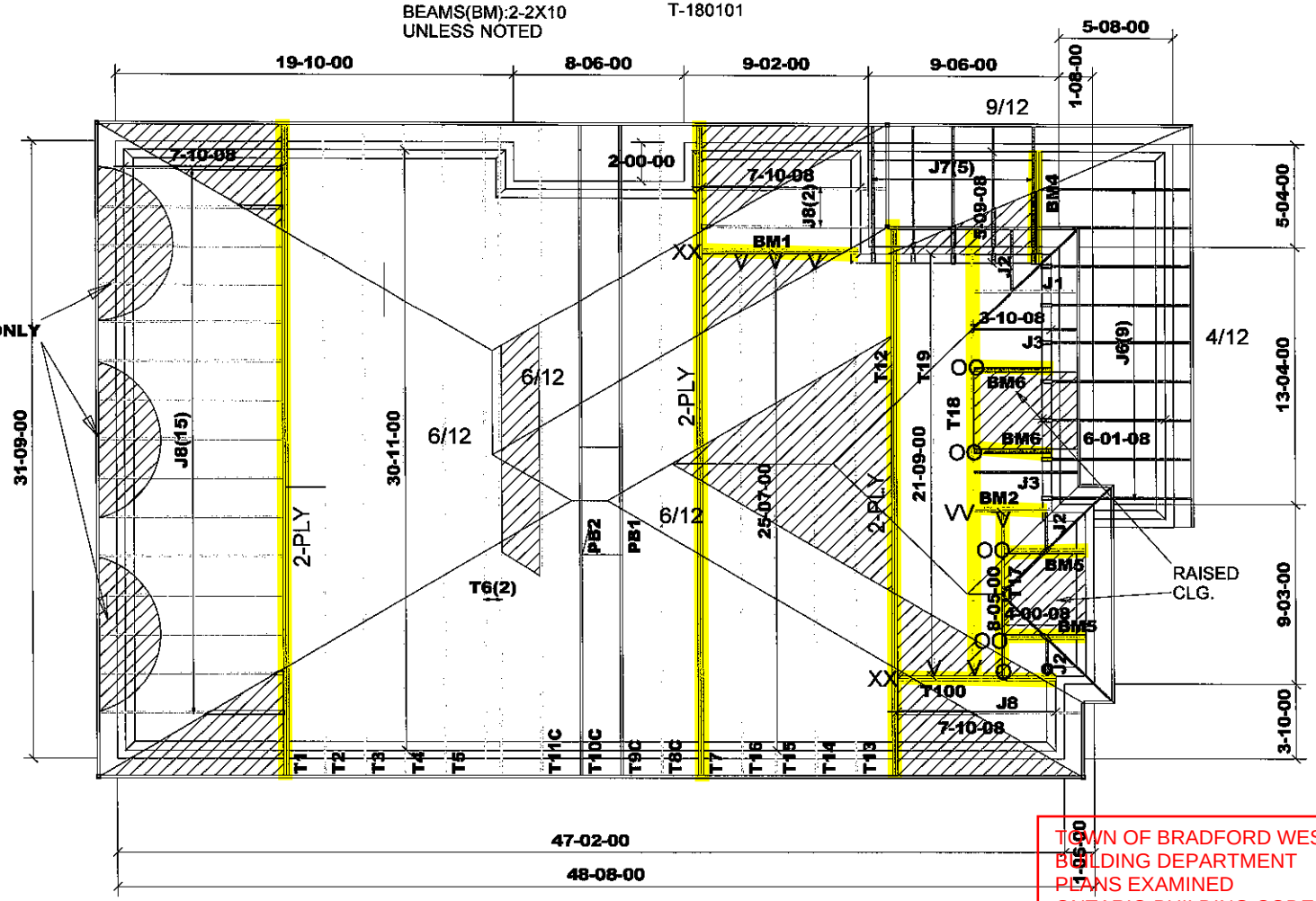
10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)
LUS26-2(VV)

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-10-24
INSPECTOR: BG

SITE COPY

M11,446

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	297473
Plan Log:	96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: **GREEN VALLEY EAST**

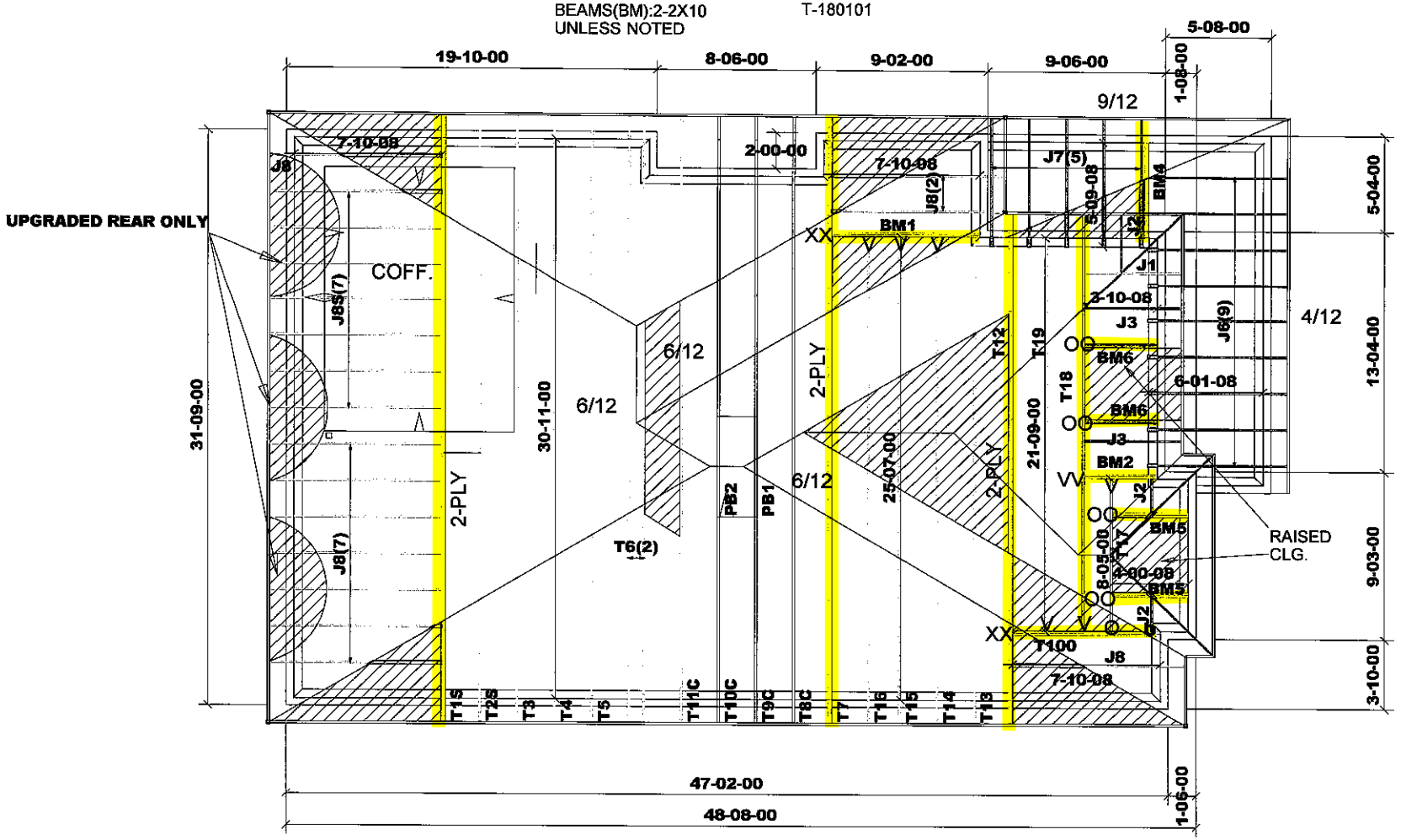
Date: 2/26/2018 Designer: JG

Model / Elevation:
S38-3 / A - STD

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)
LUS26-2(VV)

SITE COPY

M11,446

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S38-3 / A - OPT.COFF
Layout ID: 287443	Project: GREEN VALLEY EAST		
Plan Log: 96660	Date: 2/26/2018	Designer: JG	

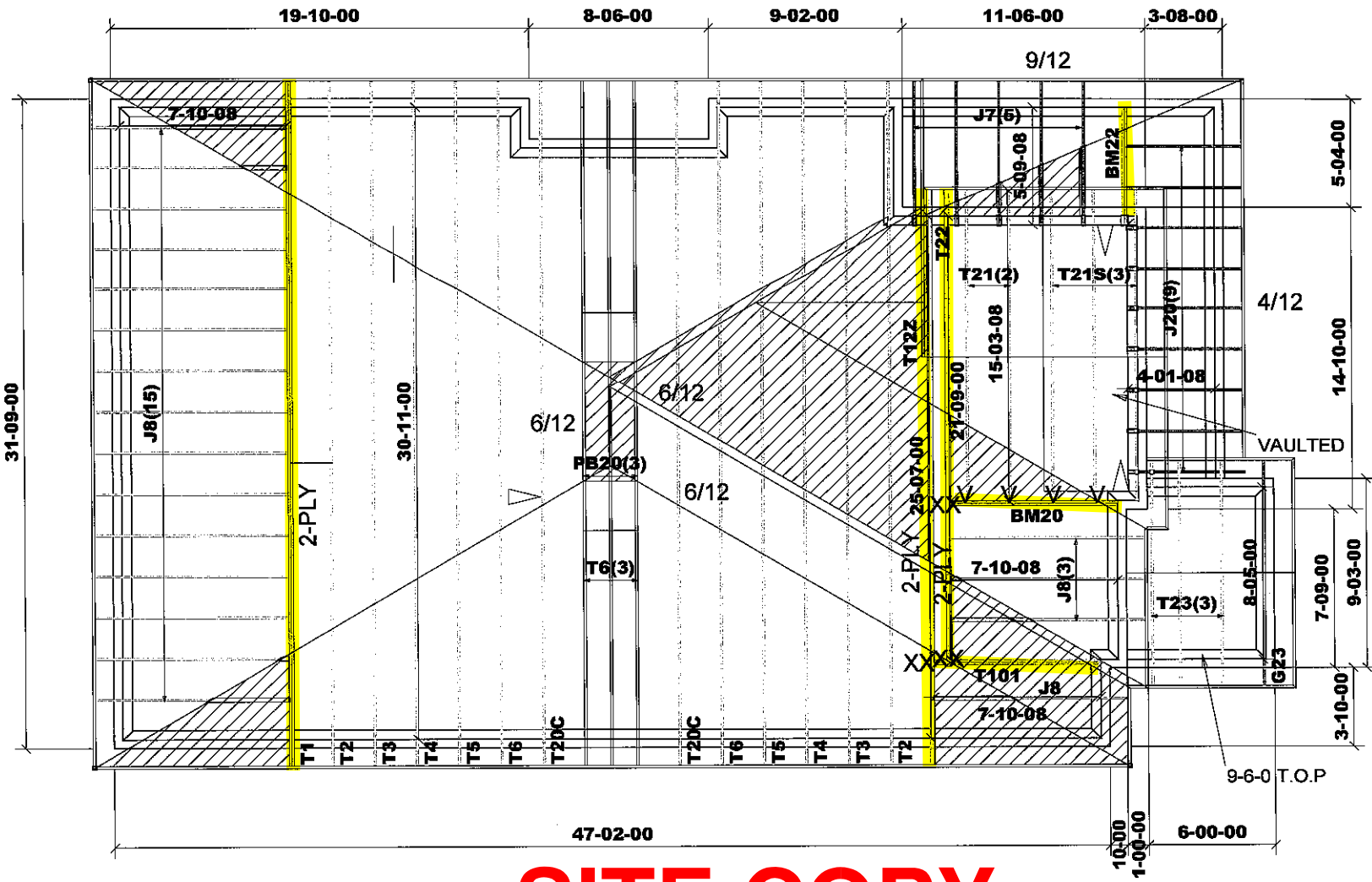
Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.



DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,446

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287475
Plan Log: 96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

Date: 2/26/2018 Designer: JG

Model / Elevation:
S38-3 / B - STD.

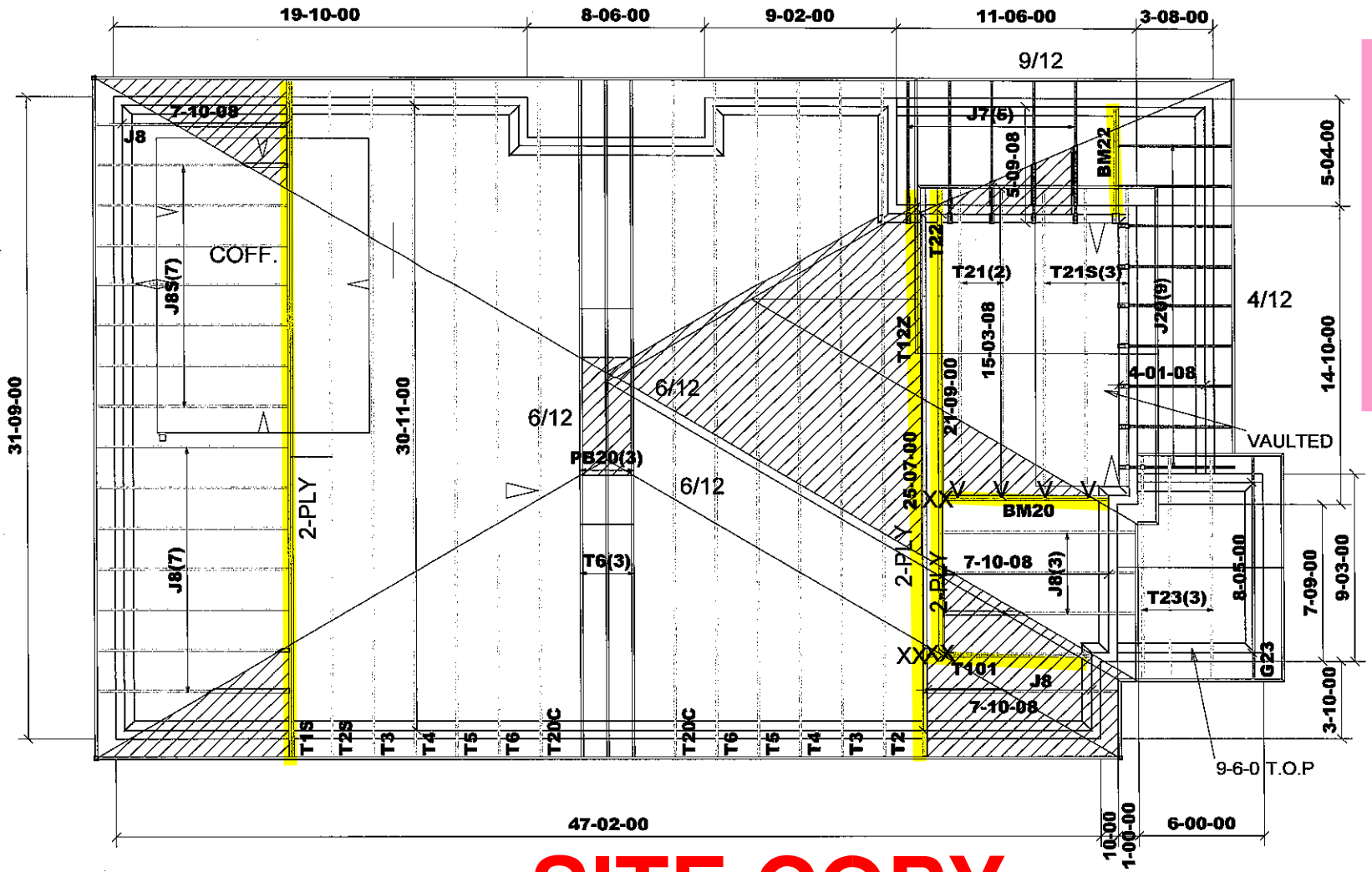
Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

M11,446

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287444
Plan Log: 93767

Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

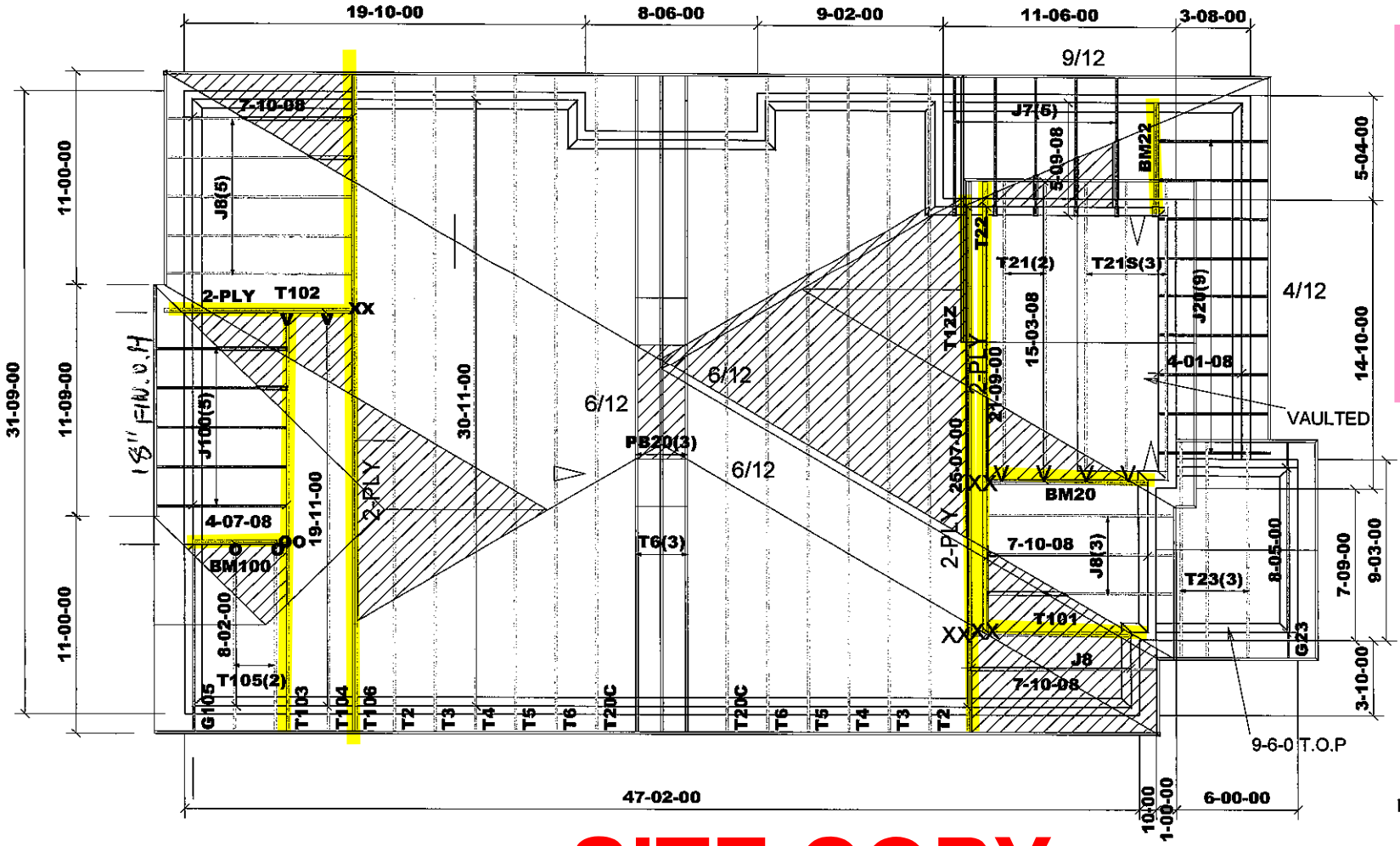
Date: 2/26/2018 Designer: JG

Model / Elevation: S38-3 / B - OPT.COFF

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)
LUS24(O)
LUS24-2(OO)

SITE COPY

M11,446

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: **44755**
Layout ID: **295208**
Plan Log: **96660**

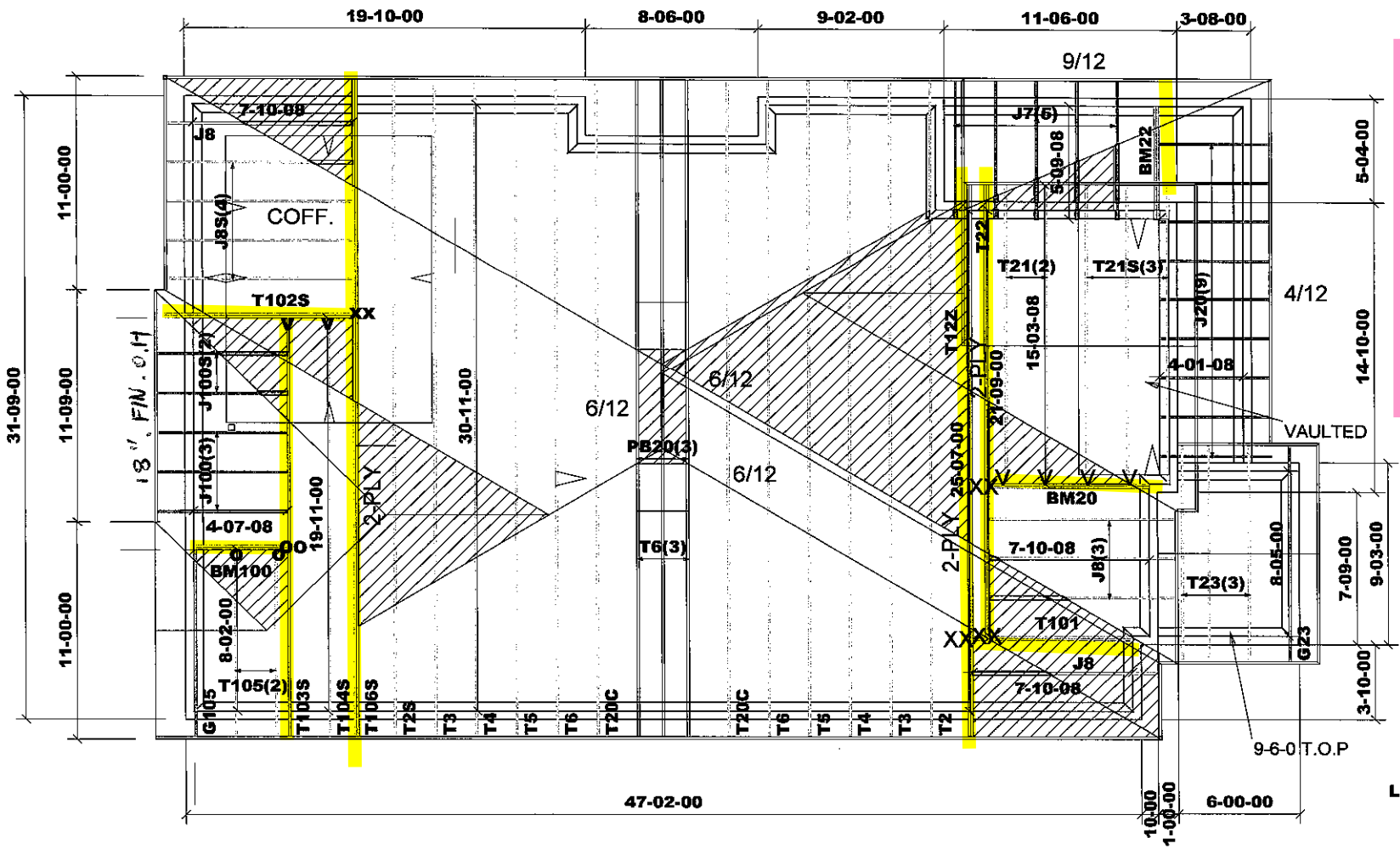
Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD
Project: **GREEN VALLEY EAST**
Date: **2/26/2018** Designer: JG

Model / Elevation:
S38-3 / B - REAR UPG

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)
LUS24-2(OO)
LUS24(O)

SITE COPY

M11,440

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	295207
Plan Log:	96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: **GREEN VALLEY EAST**

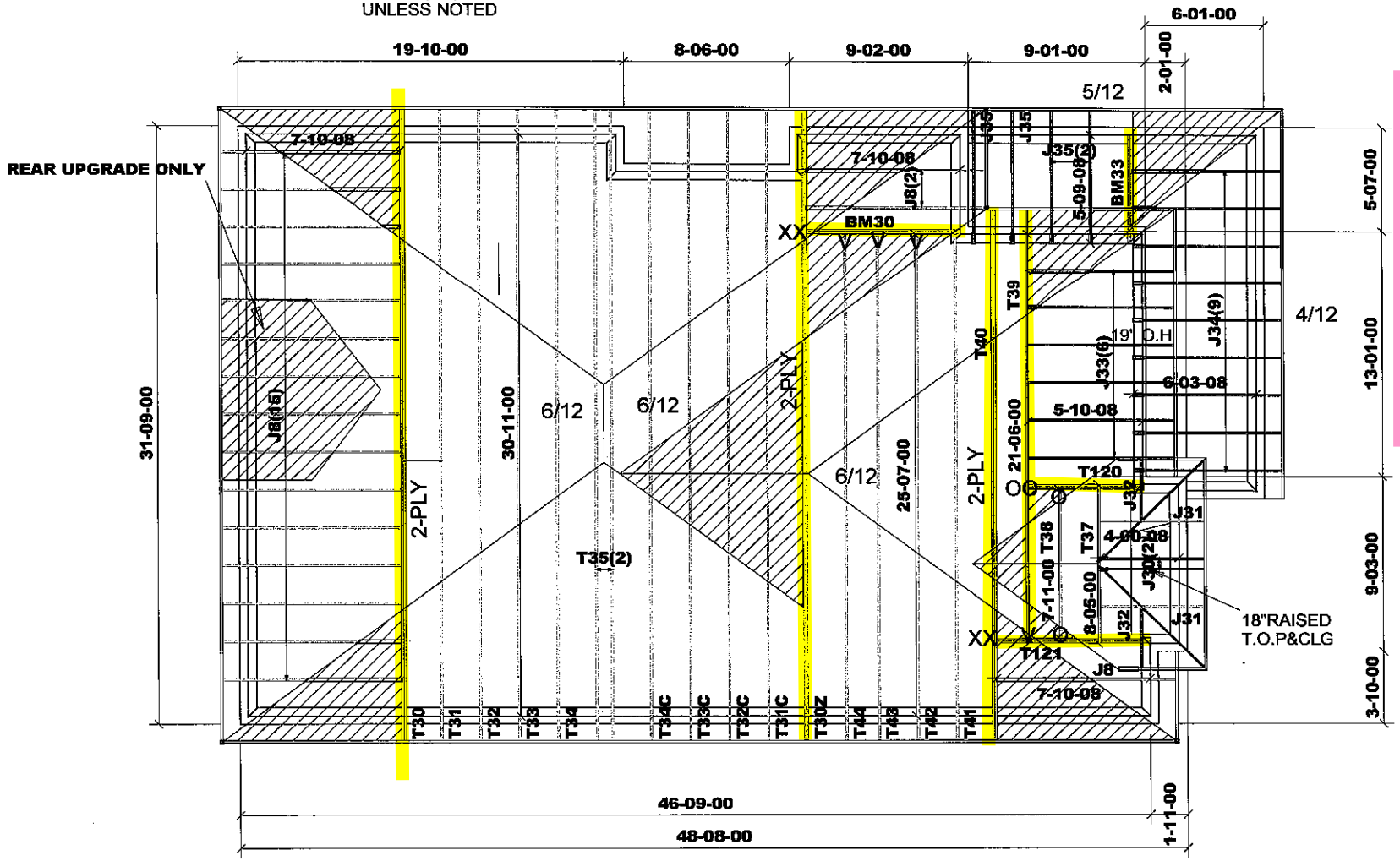
Date: 2/26/2018 Designer: JG

Model / Elevation:
S38-3 / B - OPT.COFF.REAR UPG

Mitek ver 7.5.0

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-18010



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)

SITE COPY

M11,440

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287476
Plan Log: 96660

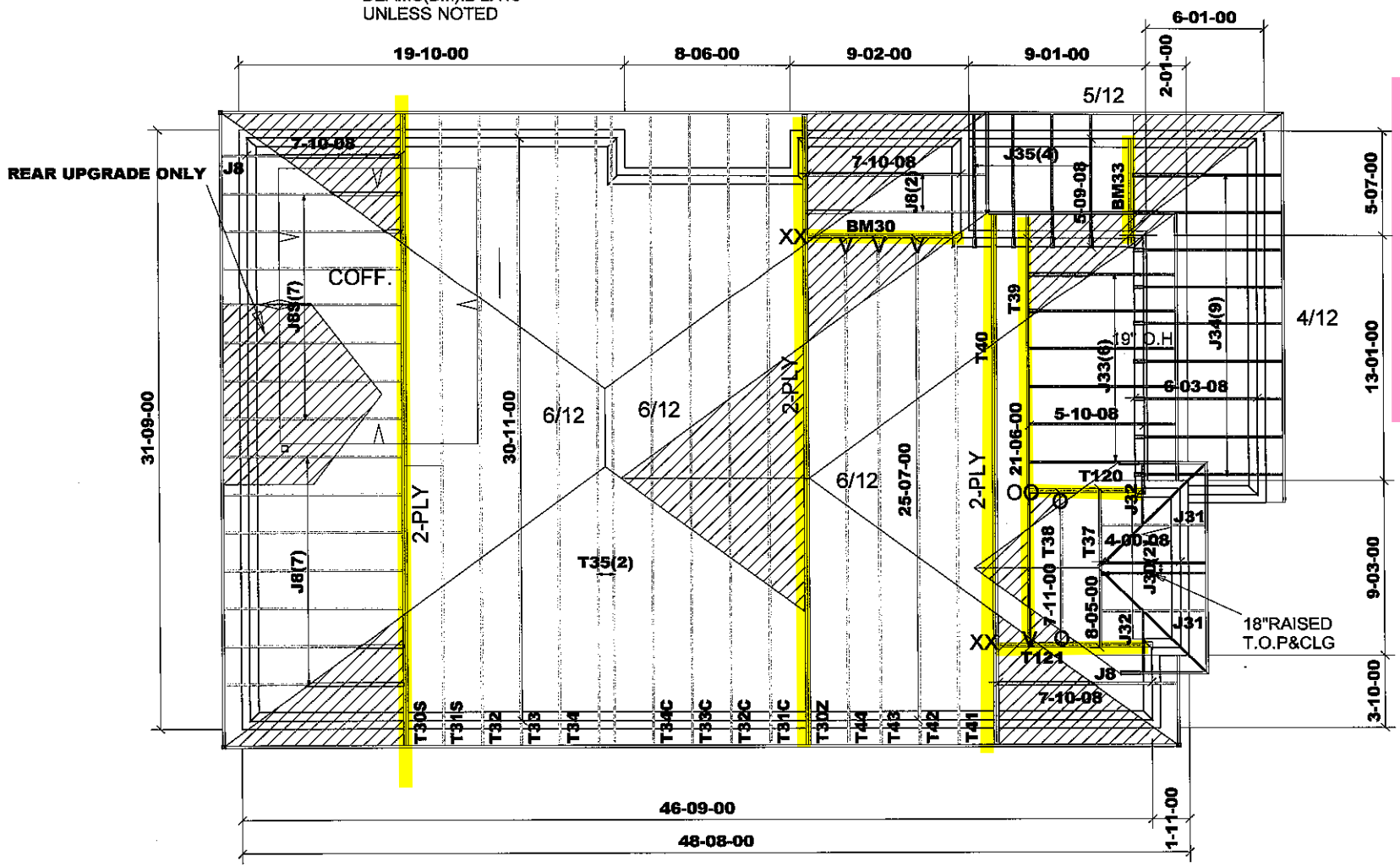
Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD
Project: GREEN VALLEY EAST
Date: 2/27/2018 Designer: JG

Model / Elevation: S38-3 / C

Mitek ver 7.5.0

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-18010



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)

SITE COPY

M11,446

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	287445
Plan Log:	96660

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Model / Elevation: **S38-3 / C - OPT.COFF**

Project: **GREEN VALLEY EAST**

Date: 2/27/2018 Designer: JG

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

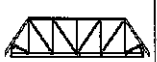
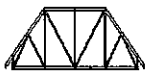
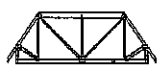
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287473 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	297.52 185.00		
	1	T2 HIP	10.00 0.00	30-11-00	09-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.58 80.83		
	1	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.35 87.50		
	1	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.74 92.83		
	1	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	154.27 96.50		
	2	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.10 201.34		
	1 2 Ply	T7 ROOF	10.00 0.00	30-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	362.96 228.00		
	1	T8C ROOF	10.00 0.00	30-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.20 109.67		
	1	T9C PIGGYBACK	10.00 0.00	30-11-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.08 100.83		
	1	T10C PIGGYBACK	10.00 0.00	30-11-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	162.61 103.00		
	1	T11C HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	152.46 95.50		
	1 2 Ply	T12 HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	1	T13 HIP	10.00 0.00	25-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.35 72.00		
	1	T14 HIP	10.00 0.00	25-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.14 72.83		
	1	T15 HIP	10.00 0.00	25-07-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.04 77.67		
	1	T16 HIP	10.00 0.00	25-07-00	08-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.94 85.17		
	1	T17 HIP GIRDER	10.00 0.00	08-05-00	05-00-02	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	35.37 23.17		
	1	T18 HIP GIRDER	10.00 0.00	21-09-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	112.50 68.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

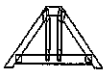
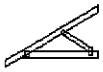
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287473 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T19 HIP	10.00 0.00	21-09-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	102.12 64.33		
	1 2 Ply	T100 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	82.24 51.34		
	1	PB1 PIGGYBACK	10.00 0.00	04-03-02	01-09-12	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	14.75 11.33		
	1	PB2 PIGGYBACK	10.00 0.00	04-03-02	02-02-02	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	13.05 8.67		
	1	J1 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	12.02 8.33		
	3	J2 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	25.11 18.00		
	2	J3 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	31.14 20.34		
	9	J6 JACK-OPEN	4.00 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-07	163.08 103.50		
	5	J7 JACK-OPEN	9.00 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 00-00-00	00-11-00 05-03-02	87.50 56.65		
	18	J8 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	543.96 347.94		

TOTAL # TRUSS= **65** TOTAL BFT OF ALL TRUSSES= **2636.94** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4166.64** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
4	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **15**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

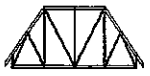
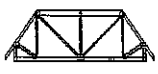
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287443 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.00 204.00		
	1	T2S HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	143.71 92.17		
	1	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	140.35 87.50		
	1	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.74 92.83		
	1	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	154.27 96.50		
	2	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.10 201.34		
	1 2 Ply	T7 ROOF	10.00 0.00	30-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	362.96 228.00		
	1	T8C ROOF	10.00 0.00	30-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	174.20 109.67		
	1	T9C PIGGYBACK	10.00 0.00	30-11-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	159.08 100.83		
	1	T10C PIGGYBACK	10.00 0.00	30-11-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	162.61 103.00		
	1	T11C HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	152.46 95.50		
	1 2 Ply	T12 HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	1	T13 HIP	10.00 0.00	25-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.35 72.00		
	1	T14 HIP	10.00 0.00	25-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	116.14 72.83		
	1	T15 HIP	10.00 0.00	25-07-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.04 77.67		
	1	T16 HIP	10.00 0.00	25-07-00	08-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.94 85.17		
	1	T17 HIP GIRDER	10.00 0.00	08-05-00	05-00-02	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	35.37 23.17		
	1	T18 HIP GIRDER	10.00 0.00	21-09-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	112.50 68.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

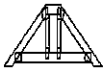
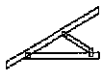
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287443 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T19 HIP	10.00 0.00	21-09-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	102.12 64.33		
	1 2 Ply	T100 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	82.24 51.34		
	1	PB1 PIGGYBACK	10.00 0.00	04-03-02	01-09-12	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	14.75 11.33		
	1	PB2 PIGGYBACK	10.00 0.00	04-03-02	02-02-02	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	13.05 8.67		
	1	J1 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	12.02 8.33		
	3	J2 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	25.11 18.00		
	2	J3 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	31.14 20.34		
	9	J6 JACK-OPEN	4.00 0.00	06-01-08	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-07	163.08 103.50		
	5	J7 JACK-OPEN	9.00 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 00-00-00	00-11-00 05-03-02	87.50 56.65		
	11	J8 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	332.42 212.63		
	7	J8S JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	222.18 152.81		

TOTAL # TRUSS= **65.00** TOTAL BFT OF ALL TRUSSES= **2684.78** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4212.89** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
4	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **15.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

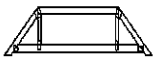
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287475 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1 HIP GIRDER	10.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	297.54 185.00		
	2	T2 HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	257.16 161.66		
	2	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	280.70 175.00		
	2	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	295.48 185.66		
	2	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	308.54 193.00		
	5	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	805.25 503.35		
	1 2 Ply	T12Z HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	2	T20C HIP	10.00 0.00	30-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	373.94 234.00		
	2	T21 COMMON	10.00 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	132.22 87.00		
	3	T21S SCISSORS	10.00 4.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.15 122.49		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	21-09-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	226.58 144.00		
	3	T23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.11 86.01		
	1	G23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	45.19 30.67		
	1 2 Ply	T101 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	86.32 53.66		
	3	PB20 PIGGYBACK	10.00 0.00	09-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	79.50 54.00		
	5	J7 JACK-OPEN	9.00 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 00-00-00	00-11-00 05-03-02	87.50 56.65		
	19	J8 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	574.18 367.27		
	9	J20 JACK-OPEN	4.00 0.00	04-01-08	01-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-08-07	101.97 65.97		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2871.73** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4532.79** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK:	44755	LAYOUT ID: 287475	LOCATION: BRADFORD
BUILDER:	BAYVIEW WELLINGTON-GREEN VALLE		SUB-BUILDER:
MODEL:	S38-3		ELEVATION: B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

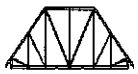
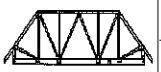
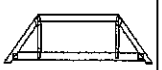
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287444 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.00 204.00		
	1	T2 HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.58 80.83		
	1	T2S HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	143.71 92.17		
	2	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	280.70 175.00		
	2	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	295.48 185.66		
	2	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	308.54 193.00		
	5	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	805.25 503.35		
	1 2 Ply	T12Z HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	2	T20C HIP	10.00 0.00	30-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	373.94 234.00		
	2	T21 COMMON	10.00 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	132.22 87.00		
	3	T21S SCISSORS	10.00 4.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.15 122.49		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	21-09-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	226.58 144.00		
	3	T23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.11 86.01		
	1	G23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	45.19 30.67		
	1 2 Ply	T101 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	86.32 53.66		
	3	PB20 PIGGYBACK	10.00 0.00	09-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	79.50 54.00		
	5	J7 JACK-OPEN	9.00 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 00-00-00	00-11-00 05-03-02	87.50 56.65		
	12	J8 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	362.64 231.96		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287444 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	7	J8S JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	222.18 / 152.81		
	9	J20 JACK-OPEN	4.00 / 0.00	04-01-08	01-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-08-07	101.97 / 65.97		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2919.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4579.02** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

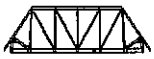
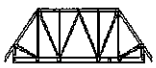
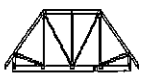
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295208 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B-REAR UPG

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T2 HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	257.16 161.66		
	2	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	280.70 175.00		
	2	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	295.48 185.66		
	2	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	308.54 193.00		
	5	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	805.25 503.35		
	1 2 Ply	T12Z HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	2	T20C HIP	10.00 0.00	30-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	373.94 234.00		
	2	T21 COMMON	10.00 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	132.22 87.00		
	3	T21S SCISSORS	10.00 4.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.15 122.49		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	21-09-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	226.58 144.00		
	3	T23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.11 86.01		
	1	G23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	45.19 30.67		
	1 2 Ply	T101 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	86.32 53.66		
	1 2 Ply	T102 JACK-CLOSED	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 6	01-05-00 00-00-00	01-02-00 05-01-04	82.00 51.34		
	1	T103 HIP GIRDER	10.00 0.00	19-11-00	05-10-15	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	102.18 63.67		
	1	T104 HIP	10.00 0.00	19-11-00	07-06-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	93.04 59.50		
	2	T105 COMMON	10.00 0.00	08-02-00	05-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.94 51.66		
	1	G105 COMMON	10.00 0.00	08-02-00	05-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	40.08 27.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

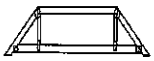
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295208 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-3 **ELEVATION:** B-REAR UPG

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T106 HIP GIRDER	10.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	297.74 185.00		
	3	PB20 PIGGYBACK	10.00 0.00	09-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	79.50 54.00		
	5	J7 JACK-OPEN	9.00 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 00-00-00	00-11-00 05-03-02	87.50 56.65		
	9	J8 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	271.98 173.97		
	9	J20 JACK-OPEN	4.00 0.00	04-01-08	01-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-08-07	101.97 65.97		
	5	J100 JACK-OPEN	10.00 0.00	04-07-08	05-10-15	2 X 4	2 X 4	01-09-08 00-00-00	02-00-11 05-10-15	94.70 60.85		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2993.28** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4719.73** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **19.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

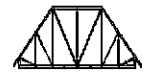
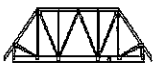
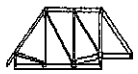
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295207 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B-OPT.COFF.REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T2 HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.58 80.83		
	1	T2S HIP	10.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	143.71 92.17		
	2	T3 HIP	10.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	280.70 175.00		
	2	T4 HIP	10.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	295.48 185.66		
	2	T5 HIP	10.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	308.54 193.00		
	5	T6 HIP	10.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	805.25 503.35		
	1 2 Ply	T12Z HIP GIRDER	10.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	258.46 166.34		
	2	T20C HIP	10.00 0.00	30-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	373.94 234.00		
	2	T21 COMMON	10.00 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	132.22 87.00		
	3	T21S SCISSORS	10.00 4.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.15 122.49		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	21-09-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	226.58 144.00		
	3	T23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.11 86.01		
	1	G23 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	45.19 30.67		
	1 2 Ply	T101 HALF HIP	6.00 0.00	07-10-08	04-07-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-07-08	86.32 53.66		
	1 2 Ply	T102S JACK-CLOSED	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	80.32 53.66		
	1	T103S HIP GIRDER	10.00 0.00	19-11-00	05-10-15	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 00-07-11	106.97 68.00		
	1	T104S HIP	10.00 0.00	19-11-00	07-06-15	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-07-11	100.14 63.50		
	2	T105 COMMON	10.00 0.00	08-02-00	05-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	76.94 51.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

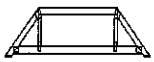
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295207 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: B-OPT.COFF.REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	G105 COMMON	10.00 / 0.00	08-02-00	05-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	40.08 / 27.83		
	1 / 2 Ply	T106S HIP GIRDER	10.00 / 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	345.60 / 220.34		
	3	PB20 PIGGYBACK	10.00 / 0.00	09-04-02	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	79.50 / 54.00		
	5	J7 JACK-OPEN	9.00 / 0.00	05-09-08	05-03-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-00 / 05-03-02	87.50 / 56.65		
	5	J8 JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-01-04	151.10 / 96.65		
	4	J8S JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	126.96 / 87.32		
	9	J20 JACK-OPEN	4.00 / 0.00	04-01-08	01-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-08-07	101.97 / 65.97		
	3	J100 JACK-OPEN	10.00 / 0.00	04-07-08	05-10-15	2 X 4	2 X 4	01-09-08 / 00-00-00	02-00-11 / 05-10-15	56.82 / 36.51		
	2	J100S JACK-OPEN	10.00 / 0.00	04-07-08	05-10-15	2 X 4	2 X 4	01-09-08 / 00-00-00	02-00-11 / 04-10-15	45.44 / 30.00		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **3066.27** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4806.57** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

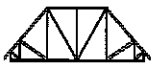
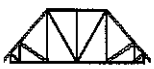
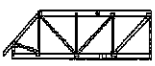
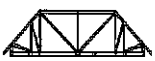
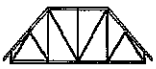
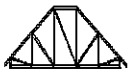
Delivery Shiplist

DATE	02/27/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287476 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T30 HIP GIRDER	8.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.82 185.00		
	1 2 Ply	T30Z HIP GIRDER	8.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.82 185.00		
	1	T31 HIP	8.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.82 85.00		
	1	T31C HIP	8.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.82 85.00		
	1	T32 HIP	8.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.06 83.17		
	1	T32C HIP	8.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.06 83.17		
	1	T33 HIP	8.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	143.30 91.00		
	1	T33C HIP	8.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	143.30 91.00		
	1	T34 HIP	8.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	146.04 92.67		
	1	T34C HIP	8.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	146.04 92.67		
	2	T35 HIP	8.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	284.70 177.34		
	1	T37 HIP GIRDER	8.00 0.00	08-05-00	04-01-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	32.91 21.33		
	1	T38 COMMON	8.00 0.00	07-11-00	05-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-00-13 03-00-13	37.62 24.00		
	1	T39 HIP GIRDER	8.00 0.00	21-06-00	04-03-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 03-09-12	111.83 68.00		
	1 2 Ply	T40 HIP GIRDER	8.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	251.52 157.00		
	1	T41 HIP	8.00 0.00	25-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	108.86 68.33		
	1	T42 HIP	8.00 0.00	25-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	118.56 75.50		
	1	T43 HIP	8.00 0.00	25-07-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	114.03 71.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

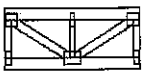
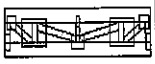
Delivery Shiplist

DATE	02/27/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287476 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T44 HIP	8.00 / 0.00	25-07-00	08-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	118.58 / 74.50		
	1 / 2 Ply	T120 FLAT GIRDER	0.00 / 0.00	05-10-08	01-06-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-00 / 01-06-00	41.28 / 25.66		
	1 / 2 Ply	T121 FLAT GIRDER	0.00 / 0.00	07-10-08	01-06-00	2 X 6	2 X 6	00-00-00 / 00-00-00	01-06-00 / 01-06-00	73.96 / 48.34		
	18	J8 JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-01-04	543.96 / 347.94		
	2	J30 JACK-OPEN	8.00 / 0.00	04-00-08	04-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-01-02	29.94 / 20.34		
	2	J31 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 02-03-01	01-04-13 / 00-03-08	22.76 / 15.34		
	2	J32 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	01-04-13 / 00-03-08	15.34 / 9.34		
	6	J33 JACK-OPEN	6.00 / 0.00	05-10-08	04-03-04	2 X 4	2 X 4	01-07-08 / 00-00-00	01-04-00 / 04-03-04	104.46 / 64.02		
	9	J34 JACK-OPEN	4.00 / 0.00	06-03-08	02-05-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-02	167.22 / 109.53		
	4	J35 JACK-OPEN	5.00 / 0.00	05-09-08	02-10-05	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-10-05	65.16 / 40.00		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2491.52** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3950.77** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **9.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

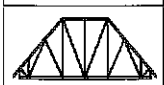
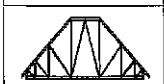
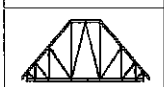
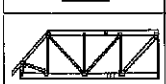
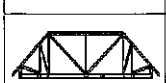
Delivery Shiplist

DATE	02/27/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287445 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: C-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T30S HIP GIRDER	8.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	311.60 199.66		
	1 2 Ply	T30Z HIP GIRDER	8.00 0.00	30-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.82 185.00		
	1	T31C HIP	8.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.82 85.00		
	1	T31S HIP	8.00 0.00	30-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	142.73 90.50		
	1	T32 HIP	8.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.06 83.17		
	1	T32C HIP	8.00 0.00	30-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.06 83.17		
	1	T33 HIP	8.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	143.30 91.00		
	1	T33C HIP	8.00 0.00	30-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	143.30 91.00		
	1	T34 HIP	8.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	146.04 92.67		
	1	T34C HIP	8.00 0.00	30-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	146.04 92.67		
	2	T35 HIP	8.00 0.00	30-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	284.70 177.34		
	1	T37 HIP GIRDER	8.00 0.00	08-05-00	04-01-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	32.91 21.33		
	1	T38 COMMON	8.00 0.00	07-11-00	05-08-08	2 X 4	2 X 4	00-00-00 00-00-00	03-00-13 03-00-13	37.62 24.00		
	1	T39 HIP GIRDER	8.00 0.00	21-06-00	04-03-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 03-09-12	111.83 68.00		
	1 2 Ply	T40 HIP GIRDER	8.00 0.00	25-07-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	251.52 157.00		
	1	T41 HIP	8.00 0.00	25-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	108.86 68.33		
	1	T42 HIP	8.00 0.00	25-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	118.56 75.50		
	1	T43 HIP	8.00 0.00	25-07-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	114.03 71.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

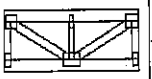
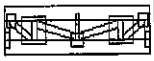
Delivery Shiplist

DATE	02/27/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287445 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-3 ELEVATION: C-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T44 HIP	8.00 / 0.00	25-07-00	08-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	118.58 / 74.50		
	1 / 2 Ply	T120 FLAT GIRDER	0.00 / 0.00	05-10-08	01-06-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-00 / 01-06-00	41.28 / 25.66		
	1 / 2 Ply	T121 FLAT GIRDER	0.00 / 0.00	07-10-08	01-06-00	2 X 6	2 X 6	00-00-00 / 00-00-00	01-06-00 / 01-06-00	73.96 / 48.34		
	11	J8 JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-01-04	332.42 / 212.63		
	7	J8S JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	222.18 / 152.81		
	2	J30 JACK-OPEN	8.00 / 0.00	04-00-08	04-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-01-02	29.94 / 20.34		
	2	J31 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 02-03-01	01-04-13 / 00-03-08	22.76 / 15.34		
	2	J32 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	01-04-13 / 00-03-08	15.34 / 9.34		
	6	J33 JACK-OPEN	6.00 / 0.00	05-10-08	04-03-04	2 X 4	2 X 4	01-07-08 / 00-00-00	01-04-00 / 04-03-04	104.46 / 64.02		
	9	J34 JACK-OPEN	4.00 / 0.00	06-03-08	02-05-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-02	167.22 / 109.53		
	4	J35 JACK-OPEN	5.00 / 0.00	05-09-08	02-10-05	2 X 4	2 X 4	01-03-08 / 00-00-00	00-05-06 / 02-10-05	65.16 / 40.00		

TOTAL # TRUSS= **70.00** TOTAL BFT OF ALL TRUSSES= **2529.18** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3982.10** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **9.00**

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:10 2018 Page 1

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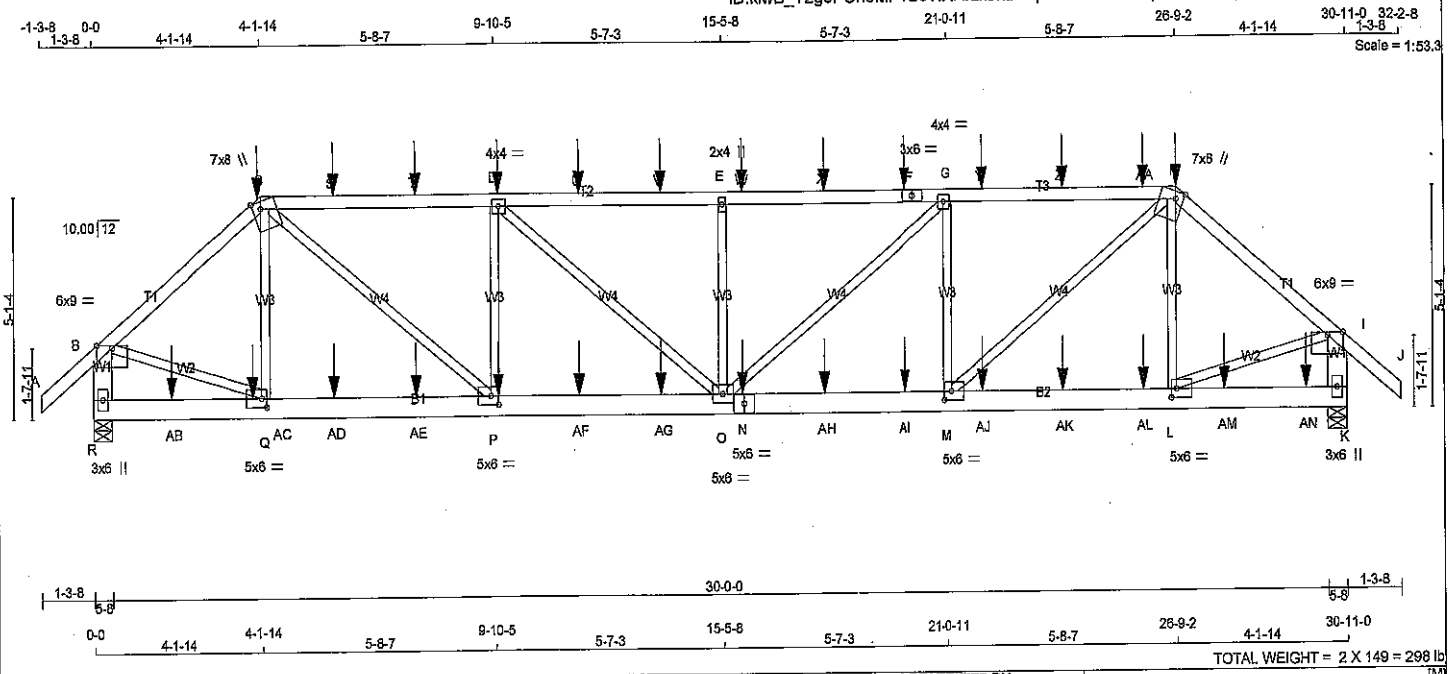


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	6	SIDE(15.5)
L- H	1	6	SIDE(15.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMWW-t	MT20	5.0	6.0	2.50	1.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	5511	0	5511	0	0	5-8	5-8
K	5651	0	5651	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	4361	2704 / 0	855 / 0	0 / 0	0 / 0	822 / 0	0 / 0
K	4497	2769 / 0	883 / 0	0 / 0	0 / 0	845 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-5780 / 0	-104.9	-104.9	0.35 (1)	3.80
C- S	-7810 / 0	-104.9	-104.9	0.72 (1)	3.00
S- T	-7810 / 0	-104.9	-104.9	0.72 (1)	3.00
T- D	-7810 / 0	-104.9	-104.9	0.72 (1)	3.00
D- U	-8841 / 0	-104.9	-104.9	0.81 (1)	2.73
U- V	-8841 / 0	-104.9	-104.9	0.81 (1)	2.73
V- E	-8841 / 0	-104.9	-104.9	0.81 (1)	2.73
E- W	-8841 / 0	-104.9	-104.9	0.83 (1)	2.71
W- X	-8841 / 0	-104.9	-104.9	0.83 (1)	2.71
X- F	-8841 / 0	-104.9	-104.9	0.83 (1)	2.71
F- G	-8841 / 0	-104.9	-104.9	0.83 (1)	2.71
G- Y	-7812 / 0	-104.9	-104.9	0.73 (1)	2.98
Y- Z	-7812 / 0	-104.9	-104.9	0.73 (1)	2.98
Z-AA	-7812 / 0	-104.9	-104.9	0.73 (1)	2.98
AA- H	-7812 / 0	-104.9	-104.9	0.73 (1)	2.98
H- I	-5775 / 0	-104.9	-104.9	0.35 (1)	3.80
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
R- B	-5347 / 0	0.0	0.0	0.20 (1)	6.33
K- I	-5346 / 0	0.0	0.0	0.20 (1)	6.33
R-AB	0 / 0	-28.0	-28.0	0.16 (1)	10.00
AB-AC	0 / 0	-28.0	-28.0	0.16 (1)	10.00
AC- Q	0 / 0	-28.0	-28.0	0.16 (1)	10.00
Q-AD	0 / 4425	-28.0	-28.0	0.45 (1)	10.00
AD-AE	0 / 4425	-28.0	-28.0	0.45 (1)	10.00
AE- P	0 / 4425	-28.0	-28.0	0.45 (1)	10.00
P-AF	0 / 7810	-28.0	-28.0	0.65 (1)	10.00
AF-AG	0 / 7810	-28.0	-28.0	0.65 (1)	10.00
AG- O	0 / 7810	-28.0	-28.0	0.65 (1)	10.00
O- N	0 / 7813	-28.0	-28.0	0.68 (1)	10.00
N-AH	0 / 7813	-28.0	-28.0	0.68 (1)	10.00
AH-AI	0 / 7813	-28.0	-28.0	0.68 (1)	10.00
AI- M	0 / 7813	-28.0	-28.0	0.68 (1)	10.00
M-AJ	0 / 4424	-28.0	-28.0	0.47 (1)	10.00
AJ-AK	0 / 4424	-28.0	-28.0	0.47 (1)	10.00
AK-AL	0 / 4424	-28.0	-28.0	0.47 (1)	10.00
AL- L	0 / 4424	-28.0	-28.0	0.47 (1)	10.00
L-AM	0 / 0	-28.0	-28.0	0.18 (1)	10.00
AM-AN	0 / 0	-28.0	-28.0	0.18 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-195 / 156	0.04 (1)
C- P	0 / 4390	0.54 (1)
P- D	-1689 / 0	0.31 (1)
D- O	0 / 1350	0.17 (1)
O- E	-875 / 0	0.13 (1)
O- G	0 / 1347	0.17 (1)
M- G	-1692 / 0	0.31 (1)
M- H	0 / 4394	0.54 (1)
L- H	-174 / 164	0.03 (1)
B- Q	0 / 4607	0.57 (1)
L- I	0 / 4606	0.57 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.83/1.00 (E-G:1) , BC=0.68/1.00 (M-O:1) , WB=0.57/1.00 (B-Q:1) , SSI=0.21/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

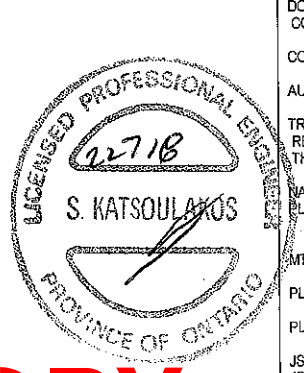
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.62 (N) (INPUT = 1.00)



DWG NO. TAM/10528-M

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 4-1-14, 526.9 lbs FACTORED DOWN AT 26-9-2, 56.9 lbs FACTORED DOWN AT 4-1-14, 53.9 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, 53.9 lbs FACTORED DOWN AT 9-11-4, 53.9 lbs FACTORED DOWN AT 11-11-4, 53.9 lbs FACTORED DOWN AT 13-11-4, 53.9 lbs FACTORED DOWN AT 15-11-4, 53.9 lbs FACTORED DOWN AT 17-11-4, 53.9 lbs FACTORED DOWN AT 19-11-4, 53.9 lbs FACTORED DOWN AT 21-11-4, AND 53.9 lbs FACTORED DOWN AT 23-11-4, AND 63.2 lbs FACTORED DOWN AT 25-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, 336.6 lbs FACTORED DOWN AT 11-11-4, 336.6 lbs FACTORED DOWN AT 13-11-4, 336.6 lbs FACTORED DOWN AT 15-11-4, 336.6 lbs FACTORED DOWN AT 17-11-4, 336.6 lbs FACTORED DOWN AT 19-11-4, 336.6 lbs FACTORED DOWN AT 21-11-4, 336.6 lbs FACTORED DOWN AT 23-11-4, 336.6 lbs FACTORED DOWN AT 25-11-4, AND 336.6 lbs FACTORED DOWN AT 27-11-4, AND 336.6 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN- K	0 / 0		-28.0	-28.0	0.18 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-14	-527	-527	—	FRONT	VERT	TOTAL
C	4-1-14	-57	-57	—	BACK	VERT	TOTAL
D	9-11-4	-54	-54	—	BACK	VERT	TOTAL
F	19-11-4	-54	-54	—	BACK	VERT	TOTAL
H	26-9-2	-527	-527	—	FRONT	VERT	TOTAL
N	15-11-4	-337	-337	—	BACK	VERT	TOTAL
P	9-11-4	-337	-337	—	BACK	VERT	TOTAL
S	5-11-4	-54	-54	—	BACK	VERT	TOTAL
T	7-11-4	-54	-54	—	BACK	VERT	TOTAL
U	11-11-4	-54	-54	—	BACK	VERT	TOTAL
V	13-11-4	-54	-54	—	BACK	VERT	TOTAL
W	15-11-4	-54	-54	—	BACK	VERT	TOTAL
X	17-11-4	-54	-54	—	BACK	VERT	TOTAL
Y	21-11-4	-54	-54	—	BACK	VERT	TOTAL
Z	23-11-4	-54	-54	—	BACK	VERT	TOTAL
AA	25-11-4	-63	-63	—	BACK	VERT	TOTAL
AB	1-11-4	-337	-337	—	BACK	VERT	TOTAL
AC	3-11-4	-337	-337	—	BACK	VERT	TOTAL
AD	5-11-4	-337	-337	—	BACK	VERT	TOTAL
AE	7-11-4	-337	-337	—	BACK	VERT	TOTAL
AF	11-11-4	-337	-337	—	BACK	VERT	TOTAL
AG	13-11-4	-337	-337	—	BACK	VERT	TOTAL
AH	17-11-4	-337	-337	—	BACK	VERT	TOTAL
AI	19-11-4	-337	-337	—	BACK	VERT	TOTAL
AJ	21-11-4	-337	-337	—	BACK	VERT	TOTAL
AK	23-11-4	-337	-337	—	BACK	VERT	TOTAL
AL	25-11-4	-337	-337	—	BACK	VERT	TOTAL
AM	27-11-4	-337	-337	—	BACK	VERT	TOTAL
AN	29-11-4	-337	-337	—	BACK	VERT	TOTAL

DWG NO. TAM/0528-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287443	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:51:02 2018 Page 1
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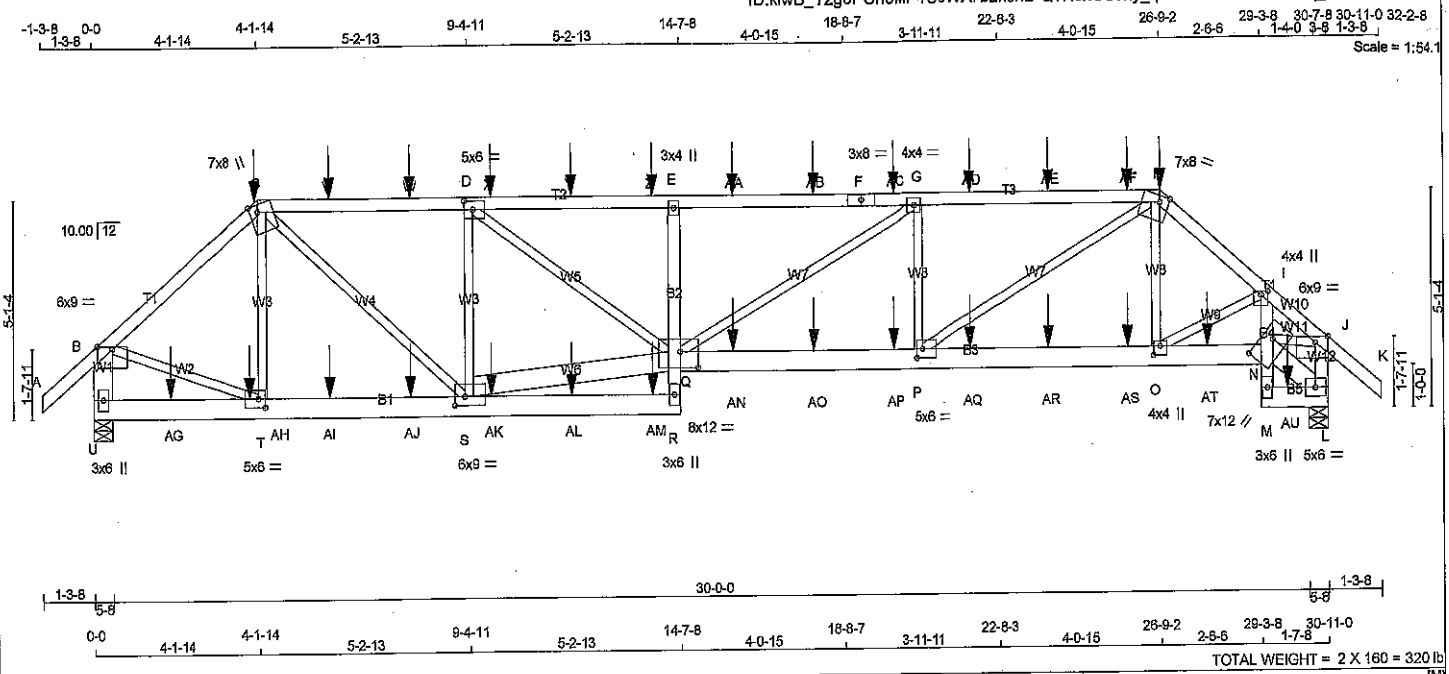

TOTAL WEIGHT = 2 X 160 = 320 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- K	1	12	SIDE(61.0)
L- J	1	12	TOP
U- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
Q- N	2	12	SIDE(0.0)
M- L	2	12	SIDE(0.0)
R- E	1	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T- C	1	6	SIDE(15.5)
O- H	1	6	SIDE(15.5)
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	7.0	8.0	1.75	2.75
I	TMVW+p	MT20	4.0	4.0	1.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	5489	0	5489	0	0	5-8	5-8
L	5627	0	5627	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	4369	2688 / 0	858 / 0	0 / 0	0 / 0	822 / 0	0 / 0
L	4487	2747 / 0	891 / 0	0 / 0	0 / 0	846 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.07 (1)	10.00	
B- C	-5730 / 0	-104.9	-104.9	0.23 (1)	4.45	
C- V	-7534 / 0	-104.9	-104.9	0.37 (1)	3.85	
V- W	-7534 / 0	-104.9	-104.9	0.37 (1)	3.85	
W- D	-7534 / 0	-104.9	-104.9	0.37 (1)	3.85	
D- X	-10923 / 0	-104.9	-104.9	0.51 (1)	3.16	
X- Y	-10923 / 0	-104.9	-104.9	0.51 (1)	3.16	
Y- Z	-10923 / 0	-104.9	-104.9	0.51 (1)	3.16	
Z- E	-10923 / 0	-104.9	-104.9	0.51 (1)	3.16	
E-AA	-10989 / 0	-104.9	-104.9	0.69 (1)	3.07	
AA-AB	-10989 / 0	-104.9	-104.9	0.69 (1)	3.07	
AB- F	-10989 / 0	-104.9	-104.9	0.69 (1)	3.07	
F-AC	-10989 / 0	-104.9	-104.9	0.69 (1)	3.07	
AC- G	-10989 / 0	-104.9	-104.9	0.69 (1)	3.07	
G-AD	-10068 / 0	-104.9	-104.9	0.64 (1)	3.23	
AD-AE	-10068 / 0	-104.9	-104.9	0.64 (1)	3.23	
AE-AF	-10068 / 0	-104.9	-104.9	0.64 (1)	3.23	
AF- H	-10068 / 0	-104.9	-104.9	0.64 (1)	3.23	
H- I	-7225 / 0	-104.9	-104.9	0.17 (1)	4.08	
I- J	-7611 / 0	-104.9	-104.9	0.14 (1)	4.02	
J- K	0 / 47	-104.9	-104.9	0.07 (1)	10.00	
U- B	-5304 / 0	0.0	0.0	0.20 (1)	6.35	
L- J	-5330 / 0	0.0	0.0	0.30 (1)	5.20	
U-AG	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
AG-AH	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
AH- T	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
T-AI	0 / 4384	-28.0	-28.0	0.46 (1)	10.00	
AI-AJ	0 / 4384	-28.0	-28.0	0.46 (1)	10.00	
AJ- S	0 / 4384	-28.0	-28.0	0.46 (1)	10.00	
S-AK	0 / 246	-28.0	-28.0	0.20 (1)	10.00	
AK-AL	0 / 246	-28.0	-28.0	0.20 (1)	10.00	
AL-AM	0 / 246	-28.0	-28.0	0.20 (1)	10.00	
AM- R	0 / 246	-28.0	-28.0	0.20 (1)	10.00	
R- Q	0 / 501	0.0	0.0	0.23 (1)	10.00	
Q- E	-638 / 0	0.0	0.0	0.19 (1)	7.81	
Q-AN	0 / 10069	-28.0	-28.0	0.90 (1)	10.00	
AN-AO	0 / 10069	-28.0	-28.0	0.90 (1)	10.00	
AO-AP	0 / 10069	-28.0	-28.0	0.90 (1)	10.00	
AP- P	0 / 10069	-28.0	-28.0	0.90 (1)	10.00	
P-AQ	0 / 5571	-28.0	-28.0	0.57 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-287 / 91	0.05 (1)
C- S	0 / 4265	0.53 (1)
S- D	-3188 / 0	0.59 (1)
S- Q	0 / 7477	0.49 (1)
D- Q	0 / 4136	0.51 (1)
Q- G	0 / 1078	0.13 (1)
P- G	-1365 / 0	0.17 (1)
P- H	0 / 5246	0.65 (1)
O- H	0 / 922	0.11 (1)
O- I	-478 / 0	0.05 (1)
B- T	0 / 4567	0.57 (1)
N- L	-257 / 0	0.01 (1)
N- J	0 / 5784	0.72 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 703 (0.53")

CSI: TC=0.69/1.00 (E-G:1) , BC=0.90/1.00 (P-Q:1) , WB=0.72/1.00 (J-N:1) , SSI=0.26/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
22718
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM10550-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287443	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TMVW-p	MT20	6.0	9.0	Edge	3.50
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-w	MT20	7.0	12.0	3.00	7.50
O	BMWW+t	MT20	4.0	4.0	2.50	2.00
P	BMWW-t	MT20	5.0	6.0	2.50	1.75
Q	BVMWWW-I	MT20	8.0	12.0	4.50	5.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0	2.50	3.00
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 56.9 lbs FACTORED DOWN AT 4-1-14, 53.9 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, 53.9 lbs FACTORED DOWN AT 9-11-4, 53.9 lbs FACTORED DOWN AT 11-11-4, 53.9 lbs FACTORED DOWN AT 13-11-4, 27.3 lbs FACTORED DOWN AT 15-11-4, 27.3 lbs FACTORED DOWN AT 17-11-4, 27.3 lbs FACTORED DOWN AT 19-11-4, 27.3 lbs FACTORED DOWN AT 21-11-4, 27.3 lbs FACTORED DOWN AT 23-11-4, 36.6 lbs FACTORED DOWN AT 25-11-4, AND 526.9 lbs FACTORED DOWN AT 4-1-14, AND 526.9 lbs FACTORED DOWN AT 26-9-2 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, 336.6 lbs FACTORED DOWN AT 11-11-4, 343.0 lbs FACTORED DOWN AT 13-11-4, 350.9 lbs FACTORED DOWN AT 15-11-4, 350.9 lbs FACTORED DOWN AT 17-11-4, 350.9 lbs FACTORED DOWN AT 19-11-4, 350.9 lbs FACTORED DOWN AT 21-11-4, 350.9 lbs FACTORED DOWN AT 23-11-4, 350.9 lbs FACTORED DOWN AT 25-11-4, AND 350.9 lbs FACTORED DOWN AT 27-11-4, AND 343.8 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	FACTORED FORCE (LBS)	MAX CSI (LC)
AQ-AR	0 / 5571	-28.0	-28.0	0.57 (1)	10.00		
AR-AS	0 / 5571	-28.0	-28.0	0.57 (1)	10.00		
AS- O	0 / 5571	-28.0	-28.0	0.57 (1)	10.00		
O-AT	0 / 5967	-28.0	-28.0	0.59 (1)	10.00		
AT- N	0 / 5967	-28.0	-28.0	0.59 (1)	10.00		
M- N	0 / 248	0.0	0.0	0.20 (1)	10.00		
N- I	0 / 322	0.0	0.0	0.21 (1)	10.00		
M-AU	0 / 201	-28.0	-28.0	0.06 (1)	10.00		
AU- L	0 / 201	-28.0	-28.0	0.06 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-14	-57	-57	—	BACK	VERT	TOTAL
C	4-1-14	-527	-527	—	FRONT	VERT	TOTAL
H	26-9-2	-527	-527	—	FRONT	VERT	TOTAL
V	5-11-4	-54	-54	—	BACK	VERT	TOTAL
W	7-11-4	-54	-54	—	BACK	VERT	TOTAL
X	9-11-4	-54	-54	—	BACK	VERT	TOTAL
Y	11-11-4	-54	-54	—	BACK	VERT	TOTAL
Z	13-11-4	-54	-54	—	BACK	VERT	TOTAL
AA	15-11-4	-27	-27	—	BACK	VERT	TOTAL
AB	17-11-4	-27	-27	—	BACK	VERT	TOTAL
AC	19-11-4	-27	-27	—	BACK	VERT	TOTAL
AD	21-11-4	-27	-27	—	BACK	VERT	TOTAL
AE	23-11-4	-27	-27	—	BACK	VERT	TOTAL
AF	25-11-4	-37	-37	—	BACK	VERT	TOTAL
AG	1-11-4	-337	-337	—	BACK	VERT	TOTAL
AH	3-11-4	-337	-337	—	BACK	VERT	TOTAL
AI	5-11-4	-337	-337	—	BACK	VERT	TOTAL
AJ	7-11-4	-337	-337	—	BACK	VERT	TOTAL
AK	9-11-4	-337	-337	—	BACK	VERT	TOTAL
AL	11-11-4	-337	-337	—	BACK	VERT	TOTAL
AM	13-11-4	-343	-343	—	BACK	VERT	TOTAL
AN	15-11-4	-351	-351	—	BACK	VERT	TOTAL
AO	17-11-4	-351	-351	—	BACK	VERT	TOTAL
AP	19-11-4	-351	-351	—	BACK	VERT	TOTAL
AQ	21-11-4	-351	-351	—	BACK	VERT	TOTAL
AR	23-11-4	-351	-351	—	BACK	VERT	TOTAL
AS	25-11-4	-351	-351	—	BACK	VERT	TOTAL
AT	27-11-4	-351	-351	—	BACK	VERT	TOTAL
AU	29-11-4	-344	-344	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.66 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2271B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0550-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:10 2018 Page 1

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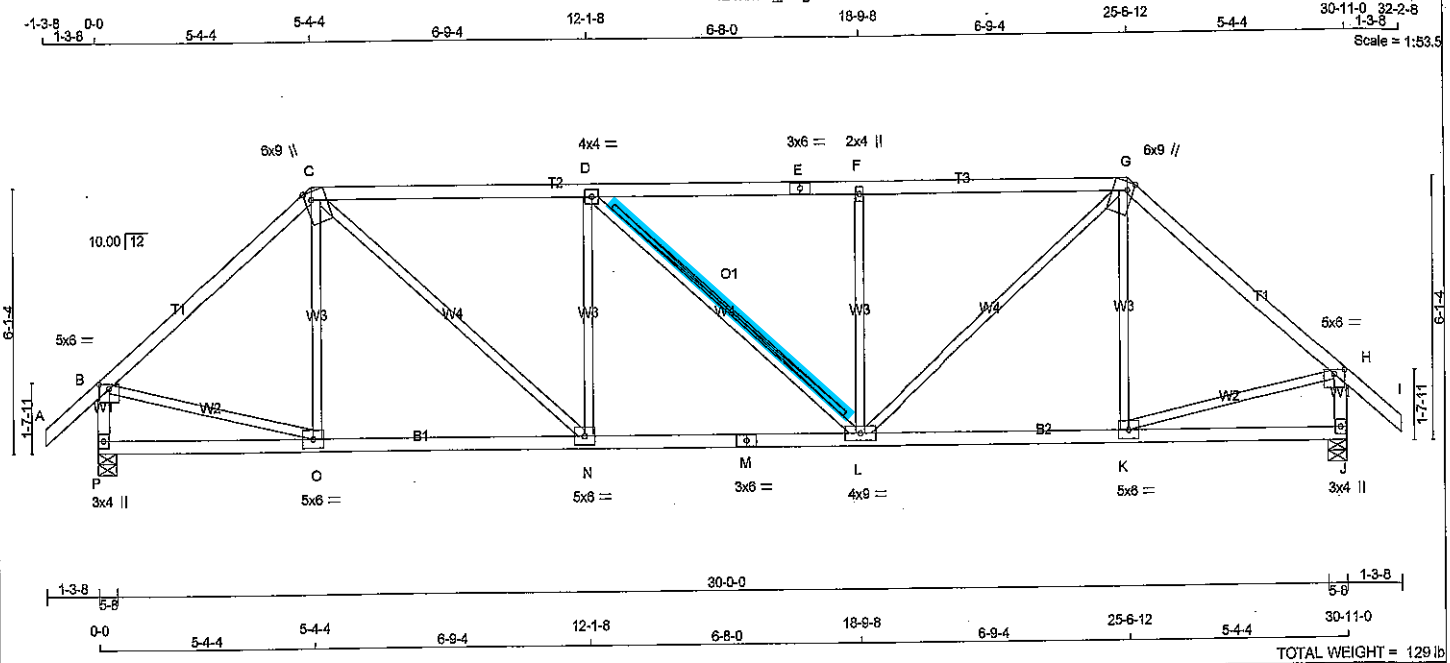

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	4.0		
K, N, O						
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2200	0	2200	0	0	5-8	5-8
J	2200	0	2200	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
J	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2079 / 0	-104.9	-104.9	0.73 (1)	3.91	
C- D	-2701 / 0	-104.9	-104.9	0.98 (1)	3.11	
D- E	-2699 / 0	-104.9	-104.9	0.97 (1)	3.11	
E- F	-2699 / 0	-104.9	-104.9	0.97 (1)	3.11	
F- G	-2699 / 0	-104.9	-104.9	0.97 (1)	3.14	
G- H	-2080 / 0	-104.9	-104.9	0.73 (1)	3.91	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
P- B	-2142 / 0	0.0	0.0	0.23 (1)	5.80	
J- H	-2143 / 0	0.0	0.0	0.23 (1)	5.80	
P- O	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
O- N	0 / 1591	-28.0	-28.0	0.42 (2)	10.00	
N- M	0 / 2701	-28.0	-28.0	0.56 (1)	10.00	
M- L	0 / 2701	-28.0	-28.0	0.56 (1)	10.00	
L- K	0 / 1591	-28.0	-28.0	0.42 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
O- C	-191 / 163	0.11 (1)
C- N	0 / 1461	0.33 (1)
N- D	-764 / 0	0.45 (1)
D- L	-2 / 0	0.00 (1)
L- F	-763 / 0	0.45 (1)
L- G	0 / 1459	0.33 (1)
K- G	-190 / 163	0.11 (1)
B- O	0 / 1639	0.37 (1)
K- H	0 / 1639	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.98/1.00 (C-D:1) , BC=0.56/1.00 (L-N:1) , WB=0.45/1.00 (D-N:1) , SSI=0.33/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.81 (M) (INPUT = 1.00)

[M]

LICENSED PROFESSIONAL ENGINEERS
2218
S. KATSOULAKOS
PROFESSIONAL ENGINEER

DWG NO. TAM/0529 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287443	T2S	1	1	44755 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:51:02 2018 Page 1

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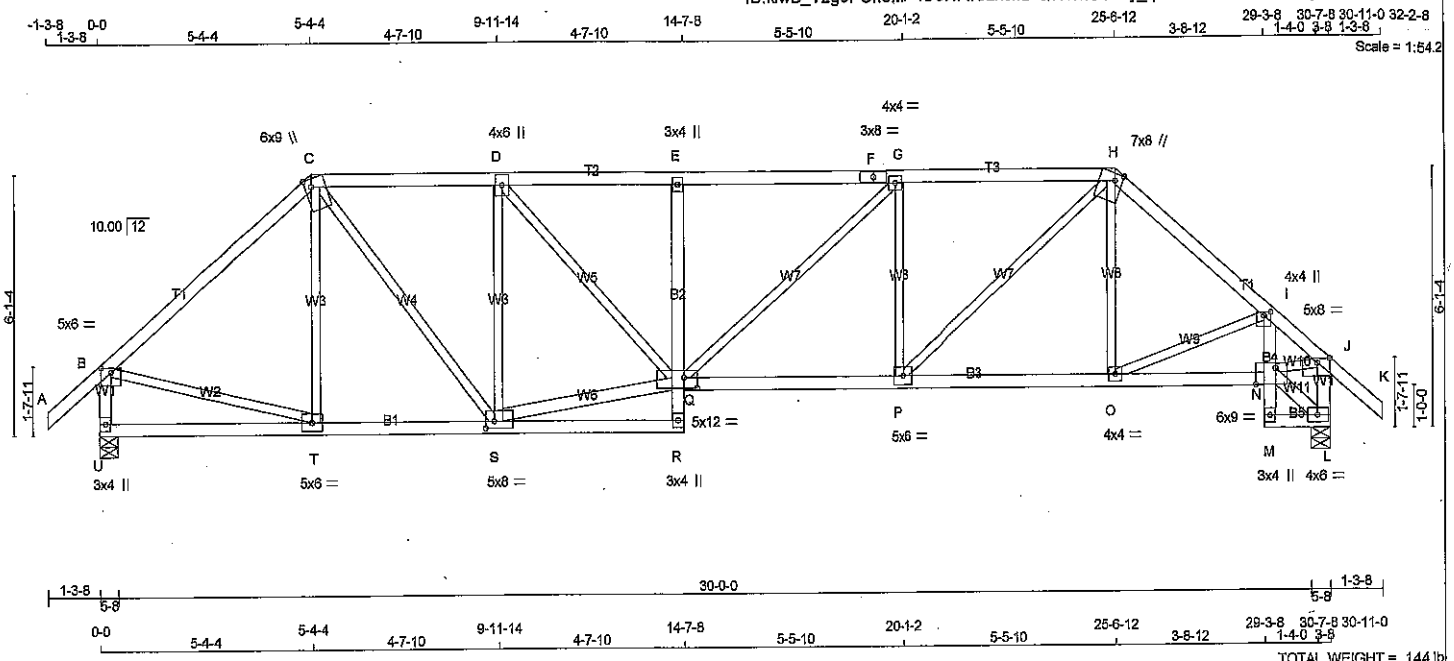

TOTAL WEIGHT = 144 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW+t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-t	MT20	6.0	9.0	4.50	5.75
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BVMWWW-t	MT20	5.0	12.0	3.00	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	5.0	8.0	2.00	2.50
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2197	0	2197	0	0	5-8	5-8
L	2203	0	2203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1734	1092 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
L	1738	1096 / 0	325 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-2069 / 0	-104.9	-104.9	0.73 (1)	3.92
C- D	-2418 / 0	-104.9	-104.9	0.41 (1)	4.02
D- E	-3297 / 0	-104.9	-104.9	0.48 (1)	3.44
E- F	-3313 / 0	-104.9	-104.9	0.75 (1)	3.13
F- G	-3313 / 0	-104.9	-104.9	0.75 (1)	3.13
G- H	-3088 / 0	-104.9	-104.9	0.72 (1)	3.27
H- I	-2487 / 0	-104.9	-104.9	0.33 (1)	4.09
I- J	-2683 / 0	-104.9	-104.9	0.21 (1)	4.04
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
U- B	-2134 / 0	0.0	0.0	0.23 (1)	5.81
L- J	-2128 / 0	0.0	0.0	0.23 (1)	5.81
U- T	0 / 0	-28.0	-28.0	0.18 (3)	10.00
T- S	0 / 1582	-28.0	-28.0	0.35 (1)	10.00
S- R	0 / 75	-28.0	-28.0	0.13 (2)	10.00
R- Q	0 / 92	0.0	0.0	0.11 (1)	10.00
Q- E	-484 / 0	0.0	0.0	0.17 (1)	7.81
Q- P	0 / 3089	-28.0	-28.0	0.60 (1)	10.00
P- O	0 / 1896	-28.0	-28.0	0.41 (2)	10.00
O- N	0 / 2118	-28.0	-28.0	0.42 (1)	10.00
M- N	0 / 36	0.0	0.0	0.11 (1)	10.00
N- I	-85 / 47	0.0	0.0	0.11 (1)	7.81
M- L	0 / 69	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-221 / 112	0.13 (1)
C- S	0 / 1350	0.30 (1)
S- D	-1448 / 0	0.65 (1)
S- Q	0 / 2416	0.39 (1)
D- Q	0 / 1264	0.28 (1)
Q- G	0 / 298	0.07 (1)
P- G	-856 / 0	0.33 (1)
P- H	0 / 1572	0.35 (1)
O- H	0 / 236	0.05 (2)
O- I	-239 / 0	0.07 (1)
B- T	0 / 1631	0.37 (1)
N- L	-88 / 0	0.01 (1)
N- J	0 / 2068	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.75/1.00 (E-G:1) , BC=0.60/1.00 (P-Q:1) , WB=0.85/1.00 (D-S:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM1055I-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:11 2018 Page 1
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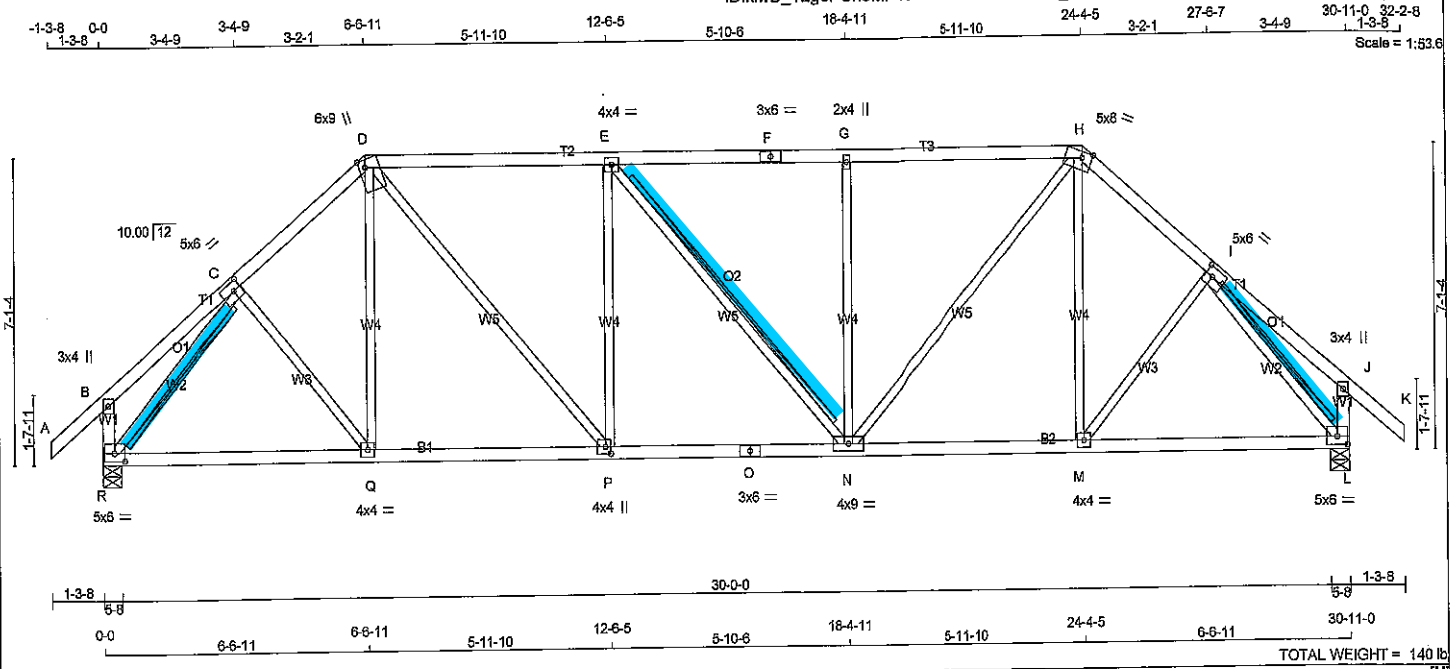

TOTAL WEIGHT = 140 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	4.0	2.00	1.50
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2200	0	2200	0	0	5-8	5-8
L	2200	0	2200	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
L	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N
2x3 DRY SPF No.2 T-BRACE AT C-R, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00
C- D	-2086 / 0	-104.9	-104.9	0.22 (1)	4.50
D- E	-2310 / 0	-104.9	-104.9	0.67 (1)	3.76
E- F	-2308 / 0	-104.9	-104.9	0.67 (1)	3.76
F- G	-2308 / 0	-104.9	-104.9	0.67 (1)	3.76
G- H	-2308 / 0	-104.9	-104.9	0.66 (1)	3.77
H- I	-2086 / 0	-104.9	-104.9	0.22 (1)	4.50
I- J	0 / 23	-104.9	-104.9	0.16 (1)	10.00
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
R- B	-281 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-281 / 0	0.0	0.0	0.03 (1)	7.81
R- Q	0 / 1493	-28.0	-28.0	0.44 (2)	10.00
Q- P	0 / 1582	-28.0	-28.0	0.46 (2)	10.00
P- O	0 / 2310	-28.0	-28.0	0.46 (1)	10.00
O- N	0 / 2310	-28.0	-28.0	0.46 (1)	10.00
N- M	0 / 1582	-28.0	-28.0	0.46 (2)	10.00
M- L	0 / 1493	-28.0	-28.0	0.44 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 174	0.04 (2)
Q- D	0 / 231	0.05 (3)
D- P	0 / 1103	0.25 (1)
P- E	-672 / 0	0.59 (1)
E- N	-2 / 0	0.00 (1)
N- G	-671 / 0	0.59 (1)
N- H	0 / 1100	0.25 (1)
M- H	0 / 231	0.05 (3)
M- I	0 / 174	0.04 (2)
R- C	-2372 / 0	0.85 (1)
I- L	-2373 / 0	0.85 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03')
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.46/1.00 (N-P:1) , WB=0.85/1.00 (I-L:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.69 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
22216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 105301B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:11 2018 Page 1

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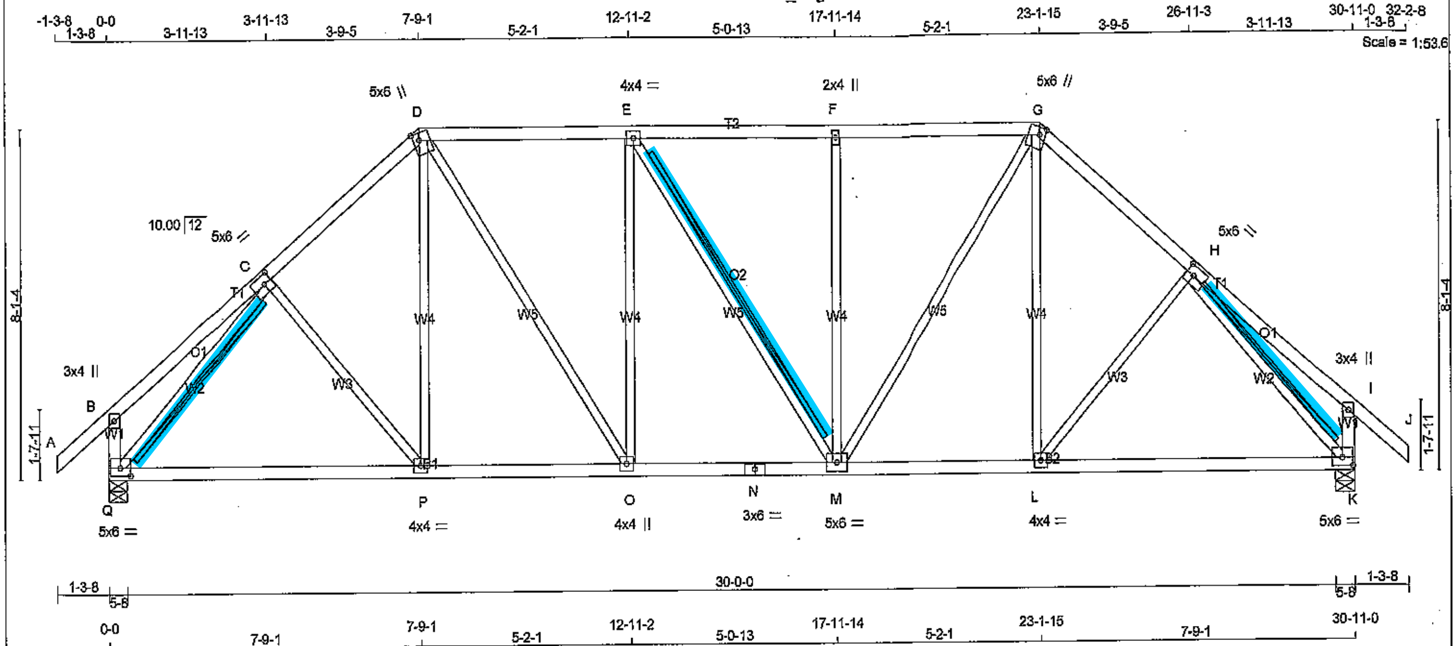

TOTAL WEIGHT = 148 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.75
H	TMWW-t	MT20	5.0	6.0	2.50	2.25
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.25	3.00
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.25	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2200	0	2200	0	0	5-8	5-8
K	2200	0	2200	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
K	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-M, C-Q, H-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 29	-104.9	-104.9	0.24 (1)	10.00
C- D	-2052 / 0	-104.9	-104.9	0.32 (1)	4.43
D- E	-2017 / 0	-104.9	-104.9	0.47 (1)	4.25
E- F	-2016 / 0	-104.9	-104.9	0.46 (1)	4.25
F- G	-2015 / 0	-104.9	-104.9	0.46 (1)	4.27
G- H	-2052 / 0	-104.9	-104.9	0.32 (1)	4.42
H- I	0 / 29	-104.9	-104.9	0.24 (1)	10.00
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-302 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-302 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 1541	-28.0	-28.0	0.53 (2)	10.00
P- O	0 / 1553	-28.0	-28.0	0.54 (2)	10.00
O- N	0 / 2017	-28.0	-28.0	0.40 (1)	10.00
N- M	0 / 2017	-28.0	-28.0	0.40 (1)	10.00
M- L	0 / 1553	-28.0	-28.0	0.54 (2)	10.00
L- K	0 / 1542	-28.0	-28.0	0.53 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 129	0.03 (3)
P- D	0 / 309	0.07 (2)
D- O	0 / 840	0.19 (1)
O- E	-579 / 0	0.73 (1)
E- M	-3 / 0	0.00 (1)
M- F	-578 / 0	0.73 (1)
M- G	0 / 836	0.19 (1)
L- G	0 / 310	0.07 (2)
L- H	0 / 129	0.03 (3)
Q- C	-2375 / 0	0.64 (1)
H- K	-2375 / 0	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.54/1.00 (L-M:2) , WB=0.73/1.00 (E-O:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.62 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2/27/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10531 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:11 2018 Page 1

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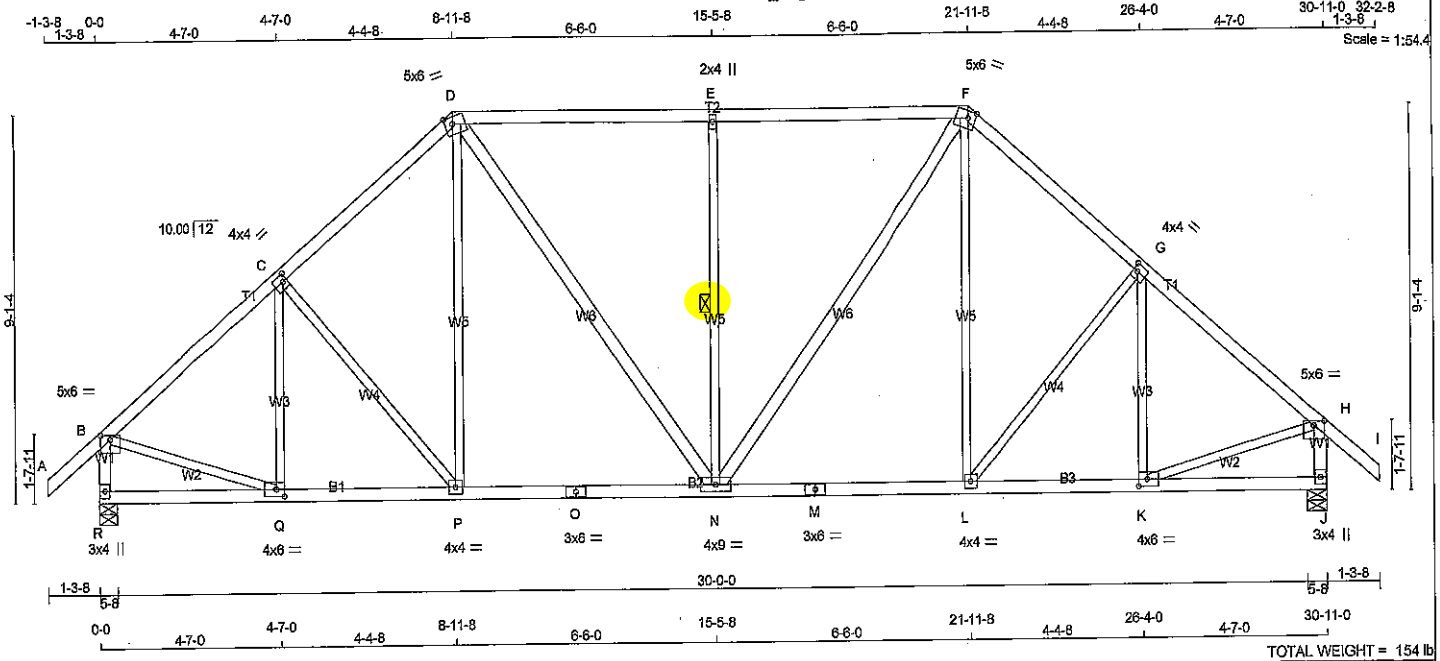


TOTAL WEIGHT = 154 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2200	0	2200	0	0	5-8	5-8
J	2200	0	2200	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
J	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00		Q- C	-334 / 38	0.15 (1)
B- C	-2076 / 0	-104.9	-104.9	0.33 (1)	4.41		C- P	-195 / 0	0.17 (1)
C- D	-1984 / 0	-104.9	-104.9	0.32 (1)	4.50		P- D	0 / 351	0.08 (2)
D- E	-1875 / 0	-104.9	-104.9	0.64 (1)	4.10		D- N	0 / 637	0.10 (1)
E- F	-1875 / 0	-104.9	-104.9	0.64 (1)	4.10		N- E	-838 / 0	0.45 (1)
F- G	-1984 / 0	-104.9	-104.9	0.32 (1)	4.50		N- F	0 / 637	0.10 (1)
G- H	-2076 / 0	-104.9	-104.9	0.33 (1)	4.41		L- F	0 / 351	0.08 (2)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00		L- G	-195 / 0	0.17 (1)
R- B	-2144 / 0	0.0	0.0	0.23 (1)	5.80		K- G	-334 / 38	0.15 (1)
J- H	-2144 / 0	0.0	0.0	0.23 (1)	5.80		B- Q	0 / 1686	0.38 (1)
							K- H	0 / 1686	0.38 (1)
R- Q	0 / 0	-28.0	-28.0	0.12 (2)	10.00				
Q- P	0 / 1621	-28.0	-28.0	0.36 (1)	10.00				
P- O	0 / 1496	-28.0	-28.0	0.42 (2)	10.00				
O- N	0 / 1496	-28.0	-28.0	0.42 (2)	10.00				
N- M	0 / 1496	-28.0	-28.0	0.42 (2)	10.00				
M- L	0 / 1496	-28.0	-28.0	0.42 (2)	10.00				
L- K	0 / 1621	-28.0	-28.0	0.36 (1)	10.00				
K- J	0 / 0	-28.0	-28.0	0.12 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.42/1.00 (N-P:2) , WB=0.45/1.00 (E-N:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

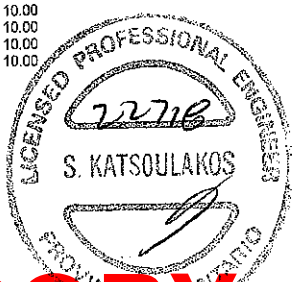
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.50 (O) (INPUT = 1.00)



DWG NO. TAM 10532-B

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:12 2018 Page 1
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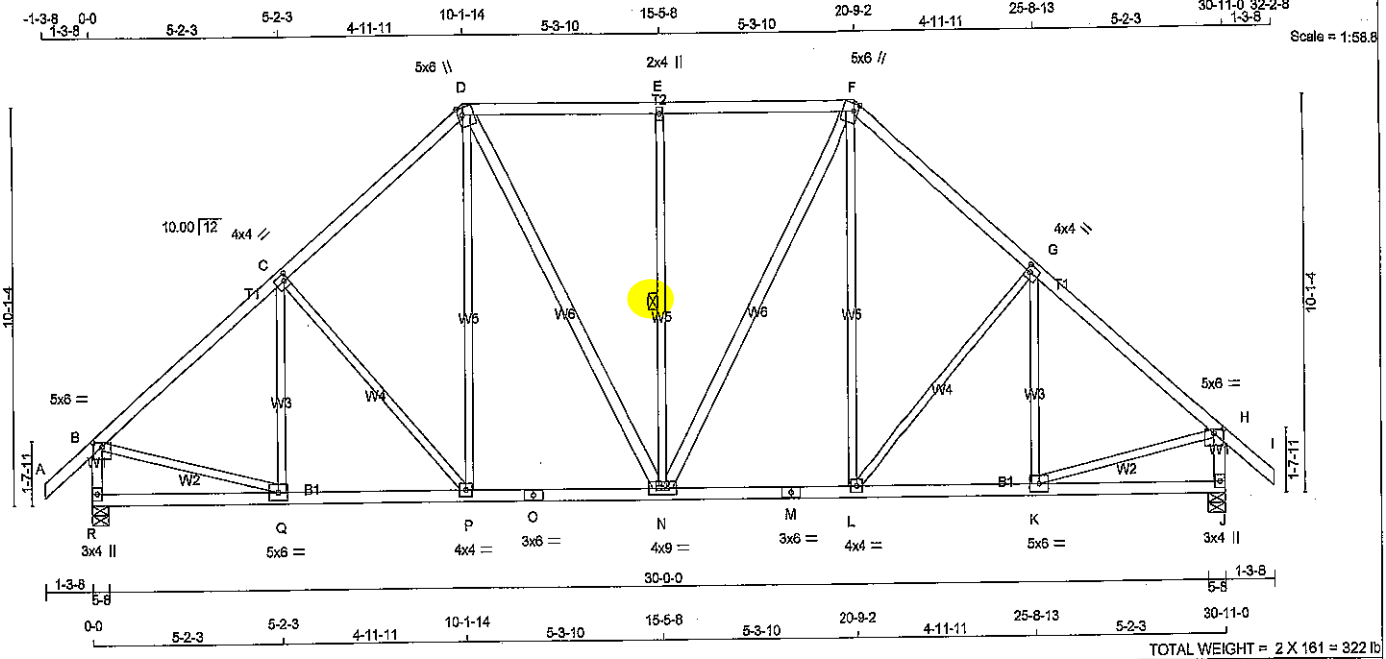


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2200	0	2200	0	0	5-8	5-8
J	2200	0	2200	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
J	1736	1094 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

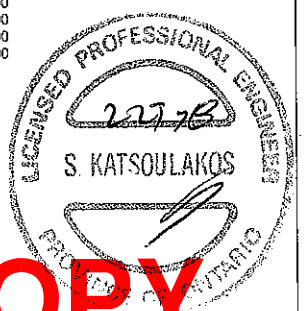
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	Q- C	-257 / 107	0.14 (1)
B- C	-2101 / 0	-104.9	-104.9	0.44 (1)	4.27	C- P	-315 / 0	0.36 (1)
C- D	-1912 / 0	-104.9	-104.9	0.41 (1)	4.46	P- D	0 / 390	0.09 (2)
D- E	-1661 / 0	-104.9	-104.9	0.41 (1)	4.69	D- N	0 / 469	0.08 (1)
E- F	-1661 / 0	-104.9	-104.9	0.41 (1)	4.69	N- E	-676 / 0	0.47 (1)
F- G	-1912 / 0	-104.9	-104.9	0.41 (1)	4.46	N- F	0 / 469	0.08 (1)
G- H	-2101 / 0	-104.9	-104.9	0.44 (1)	4.27	L- F	0 / 390	0.09 (2)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- G	-315 / 0	0.36 (1)
R- B	-2140 / 0	0.0	0.0	0.23 (1)	5.80	K- G	-257 / 107	0.14 (1)
J- H	-2140 / 0	0.0	0.0	0.23 (1)	5.80	B- Q	0 / 1696	0.38 (1)
						K- H	0 / 1696	0.38 (1)
R- Q	0 / 0	-28.0	-28.0	0.18 (3)	10.00			
Q- P	0 / 1645	-28.0	-28.0	0.35 (1)	10.00			
P- O	0 / 1438	-28.0	-28.0	0.32 (1)	10.00			
O- N	0 / 1438	-28.0	-28.0	0.32 (1)	10.00			
N- M	0 / 1438	-28.0	-28.0	0.32 (1)	10.00			
M- L	0 / 1438	-28.0	-28.0	0.32 (1)	10.00			
L- K	0 / 1645	-28.0	-28.0	0.35 (1)	10.00			
K- J	0 / 0	-28.0	-28.0	0.18 (3)	10.00			



[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.35/1.00 (P-Q:1) , WB=0.47/1.00 (E-N:1) ; SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM / 0533 -R
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T7	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:13 2018 Page 1

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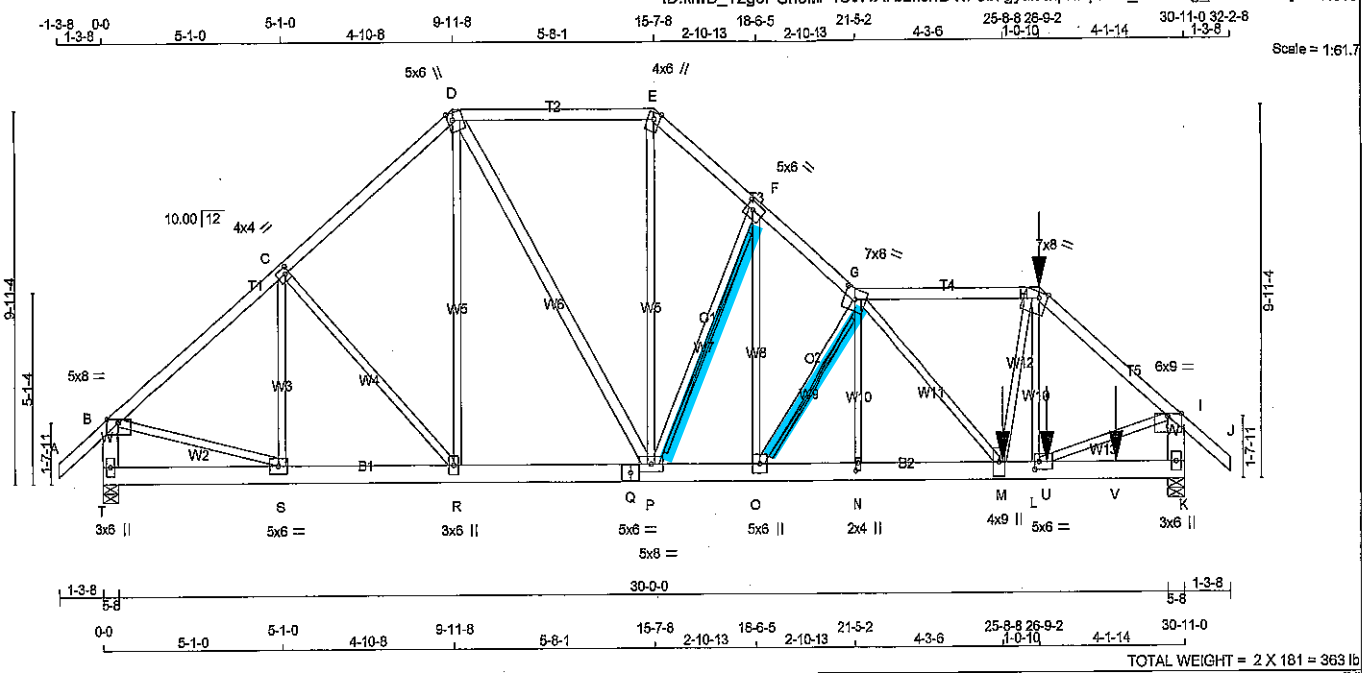

TOTAL WEIGHT = 2 X 181 = 363 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- G	1	12	TOP
G- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	TOP
Q- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(15.0)
M- H	1	3	SIDE(697.6)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table]s in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW+m	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	5.0	6.0	2.50	2.25
G	TTWWW-m	MT20	7.0	8.0	3.25	4.00
H	TTWW-m	MT20	7.0	8.0	Edge	2.75
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW+t	MT20	4.0	9.0		
N	BMW+w	MT20	2.0	4.0	2.50	1.00
O	BMWW+t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2916	0	2916	0	0	5-8	5-8
K	6118	0	6118	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2296	1454 / 0	425 / 0	0 / 0	0 / 0	418 / 0	0 / 0
K	4814	3055 / 0	885 / 0	0 / 0	0 / 0	874 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-P, G-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-2986 / 0	-104.9	-104.9	0.31 (1)	5.03
C- D	-2911 / 0	-104.9	-104.9	0.31 (1)	5.08
D- E	-2850 / 0	-104.9	-104.9	0.38 (1)	4.99
E- F	-3716 / 0	-104.9	-104.9	0.12 (1)	4.80
F- G	-5143 / 0	-104.9	-104.9	0.16 (1)	4.16
G- H	-5668 / 0	-104.9	-104.9	0.47 (1)	3.68
H- I	-6478 / 0	-104.9	-104.9	0.39 (1)	3.58
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
T- B	-2846 / 0	0.0	0.0	0.10 (1)	7.81
K- I	-5949 / 0	0.0	0.0	0.22 (1)	6.06
T- S	0 / 0	-28.0	-28.0	0.04 (2)	10.00
S- R	0 / 2324	-28.0	-28.0	0.18 (1)	10.00
R- Q	0 / 2203	-28.0	-28.0	0.23 (1)	10.00
Q- P	0 / 2203	-28.0	-28.0	0.23 (1)	10.00
P- O	0 / 3981	-28.0	-28.0	0.38 (1)	10.00
O- N	0 / 6121	-28.0	-28.0	0.52 (1)	10.00
N- M	0 / 6128	-28.0	-28.0	0.54 (1)	10.00
M- L	0 / 4965	-28.0	-28.0	0.48 (1)	10.00
L- U	0 / 0	-28.0	-28.0	0.16 (1)	10.00
U- V	0 / 0	-28.0	-28.0	0.16 (1)	10.00
V- K	0 / 0	-28.0	-28.0	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-418 / 37	0.11 (1)
C- R	-183 / 0	0.10 (1)
R- D	0 / 312	0.04 (2)
D- P	0 / 1285	0.11 (1)
P- E	0 / 1959	0.24 (1)
P- F	-2956 / 0	0.87 (1)
O- F	0 / 3268	0.40 (1)
O- G	-4081 / 0	0.61 (1)
N- G	0 / 303	0.04 (2)
L- H	0 / 174	0.02 (3)
B- S	0 / 2390	0.30 (1)
L- I	0 / 5184	0.64 (1)
M- H	0 / 3113	0.39 (1)
G- M	-887 / 0	0.24 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	26-9-2	-584	-584	---	FRONT	VERT	TOTAL
M	25-8-8	-3376	-3376	---	FRONT	VERT	TOTAL
U	26-11-12	-337	-337	---	FRONT	VERT	TOTAL
V	28-11-12	-337	-337	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.47/1.00 (G-H:1) , BC=0.54/1.00 (M-N:1) , WB=0.87/1.00 (F-P:1) , SSI=0.28/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T7	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 583.8 lbs FACTORED DOWN AT 26-9-2 ON TOP CHORD, AND 3375.9 lbs FACTORED DOWN AT 25-8-8, AND 336.6 lbs FACTORED DOWN AT 26-11-12, AND 336.6 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 20534-18 Rev 2
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T8C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:13 2018 Page 1

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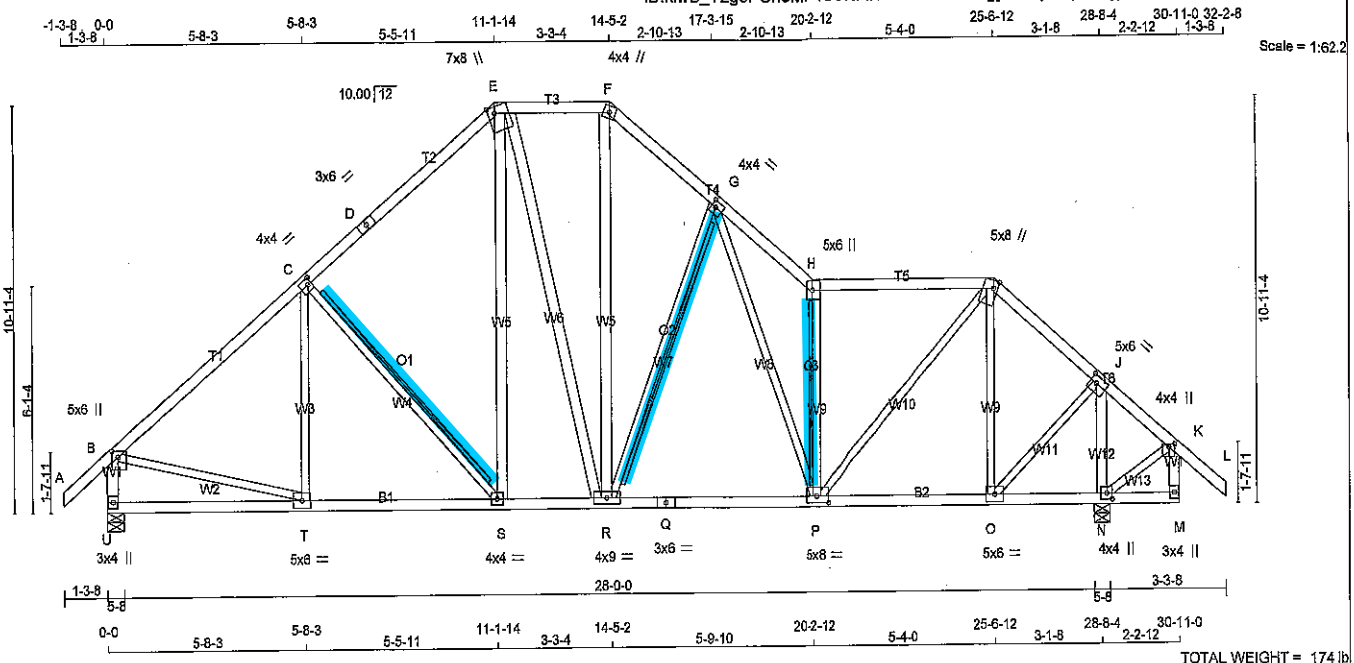

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
E - R	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW+m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TTW+p	MT20	5.0	6.0		
I	TTWW+m	MT20	5.0	8.0	Edge	1.25
J	TMWW-t	MT20	5.0	6.0	2.25	2.25
K	TMW+p	MT20	4.0	4.0	1.00	2.00
M	BMV+p	MT20	3.0	4.0		
N	BMWW1+t	MT20	4.0	4.0	2.00	1.75
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0	2.25	4.00
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	4.0	9.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2029	0	2029	0	0	5-8	5-8
N	2371	0	2371	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1601	1009 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
N	1871	1179 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-S, H-P
2x4 DRY SPF No.2 T-BRACE AT G-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00	T- C	-161 / 179	0.11 (1)
B- C	-1905 / 0	-104.9	-104.9	1	0.52 (1)	4.33	C- S	-441 / 0	0.49 (1)
C- D	-1608 / 0	-104.9	-104.9	1	0.48 (1)	4.68	S- E	0 / 419	0.07 (1)
D- E	-1608 / 0	-104.9	-104.9	1	0.48 (1)	4.68	E- R	0 / 207	0.03 (1)
E- F	-1262 / 0	-104.9	-104.9	1	0.16 (1)	5.57	R- F	0 / 766	0.12 (1)
F- G	-1861 / 0	-104.9	-104.9	1	0.16 (1)	5.01	R- G	-874 / 0	0.52 (1)
G- H	-2447 / 0	-104.9	-104.9	1	0.19 (1)	4.25	G- P	0 / 1014	0.23 (1)
H- I	-1873 / 0	-104.9	-104.9	1	0.43 (1)	4.44	P- H	-1982 / 0	0.93 (1)
I- J	-1124 / 0	-104.9	-104.9	1	0.14 (1)	5.84	P- I	0 / 1560	0.35 (1)
J- K	0 / 235	-104.9	-104.9	1	0.20 (1)	10.00	O- I	-857 / 0	0.50 (1)
K- L	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00	O- J	0 / 1396	0.31 (1)
U- B	-1965 / 0	0.0	0.0	1	0.21 (1)	6.01	N- J	-2184 / 0	0.32 (1)
M- K	0 / 36	0.0	0.0	1	0.01 (3)	10.00	B- T	0 / 1537	0.35 (1)
							N- K	-180 / 0	0.03 (1)
U- T	0 / 0	-28.0	-28.0	1	0.23 (3)	10.00			
T- S	0 / 1498	-28.0	-28.0	1	0.39 (2)	10.00			
S- R	0 / 1203	-28.0	-28.0	1	0.30 (1)	10.00			
R- Q	0 / 1562	-28.0	-28.0	1	0.36 (1)	10.00			
Q- P	0 / 1562	-28.0	-28.0	1	0.36 (1)	10.00			
P- O	0 / 827	-28.0	-28.0	1	0.26 (2)	10.00			
O- N	-155 / 0	-28.0	-28.0	1	0.08 (2)	6.25			
N- M	0 / 0	-28.0	-28.0	1	0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.39/1.00 (S-T:2) , WB=0.93/1.00 (H-P:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.46 (Q) (INPUT = 1.00)

DWG NO. TAM 10535-18
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER 22216 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T9C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:13 2018 Page 1

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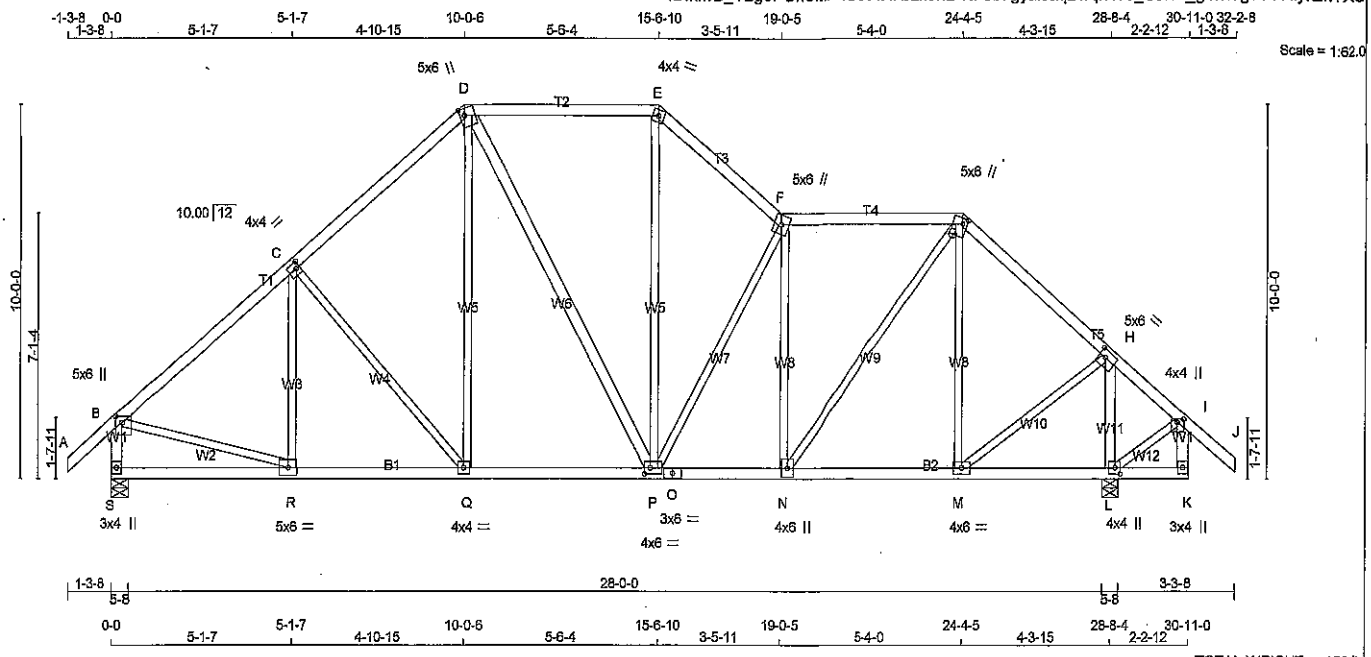

TOTAL WEIGHT = 159 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMWW-t	MT20	5.0	6.0	2.25	2.25
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	BMV+p	MT20	3.0	4.0		
L	BMWW1+t	MT20	4.0	4.0	2.00	1.75
M	BMWW-t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0	2.00	2.00
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2029	0	2029	0	0	5-8	5-8
L	2371	0	2371	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1601	1009 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
L	1871	1179 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B-C	-1895 / 0	-104.9	-104.9	0.41 (1)	4.48
C-D	-1690 / 0	-104.9	-104.9	0.39 (1)	4.71
D-E	-1369 / 0	-104.9	-104.9	0.44 (1)	5.01
E-F	-1773 / 0	-104.9	-104.9	0.20 (1)	4.84
F-G	-1725 / 0	-104.9	-104.9	0.43 (1)	4.60
G-H	-1294 / 0	-104.9	-104.9	0.24 (1)	5.41
H-I	0 / 222	-104.9	-104.9	0.30 (1)	10.00
I-J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
S-B	-1969 / 0	0.0	0.0	0.21 (1)	6.00
K-I	0 / 26	0.0	0.0	0.00 (3)	10.00
S-R	0 / 0	-28.0	-28.0	0.17 (3)	10.00
R-Q	0 / 1486	-28.0	-28.0	0.32 (2)	10.00
Q-P	0 / 1268	-28.0	-28.0	0.29 (1)	10.00
P-O	0 / 1739	-28.0	-28.0	0.35 (1)	10.00
O-N	0 / 1739	-28.0	-28.0	0.35 (1)	10.00
N-M	0 / 960	-28.0	-28.0	0.25 (2)	10.00
M-L	-128 / 0	-28.0	-28.0	0.11 (3)	6.25
L-K	0 / 0	-28.0	-28.0	0.10 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-225 / 114	0.12 (1)
C-Q	-333 / 0	0.37 (1)
Q-D	0 / 409	0.09 (2)
D-P	0 / 208	0.03 (1)
P-E	0 / 663	0.15 (1)
P-F	-808 / 0	0.96 (1)
N-F	-882 / 0	0.77 (1)
N-G	0 / 1254	0.28 (1)
M-G	-642 / 0	0.56 (1)
M-H	0 / 1349	0.30 (1)
L-H	-2179 / 0	0.32 (1)
B-R	0 / 1534	0.35 (1)
L-I	-148 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.35/1.00 (N-P:1) , WB=0.96/1.00 (F-P:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.48 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 105361B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T10C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Tue Feb 27 08:46:14 2018 Page 1

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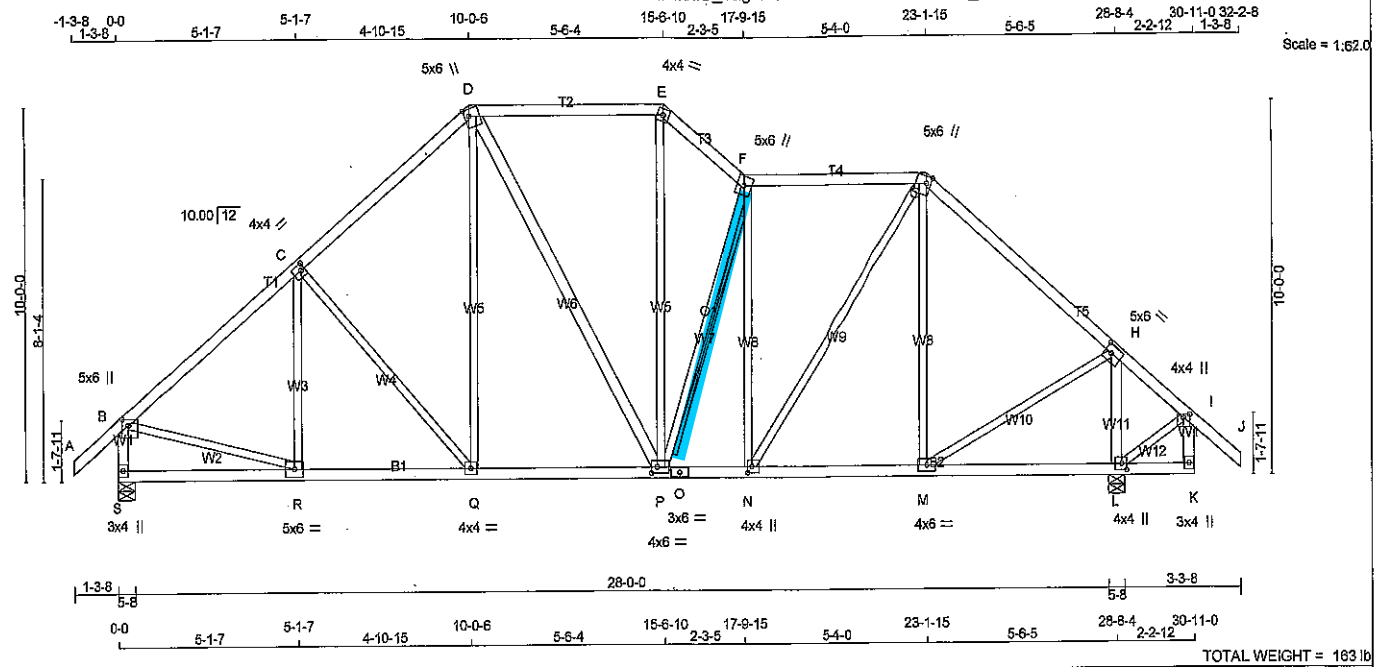

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.25
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	BMV+p	MT20	3.0	4.0		
L	BMWW1+t	MT20	4.0	4.0	2.00	1.75
M	BMWW-t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	4.0	2.00	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0	2.00	2.00
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2029	0	2029	0	0	5-8	5-8
L	2371	0	2371	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1601	1009 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
L	1871	1179 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1895 / 0	-104.9	-104.9	0.41 (1)	4.48	
C- D	-1690 / 0	-104.9	-104.9	0.39 (1)	4.71	
D- E	-1369 / 0	-104.9	-104.9	0.44 (1)	5.02	
E- F	-1771 / 0	-104.9	-104.9	0.10 (1)	4.94	
F- G	-1592 / 0	-104.9	-104.9	0.42 (1)	4.75	
G- H	-1394 / 0	-104.9	-104.9	0.42 (1)	5.04	
H- I	0 / 198	-104.9	-104.9	0.45 (1)	10.00	
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
S- B	-1969 / 0	0.0	0.0	0.21 (1)	6.00	
K- I	-4 / 7	0.0	0.0	0.00 (3)	7.81	
S- R	0 / 0	-28.0	-28.0	0.17 (3)	10.00	
R- Q	0 / 1486	-28.0	-28.0	0.32 (1)	10.00	
Q- P	0 / 1268	-28.0	-28.0	0.29 (1)	10.00	
P- O	0 / 1602	-28.0	-28.0	0.32 (1)	10.00	
O- N	0 / 1602	-28.0	-28.0	0.32 (1)	10.00	
N- M	0 / 1035	-28.0	-28.0	0.30 (2)	10.00	
M- L	-84 / 0	-28.0	-28.0	0.14 (3)	6.25	
L- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-226 / 114	0.12 (1)
C- Q	-333 / 0	0.37 (1)
Q- D	0 / 410	0.09 (2)
D- P	0 / 207	0.03 (1)
P- E	0 / 725	0.16 (1)
P- F	-825 / 0	0.90 (1)
N- F	-745 / 0	0.94 (1)
N- G	0 / 999	0.22 (1)
M- G	-471 / 17	0.60 (1)
M- H	0 / 1291	0.29 (1)
L- H	-2176 / 0	0.32 (1)
B- R	0 / 1534	0.35 (1)
L- I	-97 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.45/1.00 (H-I:1) , BC=0.32/1.00 (Q-R:1) , WB=0.94/1.00 (F-N:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/10537-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T11C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:14 2018 Page 1
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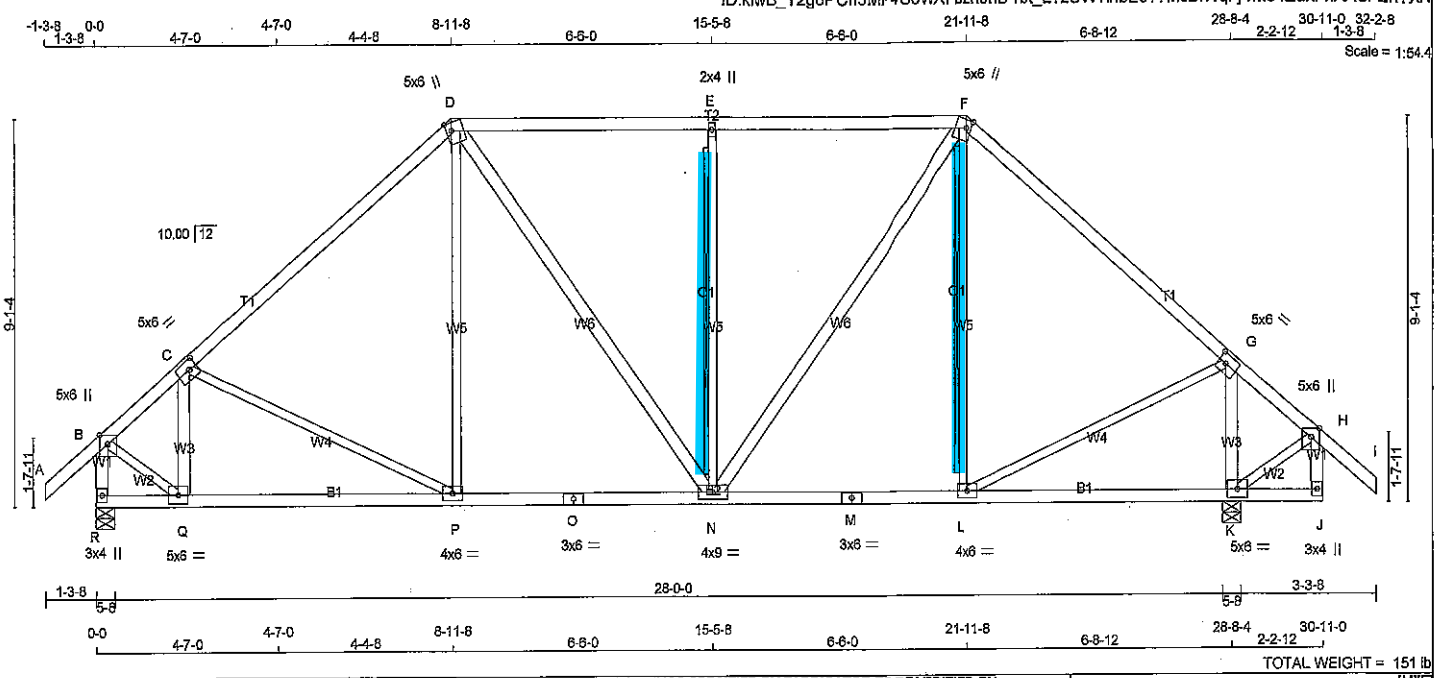

TOTAL WEIGHT = 151 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2029	0	2029	0	0	5-8	5-8
K	2371	0	2371	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1601	1009 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
K	1871	1179 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N
2x3 DRY SPF No.2 T-BRACE AT F-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1676 / 0	-104.9	-104.9	0.50 (1)	4.50	
C- D	-1769 / 0	-104.9	-104.9	0.69 (1)	4.21	
D- E	-1573 / 0	-104.9	-104.9	0.62 (1)	4.43	
E- F	-1573 / 0	-104.9	-104.9	0.62 (1)	4.43	
F- G	-1448 / 0	-104.9	-104.9	0.65 (1)	4.60	
G- H	-2 / 167	-104.9	-104.9	0.66 (1)	10.00	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
R- B	-2028 / 0	0.0	0.0	0.22 (1)	5.94	
J- H	-19 / 0	0.0	0.0	0.00 (2)	7.81	
R- Q	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
Q- P	0 / 1386	-28.0	-28.0	0.41 (2)	10.00	
P- O	0 / 1327	-28.0	-28.0	0.41 (2)	10.00	
O- N	0 / 1327	-28.0	-28.0	0.41 (2)	10.00	
N- M	0 / 1073	-28.0	-28.0	0.37 (2)	10.00	
M- L	0 / 1073	-28.0	-28.0	0.37 (2)	10.00	
L- K	-25 / 9	-28.0	-28.0	0.26 (3)	6.25	
K- J	0 / 0	-28.0	-28.0	0.22 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-661 / 0	0.10 (1)
C- P	-68 / 6	0.08 (1)
P- D	0 / 345	0.06 (2)
D- N	0 / 414	0.07 (1)
N- E	-838 / 0	0.51 (1)
N- F	0 / 841	0.14 (1)
L- F	-322 / 115	0.44 (1)
L- G	0 / 1214	0.27 (1)
K- G	-2181 / 0	0.32 (1)
B- Q	0 / 1606	0.36 (1)
K- H	-29 / 11	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.69/1.00 (C-D:1) , BC=0.41/1.00 (P-Q:2) , WB=0.51/1.00 (E-N:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.48 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0538-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:15 2018 Page 1

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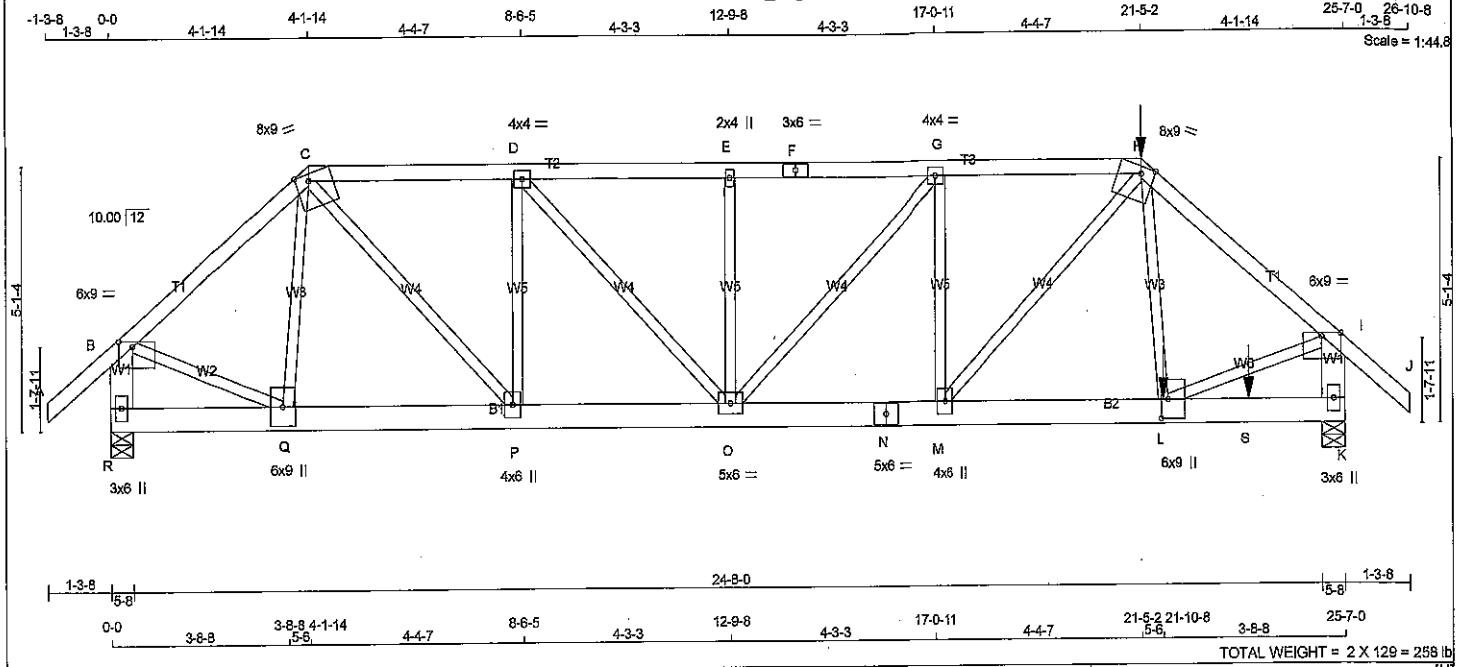


TOTAL WEIGHT = 2 X 129 = 258 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H- L	1	3	SIDE(762.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	6.0	9.0	1.25	3.50
C	TTWW-m	MT20	8.0	9.0	Edge	3.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	8.0	9.0	Edge	3.25
I	TMW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	6.0	9.0	4.50	1.75
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	6.0	9.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2480	0	2480	0	0	5-8	5-8
K	5701	0	5701	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1949	1242 / 0	355 / 0	0 / 0	0 / 0	352 / 0	0 / 0
K	4468	2867 / 0	802 / 0	0 / 0	0 / 0	799 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

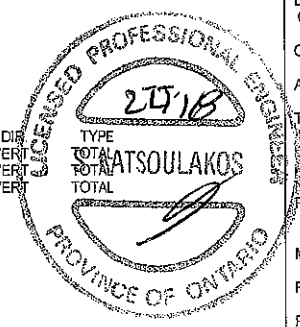
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	C- P	0 / 2065	0.26 (1)
B- C	-2317 / 0	-104.9	-104.9	0.20 (1)	5.66	P- D	-1384 / 0	0.26 (1)
C- D	-3207 / 0	-104.9	-104.9	0.19 (1)	5.00	D- O	0 / 1210	0.15 (1)
D- E	-4028 / 0	-104.9	-104.9	0.22 (1)	4.54	O- E	-430 / 0	0.08 (1)
E- F	-4028 / 0	-104.9	-104.9	0.22 (1)	4.54	O- G	-526 / 0	0.19 (1)
F- G	-4028 / 0	-104.9	-104.9	0.22 (1)	4.54	M- G	-103 / 103	0.02 (1)
G- H	-4385 / 0	-104.9	-104.9	0.23 (1)	4.38	M- H	0 / 392	0.05 (1)
H- I	-5693 / 0	-104.9	-104.9	0.35 (1)	3.83	L- I	0 / 4595	0.57 (1)
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	H- L	0 / 2433	0.30 (1)
R- B	-2440 / 0	0.0	0.0	0.09 (1)	7.81	Q- C	-458 / 0	0.08 (1)
K- I	-5465 / 0	0.0	0.0	0.20 (1)	6.27	B- Q	0 / 1870	0.23 (1)
R- Q	0 / 0	-28.0	-28.0	0.03 (3)	10.00			
Q- P	0 / 1820	-28.0	-28.0	0.13 (1)	10.00			
P- O	0 / 3207	-28.0	-28.0	0.24 (1)	10.00			
O- N	0 / 4385	-28.0	-28.0	0.32 (1)	10.00			
N- M	0 / 4385	-28.0	-28.0	0.32 (1)	10.00			
M- L	0 / 4122	-28.0	-28.0	0.31 (1)	10.00			
L- S	0 / 0	-28.0	-28.0	0.15 (1)	10.00			
S- K	0 / 0	-28.0	-28.0	0.15 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	21-5-2	-527	-527	---	FRONT	VERT	TOTAL
L	21-10-8	-3626	-3626	---	BACK	VERT	TOTAL
S	23-7-12	-337	-337	---	BACK	VERT	TOTAL



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.35/1.00 (H-I:1) , BC=0.32/1.00 (M-O:1) , WB=0.57/1.00 (I-L:1) , SSI=0.12/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.49 (H) (INPUT = 1.00)

P6 16

CONTINUED ON PAGE 2

DWG NO. TAM 10539 -18

STRUCTURAL COMPONENT ONLY

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 21-5-2 ON TOP CHORD, AND 3625.9 lbs FACTORED DOWN AT 21-10-8, AND 338.6 lbs FACTORED DOWN AT 23-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0539-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T12Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:30 2018 Page 1
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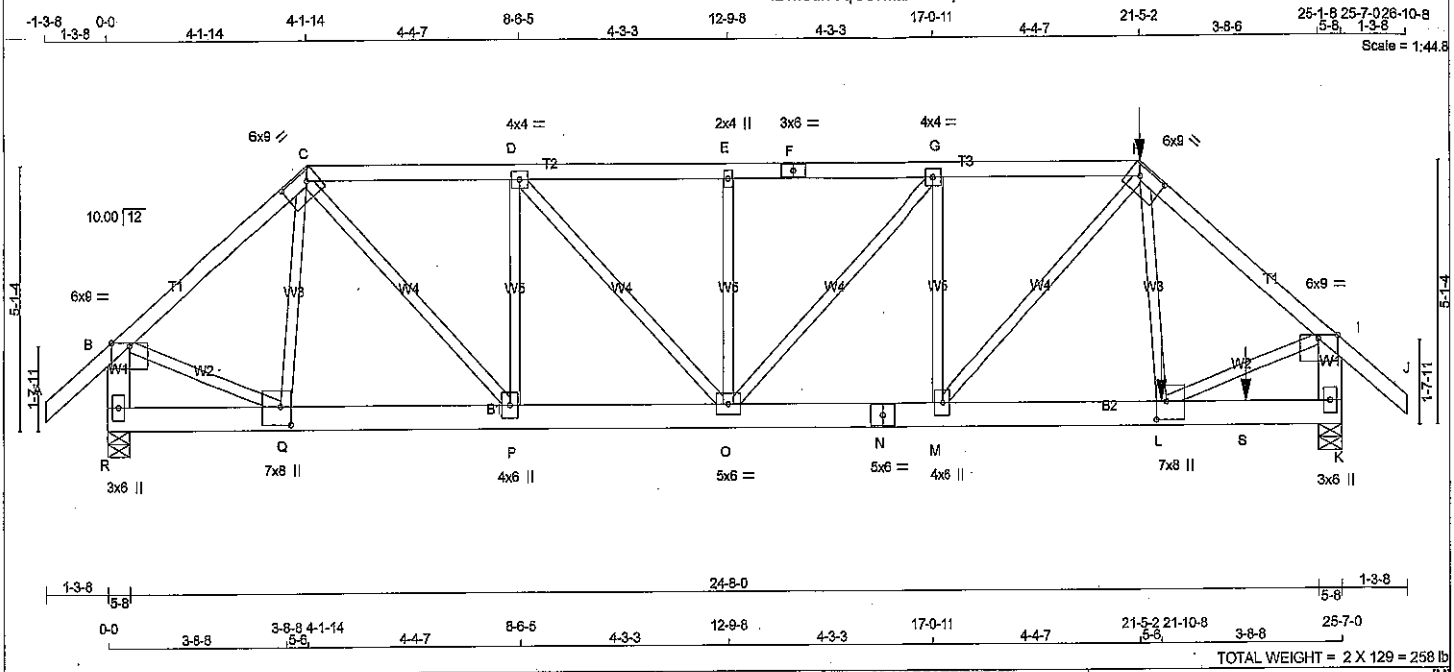

TOTAL WEIGHT = 2 X 129 = 258 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-N	2	12	TOP
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H-L	1	2	SIDE(1024.8)
2x3	1	6	
C-Q	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW-h	MT20	6.0	9.0	2.25	6.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-h	MT20	6.0	9.0	2.25	6.25
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	7.0	8.0	4.25	2.50
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	7.0	8.0	4.25	2.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2625	0	2625	0	0	5-8	5-8
K	6560	0	6560	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2064	1313 / 0	376 / 0	0 / 0	0 / 0	373 / 0	0 / 0
K	5152	3288 / 0	935 / 0	0 / 0	0 / 0	929 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	C- P	0 / 2259	0.28 (1)	
B- C	-2478 / 0	-104.9	-104.9	0.21 (1)	5.51	P- D	-1525 / 0	0.28 (1)	
C- D	-3465 / 0	-104.9	-104.9	0.20 (1)	4.85	D- O	0 / 1404	0.17 (1)	
D- E	-4417 / 0	-104.9	-104.9	0.23 (1)	4.36	O- E	-430 / 0	0.08 (1)	
E- F	-4417 / 0	-104.9	-104.9	0.23 (1)	4.36	O- G	-732 / 0	0.27 (1)	
F- G	-4417 / 0	-104.9	-104.9	0.23 (1)	4.36	M- G	0 / 167	0.02 (3)	
G- H	-4914 / 0	-104.9	-104.9	0.25 (1)	4.15	M- H	0 / 214	0.03 (1)	
H- I	-6634 / 0	-104.9	-104.9	0.40 (1)	3.53	H- L	0 / 3170	0.39 (1)	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	L- I	0 / 5354	0.66 (1)	
R- B	-2585 / 0	0.0	0.0	0.10 (1)	7.81	Q- C	-499 / 0	0.09 (1)	
K- I	-6309 / 0	0.0	0.0	0.23 (1)	5.91	B- Q	0 / 2000	0.25 (1)	
R- Q	0 / 0	-28.0	-28.0	0.03 (3)	10.00				
Q- P	0 / 1948	-28.0	-28.0	0.14 (1)	10.00				
P- O	0 / 3465	-28.0	-28.0	0.26 (1)	10.00				
O- N	0 / 4914	-28.0	-28.0	0.35 (1)	10.00				
N- M	0 / 4914	-28.0	-28.0	0.35 (1)	10.00				
M- L	0 / 4770	-28.0	-28.0	0.37 (1)	10.00				
L- S	0 / 0	-28.0	-28.0	0.16 (1)	10.00				
S- K	0 / 0	-28.0	-28.0	0.16 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	21-5-2	-527	-527	—	FRONT	VERT	TOTAL 2771
L	21-10-8	-4630	-4630	—	BACK	VERT	TOTAL
S	23-7-12	-337	-337	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.40/1.00 (H-I:1) , BC=0.37/1.00 (L-M:1) , WB=0.66/1.00 (I-L:1) , SSI=0.12/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T12Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 21-5-2 ON TOP CHORD, AND 4630.0 lbs FACTORED DOWN AT 21-10-8, AND 336.6 lbs FACTORED DOWN AT 23-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10553-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:15 2018 Page 1

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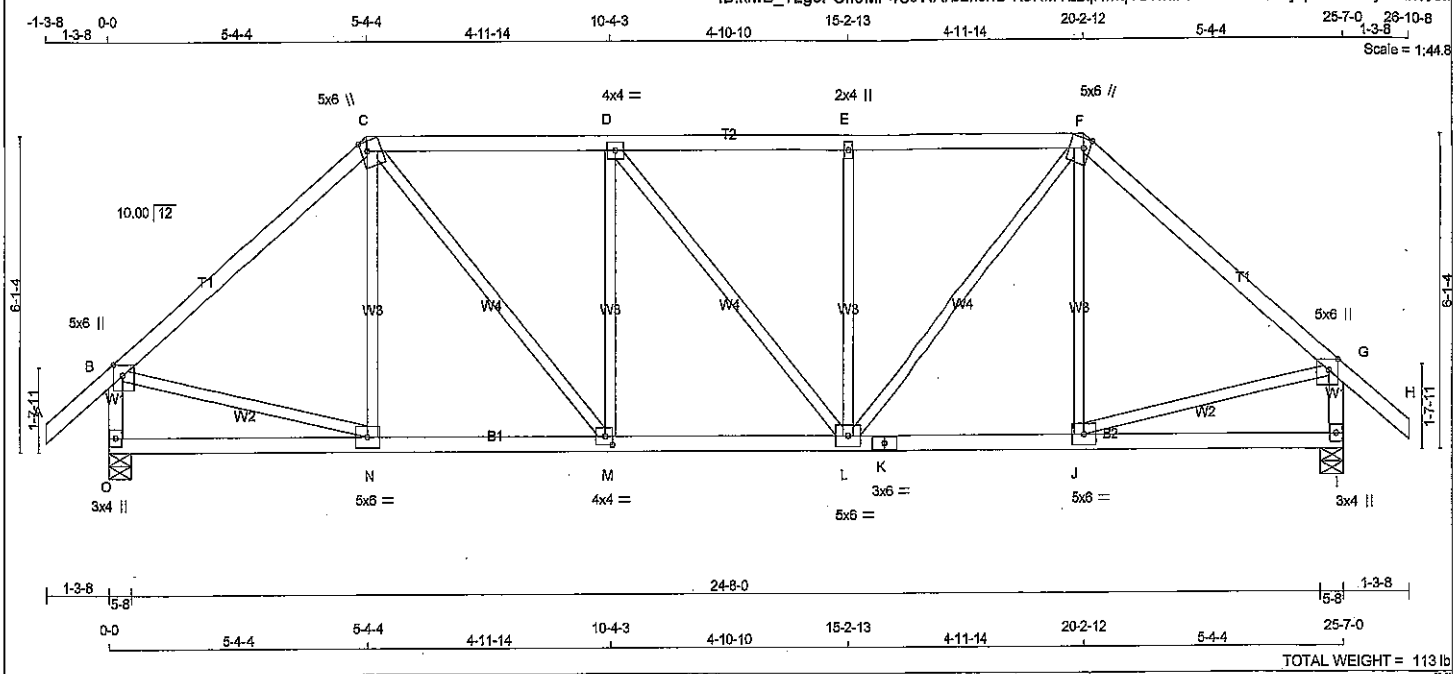

TOTAL WEIGHT = 113 lb

LUMBER

N. L., G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	2.50	2.25
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1846	0	1846	0	0	5-8	5-8
I	1846	0	1846	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
I	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	N- C	-135 / 159	0.08 (1)
B- C	-1645 / 0	-104.9	-104.9	0.66 (1)	4.36	C- M	0 / 810	0.20 (1)
C- D	-1853 / 0	-104.9	-104.9	0.42 (1)	4.46	M- D	-559 / 0	0.33 (1)
D- E	-1851 / 0	-104.9	-104.9	0.42 (1)	4.46	D- L	-3 / 0	0.00 (1)
E- F	-1851 / 0	-104.9	-104.9	0.41 (1)	4.47	L- E	-558 / 0	0.33 (1)
F- G	-1645 / 0	-104.9	-104.9	0.66 (1)	4.36	L- F	0 / 907	0.20 (1)
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- F	-133 / 159	0.08 (1)
O- B	-1784 / 0	0.0	0.0	0.19 (1)	6.26	B- N	0 / 1296	0.29 (1)
I- G	-1784 / 0	0.0	0.0	0.19 (1)	6.26	J- G	0 / 1297	0.29 (1)
O- N	0 / 0	-28.0	-28.0	0.19 (3)	10.00			
N- M	0 / 1259	-28.0	-28.0	0.33 (2)	10.00			
M- L	0 / 1853	-28.0	-28.0	0.36 (1)	10.00			
L- K	0 / 1259	-28.0	-28.0	0.33 (2)	10.00			
K- J	0 / 1259	-28.0	-28.0	0.33 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.66/1.00 (F-G:1) , BC=0.36/1.00 (L-M:1) , WB=0.33/1.00 (D-M:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.32 (G) (INPUT = 1.00)

DWG NO. TAM/0540 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:15 2018 Page 1

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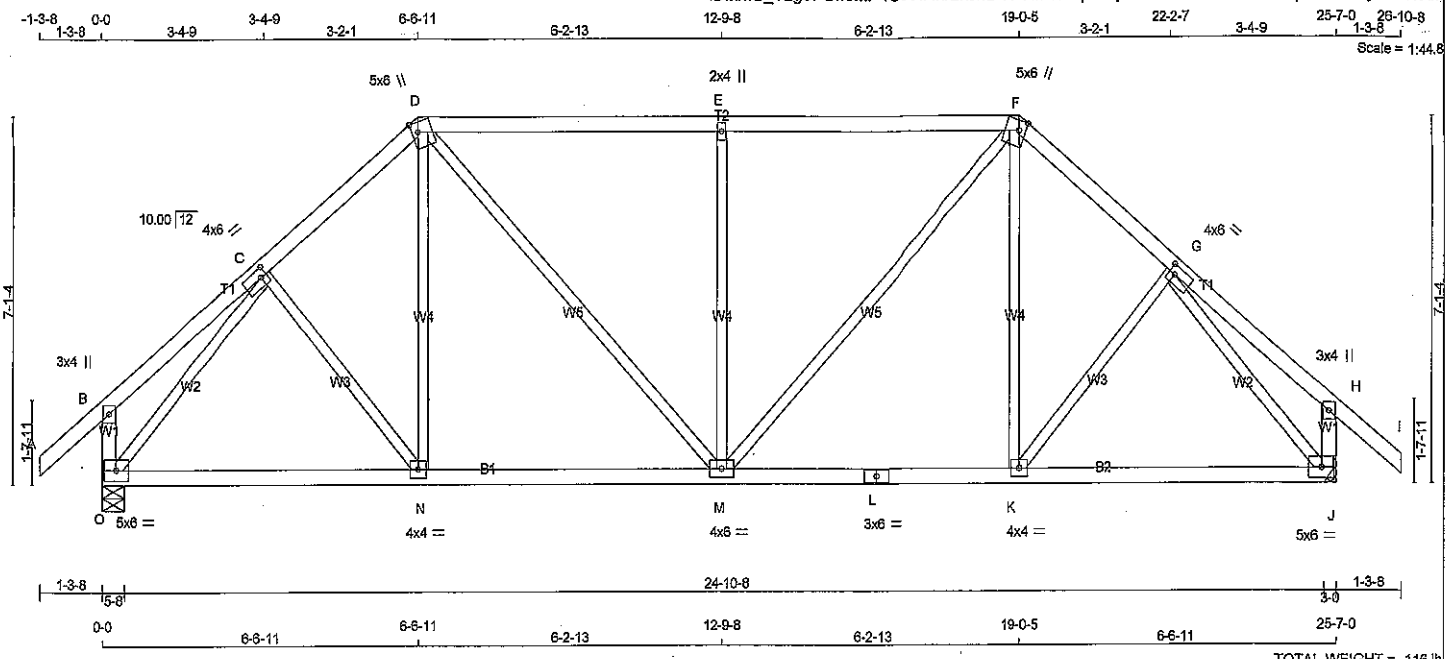

TOTAL WEIGHT = 116 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1846	0	1846	0	0	5-8	5-8
J	1846	0	1846	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	0 / 123	0.03 (3)
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	N- D	0 / 275	0.06 (2)
C- D	-1640 / 0	-104.9	-104.9	0.16 (1)	5.04	D- M	0 / 647	0.15 (1)
D- E	-1678 / 0	-104.9	-104.9	0.57 (1)	4.41	M- E	-801 / 0	0.70 (1)
E- F	-1678 / 0	-104.9	-104.9	0.57 (1)	4.41	M- F	0 / 647	0.15 (1)
F- G	-1640 / 0	-104.9	-104.9	0.16 (1)	5.04	K- F	0 / 275	0.06 (2)
G- H	0 / 23	-104.9	-104.9	0.16 (1)	10.00	K- G	0 / 123	0.03 (3)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-1919 / 0	0.83 (1)
O- B	-279 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-1919 / 0	0.83 (1)
J- H	-279 / 0	0.0	0.0	0.03 (1)	7.81			
O- N	0 / 1208	-28.0	-28.0	0.42 (2)	10.00			
N- M	0 / 1240	-28.0	-28.0	0.43 (2)	10.00			
M- L	0 / 1240	-28.0	-28.0	0.43 (2)	10.00			
L- K	0 / 1240	-28.0	-28.0	0.43 (2)	10.00			
K- J	0 / 1208	-28.0	-28.0	0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.43/1.00 (M-N:2) , WB=0.83/1.00 (C-O:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.47 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10591-R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:15 2018 Page 1

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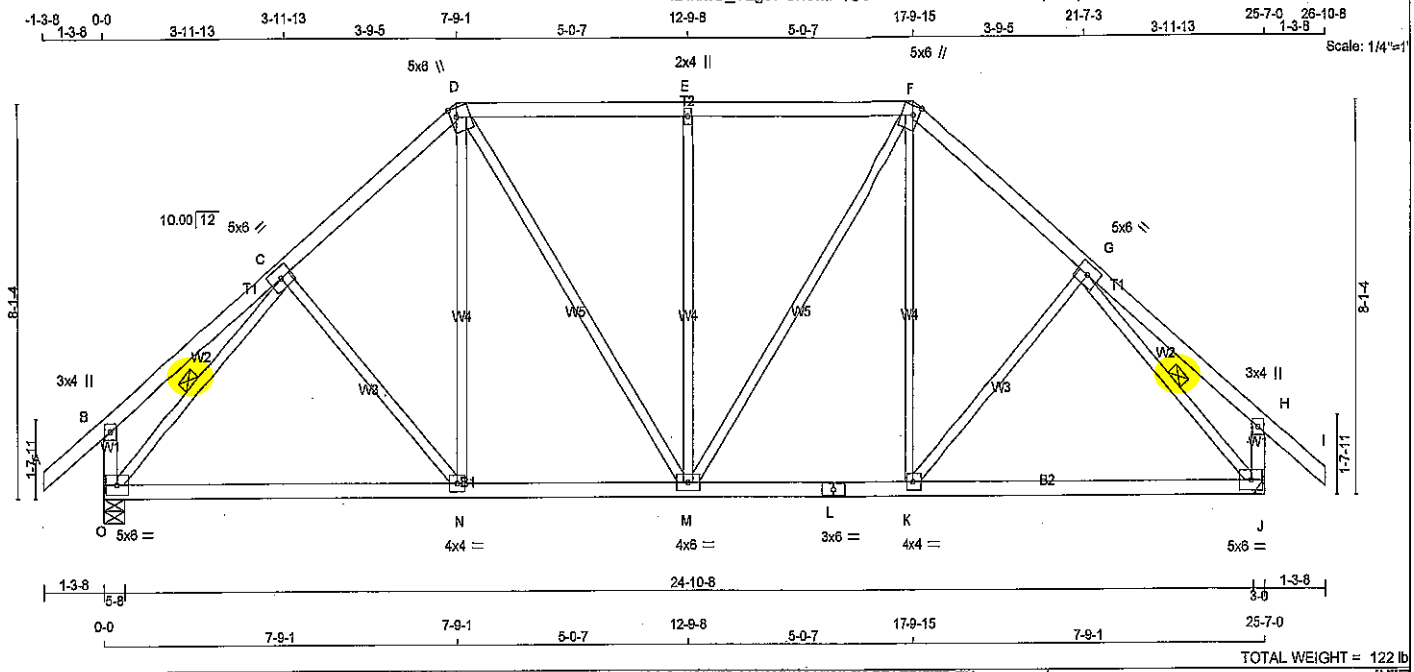


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1846	0	1846	0	0	5-8	5-8
J	1846	0	1846	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	-65 / 86	0.04 (1)
B- C	0 / 29	-104.9	-104.9	0.25 (1)	10.00	N- D	0 / 362	0.08 (2)
C- D	-1592 / 0	-104.9	-104.9	0.22 (1)	5.03	D- M	0 / 426	0.10 (1)
D- E	-1432 / 0	-104.9	-104.9	0.36 (1)	5.04	M- E	-642 / 0	0.81 (1)
E- F	-1432 / 0	-104.9	-104.9	0.36 (1)	5.04	M- F	0 / 426	0.10 (1)
F- G	-1592 / 0	-104.9	-104.9	0.22 (1)	5.03	K- F	0 / 362	0.08 (2)
G- H	0 / 29	-104.9	-104.9	0.25 (1)	10.00	K- G	-65 / 86	0.04 (1)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-1911 / 0	0.48 (1)
O- B	-301 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-1911 / 0	0.48 (1)
J- H	-301 / 0	0.0	0.0	0.03 (1)	7.81			
O- N	0 / 1240	-28.0	-28.0	0.51 (2)	10.00			
N- M	0 / 1201	-28.0	-28.0	0.52 (2)	10.00			
M- L	0 / 1201	-28.0	-28.0	0.52 (2)	10.00			
L- K	0 / 1201	-28.0	-28.0	0.52 (2)	10.00			
K- J	0 / 1240	-28.0	-28.0	0.51 (2)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.52/1.00 (M-N:2) , WB=0.81/1.00 (E-M:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

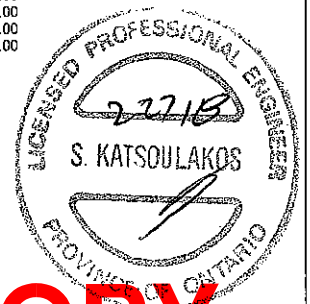
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.47 (C) (INPUT = 1.00)



DWG NO. TAM 105428

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:16 2018 Page 1

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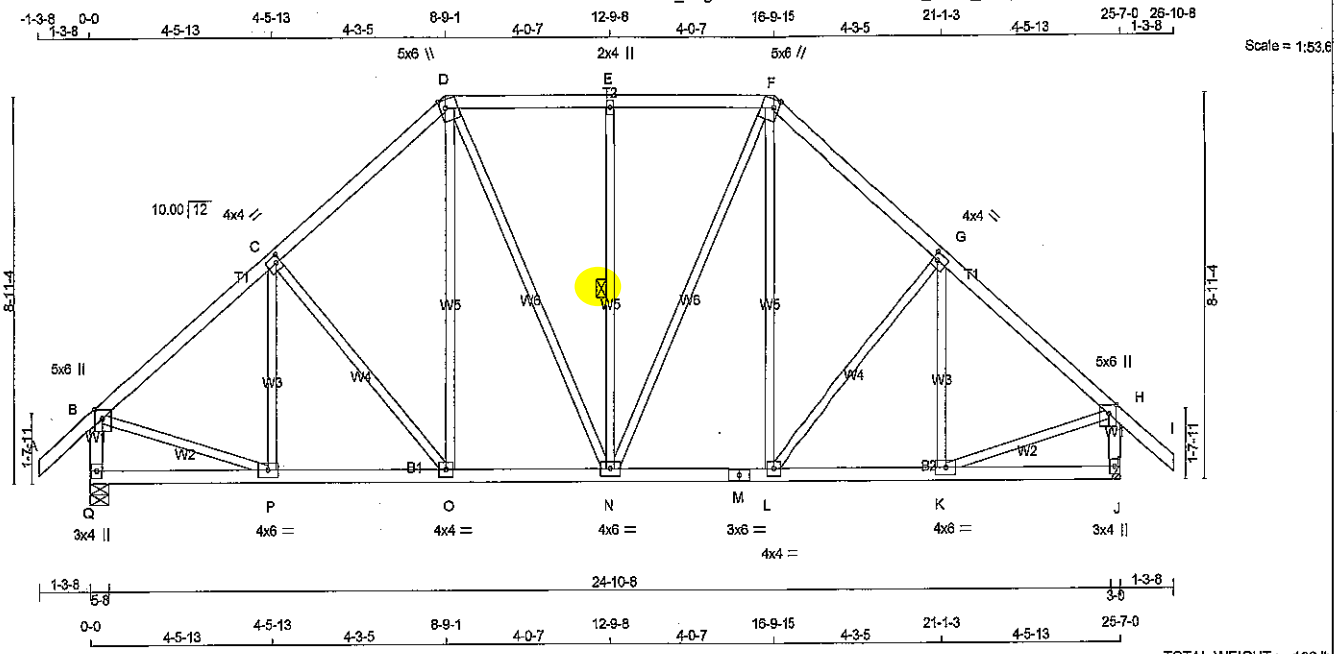

TOTAL WEIGHT = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1846	0	1846	0	0	5-8	5-8
J	1846	0	1846	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1454	921 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1663 / 0	-104.9	-104.9	0.30 (1)		4.86
C- D	-1520 / 0	-104.9	-104.9	0.29 (1)		5.04
D- E	-1284 / 0	-104.9	-104.9	0.22 (1)		5.45
E- F	-1284 / 0	-104.9	-104.9	0.22 (1)		5.45
F- G	-1520 / 0	-104.9	-104.9	0.29 (1)		5.04
G- H	-1663 / 0	-104.9	-104.9	0.30 (1)		4.86
H- I	0 / 47	-104.9	-104.9	0.14 (1)		10.00
Q- B	-1793 / 0	0.0	0.0	0.19 (1)		6.24
J- H	-1793 / 0	0.0	0.0	0.19 (1)		6.24
Q- P	0 / 0	-28.0	-28.0	0.13 (3)		10.00
P- O	0 / 1304	-28.0	-28.0	0.27 (1)		10.00
O- N	0 / 1141	-28.0	-28.0	0.23 (1)		10.00
N- M	0 / 1141	-28.0	-28.0	0.23 (1)		10.00
M- L	0 / 1141	-28.0	-28.0	0.23 (1)		10.00
L- K	0 / 1304	-28.0	-28.0	0.27 (1)		10.00
K- J	0 / 0	-28.0	-28.0	0.13 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-244 / 86	0.11 (1)
C- O	-254 / 0	0.21 (1)
O- D	0 / 319	0.07 (2)
D- N	0 / 339	0.08 (1)
N- E	-507 / 0	0.26 (1)
N- F	0 / 339	0.08 (1)
L- F	0 / 319	0.07 (2)
L- G	-254 / 0	0.21 (1)
K- G	-244 / 86	0.11 (1)
B- P	0 / 1358	0.31 (1)
K- H	0 / 1358	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.27/1.00 (O-P:1) , WB=0.31/1.00 (B-P:1) , SSI=0.20/1.00 (D-E:1)

DOL: LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.33 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
2216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10543-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:16 2018 Page 1

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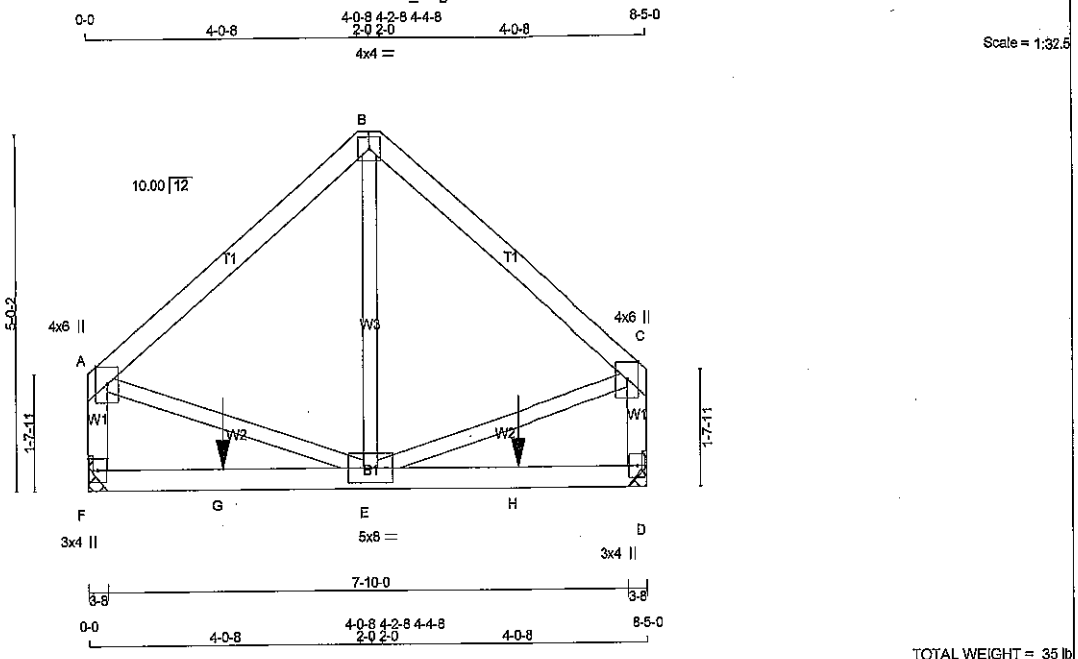

TOTAL WEIGHT = 35 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW-p	MT20	4.0	4.0		
C	TMVW+p	MT20	4.0	6.0	Edge	
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 719.1 lbs FACTORED DOWN AT 2-0-0, AND 719.1 lbs FACTORED DOWN AT 6-5-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1541	0	1541	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
D	1541	0	1541	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1215	767 / 0	227 / 0	0 / 0	0 / 0	222 / 0	0 / 0
D	1215	767 / 0	227 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1019 / 0	-167.2	-167.2	0.61 (1)	5.19	A- E	0 / 820	0.20 (1)
B- C	-1019 / 0	-167.2	-167.2	0.61 (1)	5.19	E- C	0 / 820	0.20 (1)
F- A	-1247 / 0	0.0	0.0	0.15 (1)	7.16	E- B	0 / 601	0.15 (1)
D- C	-1247 / 0	0.0	0.0	0.15 (1)	7.16			
F- G	0 / 0	-28.0	-28.0	0.90 (1)	10.00			
G- E	0 / 0	-28.0	-28.0	0.90 (1)	10.00			
E- H	0 / 0	-28.0	-28.0	0.90 (1)	10.00			
H- D	0 / 0	-28.0	-28.0	0.90 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-0	-719	-719	—	FRONT	VERT	TOTAL
H	6-5-0	-719	-719	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 8-5-0
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 780 (0.13")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.90/1.00 (E-F:1) , WB=0.20/1.00 (A-E:1) , SSI=0.47/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.26 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0544-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:16 2018 Page 1

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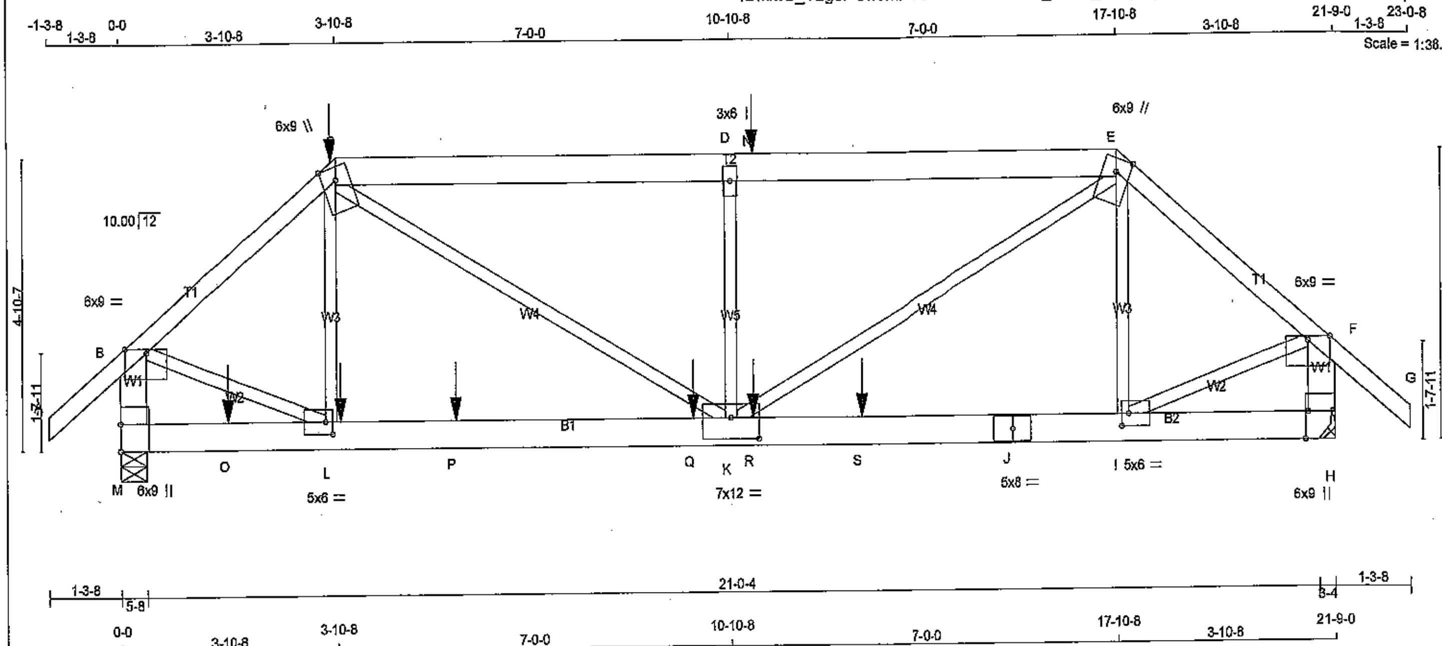

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	9.0	Edge	
F	TMVW-p	MT20	6.0	9.0	Edge	
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW-t	MT20	5.0	6.0	2.50	1.50
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	7.0	12.0	4.25	6.00
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 330.7 lbs FACTORED DOWN AT 3-10-8, AND 98.4 lbs FACTORED DOWN AT 11-3-4 ON TOP CHORD, AND 45.9 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 414.7 lbs FACTORED DOWN AT 6-0-0, 414.7 lbs FACTORED DOWN AT 10-2-8, AND 45.9 lbs FACTORED DOWN AT 11-3-4, AND 1294.0 lbs FACTORED DOWN AT 13-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	3580	0	3580	0	0	5-8	5-8
H	3403	0	3403	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-4	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2798	1808 / 0	492 / 0	0 / 0	0 / 0	497 / 0	0 / 0
H	2664	1715 / 0	475 / 0	0 / 0	0 / 0	475 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-3527 / 0	-158.9	-158.9	0.74 (1)	2.94
C- D	-5225 / 0	-158.9	-158.9	0.86 (1)	2.78
D- N	-5225 / 0	-158.9	-158.9	0.86 (1)	2.78
N- E	-5225 / 0	-158.9	-158.9	0.86 (1)	2.78
E- F	-3470 / 0	-158.9	-158.9	0.73 (1)	2.98
F- G	0 / 47	-104.9	-104.9	0.16 (1)	10.00
M- B	-3583 / 0	0.0	0.0	0.26 (1)	5.58
H- F	-3533 / 0	0.0	0.0	0.26 (1)	5.62
M- O	0 / 0	-28.0	-28.0	0.13 (1)	10.00
O- L	0 / 0	-28.0	-28.0	0.13 (1)	10.00
L- P	0 / 2726	-28.0	-28.0	0.70 (1)	10.00
P- Q	0 / 2726	-28.0	-28.0	0.70 (1)	10.00
Q- K	0 / 2726	-28.0	-28.0	0.70 (1)	10.00
K- R	0 / 2676	-28.0	-28.0	0.90 (1)	10.00
R- S	0 / 2676	-28.0	-28.0	0.90 (1)	10.00
S- J	0 / 2676	-28.0	-28.0	0.90 (1)	10.00
J- I	0 / 2676	-28.0	-28.0	0.90 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-397 / 109	0.13 (1)
C- K	0 / 2957	0.73 (1)
K- D	-1464 / 0	0.46 (1)
K- E	0 / 3016	0.75 (1)
I- E	-140 / 150	0.05 (1)
B- L	0 / 2855	0.71 (1)
I- F	0 / 2809	0.70 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-331	-331	---	BACK	VERT	TOTAL
L	3-11-4	-26	-46	---	BACK	VERT	TOTAL
N	11-3-4	-98	-98	---	BACK	VERT	TOTAL
O	1-11-4	-22	-46	---	BACK	VERT	TOTAL
P	6-0-0	-415	-415	---	BACK	VERT	TOTAL
Q	10-2-8	-415	-415	---	BACK	VERT	TOTAL
R	11-3-4	-26	-46	---	BACK	VERT	TOTAL
S	13-2-8	-1294	-1294	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
22/7/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-10-8
END DISTANCE = 21-9-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 3-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 675 (0.39")

CSI: TC=0.86/1.00 (D-E:1) , BC=0.90/1.00 (I-K:1) , WB=0.75/1.00 (E-K:1) , SSI=0.68/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.69 (J) (INPUT = 1.00)

DWG NO. TAM / 0545-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:17 2018 Page 1

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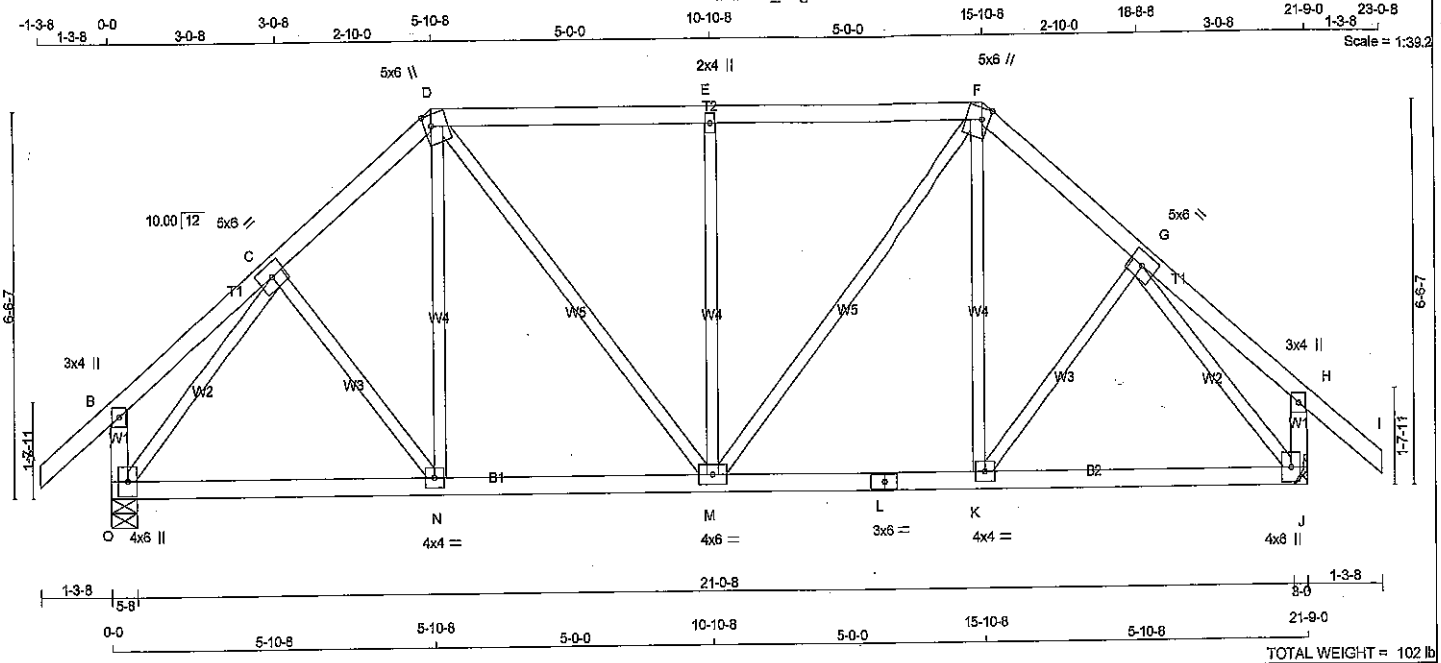

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1591	0	1591	0	0	5-8	5-8
J	1591	0	1591	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1251	796 / 0	228 / 0	0 / 0	0 / 0	226 / 0	0 / 0
J	1251	796 / 0	228 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 20	-104.9	-104.9	0.13 (1)	10.00	
C- D	-1340 / 0	-104.9	-104.9	0.12 (1)	5.49	
D- E	-1313 / 0	-104.9	-104.9	0.35 (1)	5.23	
E- F	-1313 / 0	-104.9	-104.9	0.35 (1)	5.23	
F- G	-1340 / 0	-104.9	-104.9	0.12 (1)	5.49	
G- H	0 / 20	-104.9	-104.9	0.13 (1)	10.00	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
O- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-266 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 980	-28.0	-28.0	0.33 (2)	10.00	
N- M	0 / 1012	-28.0	-28.0	0.34 (2)	10.00	
M- L	0 / 1012	-28.0	-28.0	0.34 (2)	10.00	
L- K	0 / 1012	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 980	-28.0	-28.0	0.33 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 111	0.03 (3)
N- D	0 / 235	0.05 (2)
D- M	0 / 480	0.11 (1)
M- E	-638 / 0	0.45 (1)
M- F	0 / 480	0.11 (1)
K- F	0 / 235	0.05 (2)
K- G	0 / 111	0.03 (3)
O- C	-1594 / 0	0.57 (1)
G- J	-1594 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.34/1.00 (K-M:2) , WB=0.57/1.00 (G-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (O) (INPUT = 0.90)
JSI METAL= 0.39 (G) (INPUT = 1.00)

[M][F]

LICENSED PROFESSIONAL ENGINEER
2-27-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10546-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T20C	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:31 2018 Page 1

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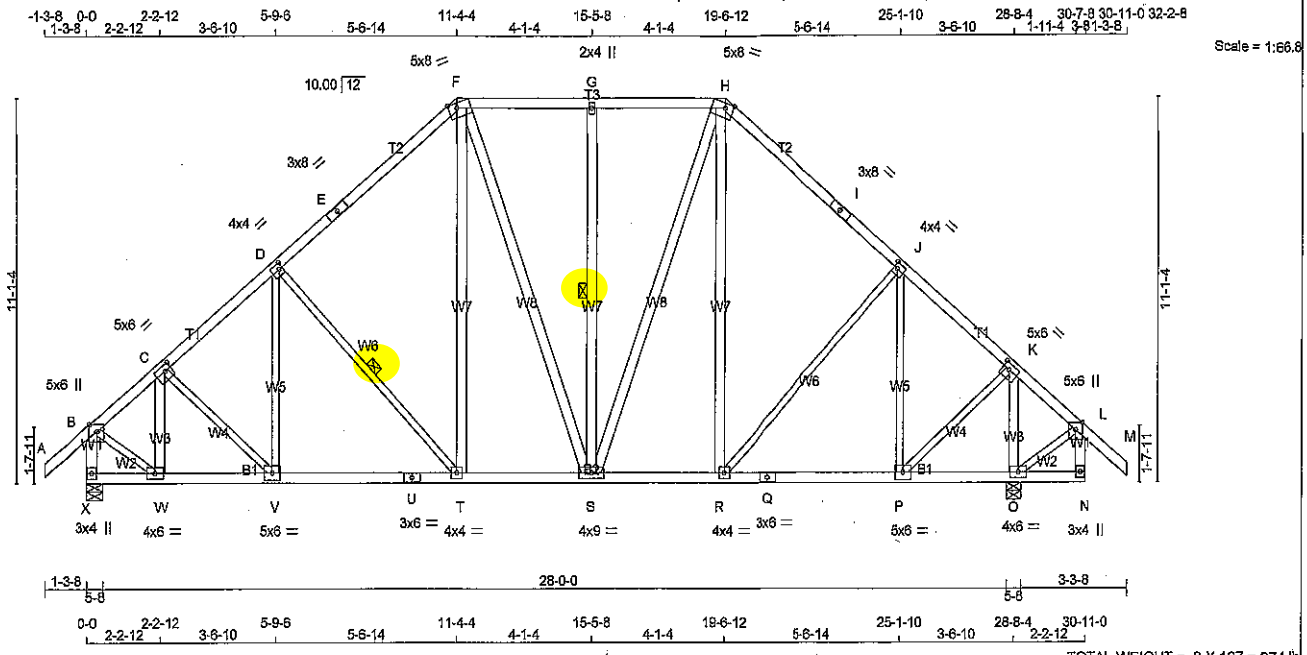

TOTAL WEIGHT = 2 X 187 = 374 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - D	2x3	DRY	No.2	SPF
D - T	2x3	DRY	No.2	SPF
R - J	2x3	DRY	No.2	SPF
P - J	2x3	DRY	No.2	SPF
C - V	2x3	DRY	No.2	SPF
B - W	2x3	DRY	No.2	SPF
O - L	2x3	DRY	No.2	SPF
P - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	5.0	6.0	2.00	2.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TS-t	MT20	3.0	8.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	4.0	2.00	1.25
K	TMWW-t	MT20	5.0	6.0	2.00	2.50
L	TMW+p	MT20	5.0	6.0	Edge	
N	BMV+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWWW-t	MT20	4.0	9.0		
T	BMWW-t	MT20	4.0	4.0		
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMWW-t	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2029	0	2029	0	0	5-8	5-8
O	2371	0	2371	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1601	1009 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
O	1871	1179 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-T, G-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1596 / 0	-104.9	-104.9	0.09 (1)		5.16
C- D	-1889 / 0	-104.9	-104.9	0.36 (1)		4.53
D- E	-1585 / 0	-104.9	-104.9	0.44 (1)		4.76
E- F	-1585 / 0	-104.9	-104.9	0.44 (1)		4.76
F- G	-1245 / 0	-104.9	-104.9	0.23 (1)		5.50
G- H	-1245 / 0	-104.9	-104.9	0.23 (1)		5.50
H- I	-1424 / 0	-104.9	-104.9	0.43 (1)		4.97
I- J	-1424 / 0	-104.9	-104.9	0.43 (1)		4.97
J- K	-1169 / 0	-104.9	-104.9	0.33 (1)		5.49
K- L	0 / 243	-104.9	-104.9	0.10 (1)		10.00
L- M	0 / 47	-104.9	-104.9	0.14 (1)		10.00
X- B	-1998 / 0	0.0	0.0	0.21 (1)		5.97
N- L	0 / 37	0.0	0.0	0.01 (3)		10.00
X- W	0 / 0	-28.0	-28.0	0.03 (2)		10.00
W- V	0 / 1230	-28.0	-28.0	0.28 (2)		10.00
V- U	0 / 1475	-28.0	-28.0	0.33 (1)		10.00
U- T	0 / 1475	-28.0	-28.0	0.33 (1)		10.00
T- S	0 / 1195	-28.0	-28.0	0.29 (2)		10.00
S- R	0 / 1066	-28.0	-28.0	0.27 (2)		10.00
R- Q	0 / 933	-28.0	-28.0	0.26 (2)		10.00
Q- P	0 / 933	-28.0	-28.0	0.26 (2)		10.00
P- O	-176 / 0	-28.0	-28.0	0.11 (3)		6.25
O- N	0 / 0	-28.0	-28.0	0.07 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- D	-74 / 163	0.05 (1)
D- T	-418 / 0	0.20 (1)
T- F	0 / 455	0.07 (1)
F- S	0 / 140	0.02 (1)
S- G	-516 / 0	0.33 (1)
S- H	0 / 505	0.08 (1)
R- H	0 / 196	0.03 (3)
R- J	0 / 194	0.04 (1)
P- J	-860 / 0	0.58 (1)
W- C	-654 / 0	0.10 (1)
C- V	0 / 327	0.07 (1)
B- W	0 / 1425	0.32 (1)
O- K	-2164 / 0	0.32 (1)
O- L	-204 / 0	0.03 (1)
P- K	0 / 1484	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.44/1.00 (D-F:1) , BC=0.33/1.00 (T-V:1) , WB=0.58/1.00 (J-P:1) , SSI=0.21/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.43 (U) (INPUT = 1.00)

DWG NO. TAM 10554-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:31 2018 Page 1
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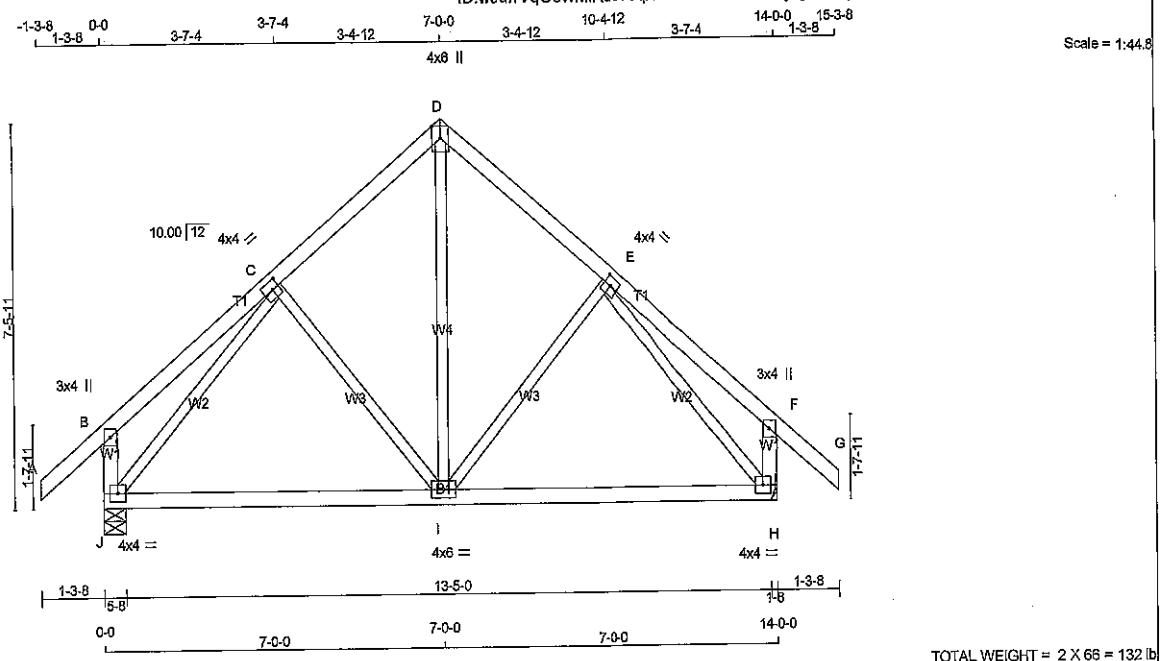

TOTAL WEIGHT = 2 X 66 = 132 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1076	0	1076	0	0	5-8	5-8
H	1076	0	1076	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	I- D	0 / 511	0.12 (1)
B- C	0 / 27	-104.9	-104.9	0.20 (1)	10.00	I- E	-176 / 38	0.09 (1)
C- D	-650 / 0	-104.9	-104.9	0.16 (1)	6.25	C- I	-176 / 38	0.09 (1)
D- E	-650 / 0	-104.9	-104.9	0.16 (1)	6.25	J- C	-929 / 0	0.45 (1)
E- F	0 / 27	-104.9	-104.9	0.20 (1)	10.00	E- H	-929 / 0	0.45 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00			
J- B	-285 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-285 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 592	-28.0	-28.0	0.46 (2)	10.00			
I- H	0 / 592	-28.0	-28.0	0.46 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.20/1.00 (E-F:1) , BC=0.46/1.00 (I-J:2) , WB=0.45/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22718
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10555-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T21S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:31 2018 Page 1

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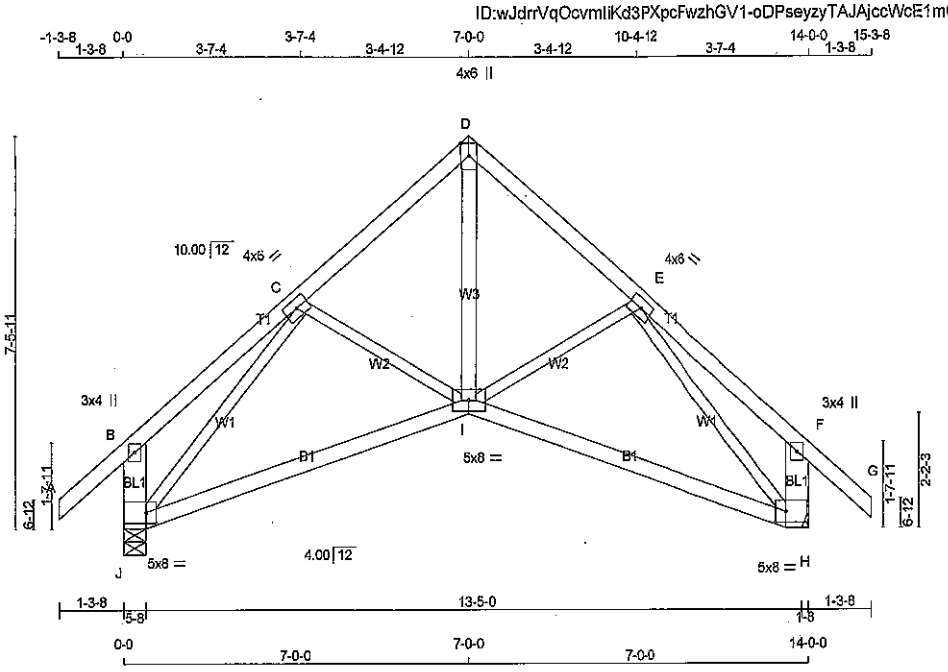

TOTAL WEIGHT = 3 X 67 = 200 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2 - 2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMK+p	MT20	3.0	4.0		
H	BWKM1*+t	MT20	5.0	8.0	2.50	2.50
I	BBWWW-p	MT20	5.0	8.0		
J	BWKM1*+t	MT20	5.0	8.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQR0
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
J	1076	0	1076	0	0	IN-SX 5-8	IN-SX 5-8
H	1076	0	1076	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 51	-104.9	-104.9	0.17 (1)	10.00	I- D	0 / 811	0.13 (1)	
B- C	-11 / 0	-104.9	-104.9	0.14 (1)	6.25	I- E	-110 / 55	0.03 (1)	
C- D	-886 / 0	-104.9	-104.9	0.15 (1)	6.25	C- I	-110 / 55	0.03 (1)	
D- E	-886 / 0	-104.9	-104.9	0.15 (1)	6.25	J- C	-1168 / 0	0.55 (1)	
E- F	-11 / 0	-104.9	-104.9	0.14 (1)	6.25	E- H	-1168 / 0	0.55 (1)	
F- G	0 / 51	-104.9	-104.9	0.17 (1)	10.00				
J- B	-320 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-320 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 799	-28.0	-28.0	0.48 (2)	10.00				
I- H	0 / 799	-28.0	-28.0	0.48 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 969 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 584 (0.29")

CSI: TC=0.17/1.00 (F-G:1) , BC=0.48/1.00 (I-J:2) , WB=0.55/1.00 (E-H:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

DWG NO. TAM/10556-T1
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T22	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:32 2018 Page 1

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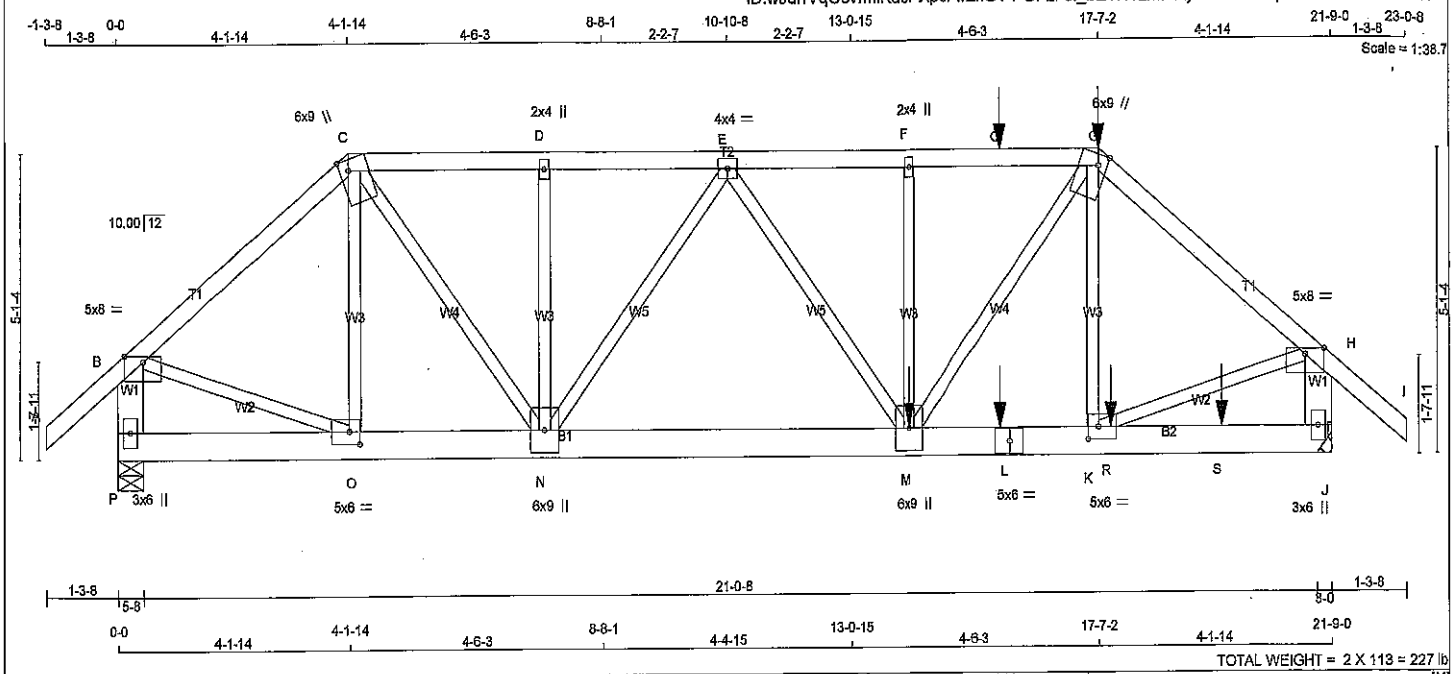

TOTAL WEIGHT = 2 X 113 = 227 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
P- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- L	2	12	SIDE(183.1)
L- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- G	1	6	SIDE(15.5)
F- M	1	5	SIDE(510.8)
2x3	1	6	
D- N	1	5	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF.SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BS-t	MT20	5.0	6.0		
M	BMWWW+t	MT20	6.0	9.0		
N	BMWWW+t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2830	0	2830	0	0	5-8	5-8
J	4659	0	4659	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2213	1429 / 0	391 / 0	0 / 0	0 / 0	393 / 0	0 / 0
J	3654	2340 / 0	657 / 0	0 / 0	0 / 0	656 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	O- C	-496 / 0	0.09 (1)
B- C	-2804 / 0	-104.9	-104.9	0.22 (1)	5.25	C- N	0 / 2366	0.29 (1)
C- D	-3541 / 0	-104.9	-104.9	0.13 (1)	4.88	N- D	-367 / 0	0.07 (1)
D- E	-3541 / 0	-104.9	-104.9	0.12 (1)	4.89	M- F	-372 / 0	0.07 (1)
E- F	-5218 / 0	-104.9	-104.9	0.16 (1)	4.13	M- G	0 / 2664	0.33 (1)
F- Q	-5218 / 0	-104.9	-104.9	0.22 (1)	4.06	K- G	-111 / 151	0.02 (1)
Q- G	-5218 / 0	-104.9	-104.9	0.22 (1)	4.06	B- O	0 / 2235	0.28 (1)
G- H	-4751 / 0	-104.9	-104.9	0.30 (1)	4.18	K- H	0 / 3787	0.47 (1)
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00	N- E	-1683 / 0	0.47 (1)
P- B	-2781 / 0	0.0	0.0	0.10 (1)	7.81	E- M	0 / 1278	0.16 (1)
J- H	-4460 / 0	0.0	0.0	0.16 (1)	6.81			
P- O	0 / 0	-28.0	-28.0	0.03 (3)	10.00			
O- N	0 / 2137	-28.0	-28.0	0.16 (1)	10.00			
N- M	0 / 4494	-28.0	-28.0	0.35 (1)	10.00			
M- L	0 / 3638	-28.0	-28.0	0.34 (1)	10.00			
L- K	0 / 3638	-28.0	-28.0	0.34 (1)	10.00			
K- R	0 / 0	-28.0	-28.0	0.12 (1)	10.00			
R- S	0 / 0	-28.0	-28.0	0.12 (1)	10.00			
S- J	0 / 0	-28.0	-28.0	0.12 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
G	17-7-2	-527	-527	---	FRONT	VERT	TOTAL
G	17-7-2	-57	-57	---	BACK	VERT	TOTAL
L	15-9-12	-337	-337	---	BACK	VERT	TOTAL
M	14-1-8	-2660	-2660	---	BACK	VERT	TOTAL
Q	15-9-12	-54	-54	---	BACK	VERT	TOTAL
R	17-9-12	-337	-337	---	BACK	VERT	TOTAL
S	19-9-12	-337	-337	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.30/1.00 (G-H:1) , BC=0.35/1.00 (M-N:1) , WB=0.47/1.00 (E-N:1) , SSI=0.20/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.45 (O) (INPUT = 1.00)

DWG NO. TAM 10557-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T22	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 17-7-2, AND 53.9 lbs FACTORED DOWN AT 15-9-12, AND 55.9 lbs FACTORED DOWN AT 17-7-2 ON TOP CHORD, AND 2660.0 lbs FACTORED DOWN AT 14-1-8, 336.6 lbs FACTORED DOWN AT 15-9-12, AND 336.6 lbs FACTORED DOWN AT 17-9-12, AND 336.6 lbs FACTORED DOWN AT 19-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9-27-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10557-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T23	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:32 2018 Page 1
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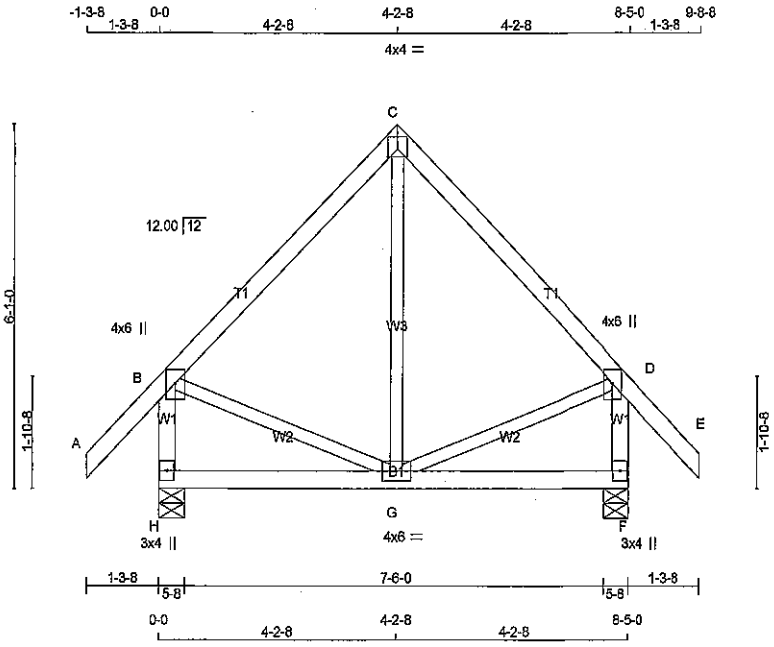

TOTAL WEIGHT = 3 X 42 = 127 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	706	0	706	0	0	5-8	5-8
F	706	0	706	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	545	364 / 0	88 / 0	0 / 0	0 / 0	93 / 0	0 / 0
F	545	364 / 0	88 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	G- C	-11 / 180	0.04 (3)	
B- C	-305 / 0	-104.9	-104.9	0.24 (1)	6.25	B- G	0 / 229	0.05 (1)	
C- D	-305 / 0	-104.9	-104.9	0.24 (1)	6.25	G- D	0 / 229	0.05 (1)	
D- E	0 / 52	-104.9	-104.9	0.15 (1)	10.00				
H- B	-660 / 0	0.0	0.0	0.07 (1)	7.81				
F- D	-660 / 0	0.0	0.0	0.07 (1)	7.81				
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00				
G- F	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.14/1.00 (G-H:3) , WB=0.05/1.00 (D-G:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10558-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	G23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:29 2018 Page 1
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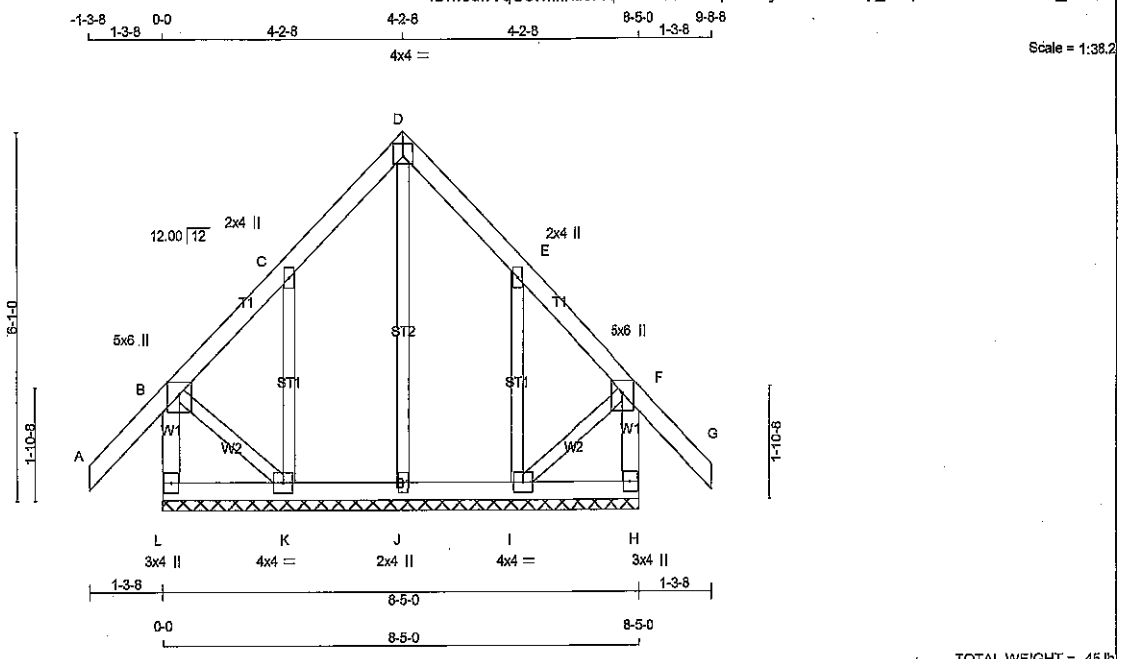

TOTAL WEIGHT = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-270 / 0	0.0	0.0	0.03 (1)	7.81	J- D	-136 / 0	0.07 (1)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	K- C	-267 / 0	0.07 (1)
B- C	-14 / 0	-104.9	-104.9	0.08 (1)	6.25	I- E	-267 / 0	0.07 (1)
C- D	-41 / 0	-104.9	-104.9	0.08 (1)	6.25	B- K	0 / 28	0.01 (1)
D- E	-41 / 0	-104.9	-104.9	0.08 (1)	6.25	I- F	0 / 28	0.01 (1)
E- F	-14 / 0	-104.9	-104.9	0.08 (1)	6.25			
F- G	0 / 52	-104.9	-104.9	0.15 (1)	10.00			
H- F	-270 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00			
K- J	0 / 16	-28.0	-28.0	0.03 (2)	10.00			
J- I	0 / 16	-28.0	-28.0	0.03 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (F-G:1) , BC=0.03/1.00 (I-J:2) , WB=0.07/1.00 (D-J:1) , SSI=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

DWG NO. TAM10559-R8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287445	T30S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:12:53 2018 Page 1

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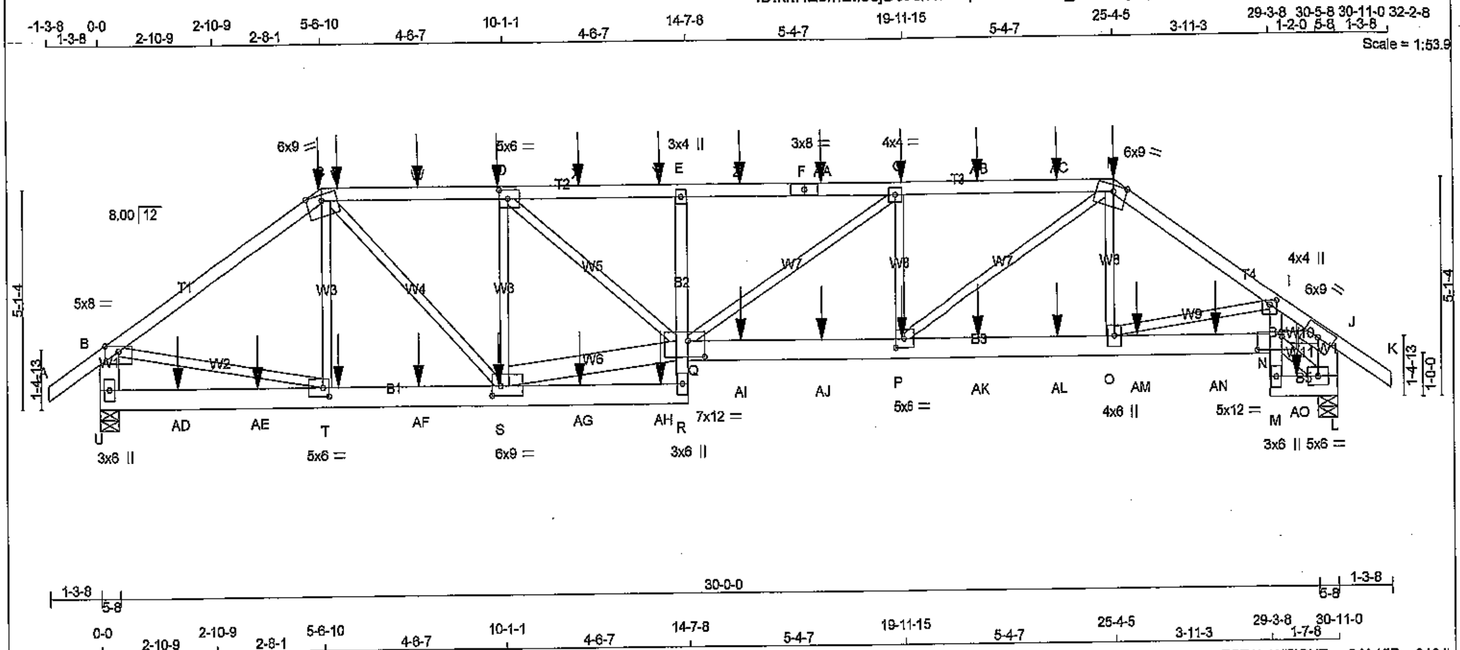

TOTAL WEIGHT = 2 X 157 = 313 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - Q	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- K	1	12	SIDE(61.0)
U- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
Q- N	2	12	SIDE(183.1)
M- L	2	12	SIDE(0.0)
R- E	1	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T- C	1	6	SIDE(15.5)
O- H	1	6	SIDE(15.5)
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	5462	0	5462	0	0	5-8	5-8
L	5591	0	5591	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
U	4349	2673 / 0	858 / 0	0 / 0	0 / 0	819 / 0	0 / 0
L	4461	2726 / 0	890 / 0	0 / 0	0 / 0	845 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-6679 / 0	-104.9	-104.9	0.68 (1)	3.32
C- V	-7886 / 0	-104.9	-104.9	0.48 (1)	3.16
V- W	-7886 / 0	-104.9	-104.9	0.48 (1)	3.16
W- D	-7886 / 0	-104.9	-104.9	0.48 (1)	3.16
D- X	-11071 / 0	-104.9	-104.9	0.73 (1)	2.42
X- Y	-11071 / 0	-104.9	-104.9	0.73 (1)	2.42
Y- E	-11071 / 0	-104.9	-104.9	0.73 (1)	2.42
E- Z	-11134 / 0	-104.9	-104.9	0.95 (1)	2.29
Z- F	-11134 / 0	-104.9	-104.9	0.95 (1)	2.29
F-AA	-11134 / 0	-104.9	-104.9	0.95 (1)	2.29
AA- G	-11134 / 0	-104.9	-104.9	0.95 (1)	2.29
G-AB	-10443 / 0	-104.9	-104.9	0.88 (1)	2.45
AB-AC	-10443 / 0	-104.9	-104.9	0.88 (1)	2.45
AC- H	-10443 / 0	-104.9	-104.9	0.88 (1)	2.45
H- I	-8357 / 0	-104.9	-104.9	0.31 (1)	3.72
I- J	-9645 / 0	-104.9	-104.9	0.18 (1)	3.58
J- K	0 / 40	-104.9	-104.9	0.06 (1)	10.00
U- B	-5128 / 0	0.0	0.0	0.18 (1)	6.44
L- J	-5194 / 0	0.0	0.0	0.19 (1)	6.41
U-AD	0 / 0	-28.0	-28.0	0.22 (1)	10.00
AD-AE	0 / 0	-28.0	-28.0	0.22 (1)	10.00
AE- T	0 / 0	-28.0	-28.0	0.22 (1)	10.00
T-AF	0 / 5553	-28.0	-28.0	0.50 (1)	10.00
AF- S	0 / 5553	-28.0	-28.0	0.50 (1)	10.00
S-AG	0 / 236	-28.0	-28.0	0.16 (1)	10.00
AG-AH	0 / 236	-28.0	-28.0	0.16 (1)	10.00
AH- R	0 / 236	-28.0	-28.0	0.16 (1)	10.00
R- Q	0 / 460	0.0	0.0	0.22 (1)	10.00
Q- E	-554 / 0	0.0	0.0	0.18 (1)	7.81
Q-AI	0 / 10443	-28.0	-28.0	0.88 (1)	10.00
AI-AJ	0 / 10443	-28.0	-28.0	0.88 (1)	10.00
AJ- P	0 / 10443	-28.0	-28.0	0.88 (1)	10.00
P-AK	0 / 6987	-28.0	-28.0	0.63 (1)	10.00
AK-AL	0 / 6987	-28.0	-28.0	0.63 (1)	10.00
AL- O	0 / 6987	-28.0	-28.0	0.63 (1)	10.00
O-AM	0 / 8393	-28.0	-28.0	0.72 (1)	10.00
AM-AN	0 / 8393	-28.0	-28.0	0.72 (1)	10.00
AN- N	0 / 8393	-28.0	-28.0	0.72 (1)	10.00
M- N	0 / 248	0.0	0.0	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- T	0 / 5636	0.70 (1)
T- C	0 / 302	0.04 (3)
C- S	0 / 3393	0.42 (1)
S- D	-3312 / 0	0.62 (1)
S- Q	0 / 7892	0.51 (1)
D- Q	0 / 4113	0.51 (1)
Q- G	0 / 839	0.10 (1)
P- G	-1191 / 0	0.15 (1)
P- H	0 / 4178	0.52 (1)
O- H	0 / 1266	0.16 (1)
O- I	-1483 / 0	0.24 (1)
N- L	-440 / 0	0.02 (1)
N- J	0 / 7981	0.71 (1)

DWG NO. TAM 10573 -18
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 688 (0.54")

CSI: TC=0.95/1.00 (E-G:1) , BC=0.88/1.00 (P-Q:1) , WB=0.71/1.00 (J-N:1) , SSI=0.20/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287445	T30S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TTWW-m	MT20	6.0	9.0	1.75	4.00
I	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	TMVW-t	MT20	6.0	9.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-I	MT20	5.0	12.0	3.50	7.00
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BVMWWW-I	MT20	7.0	12.0	4.50	5.00
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0	2.75	2.50
T	BMWW-t	MT20	5.0	6.0	2.50	1.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 86.3 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, 53.9 lbs FACTORED DOWN AT 9-11-4, 53.9 lbs FACTORED DOWN AT 11-11-4, 53.9 lbs FACTORED DOWN AT 13-11-4, 27.3 lbs FACTORED DOWN AT 15-11-4, 27.3 lbs FACTORED DOWN AT 17-11-4, 27.3 lbs FACTORED DOWN AT 19-11-4, 27.3 lbs FACTORED DOWN AT 21-11-4, 27.3 lbs FACTORED DOWN AT 23-11-4, AND 526.9 lbs FACTORED DOWN AT 5-6-11, AND 526.9 lbs FACTORED DOWN AT 25-4-6 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, 336.6 lbs FACTORED DOWN AT 11-11-4, 343.0 lbs FACTORED DOWN AT 13-11-4, 350.9 lbs FACTORED DOWN AT 15-11-4, 350.9 lbs FACTORED DOWN AT 17-11-4, 350.9 lbs FACTORED DOWN AT 19-11-4, 350.9 lbs FACTORED DOWN AT 21-11-4, 350.9 lbs FACTORED DOWN AT 23-11-4, 350.9 lbs FACTORED DOWN AT 25-11-4, AND 350.9 lbs FACTORED DOWN AT 27-11-4, AND 343.3 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- I	0 / 847	0.0	0.0	0.36 (1)	10.00		
M-AO	0 / 355	-28.0	-28.0	0.07 (1)	10.00		
AO- L	0 / 355	-28.0	-28.0	0.07 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-6-11	-527	-527	—	FRONT	VERT	TOTAL
D	9-11-4	-54	-54	—	BACK	VERT	TOTAL
G	19-11-4	-27	-27	—	BACK	VERT	TOTAL
H	25-4-6	-527	-527	—	FRONT	VERT	TOTAL
P	19-11-4	-351	-351	—	BACK	VERT	TOTAL
S	9-11-4	-337	-337	—	BACK	VERT	TOTAL
T	5-11-4	-337	-337	—	BACK	VERT	TOTAL
V	5-11-4	-86	-86	—	BACK	VERT	TOTAL
W	7-11-4	-54	-54	—	BACK	VERT	TOTAL
X	11-11-4	-54	-54	—	BACK	VERT	TOTAL
Y	13-11-4	-54	-54	—	BACK	VERT	TOTAL
Z	15-11-4	-27	-27	—	BACK	VERT	TOTAL
AA	17-11-4	-27	-27	—	BACK	VERT	TOTAL
AB	21-11-4	-27	-27	—	BACK	VERT	TOTAL
AC	23-11-4	-27	-27	—	BACK	VERT	TOTAL
AD	1-11-4	-337	-337	—	BACK	VERT	TOTAL
AE	3-11-4	-337	-337	—	BACK	VERT	TOTAL
AF	7-11-4	-337	-337	—	BACK	VERT	TOTAL
AG	11-11-4	-337	-337	—	BACK	VERT	TOTAL
AH	13-11-4	-343	-343	—	BACK	VERT	TOTAL
AI	15-11-4	-351	-351	—	BACK	VERT	TOTAL
AJ	17-11-4	-351	-351	—	BACK	VERT	TOTAL
AK	21-11-4	-351	-351	—	BACK	VERT	TOTAL
AL	23-11-4	-351	-351	—	BACK	VERT	TOTAL
AM	25-11-4	-351	-351	—	BACK	VERT	TOTAL
AN	27-11-4	-351	-351	—	BACK	VERT	TOTAL
AO	29-11-4	-343	-343	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.81 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2218
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/25773 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T30	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:41 2018 Page 1

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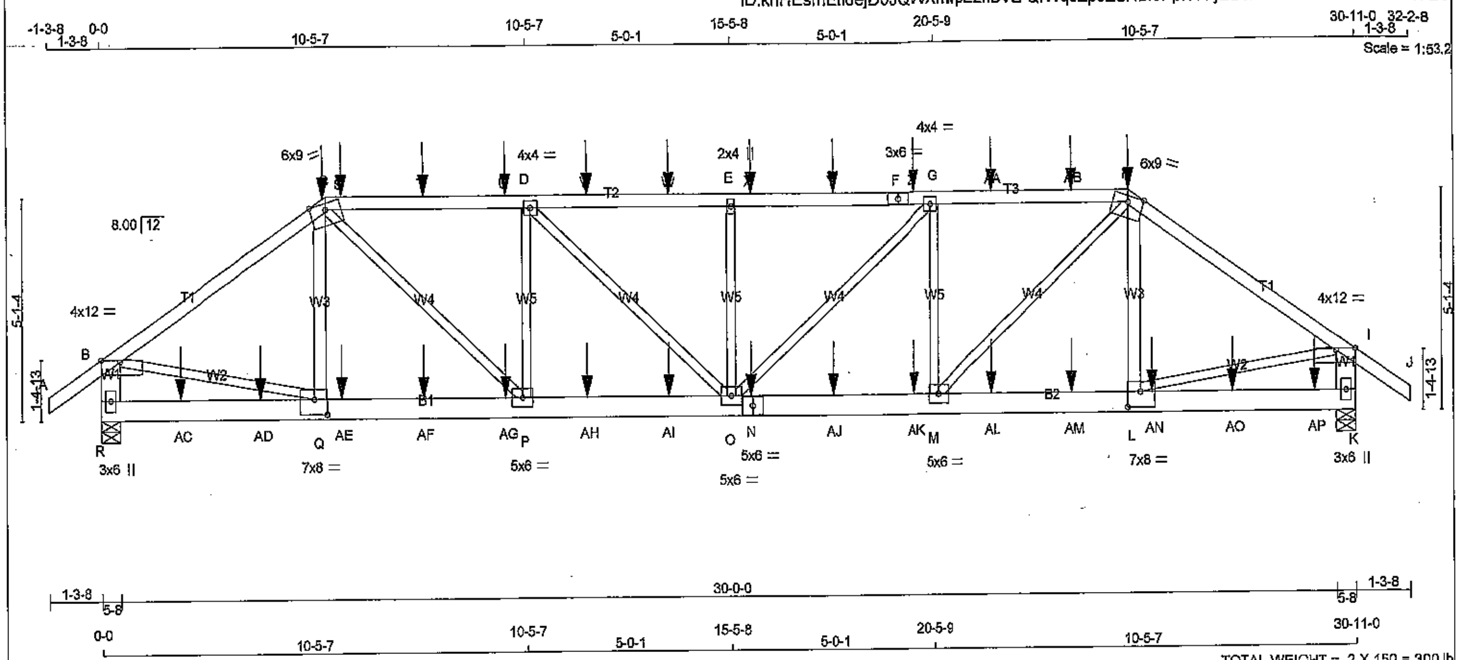

TOTAL WEIGHT = 2 X 150 = 300 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
L- H	1	6	SIDE(15.5)
Q- C	1	6	SIDE(15.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	Edge	5.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	4.0	12.0	Edge	5.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	7.0	8.0	4.25	3.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	7.0	8.0	4.25	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	5476	0	5476	0	0	5-8	5-8
K	5596	0	5596	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	4357	2685 / 0	854 / 0	0 / 0	0 / 0	818 / 0	0 / 0
K	4459	2735 / 0	883 / 0	0 / 0	0 / 0	841 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-6689 / 0	-104.9	-104.9	0.68 (1)	3.31
C- S	-8111 / 0	-104.9	-104.9	0.57 (1)	3.03
S- T	-8111 / 0	-104.9	-104.9	0.57 (1)	3.03
T- U	-8111 / 0	-104.9	-104.9	0.57 (1)	3.03
U- D	-8111 / 0	-104.9	-104.9	0.57 (1)	3.03
D- V	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
V- W	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
W- E	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
E- X	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
X- Y	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
Y- F	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
F- Z	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
Z- G	-8933 / 0	-104.9	-104.9	0.65 (1)	2.83
G-AA	-8094 / 0	-104.9	-104.9	0.57 (1)	3.04
AA-AB	-8094 / 0	-104.9	-104.9	0.57 (1)	3.04
AB- H	-8094 / 0	-104.9	-104.9	0.57 (1)	3.04
H- I	-6655 / 0	-104.9	-104.9	0.68 (1)	3.33
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00
R- B	-5180 / 0	0.0	0.0	0.19 (1)	6.41
K- I	-5156 / 0	0.0	0.0	0.18 (1)	6.42
R-AC	0 / 0	-28.0	-28.0	0.19 (1)	10.00
AC-AD	0 / 0	-28.0	-28.0	0.19 (1)	10.00
AD- Q	0 / 0	-28.0	-28.0	0.19 (1)	10.00
Q-AE	0 / 5555	-28.0	-28.0	0.53 (1)	10.00
AE-AF	0 / 5555	-28.0	-28.0	0.53 (1)	10.00
AF-AG	0 / 5555	-28.0	-28.0	0.53 (1)	10.00
AG- P	0 / 5555	-28.0	-28.0	0.53 (1)	10.00
P-AH	0 / 8111	-28.0	-28.0	0.65 (1)	10.00
AH-AI	0 / 8111	-28.0	-28.0	0.65 (1)	10.00
AI- O	0 / 8111	-28.0	-28.0	0.65 (1)	10.00
O- N	0 / 8094	-28.0	-28.0	0.68 (1)	10.00
N-AJ	0 / 8094	-28.0	-28.0	0.68 (1)	10.00
AJ-AK	0 / 8094	-28.0	-28.0	0.68 (1)	10.00
AK- M	0 / 8094	-28.0	-28.0	0.68 (1)	10.00
M-AL	0 / 5526	-28.0	-28.0	0.54 (1)	10.00
AL-AM	0 / 5526	-28.0	-28.0	0.54 (1)	10.00
AM- L	0 / 5526	-28.0	-28.0	0.54 (1)	10.00
L-AN	0 / 0	-28.0	-28.0	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	0 / 311	0.03 (3)
L- H	0 / 320	0.03 (3)
B- Q	0 / 5652	0.70 (1)
L- I	0 / 5623	0.70 (1)
M- H	0 / 3564	0.44 (1)
C- P	0 / 3547	0.44 (1)
M- G	-1507 / 0	0.28 (1)
P- D	-1493 / 0	0.28 (1)
O- G	0 / 1155	0.14 (1)
D- O	0 / 1132	0.14 (1)
O- E	-617 / 0	0.11 (1)

LICENSED PROFESSIONAL ENGINEER
2/27/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM /10575-18
STRUCTURAL COMPONENT ONLY

SITE COPY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.68/1.00 (B-C:1) , BC=0.68/1.00 (M-O:1) , WB=0.70/1.00 (B-Q:1) , SSI=0.24/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.62 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T30	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 5-6-11, 526.9 lbs FACTORED DOWN AT 25-4-6, 86.3 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, 53.9 lbs FACTORED DOWN AT 9-11-4, 53.9 lbs FACTORED DOWN AT 11-11-4, 53.9 lbs FACTORED DOWN AT 13-11-4, 53.9 lbs FACTORED DOWN AT 15-11-4, 53.9 lbs FACTORED DOWN AT 17-11-4, 53.9 lbs FACTORED DOWN AT 19-11-4, AND 53.9 lbs FACTORED DOWN AT 21-11-4, AND 53.9 lbs FACTORED DOWN AT 23-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, 336.6 lbs FACTORED DOWN AT 11-11-4, 336.6 lbs FACTORED DOWN AT 13-11-4, 336.6 lbs FACTORED DOWN AT 15-11-4, 336.6 lbs FACTORED DOWN AT 17-11-4, 336.6 lbs FACTORED DOWN AT 19-11-4, 336.6 lbs FACTORED DOWN AT 21-11-4, 336.6 lbs FACTORED DOWN AT 23-11-4, 336.6 lbs FACTORED DOWN AT 25-11-4, AND 336.6 lbs FACTORED DOWN AT 27-11-4, AND 336.6 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN-AO	0 / 0	-28.0	-28.0	0.19 (1)	MAX. UNBRAC LENGTH	10.00	
AO-AP	0 / 0	-28.0	-28.0	0.19 (1)		10.00	
AP- K	0 / 0	-28.0	-28.0	0.19 (1)		10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-6-11	-527	-527	—	FRONT	VERT	TOTAL
H	25-4-6	-527	-527	—	FRONT	VERT	TOTAL
N	15-11-4	-337	-337	—	BACK	VERT	TOTAL
S	5-11-4	-86	-86	—	BACK	VERT	TOTAL
T	7-11-4	-54	-54	—	BACK	VERT	TOTAL
U	9-11-4	-54	-54	—	BACK	VERT	TOTAL
V	11-11-4	-54	-54	—	BACK	VERT	TOTAL
W	13-11-4	-54	-54	—	BACK	VERT	TOTAL
X	15-11-4	-54	-54	—	BACK	VERT	TOTAL
Y	17-11-4	-54	-54	—	BACK	VERT	TOTAL
Z	19-11-4	-54	-54	—	BACK	VERT	TOTAL
AA	21-11-4	-54	-54	—	BACK	VERT	TOTAL
AB	23-11-4	-54	-54	—	BACK	VERT	TOTAL
AC	1-11-4	-337	-337	—	BACK	VERT	TOTAL
AD	3-11-4	-337	-337	—	BACK	VERT	TOTAL
AE	5-11-4	-337	-337	—	BACK	VERT	TOTAL
AF	7-11-4	-337	-337	—	BACK	VERT	TOTAL
AG	9-11-4	-337	-337	—	BACK	VERT	TOTAL
AH	11-11-4	-337	-337	—	BACK	VERT	TOTAL
AI	13-11-4	-337	-337	—	BACK	VERT	TOTAL
AJ	17-11-4	-337	-337	—	BACK	VERT	TOTAL
AK	19-11-4	-337	-337	—	BACK	VERT	TOTAL
AL	21-11-4	-337	-337	—	BACK	VERT	TOTAL
AM	23-11-4	-337	-337	—	BACK	VERT	TOTAL
AN	25-11-4	-337	-337	—	BACK	VERT	TOTAL
AO	27-11-4	-337	-337	—	BACK	VERT	TOTAL
AP	29-11-4	-337	-337	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
22218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0575
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T30Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:42 2018 Page 1

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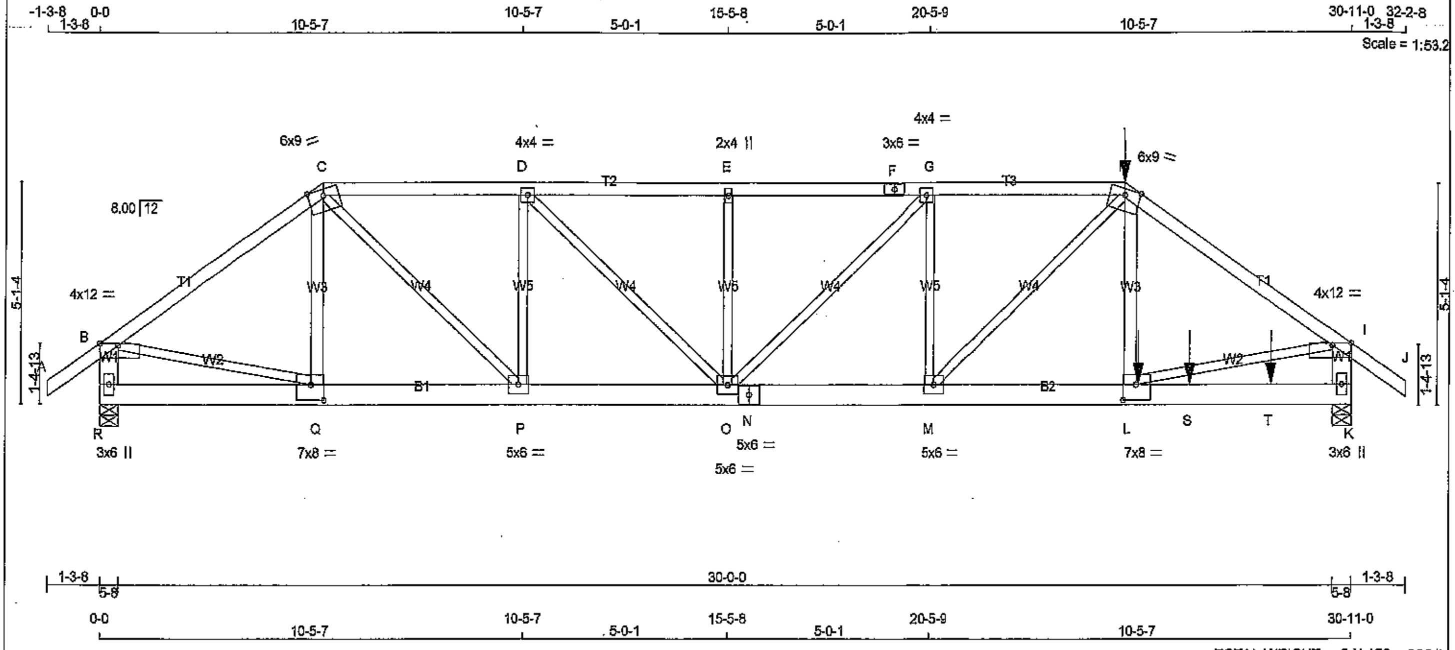

TOTAL WEIGHT = 2 X 150 = 300 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x8	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
L- H	1	3	SIDE(697.0)
2x4	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	12.0	Edge	5.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	4.0	12.0	Edge	5.50
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	7.0	8.0	4.25	3.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	7.0	8.0	4.25	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2948	0	2948	0	0	5-8	5-8
K	6024	0	6024	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2323	1469 / 0	431 / 0	0 / 0	0 / 0	423 / 0	0 / 0
K	4745	3004 / 0	878 / 0	0 / 0	0 / 0	863 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-3447 / 0	-104.9	-104.9	0.42 (1)	4.62
C- D	-4719 / 0	-104.9	-104.9	0.31 (1)	4.17
D- E	-5831 / 0	-104.9	-104.9	0.37 (1)	3.75
E- F	-5831 / 0	-104.9	-104.9	0.37 (1)	3.75
F- G	-5831 / 0	-104.9	-104.9	0.37 (1)	3.75
G- H	-6294 / 0	-104.9	-104.9	0.39 (1)	3.61
H- I	-7360 / 0	-104.9	-104.9	0.76 (1)	3.12
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00
R- B	-2881 / 0	0.0	0.0	0.10 (1)	7.81
K- I	-5670 / 0	0.0	0.0	0.20 (1)	6.18
R- Q	0 / 0	-28.0	-28.0	0.05 (3)	10.00
Q- P	0 / 2876	-28.0	-28.0	0.21 (1)	10.00
P- O	0 / 4719	-28.0	-28.0	0.35 (1)	10.00
O- N	0 / 6294	-28.0	-28.0	0.46 (1)	10.00
N- M	0 / 6294	-28.0	-28.0	0.46 (1)	10.00
M- L	0 / 6046	-28.0	-28.0	0.45 (1)	10.00
L- S	0 / 0	-28.0	-28.0	0.23 (1)	10.00
S- T	0 / 0	-28.0	-28.0	0.23 (1)	10.00
T- K	0 / 0	-28.0	-28.0	0.23 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-366 / 66	0.05 (1)
L- H	0 / 2624	0.25 (1)
B- Q	0 / 2913	0.36 (1)
L- I	0 / 6236	0.77 (1)
M- H	0 / 345	0.04 (1)
C- P	0 / 2557	0.32 (1)
P- D	-1625 / 0	0.30 (1)
M- G	-131 / 115	0.02 (1)
O- G	-837 / 0	0.28 (1)
D- O	0 / 1531	0.19 (1)
O- E	-492 / 0	0.09 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	25-4-6	-527	-527	—	FRONT	VERT	TOTAL
L	25-8-8	-3374	-3374	—	FRONT	VERT	TOTAL
S	26-11-12	-337	-337	—	FRONT	VERT	TOTAL
T	28-11-12	-337	-337	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.76/1.00 (H-I:1) , BC=0.46/1.00 (M-O:1) , WB=0.77/1.00 (I-L:1) , SSI=0.16/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 13576 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T30Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:42 2018 Page 2

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 25-4-6 ON TOP CHORD, AND 3373.8 lbs FACTORED DOWN AT 25-8-8, AND 336.6 lbs FACTORED DOWN AT 26-11-12, AND 336.6 lbs FACTORED DOWN AT 28-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
22716
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/ 5576.18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:42 2018 Page 1

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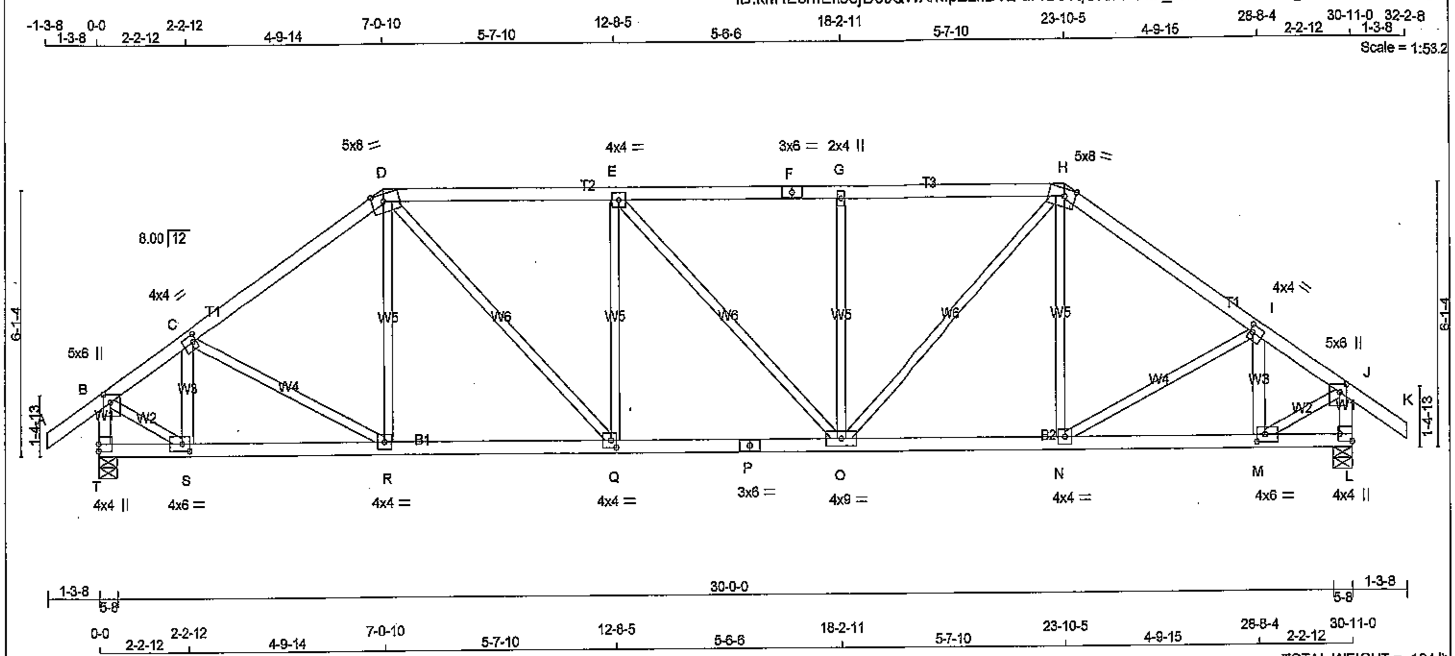

TOTAL WEIGHT = 134 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - I	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW+p	MT20	5.0	6.0	2.25	2.00
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	4.0	6.0	2.00	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	6.0	2.00	2.25
T	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2199	0	2199	0	0	5-8	5-8
L	2199	0	2199	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
L	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2043 / 0	-104.9	-104.9	0.33 (1)	4.36
C- D	-2362 / 0	-104.9	-104.9	0.43 (1)	4.08
D- E	-2710 / 0	-104.9	-104.9	0.63 (1)	3.57
E- F	-2708 / 0	-104.9	-104.9	0.63 (1)	3.58
F- G	-2708 / 0	-104.9	-104.9	0.63 (1)	3.58
G- H	-2708 / 0	-104.9	-104.9	0.63 (1)	3.59
H- I	-2362 / 0	-104.9	-104.9	0.43 (1)	4.08
I- J	-2043 / 0	-104.9	-104.9	0.33 (1)	4.36
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T- B	-2174 / 0	0.0	0.0	0.22 (1)	5.76
L- J	-2174 / 0	0.0	0.0	0.22 (1)	5.76
T- S	0 / 0	-28.0	-28.0	0.08 (3)	10.00
S- R	0 / 1743	-28.0	-28.0	0.38 (1)	10.00
R- Q	0 / 1944	-28.0	-28.0	0.41 (1)	10.00
Q- P	0 / 2710	-28.0	-28.0	0.52 (1)	10.00
P- O	0 / 2710	-28.0	-28.0	0.52 (1)	10.00
O- N	0 / 1944	-28.0	-28.0	0.41 (1)	10.00
N- M	0 / 1743	-28.0	-28.0	0.38 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 211	0.05 (3)
D- Q	0 / 1099	0.25 (1)
Q- E	-634 / 0	0.37 (1)
E- O	-2 / 0	0.00 (1)
O- G	-633 / 0	0.37 (1)
O- H	0 / 1096	0.25 (1)
N- H	0 / 211	0.05 (3)
N- I	0 / 225	0.05 (1)
M- I	-750 / 0	0.09 (1)
M- J	0 / 1937	0.44 (1)
S- C	-749 / 0	0.09 (1)
B- S	0 / 1937	0.44 (1)
C- R	0 / 225	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.52/1.00 (O-Q:1) , WB=0.44/1.00 (B-S:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.77 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM /0577 '18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T31C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:08:42 2018 Page 1

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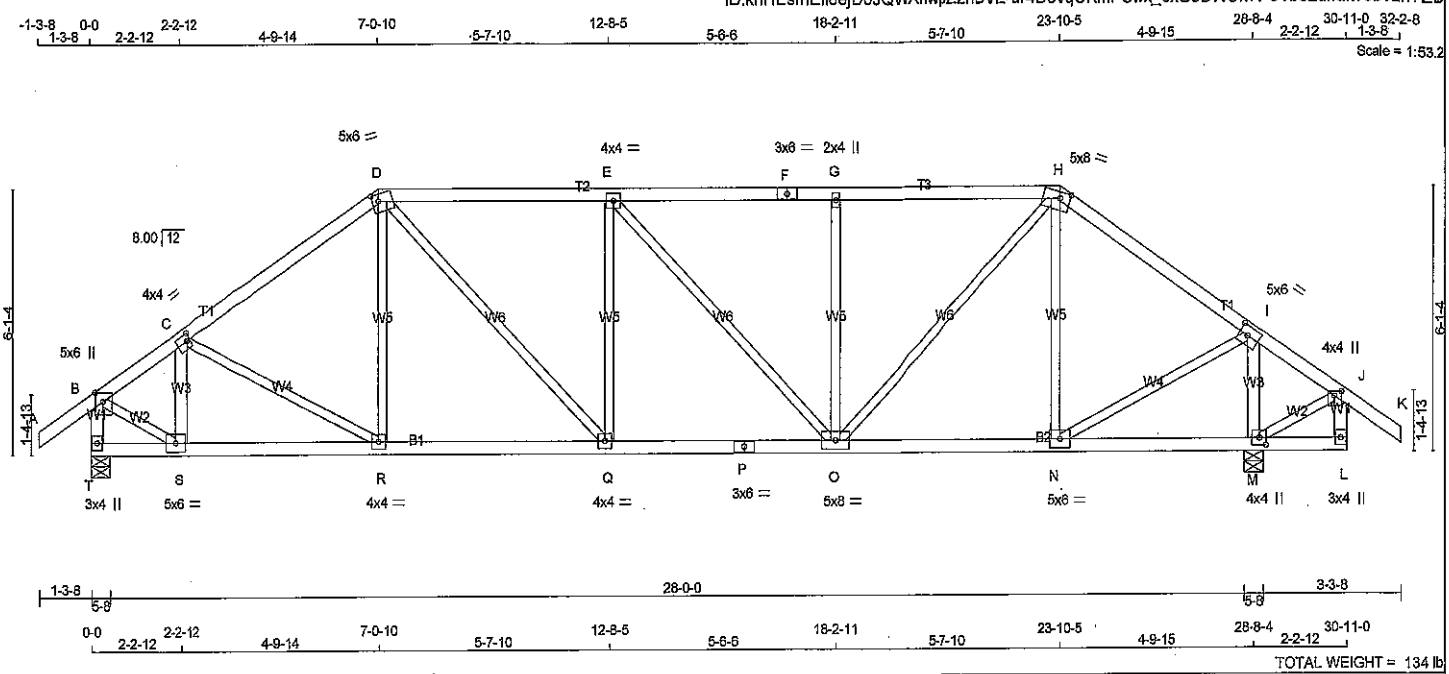

TOTAL WEIGHT = 134 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV+p	MT20	3.0	4.0		
M	BMWW1+t	MT20	4.0	4.0	2.00	1.75
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2028	0	2028	0	0	5-8	5-8
M	2370	0	2370	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1600	1008 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
M	1870	1178 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1866 / 0	-104.9	-104.9	0.33 (1)	4.53
C- D	-2111 / 0	-104.9	-104.9	0.40 (1)	4.29
D- E	-2337 / 0	-104.9	-104.9	0.59 (1)	3.85
E- F	-2173 / 0	-104.9	-104.9	0.57 (1)	3.99
F- G	-2173 / 0	-104.9	-104.9	0.57 (1)	3.99
G- H	-2173 / 0	-104.9	-104.9	0.57 (1)	4.00
H- I	-1531 / 0	-104.9	-104.9	0.40 (1)	4.85
I- J	0 / 220	-104.9	-104.9	0.38 (1)	10.00
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T- B	-2004 / 0	0.0	0.0	0.21 (1)	5.98
L- J	0 / 18	0.0	0.0	0.00 (3)	10.00
T- S	0 / 0	-28.0	-28.0	0.08 (3)	10.00
S- R	0 / 1596	-28.0	-28.0	0.35 (1)	10.00
R- Q	0 / 1736	-28.0	-28.0	0.38 (1)	10.00
Q- P	0 / 2337	-28.0	-28.0	0.46 (1)	10.00
P- O	0 / 2337	-28.0	-28.0	0.46 (1)	10.00
O- N	0 / 1239	-28.0	-28.0	0.30 (2)	10.00
N- M	-133 / 0	-28.0	-28.0	0.16 (3)	6.25
M- L	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 224	0.05 (3)
D- Q	0 / 863	0.19 (1)
Q- E	-465 / 26	0.27 (1)
E- O	-238 / 0	0.32 (1)
O- G	-635 / 0	0.37 (1)
O- H	0 / 1341	0.30 (1)
N- H	-574 / 0	0.34 (1)
N- I	0 / 1558	0.35 (1)
M- I	-2173 / 0	0.27 (1)
M- J	-148 / 0	0.02 (1)
S- C	-677 / 0	0.08 (1)
B- S	0 / 1773	0.40 (1)
C- R	0 / 156	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.46/1.00 (O-Q:1) , WB=0.40/1.00 (B-S:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.68 (P) (INPUT = 1.00)

DWG NO. TAM 105758-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287445	T31S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Tue Feb 27 09:12:53 2018 Page 1

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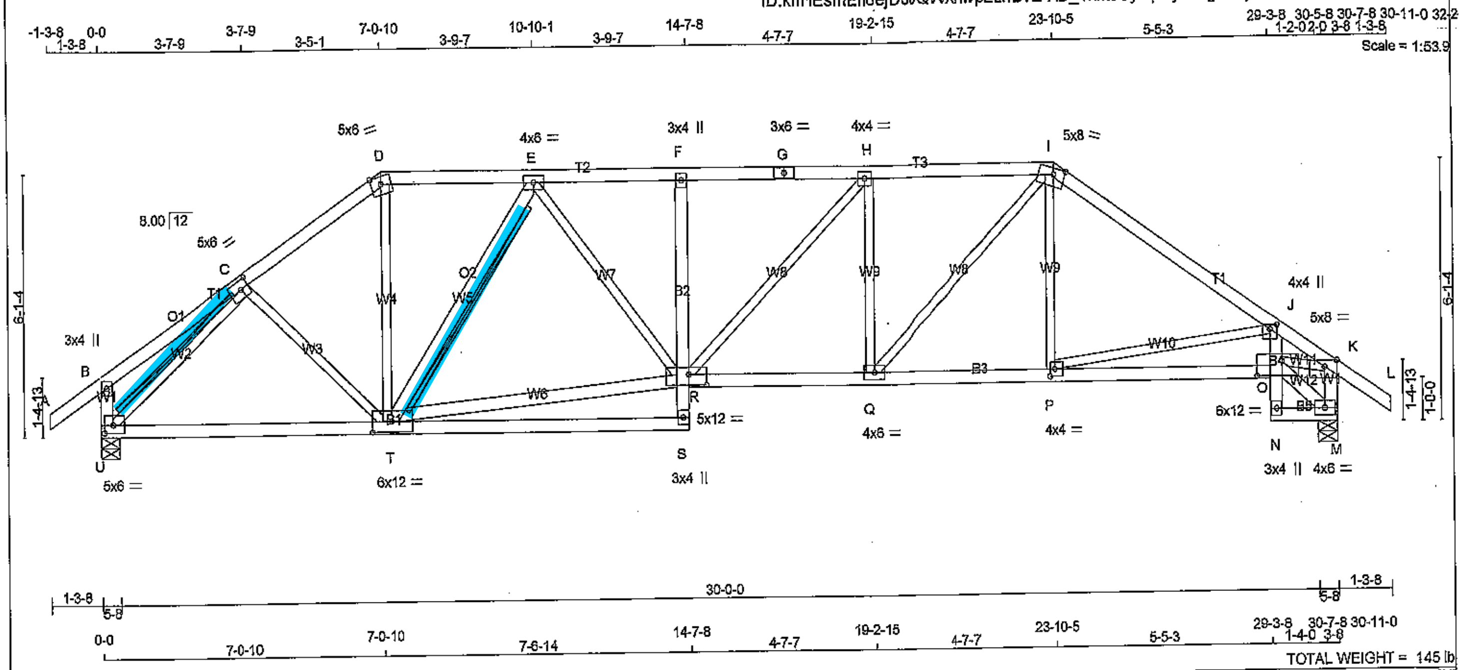

TOTAL WEIGHT = 145 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - E	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.50
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-t	MT20	6.0	12.0	4.00	7.25
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BMWW-t	MT20	4.0	6.0		
R	BVMWWW-t	MT20	5.0	12.0	3.00	5.25
S	BMV+p	MT20	3.0	4.0		
T	BMWWWW*-t	MT20	6.0	12.0	3.00	5.75
U	BMVW1-t	MT20	5.0	6.0	2.25	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2196	0	2196	0	0	5-8	5-8
M	2202	0	2202	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1733	1091 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
M	1737	1095 / 0	325 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-U, E-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A - B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B - C	0 / 20	-104.9	-104.9	0.18 (1)	10.00
C - D	-2372 / 0	-104.9	-104.9	0.28 (1)	4.23
D - E	-1990 / 0	-104.9	-104.9	0.24 (1)	4.55
E - F	-3281 / 0	-104.9	-104.9	0.33 (1)	3.59
F - G	-3301 / 0	-104.9	-104.9	0.53 (1)	3.38
G - H	-3301 / 0	-104.9	-104.9	0.53 (1)	3.38
H - I	-3158 / 0	-104.9	-104.9	0.51 (1)	3.47
I - J	-2823 / 0	-104.9	-104.9	0.73 (1)	3.50
J - K	-3758 / 0	-104.9	-104.9	0.42 (1)	3.22
K - L	0 / 40	-104.9	-104.9	0.14 (1)	10.00
U - B	-293 / 0	0.0	0.0	0.03 (1)	7.81
M - K	-2060 / 0	0.0	0.0	0.21 (1)	5.89
U - T	0 / 1874	-28.0	-28.0	0.62 (2)	10.00
T - S	0 / 96	-28.0	-28.0	0.44 (3)	10.00
S - R	0 / 133	0.0	0.0	0.14 (1)	10.00
R - F	-402 / 0	0.0	0.0	0.18 (1)	7.89
R - Q	0 / 3158	-28.0	-28.0	0.59 (1)	10.00
Q - P	0 / 2335	-28.0	-28.0	0.49 (1)	10.00
P - O	0 / 3361	-28.0	-28.0	0.65 (1)	10.00
N - O	0 / 36	0.0	0.0	0.25 (1)	10.00
O - J	0 / 266	0.0	0.0	0.29 (1)	10.00
N - M	0 / 153	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C - T	0 / 175	0.04 (2)
T - D	0 / 991	0.22 (1)
E - R	0 / 843	0.21 (1)
R - H	0 / 206	0.05 (1)
Q - H	-706 / 0	0.27 (1)
Q - I	0 / 1173	0.26 (1)
P - I	0 / 337	0.08 (2)
P - J	-1048 / 0	0.61 (1)
U - C	-2616 / 0	0.85 (1)
T - E	-1380 / 0	0.77 (1)
T - R	0 / 2642	0.42 (1)
O - M	-195 / 0	0.02 (1)
O - K	0 / 3165	0.71 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.73/1.00 (I-J:1) , BC=0.65/1.00 (O-P:1) , WB=0.85/1.00 (C-U:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.65 (C) (INPUT = 1.00)

PROFESSIONAL ENGINEER 22716 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM/ 0574-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:43 2018 Page 1
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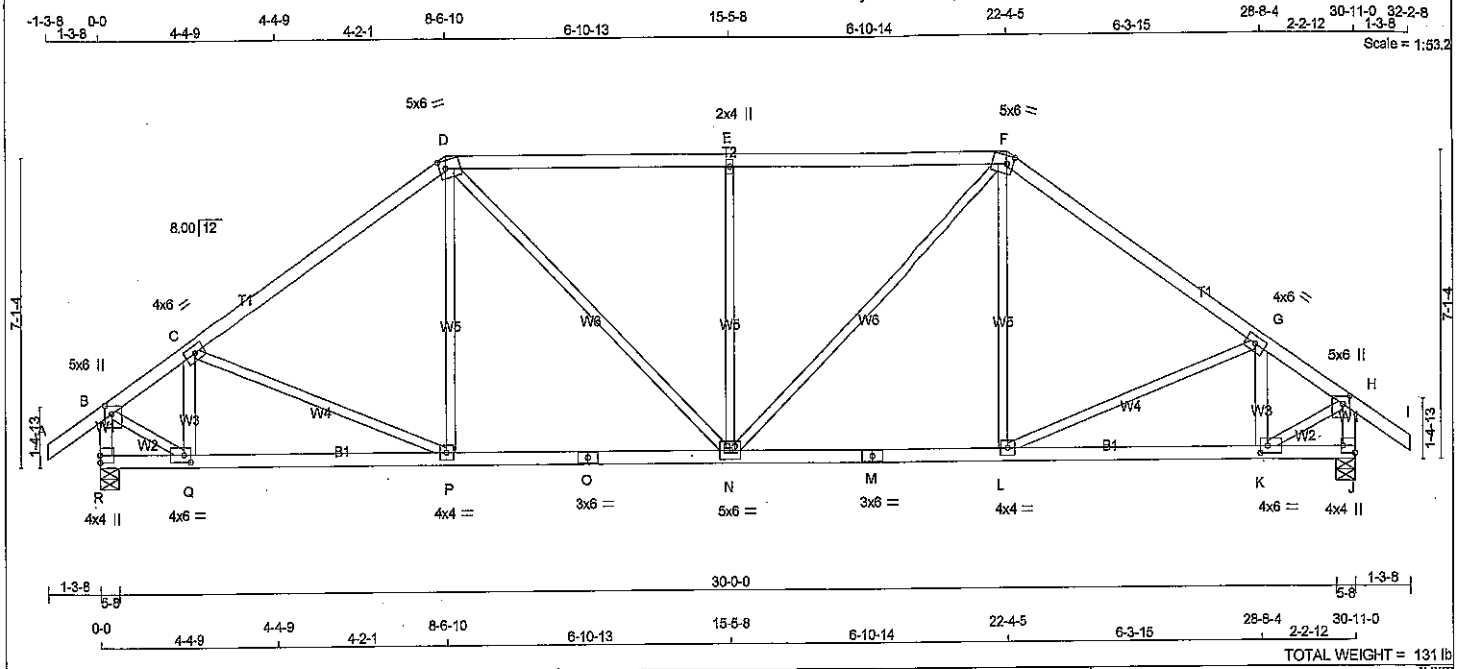

TOTAL WEIGHT = 131 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMWW-t	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0	2.25	2.00
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	4.0	6.0	2.00	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.00
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2199	0	2199	0	0	5-8	5-8
J	2199	0	2199	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
J	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2096 / 0	-104.9	-104.9	0.45 (1)		4.19
C- D	-2303 / 0	-104.9	-104.9	0.64 (1)		3.86
D- E	-2434 / 0	-104.9	-104.9	0.77 (1)		3.44
E- F	-2434 / 0	-104.9	-104.9	0.77 (1)		3.44
F- G	-2303 / 0	-104.9	-104.9	0.64 (1)		3.86
G- H	-2096 / 0	-104.9	-104.9	0.45 (1)		4.19
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
R- B	-2188 / 0	0.0	0.0	0.23 (1)		5.74
J- H	-2188 / 0	0.0	0.0	0.23 (1)		5.74
R- Q	0 / 0	-28.0	-28.0	0.16 (3)		10.00
Q- P	0 / 1823	-28.0	-28.0	0.45 (2)		10.00
P- O	0 / 1889	-28.0	-28.0	0.47 (2)		10.00
O- N	0 / 1889	-28.0	-28.0	0.47 (2)		10.00
N- M	0 / 1889	-28.0	-28.0	0.47 (2)		10.00
M- L	0 / 1889	-28.0	-28.0	0.47 (2)		10.00
L- K	0 / 1823	-28.0	-28.0	0.45 (2)		10.00
K- J	0 / 0	-28.0	-28.0	0.16 (3)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-753 / 0	0.09 (1)
C- P	0 / 78	0.02 (2)
P- D	0 / 311	0.07 (3)
D- N	0 / 765	0.17 (1)
N- E	-890 / 0	0.78 (1)
N- F	0 / 765	0.17 (1)
L- F	0 / 311	0.07 (3)
L- G	0 / 78	0.02 (2)
K- G	-753 / 0	0.09 (1)
K- H	0 / 2026	0.46 (1)
B- Q	0 / 2026	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.77/1.00 (E-F:1) , BC=0.47/1.00 (L-N:2) , WB=0.78/1.00 (E-N:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.80 (O) (INPUT = 1.00)

DWG NO. TAM 105794-A
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T32C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:43 2018 Page 1

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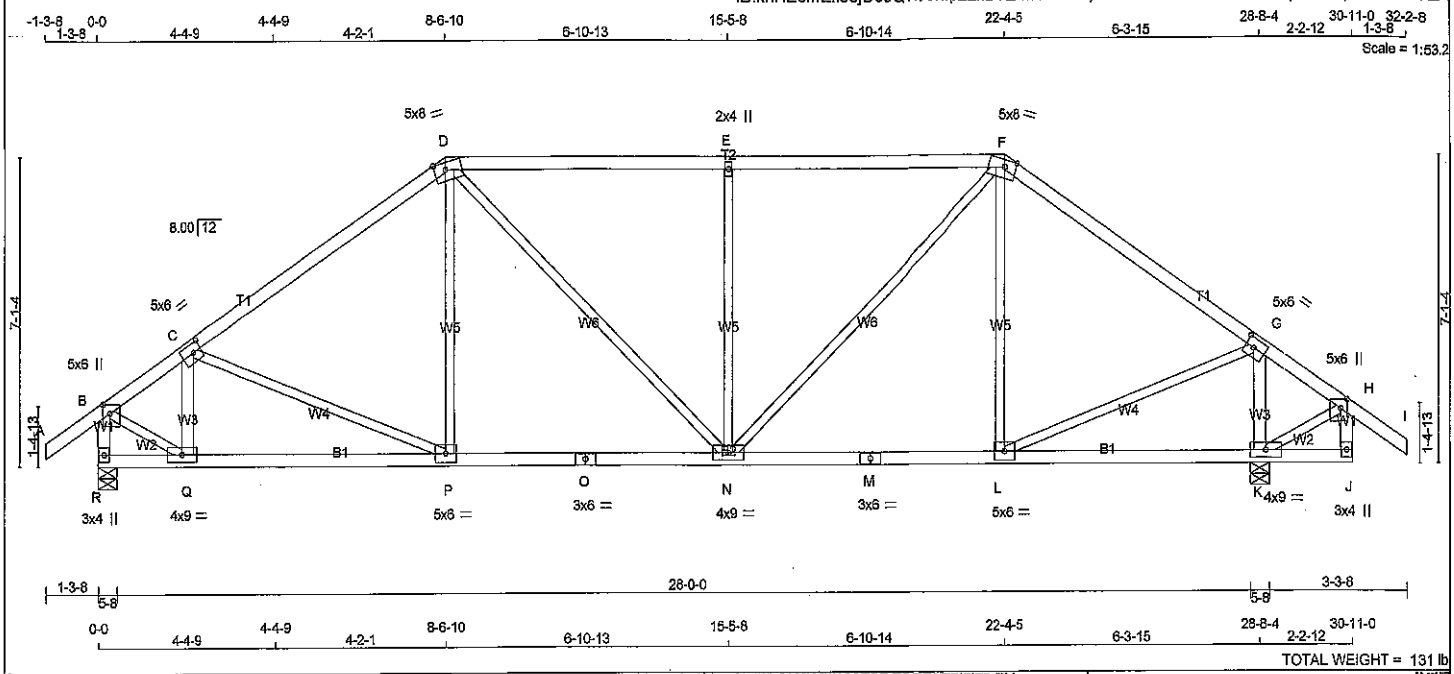

TOTAL WEIGHT = 131 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.25
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	9.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	9.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2028	0	2028	0	0	5-8	5-8
K	2370	0	2370	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1600	1008 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
K	1870	1178 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-1919 / 0	-104.9	-104.9	0.44 (1)		4.36
C- D	-2043 / 0	-104.9	-104.9	0.61 (1)		4.09
D- E	-2047 / 0	-104.9	-104.9	0.73 (1)		3.77
E- F	-2047 / 0	-104.9	-104.9	0.73 (1)		3.77
F- G	-1637 / 0	-104.9	-104.9	0.56 (1)		4.51
G- H	0 / 169	-104.9	-104.9	0.59 (1)		10.00
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
R- B	-2018 / 0	0.0	0.0	0.21 (1)		5.84
J- H	-12 / 0	0.0	0.0	0.00 (1)		7.81
R- Q	0 / 0	-28.0	-28.0	0.16 (3)		10.00
Q- P	0 / 1676	-28.0	-28.0	0.44 (2)		10.00
P- O	0 / 1673	-28.0	-28.0	0.44 (2)		10.00
O- N	0 / 1673	-28.0	-28.0	0.44 (2)		10.00
N- M	0 / 1325	-28.0	-28.0	0.40 (2)		10.00
M- L	0 / 1325	-28.0	-28.0	0.40 (2)		10.00
L- K	-55 / 2	-28.0	-28.0	0.25 (3)		6.25
K- J	0 / 0	-28.0	-28.0	0.19 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
Q- C	-680 / 0	0.08 (1)
C- P	-6 / 40	0.01 (3)
P- D	0 / 327	0.07 (2)
D- N	0 / 524	0.12 (1)
N- E	-892 / 0	0.78 (1)
N- F	0 / 1014	0.23 (1)
L- F	-366 / 94	0.32 (1)
L- G	0 / 1491	0.34 (1)
K- G	-2180 / 0	0.27 (1)
K- H	-61 / 2	0.01 (1)
B- Q	0 / 1862	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.44/1.00 (N-P:2) , WB=0.78/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/05580-RB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:08:43 2018 Page 1

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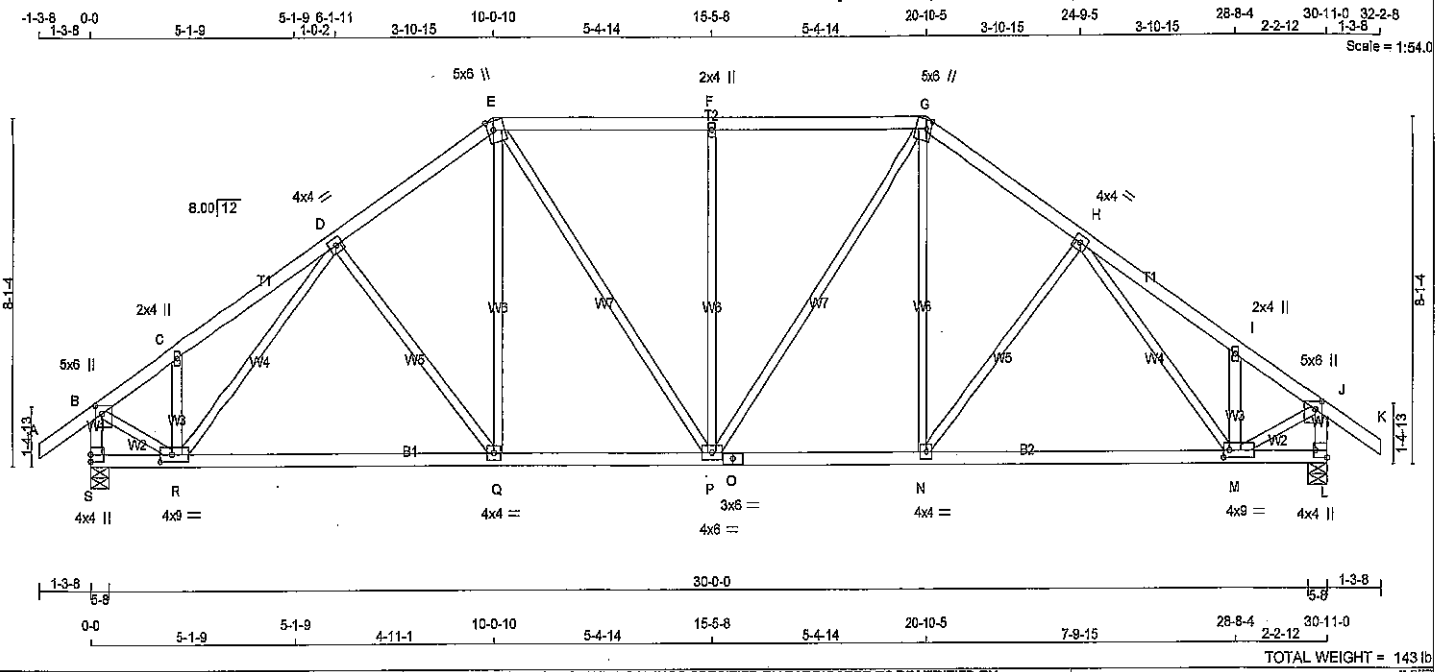

TOTAL WEIGHT = 143 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B C, F, I	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TMWW-t	MT20	4.0	4.0		
J	TMVW+p	MT20	5.0	6.0	2.25	2.00
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWWW-t	MT20	4.0	9.0	2.00	1.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	4.0	9.0	2.00	3.75
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2199	0	2199	0	0	5-8	5-8
L	2199	0	2199	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
L	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2061 / 0	-104.9	-104.9	0.13 (1)	4.63
C- D	-2055 / 0	-104.9	-104.9	0.24 (1)	4.53
D- E	-2192 / 0	-104.9	-104.9	0.24 (1)	4.40
E- F	-2083 / 0	-104.9	-104.9	0.45 (1)	4.23
F- G	-2083 / 0	-104.9	-104.9	0.45 (1)	4.23
G- H	-2191 / 0	-104.9	-104.9	0.24 (1)	4.40
H- I	-2055 / 0	-104.9	-104.9	0.24 (1)	4.53
I- J	-2061 / 0	-104.9	-104.9	0.13 (1)	4.63
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
S- B	-2214 / 0	0.0	0.0	0.23 (1)	5.71
L- J	-2214 / 0	0.0	0.0	0.23 (1)	5.71
S- R	0 / 0	-28.0	-28.0	0.29 (3)	10.00
R- Q	0 / 1967	-28.0	-28.0	0.51 (2)	10.00
Q- P	0 / 1806	-28.0	-28.0	0.50 (2)	10.00
P- O	0 / 1806	-28.0	-28.0	0.50 (2)	10.00
O- N	0 / 1806	-28.0	-28.0	0.50 (2)	10.00
N- M	0 / 1967	-28.0	-28.0	0.51 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-328 / 0	0.04 (1)
Q- E	0 / 442	0.10 (2)
E- P	0 / 487	0.11 (1)
P- F	-691 / 0	0.88 (1)
P- G	0 / 487	0.11 (1)
N- G	0 / 442	0.10 (2)
N- H	-273 / 0	0.21 (1)
H- M	-409 / 0	0.31 (1)
M- I	-328 / 0	0.04 (1)
B- R	0 / 1924	0.43 (1)
M- J	0 / 1924	0.43 (1)
R- D	-409 / 0	0.31 (1)
D- Q	-273 / 0	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.45/1.00 (F-G:1) , BC=0.51/1.00 (M-N:2) , WB=0.88/1.00 (F-P:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.45 (O) (INPUT = 1.00)

DWG NO. TAM 1561 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T33C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:44 2018 Page 1

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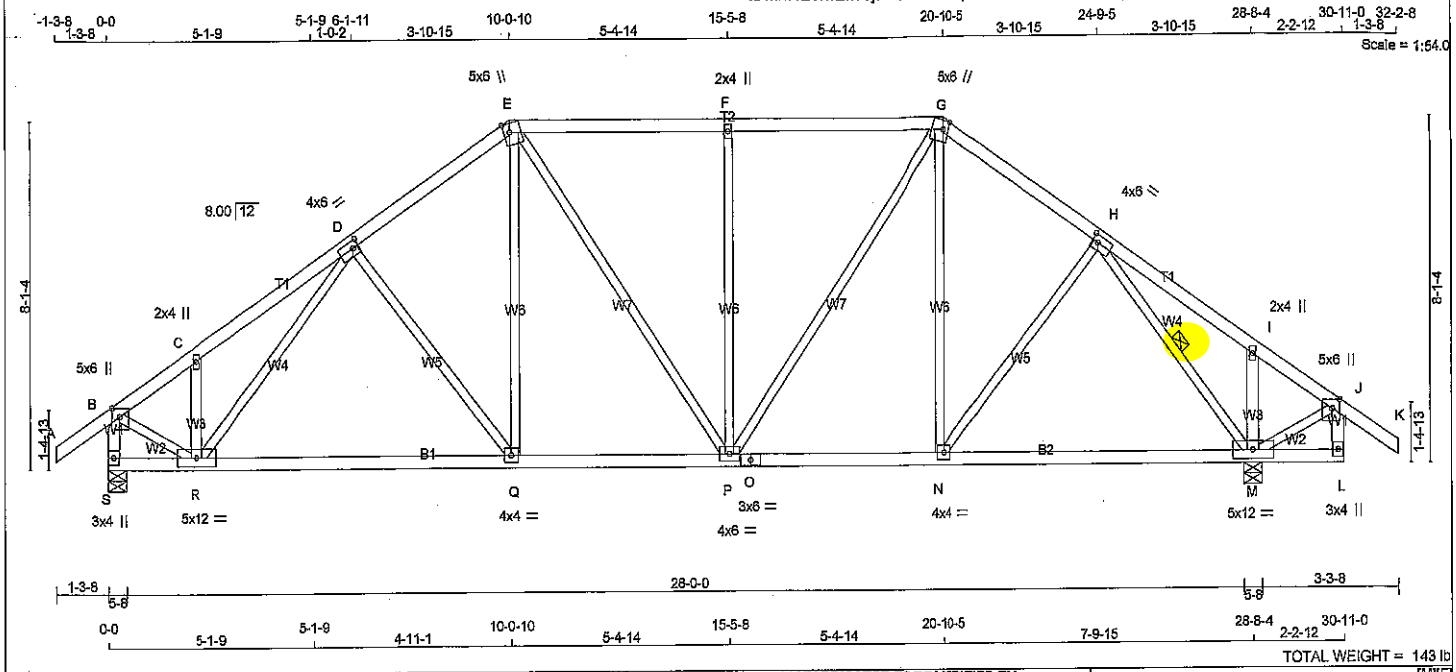

[MII]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C, F, I						
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	6.0	2.00	1.75
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TMWW-t	MT20	4.0	6.0	2.00	1.75
J	TMVW+p	MT20	5.0	6.0	2.50	2.25
L	BMV+p	MT20	3.0	4.0		
M	BMWWW1-t	MT20	5.0	12.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	12.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2028	0	2028	0	0	5-8	5-8
M	2370	0	2370	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1600	1008 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
M	1870	1178 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	R- C	-328 / 0	0.04 (1)
B- C	-1883 / 0	-104.9	-104.9	0.12 (1)	4.81	Q- E	0 / 457	0.10 (2)
C- D	-1878 / 0	-104.9	-104.9	0.23 (1)	4.70	E- P	0 / 282	0.06 (1)
D- E	-1926 / 0	-104.9	-104.9	0.23 (1)	4.65	P- F	-693 / 0	0.88 (1)
E- F	-1745 / 0	-104.9	-104.9	0.43 (1)	4.57	P- G	0 / 700	0.16 (1)
F- G	-1745 / 0	-104.9	-104.9	0.43 (1)	4.57	N- G	-59 / 180	0.08 (1)
G- H	-1644 / 0	-104.9	-104.9	0.21 (1)	4.98	N- H	0 / 324	0.07 (1)
H- I	-43 / 175	-104.9	-104.9	0.23 (1)	6.25	H- M	-2145 / 0	0.62 (1)
I- J	-36 / 197	-104.9	-104.9	0.23 (1)	6.25	M- I	-379 / 0	0.05 (1)
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00	B- R	0 / 1760	0.40 (1)
S- B	-2043 / 0	0.0	0.0	0.21 (1)	5.91	M- J	-148 / 39	0.02 (1)
L- J	-51 / 0	0.0	0.0	0.01 (2)	7.81	R- D	-318 / 8	0.24 (1)
						D- Q	-303 / 0	0.24 (1)
S- R	0 / 0	-28.0	-28.0	0.29 (3)	10.00			
R- Q	0 / 1765	-28.0	-28.0	0.49 (2)	10.00			
Q- P	0 / 1585	-28.0	-28.0	0.47 (2)	10.00			
P- O	0 / 1347	-28.0	-28.0	0.44 (2)	10.00			
O- N	0 / 1347	-28.0	-28.0	0.44 (2)	10.00			
N- M	0 / 1149	-28.0	-28.0	0.44 (2)	10.00			
M- L	0 / 0	-28.0	-28.0	0.32 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.49/1.00 (Q-R:2) , WB=0.88/1.00 (F-P:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.51 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1550268
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:08:44 2018 Page 1

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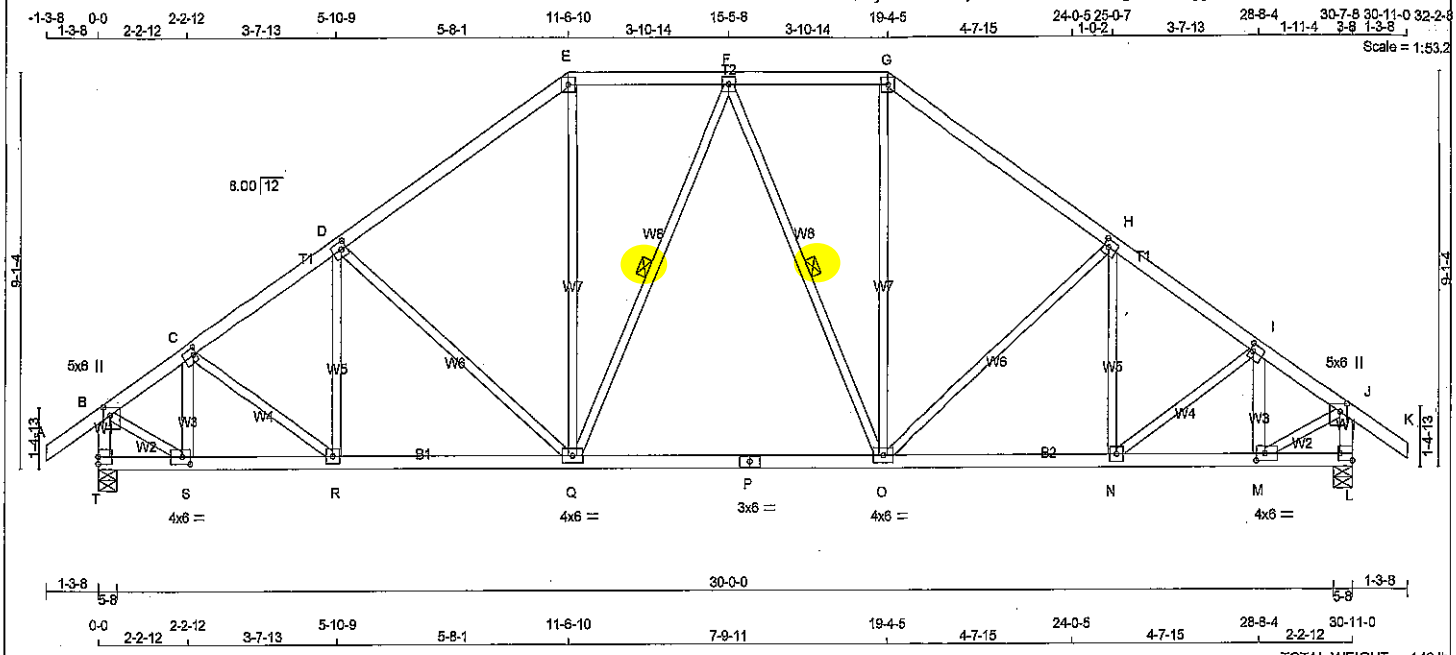

TOTAL WEIGHT = 146 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW-I	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-I	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW+p	MT20	5.0	6.0	2.25	2.00
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	6.0	2.00	2.50
T	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2199	0	2199	0	0	5-8	5-8
L	2199	0	2199	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
L	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1995 / 0	-104.9	-104.9	0.11 (1)	4.71
C- D	-2391 / 0	-104.9	-104.9	0.39 (1)	4.06
D- E	-2075 / 0	-104.9	-104.9	0.48 (1)	4.23
E- F	-1708 / 0	-104.9	-104.9	0.22 (1)	4.88
F- G	-1708 / 0	-104.9	-104.9	0.22 (1)	4.88
G- H	-2075 / 0	-104.9	-104.9	0.48 (1)	4.23
H- I	-2391 / 0	-104.9	-104.9	0.39 (1)	4.06
I- J	-1995 / 0	-104.9	-104.9	0.11 (1)	4.71
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T- B	-2168 / 0	0.0	0.0	0.22 (1)	5.77
L- J	-2168 / 0	0.0	0.0	0.22 (1)	5.77
T- S	0 / 0	-28.0	-28.0	0.04 (3)	10.00
S- R	0 / 1664	-28.0	-28.0	0.32 (1)	10.00
R- Q	0 / 2026	-28.0	-28.0	0.50 (2)	10.00
Q- P	0 / 1813	-28.0	-28.0	0.48 (2)	10.00
P- O	0 / 1813	-28.0	-28.0	0.48 (2)	10.00
O- N	0 / 2026	-28.0	-28.0	0.50 (2)	10.00
N- M	0 / 1664	-28.0	-28.0	0.32 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	-126 / 126	0.05 (1)
D- Q	-432 / 0	0.51 (1)
Q- E	0 / 728	0.16 (1)
O- G	0 / 728	0.16 (1)
S- C	-733 / 0	0.09 (1)
B- S	0 / 1849	0.42 (1)
C- R	0 / 442	0.10 (1)
M- I	-733 / 0	0.09 (1)
M- J	0 / 1849	0.42 (1)
N- H	-126 / 126	0.05 (1)
N- I	0 / 442	0.10 (1)
O- H	-432 / 0	0.51 (1)
Q- F	-266 / 0	0.17 (1)
F- O	-266 / 0	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.48/1.00 (G-H:1) , BC=0.50/1.00 (N-O:2) , WB=0.51/1.00 (H-O:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 0563
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T34C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:45 2018 Page 1
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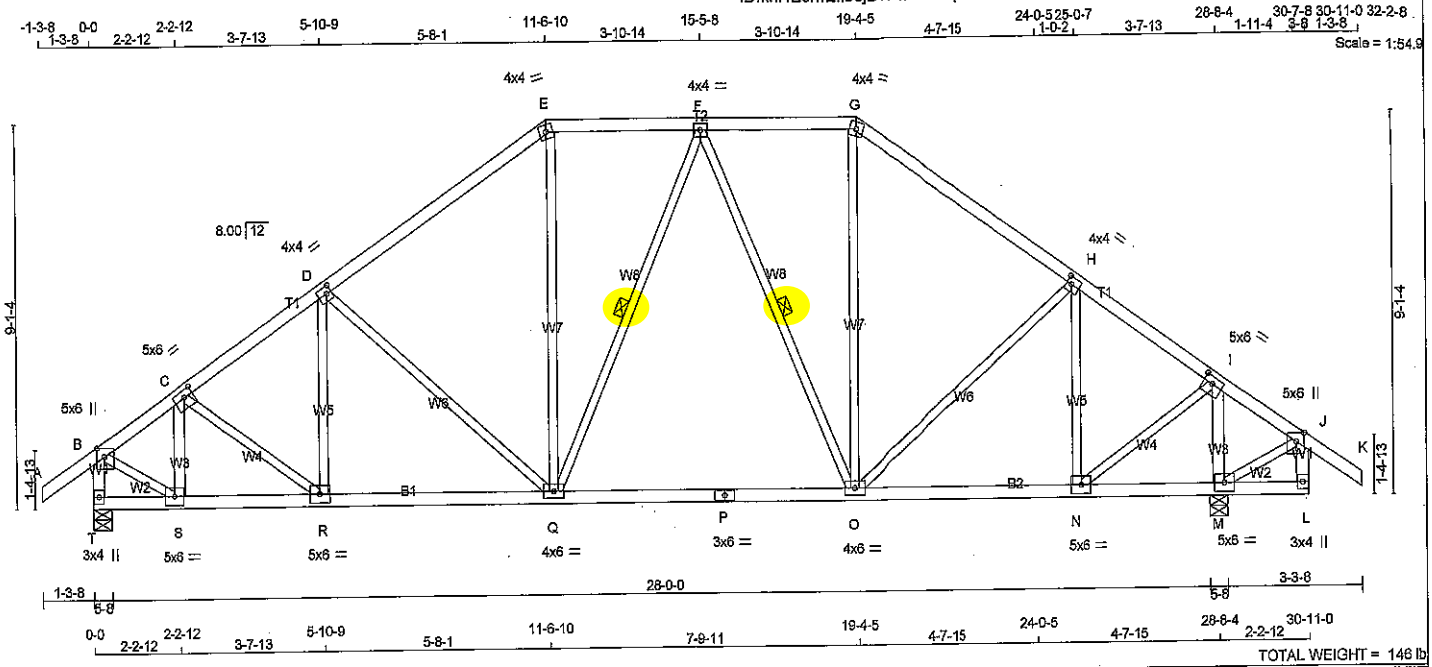

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	5.0	6.0	2.00	2.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-t	MT20	5.0	6.0	2.00	2.75
J	TMWW+p	MT20	5.0	6.0	2.50	2.25
L	BMV+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	5.0	6.0		
N, R, S						
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2028	0	2028	0	0	5-8	5-8
M	2370	0	2370	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1600	1008 / 0	299 / 0	0 / 0	0 / 0	293 / 0	0 / 0
M	1870	1178 / 0	350 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-1818 / 0	-104.9	-104.9	0.10 (1)		4.90
C- D	-2150 / 0	-104.9	-104.9	0.38 (1)		4.26
D- E	-1804 / 0	-104.9	-104.9	0.46 (1)		4.50
E- F	-1481 / 0	-104.9	-104.9	0.22 (1)		5.17
F- G	-1334 / 0	-104.9	-104.9	0.21 (1)		5.39
G- H	-1628 / 0	-104.9	-104.9	0.45 (1)		4.70
H- I	-1367 / 0	-104.9	-104.9	0.34 (1)		5.16
I- J	0 / 266	-104.9	-104.9	0.12 (1)		10.00
J- K	0 / 40	-104.9	-104.9	0.14 (1)		10.00
T- B	-1998 / 0	0.0	0.0	0.21 (1)		5.97
L- J	0 / 32	0.0	0.0	0.01 (3)		10.00
T- S	0 / 0	-28.0	-28.0	0.04 (3)		10.00
S- R	0 / 1516	-28.0	-28.0	0.30 (1)		10.00
R- Q	0 / 1825	-28.0	-28.0	0.48 (2)		10.00
Q- P	0 / 1514	-28.0	-28.0	0.45 (2)		10.00
P- O	0 / 1514	-28.0	-28.0	0.45 (2)		10.00
O- N	0 / 1168	-28.0	-28.0	0.41 (2)		10.00
N- M	-209 / 0	-28.0	-28.0	0.08 (3)		6.25
M- L	0 / 0	-28.0	-28.0	0.08 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	-89 / 141	0.04 (1)
D- Q	-464 / 0	0.55 (1)
Q- E	0 / 580	0.13 (1)
E- G	0 / 461	0.11 (1)
S- C	-661 / 0	0.08 (1)
B- S	0 / 1685	0.38 (1)
C- R	0 / 378	0.08 (1)
M- I	-2153 / 0	0.27 (1)
M- J	-233 / 0	0.04 (1)
N- H	-858 / 0	0.36 (1)
N- I	0 / 1684	0.38 (1)
O- H	0 / 211	0.05 (1)
Q- F	-82 / 60	0.05 (1)
F- O	-454 / 0	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.48/1.00 (Q-R:2) , WB=0.55/1.00 (D-Q:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (P) (INPUT = 0.90)
JSI METAL= 0.57 (P) (INPUT = 1.00)

DWG NO. TAM/0384-18
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
27/2/18

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T35	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:45 2018 Page 1

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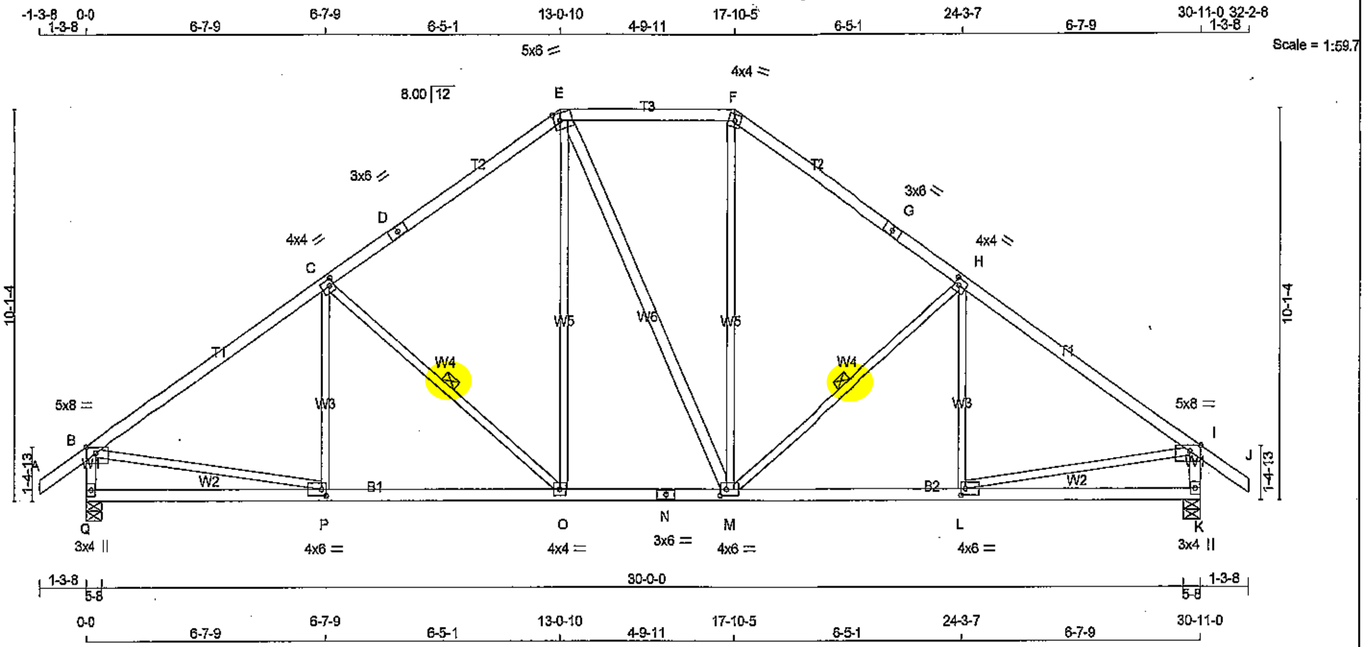

TOTAL WEIGHT = 2 X 142 = 285 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	Edge	3.50
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.50
M	BMWWW-t	MT20	4.0	6.0	2.00	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2199	0	2199	0	0	5-8	5-8
K	2199	0	2199	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0
K	1735	1093 / 0	325 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	P- C	-126 / 232	0.07 (1)
B- C	-2438 / 0	-104.9	-104.9	0.77 (1)	3.55	C- O	-632 / 0	0.31 (1)
C- D	-1951 / 0	-104.9	-104.9	0.69 (1)	4.00	O- E	0 / 553	0.12 (1)
D- E	-1951 / 0	-104.9	-104.9	0.69 (1)	4.00	E- M	0 / 2	0.00 (1)
E- F	-1590 / 0	-104.9	-104.9	0.34 (1)	4.86	M- F	0 / 555	0.12 (1)
F- G	-1952 / 0	-104.9	-104.9	0.69 (1)	4.00	M- H	-630 / 0	0.31 (1)
G- H	-1952 / 0	-104.9	-104.9	0.69 (1)	4.00	L- H	-128 / 231	0.07 (1)
H- I	-2437 / 0	-104.9	-104.9	0.77 (1)	3.55	B- P	0 / 2093	0.47 (1)
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	L- I	0 / 2093	0.47 (1)
Q- B	-2124 / 0	0.0	0.0	0.22 (1)	5.81			
K- I	-2123 / 0	0.0	0.0	0.22 (1)	5.81			
Q- P	0 / 0	-28.0	-28.0	0.31 (3)	10.00			
P- O	0 / 2066	-28.0	-28.0	0.53 (2)	10.00			
O- N	0 / 1589	-28.0	-28.0	0.35 (1)	10.00			
N- M	0 / 1589	-28.0	-28.0	0.35 (1)	10.00			
M- L	0 / 2066	-28.0	-28.0	0.52 (2)	10.00			
L- K	0 / 0	-28.0	-28.0	0.31 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.77/1.00 (B-C:1) , BC=0.53/1.00 (O-P:2) , WB=0.47/1.00 (B-P:1) , SSI=0.28/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22718
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10585 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:45 2018 Page 1
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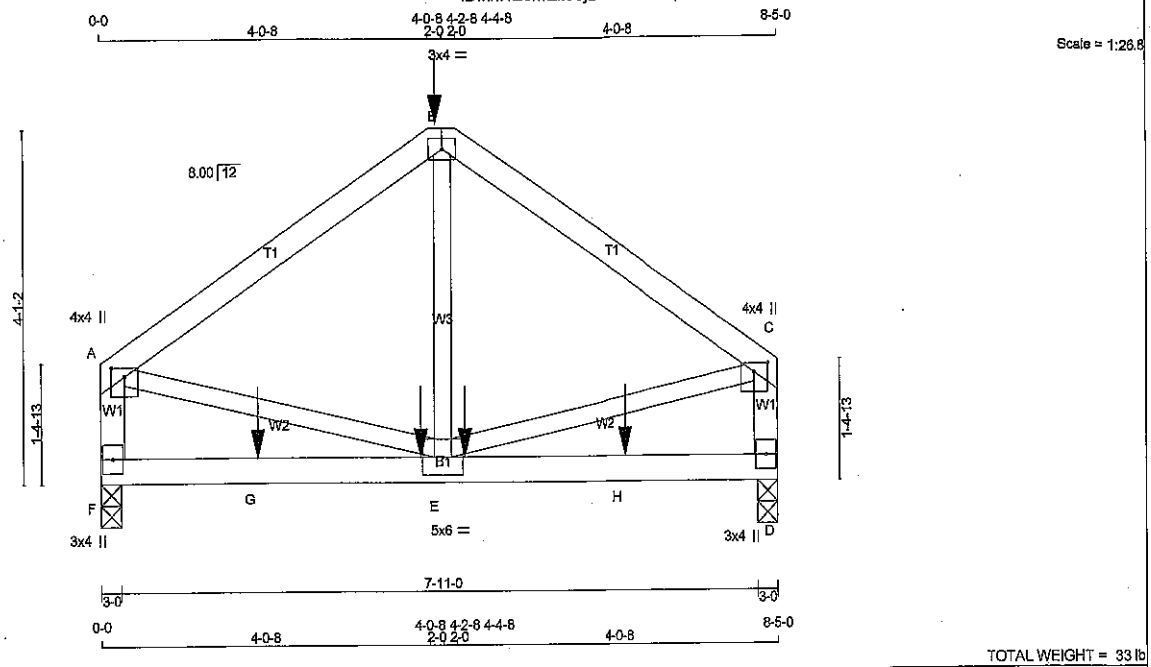

TOTAL WEIGHT = 33 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	3.0	4.0		
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 808.2 lbs FACTORED DOWN AT 4-2-8 ON TOP CHORD, AND 50.0 lbs FACTORED DOWN AT 1-11-4, 50.0 lbs FACTORED DOWN AT 3-11-4, AND 50.0 lbs FACTORED DOWN AT 4-5-12, AND 50.0 lbs FACTORED DOWN AT 6-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1017	0	1017	0	0	3-0	3-0
D	1017	0	1017	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	789	519 / 0	132 / 0	0 / 0	0 / 0	138 / 0	0 / 0
D	789	519 / 0	132 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-962 / 0	-104.9	-104.9	0.38 (1)	5.77	A- E	0 / 827	0.20 (1)	
B- C	-962 / 0	-104.9	-104.9	0.38 (1)	5.77	E- C	0 / 827	0.20 (1)	
F- A	-960 / 0	0.0	0.0	0.11 (1)	7.81	E- B	-182 / 276	0.07 (3)	
D- C	-960 / 0	0.0	0.0	0.11 (1)	7.81				
F- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00				
G- E	0 / 0	-28.0	-28.0	0.21 (3)	10.00				
E- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				
H- D	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
B	4-2-8	-808	-808	—	BACK	VERT	TOTAL
E	3-11-4	-29	-50	—	BACK	VERT	TOTAL
E	4-5-12	-29	-50	—	BACK	VERT	TOTAL
G	1-11-4	-24	-50	—	BACK	VERT	TOTAL
H	6-5-12	-24	-50	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.21/1.00 (E-F:3) , WB=0.20/1.00 (A-E:1) , SSI=0.16/1.00 (B-C:1)

DOL.LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM1-0586-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T38	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 8 2018 MiTek Industries, Inc. Tue Feb 27 09:06:45 2018 Page 1
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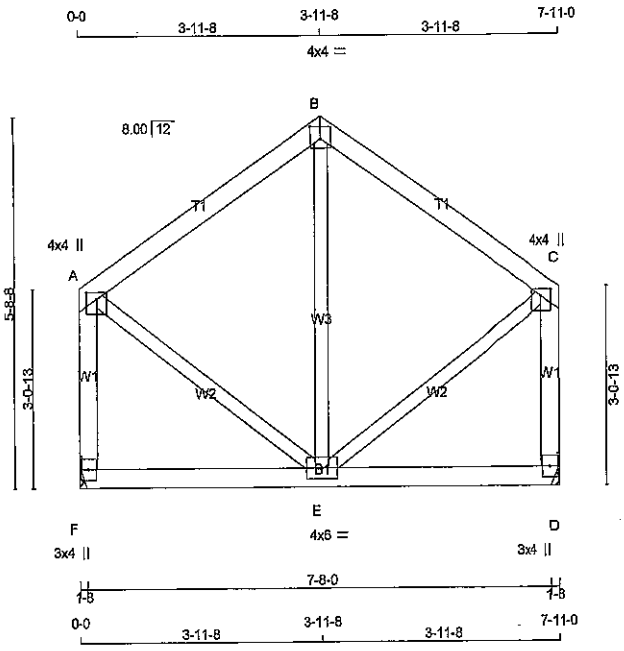

TOTAL WEIGHT = 38 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	526	0	526	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	526	0	526	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	419	257 / 0	83 / 0	0 / 0	0 / 0	79 / 0	0 / 0
D	419	257 / 0	83 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-244 / 0	-104.9	-104.9	0.21 (1)	6.25	E- B	-145 / 103	0.07 (1)
B- C	-244 / 0	-104.9	-104.9	0.21 (1)	6.25	A- E	0 / 247	0.06 (1)
F- A	-483 / 0	0.0	0.0	0.08 (1)	7.81	E- C	0 / 247	0.06 (1)
D- C	-483 / 0	0.0	0.0	0.08 (1)	7.81			
F- E	0 / 0	-28.0	-28.0	0.13 (3)	10.00			
E- D	0 / 0	-28.0	-28.0	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	53.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.13/1.00 (E-F:3) , WB=0.07/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

DWG NO. TAM 10587-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:46 2018 Page 1
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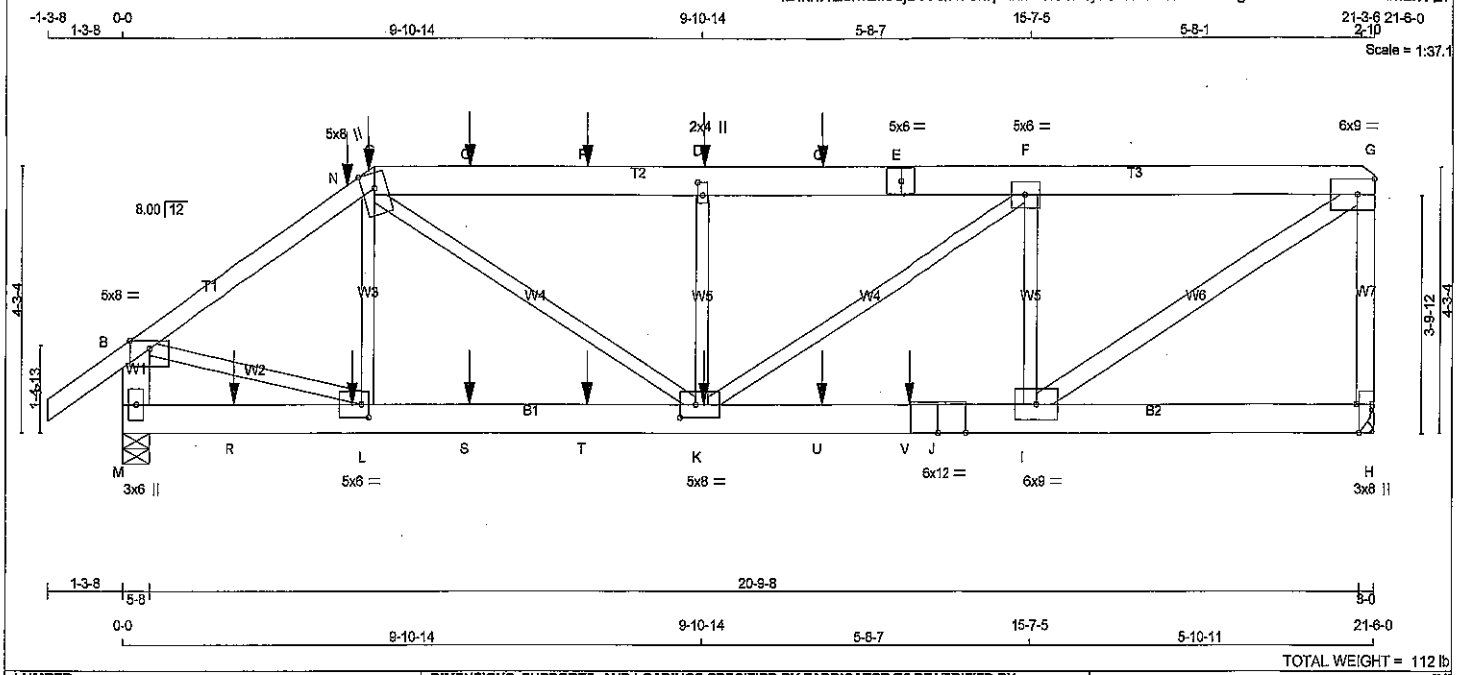

TOTAL WEIGHT = 112 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	6.0	9.0		Edge
H	BMV1+t	MT20	3.0	8.0	Edge	0.50
I	BMWW-t	MT20	6.0	9.0		
J	BS-t	MT20	6.0	12.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	3.25
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED *LOAD(S) 279.8 lbs* FACTORED DOWN AT 4-3-11, 159.5 lbs FACTORED DOWN AT 3-11-4, 126.3 lbs FACTORED DOWN AT 5-11-4, 126.3 lbs FACTORED DOWN AT 7-11-4, AND 126.3 lbs FACTORED DOWN AT 9-11-4, AND 126.3 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 71.3 lbs FACTORED DOWN AT 1-11-4, 71.3 lbs FACTORED DOWN AT 3-11-4, 71.3 lbs FACTORED DOWN AT 5-11-4, 71.3 lbs FACTORED DOWN AT 7-11-4, 71.3 lbs FACTORED DOWN AT 9-11-4, AND 71.3 lbs FACTORED DOWN AT 11-11-4, AND 1077.0 lbs FACTORED DOWN AT 13-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2790	0	2790	0	0	5-8	5-8
H	2478	0	2478	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2221	1367 / 0	436 / 0	0 / 0	0 / 0	417 / 0	0 / 0
H	1966	1220 / 0	380 / 0	0 / 0	0 / 0	366 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	L- C	-448 / 151	0.12 (1)
B- N	-3066 / 0	-104.9	-104.9	0.64 (1)	3.30	B- L	0 / 2660	0.66 (1)
N- C	-3066 / 0	-104.9	-104.9	0.64 (1)	3.30	I- G	0 / 4099	0.72 (1)
C- O	-4302 / 0	-104.9	-104.9	0.49 (1)	3.66	C- K	0 / 2062	0.51 (1)
O- P	-4302 / 0	-104.9	-104.9	0.49 (1)	3.66	I- F	-1270 / 0	0.32 (1)
P- D	-4302 / 0	-104.9	-104.9	0.49 (1)	3.66	K- D	-1019 / 0	0.26 (1)
D- Q	-4301 / 0	-104.9	-104.9	0.42 (1)	3.78	K- F	0 / 1034	0.26 (1)
Q- E	-4301 / 0	-104.9	-104.9	0.42 (1)	3.78			
E- F	-4301 / 0	-104.9	-104.9	0.42 (1)	3.78			
F- G	-3442 / 0	-104.9	-104.9	0.32 (1)	4.28			
M- B	-2707 / 0	0.0	0.0	0.19 (1)	6.30			
H- G	-2483 / 0	0.0	0.0	0.62 (1)	5.36			
M- R	0 / 0	-28.0	-28.0	0.07 (3)	10.00			
R- L	0 / 0	-28.0	-28.0	0.07 (3)	10.00			
L- S	0 / 2601	-28.0	-28.0	0.41 (1)	10.00			
S- T	0 / 2601	-28.0	-28.0	0.41 (1)	10.00			
T- K	0 / 2601	-28.0	-28.0	0.41 (1)	10.00			
K- U	0 / 3442	-28.0	-28.0	0.85 (1)	10.00			
U- V	0 / 3442	-28.0	-28.0	0.85 (1)	10.00			
V- J	0 / 3442	-28.0	-28.0	0.85 (1)	10.00			
J- I	0 / 3442	-28.0	-28.0	0.85 (1)	10.00			
I- H	0 / 0	-28.0	-28.0	0.26 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-3-11	-280	-280	---	FRONT	VERT	TOTAL
D	9-11-4	-126	-126	---	BACK	VERT	TOTAL
K	9-11-4	-41	-71	---	BACK	VERT	TOTAL
L	3-11-4	-41	-71	---	BACK	VERT	TOTAL
N	3-11-4	-160	-160	---	BACK	VERT	TOTAL
O	5-11-4	-126	-126	---	BACK	VERT	TOTAL
P	7-11-4	-126	-126	---	BACK	VERT	TOTAL
Q	11-11-4	-126	-126	---	BACK	VERT	TOTAL
R	1-11-4	-41	-71	---	BACK	VERT	TOTAL
S	5-11-4	-41	-71	---	BACK	VERT	TOTAL
T	7-11-4	-41	-71	---	BACK	VERT	TOTAL
U	11-11-4	-41	-71	---	BACK	VERT	TOTAL
V	13-5-8	-1077	-1077	---	BACK	VERT	TOTAL

DESIGN CRITERIA

[M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 828 (0.31")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.85/1.00 (I-K:1) , WB=0.72/1.00 (G-I:1) , SSI=0.52/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.88 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 22718 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM10528-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T40	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:46 2018 Page 1

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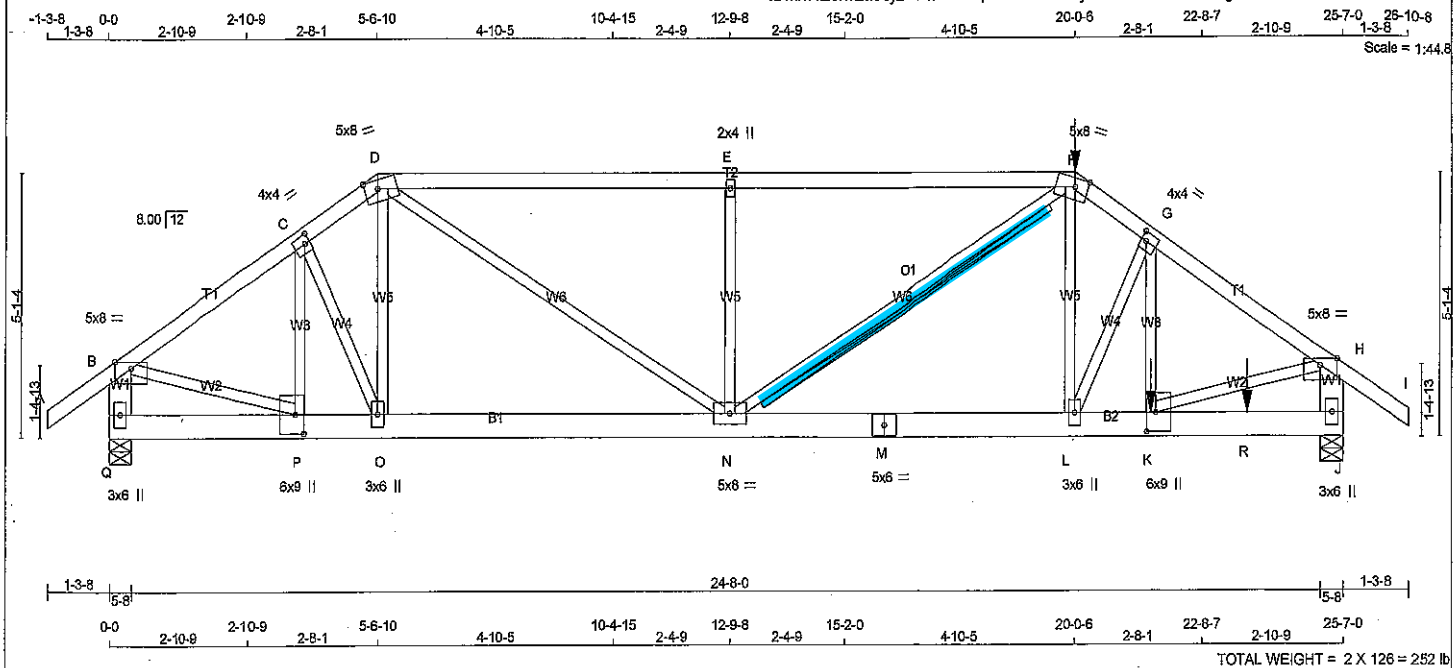

TOTAL WEIGHT = 2 X 126 = 252 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	SIDE(61.0)
F- I	1	12	SIDE(61.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	TOP
M- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- F	1	6	SIDE(14.8)
G- K	1	2	SIDE(613.4)
2x3	1	2	
O- C	1	6	
O- D	1	6	
N- D	1	6	
L- G	1	6	
P- B	1	6	
K- H	1	6	
N- E	1	6	
N- F	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	6.0	9.0	4.50	2.25
L	BMWW+t	MT20	3.0	6.0		
M	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2456	0	2456	0	0	5-8	5-8
J	5147	0	5147	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1934	1225 / 0	358 / 0	0 / 0	0 / 0	352 / 0	0 / 0
J	4056	2565 / 0	752 / 0	0 / 0	0 / 0	739 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	P- C	-634 / 0	0.08 (1)
B- C	-2683 / 0	-104.9	-104.9	0.14 (1)	5.43	C- O	0 / 176	0.02 (1)
C- D	-2848 / 0	-104.9	-104.9	0.12 (1)	5.32	O- D	0 / 192	0.02 (2)
D- E	-4094 / 0	-104.9	-104.9	0.73 (1)	3.92	D- N	0 / 2111	0.26 (1)
E- F	-4094 / 0	-104.9	-104.9	0.73 (1)	3.92	L- F	0 / 2013	0.25 (1)
F- G	-5040 / 0	-104.9	-104.9	0.12 (1)	4.23	L- G	-1763 / 0	0.25 (1)
G- H	-5891 / 0	-104.9	-104.9	0.29 (1)	3.81	K- G	0 / 1720	0.21 (1)
H- I	0 / 40	-104.9	-104.9	0.08 (1)	10.00	B- P	0 / 2318	0.29 (1)
Q- B	-2384 / 0	0.0	0.0	0.09 (1)	7.81	K- H	0 / 5061	0.63 (1)
J- H	-4846 / 0	0.0	0.0	0.17 (1)	6.59	N- E	-924 / 0	0.47 (1)
						N- F	-133 / 0	0.05 (1)
Q- P	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
P- O	0 / 2248	-28.0	-28.0	0.20 (1)	10.00			
O- N	0 / 2325	-28.0	-28.0	0.21 (1)	10.00			
N- M	0 / 4205	-28.0	-28.0	0.33 (1)	10.00			
M- L	0 / 4205	-28.0	-28.0	0.33 (1)	10.00			
L- K	0 / 4908	-28.0	-28.0	0.43 (1)	10.00			
K- R	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
R- J	0 / 0	-28.0	-28.0	0.19 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	20-0-6	-524	-524	—	FRONT	VERT	TOTAL
K	21-7-8	-3053	-3053	—	BACK	VERT	TOTAL
R	23-7-12	-337	-337	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.43/1.00 (K-L:1) , WB=0.63/1.00 (H-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.53 (P) (INPUT = 1.00)

P6 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T40	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	3.0	6.0		
P	BMWW+t	MT20	6.0	9.0	4.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 524.4 lbs FACTORED DOWN AT 20-0-6 ON TOP CHORD, AND 3053.1 lbs FACTORED DOWN AT 21-7-8, AND 336.6 lbs FACTORED DOWN AT 23-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
22718
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM13599-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:46 2018 Page 1

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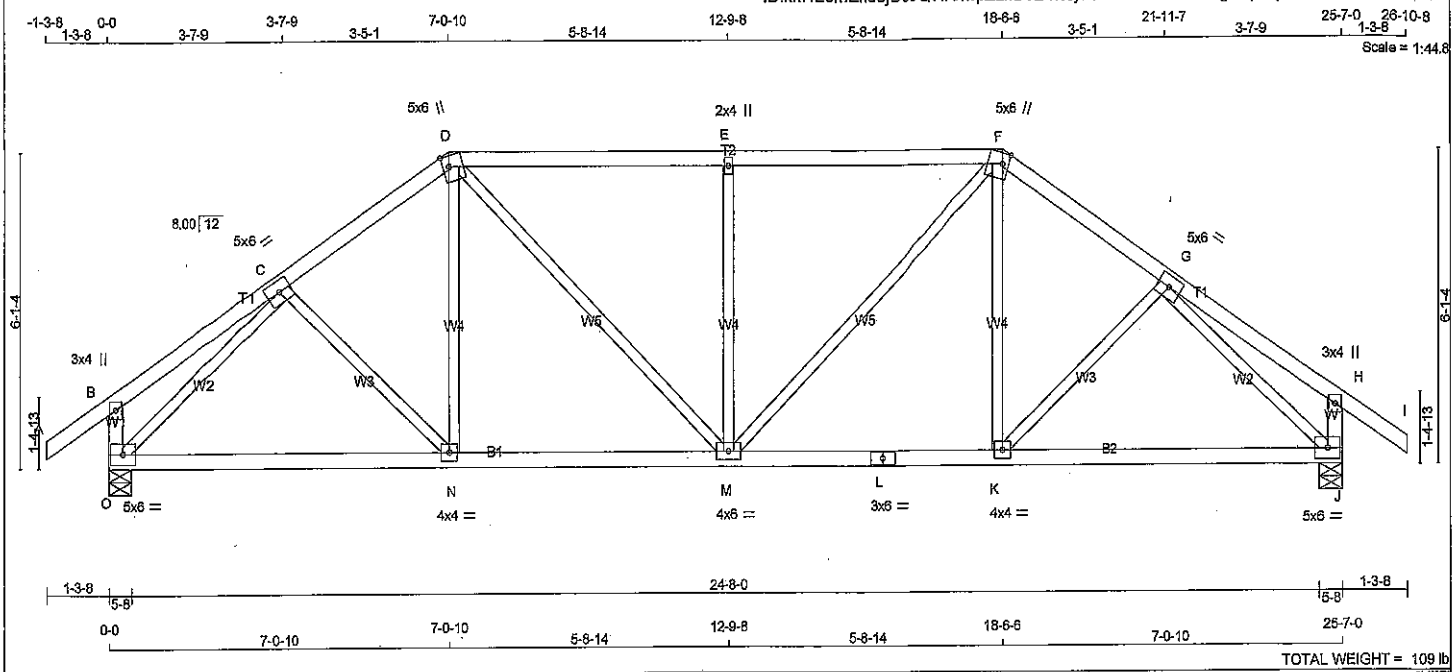

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1844	0	1844	0	0	5-8	5-8
J	1844	0	1844	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 22	-104.9	-104.9	0.19 (1)		10.00
C- D	-1851 / 0	-104.9	-104.9	0.19 (1)		4.77
D- E	-1948 / 0	-104.9	-104.9	0.50 (1)		4.28
E- F	-1948 / 0	-104.9	-104.9	0.50 (1)		4.28
F- G	-1851 / 0	-104.9	-104.9	0.19 (1)		4.77
G- H	0 / 22	-104.9	-104.9	0.19 (1)		10.00
H- I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
O- B	-290 / 0	0.0	0.0	0.03 (1)		7.81
J- H	-290 / 0	0.0	0.0	0.03 (1)		7.81
O- N	0 / 1513	-28.0	-28.0	0.48 (2)		10.00
N- M	0 / 1523	-28.0	-28.0	0.49 (2)		10.00
M- L	0 / 1523	-28.0	-28.0	0.49 (2)		10.00
L- K	0 / 1523	-28.0	-28.0	0.49 (2)		10.00
K- J	0 / 1513	-28.0	-28.0	0.48 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 128	0.03 (3)
N- D	0 / 304	0.07 (2)
D- M	0 / 604	0.14 (1)
M- E	-736 / 0	0.43 (1)
M- F	0 / 604	0.14 (1)
K- F	0 / 304	0.07 (2)
K- G	0 / 128	0.03 (3)
O- C	-2111 / 0	0.81 (1)
G- J	-2111 / 0	0.81 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.49/1.00 (M-N:2) , WB=0.81/1.00 (C-O:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.52 (G) (INPUT = 1.00)

DWG NO. TAM 105908

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:47 2018 Page 1

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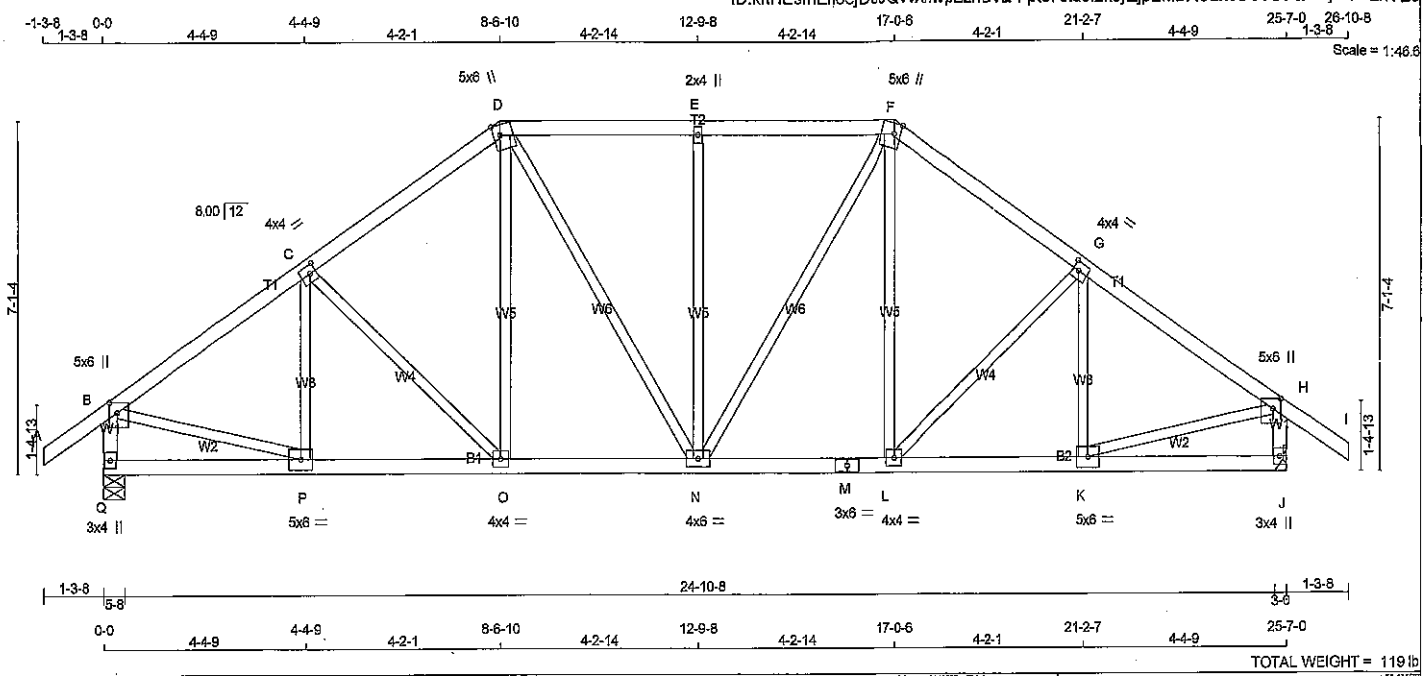

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1844	0	1844	0	0	IN-SX	IN-SX
J	1844	0	1844	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1904 / 0	-104.9	-104.9	0.29 (1)	4.61	
C- D	-1747 / 0	-104.9	-104.9	0.28 (1)	4.78	
D- E	-1633 / 0	-104.9	-104.9	0.26 (1)	4.92	
E- F	-1633 / 0	-104.9	-104.9	0.26 (1)	4.92	
F- G	-1747 / 0	-104.9	-104.9	0.28 (1)	4.78	
G- H	-1904 / 0	-104.9	-104.9	0.29 (1)	4.61	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-1791 / 0	0.0	0.0	0.18 (1)	6.24	
J- H	-1791 / 0	0.0	0.0	0.18 (1)	6.24	
Q- P	0 / 0	-28.0	-28.0	0.12 (3)	10.00	
P- O	0 / 1608	-28.0	-28.0	0.31 (1)	10.00	
O- N	0 / 1431	-28.0	-28.0	0.28 (1)	10.00	
N- M	0 / 1431	-28.0	-28.0	0.28 (1)	10.00	
M- L	0 / 1431	-28.0	-28.0	0.28 (1)	10.00	
L- K	0 / 1608	-28.0	-28.0	0.31 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.12 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
P- C	-268 / 71	0.07 (1)
C- O	-248 / 0	0.15 (1)
O- D	0 / 308	0.07 (2)
D- N	0 / 382	0.09 (1)
N- E	-536 / 0	0.47 (1)
N- F	0 / 382	0.09 (1)
L- F	0 / 308	0.07 (2)
L- G	-248 / 0	0.15 (1)
K- G	-268 / 71	0.07 (1)
B- P	0 / 1656	0.37 (1)
K- H	0 / 1656	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.29/1.00 (G-H:1) , BC=0.31/1.00 (K-L:1) , WB=0.47/1.00 (E-N:1) , SSI=0.21/1.00 (E-F:1)

DGL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (H) (INPUT = 0.90)
JSI METAL= 0.40 (P) (INPUT = 1.00)

DWG NO. TAM/9591-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:47 2018 Page 1

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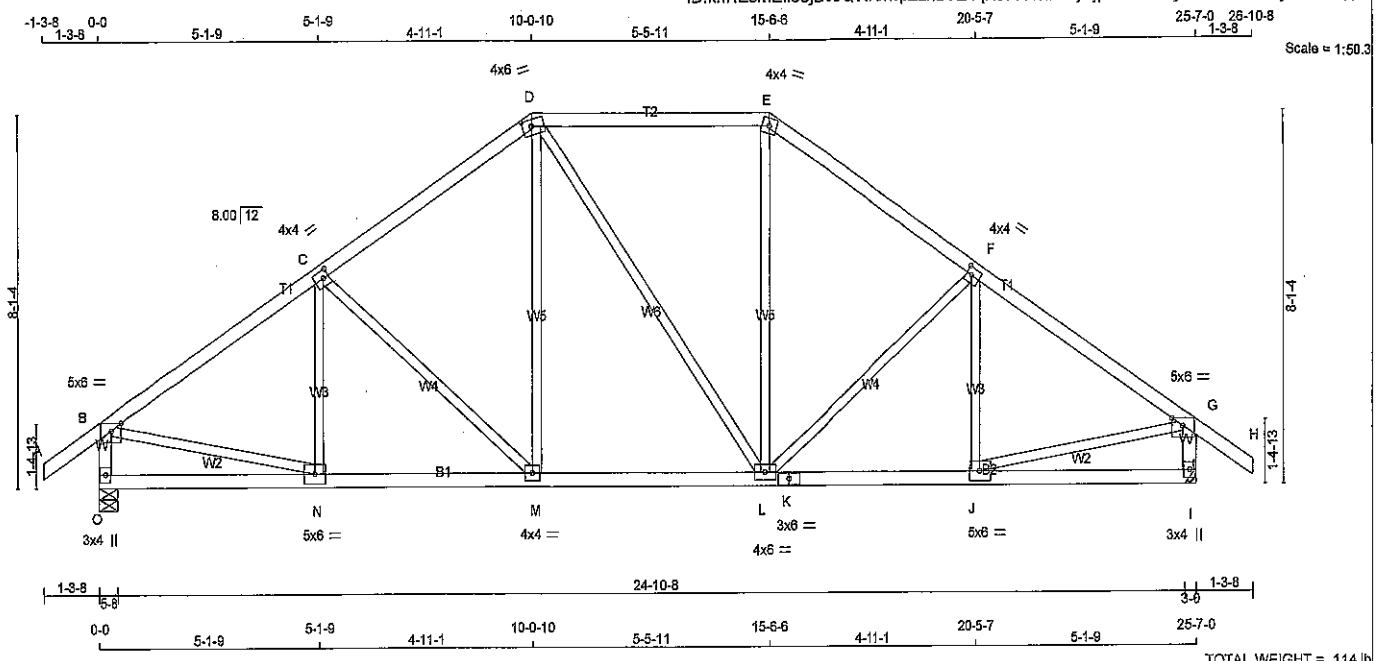

TOTAL WEIGHT = 114 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1844	0	1844	0	0	IN-SX	IN-SX
I	1844	0	1844	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
I	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	N- C	-190 / 132	0.07 (1)
B- C	-1928 / 0	-104.9	-104.9	0.40 (1)	4.46	C- M	-401 / 0	0.34 (1)
C- D	-1641 / 0	-104.9	-104.9	0.38 (1)	4.77	M- D	0 / 417	0.09 (2)
D- E	-1340 / 0	-104.9	-104.9	0.43 (1)	5.07	D- L	0 / 1	0.00 (2)
E- F	-1641 / 0	-104.9	-104.9	0.38 (1)	4.77	L- E	0 / 419	0.09 (2)
F- G	-1928 / 0	-104.9	-104.9	0.40 (1)	4.46	L- F	-400 / 0	0.34 (1)
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00	J- F	-191 / 131	0.07 (1)
O- B	-1784 / 0	0.0	0.0	0.18 (1)	6.26	B- N	0 / 1668	0.38 (1)
I- G	-1784 / 0	0.0	0.0	0.18 (1)	6.26	J- G	0 / 1668	0.38 (1)
O- N	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
N- M	0 / 1633	-28.0	-28.0	0.34 (1)	10.00			
M- L	0 / 1339	-28.0	-28.0	0.30 (1)	10.00			
L- K	0 / 1632	-28.0	-28.0	0.35 (1)	10.00			
K- J	0 / 1632	-28.0	-28.0	0.35 (1)	10.00			
J- I	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	53.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.35/1.00 (J-L:1) , WB=0.38/1.00 (B-N:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.46 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM1089287
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287476	T44	1	1	44755 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:47 2018 Page 1

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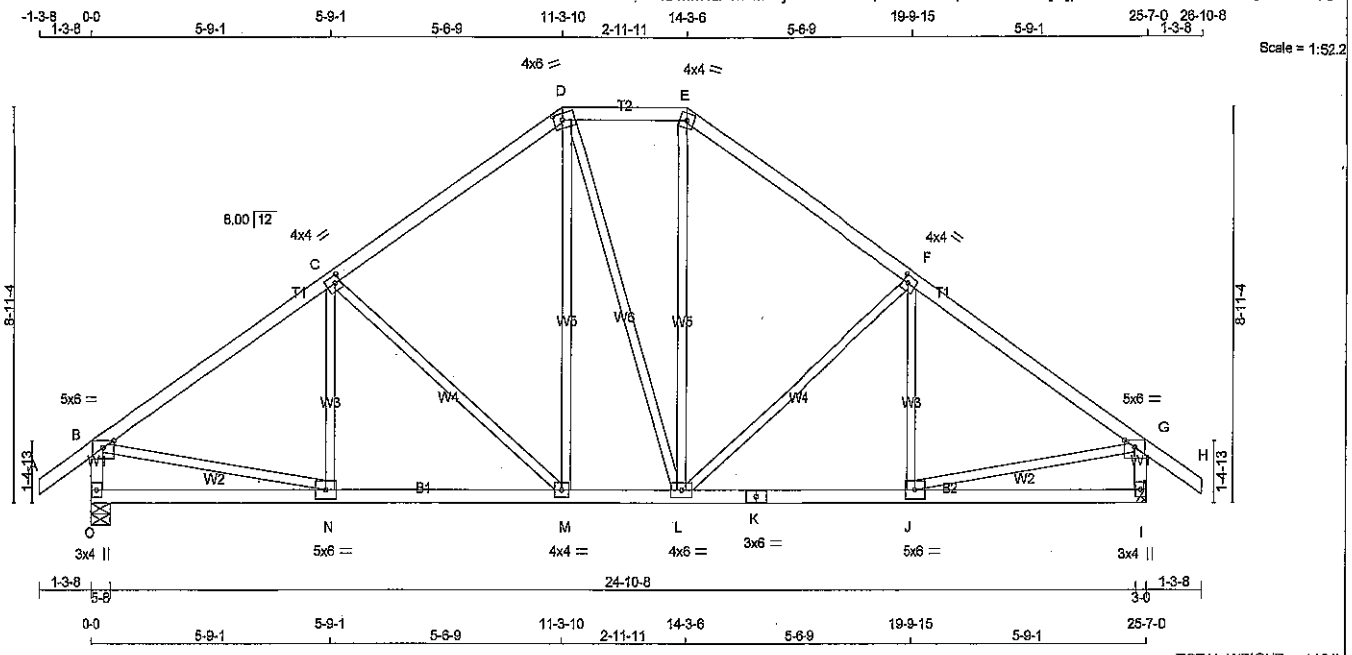

TOTAL WEIGHT = 119 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1844	0	1844	0	0	5-8	5-8
I	1844	0	1844	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0
I	1453	920 / 0	269 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	N- C	-125 / 197	0.05 (1)
B- C	-1935 / 0	-104.9	-104.9	0.51 (1)	4.30	C- M	-530 / 0	0.59 (1)
C- D	-1535 / 0	-104.9	-104.9	0.47 (1)	4.76	M- D	0 / 452	0.10 (1)
D- E	-1249 / 0	-104.9	-104.9	0.14 (1)	5.62	D- L	0 / 5	0.00 (1)
E- F	-1537 / 0	-104.9	-104.9	0.47 (1)	4.75	L- E	0 / 458	0.10 (1)
F- G	-1934 / 0	-104.9	-104.9	0.51 (1)	4.30	L- F	-527 / 0	0.59 (1)
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00	J- F	-128 / 195	0.05 (1)
O- B	-1779 / 0	0.0	0.0	0.18 (1)	6.26	B- N	0 / 1671	0.38 (1)
I- G	-1776 / 0	0.0	0.0	0.18 (1)	6.26	J- G	0 / 1670	0.38 (1)
O- N	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
N- M	0 / 1642	-28.0	-28.0	0.41 (2)	10.00			
M- L	0 / 1248	-28.0	-28.0	0.27 (1)	10.00			
L- K	0 / 1642	-28.0	-28.0	0.40 (2)	10.00			
K- J	0 / 1642	-28.0	-28.0	0.40 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.41/1.00 (M-N:2) , WB=0.59/1.00 (C-M:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.49 (K) (INPUT = 1.00)

DWG NO. TAM/ 543 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	T100	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:17 2018 Page 1
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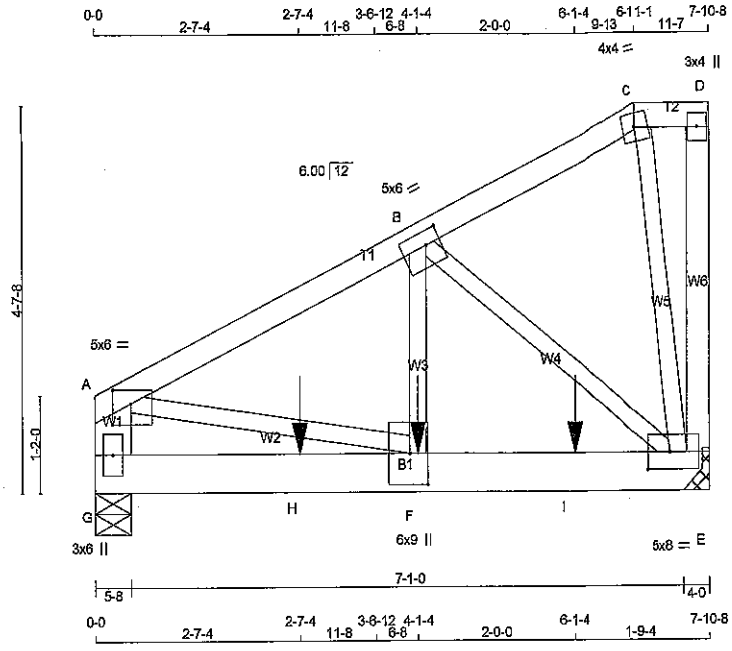

TOTAL WEIGHT = 2 X 41 = 82 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
G- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- F	1	2	SIDE(527.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.00	2.75
B	TMWW-t	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	5.0	8.0	2.50	3.50
F	BMWW+t	MT20	6.0	9.0	4.50	2.75
G	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1512.8 lbs FACTORED DOWN AT 2-7-4, AND 2722.3 lbs FACTORED DOWN AT 4-1-4, AND 1562.9 lbs FACTORED DOWN AT 6-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	3654	0	3654	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 4-0	IN-SX
G	3191	0	3191	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	2851	1852 / 0	497 / 0	0 / 0	0 / 0	502 / 0	0 / 0
G	2492	1614 / 0	437 / 0	0 / 0	0 / 0	441 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4168 / 0	-104.9	-104.9	0.21 (1)	4.49		C- E	-154 / 0	0.02 (1)
B- C	-52 / 0	-104.9	-104.9	0.07 (1)	6.25		A- F	0 / 3802	0.47 (1)
C- D	0 / 0	-104.9	-104.9	0.01 (1)	10.00		F- B	0 / 4211	0.52 (1)
E- D	-50 / 0	0.0	0.0	0.01 (1)	7.81		B- E	-4627 / 0	0.76 (1)
G- A	-2767 / 0	0.0	0.0	0.10 (1)	7.81				
G- H	0 / 0	-28.0	-28.0	0.37 (1)	10.00				
H- F	0 / 0	-28.0	-28.0	0.37 (1)	10.00				
F- I	0 / 3736	-28.0	-28.0	0.67 (1)	10.00				
I- E	0 / 3736	-28.0	-28.0	0.67 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
F	4-1-4	-2722	-2722	—	FRONT	VERT	TOTAL
H	2-7-4	-1513	-1513	—	FRONT	VERT	TOTAL
I	6-1-4	-1563	-1563	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.67/1.00 (E-F:1) , WB=0.76/1.00 (B-E:1) , SSI=0.40/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

DWG NO. TAM 10547-A
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	T101	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:54:32 2018 Page 1

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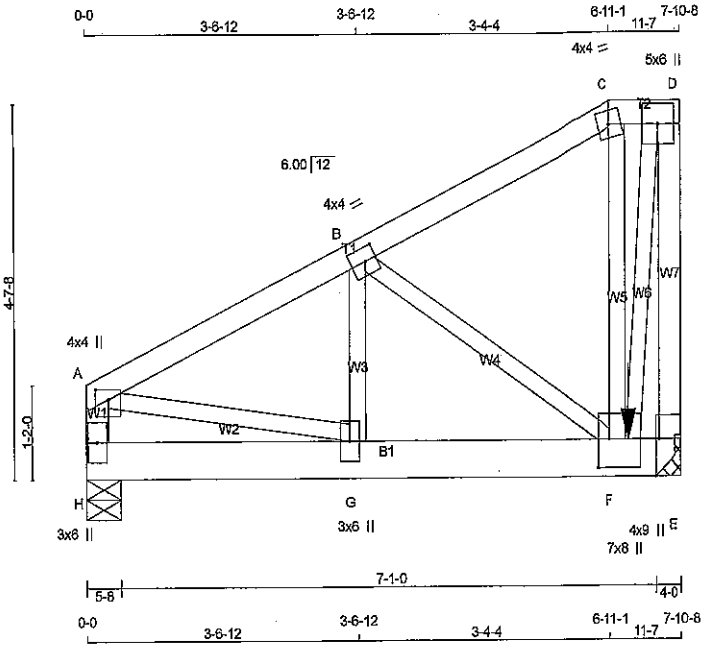

Scale = 1:28.5

TOTAL WEIGHT = 2 X 43 = 86 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
H- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
C- F	1	4	SIDE(513.5)
D- F	1	4	SIDE(512.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	5.0	6.0		
E	BMV1+t	MT20	4.0	9.0	Edge	0.50
F	BMWWW+t	MT20	7.0	8.0	4.25	2.00
G	BMWW+t	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4635.4 lbs FACTORED DOWN AT 7-2-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	4658	0	4658	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
H	1024	0	1024	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	3649	2343 / 0	853 / 0	0 / 0	0 / 0	653 / 0	0 / 0
H	809	509 / 0	152 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1341 / 0	-104.9	-104.9	0.11 (1)	6.25	G- B	0 / 240	0.03 (2)
B- C	-813 / 0	-104.9	-104.9	0.11 (1)	6.25	A- G	0 / 1243	0.15 (1)
C- D	-714 / 0	-104.9	-104.9	0.01 (1)	6.25	F- C	0 / 165	0.02 (1)
E- D	-3611 / 0	0.0	0.0	0.59 (1)	6.18	F- D	0 / 3638	0.45 (1)
H- A	-1020 / 0	0.0	0.0	0.06 (1)	7.81	B- F	-627 / 0	0.10 (1)
H- G	0 / 0	-28.0	-28.0	0.06 (1)	10.00			
G- F	0 / 1214	-28.0	-28.0	0.40 (1)	10.00			
F- E	0 / 0	-28.0	-28.0	0.32 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	7-2-0	-4635	-4635	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	53.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.40/1.00 (F-G:1) , WB=0.45/1.00 (D-F:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.47 (F) (INPUT = 1.00)

DWG NO. TAM 10560-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T102	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:57:34 2018 Page 1

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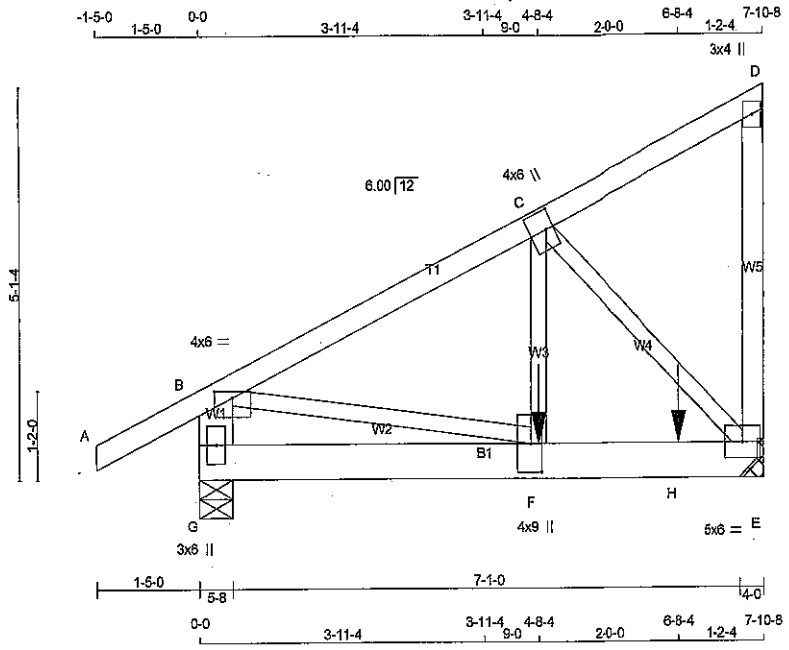

TOTAL WEIGHT = 2 X 41 = 82 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
G- B	2	12	TOP
A- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
C- F	1	3	SIDE(467.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW+t	MT20	4.0	6.0	3.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	5.0	6.0		
F	BMWW+t	MT20	4.0	9.0	4.50	1.75
G	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2491.9 lbs FACTORED DOWN AT 4-8-4, AND 1441.1 lbs FACTORED DOWN AT 6-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1905	0	1905	0	0	5-8	5-8
E	3231	0	3231	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1485	966 / 0	258 / 0	0 / 0	0 / 0	261 / 0	0 / 0
E	2535	1622 / 0	458 / 0	0 / 0	0 / 0	455 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-1858 / 0	0.0	0.0	0.07 (1)		7.81	B- F	0 / 2278	0.28 (1)
A- B	0 / 35	-104.9	-104.9	0.09 (1)		10.00	F- C	0 / 2813	0.35 (1)
B- C	-2498 / 0	-104.9	-104.9	0.19 (1)		5.52	C- E	-3144 / 0	0.50 (1)
C- D	-26 / 0	-104.9	-104.9	0.14 (1)		6.25			
E- D	-110 / 0	0.0	0.0	0.02 (1)		7.81			
G- F	0 / 0	-28.0	-28.0	0.04 (2)		10.00			
F- H	0 / 2248	-28.0	-28.0	0.54 (1)		10.00			
H- E	0 / 2248	-28.0	-28.0	0.54 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	4-8-4	-2492	-2492	---	FRONT	VERT	TOTAL
H	6-8-4	-1441	-1441	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.54/1.00 (E-F:1) , WB=0.50/1.00 (C-E:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.44 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 15562-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T102S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:03:32 2018 Page 1
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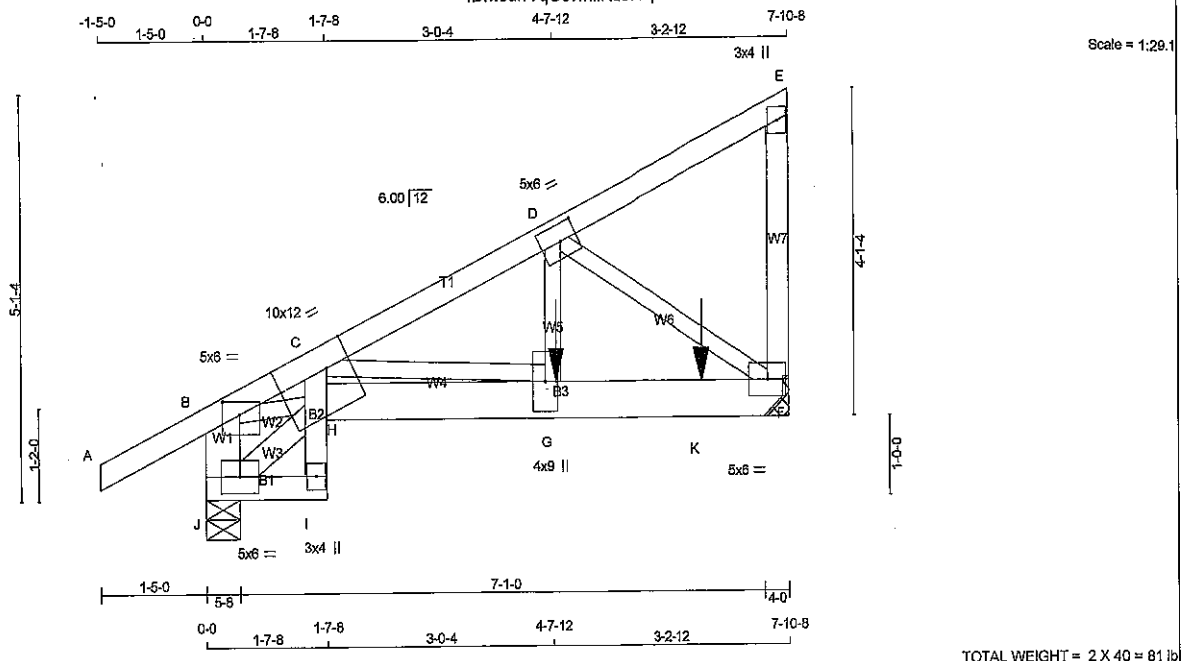

TOTAL WEIGHT = 2 X 40 = 81 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - B	2x6	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J- B	2	12	TOP
A- E	1	12	TOP
E- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- I	1	12	TOP
I- C	1	12	TOP
H- F	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	2	SIDE(362.9)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C						
C	TMBVWWW*-tMT20		10.0	12.0	Edge	3.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	5.0	6.0		
G	BMWW+t	MT20	4.0	9.0		
H						
I	BMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1848	0	1848	0	0	IN-SX	IN-SX
F	3011	0	3011	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1445	934 / 0	255 / 0	0 / 0	0 / 0	256 / 0	0 / 0
F	2392	1480 / 0	465 / 0	0 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-1651 / 0	0.0	0.0	0.06 (1)		7.81	C- G	-334 / 0	0.04 (1)
A- B	0 / 35	-104.9	-104.9	0.09 (1)		10.00	G- D	0 / 3114	0.39 (1)
B- C	-3422 / 0	-104.9	-104.9	0.12 (1)		4.94	D- F	-3684 / 0	0.46 (1)
C- D	-3424 / 0	-104.9	-104.9	0.18 (1)		4.91	J- H	-322 / 0	0.02 (1)
D- E	-15 / 0	-104.9	-104.9	0.08 (1)		6.25	B- H	0 / 3010	0.37 (1)
F- E	-136 / 0	0.0	0.0	0.02 (1)		7.81			
J- I	0 / 269	-28.0	-28.0	0.03 (1)		10.00			
I- H	0 / 36	0.0	0.0	0.20 (1)		10.00			
H- C	-17 / 63	0.0	0.0	0.20 (1)		7.81			
H- G	0 / 3422	-28.0	-28.0	0.24 (1)		10.00			
G- K	0 / 3089	-28.0	-28.0	0.57 (1)		10.00			
K- F	0 / 3089	-28.0	-28.0	0.57 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	4-8-4	-2327	-2327	---	FRONT	VERT	TOTAL
K	6-8-4	-1330	-1330	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.57/1.00 (F-G:1) , WB=0.46/1.00 (D-F:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.44 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
27/16
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM/3568-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T102S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2327.1 lbs FACTORED DOWN AT 4-8-4, AND 1330.3 lbs FACTORED DOWN AT 6-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
22216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 135688
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T103	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:57:34 2018 Page 1

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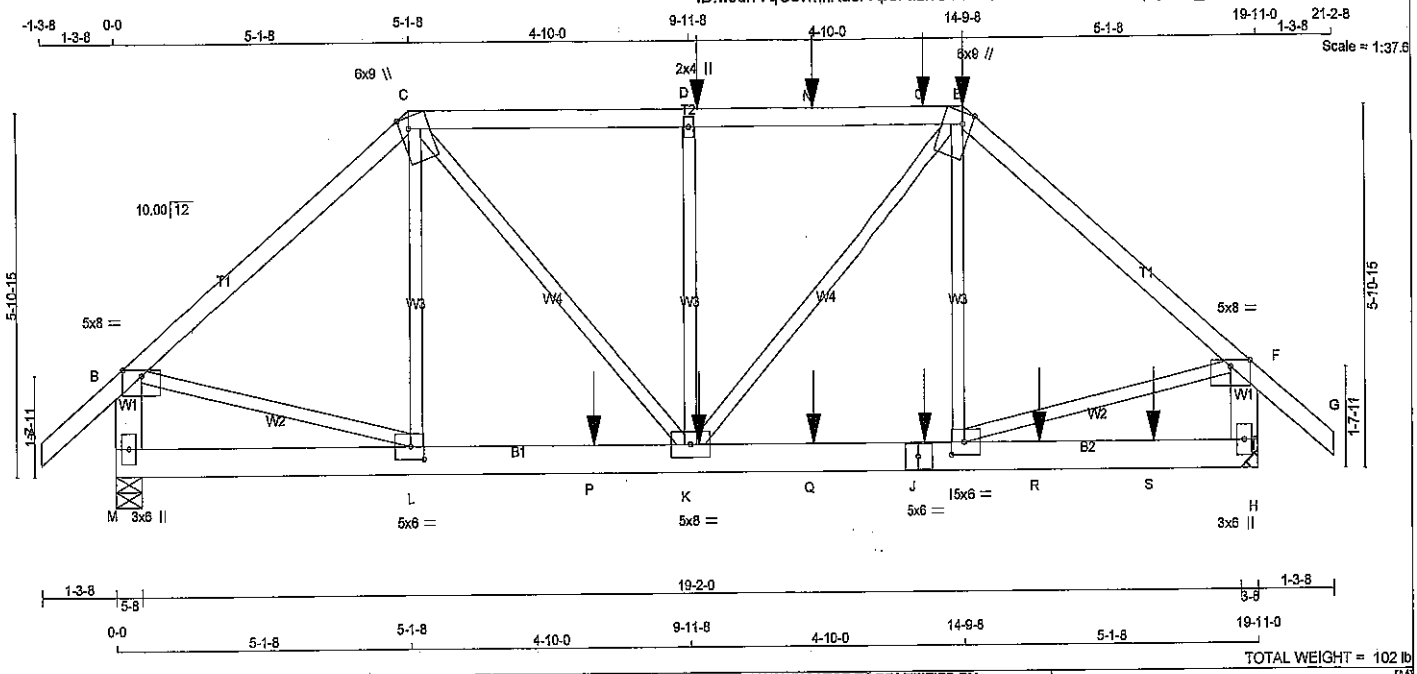

TOTAL WEIGHT = 102 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 137.7 lbs FACTORED DOWN AT 10-1-4, 137.7 lbs FACTORED DOWN AT 12-1-4, AND 154.0 lbs FACTORED DOWN AT 14-1-4, AND 238.4 lbs FACTORED DOWN AT 14-9-8 ON TOP CHORD, AND 1159.0 lbs FACTORED DOWN AT 8-3-8, 84.3 lbs FACTORED DOWN AT 10-1-4, 64.3 lbs FACTORED DOWN AT 12-1-4, 64.3 lbs FACTORED DOWN AT 14-1-4, AND 64.3 lbs FACTORED DOWN AT 16-1-4, AND 64.3 lbs FACTORED DOWN AT 18-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2429	0	2429	0	0	IN-SX 5-8	IN-SX 5-8
H	2520	0	2520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1893	1233 / 0	328 / 0	0 / 0	0 / 0	332 / 0	0 / 0
H	1997	1244 / 0	383 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-2446 / 0	-104.9	-104.9	0.79 (1)	3.53
C- D	-2710 / 0	-104.9	-104.9	0.74 (1)	3.25
D- N	-2710 / 0	-104.9	-104.9	0.74 (1)	3.25
N- O	-2710 / 0	-104.9	-104.9	0.74 (1)	3.25
O- E	-2710 / 0	-104.9	-104.9	0.74 (1)	3.25
E- F	-2438 / 0	-104.9	-104.9	0.79 (1)	3.53
F- G	0 / 47	-104.9	-104.9	0.16 (1)	10.00
M- B	-2423 / 0	0.0	0.0	0.18 (1)	6.59
H- F	-2416 / 0	0.0	0.0	0.18 (1)	6.59
M- L	0 / 0	-28.0	-28.0	0.26 (1)	10.00
L- P	0 / 1878	-28.0	-28.0	0.93 (1)	10.00
P- K	0 / 1878	-28.0	-28.0	0.93 (1)	10.00
K- Q	0 / 1863	-28.0	-28.0	0.55 (1)	10.00
Q- J	0 / 1863	-28.0	-28.0	0.55 (1)	10.00
J- I	0 / 1863	-28.0	-28.0	0.55 (1)	10.00
I- R	0 / 0	-28.0	-28.0	0.15 (2)	10.00
R- S	0 / 0	-28.0	-28.0	0.15 (2)	10.00
S- H	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 242	0.06 (2)
C- K	0 / 1262	0.31 (1)
K- D	-870 / 0	0.45 (1)
K- E	0 / 1284	0.32 (1)
I- E	-291 / 240	0.15 (1)
B- L	0 / 1925	0.48 (1)
I- F	0 / 1919	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	10-1-4	-138	-138	---	BACK	VERT	TOTAL
E	14-9-8	-238	-238	---	FRONT	VERT	TOTAL
J	14-1-4	-37	-64	---	BACK	VERT	TOTAL
K	10-1-4	-37	-64	---	BACK	VERT	TOTAL
N	12-1-4	-138	-138	---	BACK	VERT	TOTAL
O	14-1-4	-154	-154	---	BACK	VERT	TOTAL
P	8-3-8	-1159	-1159	---	BACK	VERT	TOTAL
Q	12-1-4	-37	-64	---	BACK	VERT	TOTAL
R	16-1-4	-37	-64	---	BACK	VERT	TOTAL
S	18-1-4	-37	-64	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.79/1.00 (B-C:1) , BC=0.93/1.00 (K-L:1) , WB=0.48/1.00 (B-L:1) , SSI=0.55/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.47 (I) (INPUT = 1.00)

DWG NO. TAM12563-18

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
2918
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T103S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:03:32 2018 Page 1

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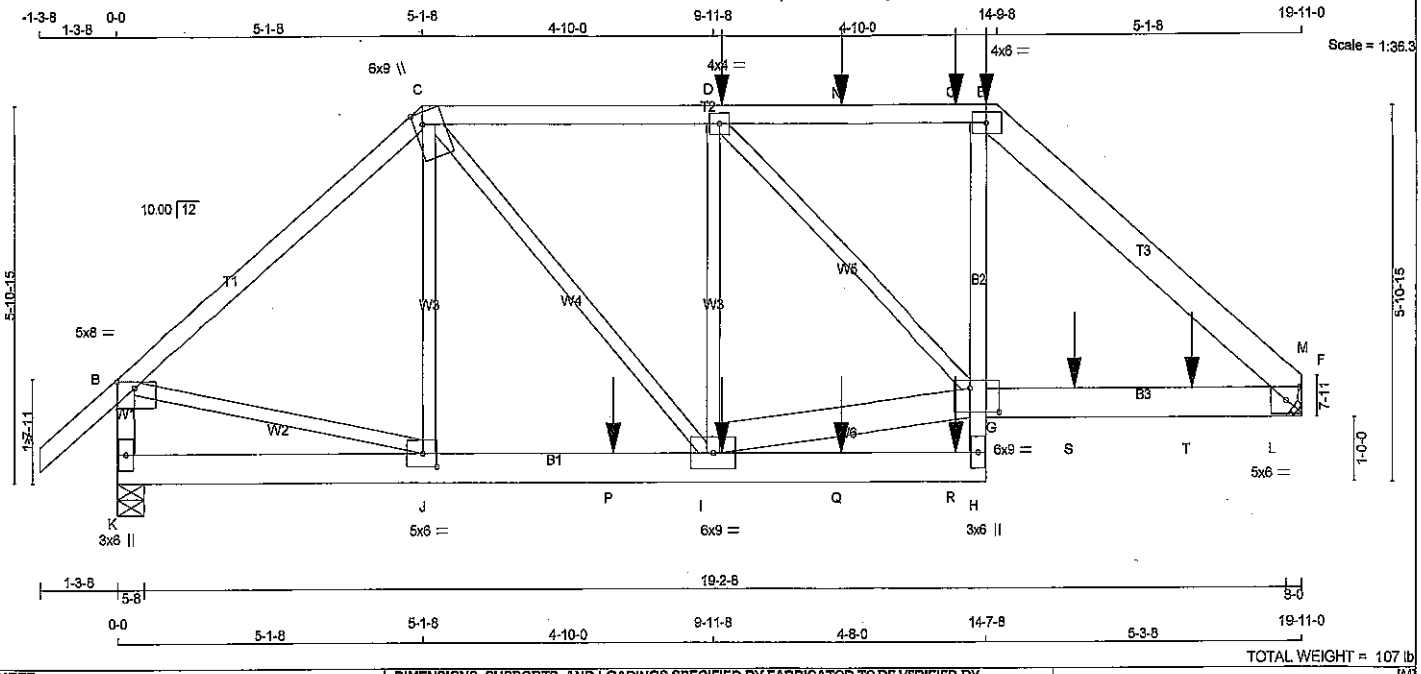

TOTAL WEIGHT = 107 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TTV-I	MT20	4.0	6.0		
F	TMB1-I	MT20	5.0	6.0		
G	BVMWW-I	MT20	6.0	9.0	4.50	6.00
H	BMV+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	6.0	9.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 245.9 lbs FACTORED DOWN AT 14-7-8, 137.7 lbs FACTORED DOWN AT 10-1-4, AND 137.7 lbs FACTORED DOWN AT 12-1-4, AND 162.9 lbs FACTORED DOWN AT 14-1-4 ON TOP CHORD, AND 1159.0 lbs FACTORED DOWN AT 8-3-8, 64.3 lbs FACTORED DOWN AT 10-1-4, 64.3 lbs FACTORED DOWN AT 12-1-4, 79.6 lbs FACTORED DOWN AT 14-1-4, AND 28.1 lbs FACTORED DOWN AT 16-1-4, AND 28.1 lbs FACTORED DOWN AT 18-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	2355	0	2355	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
K	2432	0	2432	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1869	1159 / 0	362 / 0	0 / 0	0 / 0	348 / 0	0 / 0
K	1894	1236 / 0	326 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	1	0.16 (1)	10.00
B- C	-2444 / 0	-104.9	-104.9	1	0.79 (1)	3.53
C- D	-2718 / 0	-104.9	-104.9	1	0.72 (1)	3.28
D- N	-2357 / 0	-104.9	-104.9	1	0.67 (1)	3.56
N- O	-2357 / 0	-104.9	-104.9	1	0.67 (1)	3.56
O- E	-2357 / 0	-104.9	-104.9	1	0.67 (1)	3.56
E- M	-3057 / 0	-104.9	-104.9	1	0.25 (1)	4.58
M- F	-3191 / 0	-104.9	-104.9	1	0.17 (1)	4.57
K- B	-2421 / 0	0.0	0.0	1	0.28 (1)	5.44
K- J	0 / 0	-28.0	-28.0	1	0.24 (1)	10.00
J- P	0 / 1876	-28.0	-28.0	1	0.89 (1)	10.00
P- I	0 / 1876	-28.0	-28.0	1	0.89 (1)	10.00
I- Q	0 / 38	-28.0	-28.0	1	0.34 (1)	10.00
Q- R	0 / 38	-28.0	-28.0	1	0.34 (1)	10.00
R- H	0 / 38	-28.0	-28.0	1	0.34 (1)	10.00
H- G	0 / 163	0.0	0.0	1	0.06 (1)	10.00
G- E	0 / 1095	0.0	0.0	1	0.25 (1)	10.00
G- S	0 / 2330	-28.0	-28.0	1	0.41 (1)	10.00
S- T	0 / 2330	-28.0	-28.0	1	0.41 (1)	10.00
T- L	0 / 2330	-28.0	-28.0	1	0.41 (1)	10.00
L- F	0 / 2330	-28.0	-28.0	1	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 218	0.05 (2)
C- I	0 / 1288	0.32 (1)
I- D	-496 / 36	0.26 (1)
I- G	0 / 2752	0.36 (1)
D- G	-519 / 0	0.37 (1)
B- J	0 / 1924	0.48 (1)
L- M	-56 / 413	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	10-1-4	-138	-138	---	BACK	VERT	TOTAL
E	14-7-8	-246	-246	---	FRONT	VERT	TOTAL
I	10-1-4	-37	-84	---	BACK	VERT	TOTAL
N	12-1-4	-138	-138	---	BACK	VERT	TOTAL
O	14-1-4	-163	-163	---	BACK	VERT	TOTAL
P	8-3-8	-1159	-1159	---	BACK	VERT	TOTAL
Q	12-1-4	-37	-64	---	BACK	VERT	TOTAL
R	14-1-4	-46	-80	---	BACK	VERT	TOTAL
S	16-1-4	-16	-28	---	TOP	VERT	TOTAL
T	18-1-4	-16	-28	---	TOP	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.79/1.00 (8-C:1) , BC=0.89/1.00 (I-J:1) , WB=0.48/1.00 (B-J:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.55 (E) (INPUT = 1.00)

DWG NO. TAM 10569-18
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER 22718 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T104	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:57:35 2018 Page 1
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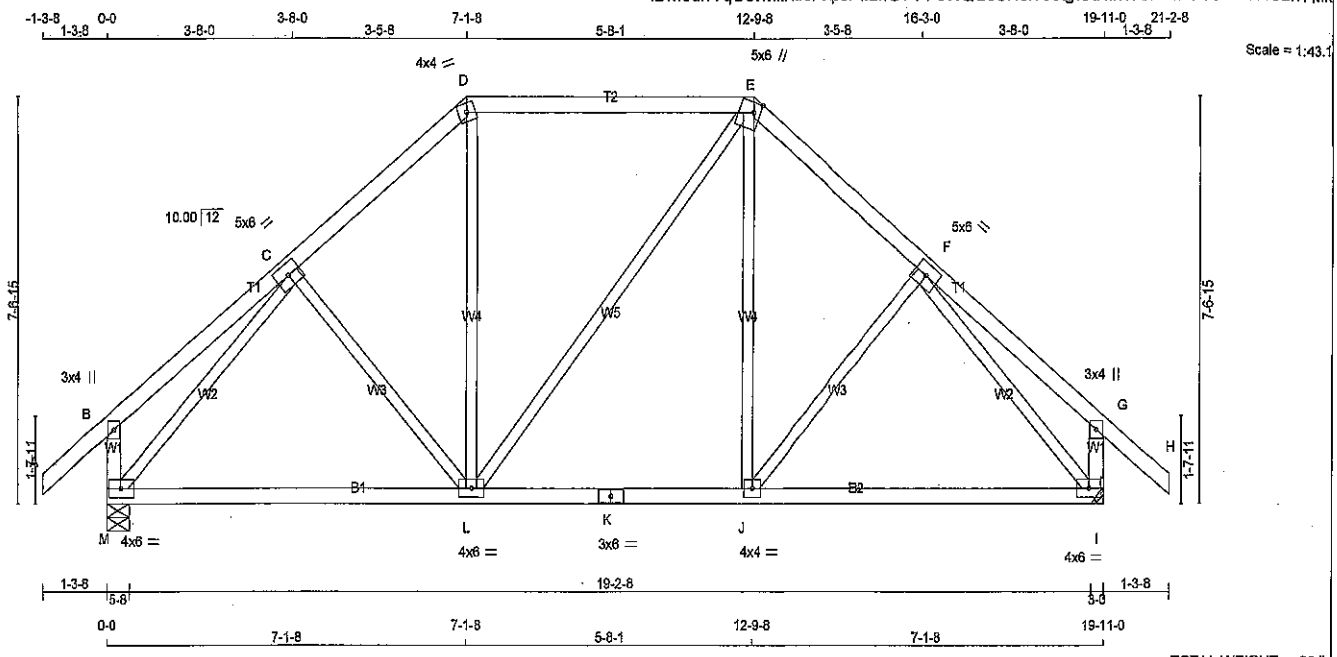

TOTAL WEIGHT = 93 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1469	0	1469	0	0	5-8	5-8
I	1469	0	1469	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1153	737 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
I	1153	737 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 27	-104.9	-104.9	0.21 (1)	10.00	
C- D	-1133 / 0	-104.9	-104.9	0.17 (1)	5.78	
D- E	-851 / 0	-104.9	-104.9	0.44 (1)	5.98	
E- F	-1133 / 0	-104.9	-104.9	0.17 (1)	5.78	
F- G	0 / 27	-104.9	-104.9	0.21 (1)	10.00	
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
M- B	-288 / 0	0.0	0.0	0.03 (1)	7.81	
I- G	-288 / 0	0.0	0.0	0.03 (1)	7.81	
M- L	0 / 911	-28.0	-28.0	0.39 (2)	10.00	
L- K	0 / 851	-28.0	-28.0	0.38 (2)	10.00	
K- J	0 / 851	-28.0	-28.0	0.38 (2)	10.00	
J- I	0 / 911	-28.0	-28.0	0.40 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-97 / 71	0.05 (1)
L- D	0 / 320	0.07 (2)
L- E	0 / 0	0.00 (1)
J- E	0 / 320	0.07 (2)
J- F	-97 / 71	0.05 (1)
M- C	-1425 / 0	0.71 (1)
F- I	-1425 / 0	0.71 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.40/1.00 (I-J:2) , WB=0.71/1.00 (C-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM/0564-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T104S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:03:33 2018 Page 1

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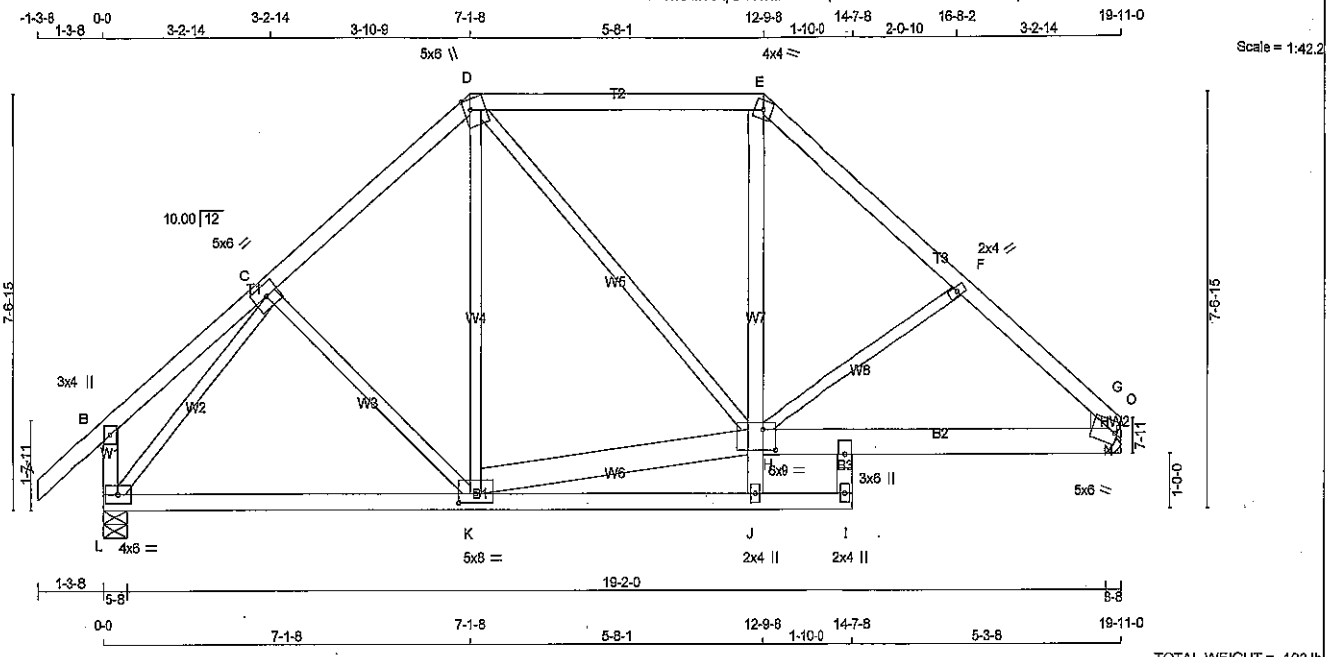

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMB1-m	MT20	5.0	6.0		Edge
H	BWMWWW*-I	MT20	6.0	9.0	4.50	3.00
I	NP+w	MT20	2.0	4.0		
J	BMW+w	MT20	2.0	4.0		
K	BMWWW-t	MT20	5.0	6.0	2.00	4.00
L	BMWW1-t	MT20	4.0	6.0		
M	NP+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
G	1358	0	1358	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 R
L	1489	0	1489	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1098	647 / 0	235 / 0	0 / 0	0 / 0	216 / 0	0 / 0
L	1178	737 / 0	224 / 0	0 / 0	0 / 0	217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 32	-104.9	-104.9	0.22 (1)	10.00
C- D	-1164 / 0	-104.9	-104.9	0.20 (1)	5.69
D- E	-1020 / 0	-104.9	-104.9	0.45 (1)	5.59
E- F	-1346 / 0	-104.9	-104.9	0.22 (1)	5.36
F- O	-1868 / 0	-104.9	-104.9	0.17 (1)	4.99
O- G	-2531 / 0	-104.9	-104.9	0.16 (1)	4.22
L- B	-258 / 0	0.0	0.0	0.03 (1)	7.51
L- K	0 / 919	-28.0	-28.0	0.44 (2)	10.00
K- J	0 / 15	-28.0	-28.0	0.35 (3)	10.00
J- I	0 / 0	-28.0	-28.0	0.13 (2)	10.00
H- N	0 / 1288	-28.0	-28.0	0.25 (2)	10.00
N- G	0 / 1288	-28.0	-28.0	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 247	0.08 (3)
J- H	0 / 186	0.04 (2)
H- E	0 / 427	0.08 (2)
H- F	-333 / 0	0.14 (1)
K- H	0 / 874	0.10 (1)
D- H	0 / 220	0.05 (1)
C- K	-64 / 91	0.04 (1)
L- C	-1472 / 0	0.59 (1)
N- O	0 / 1072	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.44/1.00 (K-L:2) , WB=0.59/1.00 (C-L:1) , SSI=0.53/1.00 (G-N:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.36 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2276
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1057048
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T105	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Tue Feb 27 08:57:35 2018 Page 1

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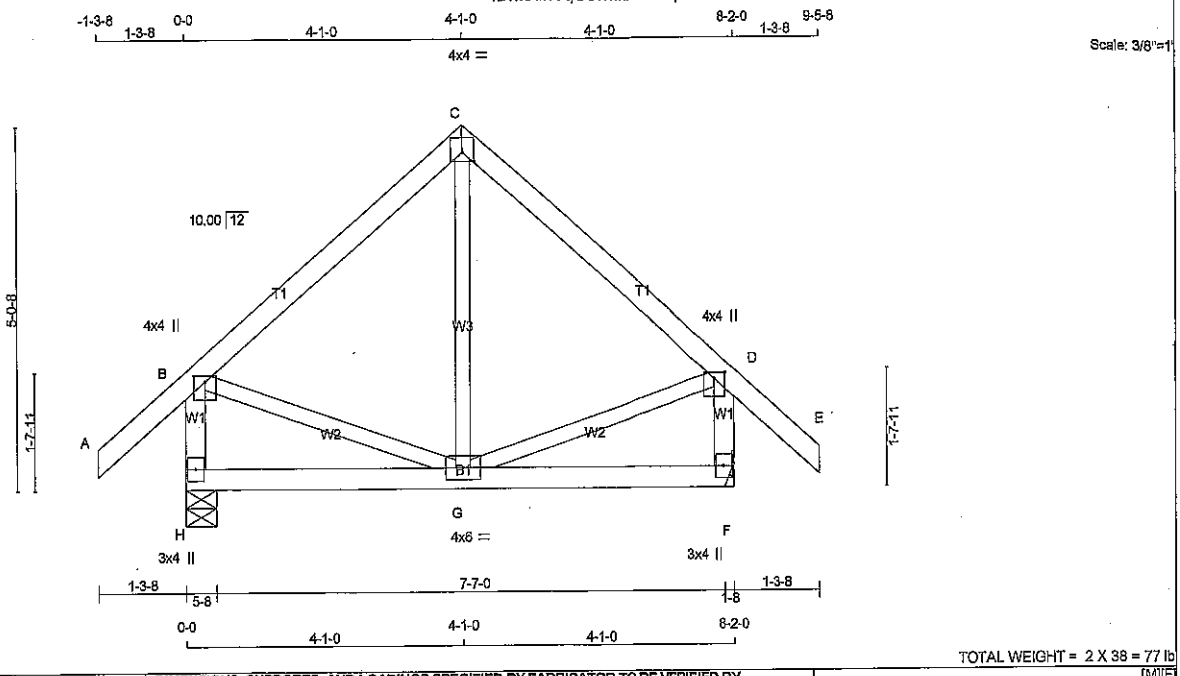

TOTAL WEIGHT = 2 X 38 = 77 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	688	0	688	0	0	IN-SX 5-8	IN-SX 5-8
F	688	0	688	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	531	355 / 0	86 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	531	355 / 0	86 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G- C	-17 / 171	0.04 (3)
B- C	-321 / 0	-104.9	-104.9	0.22 (1)	6.25	B- G	0 / 259	0.06 (1)
C- D	-321 / 0	-104.9	-104.9	0.22 (1)	6.25	G- D	0 / 259	0.06 (1)
D- E	0 / 47	-104.9	-104.9	0.14 (1)	10.00			
H- B	-644 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-644 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.14/1.00 (F-G:3) , WB=0.06/1.00 (B-G:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (C) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9271B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10565-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	G105	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:57:34 2018 Page 1
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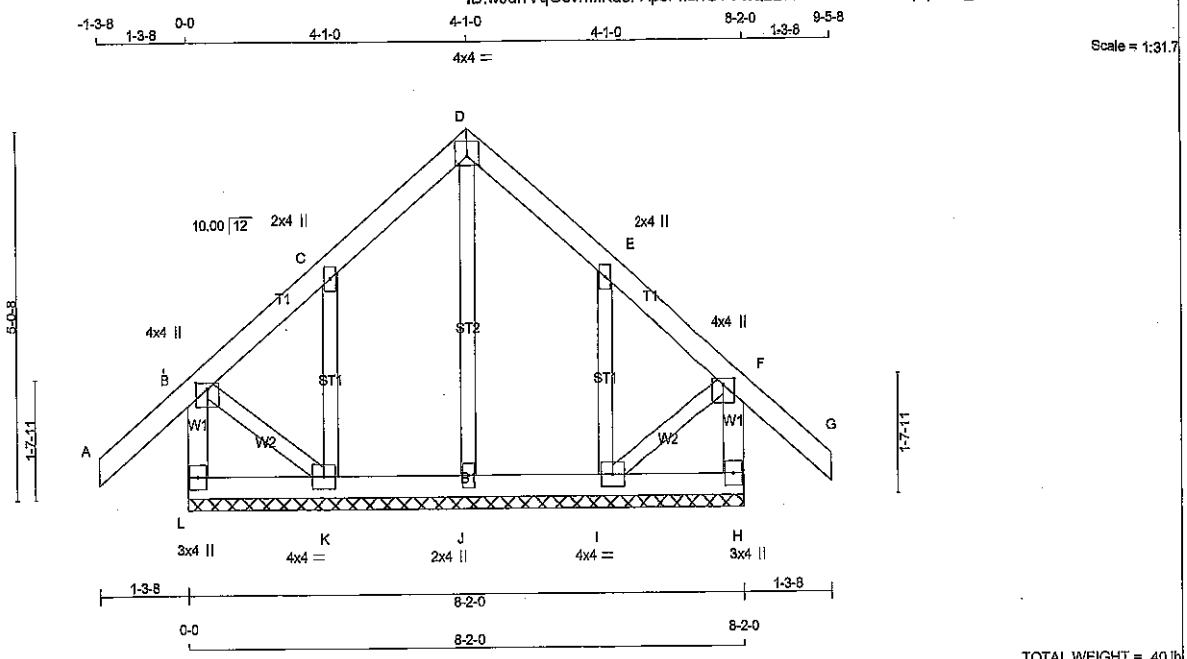

TOTAL WEIGHT = 40 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-13 / 0	-104.9	-104.9	0.07 (1)	6.25
C- D	-35 / 0	-104.9	-104.9	0.07 (1)	6.25
D- E	-35 / 0	-104.9	-104.9	0.07 (1)	6.25
E- F	-13 / 0	-104.9	-104.9	0.07 (1)	6.25
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
H- F	-256 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 0	-28.0	-28.0	0.03 (3)	10.00
K- J	0 / 15	-28.0	-28.0	0.03 (2)	10.00
J- I	0 / 15	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-145 / 0	0.05 (1)
K- C	-260 / 0	0.05 (1)
I- E	-260 / 0	0.05 (1)
B- K	0 / 27	0.01 (1)
I- F	0 / 27	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (I-J:2) , WB=0.05/1.00 (D-J:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9226
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10566-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T106	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 8 2018 MiTek Industries, Inc. Tue Feb 27 08:57:35 2018 Page 1
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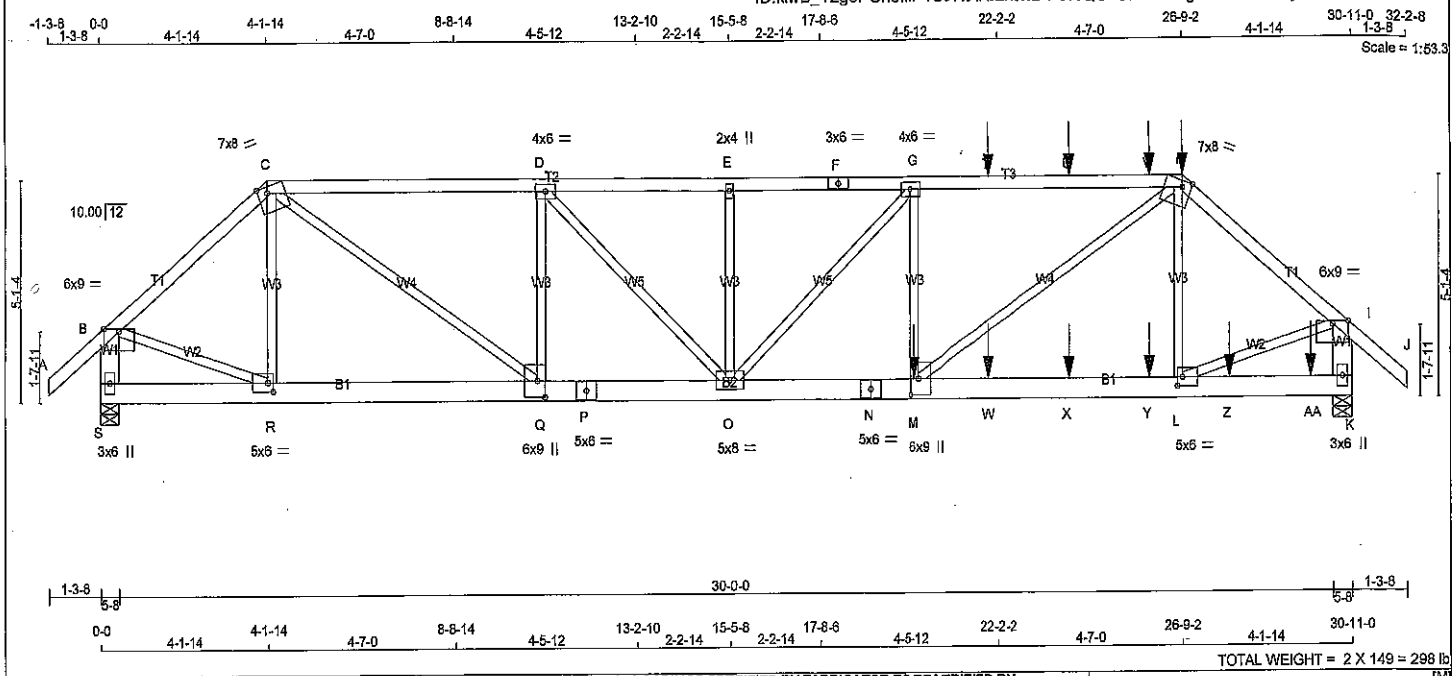

TOTAL WEIGHT = 2 X 149 = 298 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	TOP
P- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(15.5)
G- M	1	4	SIDE(652.3)
2x3	1	6	
D- Q	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW-m	MT20	7.0	8.0	1.75	2.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	1.75	2.75
I	TMVW-p	MT20	6.0	9.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW+t	MT20	6.0	9.0	4.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3707	0	3707	0	0	5-8	5-8
K	6277	0	6277	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2919	1849 / 0	539 / 0	0 / 0	0 / 0	530 / 0	0 / 0
K	4962	3112 / 0	937 / 0	0 / 0	0 / 0	912 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 47	-104.9	-104.9	0.07 (1)	10.00		R- C	-636 / 0	0.12 (1)
B- C	-3846 / 0	-104.9	-104.9	0.18 (1)	5.24		C- Q	0 / 4491	0.56 (1)
C- D	-6611 / 0	-104.9	-104.9	0.46 (1)	4.02		Q- D	-2421 / 0	0.45 (1)
D- E	-8258 / 0	-104.9	-104.9	0.42 (1)	3.61		M- G	0 / 562	0.07 (2)
E- F	-8258 / 0	-104.9	-104.9	0.46 (1)	3.56		M- H	0 / 5541	0.69 (1)
F- G	-8258 / 0	-104.9	-104.9	0.46 (1)	3.56		L- H	-113 / 247	0.03 (3)
G- T	-9576 / 0	-104.9	-104.9	0.76 (1)	3.22		B- R	0 / 3066	0.38 (1)
T- U	-9576 / 0	-104.9	-104.9	0.76 (1)	3.22		L- I	0 / 5244	0.65 (1)
U- V	-9576 / 0	-104.9	-104.9	0.76 (1)	3.22		O- E	-291 / 0	0.05 (1)
V- H	-9576 / 0	-104.9	-104.9	0.76 (1)	3.22		D- O	0 / 2365	0.29 (1)
H- I	-6579 / 0	-104.9	-104.9	0.25 (1)	4.19		O- G	-1895 / 0	0.74 (1)
I- J	0 / 47	-104.9	-104.9	0.07 (1)	10.00				
S- B	-3680 / 0	0.0	0.0	0.14 (1)	7.34				
K- I	-6036 / 0	0.0	0.0	0.22 (1)	6.02				
S- R	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
R- Q	0 / 2933	-28.0	-28.0	0.24 (1)	10.00				
Q- P	0 / 6612	-28.0	-28.0	0.49 (1)	10.00				
P- O	0 / 6612	-28.0	-28.0	0.49 (1)	10.00				
O- N	0 / 9577	-28.0	-28.0	0.67 (1)	10.00				
N- M	0 / 9577	-28.0	-28.0	0.67 (1)	10.00				
M- W	0 / 5038	-28.0	-28.0	0.61 (1)	10.00				
W- X	0 / 5038	-28.0	-28.0	0.61 (1)	10.00				
X- Y	0 / 5038	-28.0	-28.0	0.61 (1)	10.00				
Y- L	0 / 5038	-28.0	-28.0	0.61 (1)	10.00				
L- Z	0 / 0	-28.0	-28.0	0.28 (1)	10.00				
Z-AA	0 / 0	-28.0	-28.0	0.28 (1)	10.00				
AA- K	0 / 0	-28.0	-28.0	0.28 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	26-9-2	-527	-527	---	FRONT	VERT	TOTAL
M	20-0-8	-3202	-3202	---	BACK	VERT	TOTAL
T	21-11-4	-54	-54	---	BACK	VERT	TOTAL
U	23-11-4	-54	-54	---	BACK	VERT	TOTAL
V	25-11-4	-63	-63	---	BACK	VERT	TOTAL
W	21-11-4	-337	-337	---	BACK	VERT	TOTAL
X	23-11-4	-337	-337	---	BACK	VERT	TOTAL
Y	25-11-4	-337	-337	---	BACK	VERT	TOTAL
L	27-11-4	-337	-337	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.76/1.00 (G-H:1) , BC=0.67/1.00 (M-O:1) , WB=0.74/1.00 (G-O:1) , SSI=0.26/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.72 (N) (INPUT = 1.00)

DWG NO. TAM/0567-18 pg 11

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295208	T106	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW+t	MT20	6.0	9.0	4.50	2.25
R	BMWW-t	MT20	5.0	6.0	2.50	1.50
S	BMV1+p	MT20	3.0	6.0		

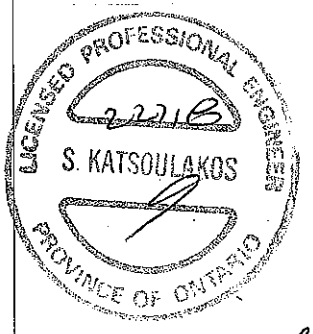
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 26-9-2, 53.9 lbs FACTORED DOWN AT 21-11-4, AND 53.9 lbs FACTORED DOWN AT 23-11-4, AND 63.2 lbs FACTORED DOWN AT 25-11-4 ON TOP CHORD, AND 3202.5 lbs FACTORED DOWN AT 20-0-8, 336.6 lbs FACTORED DOWN AT 21-11-4, 336.6 lbs FACTORED DOWN AT 23-11-4, 336.6 lbs FACTORED DOWN AT 25-11-4, AND 336.6 lbs FACTORED DOWN AT 27-11-4, AND 336.8 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AA	29-11-4	-337	-337	---	BACK	VERT	TOTAL



DWG NO. TAM / 0567-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T106S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:03:33 2018 Page 1

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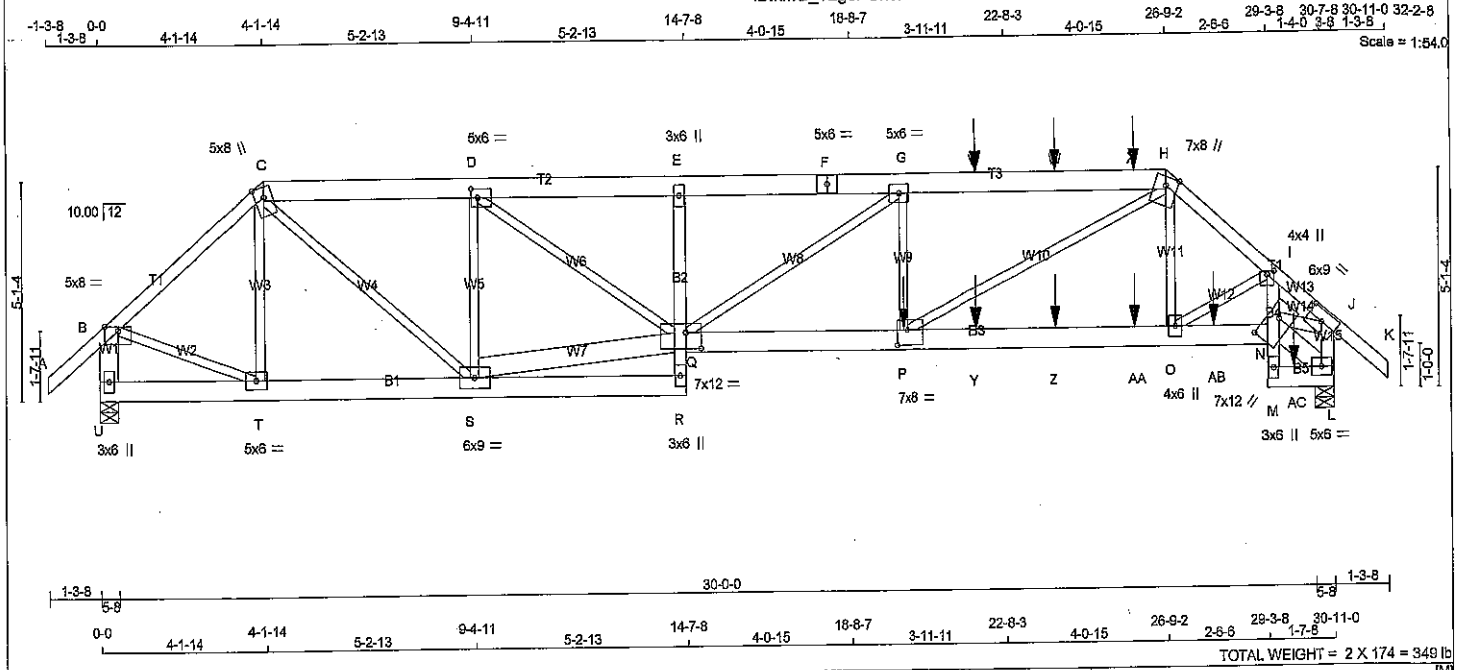

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- K	1	12	TOP
L- J	1	12	TOP
C- F	2	12	TOP
F- H	2	12	SIDE(0.0)
U- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	TOP
Q- N	2	12	SIDE(183.1)
M- L	2	12	SIDE(0.0)
R- E	1	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
G- P	1	3	SIDE(595.2)
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.00
E	TMV+p	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3549	0	3549	0	0	5-8	5-8
L	5674	0	5674	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2806	1759 / 0	531 / 0	0 / 0	0 / 0	516 / 0	0 / 0
L	4507	2789 / 0	876 / 0	0 / 0	0 / 0	841 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-3615 / 0	-104.9	-104.9	0.25 (1)	4.72
C- D	-5632 / 0	-104.9	-104.9	0.15 (1)	4.85
D- E	-9654 / 0	-104.9	-104.9	0.25 (1)	3.79
E- F	-9711 / 0	-104.9	-104.9	0.28 (1)	3.76
F- G	-9711 / 0	-104.9	-104.9	0.28 (1)	3.76
G- V	-11460 / 0	-104.9	-104.9	0.49 (1)	3.30
V- W	-11460 / 0	-104.9	-104.9	0.49 (1)	3.30
W- X	-11460 / 0	-104.9	-104.9	0.49 (1)	3.30
X- H	-11460 / 0	-104.9	-104.9	0.49 (1)	3.30
H- I	-7475 / 0	-104.9	-104.9	0.30 (1)	3.38
I- J	-7702 / 0	-104.9	-104.9	0.26 (1)	3.35
J- K	0 / 47	-104.9	-104.9	0.08 (1)	10.00
U- B	-3509 / 0	0.0	0.0	0.13 (1)	7.47
L- J	-5364 / 0	0.0	0.0	0.31 (1)	5.18
U- T	0 / 0	-28.0	-28.0	0.04 (2)	10.00
T- S	0 / 2799	-28.0	-28.0	0.21 (1)	10.00
S- R	0 / 209	-28.0	-28.0	0.05 (3)	10.00
R- Q	0 / 105	0.0	0.0	0.16 (1)	10.00
Q- E	-502 / 0	0.0	0.0	0.16 (1)	7.81
Q- P	0 / 11460	-28.0	-28.0	0.78 (1)	10.00
P- Y	0 / 5745	-28.0	-28.0	0.71 (1)	10.00
Y- Z	0 / 5745	-28.0	-28.0	0.71 (1)	10.00
Z-AA	0 / 5745	-28.0	-28.0	0.71 (1)	10.00
AA- O	0 / 5745	-28.0	-28.0	0.71 (1)	10.00
O-AB	0 / 6023	-28.0	-28.0	0.73 (1)	10.00
AB- N	0 / 6023	-28.0	-28.0	0.73 (1)	10.00
M- N	0 / 246	0.0	0.0	0.19 (1)	10.00
N- I	0 / 187	0.0	0.0	0.19 (1)	10.00
M-AC	0 / 192	-28.0	-28.0	0.06 (1)	10.00
AC- L	0 / 192	-28.0	-28.0	0.06 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- C	-686 / 0	0.12 (1)
C- S	0 / 3822	0.47 (1)
S- D	-3480 / 0	0.61 (1)
S- Q	0 / 5587	0.36 (1)
D- Q	0 / 4871	0.60 (1)
Q- G	-2091 / 0	0.82 (1)
P- G	0 / 486	0.06 (2)
P- H	0 / 6510	0.81 (1)
O- H	0 / 1138	0.14 (1)
O- I	-296 / 0	0.03 (1)
B- T	0 / 2907	0.36 (1)
N- L	-245 / 0	0.01 (1)
N- J	0 / 5860	0.52 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	20-0-8	-2983	-2983	---	BACK	VERT	TOTAL
V	21-11-4	-27	-27	---	BACK	VERT	TOTAL
W	23-11-4	-27	-27	---	BACK	VERT	TOTAL
X	25-11-4	-37	-37	---	BACK	VERT	TOTAL
Y	21-11-4	-351	-351	---	BACK	VERT	TOTAL
Z	23-11-4	-351	-351	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 904 (0.41")

CSI: TC=0.49/1.00 (G-H:1) , BC=0.78/1.00 (P-Q:1) , WB=0.82/1.00 (G-Q:1) , SSI=0.27/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)

DWG NO. TAM 10571-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	T106S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-t	MT20	6.0	9.0	3.00	4.50
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-w	MT20	7.0	12.0	3.25	7.50
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	7.0	8.0	4.25	3.25
Q	BVMWWW-I	MT20	7.0	12.0	4.50	4.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 27.3 lbs FACTORED DOWN AT 21-11-4, AND 27.3 lbs FACTORED DOWN AT 23-11-4, AND 36.6 lbs FACTORED DOWN AT 25-11-4 ON TOP CHORD, AND 2983.3 lbs FACTORED DOWN AT 20-0-8, 350.9 lbs FACTORED DOWN AT 21-11-4, 350.9 lbs FACTORED DOWN AT 23-11-4, 350.9 lbs FACTORED DOWN AT 25-11-4, AND 350.9 lbs FACTORED DOWN AT 27-11-4, AND 343.8 lbs FACTORED DOWN AT 29-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AA	25-11-4	-351	-351	---	BACK	VERT	TOTAL
AB	27-11-4	-351	-351	---	BACK	VERT	TOTAL
AC	29-11-4	-344	-344	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
22718
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10571 -R
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T120	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:47 2018 Page 1
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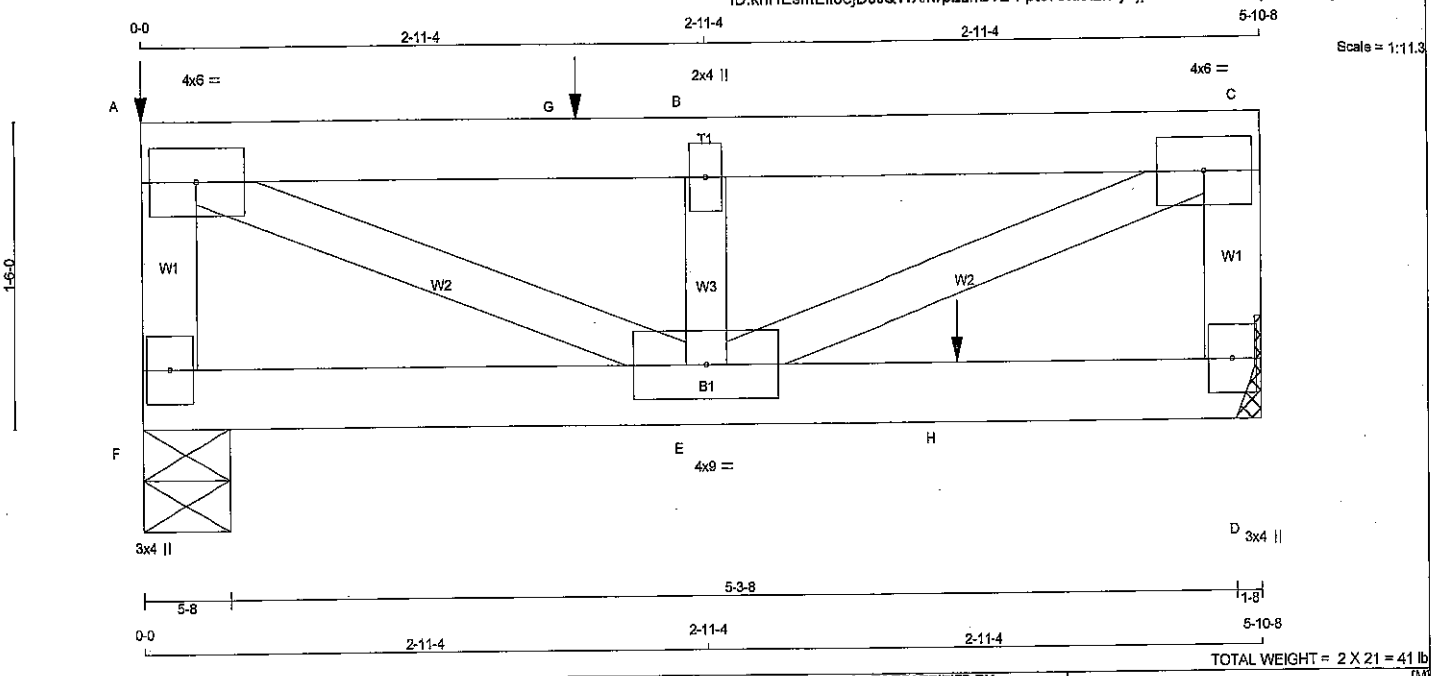

TOTAL WEIGHT = 2 X 21 = 41 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	9	SIDE(10.1)
A- C	1	12	SIDE(61.0)
C- D	1	9	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	BMV1+p	MT20	3.0	4.0	
E	BMWWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 272.5 lbs FACTORED DOWN AT 0-0, AND 911.6 lbs FACTORED DOWN AT 2-3-4 ON TOP CHORD, AND 498.2 lbs FACTORED DOWN AT 4-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1358	0	1358	0	0	5-8	5-8
D	1105	0	1105	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1056	692 / 0	179 / 0	0 / 0	0 / 0	185 / 0	0 / 0
D	868	553 / 0	158 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1342 / 0	0.0	0.0	0.08 (1)	7.81	A- E	0 / 2063	0.26 (1)
A- G	-1908 / 0	-104.9	-104.9	0.30 (1)	5.91	E- B	-1165 / 0	0.09 (1)
G- B	-1908 / 0	-104.9	-104.9	0.30 (1)	5.91	E- C	0 / 2063	0.26 (1)
B- C	-1908 / 0	-104.9	-104.9	0.18 (1)	6.10			
D- C	-863 / 0	0.0	0.0	0.05 (1)	7.81			
F- E	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
E- H	0 / 0	-28.0	-28.0	0.26 (1)	10.00			
H- D	0 / 0	-28.0	-28.0	0.26 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-272	-272	---	FRONT	VERT	TOTAL
G	2-3-4	-912	-912	---	TOP	VERT	TOTAL
H	4-3-4	-498	-498	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.26/1.00 (D-E:1) , WB=0.26/1.00 (A-E:1) , SSI=0.40/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (A) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1254-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287476	T121	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:06:48 2018 Page 1

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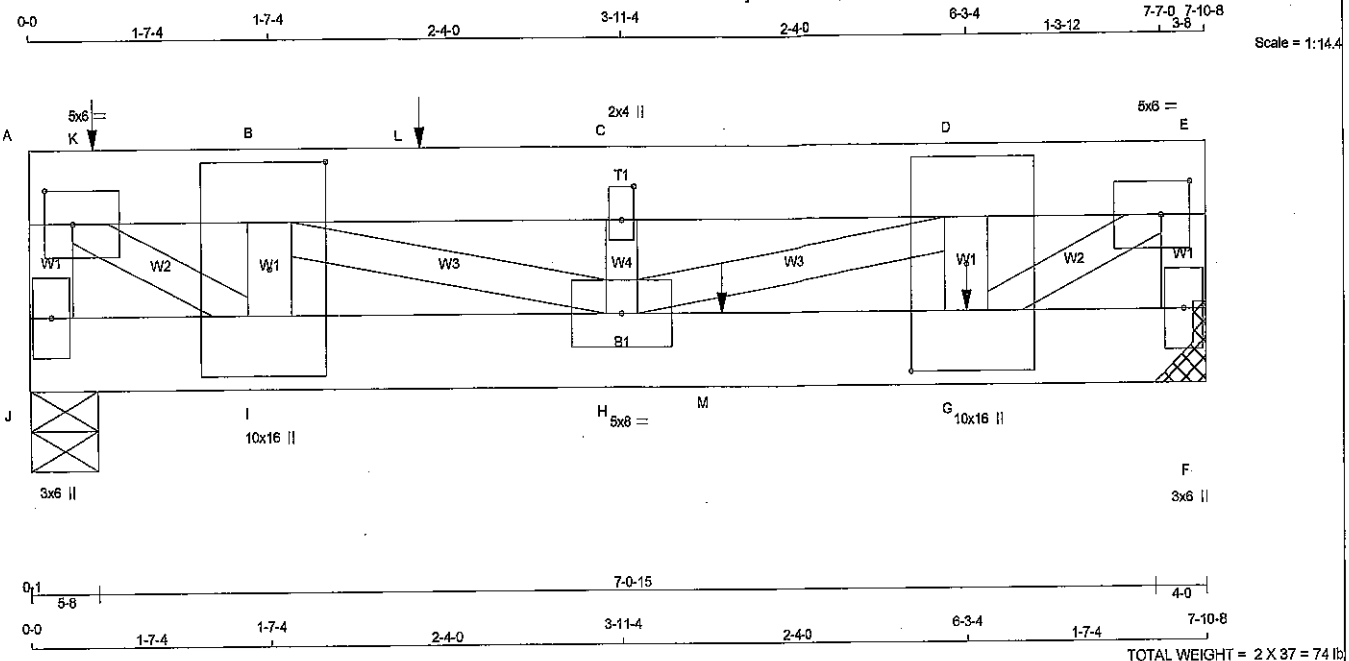

TOTAL WEIGHT = 2 X 37 = 74 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J- A	1	12	TOP
E- F	1	12	TOP
A- E	2	12	SIDE(122.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	4	SIDE(671.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.25
B, D, G, I						
B						
C	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMVW-t	MT20	5.0	6.0	2.50	2.25
F	BMV1+p	MT20	3.0	6.0		
G	TMBMWWW+w	MT20	10.0	16.0	8.00	4.50
H	BMWWW-t	MT20	5.0	8.0		
I	TMBMWWW+w	MT20	10.0	16.0	8.00	4.50
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 258.5 lbs FACTORED DOWN AT 5-4, AND 911.6 lbs FACTORED DOWN AT 2-7-4 ON TOP CHORD, AND 498.2 lbs FACTORED DOWN AT 4-7-4, AND 2449.6 lbs FACTORED DOWN AT 6-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	2084	0	2084	0	0	5-8	5-8
F	3081	0	3081	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1630	1051 / 0	289 / 0	0 / 0	0 / 0	291 / 0	0 / 0
F	2427	1535 / 0	449 / 0	0 / 0	0 / 0	442 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J- A	-1977 / 0	0.0	0.0	1	0.11	7.81	H- C	-630 / 0	0.05 (1)
A- K	-2762 / 0	-104.9	-104.9	1	0.06	6.25	G- D	-166 / 13	0.01 (1)
K- B	-2762 / 0	-104.9	-104.9	1	0.06	6.25	I- B	-1799 / 0	0.10 (1)
B- L	-4564 / 0	-104.9	-104.9	1	0.23	5.15	A- I	0 / 3293	0.41 (1)
L- C	-4564 / 0	-104.9	-104.9	1	0.23	5.15	B- H	0 / 1974	0.24 (1)
C- D	-4564 / 0	-104.9	-104.9	1	0.16	5.24	H- D	0 / 689	0.09 (1)
D- E	-3935 / 0	-104.9	-104.9	1	0.15	5.56	G- E	0 / 4692	0.58 (1)
F- E	-2858 / 0	0.0	0.0	1	0.16	6.79			
J- I	0 / 0	-28.0	-28.0	1	0.05	10.00			
I- H	0 / 2762	-28.0	-28.0	1	0.25	10.00			
H- M	0 / 3935	-28.0	-28.0	1	0.43	10.00			
M- G	0 / 3935	-28.0	-28.0	1	0.43	10.00			
G- F	0 / 0	-28.0	-28.0	1	0.12	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
G	6-3-4	1	-2450	-2450	---	FRONT	VERT TOTAL
K	5-4	1	-258	-258	---	BACK	VERT TOTAL
L	2-7-4	1	-912	-912	---	TOP	VERT TOTAL
M	4-7-4	1	-498	-498	---	FRONT	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.43/1.00 (G-H:1) , WB=0.58/1.00 (E-G:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.49 (E) (INPUT = 1.00)

DWG NO. TAM 10595-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	PB1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:09 2018 Page 1
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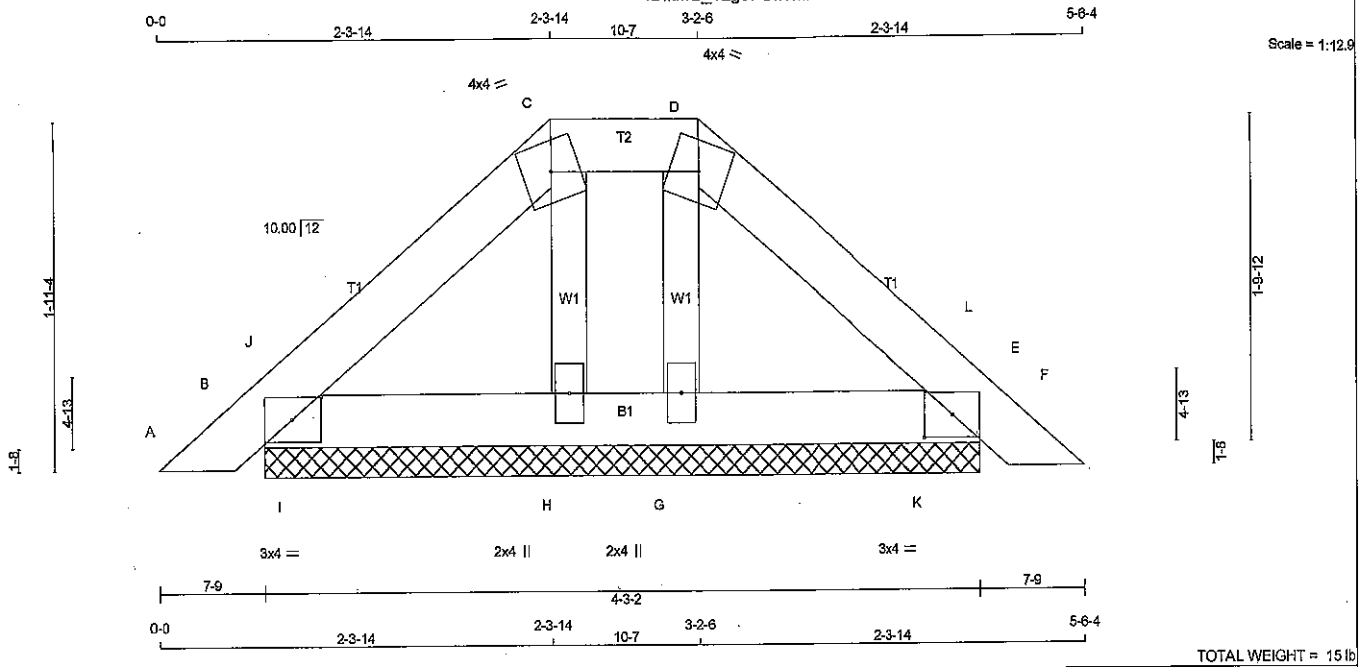

TOTAL WEIGHT = 15 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	199	0	199	0	0	4-3-2	4-3-2
E	199	0	199	0	0	4-3-2	4-3-2
H	140	0	140	0	0	4-3-2	4-3-2
G	140	0	140	0	0	4-3-2	4-3-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	149	108 / 0	18 / 0	0 / 0	0 / 0	22 / 0	0 / 0
E	149	108 / 0	18 / 0	0 / 0	0 / 0	22 / 0	0 / 0
H	115	65 / 0	27 / 0	0 / 0	0 / 0	24 / 0	0 / 0
G	115	65 / 0	27 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-49 / 0	-104.9	-104.9	0.01 (3)	6.25
J- C	-65 / 0	-104.9	-104.9	0.04 (1)	6.25
C- D	-35 / 0	-104.9	-104.9	0.01 (1)	6.25
D- L	-55 / 0	-104.9	-104.9	0.04 (1)	6.25
L- E	-49 / 0	-104.9	-104.9	0.01 (3)	6.25
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 41	-28.0	-28.0	0.04 (1)	10.00
I- H	0 / 41	-28.0	-28.0	0.04 (1)	10.00
H- G	0 / 35	-28.0	-28.0	0.03 (1)	10.00
G- K	0 / 41	-28.0	-28.0	0.04 (1)	10.00
K- E	0 / 41	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-85 / 0	0.01 (1)
G- D	-85 / 0	0.01 (1)
I- J	-97 / 25	0.00 (1)
K- L	-97 / 25	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-J:1) , BC=0.04/1.00 (B-I:1) , WB=0.01/1.00 (C-H:1) , SSI=0.08/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22918
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 105487-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287473	PB2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:46:09 2018 Page 1

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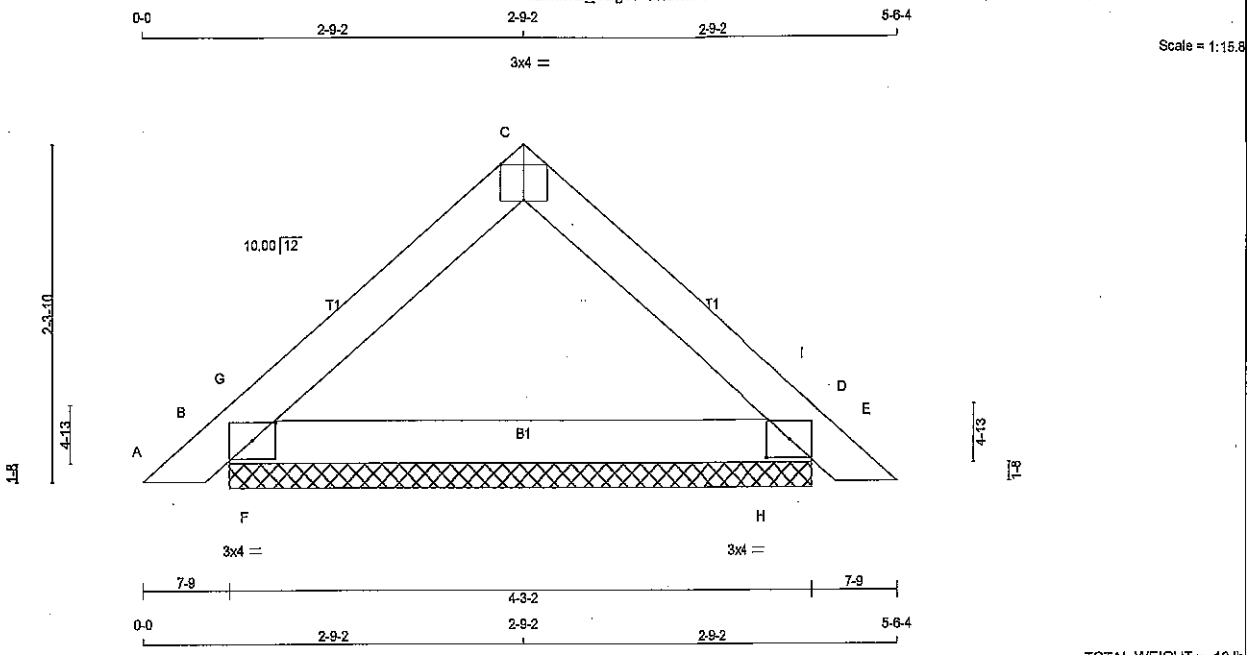

Scale = 1:15.8

TOTAL WEIGHT = 13 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	339	0	339	0	0	4-3-2	4-3-2
D	339	0	339	0	0	4-3-2	4-3-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	264	173 / 0	45 / 0	0 / 0	0 / 0	46 / 0	0 / 0
D	264	173 / 0	45 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- G	-292 / 0	-104.9	-104.9	0.07 (3)	6.25	
G- C	-169 / 0	-104.9	-104.9	0.09 (1)	6.25	
C- I	-169 / 0	-104.9	-104.9	0.09 (1)	6.25	
I- D	-292 / 0	-104.9	-104.9	0.07 (3)	6.25	
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- F	0 / 137	-28.0	-28.0	0.05 (3)	10.00	
F- H	0 / 137	-28.0	-28.0	0.09 (2)	10.00	
H- D	0 / 137	-28.0	-28.0	0.05 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 231	0.00 (1)
H- I	0 / 231	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-G:1) , BC=0.09/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.11/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10549-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287475	PB20	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 8 2018 MiTek Industries, Inc. Tue Feb 27 08:54:30 2018 Page 1
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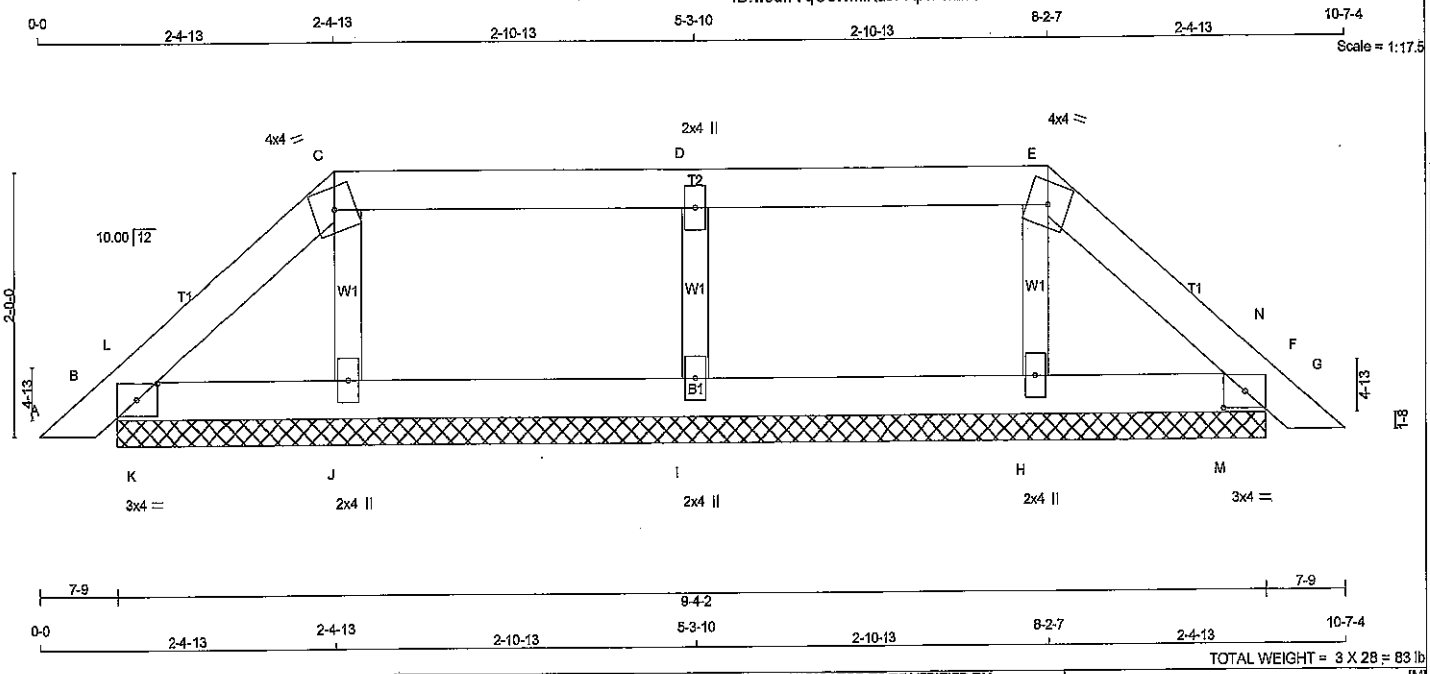


TOTAL WEIGHT = 3 X 28 = 83 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	200	0	200	0	0	9-4-2	9-4-2
F	200	0	200	0	0	9-4-2	9-4-2
J	250	0	250	0	0	9-4-2	9-4-2
H	250	0	250	0	0	9-4-2	9-4-2
I	454	0	454	0	0	9-4-2	9-4-2

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	146	112 / 0	14 / 0	0 / 0	0 / 0	20 / 0	0 / 0
F	146	112 / 0	14 / 0	0 / 0	0 / 0	20 / 0	0 / 0
J	210	111 / 0	53 / 0	0 / 0	0 / 0	46 / 0	0 / 0
H	210	111 / 0	53 / 0	0 / 0	0 / 0	46 / 0	0 / 0
I	354	230 / 0	62 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	J- C	-157 / 0	0.02 (1)
B- L	-41 / 0	-104.9	-104.9	0.01 (1)	6.25	H- E	-157 / 0	0.02 (1)
L- C	-56 / 0	-104.9	-104.9	0.04 (1)	6.25	I- D	-379 / 0	0.06 (1)
C- D	-30 / 0	-104.9	-104.9	0.15 (1)	6.25	K- L	-113 / 10	0.00 (1)
D- E	-30 / 0	-104.9	-104.9	0.15 (1)	6.25	M- N	-113 / 10	0.00 (1)
E- N	-56 / 0	-104.9	-104.9	0.04 (1)	6.25			
N- F	-41 / 0	-104.9	-104.9	0.01 (1)	6.25			
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00			
B- K	0 / 40	-28.0	-28.0	0.04 (1)	10.00			
K- J	0 / 40	-28.0	-28.0	0.04 (1)	10.00			
J- I	0 / 30	-28.0	-28.0	0.05 (3)	10.00			
I- H	0 / 30	-28.0	-28.0	0.05 (3)	10.00			
H- M	0 / 40	-28.0	-28.0	0.04 (1)	10.00			
M- F	0 / 40	-28.0	-28.0	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (D-E:1) , BC=0.05/1.00 (I-J:3) , WB=0.06/1.00 (D-I:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

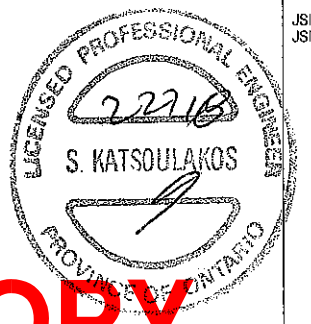
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



DWG NO. TAM/0561-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287443	J8S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 08:51:01 2018 Page 1

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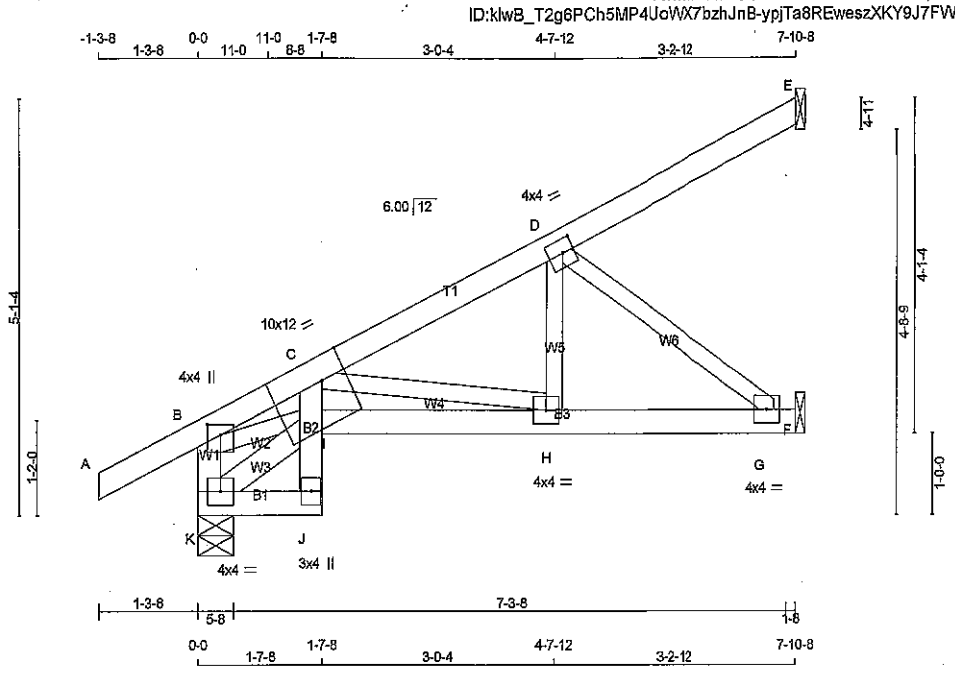


TOTAL WEIGHT = 7 X 32 = 227 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
K - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C						
C	TMBVWWW*-t	MT20	10.0	12.0	Edge	3.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	BMW-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I						
J	BMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	678	0	678	0	0	5-8	5-8
E	132	0	132	0	0	1-8	1-8
F	379	0	379	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E , F

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	522	351 / 0	83 / 0	0 / 0	0 / 0	88 / 0	0 / 0
E	90	82 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
F	319	167 / 0	82 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
K- B	-613 / 0	0.0	0.0	0.06 (1)	7.81
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00
B- C	-863 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-563 / 0	-104.9	-104.9	0.17 (1)	6.25
D- E	-17 / 0	-104.9	-104.9	0.16 (1)	6.25
K- J	0 / 69	-28.0	-28.0	0.03 (2)	10.00
J- I	0 / 36	0.0	0.0	0.09 (1)	10.00
I- C	0 / 89	0.0	0.0	0.10 (1)	10.00
I- H	0 / 848	-28.0	-28.0	0.21 (1)	10.00
H- G	0 / 519	-28.0	-28.0	0.28 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.20 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-334 / 0	0.07 (1)
H- D	0 / 286	0.06 (2)
D- G	-654 / 0	0.16 (1)
K- I	-83 / 0	0.01 (1)
B- I	0 / 746	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.28/1.00 (G-H:1) , WB=0.17/1.00 (B-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

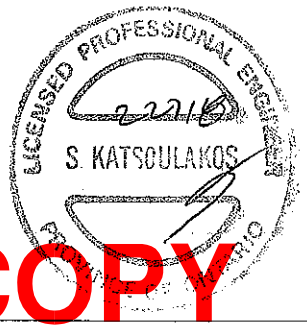
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



DWG NO. TAM 105526

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295207	J100S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Feb 27 09:03:32 2018 Page 1
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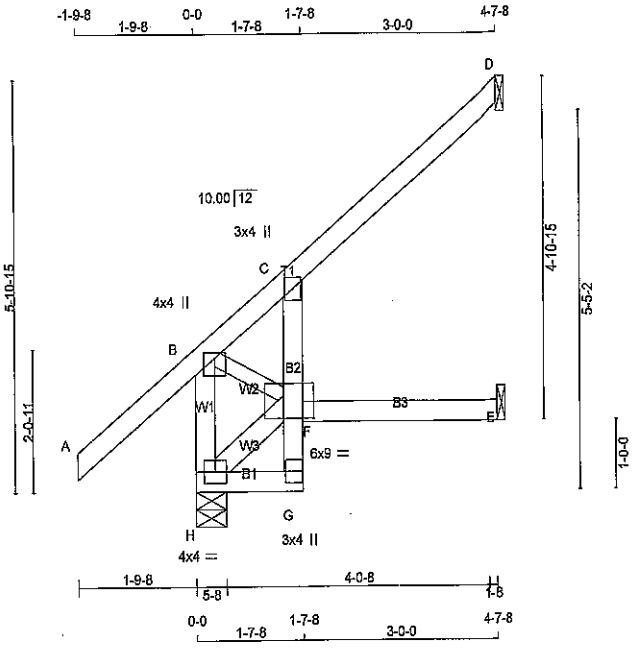


TOTAL WEIGHT = 2 X 23 = 45 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
F	BVMWW-I	MT20	6.0	9.0	4.00	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	546	0	546	0	0	5-8	5-8
D	223	0	223	0	-19	1-8	1-8
E	44	0	77	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	406	298 / 0	49 / 0	0 / 0	0 / 0	60 / 0	0 / 0
D	163	125 / -25	16 / 0	0 / 0	0 / 0	22 / 0	0 / 0
E	55	0 / 0	33 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
H- B	-521 / 0	0.0	0.0	0.06 (1)	7.81	H- F	-7 / 0	0.00 (1)
A- B	0 / 63	-104.9	-104.9	0.26 (1)	10.00	B- F	-9 / 0	0.00 (2)
B- C	-156 / 0	-104.9	-104.9	0.25 (1)	6.25			
C- D	-17 / 34	-104.9	-104.9	0.33 (1)	6.25			
H- G	0 / 5	-28.0	-28.0	0.02 (2)	10.00			
G- F	0 / 36	0.0	0.0	0.01 (1)	10.00			
F- C	0 / 115	0.0	0.0	0.02 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.08 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 822 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 516 (0.11")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.08/1.00 (E-F:3) , WB=0.00/1.00 (B-F:2) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 125722-18
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

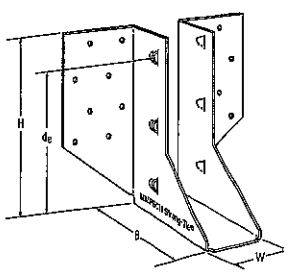
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

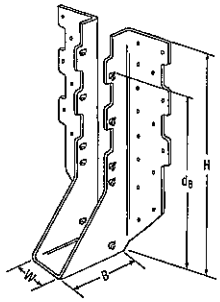
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

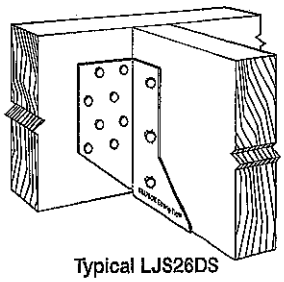
- See current catalogue for options



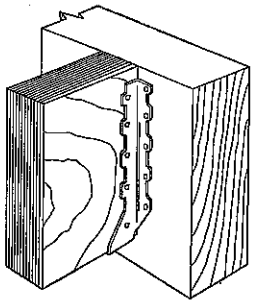
LJS26DS



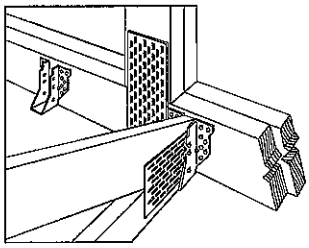
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



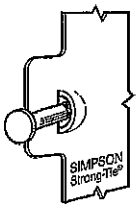
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

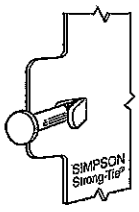
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

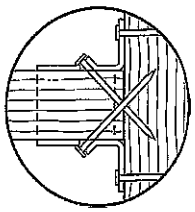


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

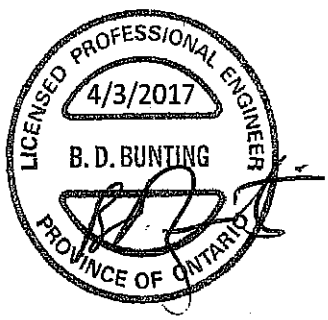
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

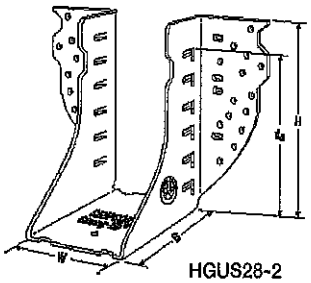
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

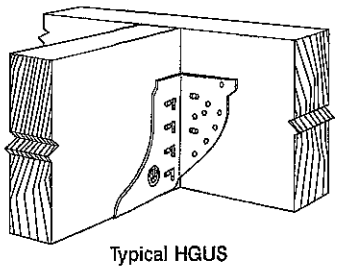
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

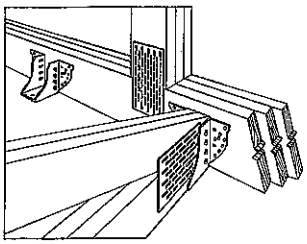
Options:

- See current catalogue for options



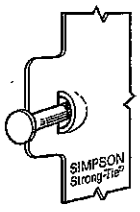
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

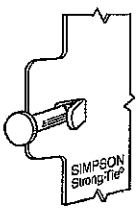
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

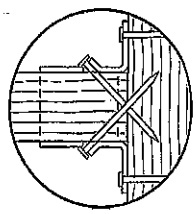


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

T-SPECHGUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

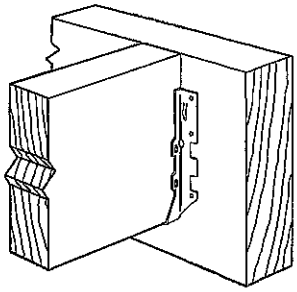
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

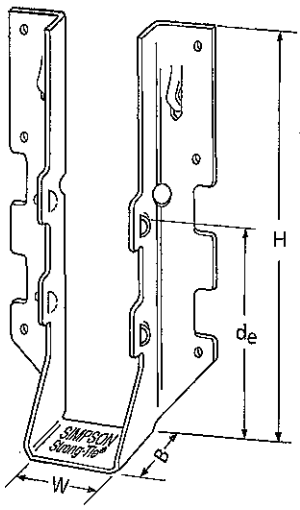
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



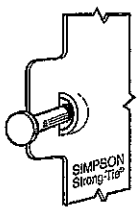
Typical LUS Installation



LUS28

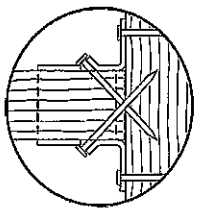
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

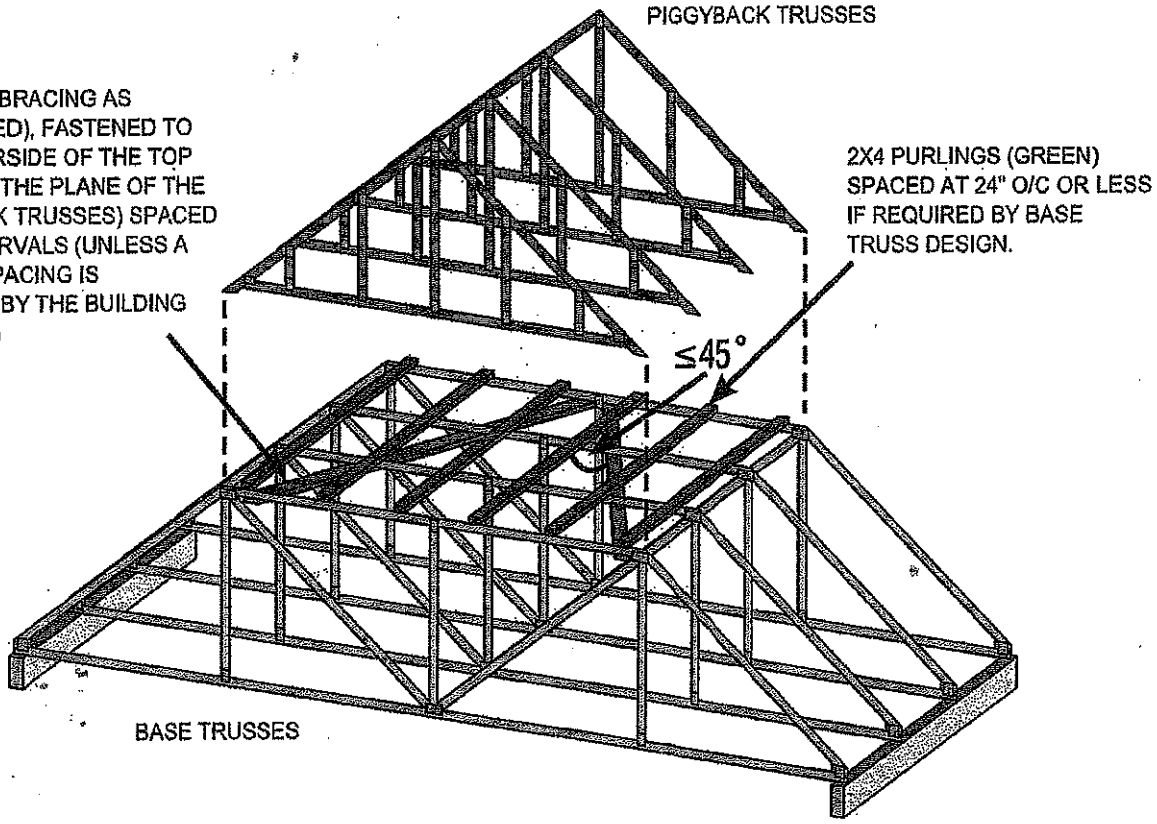
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

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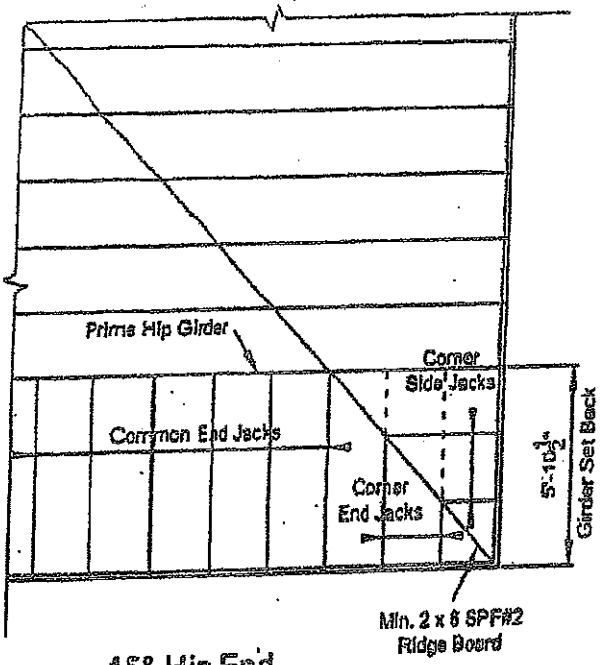


45° Hip End

LUMBER SPECIFICATION

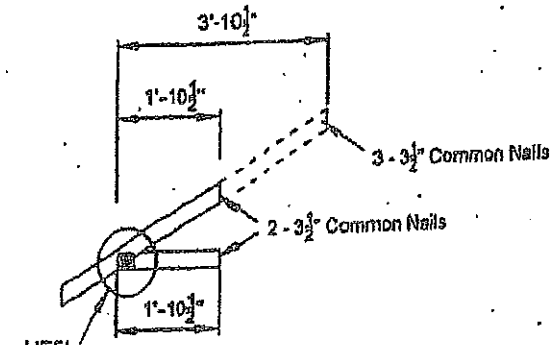
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

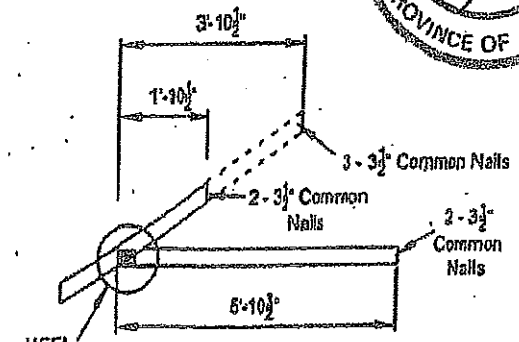
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

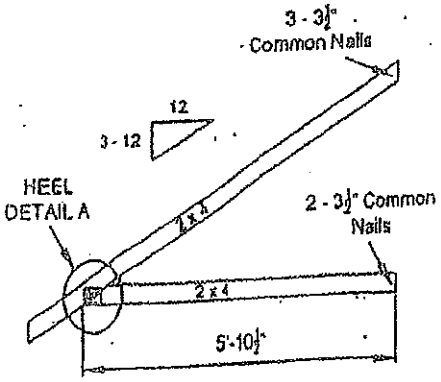
LICENSED PROFESSIONAL ENGINEER
1-2-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



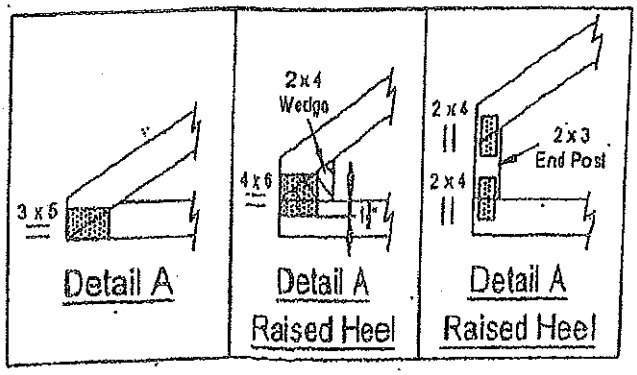
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY

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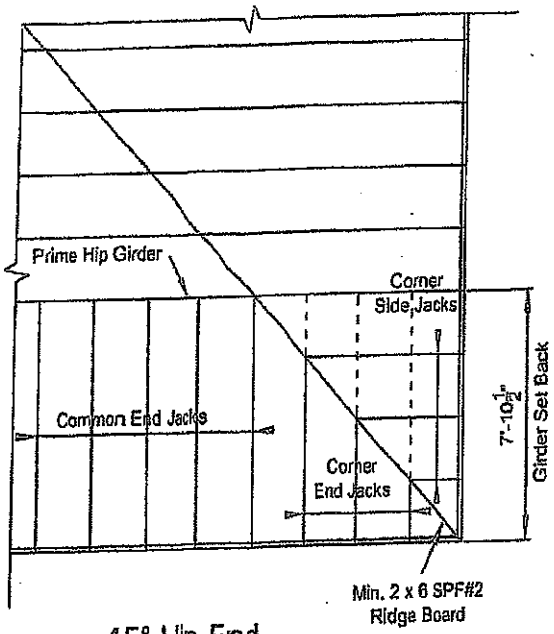
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



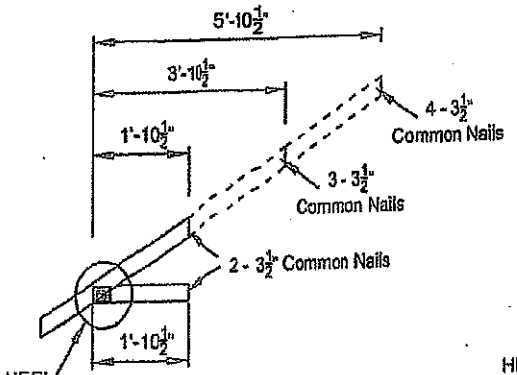
45° Hip End

DESIGN LOAD:

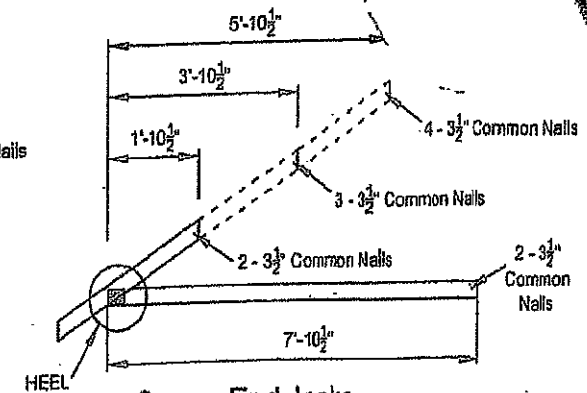
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 350314
STRUCTURAL
COMPONENT ONLY

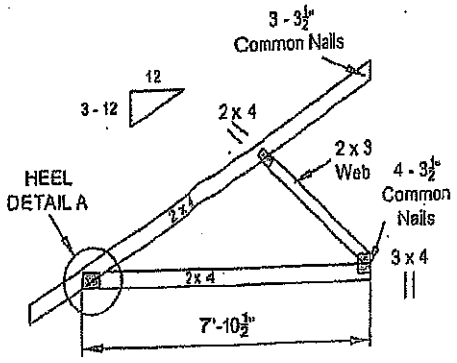
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



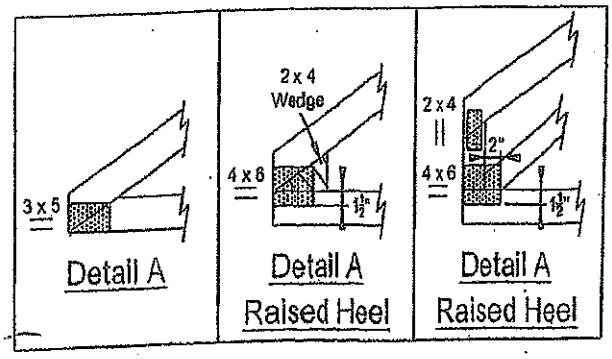
Corner Side Jacks



Corner End Jacks



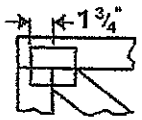
Common End Jacks



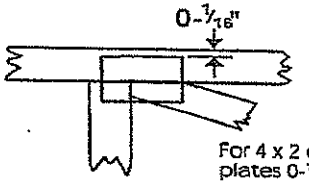
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

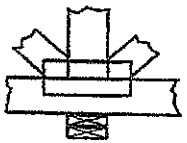
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

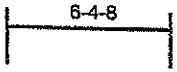
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

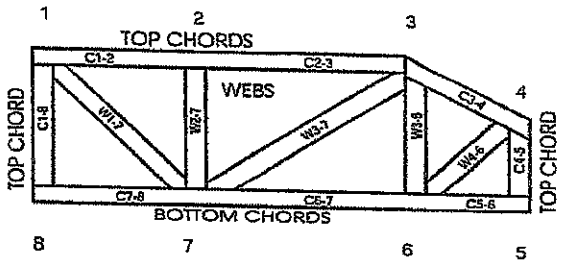
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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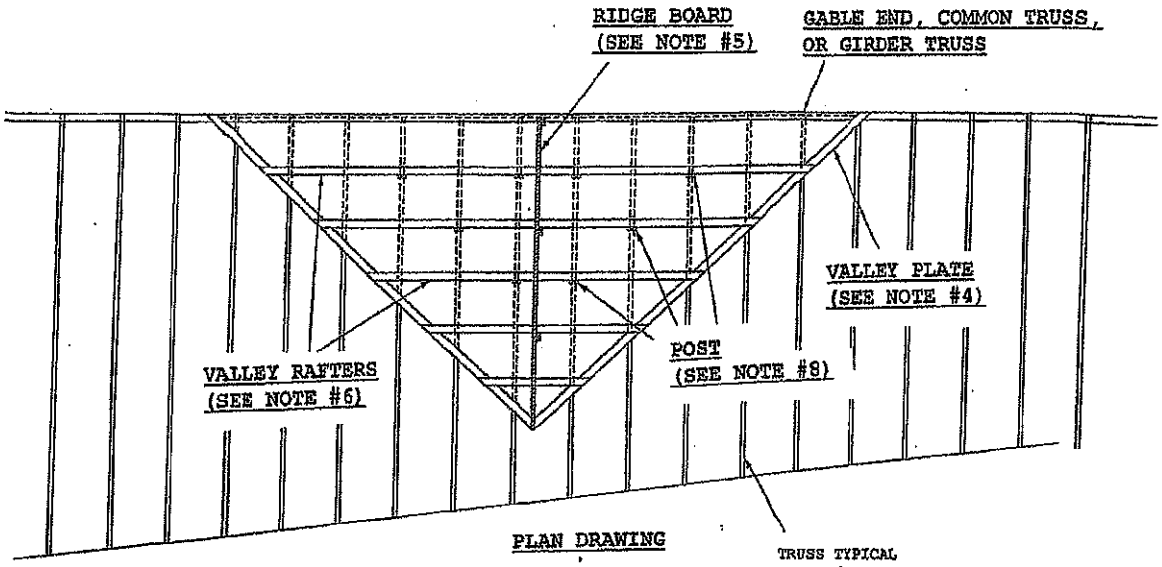
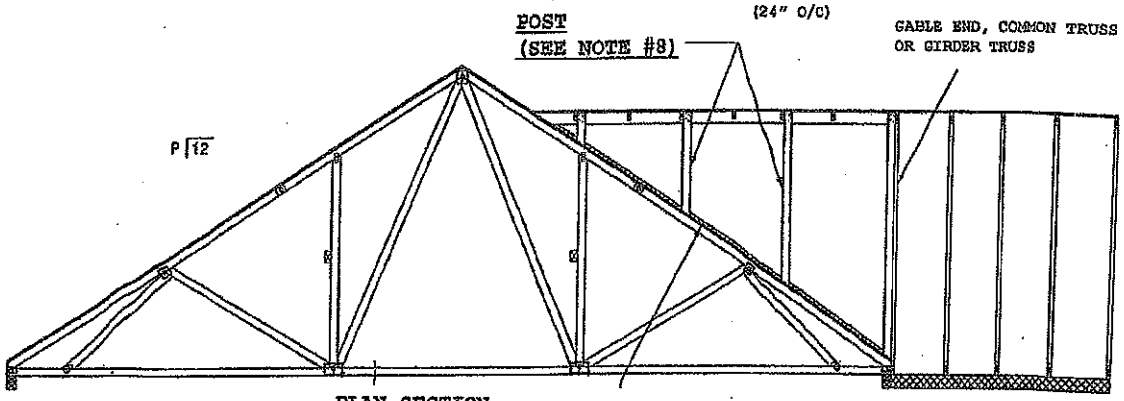
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

SITE COPY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM 14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.