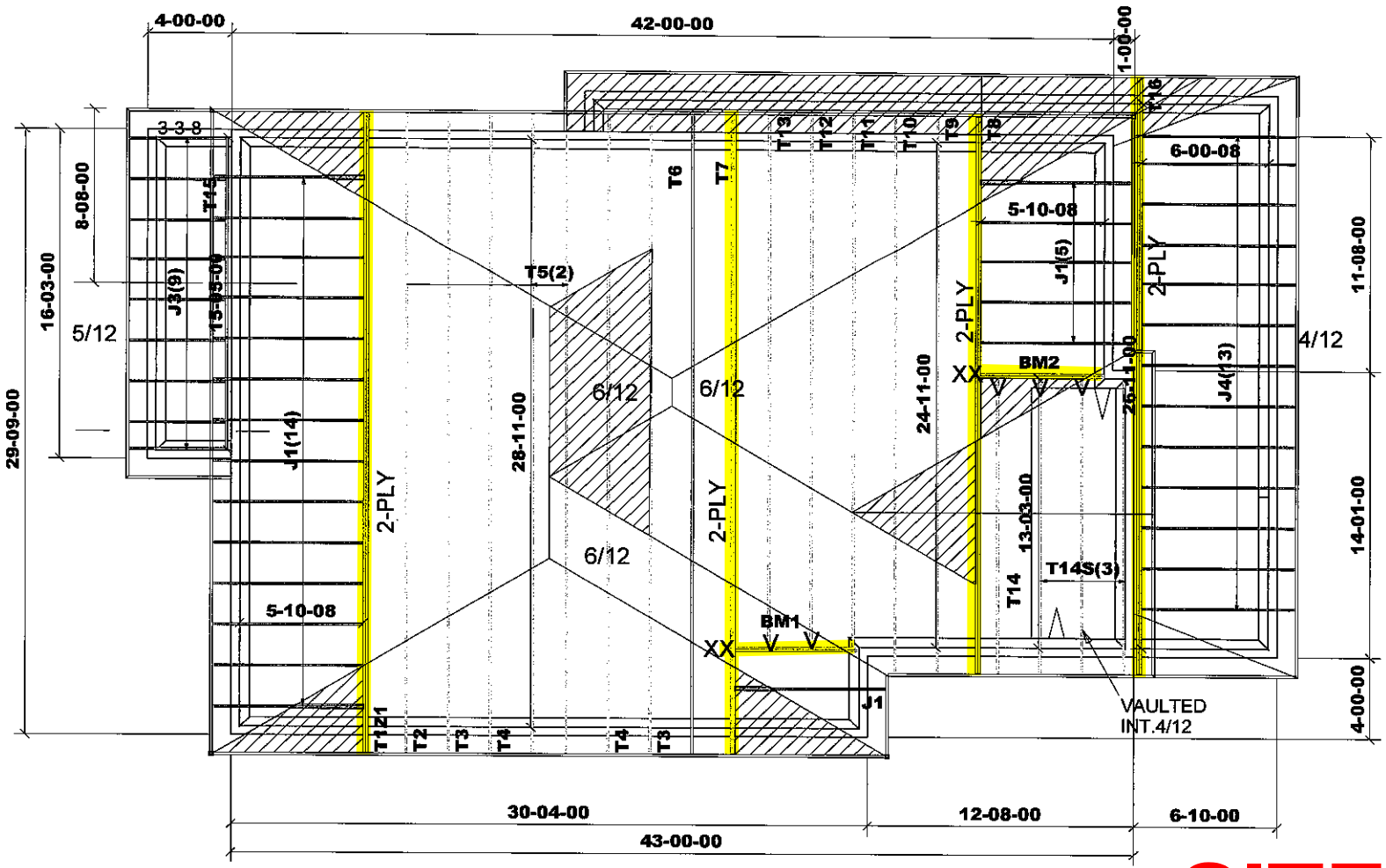
TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-10-23
INSPECTOR: BG

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T- 180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,445

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	295155
Plan Log:	96660

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

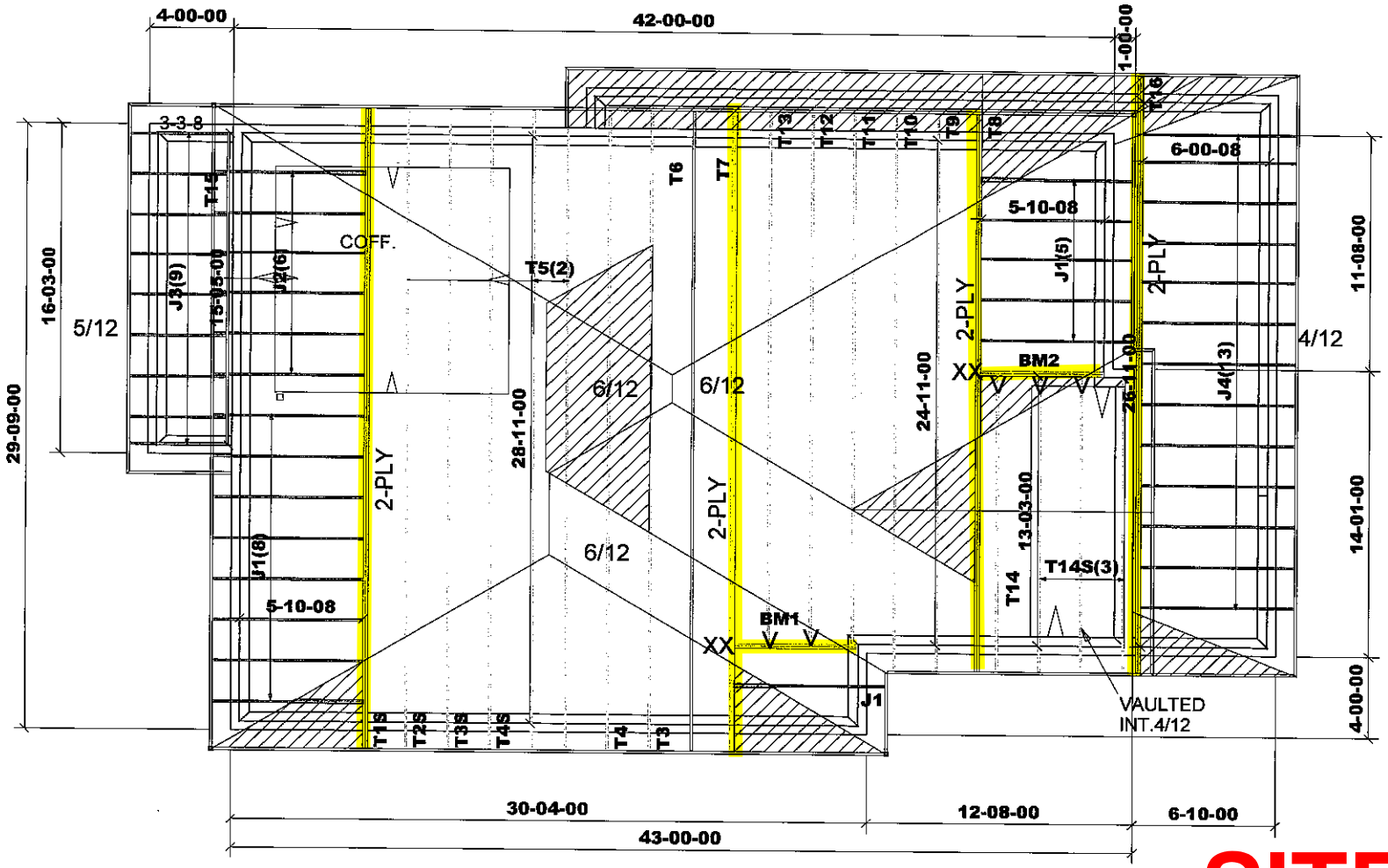
Date: 2/23/2018 Designer: JG

Model / Elevation: **S38-1 / A - STD.**

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T- 180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,445

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287391
Plan Log: 96660

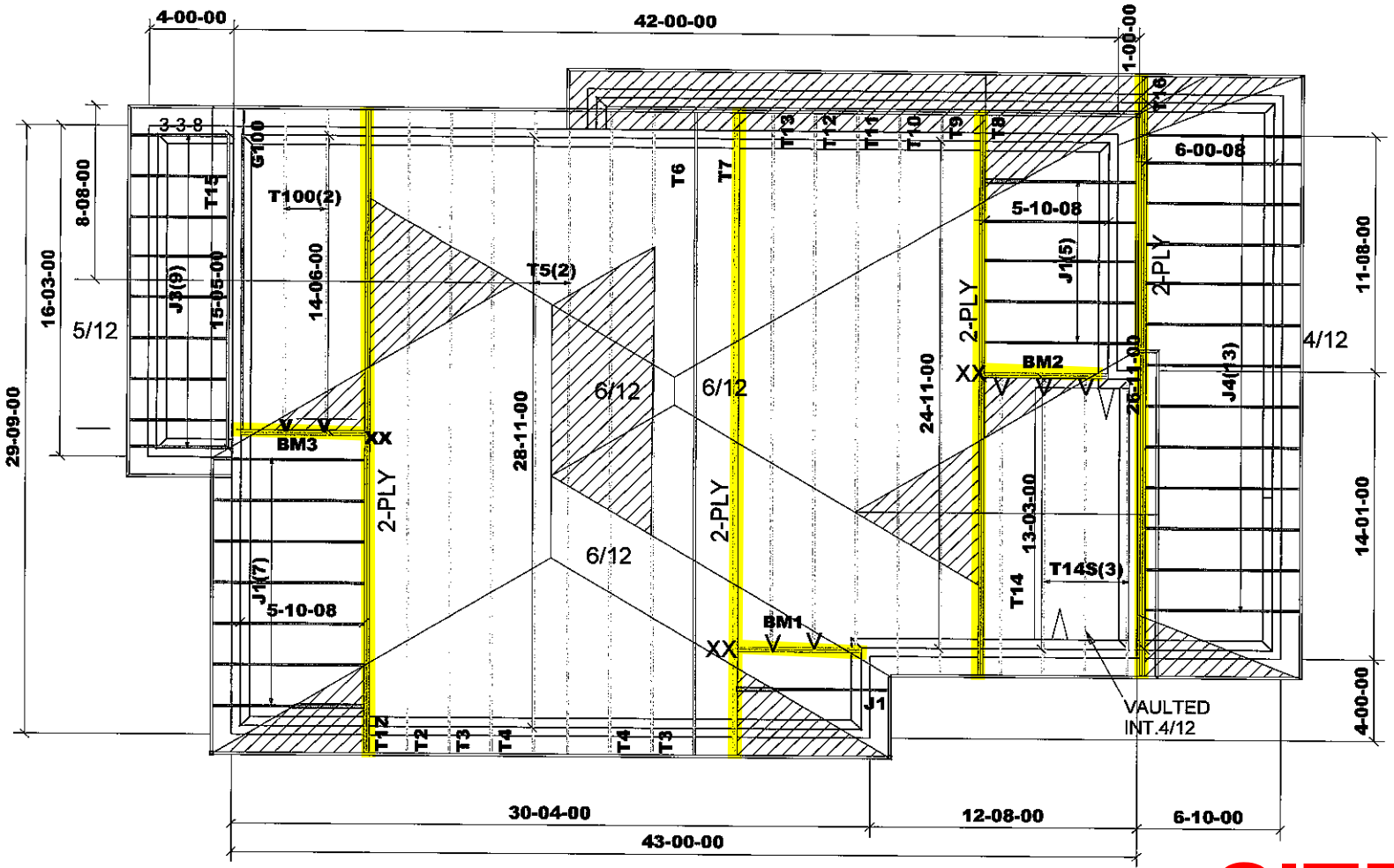
Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD
Project: GREEN VALLEY EAST
Date: 2/23/2018 Designer: JG

Model / Elevation: S38-1 / A - OPT.COFF

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T- 180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,445

TAMARACK LUMBER INC. — ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	295156
Plan Log:	96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Model / Elevation:
S38-1 / A - REAR UPG.

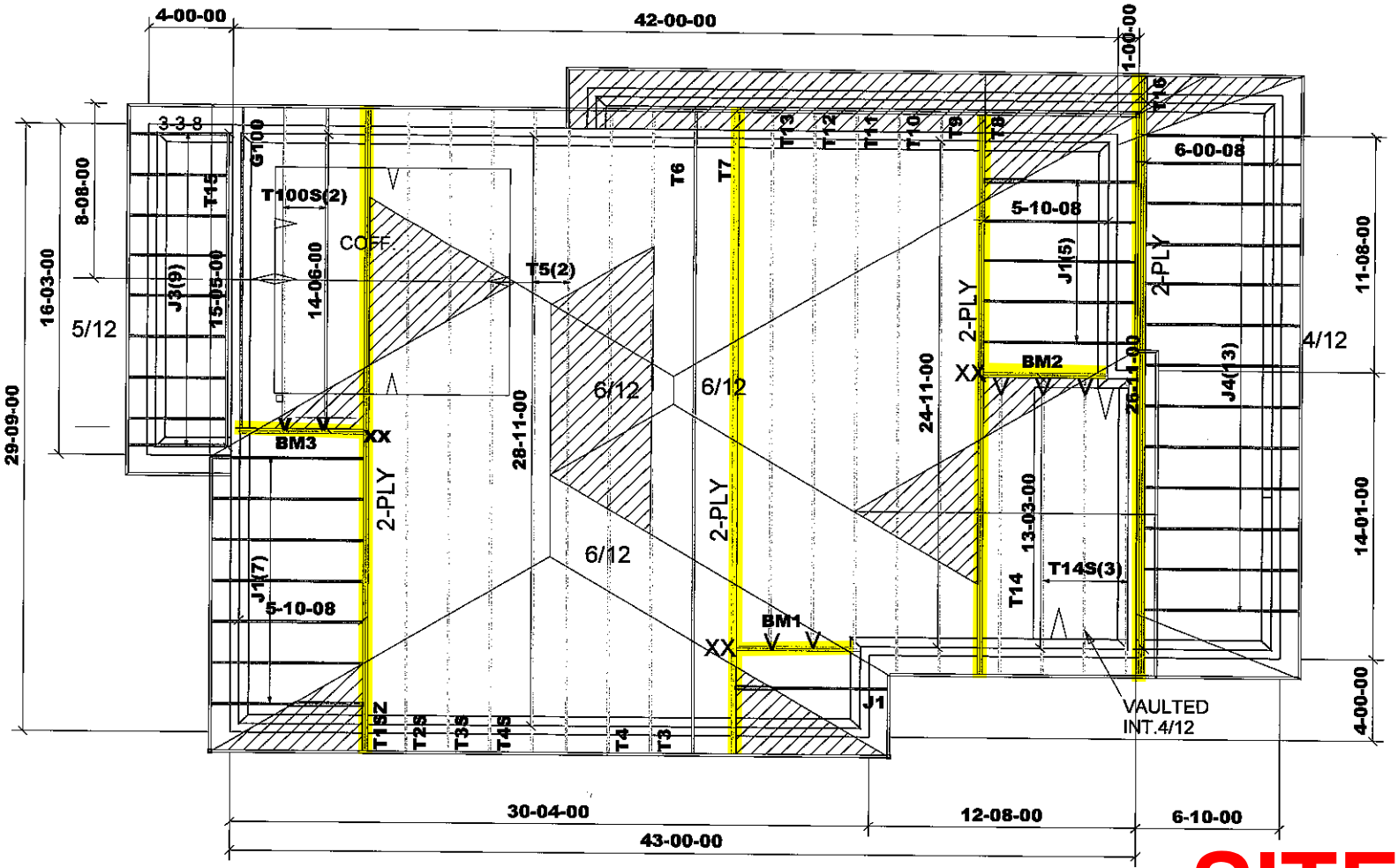
Project: **GREEN VALLEY EAST**

Date: 2/23/2018 Designer: JG

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T- 180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,445

TAMARACK LUMBER INC.

Job Track:	44755
Layout ID:	295159
Plan Log:	96660

Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

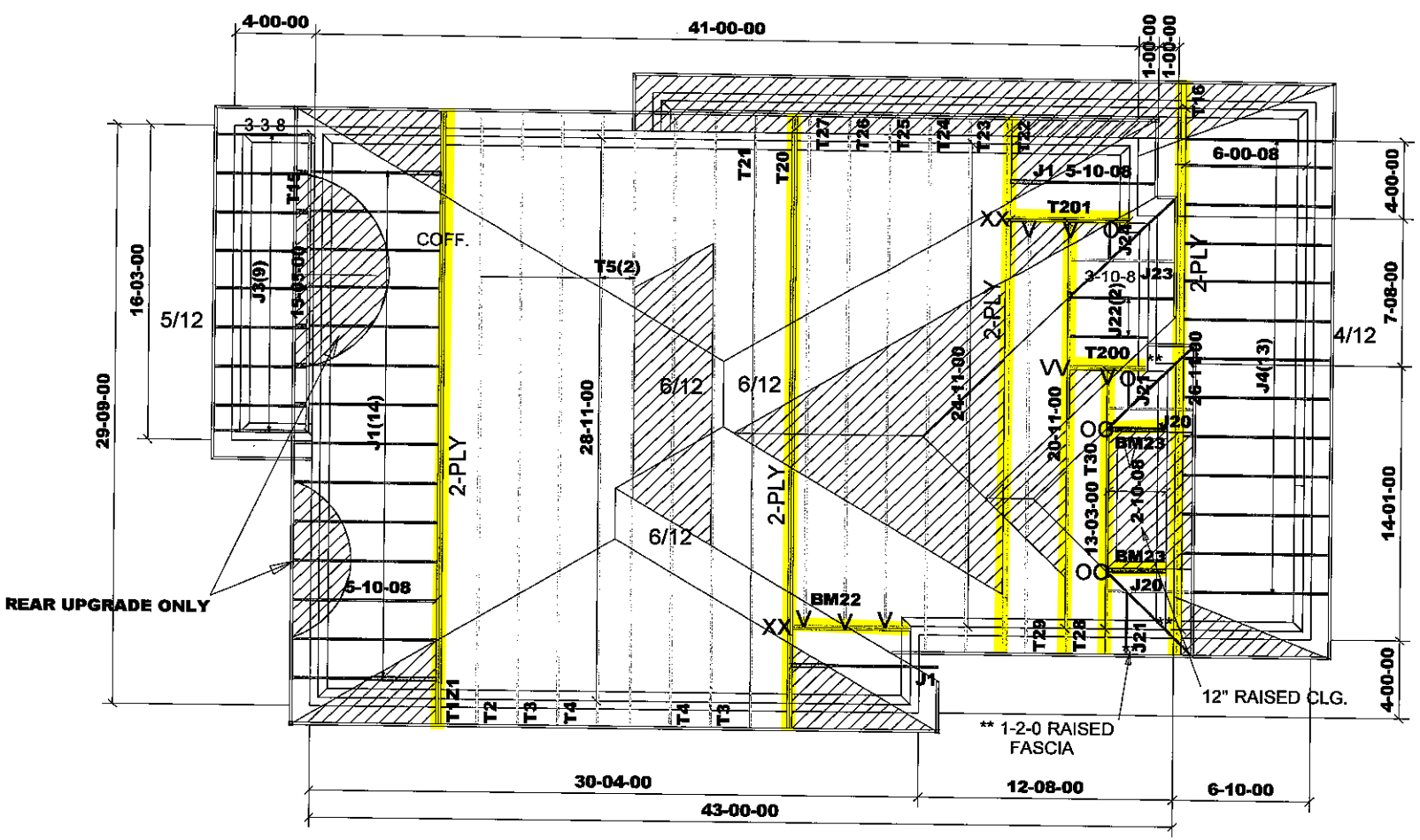
Date: 2/23/2018 Designer: JG

Model / Elevation: S38-1 / A - OPT.COFF.REAR UPG.

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-18010



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
LUS26-2(VV)
LUS24-2(OO)

SITE COPY

M11,445

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 295157
Plan Log: 96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

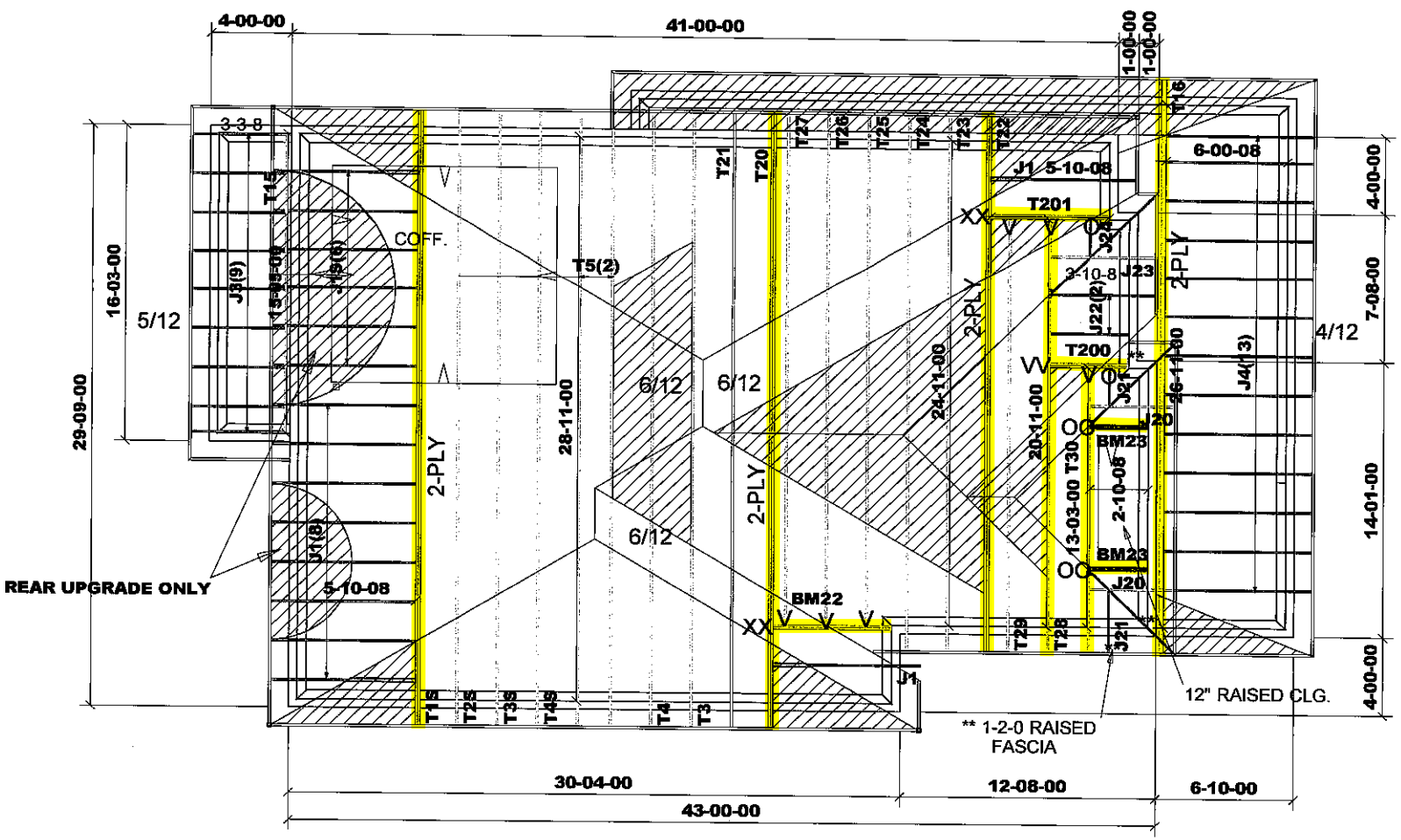
Date: 2/23/2018 Designer: JG

Model / Elevation:
S38-1 / B -STD

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-18010



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
LUS26-2(VV)
LUS24-2(OO)

SITE COPY

M11,445

TAMARACK LUMBER INC. — ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	287392
Plan Log:	96660

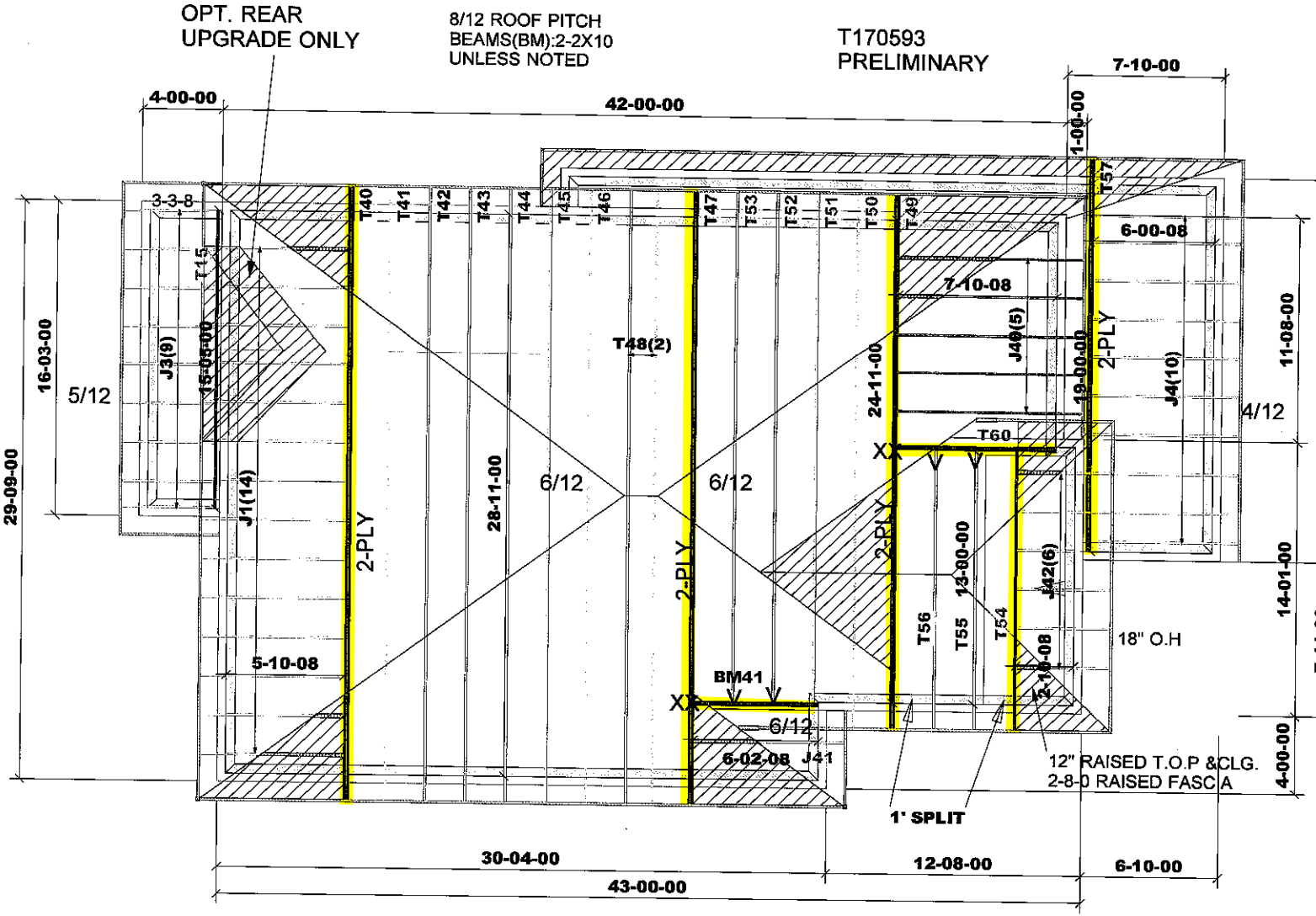
Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: 2/23/2018 Designer: JG

Model / Elevation: **S38-1 / B - OPT.COFF.**

Mitek ver 7.5.0



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,446

TAMARACK TRUSS INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	295162
Plan Log:	93767

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: 2/26/2018 Designer: JG

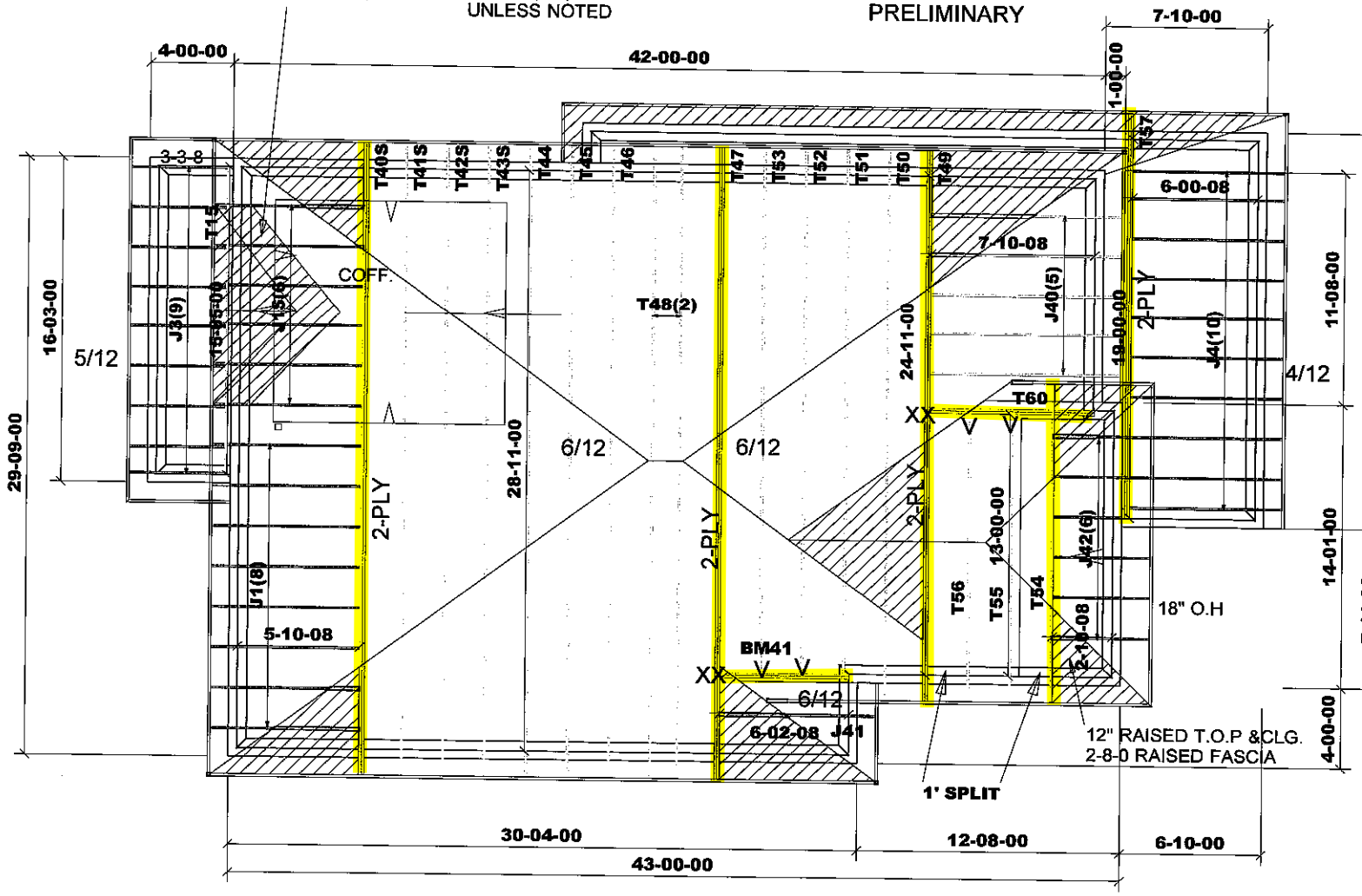
Model / Elevation: **S38-1 / C -STD**

Mitek ver 7.5.0

OPT. REAR
UPGRADE ONLY

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T170593
PRELIMINARY



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9
DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1 kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,445

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	287393
Plan Log:	93767

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Model / Elevation: **S38-1 / C- OPT.COFF.**

Project: **GREEN VALLEY EAST**

Date: 2/26/2018 Designer: JG

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

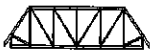
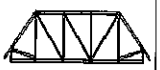
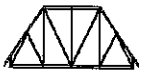
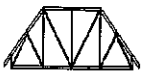
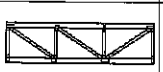
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295155 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z1 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1	T2 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.68 78.17		
	2	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.62 153.34		
	2	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.56 169.66		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T6 ROOF	10.00 0.00	28-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	160.83 101.50		
	1 2 Ply	T7 ROOF	10.00 0.00	28-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.36 200.34		
	1 2 Ply	T8 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	240.28 150.66		
	1	T9 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.11 66.83		
	1	T10 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	111.47 71.33		
	1	T11 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	114.26 71.50		
	1	T12 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	120.24 76.33		
	1	T13 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.51 79.33		
	1	T14 COMMON	10.00 0.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	63.10 40.17		
	3	T14S SCISSORS	10.00 4.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.05 114.51		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	20	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	335.80 213.40		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295155 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-1 **ELEVATION:** A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	J3 JACK-OPEN	5.00 / 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-09 / 02-05-00	91.80 / 65.97		
	13	J4 JACK-OPEN	4.00 / 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-04-02	230.36 / 149.50		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2329.54** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3679.25** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

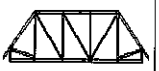
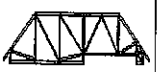
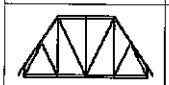
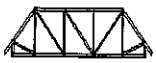
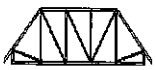
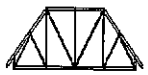
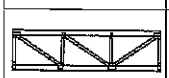
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287391 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-1 **ELEVATION:** A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	295.10 189.66		
	1	T2S HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.63 86.16		
	1	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.81 76.67		
	1	T3S HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	135.24 87.33		
	1	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.78 84.83		
	1	T4S HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 91.50		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T6 ROOF	10.00 0.00	28-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	160.83 101.50		
	1 2 Ply	T7 ROOF	10.00 0.00	28-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.36 200.34		
	1 2 Ply	T8 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	240.28 150.66		
	1	T9 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.11 66.83		
	1	T10 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	111.47 71.33		
	1	T11 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	114.26 71.50		
	1	T12 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	120.24 76.33		
	1	T13 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.51 79.33		
	1	T14 COMMON	10.00 0.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	63.10 40.17		
	3	T14S SCISSORS	10.00 4.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.05 114.51		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287391 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-1 **ELEVATION:** A-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	6	J1S JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	126.06 91.02		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	13	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	230.36 149.50		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2401.52** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3758.56** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

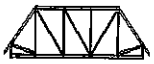
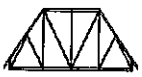
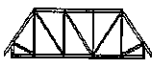
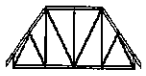
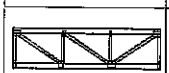
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295156 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1. ELEVATION: A-REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1	T2 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.68 78.17		
	2	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.62 153.34		
	2	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.56 169.66		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T6 ROOF	10.00 0.00	28-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	160.83 101.50		
	1 2 Ply	T7 ROOF	10.00 0.00	28-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.36 200.34		
	1 2 Ply	T8 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	240.28 150.66		
	1	T9 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.11 66.83		
	1	T10 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	111.47 71.33		
	1	T11 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	114.26 71.50		
	1	T12 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	120.24 76.33		
	1	T13 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.51 79.33		
	1	T14 COMMON	10.00 0.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	63.10 40.17		
	3	T14S SCISSORS	10.00 4.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.05 114.51		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	2	T100 COMMON	10.00 0.00	14-06-00	07-08-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	136.20 89.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295156 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-1. **ELEVATION:** A-REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	G100 COMMON	10.00 0.00	14-06-00	07-08-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	71.75 47.67		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	13	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	230.36 149.50		

TOTAL # TRUSS= **64.00** TOTAL BFT OF ALL TRUSSES= **2391.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3769.67** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

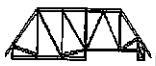
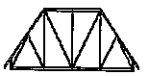
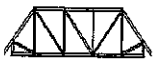
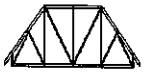
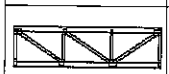
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295159 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: A-OPT.COFF.REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1SZ HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	295.10 189.66		
	1	T2S HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.63 86.16		
	1	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.81 76.67		
	1	T3S HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	135.24 87.33		
	1	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.78 84.83		
	1	T4S HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 91.50		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T6 ROOF	10.00 0.00	28-11-00	10-11-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	160.83 101.50		
	1 2 Ply	T7 ROOF	10.00 0.00	28-11-00	09-11-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	318.36 200.34		
	1 2 Ply	T8 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	240.28 150.66		
	1	T9 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.11 66.83		
	1	T10 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	111.47 71.33		
	1	T11 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	114.26 71.50		
	1	T12 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	120.24 76.33		
	1	T13 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.51 79.33		
	1	T14 COMMON	10.00 0.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	63.10 40.17		
	3	T14S SCISSORS	10.00 4.00	13-03-00	07-01-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	181.05 114.51		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295159 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: A-OPT.COFF.REAR UPG.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	2	T100S ROOF	10.00 0.00	14-06-00	07-08-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	139.88 95.00		
	1	G100 COMMON	10.00 0.00	14-06-00	07-08-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	71.75 47.67		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	13	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	230.36 149.50		

TOTAL # TRUSS= **64.00** TOTAL BFT OF ALL TRUSSES= **2442.50** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3827.34** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= **10.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

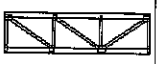
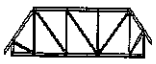
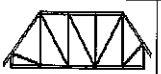
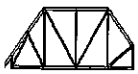
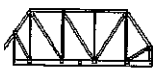
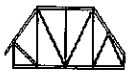
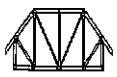
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295157 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T1Z1 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	269.14 170.00		
	1	T2 HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.68 78.17		
	2	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	245.62 153.34		
	2	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	269.56 169.66		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	1 2 Ply	T20 ROOF	10.00 0.00	28-11-00	09-05-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	314.62 197.66		
	1	T21 ROOF	10.00 0.00	28-11-00	10-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	151.46 95.83		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 02-09-11	232.30 145.00		
	1	T23 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-09-11	106.33 68.50		
	1	T24 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-09-11	112.37 71.50		
	1	T25 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	110.44 69.83		
	1	T26 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	119.13 75.50		
	1	T27 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	128.44 81.67		
	1	T28 HIP GIRDER	10.00 0.00	20-11-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	02-09-11 01-07-11	104.57 68.50		
	1	T29 HIP	10.00 0.00	20-11-00	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	02-09-11 01-07-11	96.15 59.83		
	1	T30 HIP GIRDER	10.00 0.00	13-03-00	05-02-07	2 X 4	2 X 4	01-03-08 01-03-08	02-09-11 02-09-11	68.40 44.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

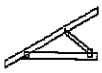
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 295157 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-1 **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T200 JACK-CLOSED	6.00 0.00	03-10-08	03-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 03-01-04	38.92 27.00		
	1 2 Ply	T201 JACK-CLOSED	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-01-04	57.62 37.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	13	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	230.36 149.50		
	2	J20 JACK-OPEN	10.00 0.00	02-10-08	04-03-09	2 X 4	2 X 4	01-03-08 -01-01-01	02-09-11 00-03-08	26.32 17.66		
	2	J21 JACK-OPEN	10.00 0.00	01-10-08	04-03-09	2 X 4	2 X 4	01-03-08 -00-01-01	02-09-11 00-03-08	23.80 16.34		
	2	J22 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	31.14 20.34		
	1	J23 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	12.28 8.33		
	1	J24 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	01-07-11 00-03-08	7.88 5.67		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **2424.69** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3815.05** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
2	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

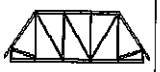
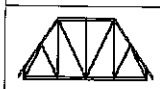
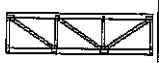
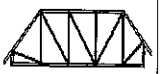
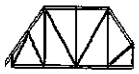
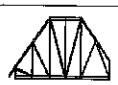
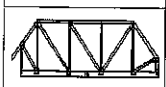
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287392 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: B-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1S HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	293.46 189.34		
	1	T2S HIP	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.63 86.16		
	1	T3 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.81 76.67		
	1	T3S HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	135.24 87.33		
	1	T4 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.78 84.83		
	1	T4S HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.43 91.50		
	2	T5 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	1	T15 FLAT GIRDER	0.00 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 02-05-00	54.90 35.00		
	1 2 Ply	T16 HIP GIRDER	10.00 0.00	26-11-00	02-03-15	2 X 4	2 X 6	01-03-08 01-03-08	01-00-07 01-00-07	257.68 156.66		
	1 2 Ply	T20 ROOF	10.00 0.00	28-11-00	09-05-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	314.62 197.66		
	1	T21 ROOF	10.00 0.00	28-11-00	10-05-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	151.46 95.83		
	1 2 Ply	T22 HIP GIRDER	10.00 0.00	24-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 02-09-11	232.30 145.00		
	1	T23 HIP	10.00 0.00	24-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-09-11	106.33 68.50		
	1	T24 HIP	10.00 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 02-09-11	112.37 71.50		
	1	T25 HIP	10.00 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	110.44 69.83		
	1	T26 HIP	10.00 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	119.13 75.50		
	1	T27 HIP	10.00 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 02-09-11	128.44 81.67		
	1	T28 HIP GIRDER	10.00 0.00	20-11-00	04-10-07	2 X 4	2 X 6	01-03-08 00-00-00	02-09-11 01-07-11	104.57 68.50		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

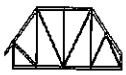
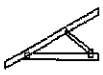
Delivery Shiplist

DATE	02/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287392 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: B-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T29 HIP	10.00 0.00	20-11-00	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	02-09-11 01-07-11	96.15 59.83		
	1	T30 HIP GIRDER	10.00 0.00	13-03-00	05-02-07	2 X 4	2 X 4	01-03-08 01-03-08	02-09-11 02-09-11	68.40 44.17		
	1 2 Ply	T200 JACK-CLOSED	6.00 0.00	03-10-08	03-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 03-01-04	38.92 27.00		
	1 2 Ply	T201 JACK-CLOSED	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 04-01-04	57.62 37.00		
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	6	J1S JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	126.06 91.02		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	13	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	230.36 149.50		
	2	J20 JACK-OPEN	10.00 0.00	02-10-08	04-03-09	2 X 4	2 X 4	01-03-08 -01-01-01	02-09-11 00-03-08	26.32 17.66		
	2	J21 JACK-OPEN	10.00 0.00	01-10-08	04-03-09	2 X 4	2 X 4	01-03-08 -00-01-01	02-09-11 00-03-08	23.80 16.34		
	2	J22 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	31.14 20.34		
	1	J23 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	12.28 8.33		
	1	J24 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	01-07-11 00-03-08	7.88 5.67		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **2496.35** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3892.72** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	
2	Hangers	LUS24	
2	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

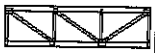
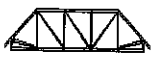
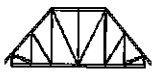
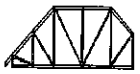
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295162 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T15 FLAT GIRDER	0.00 / 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-05-00 / 02-05-00	54.90 / 35.00		
	1 / 2 Ply	T40 HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	266.02 / 162.66		
	1	T41 HIP	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	115.80 / 72.67		
	1	T42 HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	126.00 / 79.33		
	1	T43 HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	127.76 / 80.50		
	1	T44 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	134.32 / 83.33		
	1	T45 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	131.53 / 81.34		
	1	T46 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	133.82 / 83.67		
	1 / 2 Ply	T47 HIP GIRDER	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	328.94 / 204.00		
	2	T48 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	269.12 / 167.34		
	1 / 2 Ply	T49 HIP GIRDER	8.00 / 0.00	24-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 04-00-13	257.74 / 161.66		
	1	T50 HIP	8.00 / 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 04-00-13	115.20 / 72.83		
	1	T51 HIP	8.00 / 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	112.64 / 70.67		
	1	T52 HIP	8.00 / 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	122.98 / 77.17		
	1	T53 HIP	8.00 / 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	119.77 / 74.67		
	1	T54 HIP GIRDER	8.00 / 0.00	13-03-00	05-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-00-13 / 03-00-13	68.97 / 44.50		
	1	T55 HIP	8.00 / 0.00	13-00-00	07-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	04-00-13 / 04-02-13	70.33 / 45.67		
	1	T56 COMMON	8.00 / 0.00	13-00-00	08-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	04-00-13 / 04-02-13	70.63 / 44.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

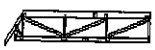
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 295162 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T57 HALF HIP	11.00 0.00	19-00-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-01-14 02-04-02	159.08 98.00		
	1 2 Ply	T60 FLAT GIRDER	0.00 0.00	07-10-08	01-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-00-00 01-00-00	50.98 35.00		
	14	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	10	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	177.20 115.00		
	5	J40 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	151.10 96.65		
	1	J41 JACK-OPEN	6.00 0.00	06-02-08	04-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-03-04	17.60 11.33		
	6	J42 JACK-OPEN	8.00 0.00	02-10-08	05-03-13	2 X 4	2 X 4	01-09-08 00-00-00	03-04-13 05-03-13	96.60 60.00		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2272.34** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3605.89** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

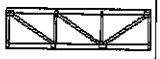
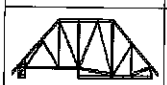
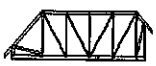
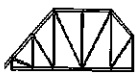
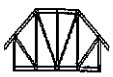
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287393 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: C -OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T15 FLAT GIRDER	0.00 / 0.00	15-05-00	02-05-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-05-00 / 02-05-00	54.90 / 35.00		
	1 / 2 Ply	T40S HIP GIRDER	8.00 / 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	314.32 / 195.00		
	1	T41S HIP	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	126.53 / 80.83		
	1	T42S HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	132.32 / 85.17		
	1	T43S HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	141.44 / 91.00		
	1	T44 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	134.32 / 83.33		
	1	T45 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	131.53 / 81.34		
	1	T46 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	133.82 / 83.67		
	1 / 2 Ply	T47 HIP GIRDER	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	328.94 / 204.00		
	2	T48 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	269.12 / 167.34		
	1 / 2 Ply	T49 HIP GIRDER	8.00 / 0.00	24-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 04-00-13	257.74 / 161.66		
	1	T50 HIP	8.00 / 0.00	24-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 04-00-13	115.20 / 72.83		
	1	T51 HIP	8.00 / 0.00	24-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	112.64 / 70.67		
	1	T52 HIP	8.00 / 0.00	24-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	122.98 / 77.17		
	1	T53 HIP	8.00 / 0.00	24-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-00-13	119.77 / 74.67		
	1	T54 HIP GIRDER	8.00 / 0.00	13-03-00	05-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	03-00-13 / 03-00-13	68.97 / 44.50		
	1	T55 HIP	8.00 / 0.00	13-00-00	07-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	04-00-13 / 04-02-13	70.33 / 45.67		
	1	T56 COMMON	8.00 / 0.00	13-00-00	08-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	04-00-13 / 04-02-13	70.63 / 44.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

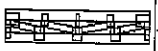
Delivery Shiplist

DATE	02/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287393 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-1 ELEVATION: C -OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T57 HALF HIP	11.00 0.00	19-00-00	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-01-14 02-04-02	159.08 98.00		
	1 2 Ply	T60 FLAT GIRDER	0.00 0.00	07-10-08	01-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-00-00 01-00-00	50.98 35.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	6	J1S JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-01-04	124.08 91.98		
	9	J3 JACK-OPEN	5.00 0.00	03-03-08	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-00-09 02-05-00	91.80 65.97		
	10	J4 JACK-OPEN	4.00 0.00	06-00-08	02-04-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-04-02	177.20 115.00		
	5	J40 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	151.10 96.65		
	1	J41 JACK-OPEN	6.00 0.00	06-02-08	04-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-03-04	17.60 11.33		
	6	J42 JACK-OPEN	8.00 0.00	02-10-08	05-03-13	2 X 4	2 X 4	01-09-08 00-00-00	03-04-13 05-03-13	96.60 60.00		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2357.14** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3708.26** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= **6.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295156	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 26 09:49:11 2018 Page 1

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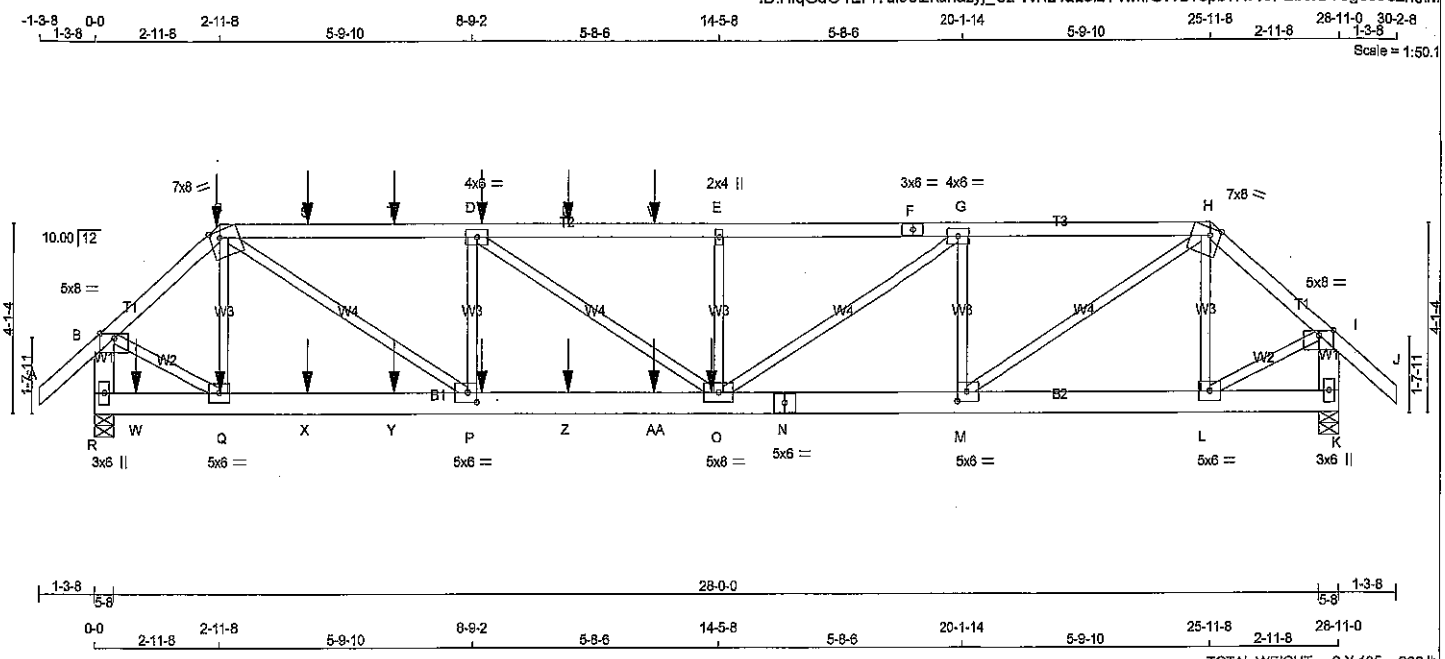

TOTAL WEIGHT = 2 X 135 = 268 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- O	1	6	SIDE(205.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	2.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3758	0	3758	0	0	5-8	5-8
K	3108	0	3108	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2985	1845 / 0	580 / 0	0 / 0	0 / 0	560 / 0	0 / 0
K	2443	1556 / 0	446 / 0	0 / 0	0 / 0	441 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-3624 / 0	-104.9	-104.9	0.14 (1)	4.83	
C- S	-6364 / 0	-104.9	-104.9	0.74 (1)	3.21	
S- T	-6364 / 0	-104.9	-104.9	0.74 (1)	3.21	
T- D	-6364 / 0	-104.9	-104.9	0.74 (1)	3.21	
D- U	-7791 / 0	-104.9	-104.9	0.87 (1)	2.79	
U- V	-7791 / 0	-104.9	-104.9	0.87 (1)	2.79	
V- E	-7791 / 0	-104.9	-104.9	0.87 (1)	2.79	
E- F	-7791 / 0	-104.9	-104.9	0.64 (1)	3.09	
F- G	-7791 / 0	-104.9	-104.9	0.64 (1)	3.09	
G- H	-5678 / 0	-104.9	-104.9	0.48 (1)	3.72	
H- I	-2958 / 0	-104.9	-104.9	0.12 (1)	5.25	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
R- B	-3740 / 0	0.0	0.0	0.14 (1)	7.29	
K- I	-3108 / 0	0.0	0.0	0.11 (1)	7.81	
R- W	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
W- Q	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
Q- X	0 / 2751	-28.0	-28.0	0.24 (1)	10.00	
X- Y	0 / 2751	-28.0	-28.0	0.24 (1)	10.00	
Y- P	0 / 2751	-28.0	-28.0	0.24 (1)	10.00	
P- Z	0 / 6365	-28.0	-28.0	0.51 (1)	10.00	
Z-AA	0 / 6365	-28.0	-28.0	0.51 (1)	10.00	
AA- O	0 / 6365	-28.0	-28.0	0.51 (1)	10.00	
O- N	0 / 5678	-28.0	-28.0	0.44 (1)	10.00	
N- M	0 / 5678	-28.0	-28.0	0.44 (1)	10.00	
M- L	0 / 2244	-28.0	-28.0	0.19 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.05 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-815 / 0	0.10 (1)
C- P	0 / 4286	0.53 (1)
P- D	-2031 / 0	0.26 (1)
D- O	0 / 1705	0.21 (1)
O- E	-672 / 0	0.09 (1)
O- G	0 / 2526	0.31 (1)
M- G	-2037 / 0	0.26 (1)
M- H	0 / 4075	0.50 (1)
L- H	-714 / 0	0.09 (1)
B- Q	0 / 2987	0.37 (1)
L- I	0 / 2438	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-201	-201	---	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	---	BACK	VERT	TOTAL
D	8-11-12	-126	-126	---	BACK	VERT	TOTAL
O	14-3-8	-1490	-1490	---	BACK	VERT	TOTAL
P	8-11-12	-40	-70	---	BACK	VERT	TOTAL
Q	2-11-12	-40	-70	---	BACK	VERT	TOTAL
S	4-11-12	-126	-126	---	BACK	VERT	TOTAL
T	6-11-12	-126	-126	---	BACK	VERT	TOTAL
U	10-11-12	-126	-126	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.87/1.00 (D-E:1) , BC=0.51/1.00 (O-P:1) , WB=0.53/1.00 (C-P:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (P) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.7 lbs FACTORED DOWN AT 2-11-8, 130.0 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 4-11-12, 126.3 lbs FACTORED DOWN AT 6-11-12, 126.3 lbs FACTORED DOWN AT 8-11-12, AND 126.3 lbs FACTORED DOWN AT 10-11-12, AND 126.3 lbs FACTORED DOWN AT 12-11-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 11-12, 69.9 lbs FACTORED DOWN AT 2-11-12, 69.9 lbs FACTORED DOWN AT 4-11-12, 69.9 lbs FACTORED DOWN AT 6-11-12, 69.9 lbs FACTORED DOWN AT 8-11-12, 69.9 lbs FACTORED DOWN AT 10-11-12, AND 69.9 lbs FACTORED DOWN AT 12-11-12, AND 1489.6 lbs FACTORED DOWN AT 14-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	12-11-12	-126	-126	---	BACK	VERT	TOTAL
W	11-12	-40	-70	---	BACK	VERT	TOTAL
X	4-11-12	-40	-70	---	BACK	VERT	TOTAL
Y	6-11-12	-40	-70	---	BACK	VERT	TOTAL
Z	10-11-12	-40	-70	---	BACK	VERT	TOTAL
AA	12-11-12	-40	-70	---	BACK	VERT	TOTAL

SITE COPY

DWG NO. TAM 1026378
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T1Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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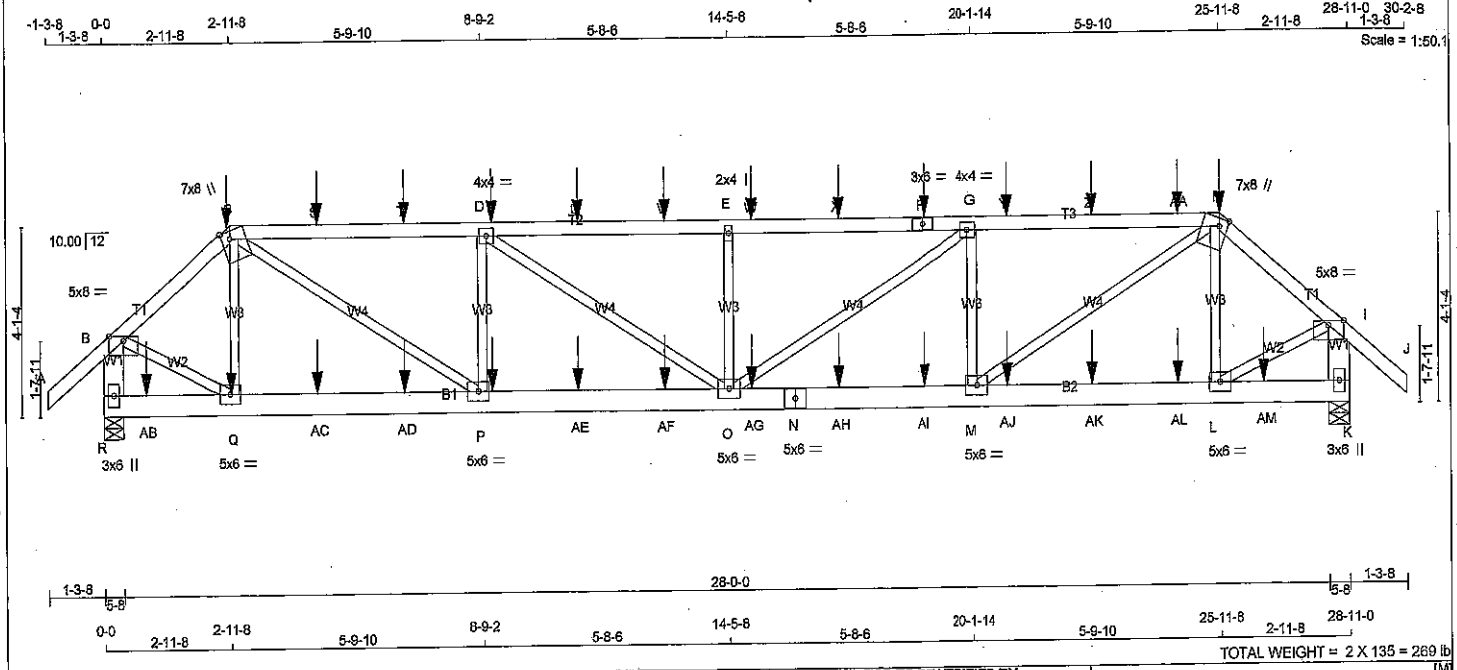


TOTAL WEIGHT = 2 X 135 = 269 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	SIDE(61.0)
C- F 1	12	SIDE(61.0)
F- H 1	12	SIDE(61.0)
H- J 1	12	SIDE(61.0)
R- B 2	12	TOP
K- I 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
R- N 2	12	SIDE(183.1)
N- K 2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3345	0	3345	0	0	5-8	5-8
K	3271	0	3271	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2685	1613 / 0	552 / 0	0 / 0	0 / 0	520 / 0	0 / 0
K	2623	1580 / 0	538 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)		10.00
B- C	-3187 / 0	-104.9	-104.9	0.13 (1)		5.10
C- S	-5387 / 0	-104.9	-104.9	0.64 (1)		3.56
S- T	-5387 / 0	-104.9	-104.9	0.64 (1)		3.56
T- D	-5387 / 0	-104.9	-104.9	0.64 (1)		3.56
D- U	-6250 / 0	-104.9	-104.9	0.70 (1)		3.27
U- V	-6250 / 0	-104.9	-104.9	0.70 (1)		3.27
V- E	-6250 / 0	-104.9	-104.9	0.70 (1)		3.27
E- W	-6250 / 0	-104.9	-104.9	0.74 (1)		3.21
W- X	-6250 / 0	-104.9	-104.9	0.74 (1)		3.21
X- F	-6250 / 0	-104.9	-104.9	0.74 (1)		3.21
F- G	-6250 / 0	-104.9	-104.9	0.74 (1)		3.21
G- Y	-5369 / 0	-104.9	-104.9	0.68 (1)		3.52
Y- Z	-5369 / 0	-104.9	-104.9	0.68 (1)		3.52
Z-AA	-5369 / 0	-104.9	-104.9	0.68 (1)		3.52
AA- H	-5369 / 0	-104.9	-104.9	0.68 (1)		3.52
H- I	-3125 / 0	-104.9	-104.9	0.12 (1)		5.13
I- J	0 / 47	-104.9	-104.9	0.08 (1)		10.00
R- B	-3325 / 0	0.0	0.0	0.12 (1)		7.63
K- I	-3266 / 0	0.0	0.0	0.12 (1)		7.68
R-AB	0 / 0	-28.0	-28.0	0.07 (2)		10.00
AB- Q	0 / 0	-28.0	-28.0	0.07 (2)		10.00
Q-AC	0 / 2420	-28.0	-28.0	0.21 (1)		10.00
AC-AD	0 / 2420	-28.0	-28.0	0.21 (1)		10.00
AD- P	0 / 2420	-28.0	-28.0	0.21 (1)		10.00
P-AE	0 / 5387	-28.0	-28.0	0.42 (1)		10.00
AE-AF	0 / 5387	-28.0	-28.0	0.42 (1)		10.00
AF- O	0 / 5387	-28.0	-28.0	0.42 (1)		10.00
O-AG	0 / 5370	-28.0	-28.0	0.42 (1)		10.00
AG- N	0 / 5370	-28.0	-28.0	0.42 (1)		10.00
N-AH	0 / 5370	-28.0	-28.0	0.42 (1)		10.00
AH-AI	0 / 5370	-28.0	-28.0	0.42 (1)		10.00
AI- M	0 / 5370	-28.0	-28.0	0.42 (1)		10.00
M-AJ	0 / 2373	-28.0	-28.0	0.21 (1)		10.00
AJ-AK	0 / 2373	-28.0	-28.0	0.21 (1)		10.00
AK-AL	0 / 2373	-28.0	-28.0	0.21 (1)		10.00
AL- L	0 / 2373	-28.0	-28.0	0.21 (1)		10.00
L-AM	0 / 0	-28.0	-28.0	0.09 (2)		10.00
AM- K	0 / 0	-28.0	-28.0	0.08 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-683 / 0	0.09 (1)
C- P	0 / 3521	0.44 (1)
P- D	-1643 / 0	0.21 (1)
D- O	0 / 1031	0.13 (1)
O- E	-878 / 0	0.11 (1)
O- G	0 / 1052	0.13 (1)
M- G	-1662 / 0	0.21 (1)
M- H	0 / 3556	0.44 (1)
L- H	-669 / 0	0.08 (1)
B- Q	0 / 2626	0.33 (1)
L- I	0 / 2575	0.32 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.74/1.00 (E-G:1) , BC=0.42/1.00 (M-O:1) , WB=0.44/1.00 (H-M:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

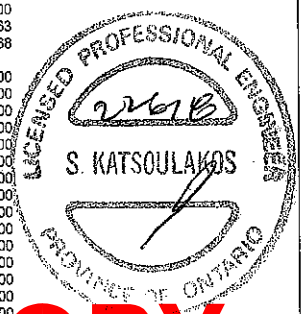
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (P) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)



DWG NO. TAM 10181 -B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T1Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.7 lbs FACTORED DOWN AT 2-11-8, 200.7 lbs FACTORED DOWN AT 25-11-8, 130.0 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 4-11-12, 126.3 lbs FACTORED DOWN AT 6-11-12, 126.3 lbs FACTORED DOWN AT 8-11-12, 126.3 lbs FACTORED DOWN AT 10-11-12, 126.3 lbs FACTORED DOWN AT 12-11-12, 126.3 lbs FACTORED DOWN AT 14-11-12, 126.3 lbs FACTORED DOWN AT 16-11-12, 126.3 lbs FACTORED DOWN AT 18-11-12, 126.3 lbs FACTORED DOWN AT 20-11-12, AND 126.3 lbs FACTORED DOWN AT 22-11-12, AND 127.3 lbs FACTORED DOWN AT 24-11-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 11-12, 69.9 lbs FACTORED DOWN AT 2-11-12, 69.9 lbs FACTORED DOWN AT 4-11-12, 69.9 lbs FACTORED DOWN AT 6-11-12, 69.9 lbs FACTORED DOWN AT 8-11-12, 69.9 lbs FACTORED DOWN AT 10-11-12, 69.9 lbs FACTORED DOWN AT 12-11-12, 69.9 lbs FACTORED DOWN AT 14-11-12, 69.9 lbs FACTORED DOWN AT 16-11-12, 69.9 lbs FACTORED DOWN AT 18-11-12, 69.9 lbs FACTORED DOWN AT 20-11-12, 69.9 lbs FACTORED DOWN AT 22-11-12, AND 69.9 lbs FACTORED DOWN AT 24-11-12, AND 69.9 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-201	-201	—	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	—	BACK	VERT	TOTAL
D	8-11-12	-126	-126	—	BACK	VERT	TOTAL
F	18-11-12	-126	-126	—	BACK	VERT	TOTAL
H	25-11-8	-201	-201	—	FRONT	VERT	TOTAL
P	8-11-12	-40	-70	—	BACK	VERT	TOTAL
Q	2-11-12	-40	-70	—	BACK	VERT	TOTAL
S	4-11-12	-126	-126	—	BACK	VERT	TOTAL
T	6-11-12	-126	-126	—	BACK	VERT	TOTAL
U	10-11-12	-126	-126	—	BACK	VERT	TOTAL
V	12-11-12	-126	-126	—	BACK	VERT	TOTAL
W	14-11-12	-126	-126	—	BACK	VERT	TOTAL
X	16-11-12	-126	-126	—	BACK	VERT	TOTAL
Y	20-11-12	-126	-126	—	BACK	VERT	TOTAL
Z	22-11-12	-126	-126	—	BACK	VERT	TOTAL
AA	24-11-12	-127	-127	—	BACK	VERT	TOTAL
AB	11-12	-40	-70	—	BACK	VERT	TOTAL
AC	4-11-12	-40	-70	—	BACK	VERT	TOTAL
AD	6-11-12	-40	-70	—	BACK	VERT	TOTAL
AE	10-11-12	-40	-70	—	BACK	VERT	TOTAL
AF	12-11-12	-40	-70	—	BACK	VERT	TOTAL
AG	14-11-12	-40	-70	—	BACK	VERT	TOTAL
AH	16-11-12	-40	-70	—	BACK	VERT	TOTAL
AI	18-11-12	-40	-70	—	BACK	VERT	TOTAL
AJ	20-11-12	-40	-70	—	BACK	VERT	TOTAL
AK	22-11-12	-40	-70	—	BACK	VERT	TOTAL
AL	24-11-12	-40	-70	—	BACK	VERT	TOTAL
AM	26-11-12	-40	-70	—	BACK	VERT	TOTAL

DWG NO. TAM 10181-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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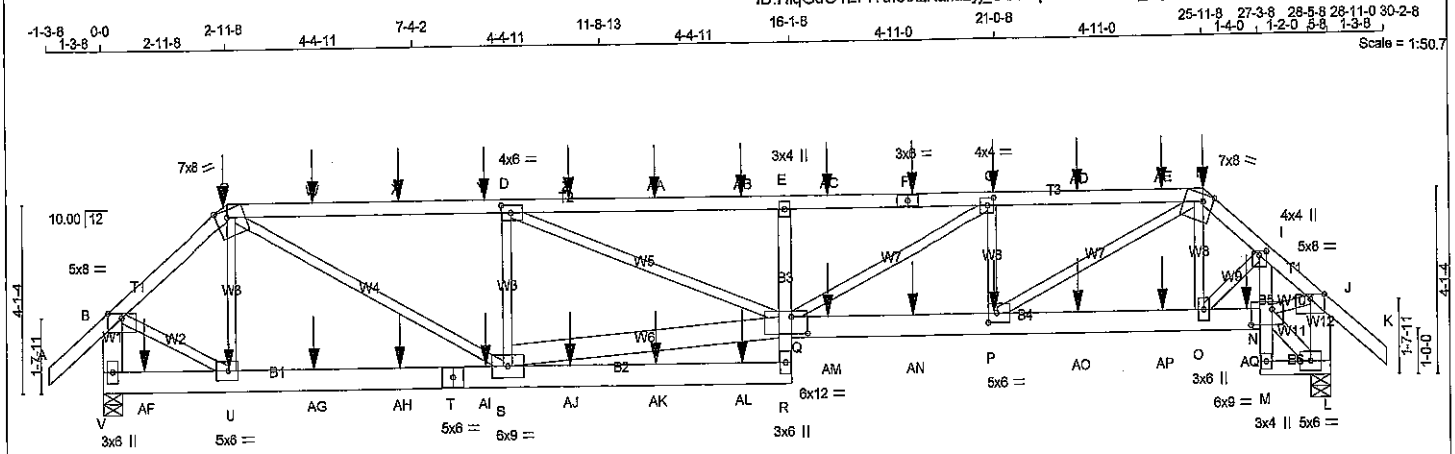

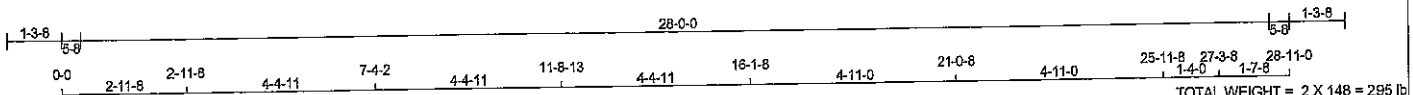

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- K	1	12	SIDE(61.0)
V- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(0.0)
Q- N	2	12	SIDE(183.1)
R- E	1	12	TOP
I- M	1	12	TOP
M- L	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	6.0	2.00	2.75
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTWW-m	MT20	7.0	8.0	Edge	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3441	0	3441	0	0	5-8	5-8
L	3618	0	3618	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2760	1660 / 0	566 / 0	0 / 0	0 / 0	533 / 0	0 / 0
L	2893	1758 / 0	583 / 0	0 / 0	0 / 0	552 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 47	-104.9	-104.9	0.07 (1)	10.00		U- C	-707 / 0	0.09 (1)
B- C	-3294 / 0	-104.9	-104.9	0.09 (1)	5.65		C- S	0 / 3923	0.49 (1)
C- W	-5900 / 0	-104.9	-104.9	0.69 (1)	3.85		S- D	-2507 / 0	0.32 (1)
W- X	-5900 / 0	-104.9	-104.9	0.69 (1)	3.85		S- Q	0 / 5712	0.37 (1)
X- Y	-5900 / 0	-104.9	-104.9	0.69 (1)	3.85		D- Q	0 / 3214	0.40 (1)
Y- D	-5900 / 0	-104.9	-104.9	0.69 (1)	3.85		Q- G	0 / 1778	0.22 (1)
D- Z	-8936 / 0	-104.9	-104.9	0.88 (1)	3.09		P- G	-1738 / 0	0.16 (1)
Z-AA	-8936 / 0	-104.9	-104.9	0.88 (1)	3.09		P- H	0 / 4404	0.55 (1)
AA-AB	-8936 / 0	-104.9	-104.9	0.88 (1)	3.09		O- H	0 / 334	0.04 (2)
AB- E	-8936 / 0	-104.9	-104.9	0.88 (1)	3.09		O- I	-32 / 0	0.00 (1)
E-AC	-9055 / 0	-104.9	-104.9	0.42 (1)	3.50		B- U	0 / 2715	0.34 (1)
AC- F	-9055 / 0	-104.9	-104.9	0.42 (1)	3.50		N- L	-137 / 0	0.01 (1)
F- G	-9055 / 0	-104.9	-104.9	0.42 (1)	3.50		N- J	0 / 3543	0.44 (1)
G-AD	-7497 / 0	-104.9	-104.9	0.37 (1)	3.85				
AD-AE	-7497 / 0	-104.9	-104.9	0.37 (1)	3.85				
AE- H	-7497 / 0	-104.9	-104.9	0.37 (1)	3.85				
H- I	-4692 / 0	-104.9	-104.9	0.09 (1)	4.94				
I- J	-4691 / 0	-104.9	-104.9	0.09 (1)	4.94				
J- K	0 / 47	-104.9	-104.9	0.07 (1)	10.00				
V- B	-3427 / 0	0.0	0.0	0.13 (1)	7.54				
L- J	-3511 / 0	0.0	0.0	0.13 (1)	7.47				
V-AF	0 / 0	-28.0	-28.0	0.06 (2)	10.00				
AF- U	0 / 0	-28.0	-28.0	0.06 (2)	10.00				
U-AG	0 / 2501	-28.0	-28.0	0.23 (1)	10.00				
AG-AH	0 / 2501	-28.0	-28.0	0.23 (1)	10.00				
AH- T	0 / 2501	-28.0	-28.0	0.23 (1)	10.00				
T-AI	0 / 2501	-28.0	-28.0	0.23 (1)	10.00				
AI- S	0 / 2501	-28.0	-28.0	0.23 (1)	10.00				
S-AJ	0 / 326	-28.0	-28.0	0.14 (2)	10.00				
AJ-AK	0 / 326	-28.0	-28.0	0.14 (2)	10.00				
AK-AL	0 / 326	-28.0	-28.0	0.14 (2)	10.00				
AL- R	0 / 326	-28.0	-28.0	0.14 (2)	10.00				
R- Q	0 / 230	0.0	0.0	0.26 (1)	10.00				
Q- E	-879 / 0	0.0	0.0	0.25 (1)	7.81				
Q-AM	0 / 7498	-28.0	-28.0	0.60 (1)	10.00				
AM-AN	0 / 7498	-28.0	-28.0	0.60 (1)	10.00				
AN- P	0 / 7498	-28.0	-28.0	0.60 (1)	10.00				
P-AO	0 / 3621	-28.0	-28.0	0.35 (1)	10.00				
AO-AP	0 / 3621	-28.0	-28.0	0.35 (1)	10.00				
AP- O	0 / 3621	-28.0	-28.0	0.35 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 777 (0.45")

CSI: TC=0.88/1.00 (D-E:1) , BC=0.60/1.00 (P-Q:1) , WB=0.55/1.00 (H-P:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 2268 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM10198-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	T1S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	4.00	8.00
O	BMWW+t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BVMWWW-I	MT20	6.0	12.0	4.50	4.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.0 lbs FACTORED DOWN AT 2-11-8, 192.0 lbs FACTORED DOWN AT 25-11-8, 130.0 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 4-11-12, 126.3 lbs FACTORED DOWN AT 6-11-12, 126.3 lbs FACTORED DOWN AT 8-11-12, 126.3 lbs FACTORED DOWN AT 10-11-12, 126.3 lbs FACTORED DOWN AT 12-11-12, 126.3 lbs FACTORED DOWN AT 14-11-12, 122.5 lbs FACTORED DOWN AT 16-11-12, 122.5 lbs FACTORED DOWN AT 18-11-12, 122.5 lbs FACTORED DOWN AT 20-11-12, AND 122.5 lbs FACTORED DOWN AT 22-11-12, AND 123.5 lbs FACTORED DOWN AT 24-11-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 11-12, 69.9 lbs FACTORED DOWN AT 2-11-12, 69.9 lbs FACTORED DOWN AT 4-11-12, 69.9 lbs FACTORED DOWN AT 6-11-12, 69.9 lbs FACTORED DOWN AT 8-11-12, 69.9 lbs FACTORED DOWN AT 10-11-12, 69.9 lbs FACTORED DOWN AT 12-11-12, 69.9 lbs FACTORED DOWN AT 14-11-12, 118.6 lbs FACTORED DOWN AT 16-11-12, 118.6 lbs FACTORED DOWN AT 18-11-12, 118.6 lbs FACTORED DOWN AT 20-11-12, 118.6 lbs FACTORED DOWN AT 22-11-12, AND 118.6 lbs FACTORED DOWN AT 24-11-12, AND 126.2 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	FACTORED FORCE (LBS)	MAX CSI (LC)
O-AQ	0 / 3633	-28.0	-28.0	0.35 (1)	10.00		
AQ- N	0 / 3633	-28.0	-28.0	0.35 (1)	10.00		
M- N	0 / 36	0.0	0.0	0.10 (1)	10.00		
N- I	-48 / 23	0.0	0.0	0.10 (1)	7.81		
M- L	0 / 105	-28.0	-28.0	0.02 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-192	-192	—	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	—	BACK	VERT	TOTAL
F	18-11-12	-123	-123	—	BACK	VERT	TOTAL
G	20-11-12	-123	-123	—	BACK	VERT	TOTAL
H	25-11-8	-192	-192	—	FRONT	VERT	TOTAL
P	20-11-12	-119	-119	—	BACK	VERT	TOTAL
U	2-11-12	-40	-70	—	BACK	VERT	TOTAL
W	4-11-12	-126	-126	—	BACK	VERT	TOTAL
X	6-11-12	-126	-126	—	BACK	VERT	TOTAL
Y	8-11-12	-126	-126	—	BACK	VERT	TOTAL
Z	10-11-12	-126	-126	—	BACK	VERT	TOTAL
AA	12-11-12	-126	-126	—	BACK	VERT	TOTAL
AB	14-11-12	-126	-126	—	BACK	VERT	TOTAL
AC	16-11-12	-123	-123	—	BACK	VERT	TOTAL
AD	22-11-12	-123	-123	—	BACK	VERT	TOTAL
AE	24-11-12	-123	-123	—	BACK	VERT	TOTAL
AF	11-12	-40	-70	—	BACK	VERT	TOTAL
AG	4-11-12	-40	-70	—	BACK	VERT	TOTAL
AH	6-11-12	-40	-70	—	BACK	VERT	TOTAL
AI	8-11-12	-40	-70	—	BACK	VERT	TOTAL
AJ	10-11-12	-40	-70	—	BACK	VERT	TOTAL
AK	12-11-12	-40	-70	—	BACK	VERT	TOTAL
AL	14-11-12	-40	-70	—	BACK	VERT	TOTAL
AM	16-11-12	-119	-119	—	BACK	VERT	TOTAL
AN	18-11-12	-119	-119	—	BACK	VERT	TOTAL
AO	22-11-12	-119	-119	—	BACK	VERT	TOTAL
AP	24-11-12	-119	-119	—	BACK	VERT	TOTAL
AQ	26-11-12	-126	-126	—	BACK	VERT	TOTAL

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10198-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295159	T1SZ	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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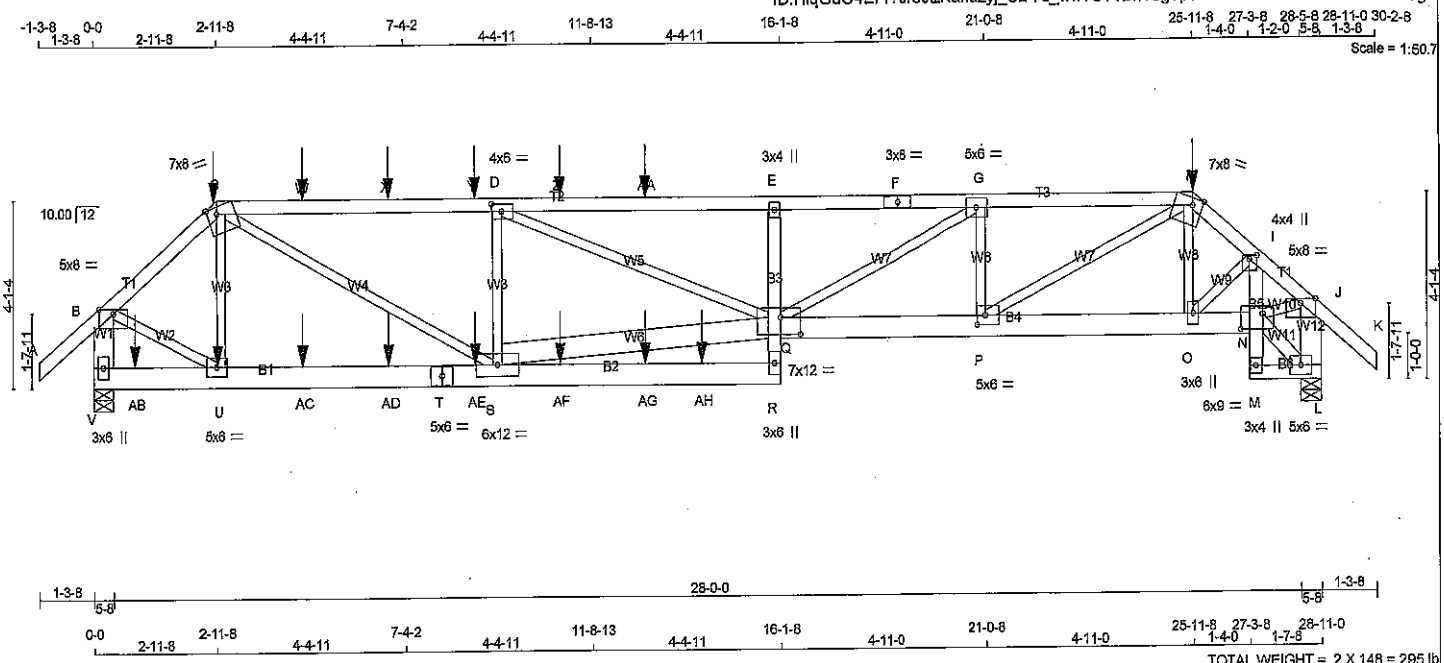


TOTAL WEIGHT = 2 X 148 = 295 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - K	2x4	DRY	2100F 1.8E	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(65.9)
C- F	1	12	SIDE(65.9)
F- H	1	12	SIDE(65.9)
H- K	1	12	SIDE(65.9)
V- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(0.0)
Q- N	2	12	TOP
R- E	1	12	TOP
I- M	1	12	TOP
M- L	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	6.0	2.00	2.75
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3773	0	3773	0	0	5-8	5-8
L	3276	0	3276	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2998	1853 / 0	583 / 0	0 / 0	0 / 0	562 / 0	0 / 0
L	2577	1638 / 0	472 / 0	0 / 0	0 / 0	467 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.05 (1)		10.00
B- C	-3584 / 0	-104.9	-104.9	0.07 (1)		5.87
C- W	-6897 / 0	-104.9	-104.9	0.56 (1)		4.06
W- X	-6897 / 0	-104.9	-104.9	0.56 (1)		4.06
X- Y	-6897 / 0	-104.9	-104.9	0.56 (1)		4.06
Y- D	-6897 / 0	-104.9	-104.9	0.56 (1)		4.06
D- Z	-9970 / 0	-104.9	-104.9	0.72 (1)		3.37
Z-AA	-9970 / 0	-104.9	-104.9	0.72 (1)		3.37
AA- E	-9970 / 0	-104.9	-104.9	0.72 (1)		3.37
E- F	-10096 / 0	-104.9	-104.9	0.28 (1)		3.74
F- G	-10096 / 0	-104.9	-104.9	0.28 (1)		3.74
G- H	-7234 / 0	-104.9	-104.9	0.21 (1)		4.36
H- I	-4230 / 0	-104.9	-104.9	0.07 (1)		5.52
I- J	-4216 / 0	-104.9	-104.9	0.07 (1)		5.53
J- K	0 / 47	-104.9	-104.9	0.05 (1)		10.00
V- B	-3702 / 0	0.0	0.0	0.14 (1)		7.32
L- J	-3184 / 0	0.0	0.0	0.12 (1)		7.75
V-AB	0 / 0	-28.0	-28.0	0.05 (3)		10.00
AB- U	0 / 0	-28.0	-28.0	0.05 (3)		10.00
U-AC	0 / 2716	-28.0	-28.0	0.43 (1)		10.00
AC-AD	0 / 2716	-28.0	-28.0	0.43 (1)		10.00
AD- T	0 / 2716	-28.0	-28.0	0.43 (1)		10.00
T-AE	0 / 2716	-28.0	-28.0	0.43 (1)		10.00
AE- S	0 / 2716	-28.0	-28.0	0.43 (1)		10.00
S-AF	0 / 343	-28.0	-28.0	0.71 (1)		10.00
AF-AG	0 / 343	-28.0	-28.0	0.71 (1)		10.00
AG-AH	0 / 343	-28.0	-28.0	0.71 (1)		10.00
AH- R	0 / 343	-28.0	-28.0	0.71 (1)		10.00
R- Q	0 / 1126	0.0	0.0	0.36 (1)		10.00
Q- E	-606 / 0	0.0	0.0	0.26 (1)		7.81
Q- P	0 / 7234	-28.0	-28.0	0.54 (1)		10.00
P- O	0 / 3259	-28.0	-28.0	0.30 (1)		10.00
O- N	0 / 3253	-28.0	-28.0	0.30 (1)		10.00
M- N	0 / 36	0.0	0.0	0.08 (1)		10.00
N- I	-61 / 0	0.0	0.0	0.08 (1)		7.81
M- L	0 / 36	-28.0	-28.0	0.02 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-977 / 0	0.12 (1)
C- S	0 / 4827	0.60 (1)
S- D	-2516 / 0	0.32 (1)
S- Q	0 / 6705	0.44 (1)
D- Q	0 / 3253	0.40 (1)
Q- G	0 / 3267	0.40 (1)
P- G	-2113 / 0	0.20 (1)
P- H	0 / 4516	0.56 (1)
O- H	0 / 149	0.02 (3)
O- I	0 / 8	0.00 (2)
B- U	0 / 2954	0.37 (1)
N- L	-112 / 0	0.01 (1)
N- J	0 / 3180	0.39 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	53.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 639 (0.54")

CSI: TC=0.72/1.00 (D-E:1) , BC=0.71/1.00 (R-S:1) , WB=0.60/1.00 (C-S:1) , SSI=0.37/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

P6

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295159	T1SZ	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	4.00	6.00
O	BMWW+t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BVMWWW-I	MT20	7.0	12.0	4.50	5.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	12.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

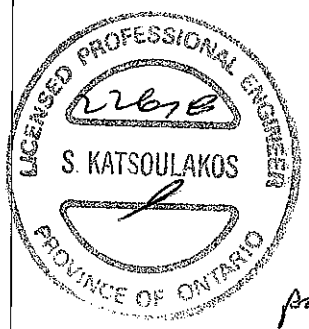
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 130.0 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 4-11-12, 126.3 lbs FACTORED DOWN AT 6-11-12, 126.3 lbs FACTORED DOWN AT 8-11-12, 126.3 lbs FACTORED DOWN AT 10-11-12, 126.3 lbs FACTORED DOWN AT 12-11-12, AND 192.0 lbs FACTORED DOWN AT 2-11-8, AND 192.0 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 11-12, 69.9 lbs FACTORED DOWN AT 2-11-12, 69.9 lbs FACTORED DOWN AT 4-11-12, 69.9 lbs FACTORED DOWN AT 6-11-12, 69.9 lbs FACTORED DOWN AT 8-11-12, 69.9 lbs FACTORED DOWN AT 10-11-12, AND 69.9 lbs FACTORED DOWN AT 12-11-12, AND 1489.6 lbs FACTORED DOWN AT 14-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-130	-130	—	BACK	VERT	TOTAL
C	2-11-8	-192	-192	—	FRONT	VERT	TOTAL
H	25-11-8	-192	-192	—	FRONT	VERT	TOTAL
U	2-11-12	-40	-70	—	BACK	VERT	TOTAL
W	4-11-12	-126	-126	—	BACK	VERT	TOTAL
X	6-11-12	-126	-126	—	BACK	VERT	TOTAL
Y	8-11-12	-126	-126	—	BACK	VERT	TOTAL
Z	10-11-12	-126	-126	—	BACK	VERT	TOTAL
AA	12-11-12	-126	-126	—	BACK	VERT	TOTAL
AB	11-12	-40	-70	—	BACK	VERT	TOTAL
AC	4-11-12	-40	-70	—	BACK	VERT	TOTAL
AD	6-11-12	-40	-70	—	BACK	VERT	TOTAL
AE	8-11-12	-40	-70	—	BACK	VERT	TOTAL
AF	10-11-12	-40	-70	—	BACK	VERT	TOTAL
AG	12-11-12	-40	-70	—	BACK	VERT	TOTAL
AH	14-3-8	-1490	-1490	—	BACK	VERT	TOTAL

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.54 (Q) (INPUT = 1.00)



DWG NO. TAM/020648
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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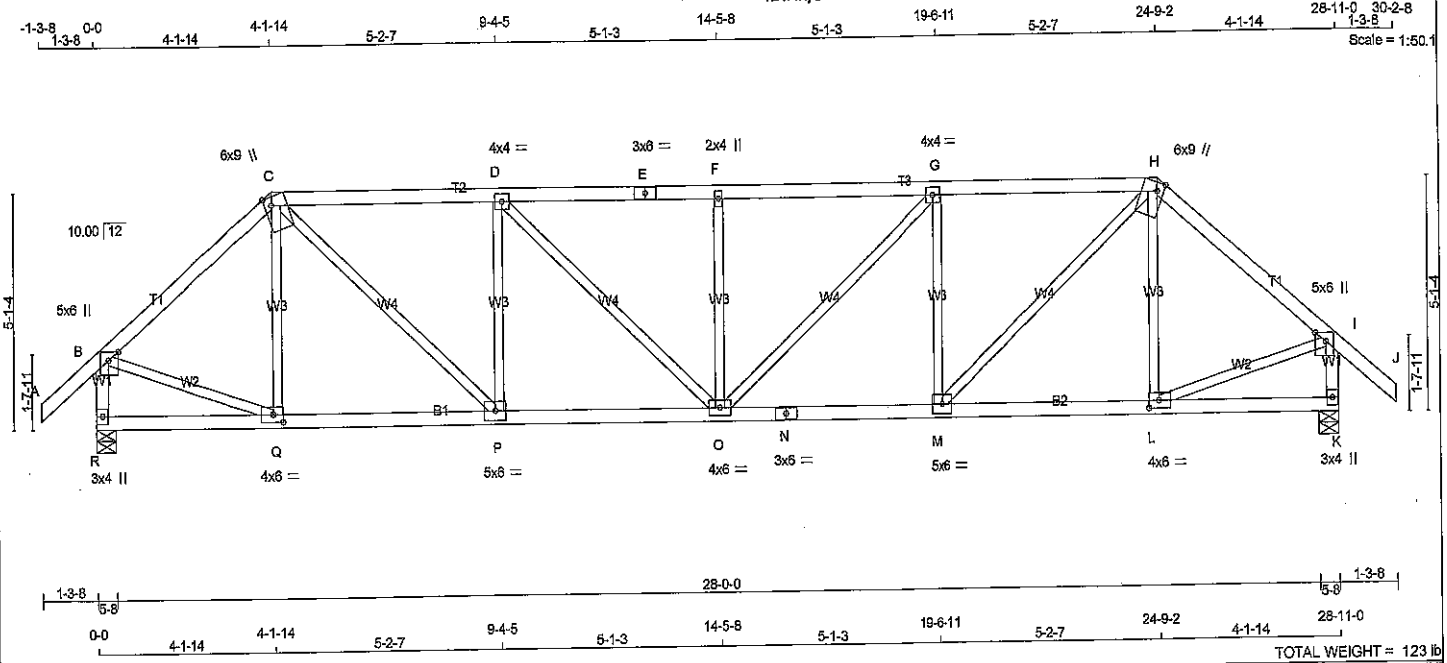

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.75
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2067	0	2067	0	0	5-8	5-8
K	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1891 / 0	-104.9	-104.9	0.40 (1)	4.47	
C- D	-2597 / 0	-104.9	-104.9	0.55 (1)	3.73	
D- E	-2933 / 0	-104.9	-104.9	0.59 (1)	3.50	
E- F	-2933 / 0	-104.9	-104.9	0.59 (1)	3.50	
F- G	-2933 / 0	-104.9	-104.9	0.59 (1)	3.50	
G- H	-2597 / 0	-104.9	-104.9	0.55 (1)	3.73	
H- I	-1891 / 0	-104.9	-104.9	0.40 (1)	4.47	
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
R- B	-2022 / 0	0.0	0.0	0.22 (1)	5.94	
K- I	-2022 / 0	0.0	0.0	0.22 (1)	5.94	
R- Q	0 / 0	-28.0	-28.0	0.15 (2)	10.00	
Q- P	0 / 1442	-28.0	-28.0	0.32 (1)	10.00	
P- O	0 / 2597	-28.0	-28.0	0.48 (1)	10.00	
O- N	0 / 2597	-28.0	-28.0	0.48 (1)	10.00	
N- M	0 / 2597	-28.0	-28.0	0.48 (1)	10.00	
M- L	0 / 1442	-28.0	-28.0	0.32 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-296 / 62	0.11 (1)
C- P	0 / 1570	0.35 (1)
P- D	-922 / 0	0.35 (1)
D- O	0 / 461	0.10 (1)
O- F	-493 / 0	0.19 (1)
O- G	0 / 461	0.10 (1)
M- G	-922 / 0	0.35 (1)
M- H	0 / 1570	0.35 (1)
L- H	-296 / 62	0.11 (1)
B- Q	0 / 1515	0.34 (1)
L- I	0 / 1515	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.59/1.00 (F-G:1) , BC=0.48/1.00 (M-O:1) , WB=0.35/1.00 (D-P:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 101B2-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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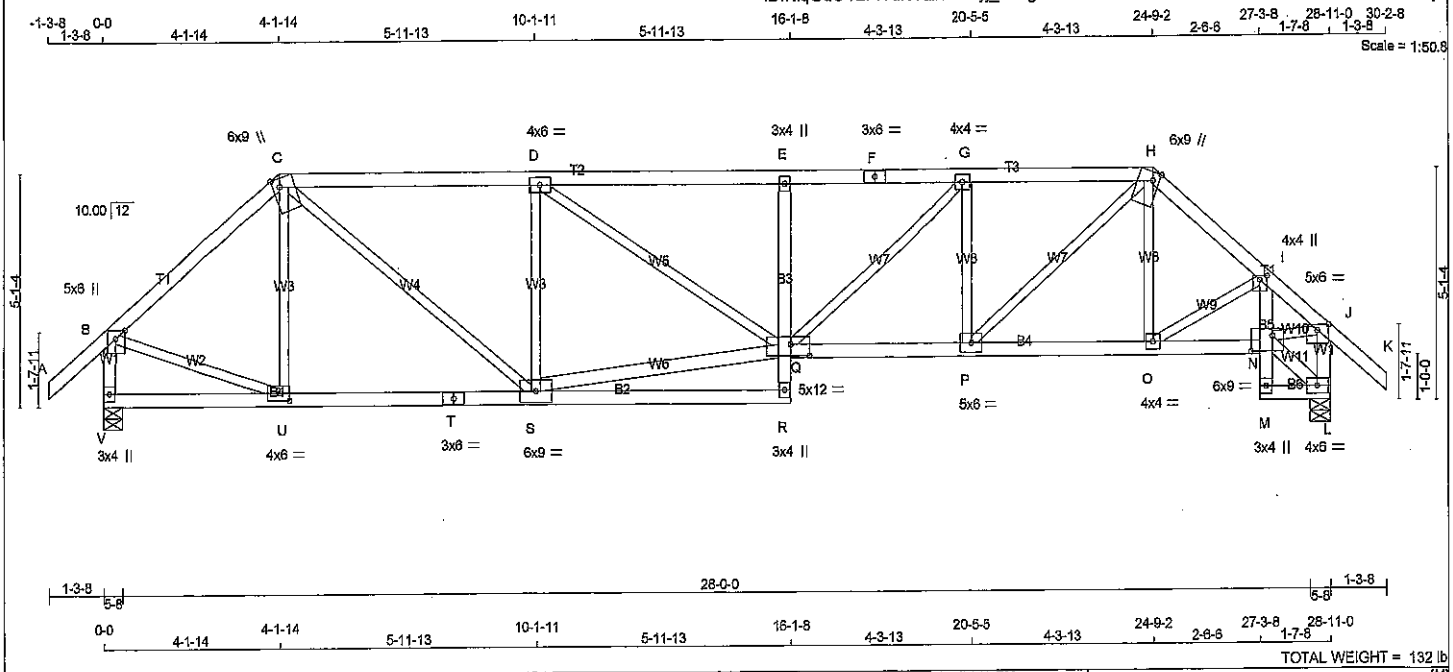

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	4.00	6.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BVMWWW-I	MT20	5.0	12.0	3.00	5.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	6.0	2.00	2.75
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2064	0	2064	0	0	5-8	5-8
L	2071	0	2071	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1628	1027 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1633	1031 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1888 / 0	-104.9	-104.9	0.40 (1)	4.47	
C- D	-2707 / 0	-104.9	-104.9	0.80 (1)	3.34	
D- E	-3606 / 0	-104.9	-104.9	0.93 (1)	2.78	
E- F	-3636 / 0	-104.9	-104.9	0.47 (1)	3.31	
F- G	-3636 / 0	-104.9	-104.9	0.47 (1)	3.31	
G- H	-3080 / 0	-104.9	-104.9	0.41 (1)	3.63	
H- I	-2344 / 0	-104.9	-104.9	0.19 (1)	4.32	
I- J	-2459 / 0	-104.9	-104.9	0.20 (1)	4.21	
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
V- B	-2019 / 0	0.0	0.0	0.21 (1)	5.94	
L- J	-2000 / 0	0.0	0.0	0.21 (1)	5.97	
V- U	0 / 0	-28.0	-28.0	0.15 (2)	10.00	
U- T	0 / 1440	-28.0	-28.0	0.39 (2)	10.00	
T- S	0 / 1440	-28.0	-28.0	0.39 (2)	10.00	
S- R	0 / 114	-28.0	-28.0	0.25 (3)	10.00	
R- Q	0 / 116	0.0	0.0	0.17 (1)	10.00	
Q- E	-496 / 0	0.0	0.0	0.18 (1)	7.81	
Q- P	0 / 3080	-28.0	-28.0	0.56 (1)	10.00	
P- O	0 / 1794	-28.0	-28.0	0.34 (1)	10.00	
O- N	0 / 1915	-28.0	-28.0	0.36 (1)	10.00	
M- N	0 / 36	0.0	0.0	0.10 (1)	10.00	
N- I	-51 / 42	0.0	0.0	0.10 (1)	7.81	
M- L	0 / 63	-28.0	-28.0	0.03 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
U- C	-291 / 66	0.11 (1)
C- S	0 / 1632	0.37 (1)
S- D	-1287 / 0	0.49 (1)
S- Q	0 / 2653	0.43 (1)
D- Q	0 / 1047	0.24 (1)
Q- G	0 / 738	0.17 (1)
P- G	-974 / 0	0.25 (1)
P- H	0 / 1694	0.38 (1)
O- H	0 / 161	0.04 (3)
O- I	-137 / 0	0.03 (1)
B- U	0 / 1513	0.34 (1)
N- L	-81 / 0	0.01 (1)
N- J	0 / 1865	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.93/1.00 (D-E:1) , BC=0.56/1.00 (P-Q:1) , WB=0.49/1.00 (D-S:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.54 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 2268 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM10199-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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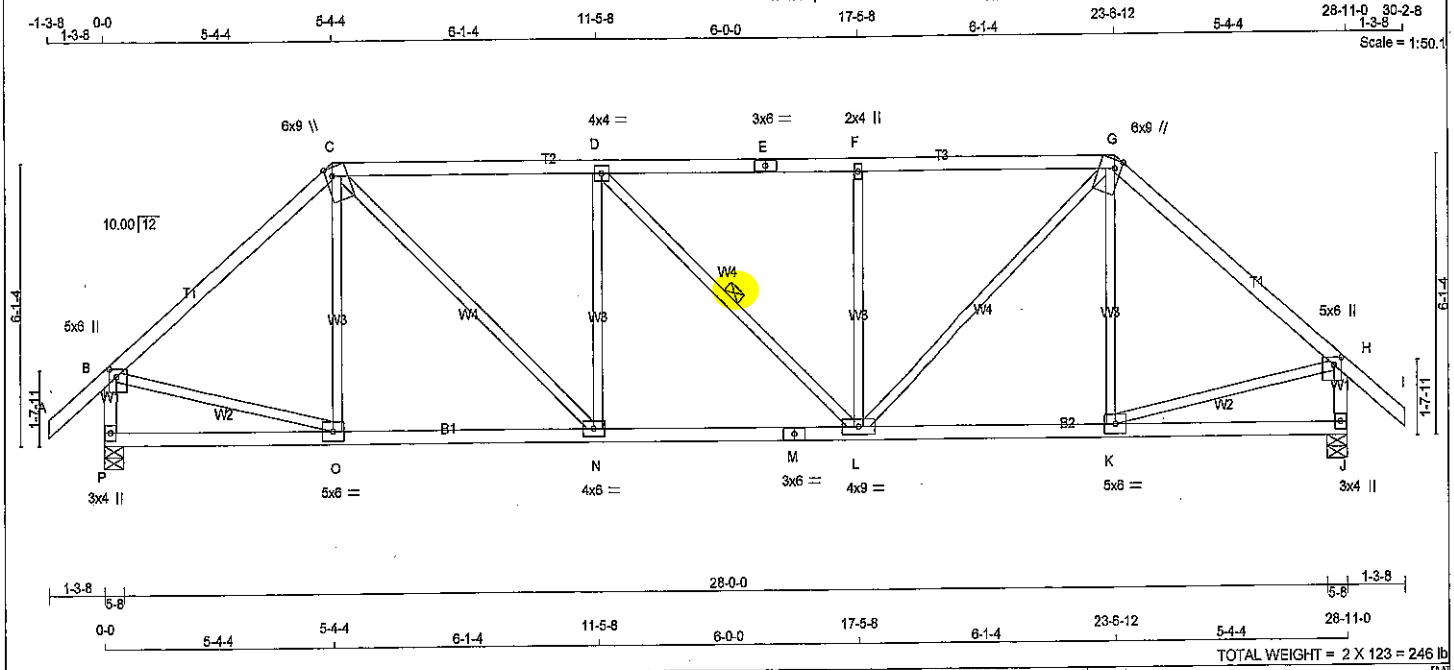

TOTAL WEIGHT = 2 X 123 = 246 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1916 / 0	-104.9	-104.9	0.70 (1)	4.06
C- D	-2364 / 0	-104.9	-104.9	0.71 (1)	3.67
D- E	-2363 / 0	-104.9	-104.9	0.71 (1)	3.67
E- F	-2363 / 0	-104.9	-104.9	0.71 (1)	3.67
F- G	-2363 / 0	-104.9	-104.9	0.71 (1)	3.69
G- H	-1916 / 0	-104.9	-104.9	0.70 (1)	4.06
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P- B	-2007 / 0	0.0	0.0	0.21 (1)	5.96
J- H	-2006 / 0	0.0	0.0	0.21 (1)	5.96
P- O	0 / 0	-28.0	-28.0	0.22 (3)	10.00
O- N	0 / 1466	-28.0	-28.0	0.38 (2)	10.00
N- M	0 / 2364	-28.0	-28.0	0.48 (1)	10.00
M- L	0 / 2364	-28.0	-28.0	0.48 (1)	10.00
L- K	0 / 1466	-28.0	-28.0	0.38 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-171 / 160	0.10 (1)
C- N	0 / 1239	0.28 (1)
N- D	-687 / 0	0.40 (1)
D- L	-2 / 0	0.00 (1)
L- F	-686 / 0	0.40 (1)
L- G	0 / 1236	0.28 (1)
K- G	-170 / 160	0.10 (1)
B- O	0 / 1510	0.34 (1)
K- H	0 / 1510	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.48/1.00 (L-N:1) , WB=0.40/1.00 (D-N:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2696
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM101B3-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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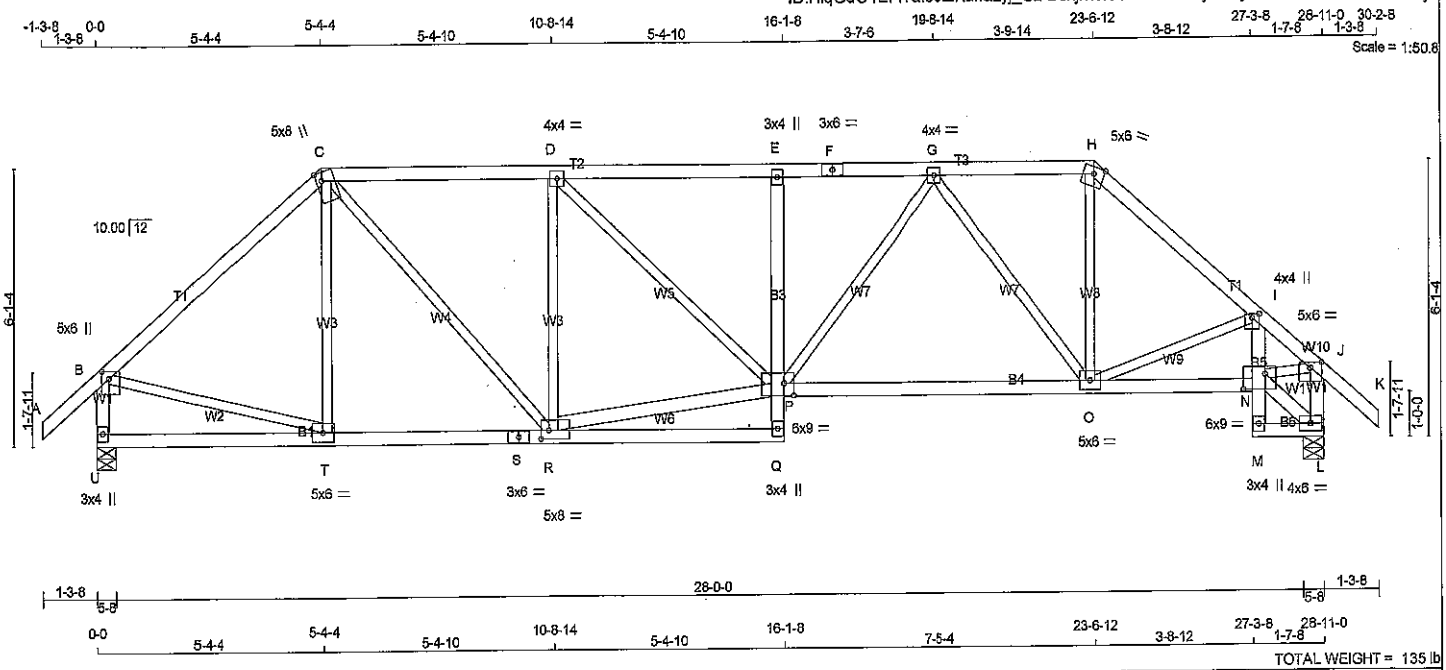

TOTAL WEIGHT = 135 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	9.0	4.00	6.00
O	BMWWW-t	MT20	5.0	6.0		
P	BVMWWW-l	MT20	6.0	9.0	3.25	2.75
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	2.25
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2064	0	2064	0	0	5-8	5-8
L	2071	0	2071	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1628	1027 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1633	1031 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B-C	-1909 / 0	-104.9	-104.9	0.70 (1)	4.07
C-D	-2282 / 0	-104.9	-104.9	0.58 (1)	3.89
D-E	-2858 / 0	-104.9	-104.9	0.64 (1)	3.46
E-F	-2873 / 0	-104.9	-104.9	0.31 (1)	3.84
F-G	-2873 / 0	-104.9	-104.9	0.31 (1)	3.84
G-H	-1785 / 0	-104.9	-104.9	0.23 (1)	4.77
H-I	-2313 / 0	-104.9	-104.9	0.31 (1)	4.22
I-J	-2500 / 0	-104.9	-104.9	0.20 (1)	4.17
J-K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
U-B	-2002 / 0	0.0	0.0	0.21 (1)	5.97
L-J	-1999 / 0	0.0	0.0	0.21 (1)	5.97
U-T	0 / 0	-28.0	-28.0	0.19 (3)	10.00
T-S	0 / 1460	-28.0	-28.0	0.35 (2)	10.00
S-R	0 / 1460	-28.0	-28.0	0.35 (2)	10.00
R-Q	0 / 70	-28.0	-28.0	0.18 (3)	10.00
Q-P	0 / 106	0.0	0.0	0.11 (1)	10.00
P-E	-440 / 0	0.0	0.0	0.15 (1)	7.81
P-O	0 / 2489	-28.0	-28.0	0.60 (2)	10.00
O-N	0 / 1972	-28.0	-28.0	0.55 (2)	10.00
M-N	0 / 36	0.0	0.0	0.11 (1)	10.00
N-I	-94 / 13	0.0	0.0	0.10 (1)	7.81
M-L	0 / 64	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-184 / 135	0.11 (1)
C-R	0 / 1217	0.27 (1)
R-D	-1158 / 0	0.68 (1)
R-P	0 / 2266	0.36 (1)
D-P	0 / 763	0.17 (1)
O-H	0 / 1121	0.25 (1)
O-I	-226 / 0	0.07 (1)
B-T	0 / 1505	0.34 (1)
N-L	-82 / 0	0.01 (1)
N-J	0 / 1924	0.43 (1)
P-G	0 / 623	0.14 (1)
G-O	-1152 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.60/1.00 (O-P:2) , WB=0.75/1.00 (G-O:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.48 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1020287
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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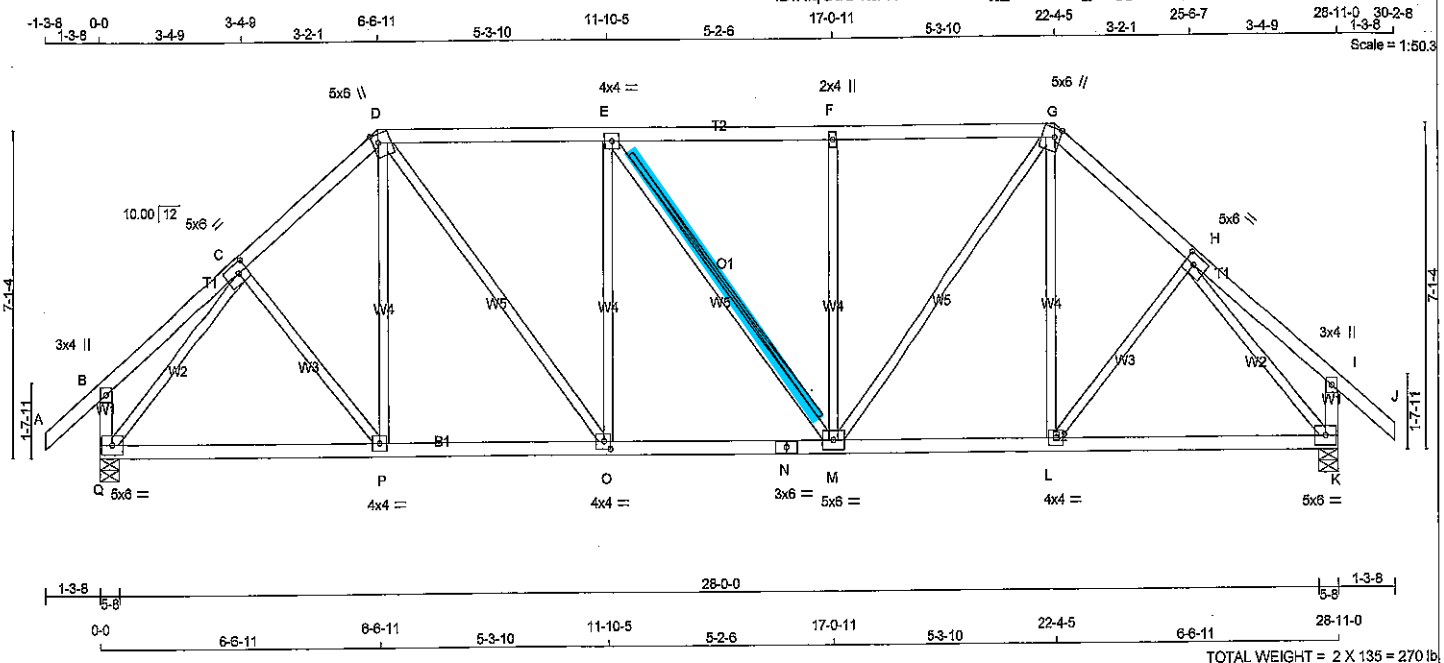

TOTAL WEIGHT = 2 X 135 = 270 lb.

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2067	0	2067	0	0	5-8	5-8
K	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- P	0 / 152	0.03 (2)
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	P- D	0 / 231	0.05 (3)
C- D	-1917 / 0	-104.9	-104.9	0.21 (1)	4.66	D- O	0 / 925	0.21 (1)
D- E	-2022 / 0	-104.9	-104.9	0.49 (1)	4.22	O- E	-594 / 0	0.52 (1)
E- F	-2020 / 0	-104.9	-104.9	0.49 (1)	4.22	E- M	-3 / 0	0.00 (1)
F- G	-2020 / 0	-104.9	-104.9	0.48 (1)	4.23	M- F	-594 / 0	0.52 (1)
G- H	-1917 / 0	-104.9	-104.9	0.21 (1)	4.66	M- G	0 / 922	0.21 (1)
H- I	0 / 23	-104.9	-104.9	0.16 (1)	10.00	L- G	0 / 231	0.05 (3)
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- H	0 / 152	0.03 (2)
Q- B	-280 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2201 / 0	0.95 (1)
K- I	-280 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2201 / 0	0.95 (1)
Q- P	0 / 1385	-28.0	-28.0	0.41 (2)	10.00			
P- O	0 / 1452	-28.0	-28.0	0.43 (2)	10.00			
O- N	0 / 2022	-28.0	-28.0	0.39 (1)	10.00			
N- M	0 / 2022	-28.0	-28.0	0.39 (1)	10.00			
M- L	0 / 1453	-28.0	-28.0	0.43 (2)	10.00			
L- K	0 / 1385	-28.0	-28.0	0.42 (2)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (L-M:2) , WB=0.95/1.00 (H-K:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.54 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10184-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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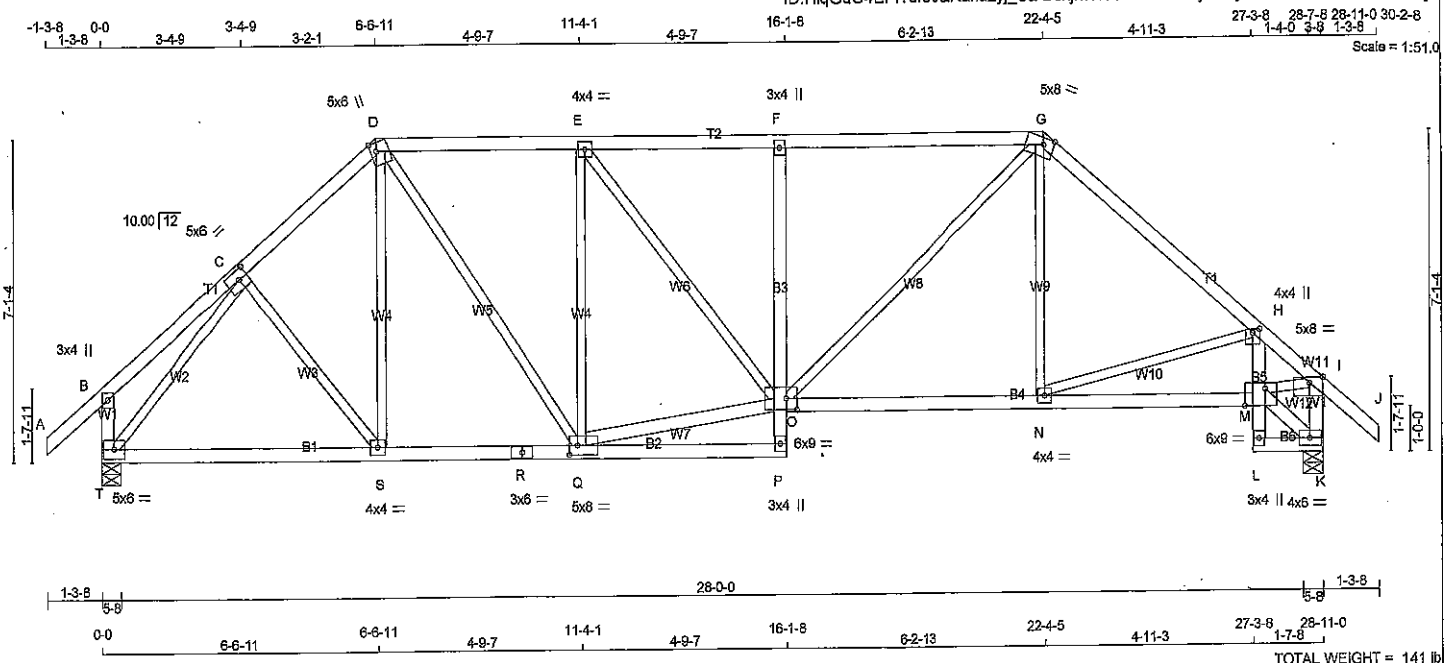

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	3.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-t	MT20	6.0	9.0	4.50	5.75
N	BMWW-t	MT20	4.0	4.0		
O	BVMWWW-t	MT20	6.0	9.0	3.00	3.00
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	2.25
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2084	0	2084	0	0	5-8	5-8
K	2071	0	2071	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1628	1027 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1633	1031 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	1	0.16 (1)	10.00
C- D	-1912 / 0	-104.9	-104.9	1	0.21 (1)	4.67
D- E	-1967 / 0	-104.9	-104.9	1	0.37 (1)	4.43
E- F	-2409 / 0	-104.9	-104.9	1	0.63 (1)	3.66
F- G	-2417 / 0	-104.9	-104.9	1	0.73 (1)	3.65
G- H	-2227 / 0	-104.9	-104.9	1	0.50 (1)	4.13
H- I	-2563 / 0	-104.9	-104.9	1	0.36 (1)	3.92
I- J	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00
T- B	-280 / 0	0.0	0.0	1	0.03 (1)	7.81
K- I	-1998 / 0	0.0	0.0	1	0.21 (1)	5.97
T- S	0 / 1382	-28.0	-28.0	1	0.40 (2)	10.00
S- R	0 / 1448	-28.0	-28.0	1	0.42 (2)	10.00
R- Q	0 / 1448	-28.0	-28.0	1	0.42 (2)	10.00
Q- P	0 / 48	-28.0	-28.0	1	0.14 (2)	10.00
P- O	0 / 98	0.0	0.0	1	0.07 (1)	10.00
O- F	-666 / 0	0.0	0.0	1	0.37 (1)	7.81
O- N	0 / 1691	-28.0	-28.0	1	0.46 (2)	10.00
N- M	0 / 2063	-28.0	-28.0	1	0.49 (2)	10.00
L- M	0 / 36	0.0	0.0	1	0.11 (1)	10.00
M- H	-91 / 58	0.0	0.0	1	0.11 (1)	7.81
L- K	0 / 66	-28.0	-28.0	1	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 151	0.03 (2)
S- D	0 / 218	0.05 (3)
D- Q	0 / 909	0.20 (1)
Q- E	-1029 / 0	0.90 (1)
Q- O	0 / 1971	0.32 (1)
E- O	0 / 894	0.16 (1)
O- G	0 / 981	0.22 (1)
N- G	0 / 365	0.08 (2)
N- H	-392 / 0	0.19 (1)
T- C	-2195 / 0	0.94 (1)
M- K	-84 / 0	0.01 (1)
M- I	0 / 2014	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.73/1.00 (F-G:1) , BC=0.49/1.00 (M-N:2) , WB=0.94/1.00 (C-T:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2676
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1020-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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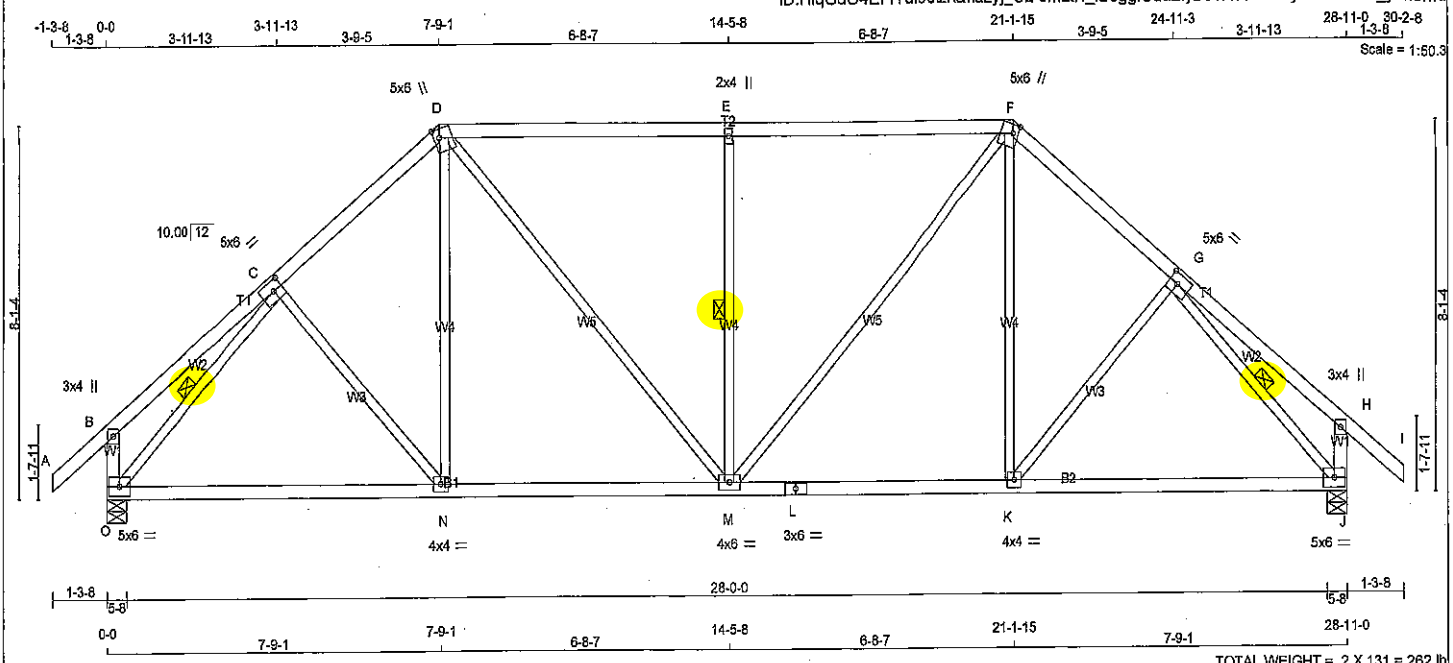

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 29	-104.9	-104.9	0.24 (1)	10.00
C- D	-1882 / 0	-104.9	-104.9	0.23 (1)	4.69
D- E	-1859 / 0	-104.9	-104.9	0.67 (1)	4.04
E- F	-1859 / 0	-104.9	-104.9	0.67 (1)	4.04
F- G	-1882 / 0	-104.9	-104.9	0.23 (1)	4.69
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
O- B	-302 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-302 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1430	-28.0	-28.0	0.55 (2)	10.00
N- M	0 / 1423	-28.0	-28.0	0.58 (2)	10.00
M- L	0 / 1423	-28.0	-28.0	0.56 (2)	10.00
L- K	0 / 1423	-28.0	-28.0	0.56 (2)	10.00
K- J	0 / 1430	-28.0	-28.0	0.55 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-16 / 117	0.03 (3)
N- D	0 / 351	0.08 (2)
D- M	0 / 670	0.15 (1)
M- E	-862 / 0	0.36 (1)
M- F	0 / 670	0.15 (1)
K- F	0 / 351	0.08 (2)
K- G	-16 / 117	0.03 (3)
O- C	-2203 / 0	0.55 (1)
G- J	-2203 / 0	0.55 (1)

DESIGN CRITERIA [M][F]

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.56/1.00 (K-M:2) , WB=0.55/1.00 (G-J:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROFESSIONAL ENGINEERING

DWG NO. TAM 10185-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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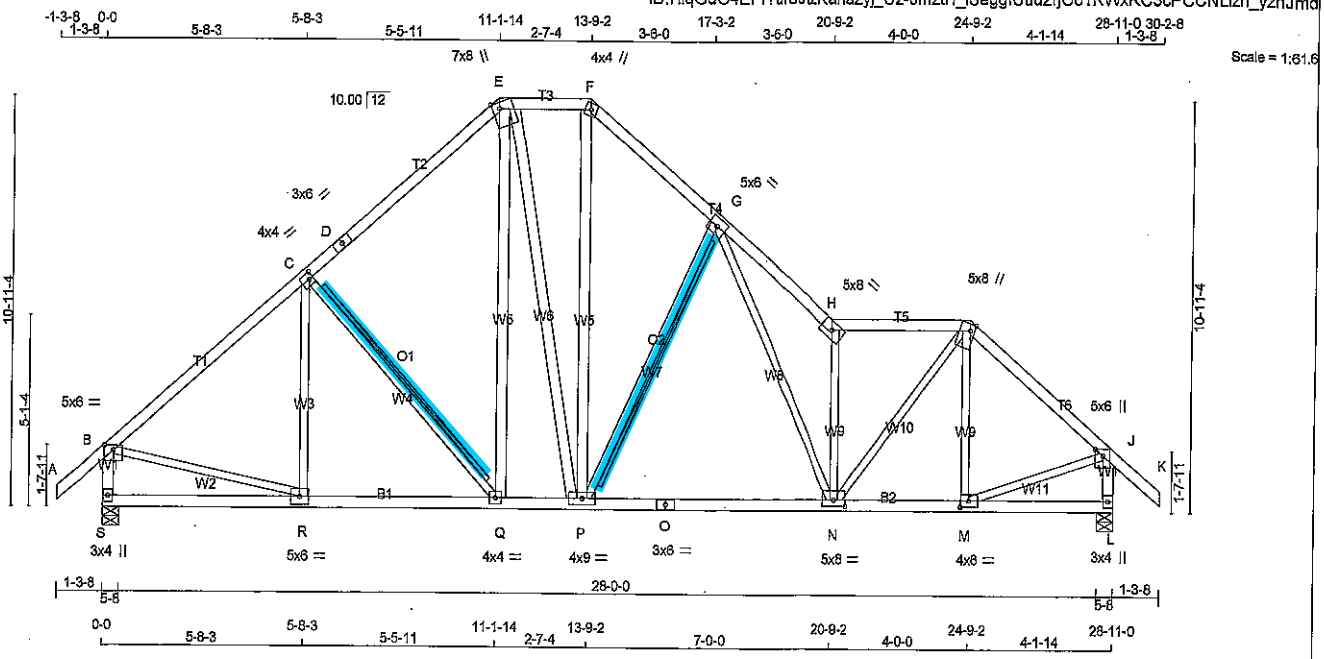


TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW+m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTW-h	MT20	5.0	8.0		
I	TTWW+m	MT20	5.0	8.0	2.25	1.50
J	TMVW+p	MT20	5.0	6.0	Edge	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.75
N	BMWWW-t	MT20	5.0	8.0	2.00	4.00
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2067	0	2067	0	0	5-8	5-8
L	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-Q
2x4 DRY SPF No.2 T-BRACE AT G-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1951 / 0	-104.9	-104.9	0.52 (1)	4.28
C- D	-1660 / 0	-104.9	-104.9	0.48 (1)	4.61
D- E	-1660 / 0	-104.9	-104.9	0.48 (1)	4.61
E- F	-1313 / 0	-104.9	-104.9	0.11 (1)	5.54
F- G	-1731 / 0	-104.9	-104.9	0.22 (1)	4.87
G- H	-3066 / 0	-104.9	-104.9	0.30 (1)	3.77
H- I	-2336 / 0	-104.9	-104.9	0.28 (1)	4.24
I- J	-1866 / 0	-104.9	-104.9	0.29 (1)	4.63
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
S- B	-2003 / 0	0.0	0.0	0.21 (1)	5.97
L- J	-2018 / 0	0.0	0.0	0.21 (1)	5.94
S- R	0 / 0	-28.0	-28.0	0.24 (3)	10.00
R- Q	0 / 1534	-28.0	-28.0	0.40 (2)	10.00
Q- P	0 / 1242	-28.0	-28.0	0.40 (2)	10.00
P- O	0 / 1758	-28.0	-28.0	0.45 (1)	10.00
O- N	0 / 1758	-28.0	-28.0	0.45 (1)	10.00
N- M	0 / 1436	-28.0	-28.0	0.34 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.11 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-167 / 179	0.11 (1)
C- Q	-435 / 0	0.49 (1)
Q- E	0 / 378	0.06 (1)
E- P	0 / 315	0.05 (1)
P- F	0 / 817	0.13 (1)
P- G	-1043 / 0	0.58 (1)
G- N	0 / 1493	0.34 (1)
N- H	-2315 / 0	0.69 (1)
N- I	0 / 1424	0.32 (1)
M- I	-321 / 0	0.12 (1)
B- R	0 / 1574	0.35 (1)
M- J	0 / 1511	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.45/1.00 (N-P:1) , WB=0.89/1.00 (H-N:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

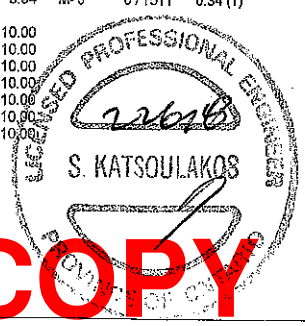
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.66 (H) (INPUT = 1.00)



DWG NO. TAM 101186-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T7	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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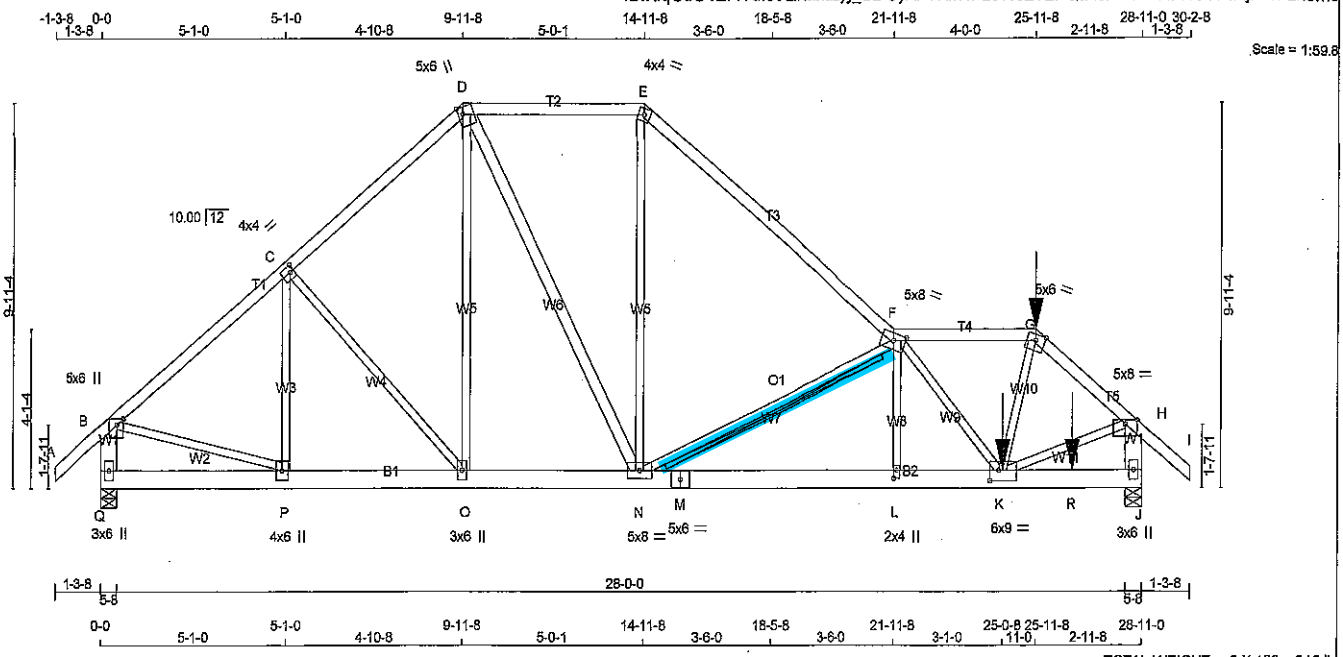

TOTAL WEIGHT = 2 X 159 = 318 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	SIDE(0.0)
G- I	1	12	SIDE(0.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	TOP
M- J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWWW-m	MT20	5.0	8.0	2.25	4.00
G	TTW-m	MT20	5.0	6.0	Edge	
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWWW-t	MT20	6.0	9.0	3.25	3.00
L	BMW+w	MT20	2.0	4.0	2.50	1.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2407	0	2407	0	0	5-8	5-8
J	4296	0	4296	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1896	1201 / 0	351 / 0	0 / 0	0 / 0	345 / 0	0 / 0
J	3384	2143 / 0	625 / 0	0 / 0	0 / 0	615 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-2372 / 0	-104.9	-104.9	0.29 (1)	5.51
C- D	-2219 / 0	-104.9	-104.9	0.28 (1)	5.66
D- E	-1997 / 0	-104.9	-104.9	0.27 (1)	5.87
E- F	-2585 / 0	-104.9	-104.9	0.69 (1)	4.87
F- G	-3988 / 0	-104.9	-104.9	0.22 (1)	4.54
G- H	-4424 / 0	-104.9	-104.9	0.16 (1)	4.43
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00
Q- B	-2340 / 0	0.0	0.0	0.09 (1)	7.81
J- H	-4200 / 0	0.0	0.0	0.15 (1)	6.97
Q- P	0 / 0	-28.0	-28.0	0.04 (2)	10.00
P- O	0 / 1852	-28.0	-28.0	0.14 (1)	10.00
O- N	0 / 1673	-28.0	-28.0	0.16 (1)	10.00
N- M	0 / 4728	-28.0	-28.0	0.37 (1)	10.00
M- L	0 / 4728	-28.0	-28.0	0.37 (1)	10.00
L- K	0 / 4731	-28.0	-28.0	0.35 (1)	10.00
K- R	0 / 0	-28.0	-28.0	0.06 (2)	10.00
R- J	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
P- C	-303 / 93	0.08 (1)
C- O	-271 / 0	0.14 (1)
O- D	0 / 329	0.04 (2)
D- N	0 / 709	0.06 (1)
N- E	0 / 1025	0.13 (1)
N- F	-3094 / 0	0.65 (1)
L- F	0 / 251	0.03 (3)
B- P	0 / 1905	0.24 (1)
K- H	0 / 3546	0.44 (1)
K- G	0 / 2368	0.29 (1)
F- K	-1176 / 0	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
G	25-11-8	-192	-192	---	FRONT	VERT	TOTAL
K	25-0-8	-2337	-2337	---	BACK	VERT	TOTAL
R	26-11-12	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.37/1.00 (L-N:1) , WB=0.65/1.00 (F-N:1) , SSI=0.12/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10167-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T7	1	2	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.0 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 2337.2 lbs FACTORED DOWN AT 25-0-8, AND 69.9 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10187-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T8	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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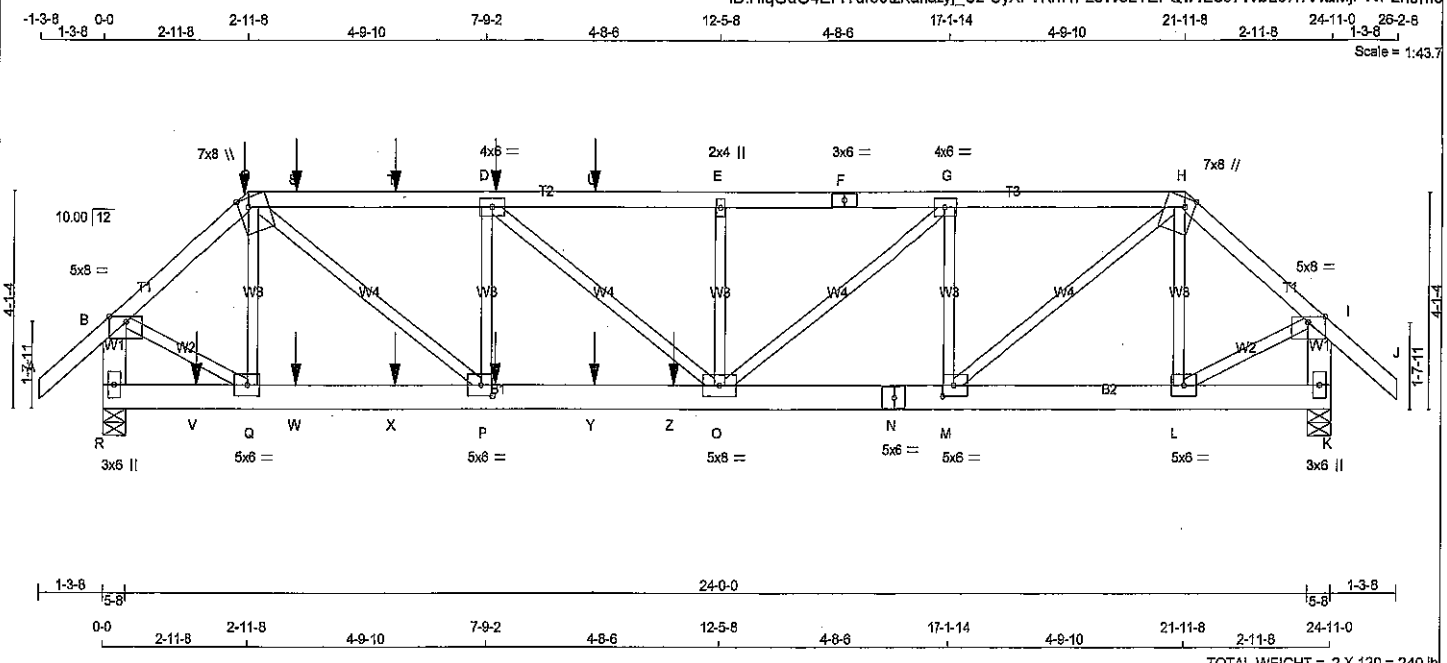

TOTAL WEIGHT = 2 X 120 = 240 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	3511	0	3511	0	0	5-8	5-8
K	2894	0	2894	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2775	1742 / 0	523 / 0	0 / 0	0 / 0	510 / 0	0 / 0
K	2266	1457 / 0	405 / 0	0 / 0	0 / 0	404 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)						
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-870 / 0	0.11 (1)
B- C	-3339 / 0	-104.9	-104.9	0.13 (1)	5.00	C- P	0 / 3798	0.47 (1)
C- S	-5537 / 0	-104.9	-104.9	0.44 (1)	3.74	P- D	-1711 / 0	0.22 (1)
S- T	-5537 / 0	-104.9	-104.9	0.44 (1)	3.74	D- O	0 / 1310	0.16 (1)
T- D	-5537 / 0	-104.9	-104.9	0.44 (1)	3.74	O- E	-480 / 0	0.06 (1)
D- U	-6564 / 0	-104.9	-104.9	0.50 (1)	3.40	O- G	0 / 2423	0.30 (1)
U- E	-6564 / 0	-104.9	-104.9	0.50 (1)	3.40	M- G	-2054 / 0	0.26 (1)
E- F	-6564 / 0	-104.9	-104.9	0.38 (1)	3.54	M- H	0 / 3275	0.41 (1)
F- G	-6564 / 0	-104.9	-104.9	0.38 (1)	3.54	L- H	-623 / 0	0.08 (1)
G- H	-4666 / 0	-104.9	-104.9	0.29 (1)	4.21	B- Q	0 / 2752	0.34 (1)
H- I	-2732 / 0	-104.9	-104.9	0.12 (1)	5.42	L- I	0 / 2251	0.28 (1)
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00			
R- B	-3469 / 0	0.0	0.0	0.13 (1)	7.51			
K- I	-2693 / 0	0.0	0.0	0.11 (1)	7.81			
R- V	0 / 0	-28.0	-28.0	0.04 (3)	10.00			
V- Q	0 / 0	-28.0	-28.0	0.04 (3)	10.00			
Q- W	0 / 2531	-28.0	-28.0	0.26 (1)	10.00			
W- X	0 / 2531	-28.0	-28.0	0.26 (1)	10.00			
X- P	0 / 2531	-28.0	-28.0	0.26 (1)	10.00			
P- Y	0 / 5538	-28.0	-28.0	0.76 (1)	10.00			
Y- Z	0 / 5538	-28.0	-28.0	0.76 (1)	10.00			
Z- O	0 / 5538	-28.0	-28.0	0.76 (1)	10.00			
O- N	0 / 4666	-28.0	-28.0	0.46 (1)	10.00			
N- M	0 / 4666	-28.0	-28.0	0.46 (1)	10.00			
M- L	0 / 2073	-28.0	-28.0	0.19 (1)	10.00			
L- K	0 / 0	-28.0	-28.0	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-192	-192	---	FRONT	VERT	TOTAL
D	7-11-4	-126	-126	---	BACK	VERT	TOTAL
P	7-11-4	-40	-70	---	BACK	VERT	TOTAL
S	3-11-4	-127	-127	---	BACK	VERT	TOTAL
T	5-11-4	-126	-126	---	BACK	VERT	TOTAL
U	9-11-4	-126	-126	---	BACK	VERT	TOTAL
V	1-11-4	-40	-70	---	BACK	VERT	TOTAL
W	3-11-4	-40	-70	---	BACK	VERT	TOTAL
X	5-11-4	-40	-70	---	BACK	VERT	TOTAL
Y	9-11-4	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.76/1.00 (O-P:1) , WB=0.47/1.00 (C-P:1) , SSI=0.55/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (P) (INPUT = 0.90)
JSI METAL= 0.50 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0198-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T8	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.0 lbs FACTORED DOWN AT 2-11-8, 127.3 lbs FACTORED DOWN AT 3-11-4, 126.3 lbs FACTORED DOWN AT 5-11-4, AND 126.3 lbs FACTORED DOWN AT 7-11-4, AND 126.3 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, AND 69.9 lbs FACTORED DOWN AT 9-11-4, AND 1904.4 lbs FACTORED DOWN AT 11-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	11-6-8	-1904	-1904	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 101883-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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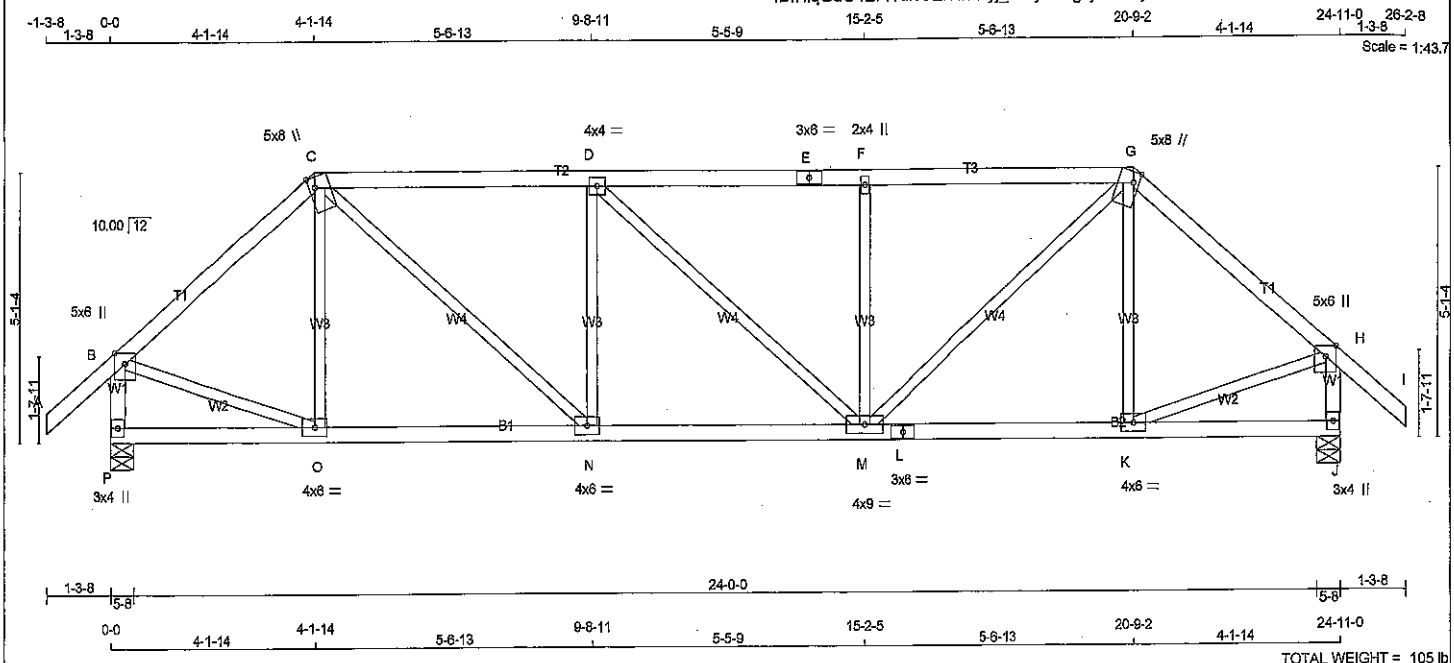

TOTAL WEIGHT = 105 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TTWW+m	MT20	5.0	8.0	Edge 1.25
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TTWW+m	MT20	5.0	8.0	Edge 1.25
H	TMVW+p	MT20	5.0	6.0	Edge
J	BMV1+p	MT20	3.0	4.0	
K, N, O					
K	BMWW-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	4.0	9.0	
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1801	0	1801	0	0	5-8	5-8
J	1801	0	1801	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1589 / 0	-104.9	-104.9	0.38 (1)	4.82
C- D	-2117 / 0	-104.9	-104.9	0.55 (1)	4.06
D- E	-2115 / 0	-104.9	-104.9	0.55 (1)	4.06
E- F	-2115 / 0	-104.9	-104.9	0.55 (1)	4.06
F- G	-2115 / 0	-104.9	-104.9	0.54 (1)	4.07
G- H	-1589 / 0	-104.9	-104.9	0.38 (1)	4.82
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P- B	-1757 / 0	0.0	0.0	0.19 (1)	6.29
J- H	-1757 / 0	0.0	0.0	0.19 (1)	6.29
P- O	0 / 0	-28.0	-28.0	0.16 (3)	10.00
O- N	0 / 1213	-28.0	-28.0	0.29 (2)	10.00
N- M	0 / 2117	-28.0	-28.0	0.42 (1)	10.00
M- L	0 / 1213	-28.0	-28.0	0.29 (2)	10.00
L- K	0 / 1213	-28.0	-28.0	0.29 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-220 / 101	0.08 (1)
C- N	0 / 1193	0.27 (1)
N- D	-625 / 0	0.24 (1)
D- M	-2 / 0	0.00 (1)
M- F	-625 / 0	0.24 (1)
M- G	0 / 1190	0.27 (1)
K- G	-219 / 101	0.08 (1)
B- O	0 / 1273	0.29 (1)
K- H	0 / 1273	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.42/1.00 (M-N:1) , WB=0.29/1.00 (H-K:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.31 (L) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 10109-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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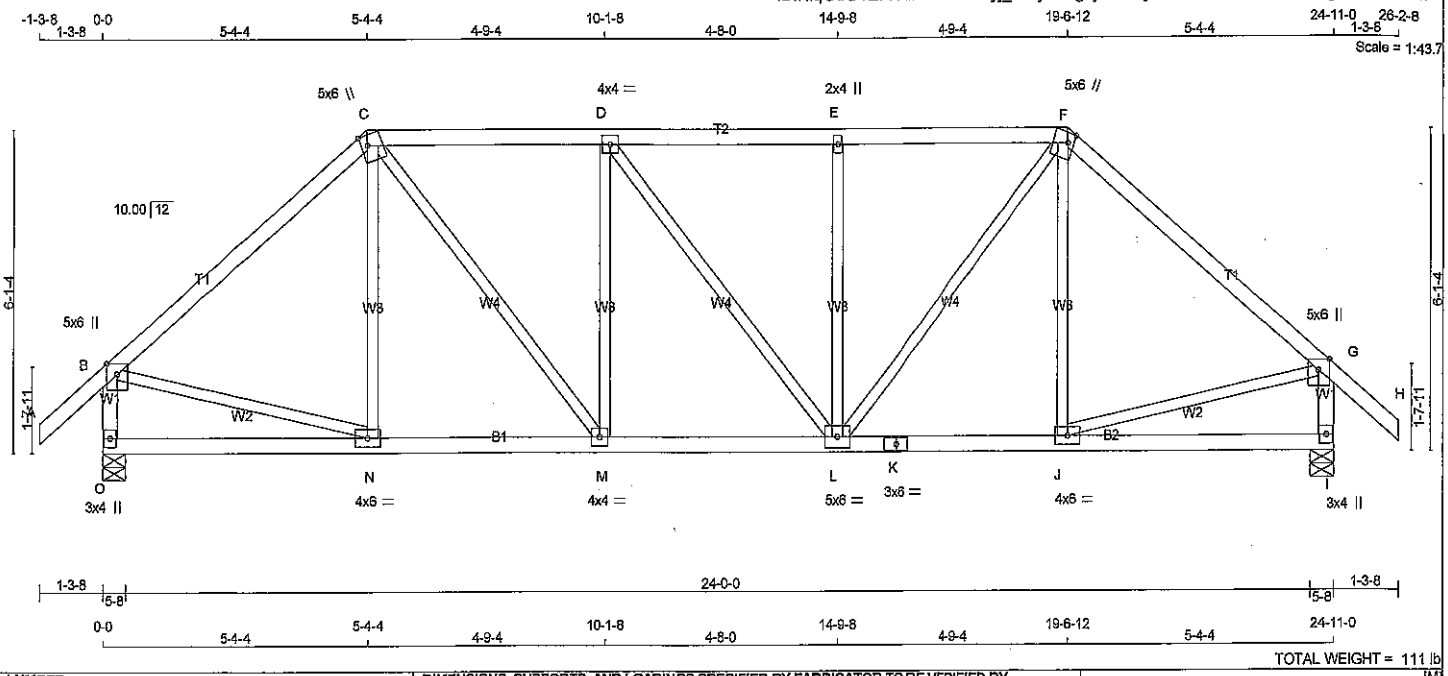

TOTAL WEIGHT = 111 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1801	0	1801	0	0	5-8	5-8
I	1801	0	1801	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	N- C	-127 / 159	0.07 (1)
B- C	-1591 / 0	-104.9	-104.9	0.66 (1)	4.42	C- M	0 / 851	0.19 (1)
C- D	-1758 / 0	-104.9	-104.9	0.37 (1)	4.61	M- D	-533 / 0	0.31 (1)
D- E	-1756 / 0	-104.9	-104.9	0.37 (1)	4.61	D- L	-3 / 0	0.00 (1)
E- F	-1756 / 0	-104.9	-104.9	0.37 (1)	4.63	L- E	-532 / 0	0.31 (1)
F- G	-1591 / 0	-104.9	-104.9	0.66 (1)	4.42	L- F	0 / 847	0.19 (1)
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- F	-126 / 159	0.07 (1)
O- B	-1739 / 0	0.0	0.0	0.19 (1)	6.32	B- N	0 / 1254	0.28 (1)
I- G	-1739 / 0	0.0	0.0	0.19 (1)	6.32	J- G	0 / 1254	0.28 (1)
O- N	0 / 0	-28.0	-28.0	0.19 (3)	10.00			
N- M	0 / 1217	-28.0	-28.0	0.32 (2)	10.00			
M- L	0 / 1758	-28.0	-28.0	0.34 (1)	10.00			
L- K	0 / 1217	-28.0	-28.0	0.32 (2)	10.00			
K- J	0 / 1217	-28.0	-28.0	0.32 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.66/1.00 (F-G:1) , BC=0.34/1.00 (L-M:1) , WB=0.31/1.00 (D-M:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

DWG NO. TAM 10190-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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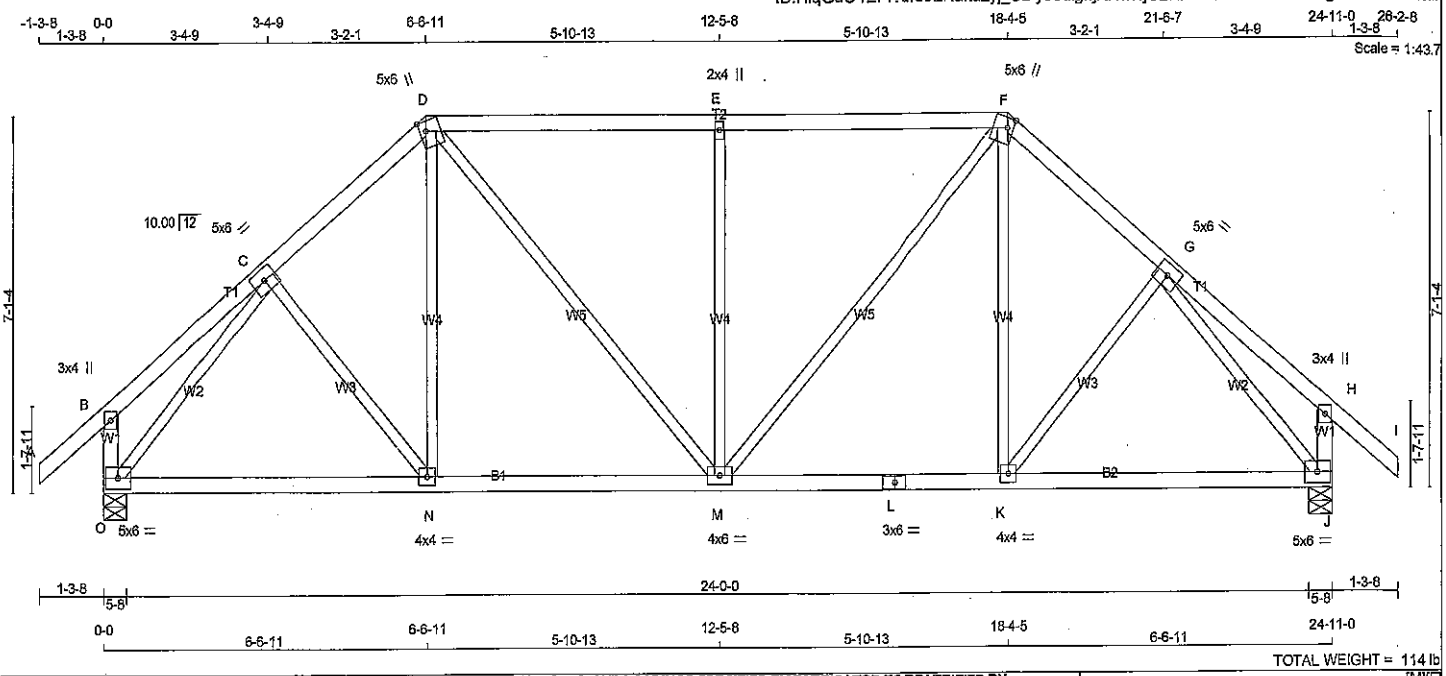


TOTAL WEIGHT = 114 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1801	0	1801	0	0	5-8	5-8
J	1801	0	1801	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	0 / 118	0.03 (3)	
B- C	0 / 23	-104.9	-104.9	0.17 (1)	10.00	N- D	0 / 276	0.06 (2)	
C- D	-1583 / 0	-104.9	-104.9	0.16 (1)	5.11	D- M	0 / 593	0.13 (1)	
D- E	-1586 / 0	-104.9	-104.9	0.50 (1)	4.63	M- E	-757 / 0	0.66 (1)	
E- F	-1586 / 0	-104.9	-104.9	0.50 (1)	4.63	M- F	0 / 593	0.13 (1)	
F- G	-1583 / 0	-104.9	-104.9	0.16 (1)	5.11	K- F	0 / 276	0.06 (2)	
G- H	0 / 23	-104.9	-104.9	0.17 (1)	10.00	K- G	0 / 118	0.03 (3)	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-1862 / 0	0.80 (1)	
O- B	-279 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-1862 / 0	0.80 (1)	
J- H	-279 / 0	0.0	0.0	0.03 (1)	7.81				
O- N	0 / 1171	-28.0	-28.0	0.41 (2)	10.00				
N- M	0 / 1197	-28.0	-28.0	0.42 (2)	10.00				
M- L	0 / 1197	-28.0	-28.0	0.42 (2)	10.00				
L- K	0 / 1197	-28.0	-28.0	0.42 (2)	10.00				
K- J	0 / 1171	-28.0	-28.0	0.41 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.42/1.00 (K-M:2) , WB=0.80/1.00 (G-J:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

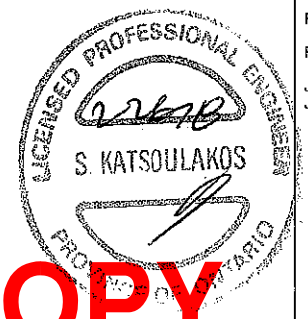
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.46 (G) (INPUT = 1.00)



DWG NO. TAM 10191

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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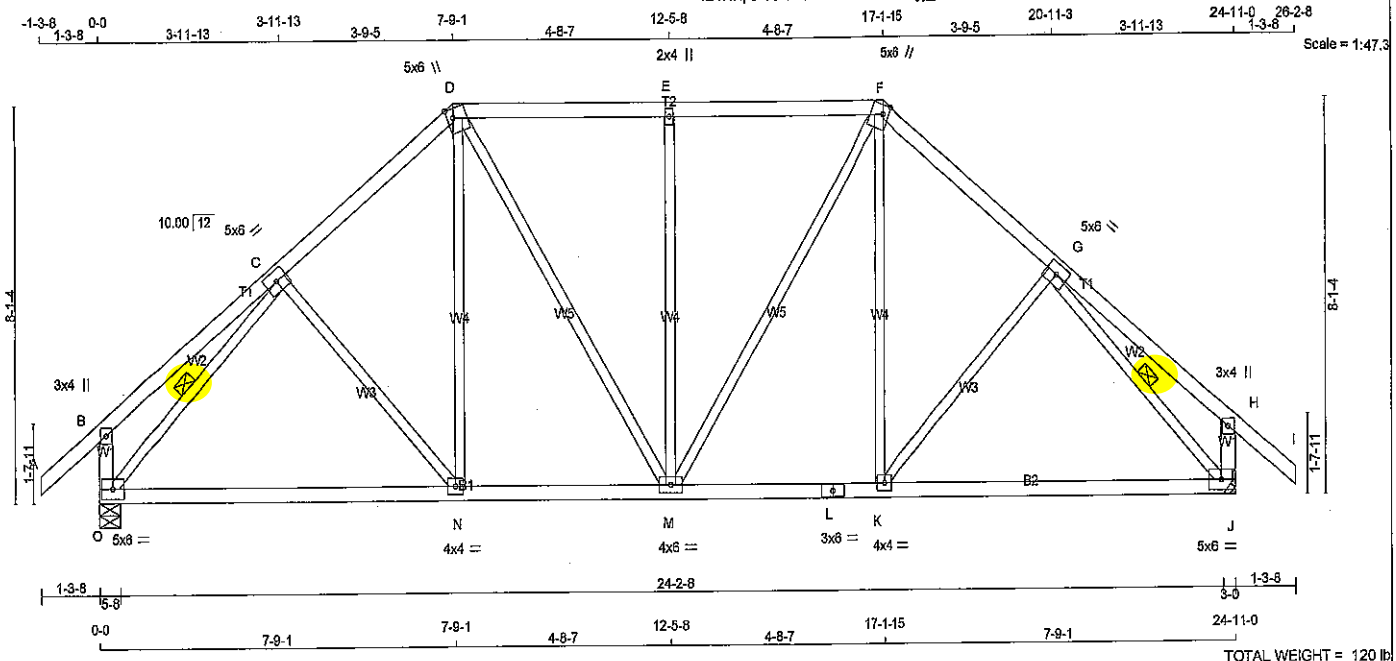


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1801	0	1801	0	0	IN-SX	IN-SX
J	1801	0	1801	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 29	-104.9	-104.9	0.25 (1)	10.00
C- D	-1534 / 0	-104.9	-104.9	0.22 (1)	5.10
D- E	-1354 / 0	-104.9	-104.9	0.31 (1)	5.22
E- F	-1354 / 0	-104.9	-104.9	0.31 (1)	5.22
F- G	-1534 / 0	-104.9	-104.9	0.22 (1)	5.10
G- H	0 / 29	-104.9	-104.9	0.25 (1)	10.00
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
O- B	-301 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-301 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1203	-28.0	-28.0	0.51 (2)	10.00
N- M	0 / 1157	-28.0	-28.0	0.51 (2)	10.00
M- L	0 / 1157	-28.0	-28.0	0.51 (2)	10.00
L- K	0 / 1157	-28.0	-28.0	0.51 (2)	10.00
K- J	0 / 1203	-28.0	-28.0	0.51 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-75 / 92	0.05 (1)
N- D	0 / 367	0.08 (2)
D- M	0 / 382	0.09 (1)
M- E	-598 / 0	0.76 (1)
M- F	0 / 382	0.09 (1)
K- F	0 / 367	0.08 (2)
K- G	-75 / 92	0.05 (1)
O- C	-1853 / 0	0.46 (1)
G- J	-1853 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.51/1.00 (M-N:2) , WB=0.76/1.00 (E-M:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

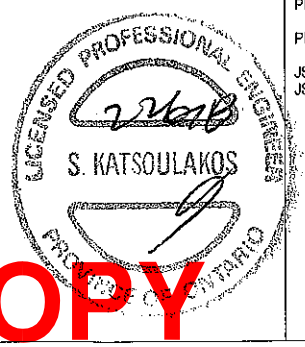
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.48 (L) (INPUT = 1.00)

[M][F]



DWG NO. TAM 10192-S2
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T13	1	1	TRUSS DESC.	

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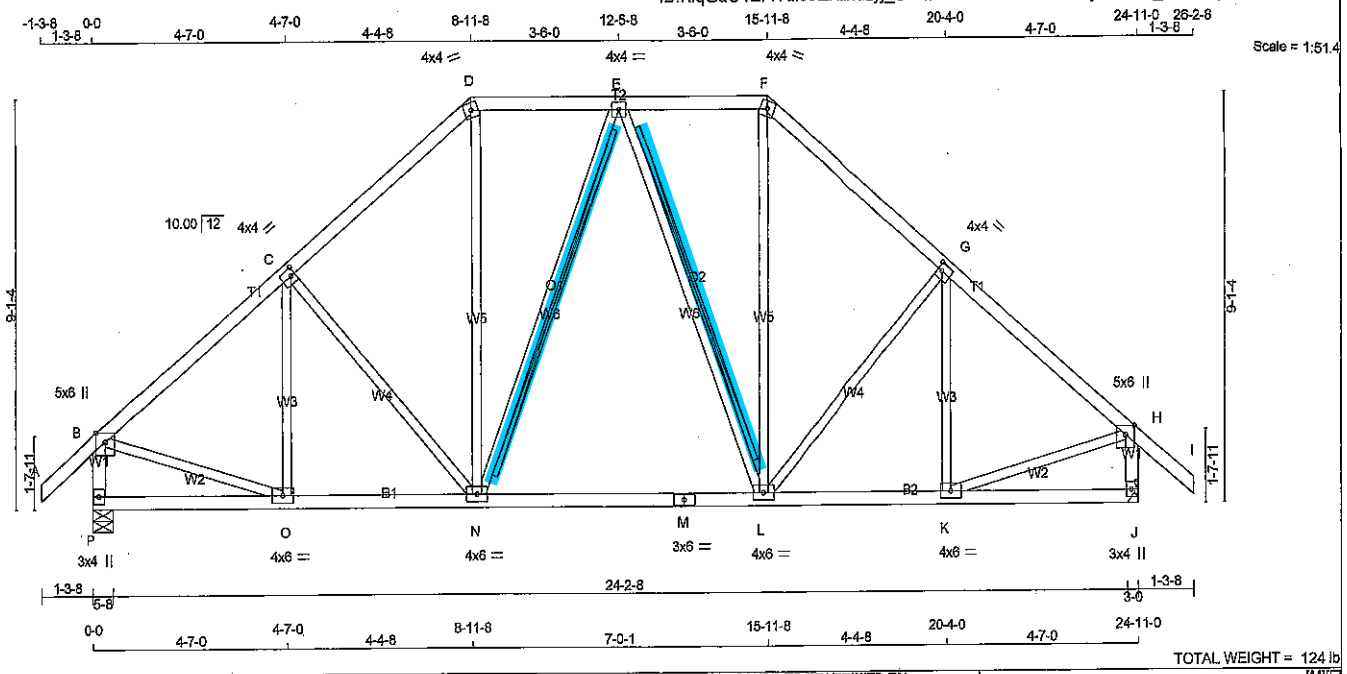


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1801	0	1801	0	0	IN-SX	IN-SX
J	1801	0	1801	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

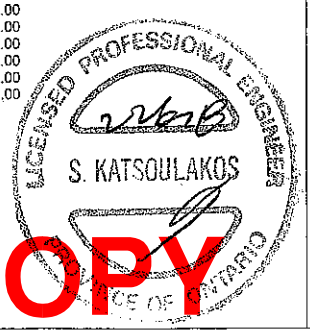
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-237 / 72	0.11 (1)
B- C	-1611 / 0	-104.9	-104.9	0.31 (1)	4.91	C- N	-269 / 0	0.23 (1)
C- D	-1456 / 0	-104.9	-104.9	0.30 (1)	5.11	N- D	0 / 589	0.13 (1)
D- E	-1094 / 0	-104.9	-104.9	0.16 (1)	5.87	N- E	-232 / 0	0.17 (1)
E- F	-1094 / 0	-104.9	-104.9	0.16 (1)	5.87	E- L	-232 / 0	0.17 (1)
F- G	-1456 / 0	-104.9	-104.9	0.30 (1)	5.11	L- F	0 / 589	0.13 (1)
G- H	-1611 / 0	-104.9	-104.9	0.31 (1)	4.91	L- G	-269 / 0	0.23 (1)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K- G	-237 / 72	0.11 (1)
P- B	-1745 / 0	0.0	0.0	0.19 (1)	6.31	B- O	0 / 1315	0.30 (1)
J- H	-1745 / 0	0.0	0.0	0.19 (1)	6.31	K- H	0 / 1315	0.30 (1)
P- O	0 / 0	-28.0	-28.0	0.13 (2)	10.00			
O- N	0 / 1265	-28.0	-28.0	0.35 (2)	10.00			
N- M	0 / 1178	-28.0	-28.0	0.33 (2)	10.00			
M- L	0 / 1178	-28.0	-28.0	0.33 (2)	10.00			
L- K	0 / 1265	-28.0	-28.0	0.35 (2)	10.00			
K- J	0 / 0	-28.0	-28.0	0.13 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.35/1.00 (N-O:2) , WB=0.30/1.00 (H-K:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.39 (M) (INPUT = 1.00)

DWG NO. TAM10193
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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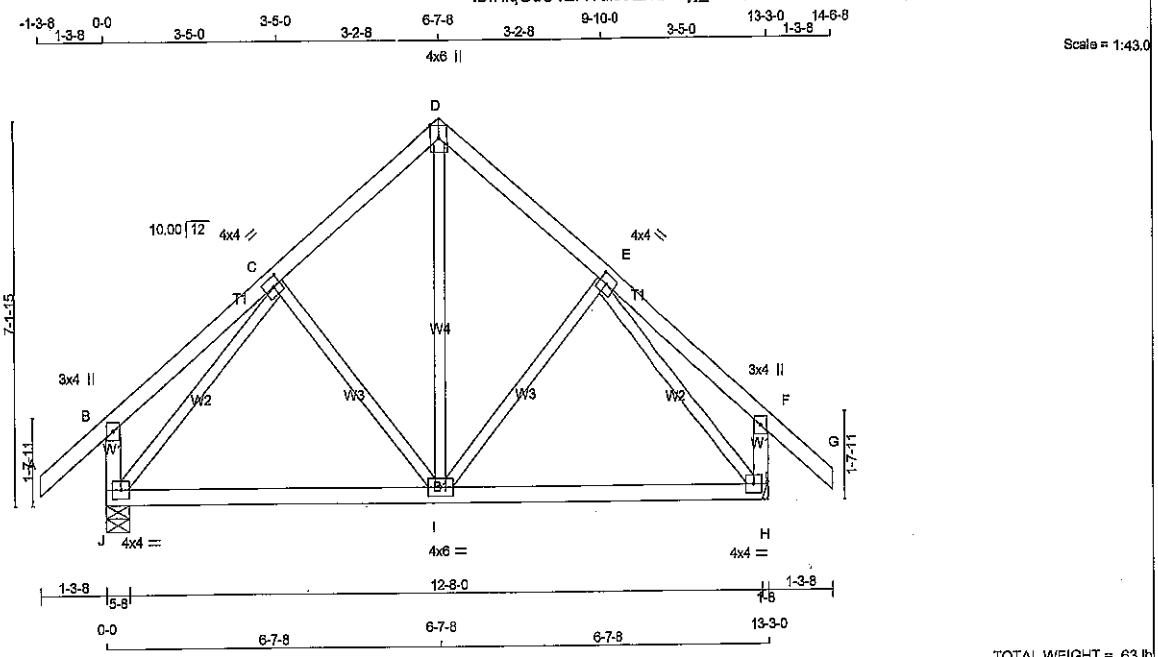

TOTAL WEIGHT = 63 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1026	0	1026	0	0	IN-SX	IN-SX
H	1026	0	1026	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	800	520 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	800	520 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00	I- D	0 / 475	0.11 (1)
B- C	0 / 25	-104.9	-104.9	0.18 (1)		10.00	I- E	-159 / 39	0.07 (1)
C- D	-609 / 0	-104.9	-104.9	0.14 (1)		6.25	C- I	-159 / 39	0.07 (1)
D- E	-609 / 0	-104.9	-104.9	0.14 (1)		6.25	J- C	-872 / 0	0.38 (1)
E- F	0 / 25	-104.9	-104.9	0.18 (1)		10.00	E- H	-872 / 0	0.38 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)		10.00			
J- B	-276 / 0	0.0	0.0	0.03 (1)		7.81			
H- F	-276 / 0	0.0	0.0	0.03 (1)		7.81			
J- I	0 / 550	-28.0	-28.0	0.41 (2)		10.00			
I- H	0 / 550	-28.0	-28.0	0.41 (2)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.41/1.00 (H-I:2) , WB=0.38/1.00 (C-J:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

DWG NO. TAM 10194-T14
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T14S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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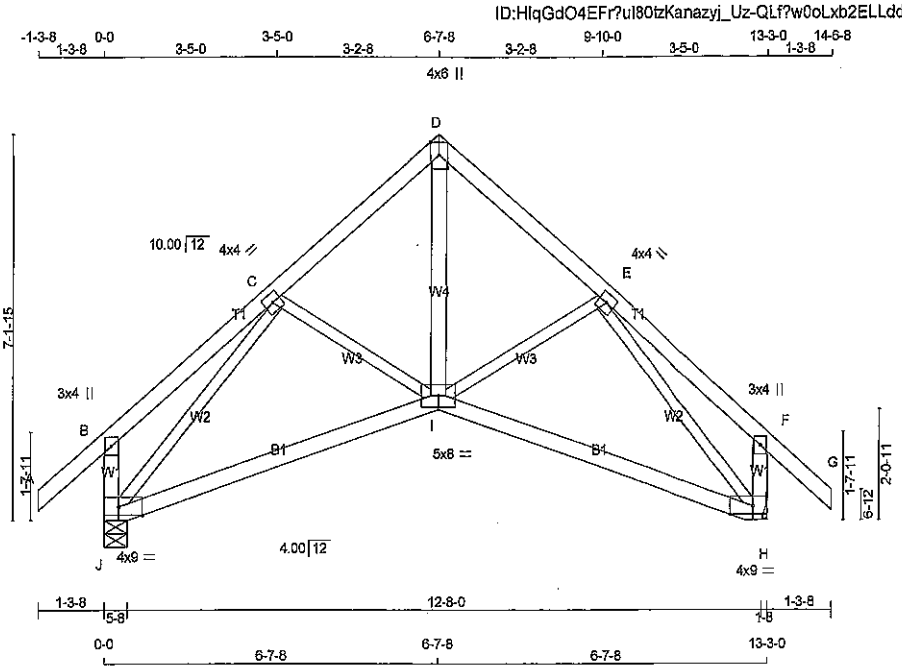


TOTAL WEIGHT = 3 X 62 = 185 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	4.0	9.0	Edge	5.50
I	BBWWW-p	MT20	5.0	8.0		
J	BVMW1-p	MT20	4.0	9.0	Edge	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1026	0	1026	0	0	5-8	5-8
H	1026	0	1026	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	800	520 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	800	520 / 0	139 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	I- D	0 / 747	0.12 (1)
B- C	0 / 24	-104.9	-104.9	0.17 (1)	10.00	I- E	-96 / 54	0.03 (1)
C- D	-821 / 0	-104.9	-104.9	0.14 (1)	6.25	C- I	-96 / 54	0.03 (1)
D- E	-821 / 0	-104.9	-104.9	0.14 (1)	6.25	J- C	-1124 / 0	0.51 (1)
E- F	0 / 24	-104.9	-104.9	0.17 (1)	10.00	E- H	-1124 / 0	0.51 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00			
J- B	-281 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-281 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 732	-28.0	-28.0	0.43 (2)	10.00			
I- H	0 / 732	-28.0	-28.0	0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 685 (0.23")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.43/1.00 (I-J:2) , WB=0.51/1.00 (C-J:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

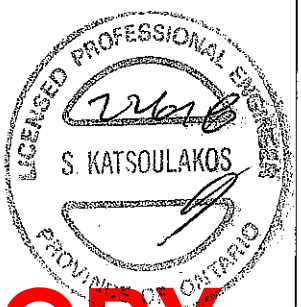
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)



DWG NO. TAM10195-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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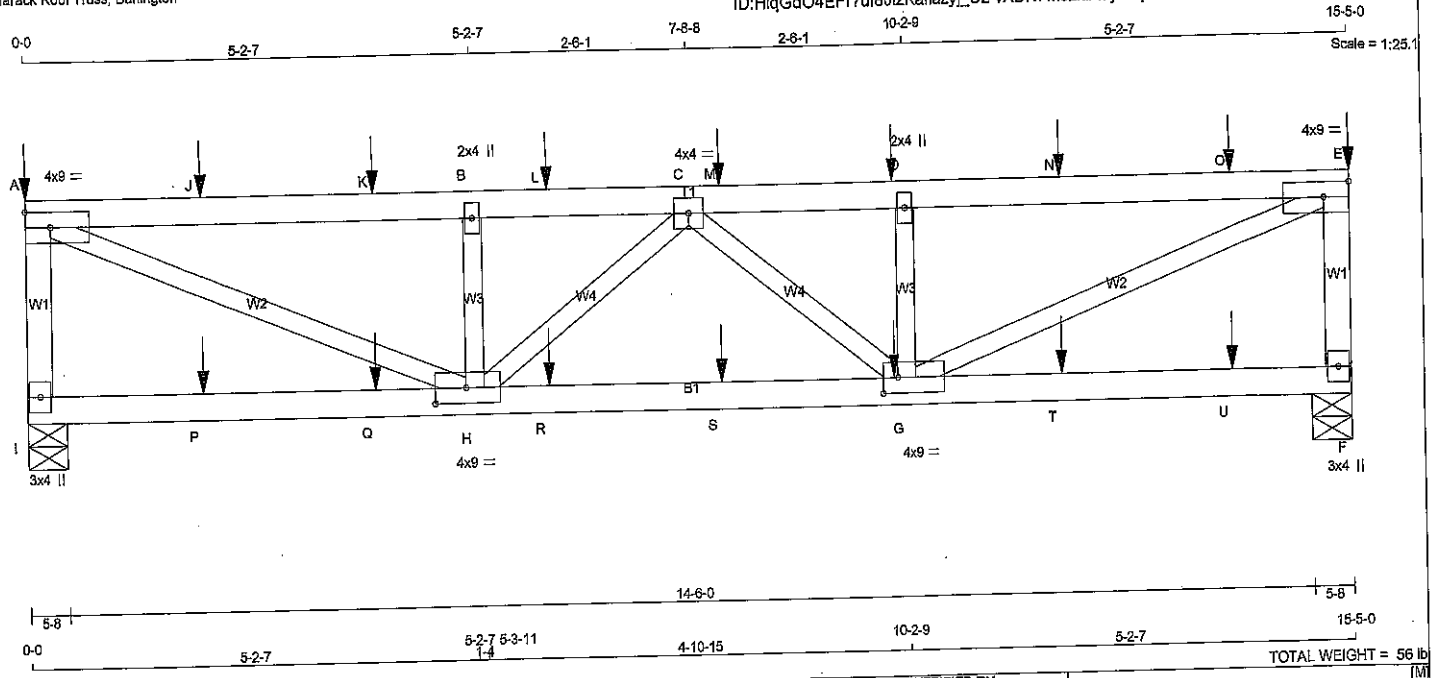


TOTAL WEIGHT = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	4.0	9.0		Edge
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	9.0		Edge
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0	2.00	2.25
H	BMWWW-t	MT20	4.0	9.0	2.00	4.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 73.9 lbs FACTORED DOWN AT 0-0, 24.8 lbs FACTORED DOWN AT 2-0-12, 24.8 lbs FACTORED DOWN AT 4-0-12, 24.8 lbs FACTORED DOWN AT 6-0-12, 24.8 lbs FACTORED DOWN AT 8-0-12, 24.8 lbs FACTORED DOWN AT 10-0-12, 24.8 lbs FACTORED DOWN AT 12-0-12, AND 24.8 lbs FACTORED DOWN AT 14-0-12, AND 73.9 lbs FACTORED DOWN AT 15-5-0 ON TOP CHORD, AND 19.6 lbs FACTORED DOWN AT 2-0-12, 19.6 lbs FACTORED DOWN AT 4-0-12, 19.6 lbs FACTORED DOWN AT 6-0-12, 19.6 lbs FACTORED DOWN AT 8-0-12, 19.6 lbs FACTORED DOWN AT 10-0-12, AND 19.6 lbs FACTORED DOWN AT 12-0-12, AND 19.6 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1218	0	1218	0	0	5-8	5-8
F	1231	0	1231	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	969	597 / 0	190 / 0	0 / 0	0 / 0	182 / 0	0 / 0
F	980	603 / 0	193 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
I- A	-1147 / 0	0.0	0.0	0.15 (1)	7.39
A- J	-2029 / 0	-104.9	-104.9	0.54 (1)	4.10
J- K	-2029 / 0	-104.9	-104.9	0.54 (1)	4.10
K- B	-2029 / 0	-104.9	-104.9	0.54 (1)	4.10
B- L	-2029 / 0	-104.9	-104.9	0.45 (1)	4.15
L- C	-2029 / 0	-104.9	-104.9	0.45 (1)	4.15
C- M	-2033 / 0	-104.9	-104.9	0.45 (1)	4.16
M- D	-2033 / 0	-104.9	-104.9	0.45 (1)	4.16
D- N	-2033 / 0	-104.9	-104.9	0.53 (1)	4.10
N- O	-2033 / 0	-104.9	-104.9	0.53 (1)	4.10
O- E	-2033 / 0	-104.9	-104.9	0.53 (1)	4.10
F- E	-1156 / 0	0.0	0.0	0.15 (1)	7.37
I- P	0 / 0	-28.0	-28.0	0.21 (3)	10.00
P- Q	0 / 0	-28.0	-28.0	0.21 (3)	10.00
Q- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00
H- R	0 / 2058	-28.0	-28.0	0.45 (1)	10.00
R- S	0 / 2058	-28.0	-28.0	0.45 (1)	10.00
S- G	0 / 2058	-28.0	-28.0	0.45 (1)	10.00
G- T	0 / 0	-28.0	-28.0	0.21 (2)	10.00
T- U	0 / 0	-28.0	-28.0	0.21 (2)	10.00
U- F	0 / 0	-28.0	-28.0	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 2197	0.54 (1)
A- H	0 / 2192	0.54 (1)
G- D	-628 / 0	0.11 (1)
H- B	-624 / 0	0.11 (1)
H- C	-37 / 0	0.01 (1)
C- G	-32 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
A	0-0	-74	-74	---	FRONT	VERT	TOTAL
D	10-0-12	-25	-25	---	FRONT	VERT	TOTAL
E	15-5-0	-74	-74	---	FRONT	VERT	TOTAL
G	10-0-12	-11	-20	---	FRONT	VERT	TOTAL
J	2-0-12	-25	-25	---	FRONT	VERT	TOTAL
K	4-0-12	-25	-25	---	FRONT	VERT	TOTAL
L	6-0-12	-25	-25	---	FRONT	VERT	TOTAL
M	8-0-12	-25	-25	---	FRONT	VERT	TOTAL
N	12-0-12	-25	-25	---	FRONT	VERT	TOTAL
O	14-0-12	-25	-25	---	FRONT	VERT	TOTAL
P	2-0-12	-11	-20	---	FRONT	VERT	TOTAL
Q	4-0-12	-11	-20	---	FRONT	VERT	TOTAL
R	6-0-12	-11	-20	---	FRONT	VERT	TOTAL
S	8-0-12	-11	-20	---	FRONT	VERT	TOTAL
T	12-0-12	-11	-20	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.54/1.00 (A-B:1) , BC=0.45/1.00 (G-H:1) , WB=0.54/1.00 (E-G:1) , SSI=0.30/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

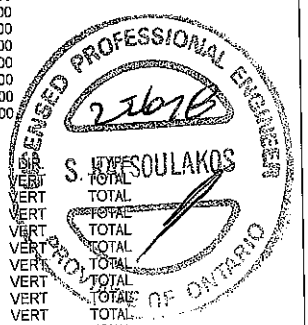
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.37 (G) (INPUT = 1.00)



DWG NO. TAM10196-18 R/L

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T15	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	14-0-12	-11	-20	—	FRONT	VERT	TOTAL

DWG NO. TAM10196-R8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T16	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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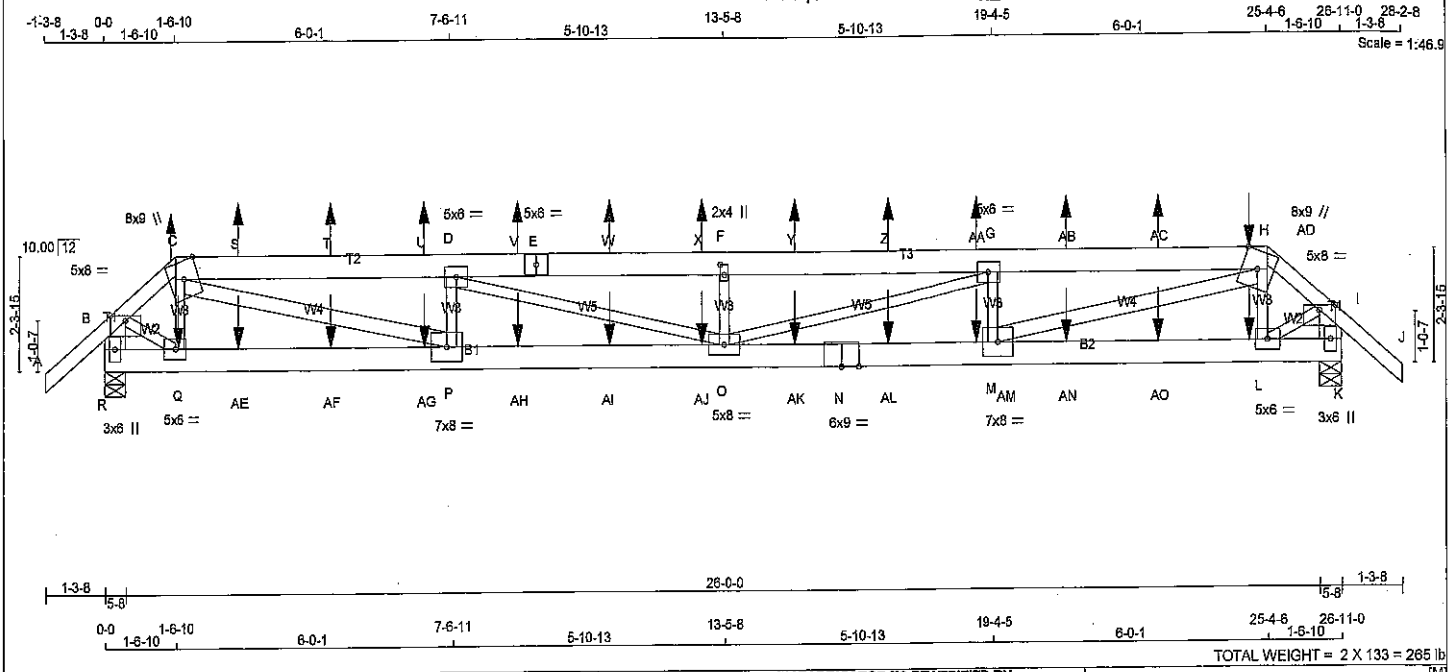


TOTAL WEIGHT = 2 X 133 = 265 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - P	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
H- J	1	12	TOP
C- E	2	12	SIDE(61.0)
E- H	2	12	SIDE(122.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	5	SIDE(72.1)
L- H	1	5	SIDE(78.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	7.0	8.0		
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3781	0	3781	0	0	5-8	5-8
K	3694	0	3694	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2992	1865 / 0	567 / 0	0 / 0	0 / 0	560 / 0	0 / 0
K	2920	1827 / 0	550 / 0	0 / 0	0 / 0	543 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

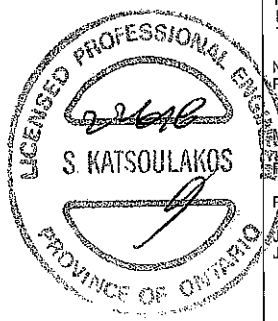
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-4215 / 0	-104.9	-104.9	0.13 (1)	4.54
C- S	-10716 / 0	-104.9	-104.9	0.38 (1)	3.50
S- T	-10716 / 0	-104.9	-104.9	0.38 (1)	3.50
T- U	-10716 / 0	-104.9	-104.9	0.38 (1)	3.50
U- D	-10716 / 0	-104.9	-104.9	0.38 (1)	3.50
D- V	-13083 / 0	-104.9	-104.9	0.51 (1)	3.08
V- E	-13083 / 0	-104.9	-104.9	0.51 (1)	3.08
E- W	-13083 / 0	-104.9	-104.9	0.51 (1)	3.08
W- X	-13083 / 0	-104.9	-104.9	0.51 (1)	3.08
X- F	-13083 / 0	-104.9	-104.9	0.51 (1)	3.08
F- Y	-13083 / 0	-104.9	-104.9	0.50 (1)	3.08
Y- Z	-13083 / 0	-104.9	-104.9	0.50 (1)	3.08
Z-AA	-13083 / 0	-104.9	-104.9	0.50 (1)	3.08
AA- G	-13083 / 0	-104.9	-104.9	0.50 (1)	3.08
G-AB	-10649 / 0	-104.9	-104.9	0.38 (1)	3.52
AB-AC	-10649 / 0	-104.9	-104.9	0.38 (1)	3.52
AC-AD	-10649 / 0	-104.9	-104.9	0.38 (1)	3.52
AD- H	-10649 / 0	-104.9	-104.9	0.38 (1)	3.52
H- I	-4082 / 0	-104.9	-104.9	0.13 (1)	4.60
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
R- B	-4144 / 0	0.0	0.0	0.15 (1)	7.00
K- I	-4021 / 0	0.0	0.0	0.14 (1)	7.09
R- Q	0 / 0	-28.0	-28.0	0.23 (1)	10.00
Q-AE	0 / 3227	-28.0	-28.0	0.44 (1)	10.00
AE-AF	0 / 3227	-28.0	-28.0	0.44 (1)	10.00
AF-AG	0 / 3227	-28.0	-28.0	0.44 (1)	10.00
AG- P	0 / 3227	-28.0	-28.0	0.44 (1)	10.00
P-AH	0 / 10716	-28.0	-28.0	0.93 (1)	10.00
AH-AI	0 / 10716	-28.0	-28.0	0.93 (1)	10.00
AI-AJ	0 / 10716	-28.0	-28.0	0.93 (1)	10.00
AJ- O	0 / 10716	-28.0	-28.0	0.93 (1)	10.00
O-AK	0 / 10649	-28.0	-28.0	0.92 (1)	10.00
AK- N	0 / 10649	-28.0	-28.0	0.92 (1)	10.00
N-AL	0 / 10649	-28.0	-28.0	0.92 (1)	10.00
AL-AM	0 / 10649	-28.0	-28.0	0.92 (1)	10.00
AM-M	0 / 10649	-28.0	-28.0	0.92 (1)	10.00
M-AN	0 / 3117	-28.0	-28.0	0.42 (1)	10.00
AN-AO	0 / 3117	-28.0	-28.0	0.42 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	0 / 168	0.02 (3)
C- P	0 / 7851	0.69 (1)
P- D	-1456 / 0	0.12 (1)
D- O	0 / 2482	0.31 (1)
O- F	-543 / 0	0.04 (1)
O- G	0 / 2553	0.32 (1)
M- G	-1481 / 0	0.12 (1)
M- H	0 / 7896	0.70 (1)
L- H	-110 / 119	0.01 (3)
B- Q	0 / 3444	0.43 (1)
L- I	0 / 3334	0.41 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 722 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 464 (0.70")

CSI: TC=0.51/1.00 (D-F:1) , BC=0.93/1.00 (O-P:1) , WB=0.70/1.00 (H-M:1) , SSI=0.19/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.85 (N) (INPUT = 1.00)

pb 66

DWG NO. TAM 10197-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295155	T16	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.6 lbs FACTORED DOWN AND 1.7 lbs FACTORED UP AT 1-6-10, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 2-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 4-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 6-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 8-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 10-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 12-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 14-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 16-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 18-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 20-11-12, AND 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 22-11-12, AND 24.3 lbs FACTORED DOWN AND 2.9 lbs FACTORED UP AT 24-11-12 ON TOP CHORD, AND 276.4 lbs FACTORED DOWN AT 1-7-6, 276.4 lbs FACTORED DOWN AT 2-11-12, 276.4 lbs FACTORED DOWN AT 4-11-12, 276.4 lbs FACTORED DOWN AT 6-11-12, 276.4 lbs FACTORED DOWN AT 8-11-12, 276.4 lbs FACTORED DOWN AT 10-11-12, 276.4 lbs FACTORED DOWN AT 12-11-12, 276.4 lbs FACTORED DOWN AT 14-11-12, 276.4 lbs FACTORED DOWN AT 16-11-12, 276.4 lbs FACTORED DOWN AT 18-11-12, 276.4 lbs FACTORED DOWN AT 20-11-12, AND 276.4 lbs FACTORED DOWN AT 22-11-12, AND 276.4 lbs FACTORED DOWN AT 24-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
AO - L	0 / 3117	-28.0	-28.0	0.42 (1)	10.00				
L - K	0 / 0	-28.0	-28.0	0.21 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-6-10	-0	-0	2	BACK	VERT	TOTAL
L	24-11-12	-276	-276	—	BACK	VERT	TOTAL
Q	1-7-6	-276	-276	—	BACK	VERT	TOTAL
S	2-11-12	1	1	3	BACK	VERT	TOTAL
T	4-11-12	1	1	3	BACK	VERT	TOTAL
U	6-11-12	1	1	3	BACK	VERT	TOTAL
V	8-11-12	1	1	3	BACK	VERT	TOTAL
W	10-11-12	1	1	3	BACK	VERT	TOTAL
X	12-11-12	1	1	3	BACK	VERT	TOTAL
Y	14-11-12	1	1	3	BACK	VERT	TOTAL
Z	16-11-12	1	1	3	BACK	VERT	TOTAL
AA	18-11-12	1	1	3	BACK	VERT	TOTAL
AB	20-11-12	1	1	3	BACK	VERT	TOTAL
AC	22-11-12	1	1	3	BACK	VERT	TOTAL
AD	24-11-12	-24	-24	3	BACK	VERT	TOTAL
AE	2-11-12	-276	-276	—	BACK	VERT	TOTAL
AF	4-11-12	-276	-276	—	BACK	VERT	TOTAL
AG	6-11-12	-276	-276	—	BACK	VERT	TOTAL
AH	8-11-12	-276	-276	—	BACK	VERT	TOTAL
AI	10-11-12	-276	-276	—	BACK	VERT	TOTAL
AJ	12-11-12	-276	-276	—	BACK	VERT	TOTAL
AK	14-11-12	-276	-276	—	BACK	VERT	TOTAL
AL	16-11-12	-276	-276	—	BACK	VERT	TOTAL
AM	18-11-12	-276	-276	—	BACK	VERT	TOTAL
AN	20-11-12	-276	-276	—	BACK	VERT	TOTAL
AO	22-11-12	-276	-276	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
22618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10197-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 26 09:57:09 2018 Page 1
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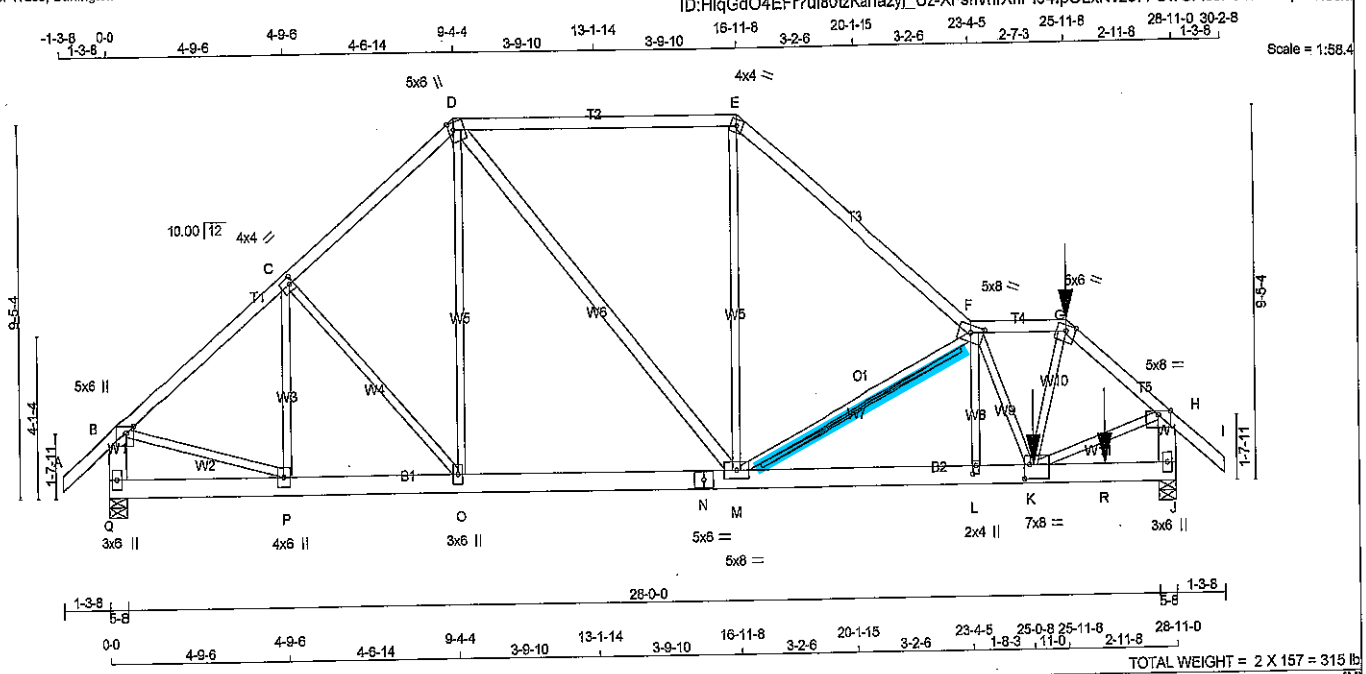

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	SIDE(0.0)
G- I	1	12	SIDE(0.0)
Q- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	TOP
N- J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWWW-m	MT20	5.0	8.0	2.25	4.00
G	TTW-m	MT20	5.0	6.0	Edge	
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWWW-t	MT20	7.0	8.0	4.25	1.75
L	BMW+w	MT20	2.0	4.0	2.50	1.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2509	0	2509	0	0	5-8	5-8
J	4941	0	4941	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1979	1248 / 0	369 / 0	0 / 0	0 / 0	362 / 0	0 / 0
J	3910	2444 / 0	744 / 0	0 / 0	0 / 0	722 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-2475 / 0	-104.9	-104.9	0.26 (1)	5.47
C- D	-2398 / 0	-104.9	-104.9	0.25 (1)	5.54
D- E	-2331 / 0	-104.9	-104.9	0.69 (1)	4.87
E- F	-3017 / 0	-104.9	-104.9	0.59 (1)	4.71
F- G	-4678 / 0	-104.9	-104.9	0.14 (1)	4.34
G- H	-5141 / 0	-104.9	-104.9	0.18 (1)	4.13
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00
Q- B	-2440 / 0	0.0	0.0	0.09 (1)	7.81
J- H	-4832 / 0	0.0	0.0	0.18 (1)	6.59
Q- P	0 / 0	-28.0	-28.0	0.04 (2)	10.00
P- O	0 / 1929	-28.0	-28.0	0.16 (1)	10.00
O- N	0 / 1813	-28.0	-28.0	0.18 (1)	10.00
N- M	0 / 1813	-28.0	-28.0	0.18 (1)	10.00
M- L	0 / 5069	-28.0	-28.0	0.40 (1)	10.00
L- K	0 / 5070	-28.0	-28.0	0.38 (1)	10.00
K- R	0 / 0	-28.0	-28.0	0.06 (2)	10.00
R- J	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-381 / 20	0.09 (1)
C- O	-180 / 0	0.08 (1)
O- D	0 / 378	0.05 (2)
D- M	0 / 812	0.07 (1)
M- E	0 / 1196	0.15 (1)
M- F	-3167 / 0	0.88 (1)
L- F	0 / 164	0.02 (3)
B- P	0 / 1991	0.25 (1)
K- H	0 / 4121	0.51 (1)
K- G	0 / 2925	0.36 (1)
F- K	-976 / 0	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. TYPE
G	25-11-8	-188	-188	---	FRONT	VERT TOTAL
K	25-0-8	-3087	-3087	---	BACK	VERT TOTAL
R	26-11-12	-40	-70	---	BACK	VERT TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.40/1.00 (L-M:1) , WB=0.88/1.00 (F-M:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.60 (G) (INPUT = 1.00)

PLEASE SEE PAGE 4

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T20	1	2	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.2 lbs FACTORED DOWN AT 25-11-8 ON TOP CHORD, AND 3086.8 lbs FACTORED DOWN AT 25-0-8, AND 69.9 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
22078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17208-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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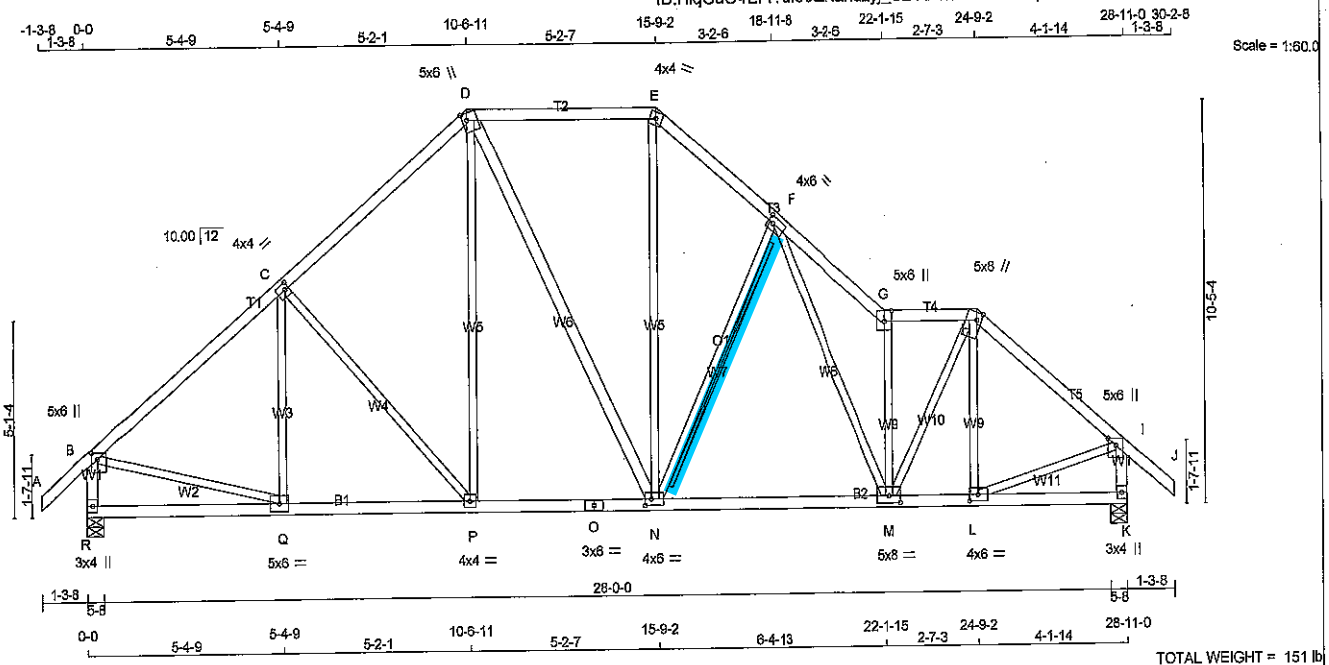


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0	2.00	1.75
G	TTW+p	MT20	5.0	6.0	3.25	2.50
H	TTWW+m	MT20	5.0	8.0	2.25	1.50
I	TMVW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMWWW-t	MT20	5.0	8.0	2.25	4.00
N	BMWWW-t	MT20	4.0	6.0	2.00	2.00
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2067	0	2067	0	0	5-8	5-8
K	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

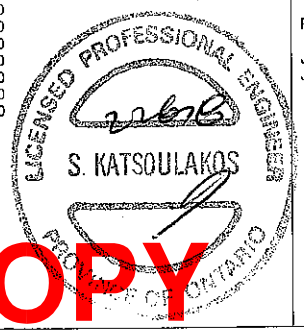
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1947 / 0	-104.9	-104.9	0.46 (1)		4.37
C- D	-1704 / 0	-104.9	-104.9	0.43 (1)		4.64
D- E	-1371 / 0	-104.9	-104.9	0.39 (1)		5.08
E- F	-1808 / 0	-104.9	-104.9	0.18 (1)		4.82
F- G	-2698 / 0	-104.9	-104.9	0.23 (1)		4.05
G- H	-2058 / 0	-104.9	-104.9	0.13 (1)		4.62
H- I	-1886 / 0	-104.9	-104.9	0.29 (1)		4.63
I- J	0 / 47	-104.9	-104.9	0.14 (1)		10.00
R- B	-2006 / 0	0.0	0.0	0.21 (1)		5.96
K- I	-2018 / 0	0.0	0.0	0.21 (1)		5.94
R- Q	0 / 0	-28.0	-28.0	0.20 (3)		10.00
Q- P	0 / 1528	-28.0	-28.0	0.36 (2)		10.00
P- O	0 / 1277	-28.0	-28.0	0.37 (2)		10.00
O- N	0 / 1277	-28.0	-28.0	0.37 (2)		10.00
N- M	0 / 1700	-28.0	-28.0	0.42 (1)		10.00
M- L	0 / 1438	-28.0	-28.0	0.30 (2)		10.00
L- K	0 / 0	-28.0	-28.0	0.11 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-201 / 144	0.12 (1)
C- P	-378 / 0	0.48 (1)
P- D	0 / 404	0.09 (1)
D- N	0 / 208	0.03 (1)
N- E	0 / 744	0.17 (1)
N- F	-815 / 0	0.89 (1)
F- M	0 / 996	0.22 (1)
M- G	-1995 / 0	0.77 (1)
M- H	0 / 1343	0.30 (1)
L- H	-334 / 0	0.13 (1)
B- Q	0 / 1572	0.35 (1)
L- I	0 / 1511	0.34 (1)



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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.42/1.00 (M-N:1) , WB=0.89/1.00 (F-N:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

DWG NO. TAM 10209-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T22	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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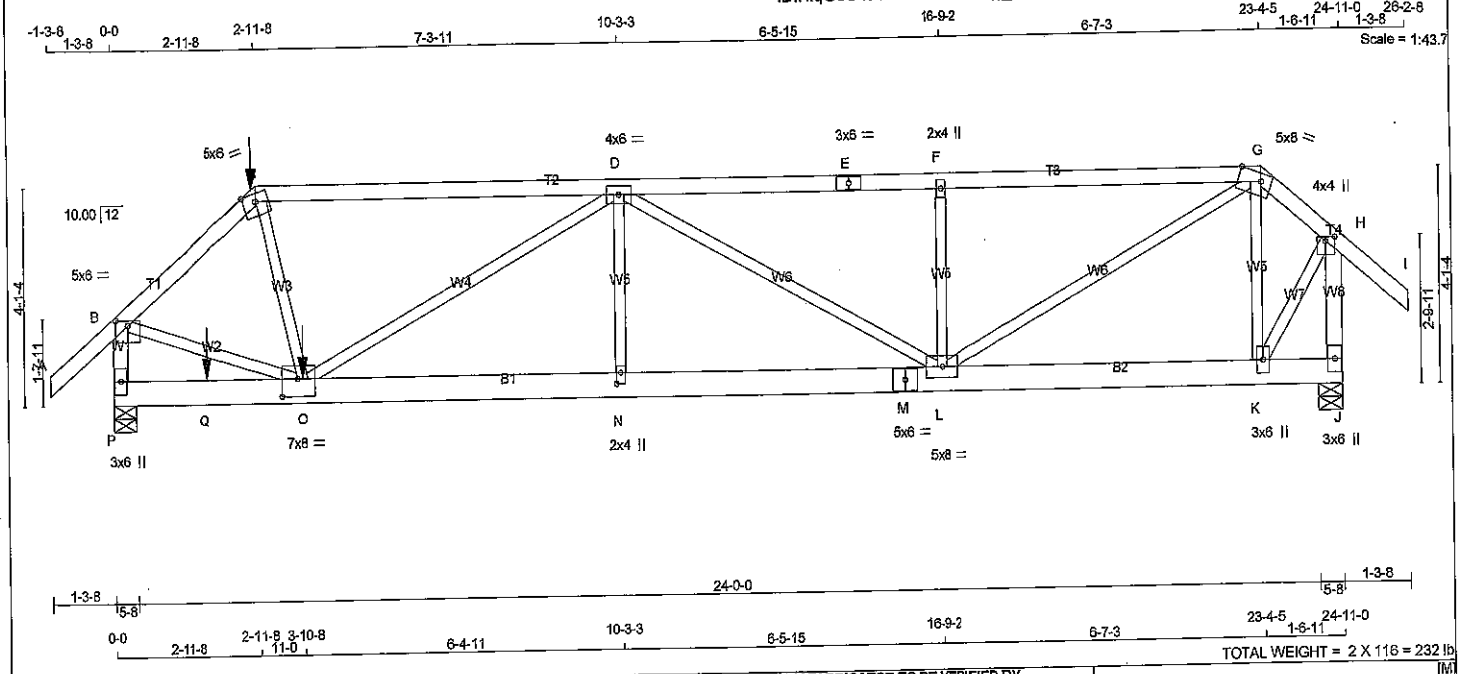

TOTAL WEIGHT = 2 X 116 = 232 lb

[M]

LUMBER

N.L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(0.0)
C- E	1	12	SIDE(0.0)
E- G	1	12	TOP
G- I	1	12	TOP
P- B	1	12	TOP
J- H	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	12	SIDE(0.0)
M- J	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	1.25	3.00
C	TTW-m	MT20	5.0	6.0	Edge	
D	TMWWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	5.50
H	TMW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMW+w	MT20	2.0	4.0	2.50	1.00
O	BMWWW-t	MT20	7.0	8.0	4.00	3.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	4345	0	4345	0	0	5-8	5-8
J	2270	0	2270	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	3418	2171 / 0	628 / 0	0 / 0	0 / 0	620 / 0	0 / 0
J	1784	1136 / 0	326 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-4488 / 0	-104.9	-104.9	0.16 (1)	4.40
C- D	-4018 / 0	-104.9	-104.9	0.60 (1)	4.19
D- E	-3535 / 0	-104.9	-104.9	0.52 (1)	4.44
E- F	-3535 / 0	-104.9	-104.9	0.52 (1)	4.44
F- G	-3534 / 0	-104.9	-104.9	0.43 (1)	4.57
G- H	-1222 / 0	-104.9	-104.9	0.08 (1)	6.25
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00
P- B	-4257 / 0	0.0	0.0	0.24 (1)	5.76
J- H	-2350 / 0	0.0	0.0	0.18 (1)	7.32
P- Q	0 / 0	-28.0	-28.0	0.05 (2)	10.00
Q- O	0 / 0	-28.0	-28.0	0.05 (2)	10.00
O- N	0 / 4645	-28.0	-28.0	0.35 (1)	10.00
N- M	0 / 4645	-28.0	-28.0	0.35 (1)	10.00
M- L	0 / 4645	-28.0	-28.0	0.35 (1)	10.00
L- K	0 / 871	-28.0	-28.0	0.12 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.07 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- G	-1047 / 0	0.13 (1)
B- O	0 / 3597	0.45 (1)
K- H	0 / 1578	0.20 (1)
L- G	0 / 3055	0.38 (1)
L- F	-721 / 0	0.09 (1)
N- D	0 / 332	0.04 (2)
D- L	-1281 / 0	0.71 (1)
C- O	0 / 2294	0.28 (1)
O- D	-727 / 0	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-188	-188	---	FRONT	VERT	TOTAL
O	3-10-8	-2784	-2784	---	BACK	VERT	TOTAL
Q	1-11-4	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.60/1.00 (C-D:1) , BC=0.35/1.00 (N-O:1) , WB=0.71/1.00 (D-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10210-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T22	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.2 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, AND 2783.5 lbs FACTORED DOWN AT 3-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0210-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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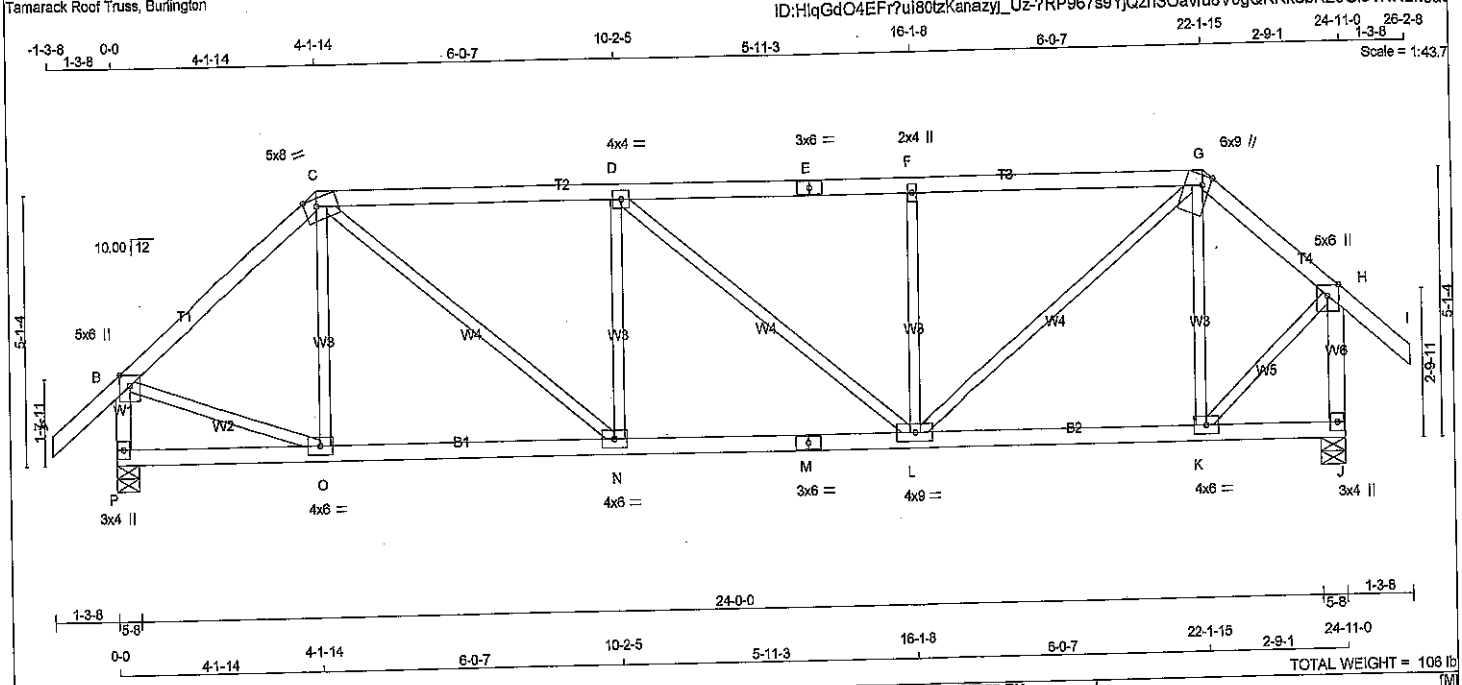

TOTAL WEIGHT = 106 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K, N, O						
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1601	0	1601	0	0	5-8	5-8
J	1601	0	1601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B-C	-1591 / 0	-104.9	-104.9	0.38 (1)	4.81
C-D	-2163 / 0	-104.9	-104.9	0.67 (1)	3.87
D-E	-2047 / 0	-104.9	-104.9	0.65 (1)	3.97
E-F	-2047 / 0	-104.9	-104.9	0.65 (1)	3.97
F-G	-2047 / 0	-104.9	-104.9	0.65 (1)	3.98
G-H	-1146 / 0	-104.9	-104.9	0.15 (1)	5.76
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P-B	-1759 / 0	0.0	0.0	0.19 (1)	6.29
J-H	-1786 / 0	0.0	0.0	0.27 (1)	6.25
P-O	0 / 0	-28.0	-28.0	0.17 (2)	10.00
O-N	0 / 1214	-28.0	-28.0	0.32 (2)	10.00
N-M	0 / 2163	-28.0	-28.0	0.45 (1)	10.00
M-L	0 / 2163	-28.0	-28.0	0.45 (1)	10.00
L-K	0 / 864	-28.0	-28.0	0.29 (2)	10.00
K-J	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
O-C	-213 / 113	0.08 (1)
C-N	0 / 1212	0.27 (1)
N-D	-587 / 0	0.23 (1)
D-L	-149 / 0	0.17 (1)
L-F	-680 / 0	0.26 (1)
L-G	0 / 1511	0.34 (1)
K-G	-610 / 0	0.23 (1)
B-O	0 / 1275	0.29 (1)
K-H	0 / 1159	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.45/1.00 (L-N:1) , WB=0.34/1.00 (G-L:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.63 (M) (INPUT = 1.00)

DWG NO. TAM 10241-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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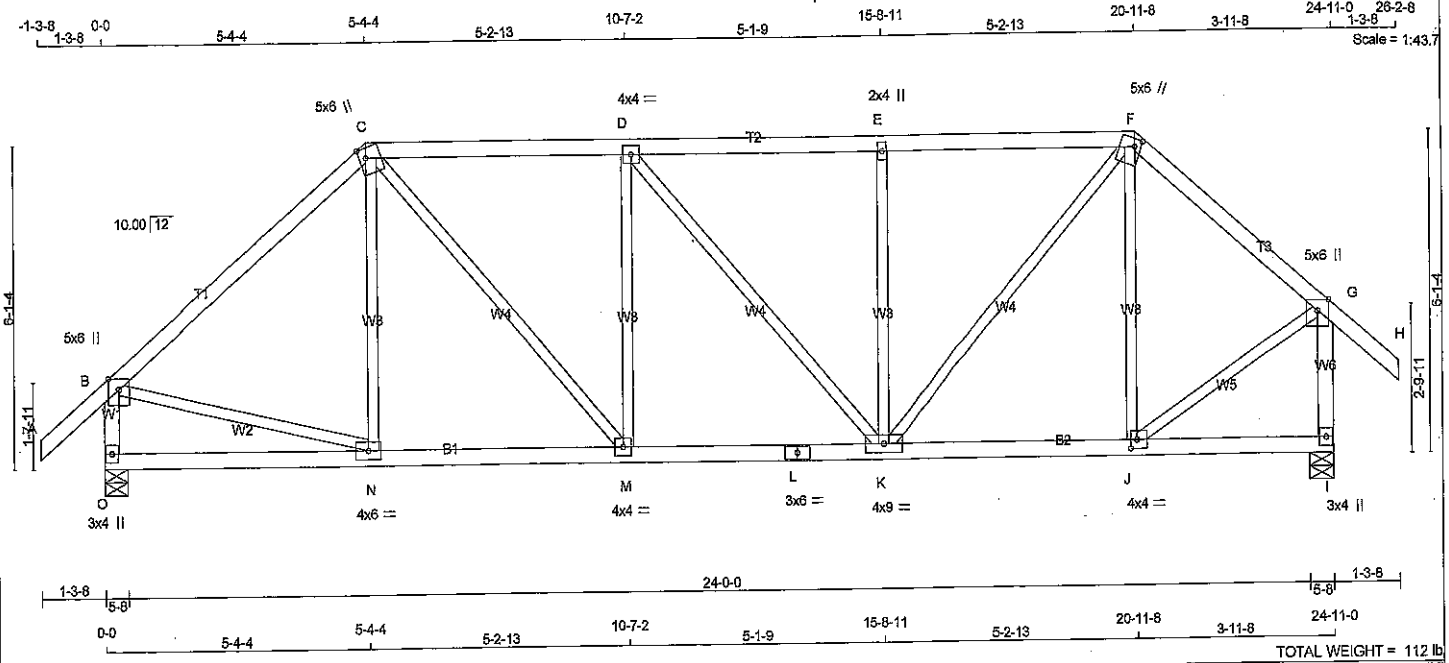

TOTAL WEIGHT = 112 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	1.75	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1801	0	1801	0	0	5-8	5-8
I	1801	0	1801	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	N- C	-122 / 168	0.07 (1)
B- C	-1592 / 0	-104.9	-104.9	0.66 (1)	4.42	C- M	0 / 854	0.19 (1)
C- D	-1790 / 0	-104.9	-104.9	0.46 (1)	4.47	M- D	-498 / 0	0.29 (1)
D- E	-1708 / 0	-104.9	-104.9	0.45 (1)	4.56	D- K	-125 / 0	0.15 (1)
E- F	-1707 / 0	-104.9	-104.9	0.44 (1)	4.57	K- E	-587 / 0	0.34 (1)
F- G	-1269 / 0	-104.9	-104.9	0.32 (1)	5.32	K- F	0 / 1109	0.25 (1)
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- F	-445 / 0	0.26 (1)
O- B	-1740 / 0	0.0	0.0	0.19 (1)	6.32	B- N	0 / 1255	0.28 (1)
I- G	-1760 / 0	0.0	0.0	0.26 (1)	6.29	J- G	0 / 1138	0.26 (1)
O- N	0 / 0	-28.0	-28.0	0.20 (3)	10.00			
N- M	0 / 1218	-28.0	-28.0	0.33 (2)	10.00			
M- L	0 / 1790	-28.0	-28.0	0.35 (1)	10.00			
L- K	0 / 1790	-28.0	-28.0	0.35 (1)	10.00			
K- J	0 / 964	-28.0	-28.0	0.25 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.35/1.00 (K-M:1) , WB=0.34/1.00 (E-K:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.52 (L) (INPUT = 1.00)

DWG NO. TAM 10212-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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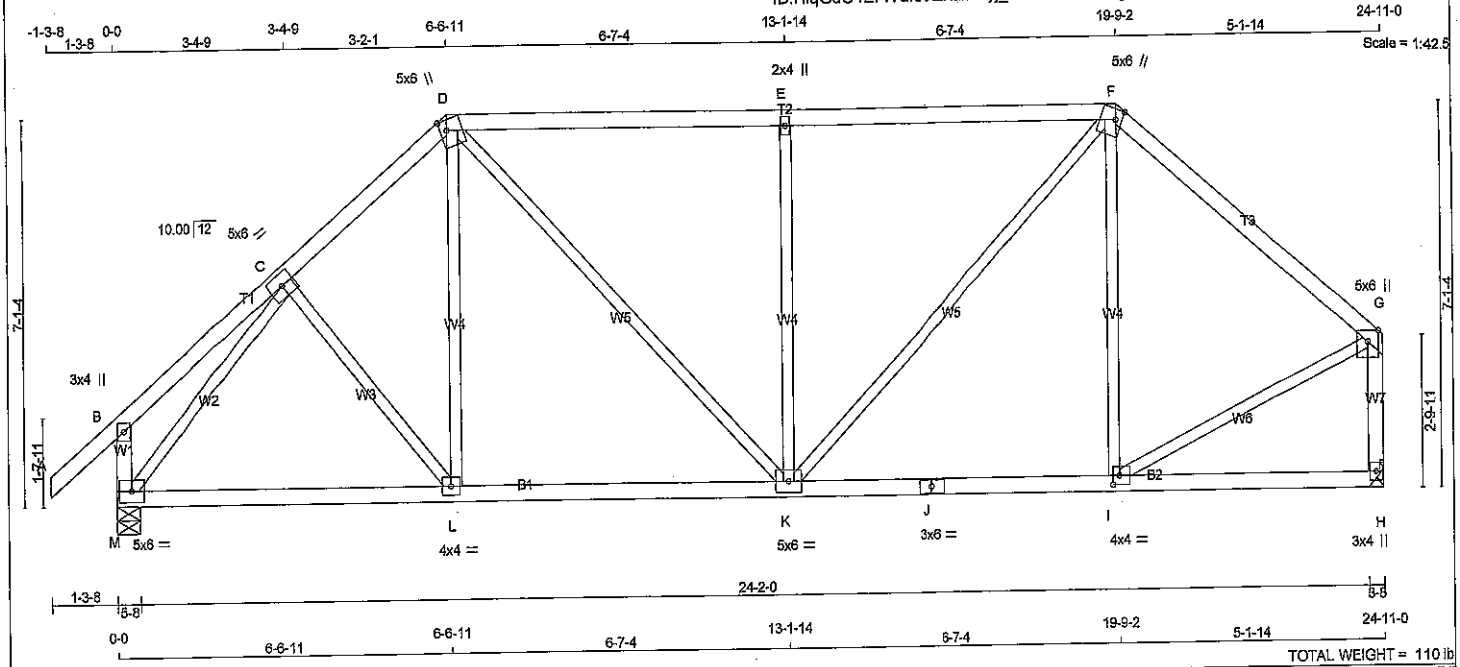

TOTAL WEIGHT = 110 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1801	0	1801	0	0	IN-SX	IN-SX
H	1656	0	1656	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
H	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.17 (1)	10.00
C- D	-1584 / 0	-104.9	-104.9	0.20 (1)	5.04
D- E	-1603 / 0	-104.9	-104.9	0.92 (1)	3.65
E- F	-1603 / 0	-104.9	-104.9	0.92 (1)	3.65
F- G	-1328 / 0	-104.9	-104.9	0.57 (1)	4.86
M- B	-279 / 0	0.0	0.0	0.03 (1)	7.81
H- G	-1601 / 0	0.0	0.0	0.24 (1)	6.53
M- L	0 / 1172	-28.0	-28.0	0.42 (2)	10.00
L- K	0 / 1198	-28.0	-28.0	0.43 (2)	10.00
K- J	0 / 1013	-28.0	-28.0	0.35 (2)	10.00
J- I	0 / 1013	-28.0	-28.0	0.35 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.24 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	0 / 119	0.03 (3)
L- D	0 / 284	0.06 (2)
D- K	0 / 581	0.13 (1)
K- E	-851 / 0	0.74 (1)
K- F	0 / 846	0.19 (1)
I- F	-292 / 112	0.25 (1)
M- C	-1663 / 0	0.80 (1)
I- G	0 / 1125	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.92/1.00 (E-F:1) , BC=0.43/1.00 (K-L:2) , WB=0.80/1.00 (C-M:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10213
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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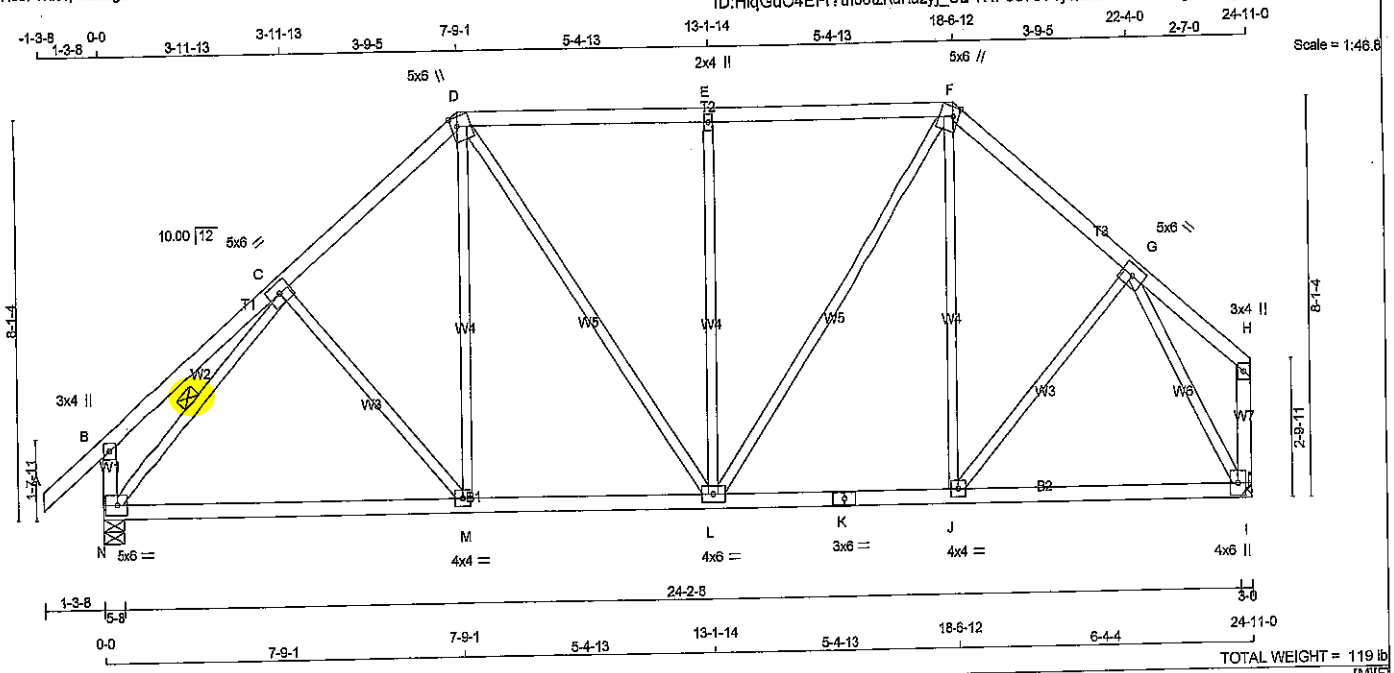

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1801	0	1801	0	0	5-8	5-8
I	1656	0	1656	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1418	899 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 29	-104.9	-104.9	0.25 (1)		10.00
C- D	-1534 / 0	-104.9	-104.9	0.22 (1)		5.10
D- E	-1366 / 0	-104.9	-104.9	0.41 (1)		5.05
E- F	-1366 / 0	-104.9	-104.9	0.41 (1)		5.05
F- G	-1357 / 0	-104.9	-104.9	0.20 (1)		5.36
G- H	0 / 34	-104.9	-104.9	0.19 (1)		10.00
N- B	-301 / 0	0.0	0.0	0.03 (1)		7.81
I- H	-77 / 0	0.0	0.0	0.01 (1)		7.81
N- M	0 / 1203	-28.0	-28.0	0.50 (2)		10.00
M- L	0 / 1157	-28.0	-28.0	0.51 (2)		10.00
L- K	0 / 1022	-28.0	-28.0	0.39 (2)		10.00
K- J	0 / 1022	-28.0	-28.0	0.39 (2)		10.00
J- I	0 / 824	-28.0	-28.0	0.36 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-75 / 92	0.05 (1)
M- D	0 / 363	0.08 (2)
D- L	0 / 367	0.08 (1)
L- E	-692 / 0	0.88 (1)
L- F	0 / 603	0.14 (1)
J- F	-27 / 186	0.04 (3)
J- G	0 / 301	0.07 (1)
N- C	-1853 / 0	0.46 (1)
G- I	-1717 / 0	0.73 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.51/1.00 (L-M:2) , WB=0.88/1.00 (E-L:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10214-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
295157	T27	1	1	44755 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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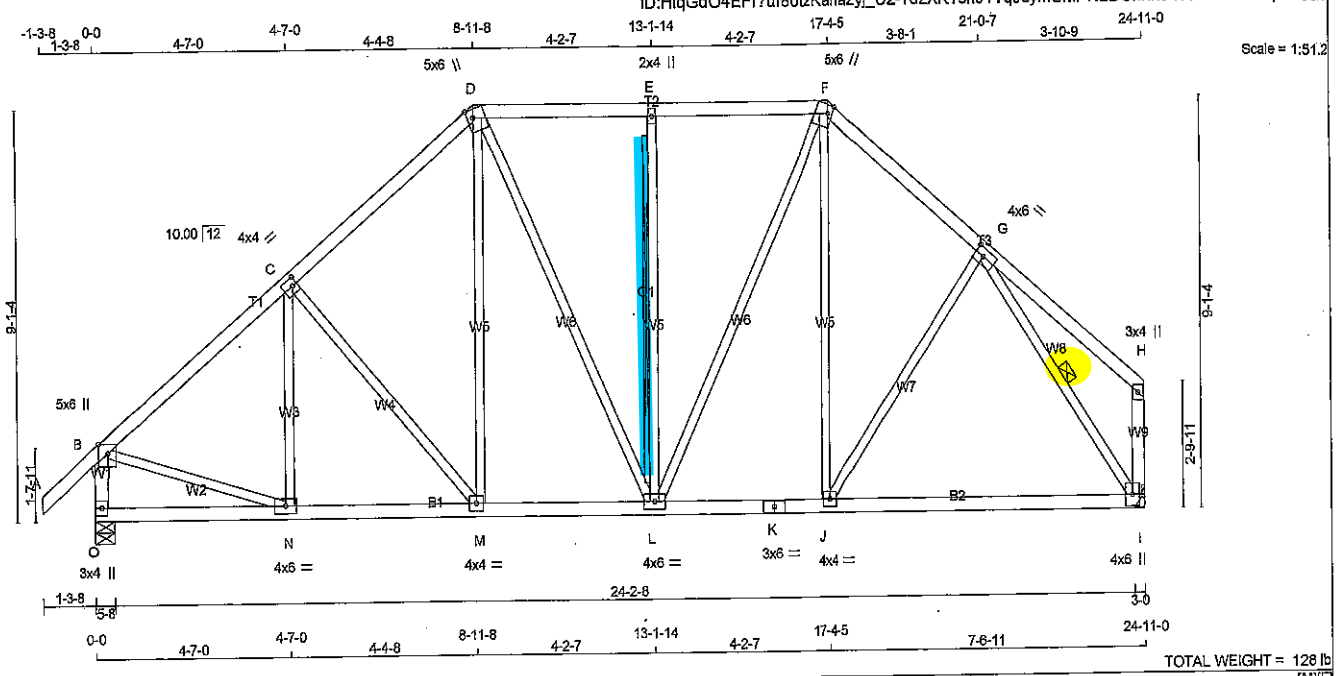

TOTAL WEIGHT = 128 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1801	0	1801	0	0	5-8	5-8
I	1656	0	1656	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1418	699 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.
2x3 DRY SPF No.2 T-BRACE AT E-I.

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1614 / 0	-104.9	-104.9	0.31 (1)	4.90	
C- D	-1450 / 0	-104.9	-104.9	0.30 (1)	5.12	
D- E	-1192 / 0	-104.9	-104.9	0.24 (1)	5.58	
E- F	-1192 / 0	-104.9	-104.9	0.24 (1)	5.58	
F- G	-1345 / 0	-104.9	-104.9	0.20 (1)	5.38	
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	
O- B	-1747 / 0	0.0	0.0	0.19 (1)	6.30	
I- H	-154 / 0	0.0	0.0	0.02 (1)	7.81	
O- N	0 / 0	-28.0	-28.0	0.13 (3)	10.00	
N- M	0 / 1267	-28.0	-28.0	0.27 (1)	10.00	
M- L	0 / 1087	-28.0	-28.0	0.24 (1)	10.00	
L- K	0 / 1010	-28.0	-28.0	0.47 (2)	10.00	
K- J	0 / 1010	-28.0	-28.0	0.47 (2)	10.00	
J- I	0 / 953	-28.0	-28.0	0.45 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-224 / 95	0.10 (1)
C- M	-279 / 0	0.24 (1)
M- D	0 / 354	0.08 (2)
D- L	0 / 243	0.05 (1)
L- E	-531 / 0	0.73 (1)
L- F	0 / 423	0.10 (1)
J- F	0 / 253	0.06 (3)
J- G	0 / 158	0.04 (2)
B- N	0 / 1317	0.30 (1)
G- I	-1705 / 0	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.47/1.00 (J-L:2) , WB=0.73/1.00 (E-L:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
2/26/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10215-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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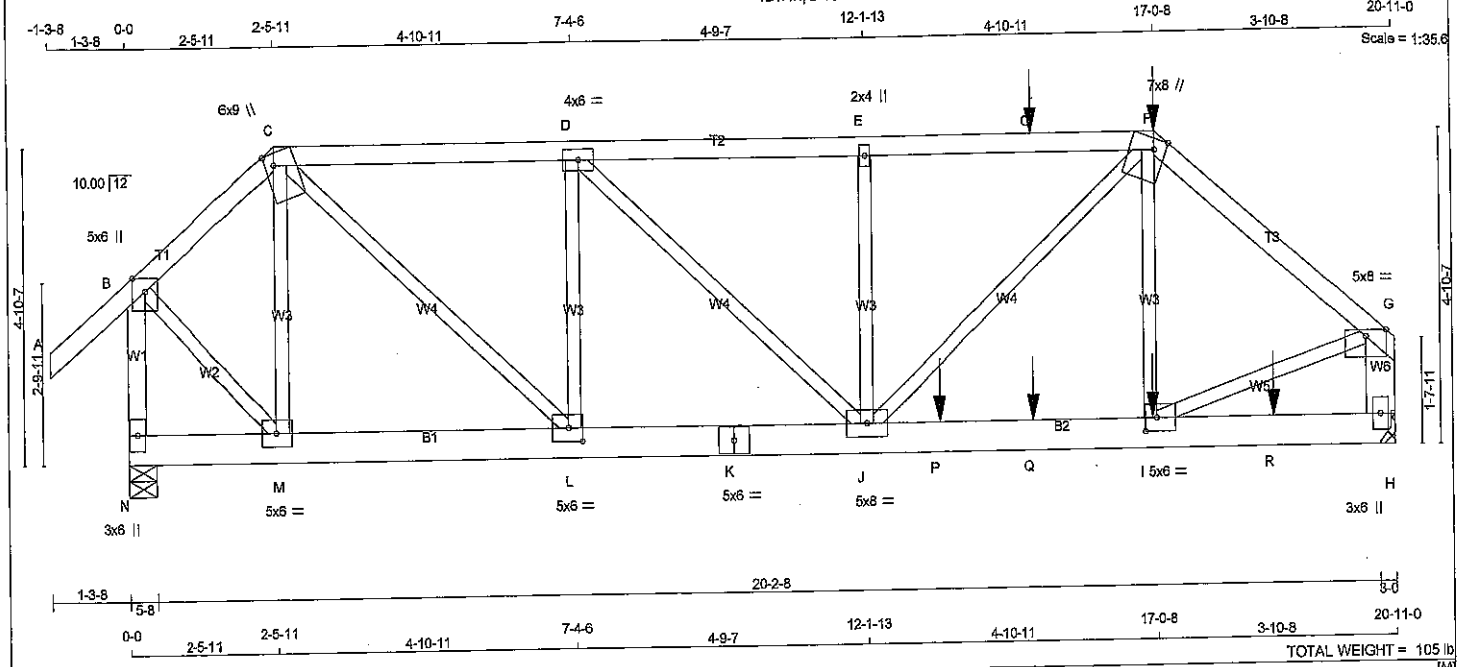

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	7.0	8.0	Edge	2.25
G	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 98.4 lbs FACTORED DOWN AT 14-11-12, AND 330.7 lbs FACTORED DOWN AT 17-0-8 ON TOP CHORD, AND 1177.4 lbs FACTORED DOWN AT 13-4-8, 45.9 lbs FACTORED DOWN AT 14-11-12, AND 45.9 lbs FACTORED DOWN AT 16-11-12, AND 46.8 lbs FACTORED DOWN AT 18-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2064	0	2064	0	0	5-8	5-8
H	2549	0	2549	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1606	1051 / 0	275 / 0	0 / 0	0 / 0	281 / 0	0 / 0
H	1989	1291 / 0	346 / 0	0 / 0	0 / 0	352 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00		M- C	-756 / 0	0.26 (1)
B- C	-1326 / 0	-104.9	-104.9	0.15 (1)	5.44		C- L	0 / 1866	0.45 (1)
C- D	-2373 / 0	-104.9	-104.9	0.47 (1)	3.92		L- D	-1260 / 0	0.43 (1)
D- E	-3144 / 0	-104.9	-104.9	0.61 (1)	3.29		D- J	0 / 1057	0.26 (1)
E- O	-3143 / 0	-104.9	-104.9	0.71 (1)	3.13		J- E	-595 / 0	0.20 (1)
O- F	-3143 / 0	-104.9	-104.9	0.71 (1)	3.13		J- F	0 / 1477	0.37 (1)
F- G	-2683 / 0	-104.9	-104.9	0.46 (1)	3.76		I- F	-81 / 217	0.05 (3)
N- B	-2089 / 0	0.0	0.0	0.31 (1)	5.83		B- M	0 / 1382	0.34 (1)
H- G	-2559 / 0	0.0	0.0	0.19 (1)	6.44		I- G	0 / 2150	0.53 (1)
N- M	0 / 0	-28.0	-28.0	0.08 (2)	10.00				
M- L	0 / 997	-28.0	-28.0	0.21 (1)	10.00				
L- K	0 / 2373	-28.0	-28.0	0.58 (1)	10.00				
K- J	0 / 2373	-28.0	-28.0	0.58 (1)	10.00				
J- P	0 / 2054	-28.0	-28.0	0.85 (1)	10.00				
P- Q	0 / 2054	-28.0	-28.0	0.85 (1)	10.00				
Q- I	0 / 2054	-28.0	-28.0	0.85 (1)	10.00				
I- R	0 / 0	-28.0	-28.0	0.24 (1)	10.00				
R- H	0 / 0	-28.0	-28.0	0.24 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	17-0-8	-331	-331	---	FRONT	VERT	TOTAL
I	16-11-12	-26	-46	---	FRONT	VERT	TOTAL
O	14-11-12	-98	-98	---	FRONT	VERT	TOTAL
P	13-4-8	-1177	-1177	---	FRONT	VERT	TOTAL
Q	14-11-12	-26	-46	---	FRONT	VERT	TOTAL
R	18-11-12	-29	-47	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.71/1.00 (E-F:1) , BC=0.85/1.00 (I-J:1) , WB=0.53/1.00 (G-I:1) , SSI=0.64/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)

DWG NO. TAM 10216-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T29	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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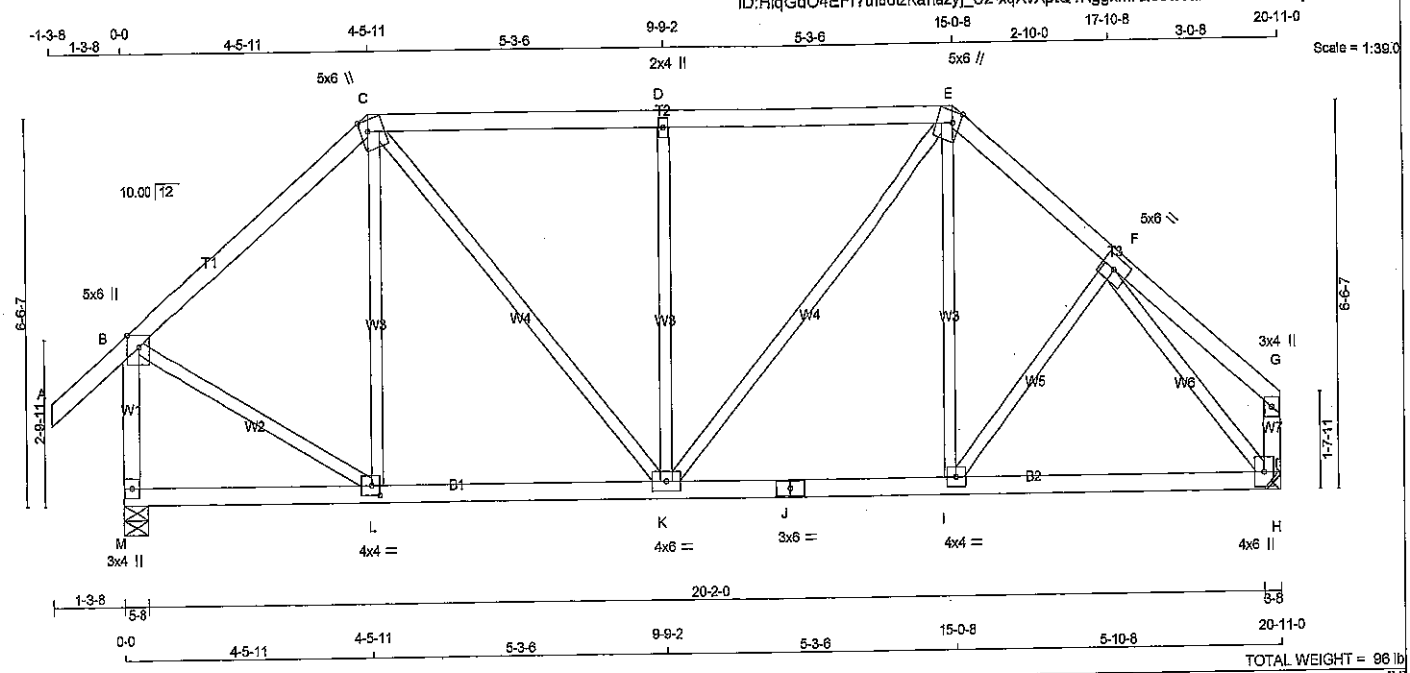

TOTAL WEIGHT = 96 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1536	0	1536	0	0	5-8	5-8
H	1390	0	1390	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1206	769 / 0	220 / 0	0 / 0	0 / 0	217 / 0	0 / 0
H	1108	679 / 0	220 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	1	0.14 (1)	10.00
B- C	-1046 / 0	-104.9	-104.9	1	0.40 (1)	5.59
C- D	-1222 / 0	-104.9	-104.9	1	0.53 (1)	5.02
D- E	-1222 / 0	-104.9	-104.9	1	0.53 (1)	5.02
E- F	-1271 / 0	-104.9	-104.9	1	0.15 (1)	5.55
F- G	0 / 20	-104.9	-104.9	1	0.13 (1)	10.00
M- B	-1487 / 0	0.0	0.0	2	0.22 (1)	6.72
H- G	-121 / 0	0.0	0.0	2	0.01 (1)	7.81
M- L	0 / 0	-28.0	-28.0	2	0.17 (2)	10.00
L- K	0 / 798	-28.0	-28.0	2	0.25 (2)	10.00
K- J	0 / 960	-28.0	-28.0	2	0.33 (2)	10.00
J- I	0 / 960	-28.0	-28.0	2	0.33 (2)	10.00
I- H	0 / 937	-28.0	-28.0	2	0.32 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-276 / 80	0.19 (1)
C- K	0 / 654	0.15 (1)
K- D	-677 / 0	0.47 (1)
K- E	0 / 404	0.09 (1)
I- E	0 / 243	0.05 (2)
I- F	0 / 105	0.02 (3)
B- L	0 / 913	0.21 (1)
F- H	-1524 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.33/1.00 (I-K:2) , WB=0.55/1.00 (F-H:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.37 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102/17-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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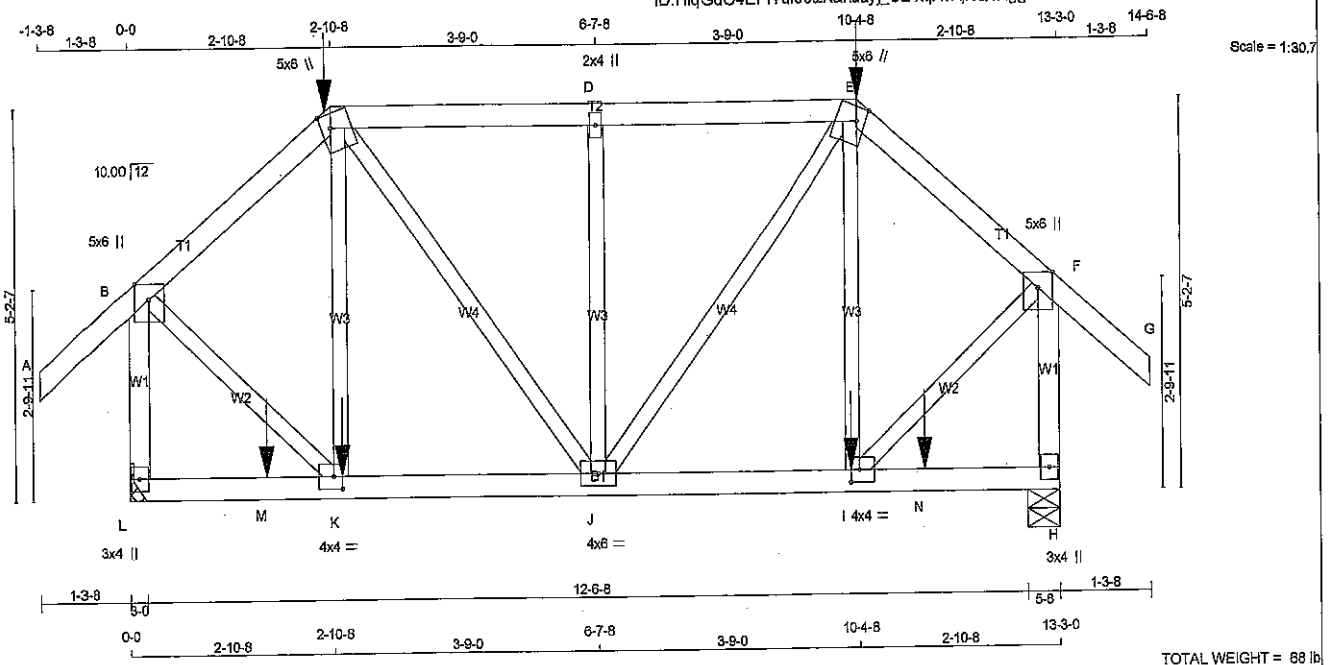

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 154.2 lbs FACTORED DOWN AT 2-10-8, AND 154.2 lbs FACTORED DOWN AT 10-4-8 ON TOP CHORD, AND 21.4 lbs FACTORED DOWN AT 1-11-4, 396.0 lbs FACTORED DOWN AT 3-0-0, AND 396.0 lbs FACTORED DOWN AT 10-3-0, AND 21.4 lbs FACTORED DOWN AT 11-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1781	0	1781	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 3-0	IN-SX
H	1781	0	1781	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1376	917 / 0	225 / 0	0 / 0	0 / 0	235 / 0	0 / 0
H	1376	917 / 0	225 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-1100 / 0	-134.0	-134.0	0.23 (1)	5.72
C- D	-1107 / 0	-134.0	-134.0	0.35 (1)	5.51
D- E	-1107 / 0	-134.0	-134.0	0.35 (1)	5.51
E- F	-1100 / 0	-134.0	-134.0	0.23 (1)	5.72
F- G	0 / 47	-104.9	-104.9	0.16 (1)	10.00
L- B	-1745 / 0	0.0	0.0	0.28 (1)	6.26
H- F	-1745 / 0	0.0	0.0	0.28 (1)	6.26
L- M	0 / 0	-28.0	-28.0	0.08 (3)	10.00
M- K	0 / 0	-28.0	-28.0	0.08 (3)	10.00
K- J	0 / 838	-28.0	-28.0	0.19 (1)	10.00
J- I	0 / 838	-28.0	-28.0	0.19 (1)	10.00
I- N	0 / 0	-28.0	-28.0	0.08 (3)	10.00
N- H	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-189 / 65	0.08 (1)
C- J	0 / 442	0.11 (1)
J- D	-605 / 0	0.25 (1)
J- E	0 / 442	0.11 (1)
I- E	-189 / 65	0.08 (1)
B- K	0 / 1094	0.27 (1)
I- F	0 / 1094	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-10-8	-154	-154	---	BACK	VERT	TOTAL
E	10-4-8	-154	-154	---	BACK	VERT	TOTAL
I	10-3-0	-396	-396	---	BACK	VERT	TOTAL
K	3-0-0	-396	-396	---	BACK	VERT	TOTAL
M	1-11-4	-12	-21	---	BACK	VERT	TOTAL
N	11-3-12	-12	-21	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-0-0
END DISTANCE = 13-3-0
END SPAN CARRIED = 3-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.19/1.00 (I-J:1) , WB=0.27/1.00 (B-K:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10210-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295162	T40	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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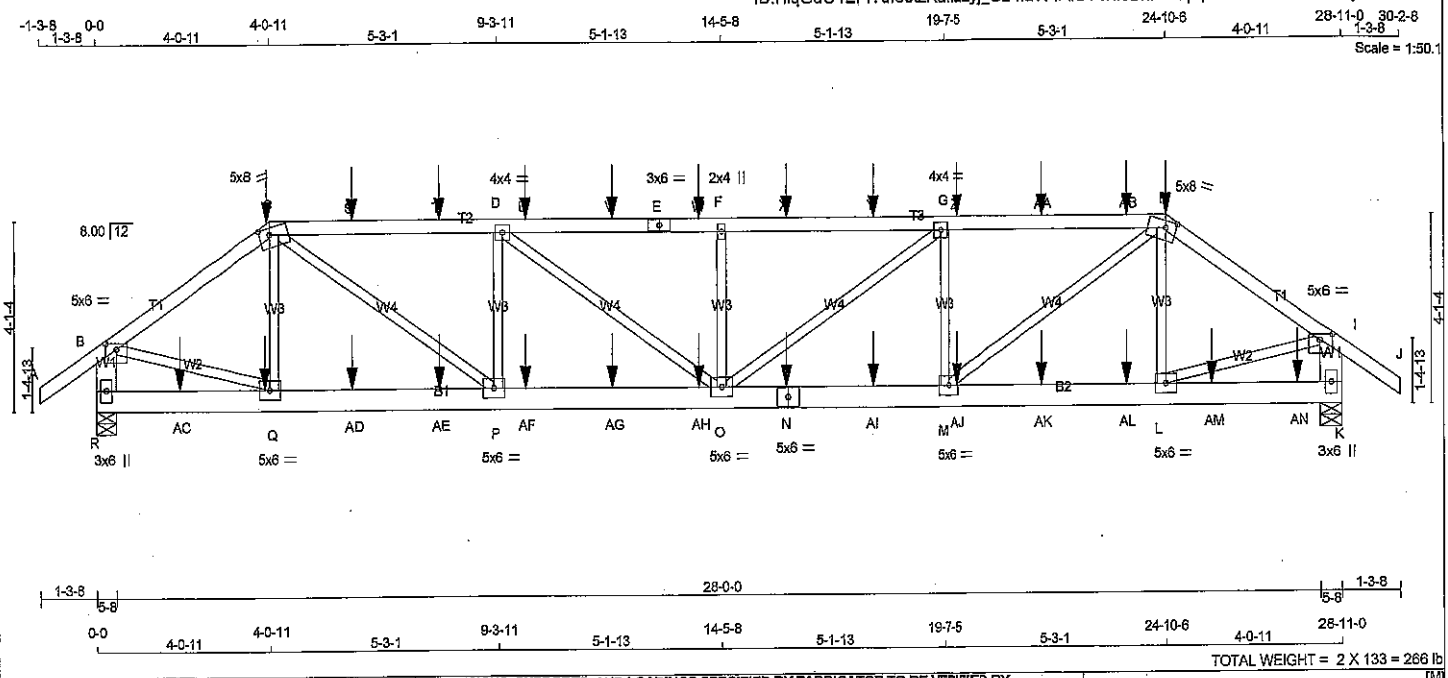

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	6	SIDE(49.3)
L- H	1	6	SIDE(15.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.00
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3586	0	3586	0	0	5-8	5-8
K	3556	0	3556	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2876	1731 / 0	589 / 0	0 / 0	0 / 0	555 / 0	0 / 0
K	2867	1703 / 0	604 / 0	0 / 0	0 / 0	560 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	Q- C	-577 / 50	0.07 (1)
B- C	-4190 / 0	-104.9	-104.9	0.25 (1)	4.44	C- P	0 / 2983	0.37 (1)
C- S	-5900 / 0	-104.9	-104.9	0.57 (1)	3.49	P- D	-1480 / 0	0.19 (1)
S- T	-5900 / 0	-104.9	-104.9	0.57 (1)	3.49	D- O	0 / 858	0.11 (1)
T- D	-5900 / 0	-104.9	-104.9	0.57 (1)	3.49	O- F	-795 / 0	0.10 (1)
D- U	-6595 / 0	-104.9	-104.9	0.61 (1)	3.27	O- G	0 / 884	0.11 (1)
U- V	-6595 / 0	-104.9	-104.9	0.61 (1)	3.27	M- G	-1503 / 0	0.19 (1)
V- E	-6595 / 0	-104.9	-104.9	0.61 (1)	3.27	M- H	0 / 3019	0.37 (1)
E- W	-6595 / 0	-104.9	-104.9	0.61 (1)	3.27	L- H	-560 / 56	0.07 (1)
W- F	-6595 / 0	-104.9	-104.9	0.61 (1)	3.27	B- Q	0 / 3579	0.44 (1)
F- X	-6595 / 0	-104.9	-104.9	0.61 (1)	3.28	L- I	0 / 3527	0.44 (1)
X- Y	-6595 / 0	-104.9	-104.9	0.61 (1)	3.28			
Y- G	-6595 / 0	-104.9	-104.9	0.61 (1)	3.28			
G- Z	-5879 / 0	-104.9	-104.9	0.57 (1)	3.50			
Z-AA	-5879 / 0	-104.9	-104.9	0.57 (1)	3.50			
AA-AB	-5879 / 0	-104.9	-104.9	0.57 (1)	3.50			
AB- H	-5879 / 0	-104.9	-104.9	0.57 (1)	3.50			
H- I	-4129 / 0	-104.9	-104.9	0.25 (1)	4.47			
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00			
R- B	-3524 / 0	0.0	0.0	0.13 (1)	7.46			
K- I	-3478 / 0	0.0	0.0	0.12 (1)	7.49			
R-AC	0 / 0	-28.0	-28.0	0.06 (3)	10.00			
AC- Q	0 / 0	-28.0	-28.0	0.06 (3)	10.00			
Q-AD	0 / 3465	-28.0	-28.0	0.27 (1)	10.00			
AD-AE	0 / 3465	-28.0	-28.0	0.27 (1)	10.00			
AE- P	0 / 3465	-28.0	-28.0	0.27 (1)	10.00			
P-AF	0 / 5900	-28.0	-28.0	0.45 (1)	10.00			
AF-AG	0 / 5900	-28.0	-28.0	0.45 (1)	10.00			
AG-AH	0 / 5900	-28.0	-28.0	0.45 (1)	10.00			
AH- O	0 / 5900	-28.0	-28.0	0.45 (1)	10.00			
O- N	0 / 5879	-28.0	-28.0	0.45 (1)	10.00			
N-AI	0 / 5879	-28.0	-28.0	0.45 (1)	10.00			
AI- M	0 / 5879	-28.0	-28.0	0.45 (1)	10.00			
M-AJ	0 / 3414	-28.0	-28.0	0.27 (1)	10.00			
AJ-AK	0 / 3414	-28.0	-28.0	0.27 (1)	10.00			
AK-AL	0 / 3414	-28.0	-28.0	0.27 (1)	10.00			
AL- L	0 / 3414	-28.0	-28.0	0.27 (1)	10.00			
L-AM	0 / 0	-28.0	-28.0	0.06 (3)	10.00			
AM-AN	0 / 0	-28.0	-28.0	0.06 (3)	10.00			

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.61/1.00 (D-F:1) , BC=0.45/1.00 (O-P:1) , WB=0.44/1.00 (B-Q:1) , SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.51 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 2676 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 10221-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295162	T40	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:HlqGdO4EFr?ul80tzKanazyj_Uz-haW4AIUT4Hi0BnEZXqApmteTHCOI5QPDIYybJEzhJOR

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 656.5 lbs FACTORED DOWN AT 4-0-11, 526.9 lbs FACTORED DOWN AT 24-10-6, 126.3 lbs FACTORED DOWN AT 5-11-4, 126.3 lbs FACTORED DOWN AT 7-11-4, 126.3 lbs FACTORED DOWN AT 9-11-4, 126.3 lbs FACTORED DOWN AT 11-11-4, 126.3 lbs FACTORED DOWN AT 13-11-4, 126.3 lbs FACTORED DOWN AT 15-11-4, 126.3 lbs FACTORED DOWN AT 17-11-4, 126.3 lbs FACTORED DOWN AT 19-11-4, AND 126.3 lbs FACTORED DOWN AT 21-11-4, AND 130.3 lbs FACTORED DOWN AT 23-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, 69.9 lbs FACTORED DOWN AT 21-11-4, 69.9 lbs FACTORED DOWN AT 23-11-4, AND 69.9 lbs FACTORED DOWN AT 25-11-4, AND 70.4 lbs FACTORED DOWN AT 27-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN- K	0 / 0	-28.0	-28.0	0.06 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-657	-657	—	FRONT	VERT	TOTAL
H	24-10-6	-527	-527	—	FRONT	VERT	TOTAL
N	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
Q	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
S	5-11-4	-126	-126	—	FRONT	VERT	TOTAL
T	7-11-4	-126	-126	—	FRONT	VERT	TOTAL
U	9-11-4	-126	-126	—	FRONT	VERT	TOTAL
V	11-11-4	-126	-126	—	FRONT	VERT	TOTAL
W	13-11-4	-126	-126	—	FRONT	VERT	TOTAL
X	15-11-4	-126	-126	—	FRONT	VERT	TOTAL
Y	17-11-4	-126	-126	—	FRONT	VERT	TOTAL
Z	19-11-4	-126	-126	—	FRONT	VERT	TOTAL
AA	21-11-4	-126	-126	—	FRONT	VERT	TOTAL
AB	23-11-4	-130	-130	—	FRONT	VERT	TOTAL
AC	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AD	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
AJ	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
AK	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AL	23-11-4	-40	-70	—	FRONT	VERT	TOTAL
AM	25-11-4	-40	-70	—	FRONT	VERT	TOTAL
AN	27-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 0221 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T40S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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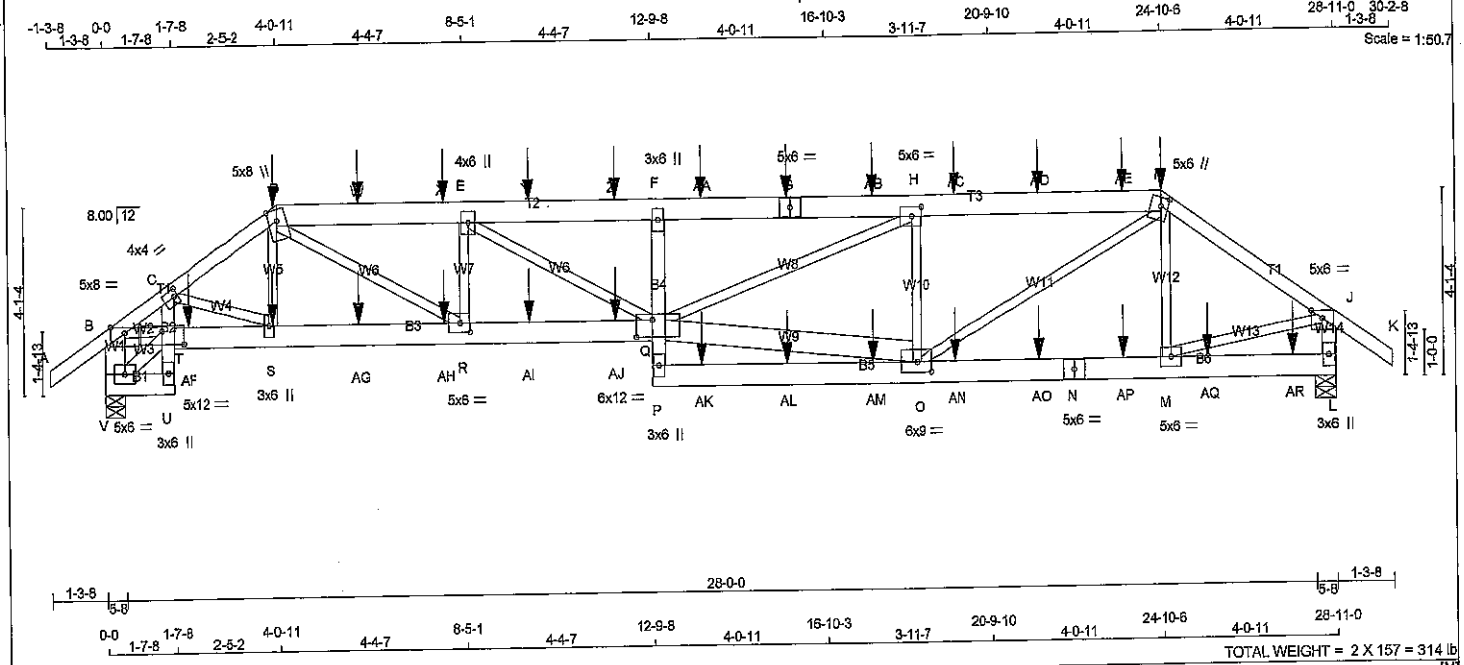


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
I- K	1	12	SIDE(61.0)
L- J	1	12	TOP
D- G	2	12	SIDE(61.0)
G- I	2	12	SIDE(61.0)
V- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- U	2	12	TOP
T- Q	2	12	SIDE(183.1)
P- N	2	12	SIDE(0.0)
N- L	2	12	SIDE(0.0)
U- C	1	12	TOP
F- P	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
S- D	1	6	SIDE(49.9)
M- I	1	6	SIDE(15.5)
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMVW+t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	3681	0	3661	0	0	5-8	5-8
V	3935	0	3935	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2949	1755 / 0	619 / 0	0 / 0	0 / 0	575 / 0	0 / 0
V	3147	1910 / 0	635 / 0	0 / 0	0 / 0	602 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-6651 / 0	-104.9	-104.9	0.24 (1)	3.61	
C- D	-6265 / 0	-104.9	-104.9	0.26 (1)	3.70	
D- W	-8454 / 0	-104.9	-104.9	0.25 (1)	4.00	
W- X	-8454 / 0	-104.9	-104.9	0.25 (1)	4.00	
X- E	-8454 / 0	-104.9	-104.9	0.25 (1)	4.00	
E- Y	-9777 / 0	-104.9	-104.9	0.27 (1)	3.74	
Y- Z	-9777 / 0	-104.9	-104.9	0.27 (1)	3.74	
Z- F	-9777 / 0	-104.9	-104.9	0.27 (1)	3.74	
F-AA	-9660 / 0	-104.9	-104.9	0.41 (1)	3.62	
AA- G	-9660 / 0	-104.9	-104.9	0.41 (1)	3.62	
G-AB	-9660 / 0	-104.9	-104.9	0.41 (1)	3.62	
AB- H	-9660 / 0	-104.9	-104.9	0.41 (1)	3.62	
H-AC	-6501 / 0	-104.9	-104.9	0.31 (1)	4.39	
AC-AD	-6501 / 0	-104.9	-104.9	0.31 (1)	4.39	
AD-AE	-6501 / 0	-104.9	-104.9	0.31 (1)	4.39	
AE- I	-6501 / 0	-104.9	-104.9	0.31 (1)	4.39	
I- J	-4244 / 0	-104.9	-104.9	0.25 (1)	4.41	
J- K	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
V- B	-3702 / 0	0.0	0.0	0.13 (1)	7.32	
L- J	-3588 / 0	0.0	0.0	0.20 (1)	6.20	
V- U	0 / 280	-28.0	-28.0	0.02 (1)	10.00	
U- T	0 / 36	0.0	0.0	0.24 (1)	10.00	
T- C	0 / 404	0.0	0.0	0.27 (1)	10.00	
T-AF	0 / 5813	-28.0	-28.0	0.42 (1)	10.00	
AF- S	0 / 5813	-28.0	-28.0	0.42 (1)	10.00	
S-AG	0 / 5246	-28.0	-28.0	0.40 (1)	10.00	
AG-AH	0 / 5246	-28.0	-28.0	0.40 (1)	10.00	
AH- R	0 / 5246	-28.0	-28.0	0.40 (1)	10.00	
R-AI	0 / 8453	-28.0	-28.0	0.64 (1)	10.00	
AI-AJ	0 / 8453	-28.0	-28.0	0.64 (1)	10.00	
AJ- Q	0 / 8453	-28.0	-28.0	0.64 (1)	10.00	
P- Q	0 / 212	0.0	0.0	0.24 (1)	10.00	
Q- F	-781 / 0	0.0	0.0	0.24 (1)	7.61	
P-AK	0 / 310	-28.0	-28.0	0.12 (3)	10.00	
AK-AL	0 / 310	-28.0	-28.0	0.12 (3)	10.00	
AL-AM	0 / 310	-28.0	-28.0	0.12 (3)	10.00	
AM- O	0 / 310	-28.0	-28.0	0.12 (3)	10.00	
O-AN	0 / 3560	-28.0	-28.0	0.28 (1)	10.00	
AN-AO	0 / 3560	-28.0	-28.0	0.28 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-590 / 0	0.06 (1)
S- D	0 / 437	0.05 (2)
D- R	0 / 3760	0.47 (1)
R- E	-1629 / 0	0.15 (1)
Q- O	0 / 6349	0.41 (1)
Q- H	0 / 3366	0.42 (1)
O- H	-2582 / 0	0.31 (1)
O- I	0 / 3484	0.43 (1)
M- I	-638 / 20	0.08 (1)
M- J	0 / 3653	0.45 (1)
E- Q	0 / 1535	0.19 (1)
V- T	-359 / 0	0.02 (1)
B- T	0 / 5470	0.68 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 863 (0.40")

CSI: TC=0.41/1.00 (F-H:1) , BC=0.64/1.00 (Q-R:1) , WB=0.68/1.00 (B-T:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T40S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TTWW+m	MT20	5.0	6.0	2.25	2.00
J	TMVW-p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0	2.75	4.50
P	BMV+p	MT20	3.0	6.0		
Q	BVMWWW-I	MT20	6.0	12.0	4.50	4.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMWW+t	MT20	3.0	6.0		
T	BVMWW-I	MT20	5.0	12.0	3.50	6.00
U	BMV+p	MT20	3.0	6.0		
V	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 658.8 lbs FACTORED DOWN AT 4-0-11, 128.6 lbs FACTORED DOWN AT 5-11-4, 128.6 lbs FACTORED DOWN AT 7-11-4, 128.6 lbs FACTORED DOWN AT 9-11-4, 128.6 lbs FACTORED DOWN AT 11-11-4, 126.3 lbs FACTORED DOWN AT 13-11-4, 126.3 lbs FACTORED DOWN AT 15-11-4, 126.3 lbs FACTORED DOWN AT 17-11-4, 126.3 lbs FACTORED DOWN AT 19-11-4, 126.3 lbs FACTORED DOWN AT 21-11-4, AND 130.3 lbs FACTORED DOWN AT 23-11-4, AND 526.9 lbs FACTORED DOWN AT 24-10-6 ON TOP CHORD, AND 120.1 lbs FACTORED DOWN AT 1-11-4, 112.5 lbs FACTORED DOWN AT 3-11-4, 112.5 lbs FACTORED DOWN AT 5-11-4, 112.5 lbs FACTORED DOWN AT 7-11-4, 112.5 lbs FACTORED DOWN AT 9-11-4, 112.5 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, 69.9 lbs FACTORED DOWN AT 21-11-4, 69.9 lbs FACTORED DOWN AT 23-11-4, AND 69.9 lbs FACTORED DOWN AT 25-11-4, AND 70.4 lbs FACTORED DOWN AT 27-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

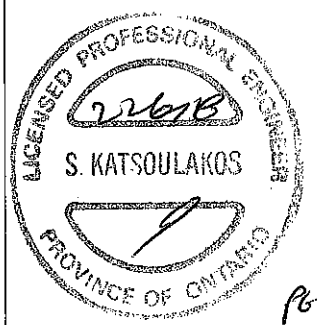
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MEMB. MAX CSI (LC)
AO- N	0 / 3560	-28.0	-28.0	0.28 (1)	10.00		
N-AP	0 / 3560	-28.0	-28.0	0.28 (1)	10.00		
AP- M	0 / 3560	-28.0	-28.0	0.28 (1)	10.00		
M-AQ	0 / 0	-28.0	-28.0	0.06 (3)	10.00		
AQ-AR	0 / 0	-28.0	-28.0	0.06 (3)	10.00		
AR- L	0 / 0	-28.0	-28.0	0.06 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-0-11	-659	-659	—	FRONT	VERT	TOTAL
G	15-11-4	-126	-126	—	FRONT	VERT	TOTAL
I	24-10-6	-527	-527	—	FRONT	VERT	TOTAL
S	3-11-4	-113	-113	—	FRONT	VERT	TOTAL
W	5-11-4	-129	-129	—	FRONT	VERT	TOTAL
X	7-11-4	-129	-129	—	FRONT	VERT	TOTAL
Y	9-11-4	-129	-129	—	FRONT	VERT	TOTAL
Z	11-11-4	-129	-129	—	FRONT	VERT	TOTAL
AA	13-11-4	-126	-126	—	FRONT	VERT	TOTAL
AB	17-11-4	-126	-126	—	FRONT	VERT	TOTAL
AC	19-11-4	-126	-126	—	FRONT	VERT	TOTAL
AD	21-11-4	-126	-126	—	FRONT	VERT	TOTAL
AE	23-11-4	-130	-130	—	FRONT	VERT	TOTAL
AF	1-11-4	-120	-120	—	FRONT	VERT	TOTAL
AG	5-11-4	-113	-113	—	FRONT	VERT	TOTAL
AH	7-11-4	-113	-113	—	FRONT	VERT	TOTAL
AI	9-11-4	-113	-113	—	FRONT	VERT	TOTAL
AJ	11-11-4	-113	-113	—	FRONT	VERT	TOTAL
AK	13-11-4	-40	-70	—	FRONT	VERT	TOTAL
AL	15-11-4	-40	-70	—	FRONT	VERT	TOTAL
AM	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
AN	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
AO	21-11-4	-40	-70	—	FRONT	VERT	TOTAL
AP	23-11-4	-40	-70	—	FRONT	VERT	TOTAL
AQ	25-11-4	-40	-70	—	FRONT	VERT	TOTAL
AR	27-11-4	-40	-70	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.60 (Q) (INPUT = 1.00)



DWG NO. TAM 10225-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295162	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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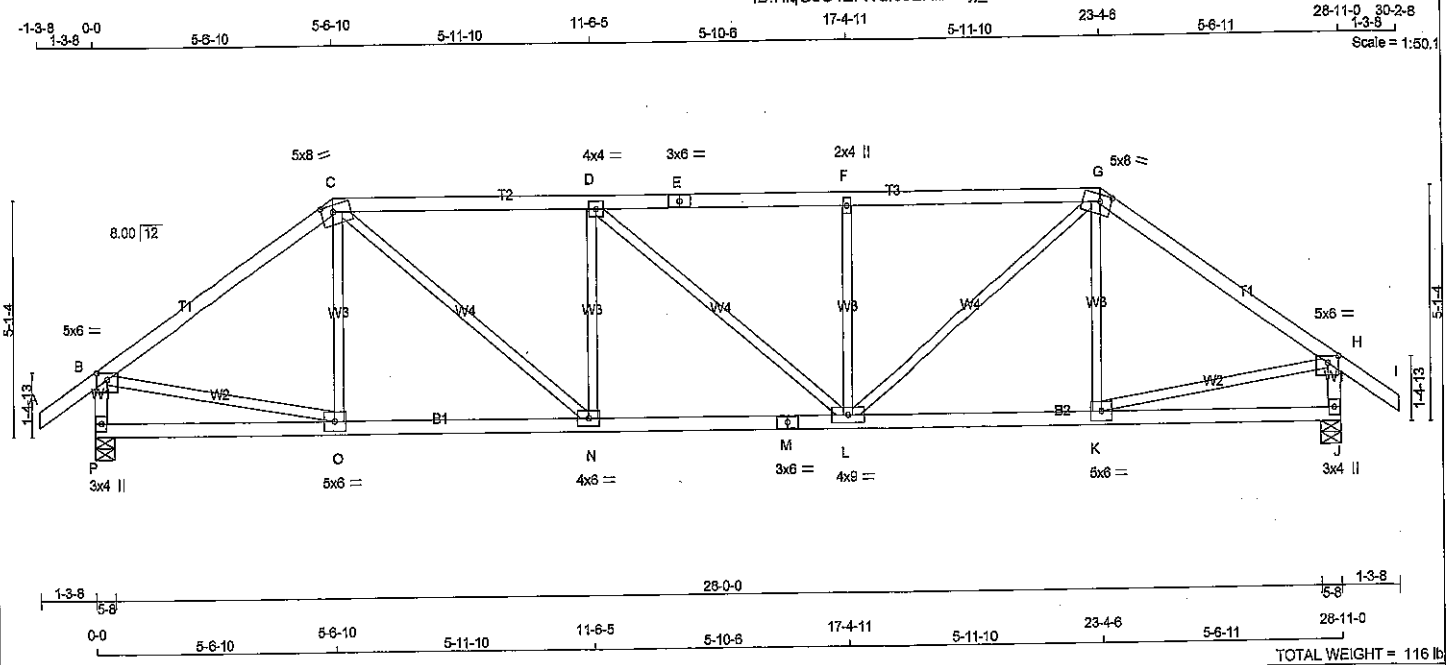

TOTAL WEIGHT = 116 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2066	0	2066	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2193 / 0	-104.9	-104.9	0.77 (1)	3.72	
C- D	-2855 / 0	-104.9	-104.9	0.74 (1)	3.38	
D- E	-2853 / 0	-104.9	-104.9	0.73 (1)	3.38	
E- F	-2853 / 0	-104.9	-104.9	0.73 (1)	3.38	
F- G	-2853 / 0	-104.9	-104.9	0.73 (1)	3.39	
G- H	-2193 / 0	-104.9	-104.9	0.77 (1)	3.72	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
P- B	-2003 / 0	0.0	0.0	0.21 (1)	5.97	
J- H	-2003 / 0	0.0	0.0	0.21 (1)	5.97	
P- O	0 / 0	-28.0	-28.0	0.22 (3)	10.00	
O- N	0 / 1818	-28.0	-28.0	0.42 (2)	10.00	
N- M	0 / 2855	-28.0	-28.0	0.55 (1)	10.00	
M- L	0 / 2855	-28.0	-28.0	0.55 (1)	10.00	
L- K	0 / 1819	-28.0	-28.0	0.42 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-166 / 165	0.06 (1)
C- N	0 / 1330	0.30 (1)
N- D	-672 / 0	0.26 (1)
D- L	-2 / 0	0.00 (1)
L- F	-672 / 0	0.26 (1)
L- G	0 / 1327	0.30 (1)
K- G	-165 / 165	0.06 (1)
B- O	0 / 1855	0.42 (1)
K- H	0 / 1855	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.77/1.00 (G-H:1) , BC=0.55/1.00 (L-N:1) , WB=0.42/1.00 (H-K:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.77 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10222-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T41S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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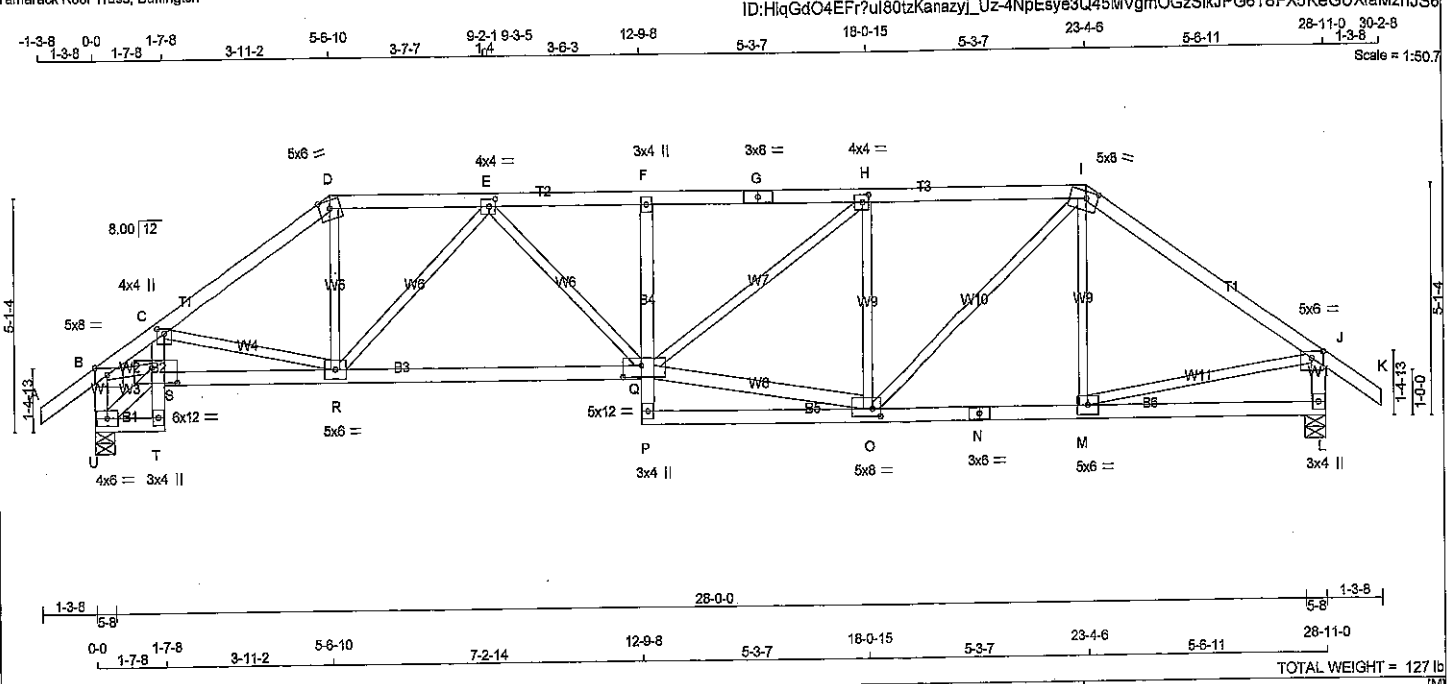

TOTAL WEIGHT = 127 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TTWW-m	MT20	5.0	8.0	1.75	3.25
J	TMVW-p	MT20	5.0	6.0	1.75	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0	2.00	2.25
P	BMV+p	MT20	3.0	4.0		
Q	BVMWWW-t	MT20	5.0	12.0	3.00	5.25
R	BMWWW-t	MT20	5.0	6.0		
S	BVMWW-t	MT20	6.0	12.0	4.00	7.00
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2063	0	2063	0	0	5-8	5-8
U	2069	0	2069	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1627	1026 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
U	1632	1030 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 40		-104.9	-104.9	0.14 (1)	10.00		C- R	-710 / 0		0.22 (1)
B- C	-3368 / 0		-104.9	-104.9	0.25 (1)	3.61		R- D	0 / 1153		0.26 (1)
C- D	-2747 / 0		-104.9	-104.9	0.41 (1)	3.82		R- E	-1218 / 0		0.57 (1)
D- E	-2330 / 0		-104.9	-104.9	0.25 (1)	4.27		E- Q	0 / 668		0.15 (1)
E- F	-3623 / 0		-104.9	-104.9	0.35 (1)	3.42		Q- O	0 / 2726		0.44 (1)
F- G	-3595 / 0		-104.9	-104.9	0.70 (1)	3.05		Q- H	0 / 1013		0.23 (1)
G- H	-3595 / 0		-104.9	-104.9	0.70 (1)	3.05		O- H	-1236 / 0		0.46 (1)
H- I	-2762 / 0		-104.9	-104.9	0.61 (1)	3.56		O- I	0 / 1292		0.29 (1)
I- J	-2185 / 0		-104.9	-104.9	0.77 (1)	3.72		M- I	-179 / 142		0.07 (1)
J- K	0 / 40		-104.9	-104.9	0.14 (1)	10.00		M- J	0 / 1848		0.42 (1)
U- B	-1940 / 0		0.0	0.0	0.20 (1)	6.04		U- S	-175 / 0		0.02 (1)
L- J	-1997 / 0		0.0	0.0	0.21 (1)	5.97		B- S	0 / 2791		0.63 (1)
U- T	0 / 137		-28.0	-28.0	0.03 (2)	10.00					
T- S	0 / 36		0.0	0.0	0.22 (1)	10.00					
S- C	0 / 236		0.0	0.0	0.26 (1)	10.00					
S- R	0 / 2987		-28.0	-28.0	0.65 (1)	10.00					
R- Q	0 / 3160		-28.0	-28.0	0.68 (1)	10.00					
P- Q	0 / 105		0.0	0.0	0.15 (1)	10.00					
Q- F	-428 / 0		0.0	0.0	0.18 (1)	7.81					
P- O	0 / 104		-28.0	-28.0	0.17 (2)	10.00					
O- N	0 / 1812		-28.0	-28.0	0.40 (1)	10.00					
N- M	0 / 1812		-28.0	-28.0	0.40 (1)	10.00					
M- L	0 / 0		-28.0	-28.0	0.20 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 862 (0.40")

CSI: TC=0.77/1.00 (I-J:1) , BC=0.68/1.00 (Q-R:1) , WB=0.63/1.00 (B-S:1) , SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102268
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295162	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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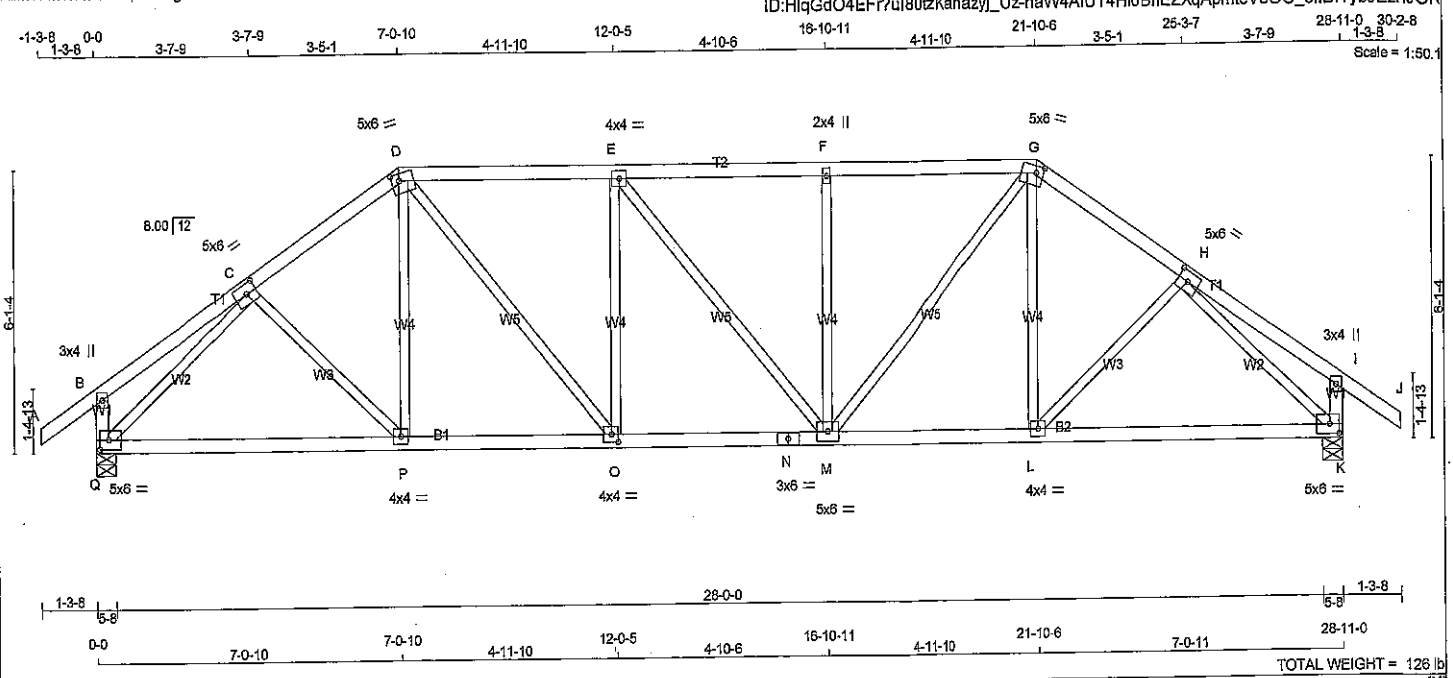

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2066	0	2066	0	0	5-8	5-8
K	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- P	0 / 152	0.03 (3)	
B- C	0 / 21	-104.9	-104.9	0.18 (1)	10.00	P- D	0 / 258	0.06 (2)	
C- D	-2174 / 0	-104.9	-104.9	0.25 (1)	4.38	D- O	0 / 891	0.20 (1)	
D- E	-2370 / 0	-104.9	-104.9	0.45 (1)	4.00	O- E	-557 / 0	0.33 (1)	
E- F	-2369 / 0	-104.9	-104.9	0.45 (1)	4.00	E- M	-3 / 0	0.00 (1)	
F- G	-2368 / 0	-104.9	-104.9	0.44 (1)	4.01	M- F	-556 / 0	0.33 (1)	
G- H	-2174 / 0	-104.9	-104.9	0.25 (1)	4.38	M- G	0 / 888	0.20 (1)	
H- I	0 / 21	-104.9	-104.9	0.18 (1)	10.00	L- G	0 / 259	0.06 (2)	
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	L- H	0 / 152	0.03 (3)	
Q- B	-291 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2426 / 0	0.93 (1)	
K- I	-291 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2426 / 0	0.93 (1)	
Q- P	0 / 1738	-28.0	-28.0	0.48 (2)	10.00				
P- O	0 / 1791	-28.0	-28.0	0.50 (2)	10.00				
O- N	0 / 2370	-28.0	-28.0	0.45 (1)	10.00				
N- M	0 / 2370	-28.0	-28.0	0.45 (1)	10.00				
M- L	0 / 1791	-28.0	-28.0	0.50 (2)	10.00				
L- K	0 / 1738	-28.0	-28.0	0.48 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.50/1.00 (L-M:2) , WB=0.93/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 22676 S. KATSOULAKOS PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10223-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T42S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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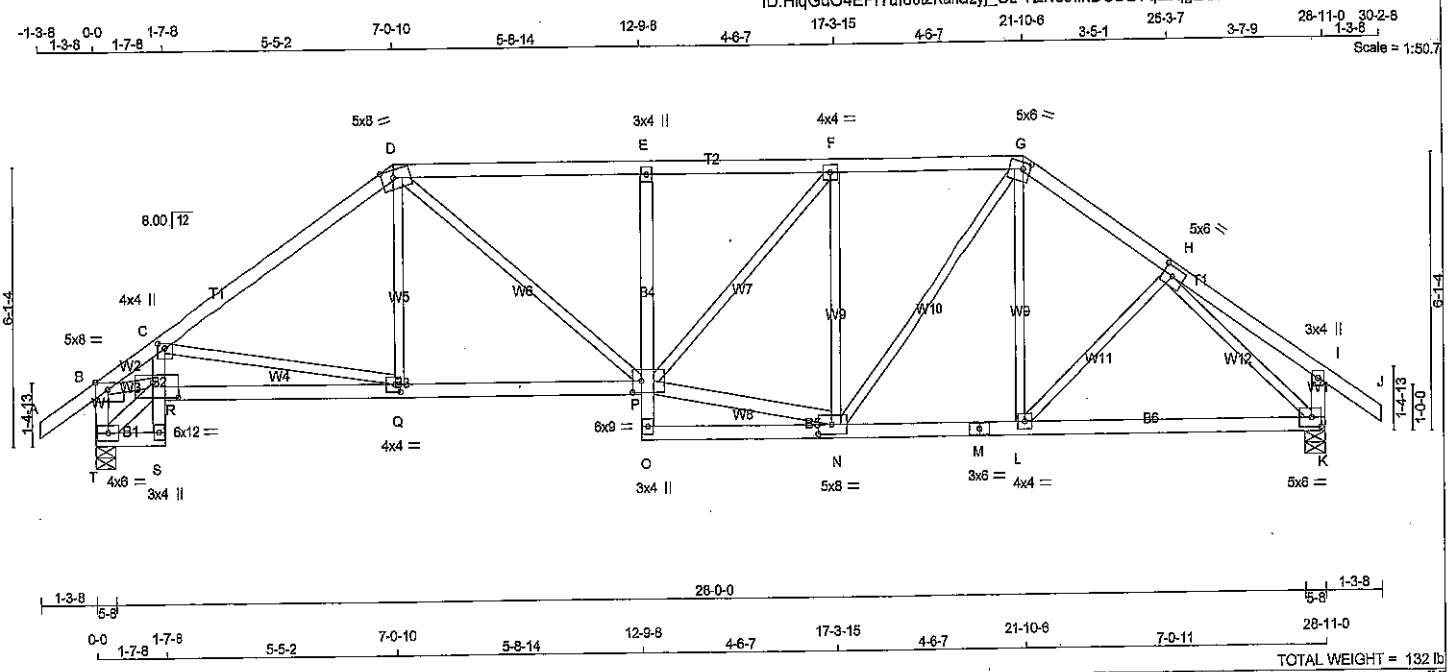

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	3.75
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-t	MT20	6.0	9.0	3.00	2.50
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BVMWW-t	MT20	6.0	12.0	4.00	7.00
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2063	0	2063	0	0	5-8	5-8
T	2069	0	2069	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1627	1026 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
T	1832	1030 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40		-104.9	-104.9	0.14 (1)	10.00
B-C	-3516 / 0		-104.9	-104.9	0.41 (1)	3.33
C-D	-2595 / 0		-104.9	-104.9	0.68 (1)	3.67
D-E	-2907 / 0		-104.9	-104.9	0.67 (1)	3.44
E-F	-2694 / 0		-104.9	-104.9	0.57 (1)	3.48
F-G	-2317 / 0		-104.9	-104.9	0.35 (1)	4.16
G-H	-2188 / 0		-104.9	-104.9	0.25 (1)	4.39
H-I	0 / 21		-104.9	-104.9	0.18 (1)	10.00
I-J	0 / 40		-104.9	-104.9	0.14 (1)	10.00
T-B	-1935 / 0		0.0	0.0	0.20 (1)	6.05
K-I	-291 / 0		0.0	0.0	0.03 (1)	7.81
T-S	0 / 143		-28.0	-28.0	0.03 (1)	10.00
S-R	0 / 36		0.0	0.0	0.23 (1)	10.00
R-C	0 / 250		0.0	0.0	0.27 (1)	10.00
R-Q	0 / 3148		-28.0	-28.0	0.66 (1)	10.00
Q-P	0 / 2146		-28.0	-28.0	0.50 (2)	10.00
O-P	0 / 95		0.0	0.0	0.10 (1)	10.00
P-E	-614 / 0		0.0	0.0	0.19 (1)	7.81
O-N	0 / 64		-28.0	-28.0	0.13 (2)	10.00
N-M	0 / 1786		-28.0	-28.0	0.49 (2)	10.00
M-L	0 / 1786		-28.0	-28.0	0.49 (2)	10.00
L-K	0 / 1734		-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
C-Q	-1025 / 0		0.59 (1)
Q-D	0 / 396		0.09 (2)
D-P	0 / 982		0.22 (1)
P-N	0 / 2322		0.37 (1)
P-F	0 / 837		0.19 (1)
N-F	-1087 / 0		0.64 (1)
N-G	0 / 871		0.20 (1)
L-G	0 / 249		0.08 (2)
L-H	0 / 152		0.03 (3)
H-K	-2420 / 0		0.93 (1)
T-R	-183 / 0		0.02 (1)
B-R	0 / 2965		0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.68/1.00 (C-D:1) , BC=0.66/1.00 (Q-R:1) , WB=0.93/1.00 (H-K:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.60 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 10227-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295162	T43	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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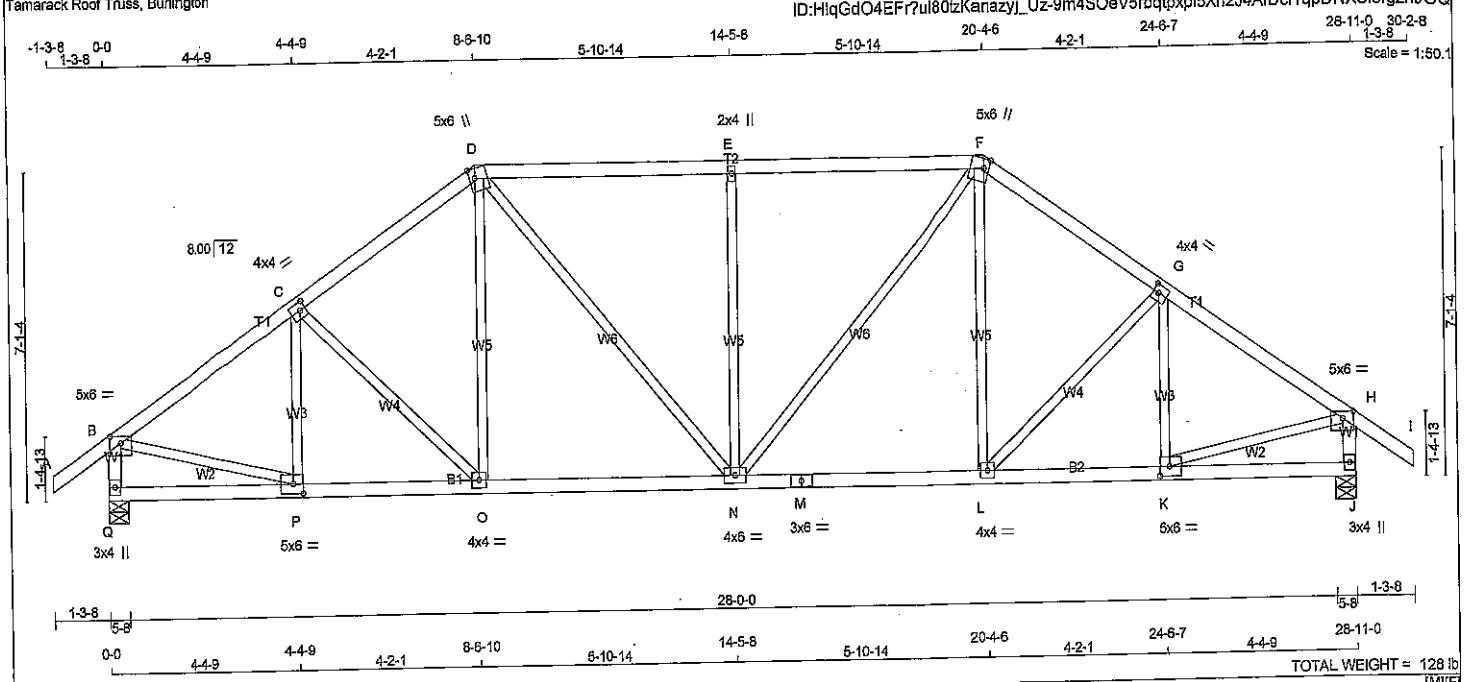

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2066	0	2066	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
A- B	0 / 40
B- C	-2193 / 0
C- D	-2087 / 0
D- E	-2115 / 0
E- F	-2115 / 0
F- G	-2087 / 0
G- H	-2193 / 0
H- I	0 / 40
Q- B	-2011 / 0
J- H	-2011 / 0
Q- P	0 / 0
P- O	0 / 1848
O- N	0 / 1714
N- M	0 / 1714
M- L	0 / 1714
L- K	0 / 1848
K- J	0 / 0

FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
-104.9	-104.9	0.14 (1)		10.00
-104.9	-104.9	0.30 (1)		4.34
-104.9	-104.9	0.29 (1)		4.44
-104.9	-104.9	0.54 (1)		4.07
-104.9	-104.9	0.54 (1)		4.07
-104.9	-104.9	0.29 (1)		4.44
-104.9	-104.9	0.30 (1)		4.34
-104.9	-104.9	0.14 (1)		10.00
0.0	0.0	0.21 (1)		5.95
0.0	0.0	0.21 (1)		5.95
-28.0	-28.0	0.11 (2)		10.00
-28.0	-28.0	0.38 (1)		10.00
-28.0	-28.0	0.39 (2)		10.00
-28.0	-28.0	0.39 (2)		10.00
-28.0	-28.0	0.39 (2)		10.00
-28.0	-28.0	0.38 (1)		10.00
-28.0	-28.0	0.11 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-336 / 32	0.09 (1)
C- O	-190 / 0	0.11 (1)
O- D	0 / 322	0.07 (2)
D- N	0 / 612	0.14 (1)
N- E	-756 / 0	0.66 (1)
N- F	0 / 612	0.14 (1)
L- F	0 / 322	0.07 (2)
L- G	-190 / 0	0.11 (1)
K- G	-336 / 32	0.09 (1)
B- P	0 / 1903	0.43 (1)
K- H	0 / 1903	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.39/1.00 (L-N:2) , WB=0.66/1.00 (E-N:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.47 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10224-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T43S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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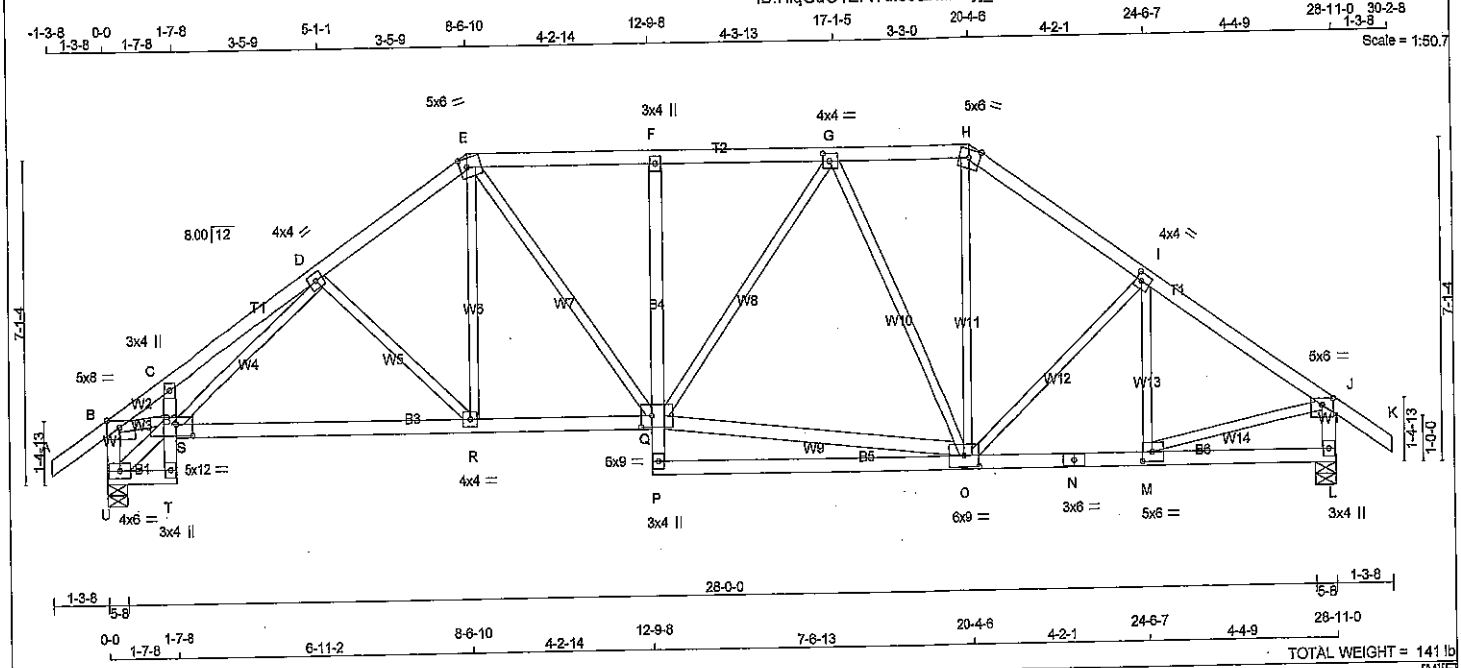

TOTAL WEIGHT = 141 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	6.0	1.75	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	3.0	6.0		
O	BMWWWW*-I	MT20	6.0	9.0	3.00	4.25
P	BMV+p	MT20	3.0	4.0		
Q	BVMWWW-I	MT20	6.0	9.0	3.00	3.00
R	BMWW-t	MT20	4.0	4.0		
S	BVMWWW-I	MT20	5.0	12.0	3.00	4.75
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2083	0	2083	0	0	5-8	5-8
U	2089	0	2089	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1627	1026 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
U	1632	1030 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B-C	-2822 / 0	-104.9	-104.9	0.18 (1)	4.00	
C-D	-2857 / 0	-104.9	-104.9	0.27 (1)	3.91	
D-E	-2432 / 0	-104.9	-104.9	0.24 (1)	4.22	
E-F	-2387 / 0	-104.9	-104.9	0.29 (1)	4.20	
F-G	-2376 / 0	-104.9	-104.9	0.27 (1)	4.22	
G-H	-1733 / 0	-104.9	-104.9	0.16 (1)	4.93	
H-I	-2092 / 0	-104.9	-104.9	0.29 (1)	4.43	
I-J	-2183 / 0	-104.9	-104.9	0.30 (1)	4.35	
J-K	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
U-B	-1972 / 0	0.0	0.0	0.20 (1)	6.00	
L-J	-2004 / 0	0.0	0.0	0.21 (1)	5.96	
U-T	0 / 113	-28.0	-28.0	0.03 (2)	10.00	
T-S	0 / 36	0.0	0.0	0.16 (1)	10.00	
S-C	-170 / 0	0.0	0.0	0.12 (1)	7.81	
S-R	0 / 2318	-28.0	-28.0	0.56 (2)	10.00	
R-Q	0 / 2008	-28.0	-28.0	0.54 (2)	10.00	
P-Q	0 / 146	0.0	0.0	0.10 (1)	10.00	
Q-F	-498 / 0	0.0	0.0	0.24 (1)	7.81	
P-O	0 / 62	-28.0	-28.0	0.32 (2)	10.00	
O-N	0 / 1840	-28.0	-28.0	0.51 (2)	10.00	
N-M	0 / 1840	-28.0	-28.0	0.51 (2)	10.00	
M-L	0 / 0	-28.0	-28.0	0.13 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-R	-434 / 0	0.18 (1)
R-E	0 / 502	0.11 (1)
E-Q	0 / 628	0.14 (1)
O-H	0 / 836	0.19 (1)
O-I	-171 / 0	0.10 (1)
M-I	-357 / 0	0.10 (1)
M-J	0 / 1695	0.43 (1)
S-D	0 / 151	0.03 (2)
U-S	-137 / 0	0.01 (1)
B-S	0 / 2306	0.52 (1)
G-O	-872 / 0	1.00 (1)
Q-O	0 / 2057	0.33 (1)
Q-G	0 / 479	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.30/1.00 (I-J:1) , BC=0.56/1.00 (R-S:2) , WB=1.00/1.00 (G-O:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10228-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T44	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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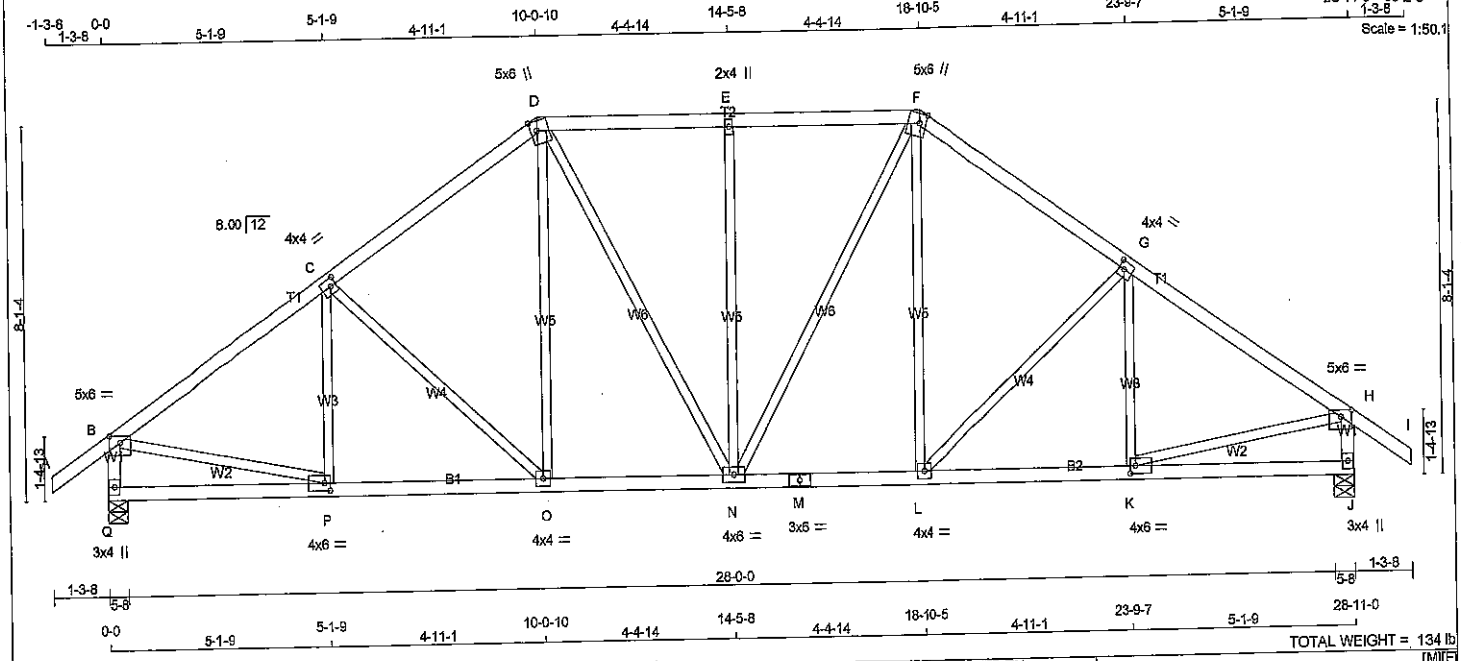

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2066	0	2066	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2231 / 0	-104.9	-104.9	0.42 (1)	4.17	
C- D	-1983 / 0	-104.9	-104.9	0.39 (1)	4.41	
D- E	-1813 / 0	-104.9	-104.9	0.29 (1)	4.69	
E- F	-1813 / 0	-104.9	-104.9	0.29 (1)	4.69	
F- G	-1983 / 0	-104.9	-104.9	0.39 (1)	4.41	
G- H	-2231 / 0	-104.9	-104.9	0.42 (1)	4.17	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-2005 / 0	0.0	0.0	0.21 (1)	5.96	
J- H	-2005 / 0	0.0	0.0	0.21 (1)	5.96	
Q- P	0 / 0	-28.0	-28.0	0.17 (3)	10.00	
P- O	0 / 1885	-28.0	-28.0	0.38 (1)	10.00	
O- N	0 / 1624	-28.0	-28.0	0.33 (1)	10.00	
N- M	0 / 1624	-28.0	-28.0	0.33 (1)	10.00	
M- L	0 / 1624	-28.0	-28.0	0.33 (1)	10.00	
L- K	0 / 1885	-28.0	-28.0	0.38 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-241 / 115	0.08 (1)
C- O	-357 / 0	0.30 (1)
O- D	0 / 375	0.06 (2)
D- N	0 / 385	0.09 (1)
N- E	-557 / 0	0.71 (1)
N- F	0 / 385	0.09 (1)
L- F	0 / 375	0.06 (2)
L- G	-357 / 0	0.30 (1)
K- G	-241 / 115	0.08 (1)
B- P	0 / 1926	0.43 (1)
K- H	0 / 1926	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.42/1.00 (G-H:1) , BC=0.38/1.00 (K-L:1) , WB=0.71/1.00 (E-N:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM/0229-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T45	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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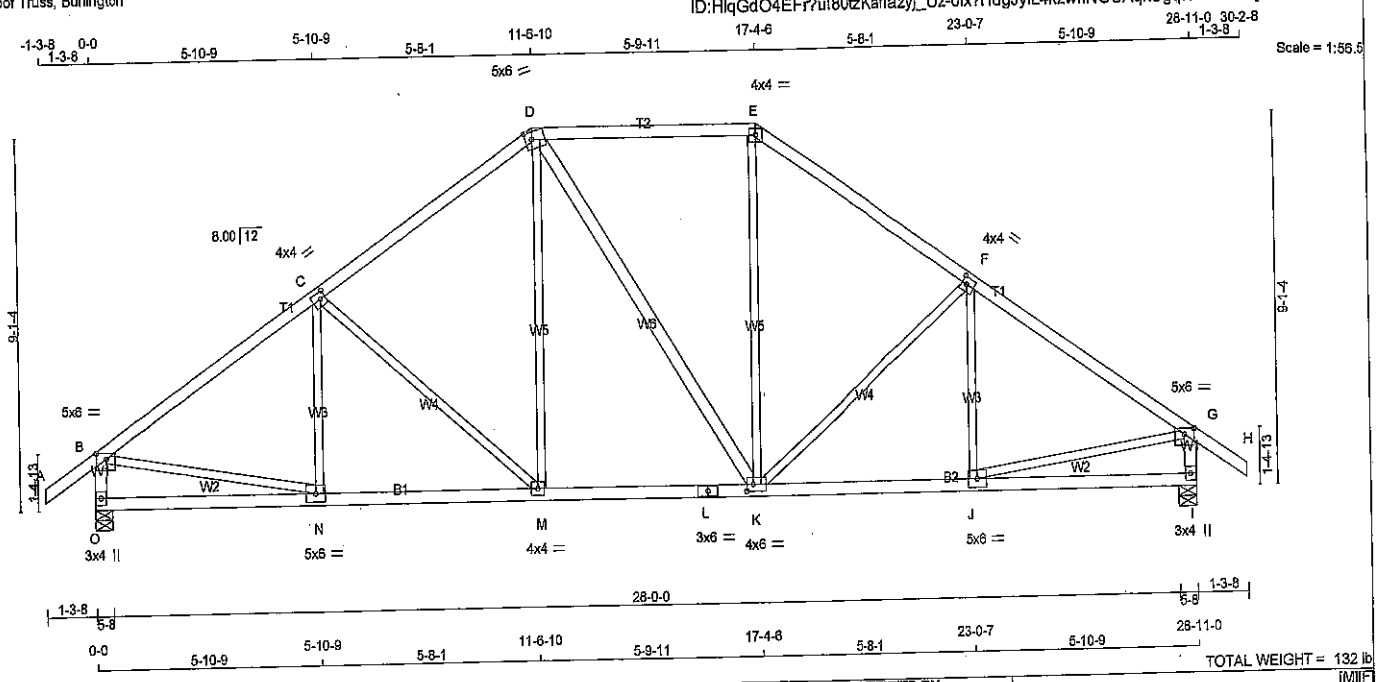

TOTAL WEIGHT = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-I	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	1.75	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2066	0	2066	0	0	5-8	5-8
I	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
I	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
A- B	0 / 40
B- C	-2245 / 0
C- D	-1868 / 0
D- E	-1525 / 0
E- F	-1869 / 0
F- G	-2245 / 0
G- H	0 / 40
O- B	-1998 / 0
I- G	-1997 / 0
O- N	0 / 0
N- M	0 / 1901
M- L	0 / 1524
L- K	0 / 1524
K- J	0 / 1901
J- I	0 / 0

FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
-104.9	-104.9	0.14 (1)	10.00
-104.9	-104.9	0.57 (1)	3.98
-104.9	-104.9	0.52 (1)	4.35
-104.9	-104.9	0.49 (1)	4.72
-104.9	-104.9	0.52 (1)	4.35
-104.9	-104.9	0.57 (1)	3.98
-104.9	-104.9	0.14 (1)	10.00
0.0	0.0	0.21 (1)	5.97
0.0	0.0	0.21 (1)	5.97
-28.0	-28.0	0.23 (3)	10.00
-28.0	-28.0	0.43 (2)	10.00
-28.0	-28.0	0.34 (1)	10.00
-28.0	-28.0	0.34 (1)	10.00
-28.0	-28.0	0.43 (2)	10.00
-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-169 / 175	0.07 (1)
C- M	-506 / 0	0.60 (1)
M- D	0 / 487	0.11 (1)
D- K	0 / 1	0.00 (1)
K- E	0 / 488	0.11 (1)
K- F	-505 / 0	0.60 (1)
J- F	-171 / 174	0.07 (1)
B- N	0 / 1933	0.43 (1)
J- G	0 / 1933	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.43/1.00 (M-N:2) , WB=0.60/1.00 (C-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 102398
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T46	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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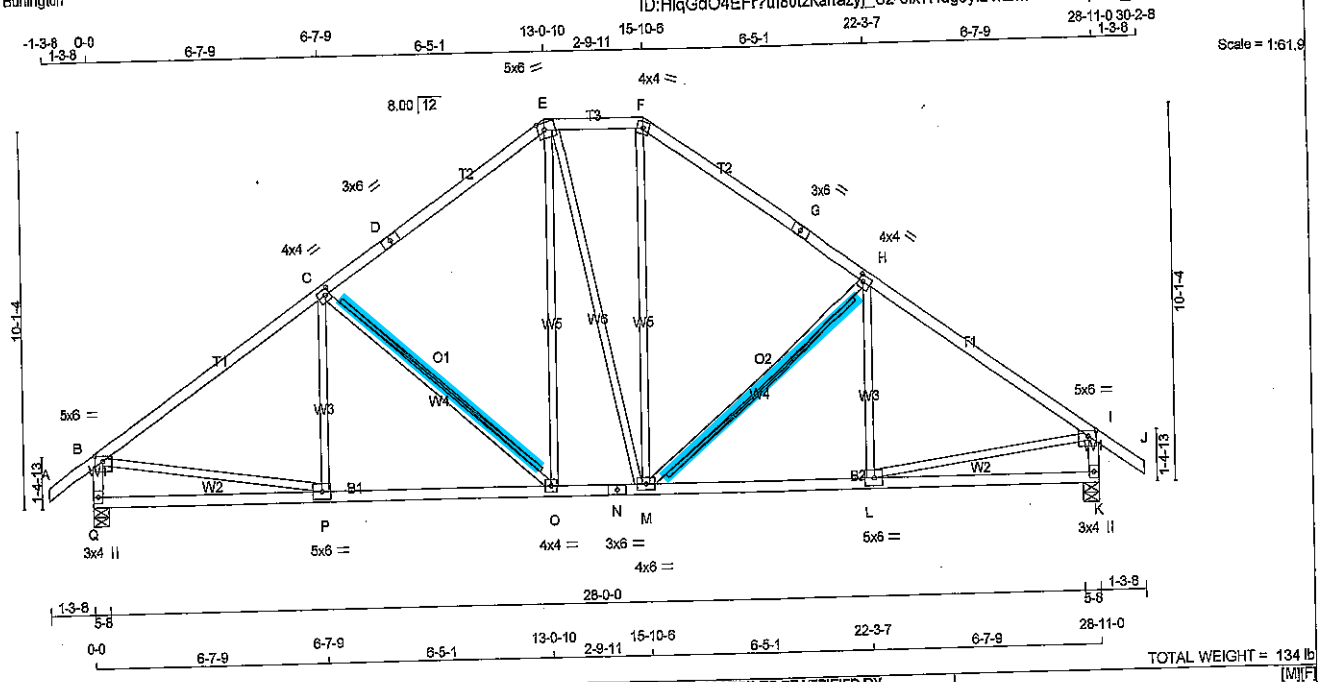

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2066	0	2066	0	0	5-8	5-8
K	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-O, H-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B-C	-2248 / 0	-104.9	-104.9	0.75 (1)	3.70	
C-D	-1737 / 0	-104.9	-104.9	0.66 (1)	4.22	
D-E	-1737 / 0	-104.9	-104.9	0.66 (1)	4.22	
E-F	-1412 / 0	-104.9	-104.9	0.13 (1)	5.37	
F-G	-1739 / 0	-104.9	-104.9	0.66 (1)	4.22	
G-H	-1739 / 0	-104.9	-104.9	0.66 (1)	4.22	
H-I	-2245 / 0	-104.9	-104.9	0.75 (1)	3.71	
I-J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q-B	-1991 / 0	0.0	0.0	0.21 (1)	5.98	
K-I	-1991 / 0	0.0	0.0	0.21 (1)	5.98	
Q-P	0 / 0	-28.0	-28.0	0.32 (3)	10.00	
P-O	0 / 1907	-28.0	-28.0	0.52 (2)	10.00	
O-N	0 / 1410	-28.0	-28.0	0.32 (1)	10.00	
N-M	0 / 1410	-28.0	-28.0	0.32 (1)	10.00	
M-L	0 / 1906	-28.0	-28.0	0.51 (2)	10.00	
L-K	0 / 0	-28.0	-28.0	0.31 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-97 / 247	0.06 (3)
C-O	-657 / 0	0.82 (1)
O-E	0 / 534	0.12 (1)
E-M	0 / 7	0.00 (1)
M-F	0 / 541	0.12 (1)
M-H	-653 / 0	0.81 (1)
L-H	-101 / 245	0.06 (3)
B-P	0 / 1932	0.43 (1)
L-I	0 / 1931	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.75/1.00 (B-C:1) , BC=0.52/1.00 (O-P:2) , WB=0.82/1.00 (C-O:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.48 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/0231.18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T47	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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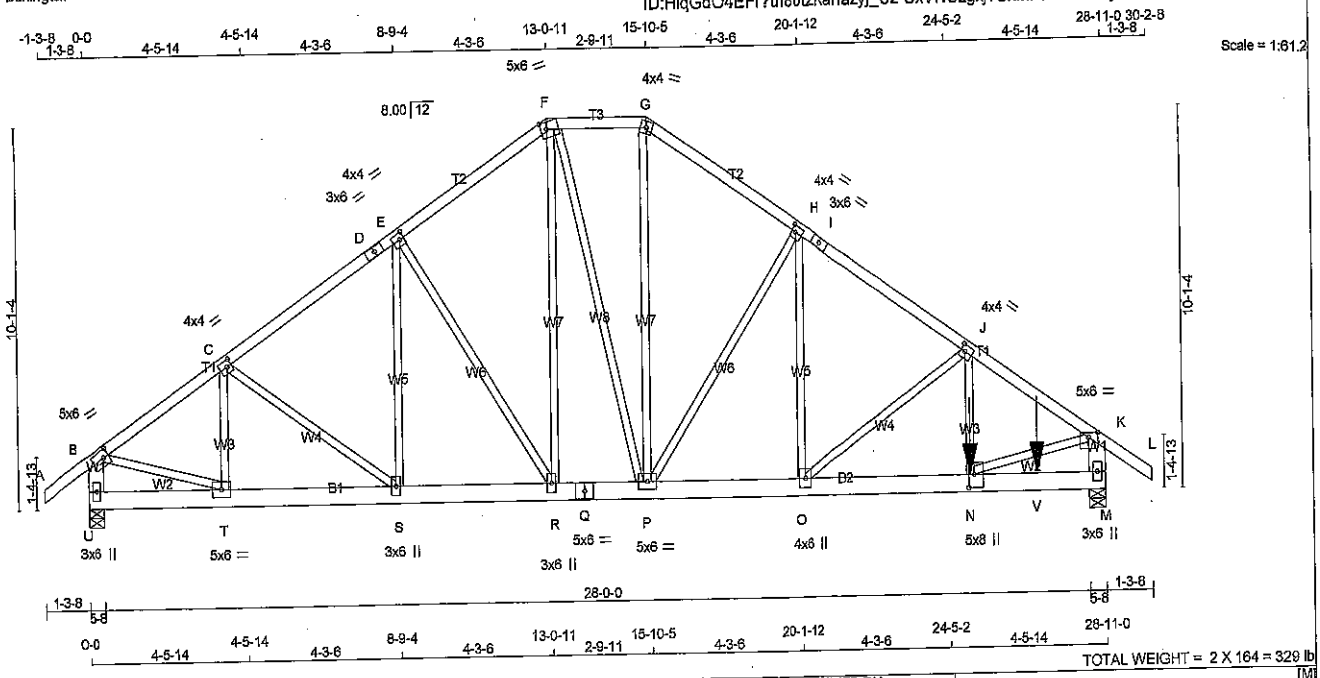


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	TOP
F-G	1	12	TOP
G-I	1	12	TOP
I-L	1	12	TOP
U-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U-Q	2	12	TOP
Q-M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
J-N	1	5	SIDE(364.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C, E, H, J						
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TTW-m	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	5.0	8.0	4.25	1.75
O	BMWW+t	MT20	4.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMWW+t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2350	0	2350	0	0	5-8	5-8
M	3925	0	3925	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1854	1170 / 0	346 / 0	0 / 0	0 / 0	339 / 0	0 / 0
M	3107	1941 / 0	592 / 0	0 / 0	0 / 0	574 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.08 (1)		10.00
B-C	-2562 / 0	-104.9	-104.9	0.17 (1)		5.49
C-D	-2539 / 0	-104.9	-104.9	0.21 (1)		5.47
D-E	-2539 / 0	-104.9	-104.9	0.21 (1)		5.47
E-F	-2168 / 0	-104.9	-104.9	0.19 (1)		5.80
F-G	-1887 / 0	-104.9	-104.9	0.09 (1)		8.25
G-H	-2288 / 0	-104.9	-104.9	0.20 (1)		5.69
H-I	-3104 / 0	-104.9	-104.9	0.23 (1)		5.04
I-J	-3104 / 0	-104.9	-104.9	0.23 (1)		5.04
J-K	-4536 / 0	-104.9	-104.9	0.21 (1)		4.34
K-L	0 / 40	-104.9	-104.9	0.08 (1)		10.00
U-B	-2290 / 0	0.0	0.0	0.08 (1)		7.81
M-K	-3811 / 0	0.0	0.0	0.14 (1)		7.23
U-T	0 / 0	-28.0	-28.0	0.03 (2)		10.00
T-S	0 / 2154	-28.0	-28.0	0.17 (1)		10.00
S-R	0 / 2116	-28.0	-28.0	0.16 (1)		10.00
R-Q	0 / 1800	-28.0	-28.0	0.15 (1)		10.00
Q-P	0 / 1800	-28.0	-28.0	0.15 (1)		10.00
P-O	0 / 2589	-28.0	-28.0	0.20 (1)		10.00
O-N	0 / 3793	-28.0	-28.0	0.31 (1)		10.00
N-V	0 / 0	-28.0	-28.0	0.07 (2)		10.00
V-M	0 / 0	-28.0	-28.0	0.07 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-447 / 0	0.05 (1)
C-S	-47 / 0	0.02 (1)
S-E	0 / 264	0.03 (2)
E-R	-597 / 0	0.41 (1)
R-F	0 / 568	0.07 (1)
F-P	0 / 325	0.04 (1)
P-G	0 / 938	0.12 (1)
P-H	-1320 / 0	0.91 (1)
O-H	0 / 1032	0.13 (1)
O-J	-1491 / 0	0.48 (1)
N-J	0 / 1195	0.15 (1)
B-T	0 / 2224	0.28 (1)
N-K	0 / 3916	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR
N	25-0-8	-2100	-2100	—	BACK	TYPE VERT TOTAL
V	26-11-12	-44	-76	—	TOP	TYPE VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.23/1.00 (H-J:1) , BC=0.31/1.00 (N-O:1) , WB=0.91/1.00 (H-P:1) , SSI=0.10/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

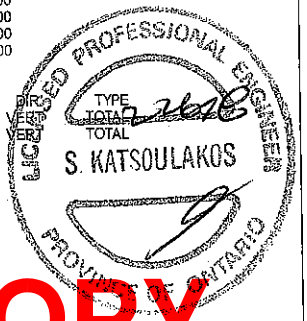
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.49 (N) (INPUT = 1.00)



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DWG NO. TAM 1023208
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T47	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2100.1 lbs FACTORED DOWN AT 25-0-8, AND 76.2 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/023-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T48	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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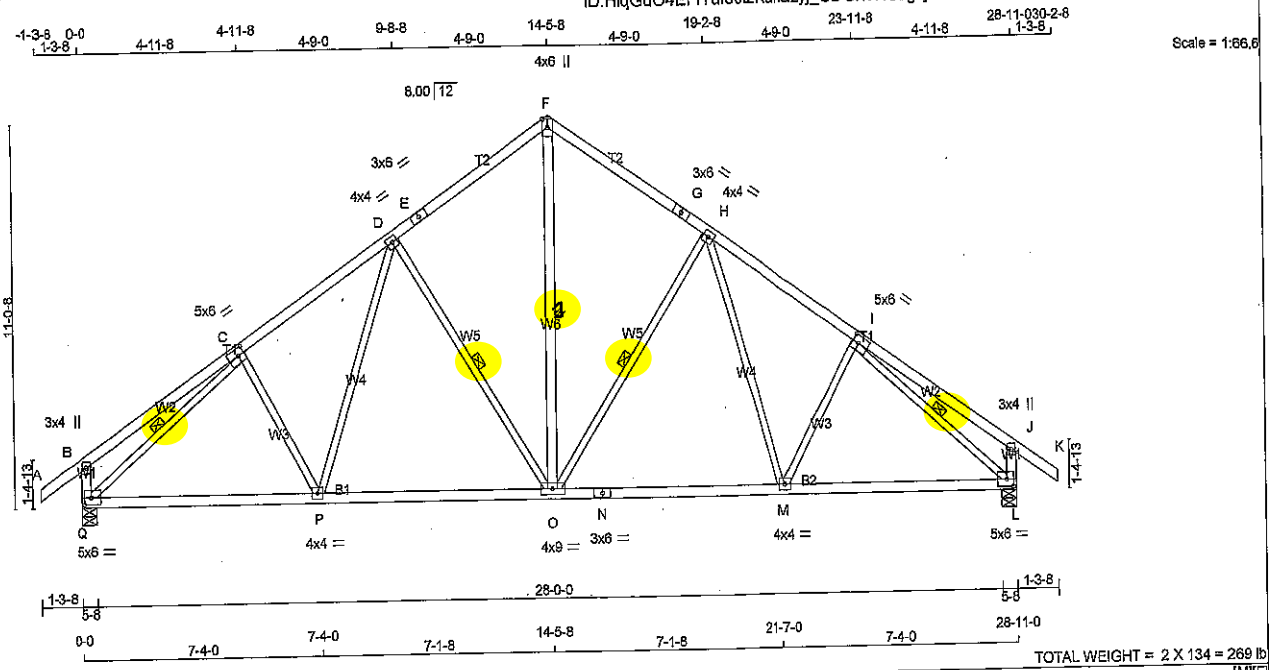

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2066	0	2066	0	0	5-8	5-8
L	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-O, D-O, C-Q, I-L. F-O ✓

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 25	-104.9	-104.9	0.30 (1)		10.00
C- D	-2131 / 0	-104.9	-104.9	0.33 (1)		4.37
D- E	-1597 / 0	-104.9	-104.9	0.32 (1)		4.91
E- F	-1597 / 0	-104.9	-104.9	0.32 (1)		4.91
F- G	-1597 / 0	-104.9	-104.9	0.32 (1)		4.91
G- H	-1597 / 0	-104.9	-104.9	0.32 (1)		4.91
H- I	-2131 / 0	-104.9	-104.9	0.33 (1)		4.37
I- J	0 / 25	-104.9	-104.9	0.30 (1)		10.00
J- K	0 / 40	-104.9	-104.9	0.14 (1)		10.00
Q- B	-352 / 0	0.0	0.0	0.04 (1)		7.81
L- J	-352 / 0	0.0	0.0	0.04 (1)		7.81
Q- P	0 / 1832	-28.0	-28.0	0.55 (2)		10.00
P- O	0 / 1673	-28.0	-28.0	0.53 (2)		10.00
O- N	0 / 1673	-28.0	-28.0	0.53 (2)		10.00
N- M	0 / 1673	-28.0	-28.0	0.53 (2)		10.00
M- L	0 / 1832	-28.0	-28.0	0.55 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1333	0.21 (1)
O- H	-678 / 0	0.37 (1)
H- M	0 / 374	0.08 (2)
M- I	-115 / 108	0.05 (1)
D- O	-678 / 0	0.37 (1)
P- D	0 / 374	0.08 (2)
C- P	-115 / 108	0.05 (1)
Q- C	-2453 / 0	0.70 (1)
I- L	-2453 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.55/1.00 (L-M:2) , WB=0.70/1.00 (C-Q:1) , SSI=0.20/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2261 B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102333-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T49	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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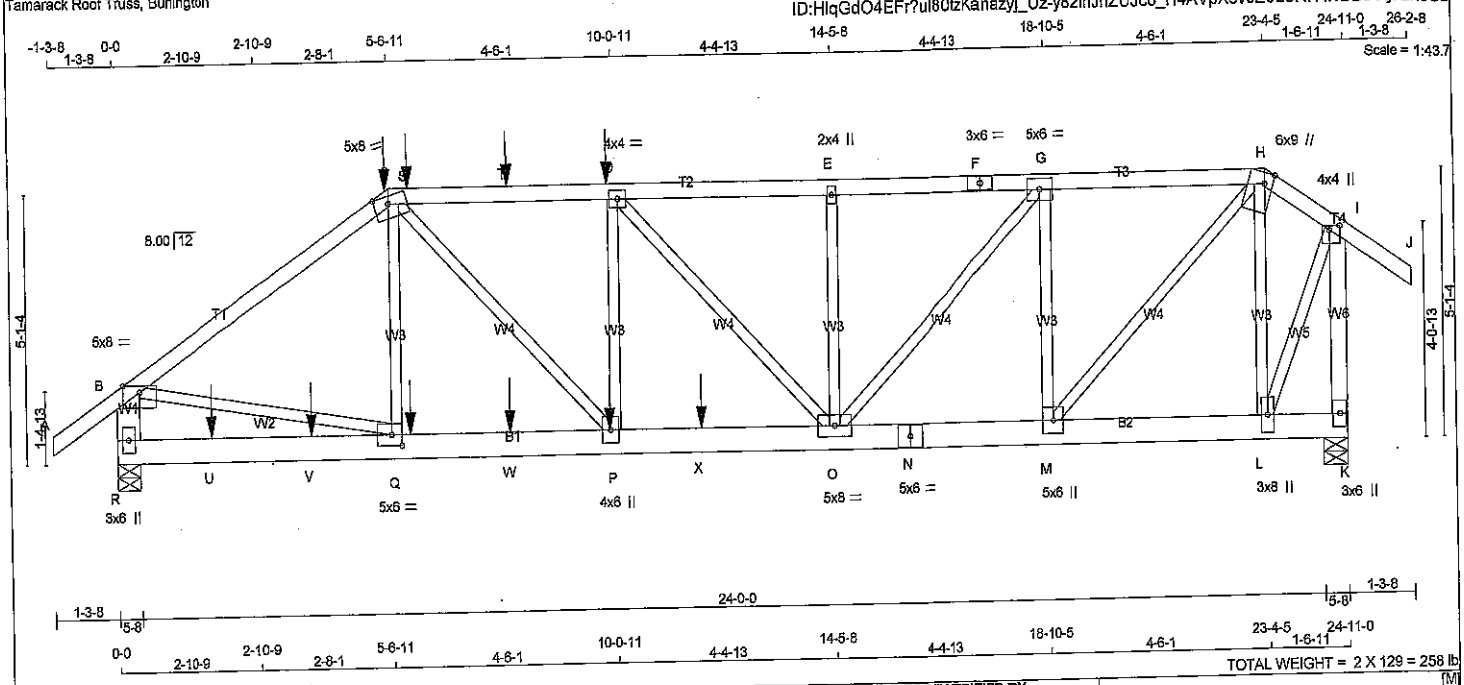

TOTAL WEIGHT = 2 X 129 = 258 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
K- I	1	12	TOP
R- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	6	SIDE(15.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	1.75	3.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW+p	MT20	4.0	4.0	1.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	3.0	8.0		
M	BMWW+t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	4586	0	4586	0	0	5-8	5-8
K	3244	0	3244	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3631	2267 / 0	693 / 0	0 / 0	0 / 0	671 / 0	0 / 0
K	2555	1618 / 0	472 / 0	0 / 0	0 / 0	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-5414 / 0	-104.9	-104.9	0.56 (1)	3.73
C- S	-6123 / 0	-104.9	-104.9	0.35 (1)	3.68
S- T	-6123 / 0	-104.9	-104.9	0.35 (1)	3.68
T- D	-6123 / 0	-104.9	-104.9	0.35 (1)	3.68
D- E	-5412 / 0	-104.9	-104.9	0.30 (1)	3.93
E- F	-5412 / 0	-104.9	-104.9	0.29 (1)	3.94
F- G	-5412 / 0	-104.9	-104.9	0.29 (1)	3.94
G- H	-3452 / 0	-104.9	-104.9	0.22 (1)	4.83
H- I	-1286 / 0	-104.9	-104.9	0.08 (1)	6.25
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00
R- B	-4239 / 0	0.0	0.0	0.15 (1)	6.94
K- I	-3277 / 0	0.0	0.0	0.43 (1)	6.42
R- U	0 / 0	-28.0	-28.0	0.23 (1)	10.00
U- V	0 / 0	-28.0	-28.0	0.23 (1)	10.00
V- Q	0 / 0	-28.0	-28.0	0.23 (1)	10.00
Q- W	0 / 4503	-28.0	-28.0	0.52 (1)	10.00
W- P	0 / 4503	-28.0	-28.0	0.52 (1)	10.00
P- X	0 / 6123	-28.0	-28.0	0.93 (1)	10.00
X- O	0 / 6123	-28.0	-28.0	0.93 (1)	10.00
O- N	0 / 3452	-28.0	-28.0	0.39 (1)	10.00
N- M	0 / 3452	-28.0	-28.0	0.39 (1)	10.00
M- L	0 / 990	-28.0	-28.0	0.11 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- Q	0 / 4569	0.57 (1)
Q- C	0 / 381	0.04 (3)
C- P	0 / 2344	0.29 (1)
P- D	0 / 266	0.03 (3)
D- O	-1043 / 0	0.39 (1)
O- E	-412 / 0	0.08 (1)
O- G	0 / 2878	0.36 (1)
M- G	-2633 / 0	0.49 (1)
M- H	0 / 3564	0.44 (1)
L- H	-2096 / 0	0.39 (1)
L- I	0 / 2511	0.31 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	5-6-11	-527	-527	---	FRONT	VERT	TOTAL
D	9-11-4	-54	-54	---	BACK	VERT	TOTAL
P	9-11-4	-337	-337	---	BACK	VERT	TOTAL
Q	5-11-4	-337	-337	---	BACK	VERT	TOTAL
S	5-11-4	-86	-86	---	BACK	VERT	TOTAL
T	7-11-4	-54	-54	---	BACK	VERT	TOTAL
U	1-11-4	-337	-337	---	BACK	VERT	TOTAL
V	3-11-4	-337	-337	---	BACK	VERT	TOTAL
W	7-11-4	-337	-337	---	BACK	VERT	TOTAL
X	11-9-8	-1826	-1826	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.93/1.00 (O-P:1) , WB=0.57/1.00 (B-Q:1) , SSI=0.40/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 10234 -B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T49	1	2	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 5-6-11, 86.3 lbs FACTORED DOWN AT 5-11-4, AND 53.9 lbs FACTORED DOWN AT 7-11-4, AND 53.9 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, AND 336.6 lbs FACTORED DOWN AT 9-11-4, AND 1825.5 lbs FACTORED DOWN AT 11-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10234-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T50	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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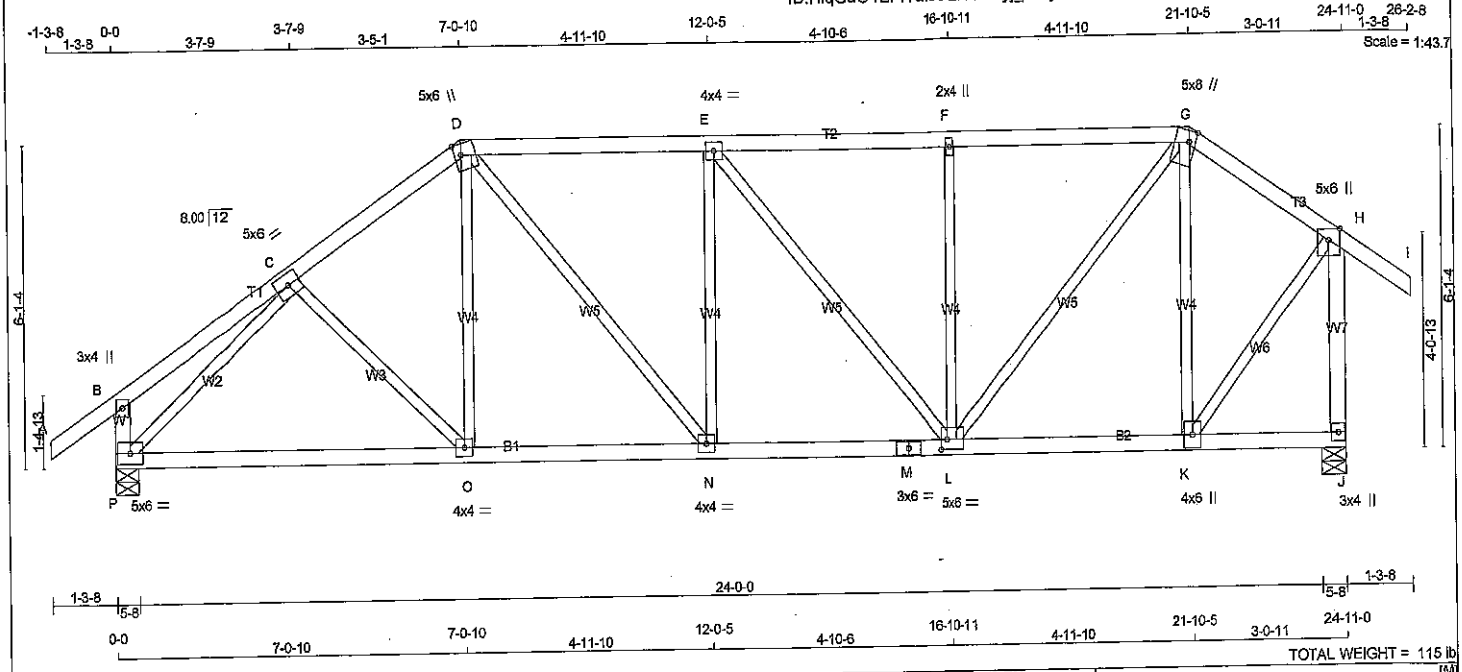

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	8.0	2.50	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.25	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED HORZ	MAXIMUM FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1800	0	1800	0	0	5-8	5-8
J	1800	0	1800	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1417	898 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
J	1417	898 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 22	-104.9	-104.9	0.19 (1)	10.00	
C- D	-1784 / 0	-104.9	-104.9	0.24 (1)	4.77	
D- E	-1820 / 0	-104.9	-104.9	0.41 (1)	4.50	
E- F	-1600 / 0	-104.9	-104.9	0.40 (1)	4.75	
F- G	-1599 / 0	-104.9	-104.9	0.39 (1)	4.77	
G- H	-952 / 0	-104.9	-104.9	0.18 (1)	6.13	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
P- B	-280 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-1772 / 0	0.0	0.0	0.49 (1)	6.27	
P- O	0 / 1466	-28.0	-28.0	0.46 (2)	10.00	
O- N	0 / 1467	-28.0	-28.0	0.47 (2)	10.00	
N- M	0 / 1820	-28.0	-28.0	0.36 (1)	10.00	
M- L	0 / 1820	-28.0	-28.0	0.36 (1)	10.00	
L- K	0 / 775	-28.0	-28.0	0.22 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.11 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-3 / 122	0.03 (3)
O- D	0 / 294	0.07 (2)
D- N	0 / 542	0.12 (1)
N- E	-298 / 66	0.17 (1)
E- L	-344 / 0	0.39 (1)
L- F	-559 / 0	0.33 (1)
L- G	0 / 1266	0.28 (1)
K- G	-808 / 0	0.47 (1)
P- C	-2046 / 0	0.79 (1)
K- H	0 / 1226	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.49/1.00 (H-J:1) , BC=0.47/1.00 (N-O:2) , WB=0.79/1.00 (C-P:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.50 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10235-R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T51	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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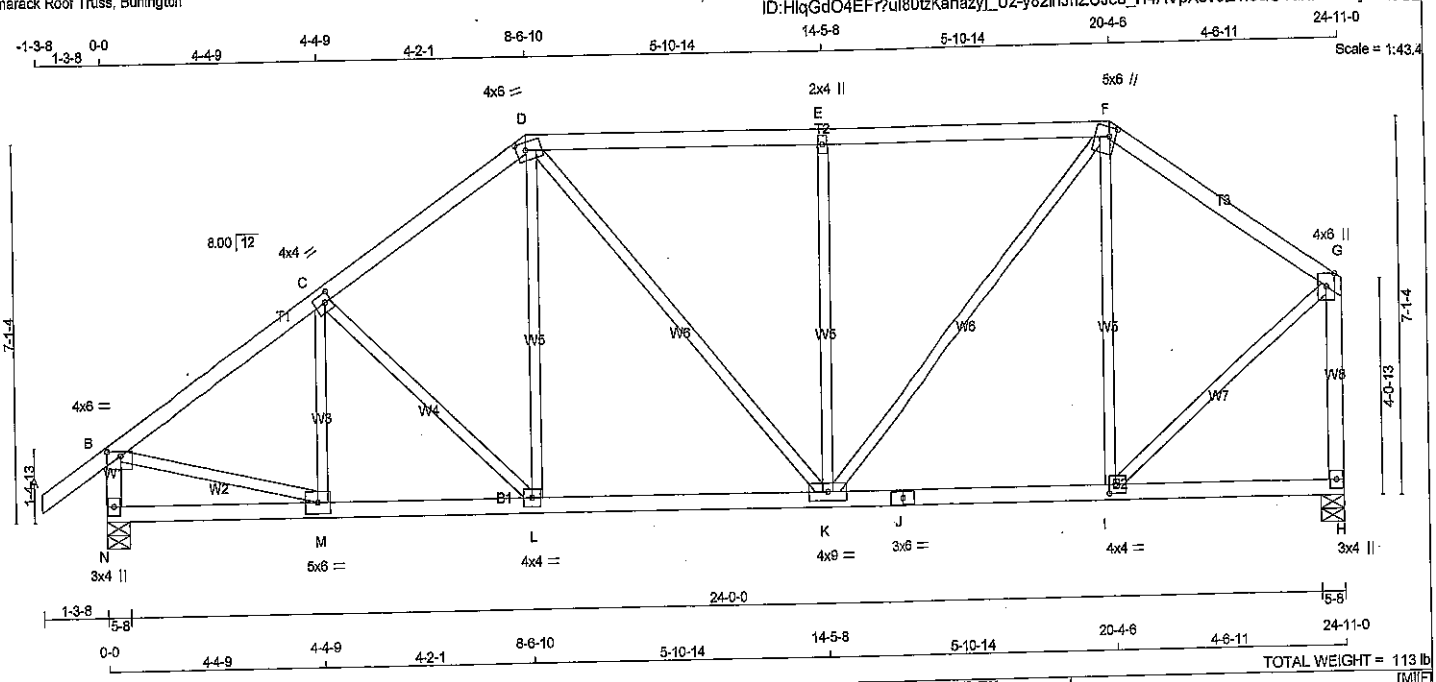

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1800	0	1800	0	0	5-8	5-8
H	1656	0	1656	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1417	898 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
H	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1844 / 0	-104.9	-104.9	0.28 (1)	4.68	
C- D	-1684 / 0	-104.9	-104.9	0.27 (1)	4.85	
D- E	-1550 / 0	-104.9	-104.9	0.50 (1)	4.67	
E- F	-1550 / 0	-104.9	-104.9	0.50 (1)	4.67	
F- G	-1118 / 0	-104.9	-104.9	0.30 (1)	5.65	
N- B	-1746 / 0	0.0	0.0	0.18 (1)	6.31	
H- G	-1608 / 0	0.0	0.0	0.44 (1)	6.52	
N- M	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
M- L	0 / 1558	-28.0	-28.0	0.34 (1)	10.00	
L- K	0 / 1379	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 920	-28.0	-28.0	0.29 (2)	10.00	
J- I	0 / 920	-28.0	-28.0	0.29 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-263 / 63	0.07 (1)
C- L	-252 / 0	0.15 (1)
L- D	0 / 352	0.08 (2)
D- K	0 / 261	0.06 (1)
K- E	-761 / 0	0.67 (1)
K- F	0 / 962	0.22 (1)
I- F	-582 / 0	0.51 (1)
B- M	0 / 1605	0.36 (1)
I- G	0 / 1191	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.34/1.00 (K-L:2) , WB=0.67/1.00 (E-K:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

DWG NO. TAM 10236-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T52	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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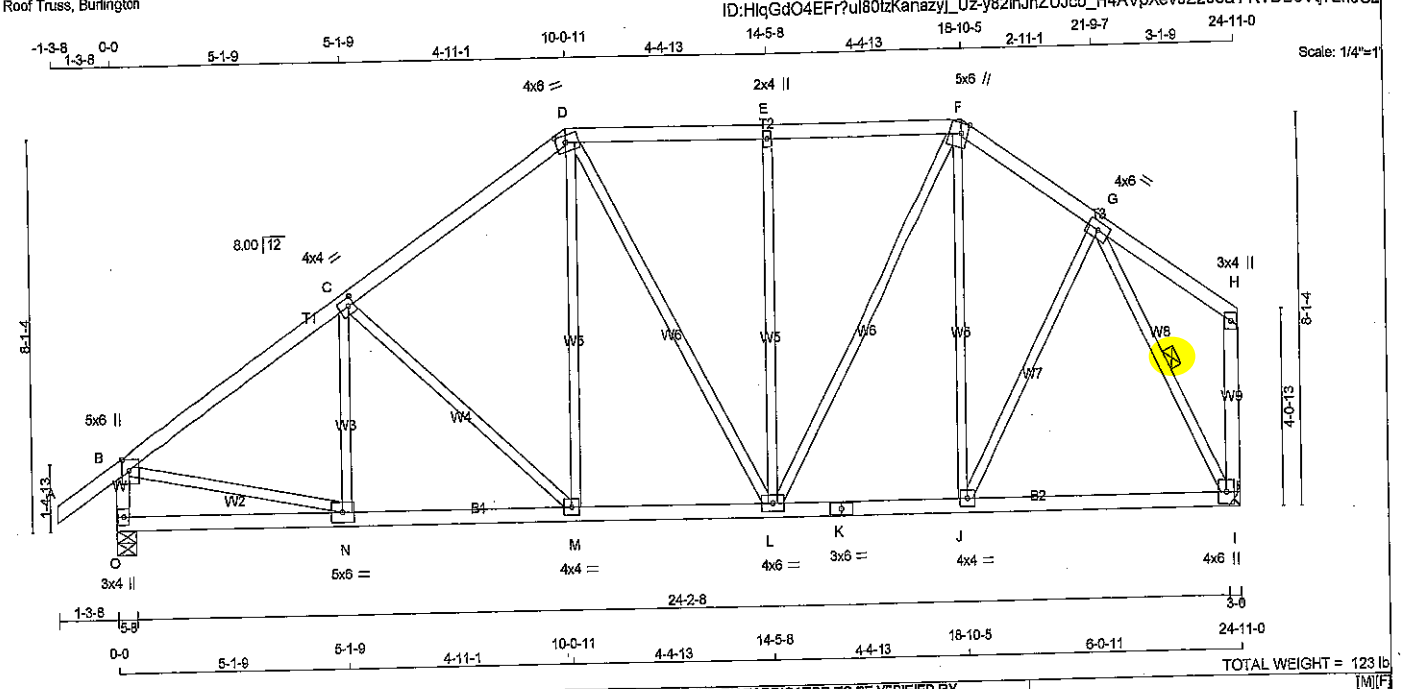

TOTAL WEIGHT = 123 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1800	0	1800	0	0	IN-SX	IN-SX
I	1656	0	1656	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1417	898 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
I	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-1868 / 0	-104.9	-104.9	0.39 (1)	4.52
C- D	-1571 / 0	-104.9	-104.9	0.37 (1)	4.86
D- E	-1319 / 0	-104.9	-104.9	0.27 (1)	5.33
E- F	-1319 / 0	-104.9	-104.9	0.27 (1)	5.33
F- G	-1212 / 0	-104.9	-104.9	0.13 (1)	5.69
G- H	0 / 19	-104.9	-104.9	0.15 (1)	10.00
O- B	-1740 / 0	0.0	0.0	0.18 (1)	6.32
I- H	-126 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 0	-28.0	-28.0	0.17 (3)	10.00
N- M	0 / 1583	-28.0	-28.0	0.34 (2)	10.00
M- L	0 / 1281	-28.0	-28.0	0.27 (1)	10.00
L- K	0 / 987	-28.0	-28.0	0.34 (2)	10.00
K- J	0 / 987	-28.0	-28.0	0.34 (2)	10.00
J- I	0 / 772	-28.0	-28.0	0.31 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-177 / 141	0.06 (1)
C- M	-412 / 0	0.35 (1)
M- D	0 / 411	0.09 (1)
D- L	0 / 78	0.02 (1)
L- E	-561 / 0	0.71 (1)
L- F	0 / 676	0.15 (1)
J- F	-226 / 60	0.29 (1)
J- G	0 / 461	0.10 (1)
B- N	0 / 1617	0.36 (1)
G- I	-1652 / 0	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.34/1.00 (J-L:2) , WB=0.71/1.00 (E-L:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.40 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 10237-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T53	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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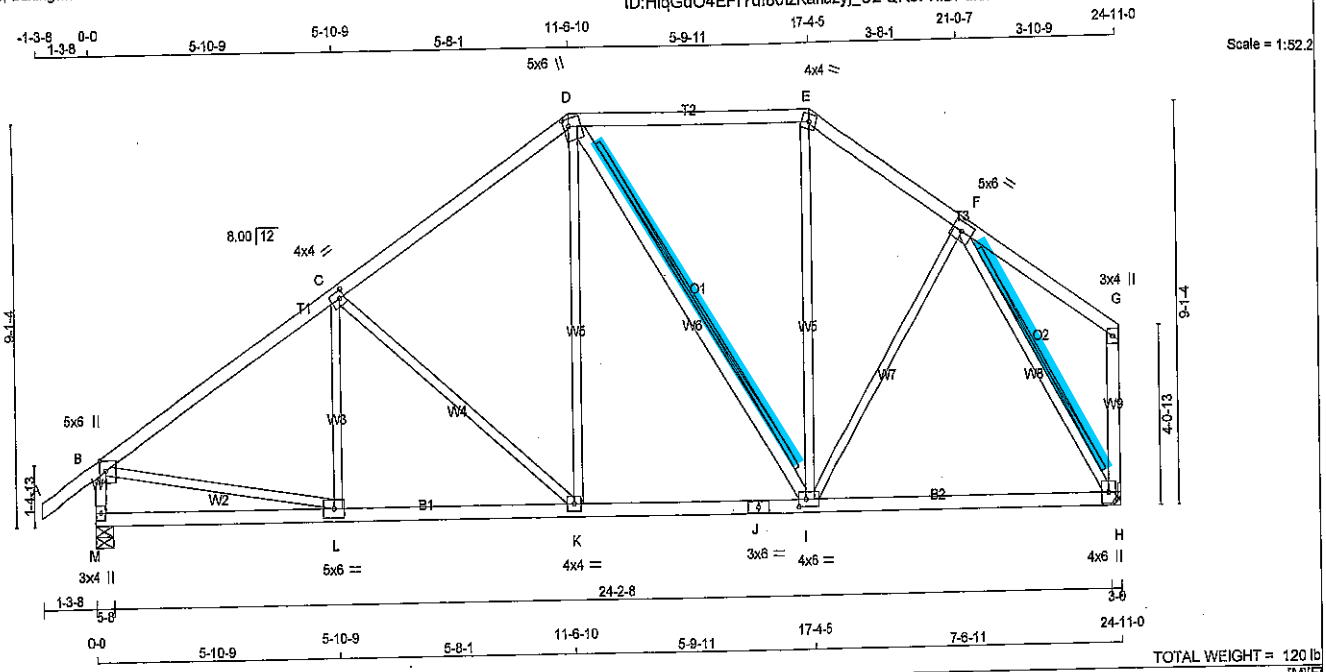

TOTAL WEIGHT = 120 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	2.00
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1600	0	1600	0	0	5-8	5-8
H	1656	0	1656	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1417	898 / 0	262 / 0	0 / 0	0 / 0	257 / 0	0 / 0
H	1320	809 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-I, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B-C	-1872 / 0	-104.9	-104.9	0.53 (1)	4.33
C-D	-1445 / 0	-104.9	-104.9	0.49 (1)	4.85
D-E	-1012 / 0	-104.9	-104.9	0.47 (1)	5.57
E-F	-1241 / 0	-104.9	-104.9	0.20 (1)	5.55
F-G	0 / 25	-104.9	-104.9	0.24 (1)	10.00
M-B	-1733 / 0	0.0	0.0	0.18 (1)	6.32
H-G	-155 / 0	0.0	0.0	0.04 (1)	7.81
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00
L-K	0 / 1590	-28.0	-28.0	0.41 (2)	10.00
K-J	0 / 1172	-28.0	-28.0	0.49 (2)	10.00
J-I	0 / 1172	-28.0	-28.0	0.49 (2)	10.00
I-H	0 / 856	-28.0	-28.0	0.45 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-106 / 209	0.05 (3)
C-K	-561 / 0	0.86 (1)
K-D	0 / 500	0.11 (1)
D-I	-295 / 0	0.22 (1)
I-E	0 / 291	0.07 (2)
I-F	0 / 293	0.07 (1)
B-L	0 / 1617	0.36 (1)
F-H	-1653 / 0	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.49/1.00 (I-K:2) , WB=0.66/1.00 (C-K:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10238-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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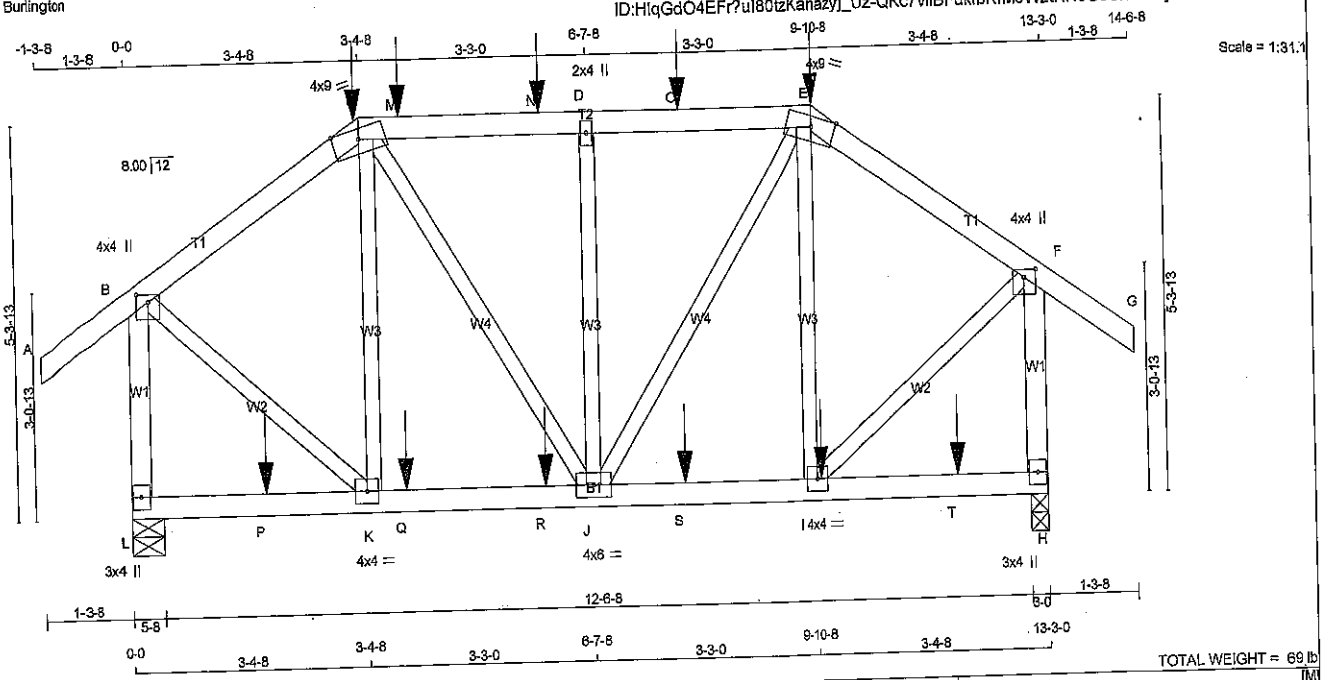

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	9.0	Edge	
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 69.0 lbs FACTORED DOWN AT 3-4-8, 118.4 lbs FACTORED DOWN AT 9-10-8, 68.9 lbs FACTORED DOWN AT 3-11-4, AND 45.9 lbs FACTORED DOWN AT 5-11-4, AND 45.9 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 21.4 lbs FACTORED DOWN AT 1-11-4, 21.4 lbs FACTORED DOWN AT 3-11-4, 21.4 lbs FACTORED DOWN AT 5-11-4, 21.4 lbs FACTORED DOWN AT 7-11-4, AND 21.4 lbs FACTORED DOWN AT 9-11-4, AND 21.4 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1234	0	1234	0	0	5-6	5-6
H	1237	0	1237	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	969	617 / 0	176 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	974	617 / 0	179 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A-B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	
B-C	-718 / 0	-104.9	-104.9	0.24 (1)	6.25	
C-M	-780 / 0	-104.9	-104.9	0.25 (1)	6.25	
M-N	-780 / 0	-104.9	-104.9	0.25 (1)	6.25	
N-D	-780 / 0	-104.9	-104.9	0.25 (1)	6.25	
D-O	-780 / 0	-104.9	-104.9	0.25 (1)	6.25	
O-E	-780 / 0	-104.9	-104.9	0.25 (1)	6.25	
E-F	-719 / 0	-104.9	-104.9	0.24 (1)	6.25	
F-G	0 / 40	-104.9	-104.9	0.15 (1)	10.00	
L-B	-1192 / 0	0.0	0.0	0.21 (1)	7.29	
H-F	-1193 / 0	0.0	0.0	0.21 (1)	7.28	
L-P	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
P-K	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
K-Q	0 / 589	-28.0	-28.0	0.18 (2)	10.00	
Q-R	0 / 589	-28.0	-28.0	0.18 (2)	10.00	
R-J	0 / 589	-28.0	-28.0	0.18 (2)	10.00	
J-S	0 / 590	-28.0	-28.0	0.17 (2)	10.00	
S-I	0 / 590	-28.0	-28.0	0.17 (2)	10.00	
I-T	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
T-H	0 / 0	-28.0	-28.0	0.10 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-334 / 19	0.15 (1)
C-J	0 / 352	0.09 (1)
J-D	-505 / 0	0.22 (1)
J-E	0 / 351	0.09 (1)
I-E	-334 / 20	0.15 (1)
B-K	0 / 759	0.19 (1)
I-F	0 / 760	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
C	3-4-8	-69	-69	—	FRONT	VERT	TOTAL
E	9-10-8	-118	-118	—	FRONT	VERT	TOTAL
I	9-11-4	-12	-21	—	FRONT	VERT	TOTAL
M	3-11-4	-69	-69	—	FRONT	VERT	TOTAL
N	5-11-4	-46	-46	—	FRONT	VERT	TOTAL
O	7-11-4	-46	-46	—	FRONT	VERT	TOTAL
P	1-11-4	-12	-21	—	FRONT	VERT	TOTAL
Q	3-11-4	-12	-21	—	FRONT	VERT	TOTAL
R	5-11-4	-12	-21	—	FRONT	VERT	TOTAL
S	7-11-4	-12	-21	—	FRONT	VERT	TOTAL
T	11-11-4	-12	-21	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.18/1.00 (J-K:2) , WB=0.22/1.00 (D-J:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.22 (K) (INPUT = 1.00)

DWG NO. TAM 10239-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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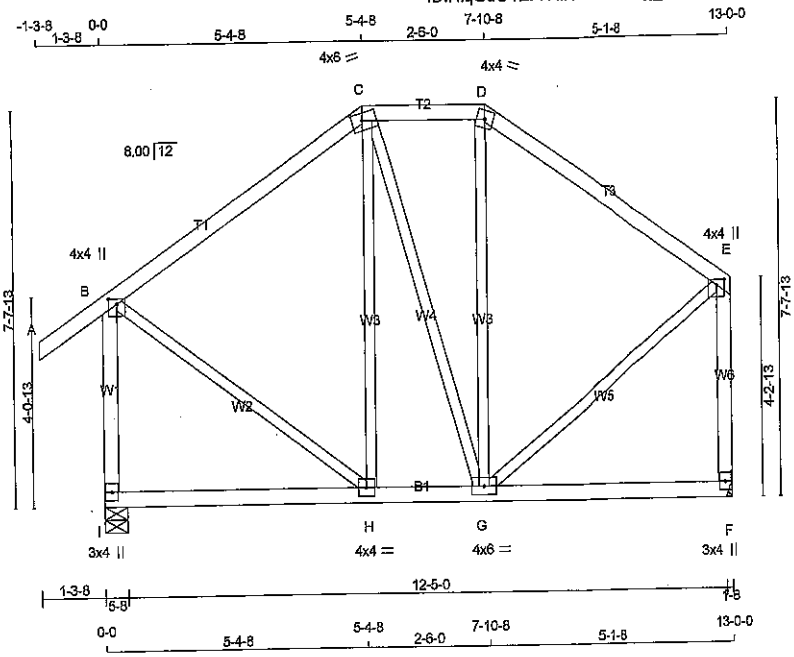

Scale = 1:44.6

TOTAL WEIGHT = 70 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1008	0	1008	0	0	5-8	5-8
F	864	0	864	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	766	511 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
F	689	422 / 0	137 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-450 / 0	-104.9	-104.9	0.39 (1)	6.25
C- D	-373 / 0	-104.9	-104.9	0.09 (1)	6.25
D- E	-452 / 0	-104.9	-104.9	0.36 (1)	6.25
I- B	-944 / 0	0.0	0.0	0.26 (1)	7.81
F- E	-801 / 0	0.0	0.0	0.24 (1)	7.81
I- H	0 / 0	-28.0	-28.0	0.18 (2)	10.00
H- G	0 / 381	-28.0	-28.0	0.21 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.16 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-135 / 99	0.15 (1)
C- G	-23 / 0	0.03 (1)
G- D	-149 / 85	0.16 (1)
B- H	0 / 463	0.10 (1)
G- E	0 / 469	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.21/1.00 (G-H:2) , WB=0.16/1.00 (D-G:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102490-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T56	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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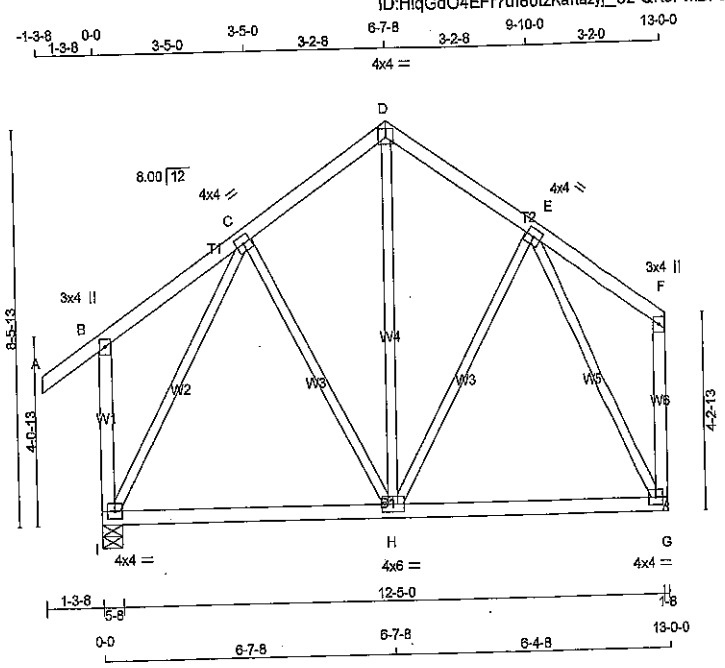

TOTAL WEIGHT = 71 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1008	0	1008	0	0	IN-SX	IN-SX
G	864	0	864	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	786	511 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
G	689	422 / 0	137 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 22	-104.9	-104.9	0.19 (1)	10.00
C- D	-457 / 0	-104.9	-104.9	0.14 (1)	6.25
D- E	-455 / 0	-104.9	-104.9	0.13 (1)	6.25
E- F	0 / 22	-104.9	-104.9	0.17 (1)	10.00
I- B	-280 / 0	0.0	0.0	0.08 (1)	7.81
G- F	-123 / 0	0.0	0.0	0.04 (1)	7.81
I- H	0 / 368	-28.0	-28.0	0.36 (2)	10.00
H- G	0 / 349	-28.0	-28.0	0.36 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-14 / 98	0.02 (3)
H- D	0 / 246	0.06 (2)
H- E	0 / 109	0.02 (3)
I- C	-753 / 0	0.63 (1)
E- G	-759 / 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.38/1.00 (H-I:2) , WB=0.63/1.00 (C-I:1) , SSI=0.16/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM1024[illegible]
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T57	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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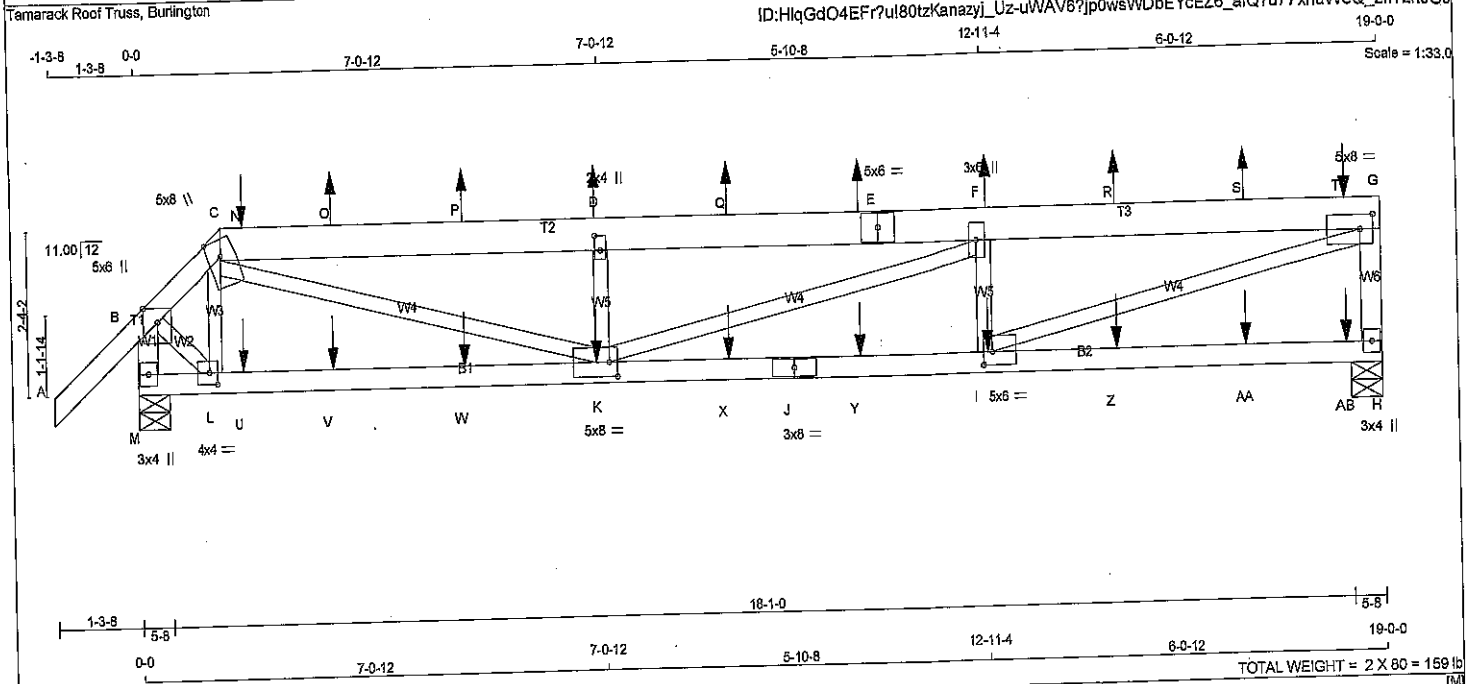

TOTAL WEIGHT = 2 X 80 = 159 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP SIDE(12.9)
G- H	1	12	
M- B	1	12	TOP SIDE(183.1)
C- E	2	12	SIDE(183.1)
E- G	2	12	
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	1	12	SIDE(61.0)
J- H	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
I- F	1	6	SIDE(11.1)
K- D	1	6	SIDE(11.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TMW-t	MT20	5.0	8.0	2.50	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0	2.25	1.50
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	1.50
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2734	0	2734	0	0	5-8	5-8
M	2746	0	2746	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2177	1334 / 0	428 / 0	0 / 0	0 / 0	414 / 0	0 / 0
M	2164	1365 / 0	401 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 49	-104.9	-104.9	0.08 (1)	10.00
B- C	-2497 / 0	-104.9	-104.9	0.10 (1)	5.63
C- N	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
N- O	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
O- P	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
P- D	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
D- Q	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
Q- E	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
E- F	-6119 / 0	-104.9	-104.9	0.21 (1)	4.63
F- R	-5779 / 0	-104.9	-104.9	0.23 (1)	4.71
R- S	-5779 / 0	-104.9	-104.9	0.23 (1)	4.71
S- T	-5779 / 0	-104.9	-104.9	0.23 (1)	4.71
T- G	-5779 / 0	-104.9	-104.9	0.23 (1)	4.71
H- G	-2199 / 0	0.0	0.0	0.14 (1)	7.51
M- B	-3166 / 0	0.0	0.0	0.18 (1)	6.52
M- L	0 / 0	-28.0	-28.0	0.39 (1)	10.00
L- U	0 / 1819	-28.0	-28.0	0.55 (1)	10.00
U- V	0 / 1819	-28.0	-28.0	0.55 (1)	10.00
V- W	0 / 1819	-28.0	-28.0	0.55 (1)	10.00
W- K	0 / 1819	-28.0	-28.0	0.55 (1)	10.00
K- X	0 / 5779	-28.0	-28.0	0.90 (1)	10.00
X- J	0 / 5779	-28.0	-28.0	0.90 (1)	10.00
J- Y	0 / 5779	-28.0	-28.0	0.90 (1)	10.00
Y- I	0 / 5779	-28.0	-28.0	0.90 (1)	10.00
I- Z	0 / 0	-28.0	-28.0	0.45 (1)	10.00
Z-AA	0 / 0	-28.0	-28.0	0.45 (1)	10.00
AA-AB	0 / 0	-28.0	-28.0	0.45 (1)	10.00
AB- H	0 / 0	-28.0	-28.0	0.45 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
L- C	-121 / 119	0.01 (3)
B- L	0 / 2193	0.27 (1)
I- G	0 / 6076	0.75 (1)
C- K	0 / 4546	0.56 (1)
I- F	-739 / 0	0.06 (1)
K- D	-652 / 0	0.05 (1)
K- F	0 / 358	0.04 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-11-12	1	1	3	BACK	VERT	TOTAL
E	10-11-12	1	1	3	BACK	VERT	TOTAL
F	12-11-12	1	1	3	BACK	VERT	TOTAL
[illegible]	12-11-12	-276	-276	—	BACK	VERT	TOTAL
K	6-11-12	-276	-276	—	BACK	VERT	TOTAL
N	1-7-8	-28	-28	3	BACK	VERT	TOTAL
O	2-11-12	1	1	3	BACK	VERT	TOTAL
P	4-11-12	1	1	3	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 729 (0.31")

CSI: TC=0.23/1.00 (F-G:1) , BC=0.90/1.00 (I-K:1) , WB=0.75/1.00 (G-I:1) , SSI=0.28/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.97 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1024208
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T57	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 28.0 lbs FACTORED DOWN AND 2.6 lbs FACTORED UP AT 1-7-6, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 2-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 4-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 6-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 8-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 10-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 12-11-12, 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 14-11-12, AND 1.0 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 16-11-12, AND 17.6 lbs FACTORED DOWN AND 3.1 lbs FACTORED UP AT 18-5-12 ON TOP CHORD, AND 276.4 lbs FACTORED DOWN AT 1-7-6, 276.4 lbs FACTORED DOWN AT 2-11-12, 276.4 lbs FACTORED DOWN AT 4-11-12, 276.4 lbs FACTORED DOWN AT 6-11-12, 276.4 lbs FACTORED DOWN AT 8-11-12, 276.4 lbs FACTORED DOWN AT 10-11-12, 276.4 lbs FACTORED DOWN AT 12-11-12, 276.4 lbs FACTORED DOWN AT 14-11-12, AND 276.4 lbs FACTORED DOWN AT 16-11-12, AND 283.1 lbs FACTORED DOWN AT 18-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	8-11-12	1	1	3	BACK	VERT	TOTAL
R	14-11-12	1	1	3	BACK	VERT	TOTAL
S	16-11-12	1	1	3	BACK	VERT	TOTAL
T	18-5-12	-18	-18	3	BACK	VERT	TOTAL
U	1-7-6	-276	-276	—	BACK	VERT	TOTAL
V	2-11-12	-276	-276	—	BACK	VERT	TOTAL
W	4-11-12	-276	-276	—	BACK	VERT	TOTAL
X	8-11-12	-276	-276	—	BACK	VERT	TOTAL
Y	10-11-12	-276	-276	—	BACK	VERT	TOTAL
Z	14-11-12	-276	-276	—	BACK	VERT	TOTAL
AA	16-11-12	-276	-276	—	BACK	VERT	TOTAL
AB	18-5-12	-283	-283	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/2017-[illegible]
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	T60	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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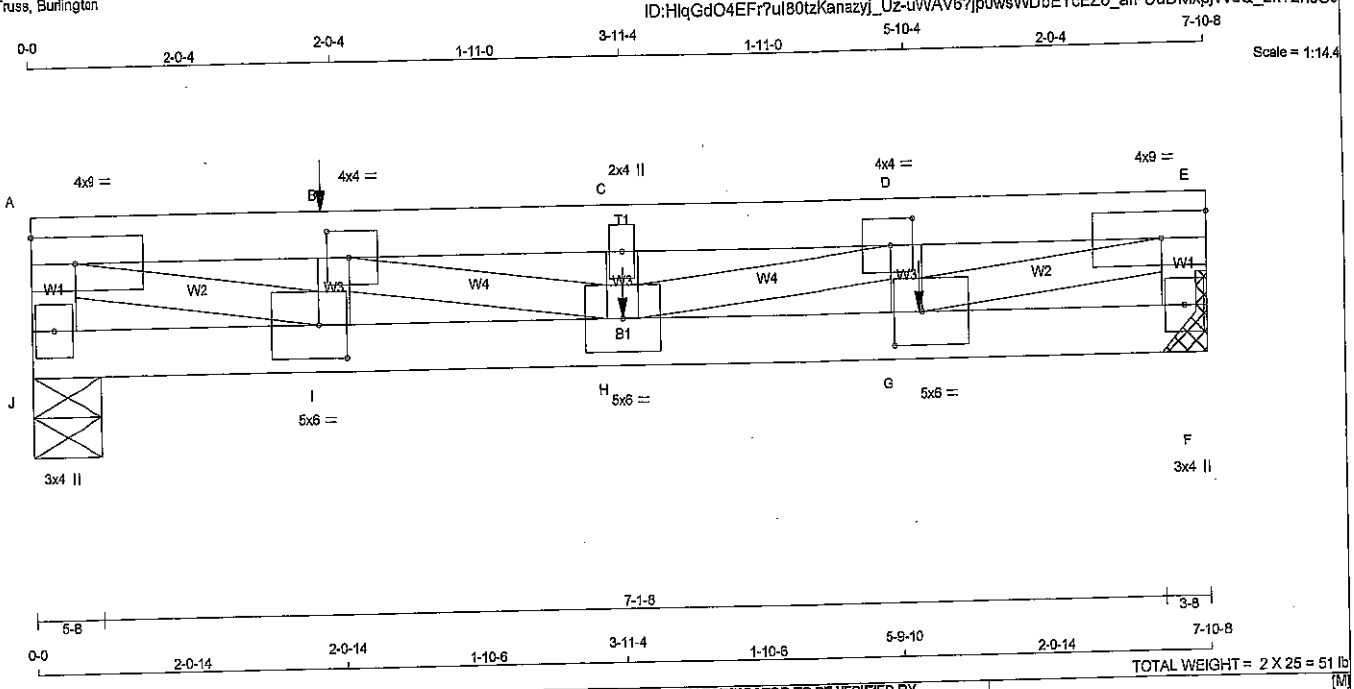

TOTAL WEIGHT = 2 X 25 = 51 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J- A	1	12	TOP
A- E	1	12	TOP
E- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	1	5	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	9.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	9.0		Edge
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	5.0	6.0	2.50	2.25
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1132.5 lbs FACTORED DOWN AT 1-11-4 ON TOP CHORD, AND 836.0 lbs FACTORED DOWN AT 3-11-4, AND 836.0 lbs FACTORED DOWN AT 5-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1998	0	1998	0	0	5-8	5-8
F	1854	0	1854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1583	985 / 0	303 / 0	0 / 0	0 / 0	295 / 0	0 / 0
F	1462	922 / 0	272 / 0	0 / 0	0 / 0	267 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1880 / 0	0.0	0.0	0.10 (1)	7.81
A- B	-4841 / 0	-104.9	-104.9	0.19 (1)	4.21
B- C	-6147 / 0	-104.9	-104.9	0.27 (1)	3.70
C- D	-6147 / 0	-104.9	-104.9	0.26 (1)	3.72
D- E	-4506 / 0	-104.9	-104.9	0.17 (1)	4.38
F- E	-1748 / 0	0.0	0.0	0.10 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.14 (1)	10.00
I- H	0 / 4841	-28.0	-28.0	0.57 (1)	10.00
H- G	0 / 4506	-28.0	-28.0	0.53 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.12 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 5130	0.63 (1)
I- B	-1737 / 0	0.13 (1)
B- H	0 / 1392	0.17 (1)
H- C	-200 / 0	0.02 (1)
H- D	0 / 1749	0.22 (1)
G- D	-759 / 0	0.06 (1)
G- E	0 / 4774	0.59 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	1-11-4	-1132	-1132	---	TOP	VERT	TOTAL
G	5-11-4	-836	-836	---	BACK	VERT	TOTAL
H	3-11-4	-836	-836	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 725 (0.13")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.57/1.00 (H-I:1) , WB=0.63/1.00 (A-I:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.60 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2/26/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102436
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295156	T100	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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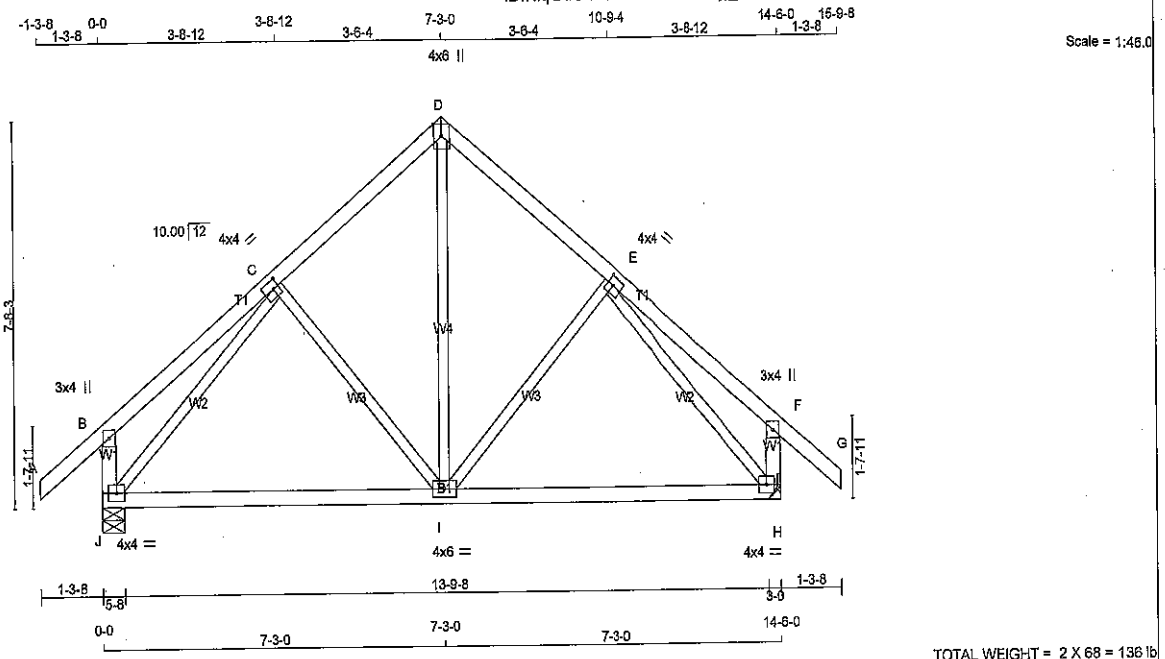

TOTAL WEIGHT = 2 X 68 = 136 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1109	0	1109	0	0	5-8	5-8
H	1109	0	1109	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	866	561 / 0	152 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	866	561 / 0	152 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 28	-104.9	-104.9	0.22 (1)	10.00
C- D	-678 / 0	-104.9	-104.9	0.17 (1)	6.25
D- E	-678 / 0	-104.9	-104.9	0.17 (1)	6.25
E- F	0 / 28	-104.9	-104.9	0.22 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- B	-290 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-290 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 620	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 620	-28.0	-28.0	0.49 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 535	0.12 (1)
I- E	-186 / 38	0.10 (1)
C- I	-186 / 38	0.10 (1)
J- C	-967 / 0	0.50 (1)
E- H	-967 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.49/1.00 (I-J:2) , WB=0.50/1.00 (E-H:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2268
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 026 / 18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295156	G100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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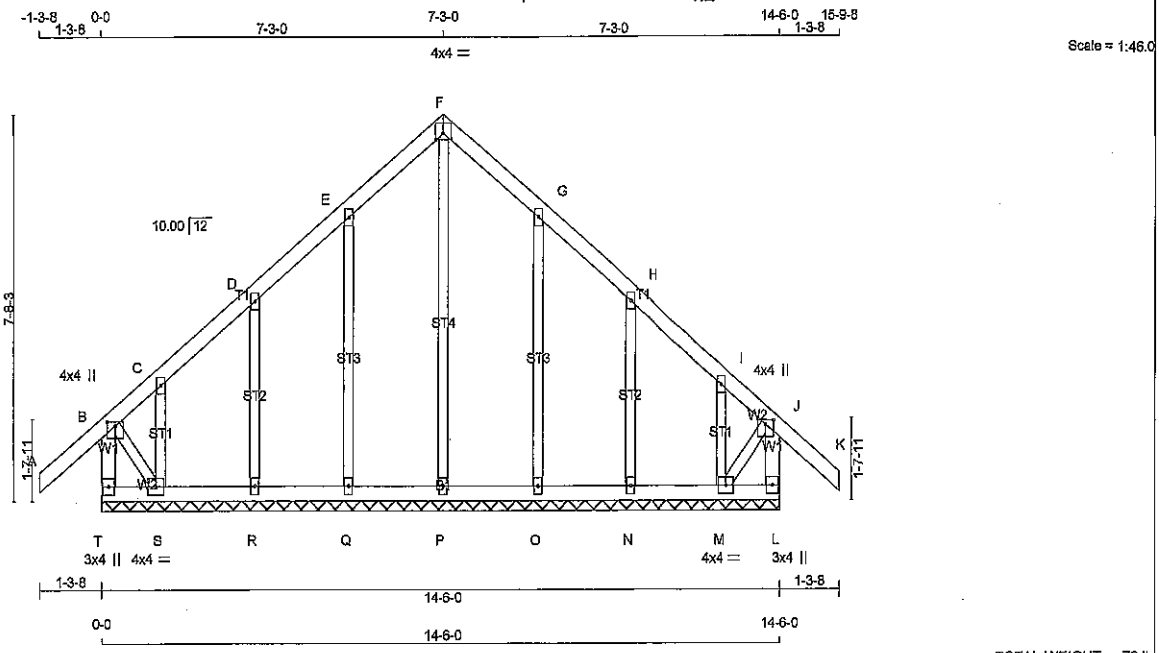

TOTAL WEIGHT = 72 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-325 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-78 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-17 / 0	-104.9	-104.9	0.05 (1)	6.25
D- E	-17 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-27 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-27 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-17 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-17 / 0	-104.9	-104.9	0.05 (1)	6.25
I- J	-78 / 0	-104.9	-104.9	0.14 (1)	6.25
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- J	-325 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 20	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 14	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 11	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 11	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 14	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 20	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-160 / 0	0.17 (1)
Q- E	-236 / 0	0.13 (1)
R- D	-215 / 0	0.06 (1)
S- C	-74 / 0	0.01 (1)
O- G	-236 / 0	0.13 (1)
N- H	-215 / 0	0.06 (1)
M- I	-74 / 0	0.01 (1)
B- S	0 / 33	0.01 (1)
M- J	0 / 33	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (J-K:1) , BC=0.02/1.00 (M-N:2) , WB=0.17/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (F) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

DWG NO. TAM 10205-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295159	T100S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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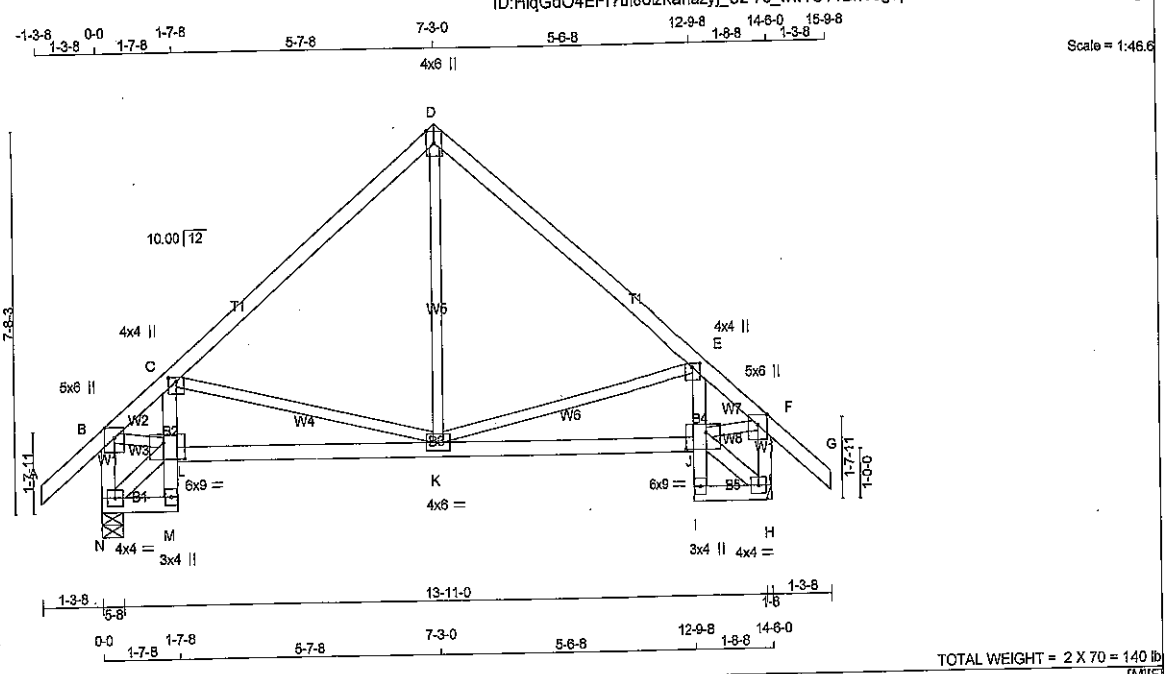

TOTAL WEIGHT = 2 X 70 = 140 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	6.0	9.0	4.00	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1109	0	1109	0	0	5-8	5-8
H	1109	0	1109	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	866	561 / 0	152 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	866	561 / 0	152 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- K	-562 / 0	0.35 (1)
B- C	-1295 / 0	-104.9	-104.9	0.35 (1)	5.25	K- D	0 / 466	0.10 (2)
C- D	-754 / 0	-104.9	-104.9	0.42 (1)	6.25	K- E	-534 / 0	0.32 (1)
D- E	-753 / 0	-104.9	-104.9	0.41 (1)	6.25	N- L	-44 / 0	0.00 (1)
E- F	-1270 / 0	-104.9	-104.9	0.34 (1)	5.30	B- L	0 / 1070	0.24 (1)
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- H	-41 / 0	0.00 (1)
N- B	-1061 / 0	0.0	0.0	0.11 (1)	7.64	J- F	0 / 1044	0.23 (1)
H- F	-1063 / 0	0.0	0.0	0.11 (1)	7.64			
N- M	0 / 34	-28.0	-28.0	0.02 (2)	10.00			
M- L	0 / 36	0.0	0.0	0.08 (1)	10.00			
L- C	-7 / 110	0.0	0.0	0.05 (2)	10.00			
L- K	0 / 1095	-28.0	-28.0	0.38 (2)	10.00			
K- J	0 / 1066	-28.0	-28.0	0.38 (2)	10.00			
I- J	0 / 38	0.0	0.0	0.08 (1)	10.00			
J- E	-12 / 107	0.0	0.0	0.05 (2)	7.81			
I- H	0 / 33	-28.0	-28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.38/1.00 (K-L:2) , WB=0.35/1.00 (C-K:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.79 (B) (INPUT = 0.90)
SSI METAL= 0.24 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 102078
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T200	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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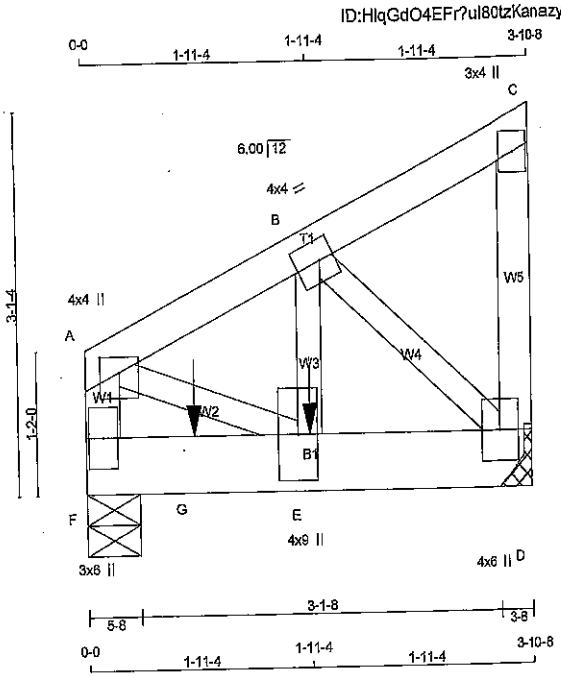

Scale = 1:18.7

TOTAL WEIGHT = 2 X 19 = 39 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(213.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMWW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 294.8 lbs FACTORED DOWN AT 11-4, AND 1753.1 lbs FACTORED DOWN AT 1-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1358	0	1358	0	0	5-8	5-8
D	1205	0	1205	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1027	723 / 0	143 / 0	0 / 0	0 / 0	161 / 0	0 / 0
D	924	629 / 0	143 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1083 / 0	0.0	0.0	0.06 (1)	7.81	E- B	0 / 1320	0.16 (1)
A- B	-1225 / 0	-104.9	-104.9	0.04 (1)	6.25	A- E	0 / 1183	0.15 (1)
B- C	-3 / 2	-104.9	-104.9	0.03 (1)	10.00	B- D	-1477 / 0	0.13 (1)
D- C	-95 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	0.09 (1)	10.00			
G- E	0 / 0	-28.0	-28.0	0.09 (1)	10.00			
E- D	0 / 1098	-28.0	-28.0	0.14 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	1-11-4	-1753	-1753	---	BACK	VERT	TOTAL
G	11-4	-295	-295	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-F:1) , BC=0.14/1.00 (D-E:1) , WB=0.16/1.00 (B-E:1) , SSI=0.09/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10219-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
295157	T201	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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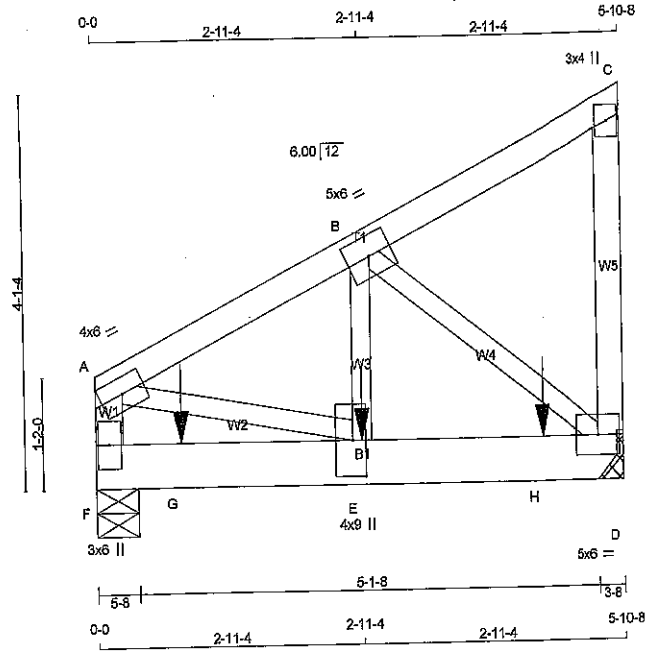

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(413.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	9.0	4.50	1.75
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 93.2 lbs FACTORED DOWN AT 11-4, AND 2521.2 lbs FACTORED DOWN AT 2-11-4, AND 1363.1 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1947	0	1947	0	0	5-8	5-8
D	2812	0	2812	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1512	994 / 0	256 / 0	0 / 0	0 / 0	263 / 0	0 / 0
D	2201	1417 / 0	392 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1844 / 0	0.0	0.0	0.10 (1)	7.81	A- E	0 / 2317	0.29 (1)
A- B	-2495 / 0	-104.9	-104.9	0.09 (1)	5.65	E- B	0 / 2481	0.31 (1)
B- C	-9 / 2	-104.9	-104.9	0.06 (1)	10.00	B- D	-2819 / 0	0.34 (1)
D- C	-134 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	0.03 (1)	10.00			
G- E	0 / 0	-28.0	-28.0	0.03 (1)	10.00			
E- H	0 / 2240	-28.0	-28.0	0.47 (1)	10.00			
H- D	0 / 2240	-28.0	-28.0	0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-11-4	-2521	-2521	---	BACK	VERT	TOTAL
G	11-4	-93	-93	---	BACK	VERT	TOTAL
H	4-11-4	-1363	-1363	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.10/1.00 (A-F:1) , BC=0.47/1.00 (D-E:1) , WB=0.34/1.00 (B-D:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.47 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM/022018
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287391	J1S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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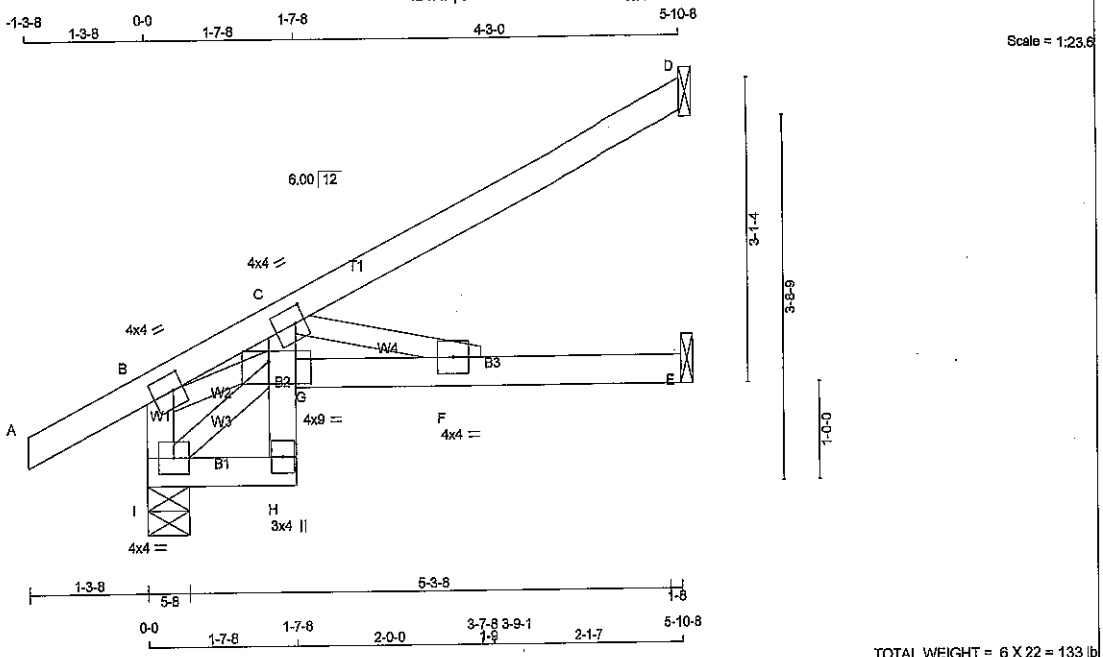

TOTAL WEIGHT = 6 X 22 = 133 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-t	MT20	4.0	9.0	2.75	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	549	0	549	0	0	5-8	5-8
D	225	0	225	0	0	1-8	1-8
E	149	0	155	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	419	289 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	158	133 / 0	8 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	142	48 / 0	54 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-478 / 0	0.0	0.0	0.05 (1)	7.61	C- F	-621 / 0	0.13 (1)
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00	I- G	-54 / 0	0.01 (1)
B- C	-726 / 0	-104.9	-104.9	0.14 (1)	6.25	B- G	0 / 638	0.14 (1)
C- D	-2 / 8	-104.9	-104.9	0.33 (1)	10.00			
I- H	0 / 67	-28.0	-28.0	0.02 (2)	10.00			
H- G	0 / 36	0.0	0.0	0.10 (1)	10.00			
G- C	0 / 302	0.0	0.0	0.15 (1)	10.00			
G- F	0 / 791	-28.0	-28.0	0.52 (1)	10.00			
F- E	0 / 0	-28.0	-28.0	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 789 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 482 (0.15")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.52/1.00 (F-G:1) , WB=0.14/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2261B
S. KATSOULAKOS
PROFESSIONAL ENGINEER ONTARIO

DWG NO. TAM 1020218
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287393	J1S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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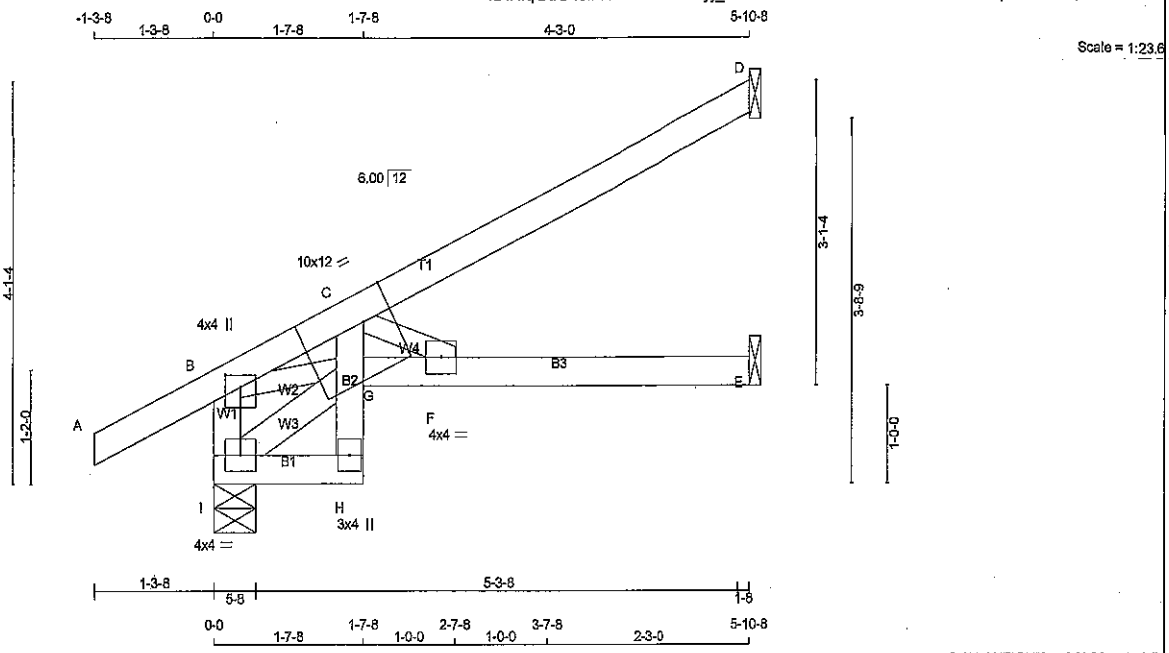


TOTAL WEIGHT = 6 X 22 = 130 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C						
C	TMBVWWW*-t	MT20	10.0	12.0	Edge	3.75
F	BMW-t	MT20	4.0	4.0		
G						
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	549	0	549	0	0	5-8	5-8
D	234	0	234	0	0	1-8	1-8
E	140	0	148	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	419	269 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	165	137 / 0	10 / 0	0 / 0	0 / 0	19 / 0	0 / 0
E	135	44 / 0	52 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-498 / 0	0.0	0.0	0.05 (1)	7.81	I- G	-56 / 0	0.01 (1)
A- B	0 / 32	-104.9	-104.9	0.14 (1)	10.00	B- G	0 / 459	0.11 (1)
B- C	-557 / 0	-104.9	-104.9	0.13 (1)	6.25	C- F	-627 / 0	0.09 (1)
C- D	0 / 10	-104.9	-104.9	0.35 (1)	10.00			
I- H	0 / 47	-28.0	-28.0	0.02 (2)	10.00			
H- G	0 / 36	0.0	0.0	0.06 (1)	10.00			
G- C	0 / 327	0.0	0.0	0.11 (1)	10.00			
G- F	0 / 523	-28.0	-28.0	0.51 (1)	10.00			
F- E	0 / 0	-28.0	-28.0	0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 675 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 418 (0.17")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.51/1.00 (F-G:1) , WB=0.11/1.00 (B-G:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

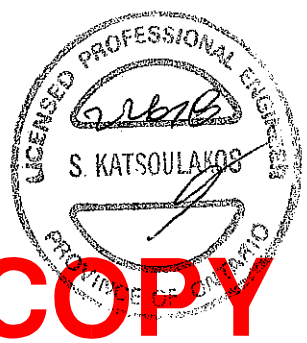
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)



DWG NO. TAM 10244-18
STRUCTURAL COMPONENT ONLY

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

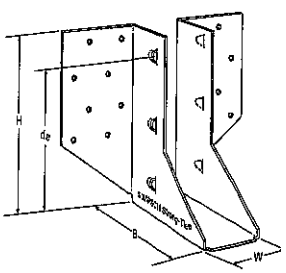
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

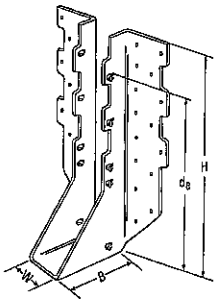
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

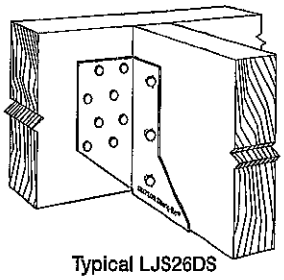
- See current catalogue for options



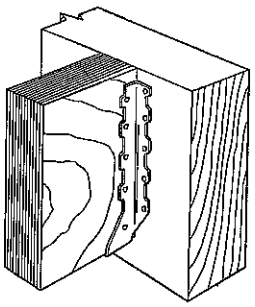
LJS26DS



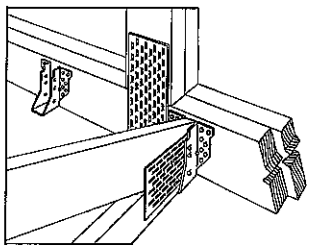
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



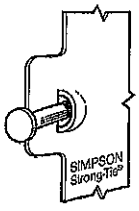
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

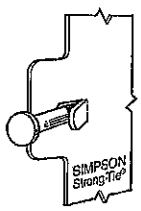
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

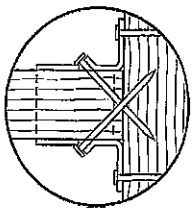


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

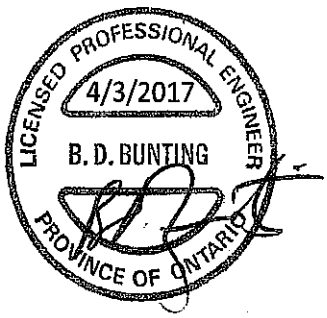
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

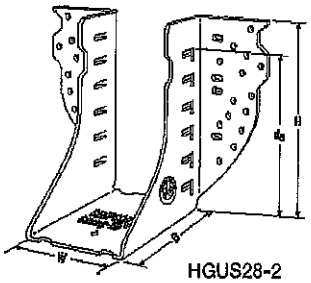
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

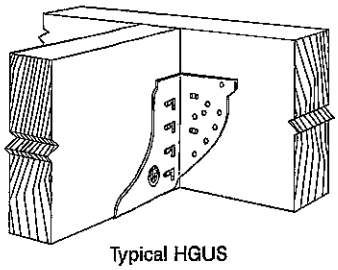
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

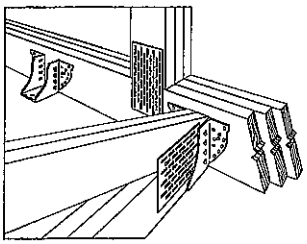
Options:

- See current catalogue for options



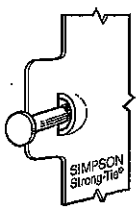
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

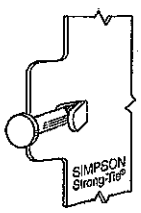
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

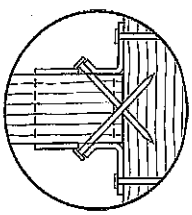


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

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T-SPECHGUS17 3/17 exp. 6/19

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SITE COPY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

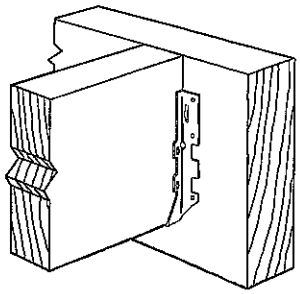
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

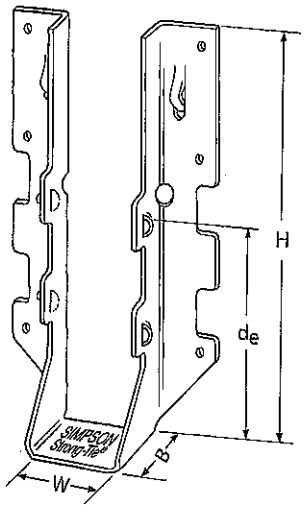
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



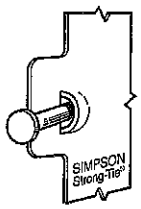
Typical LUS Installation



LUS28

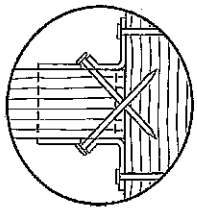
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LIMIT STATES DESIGN

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strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

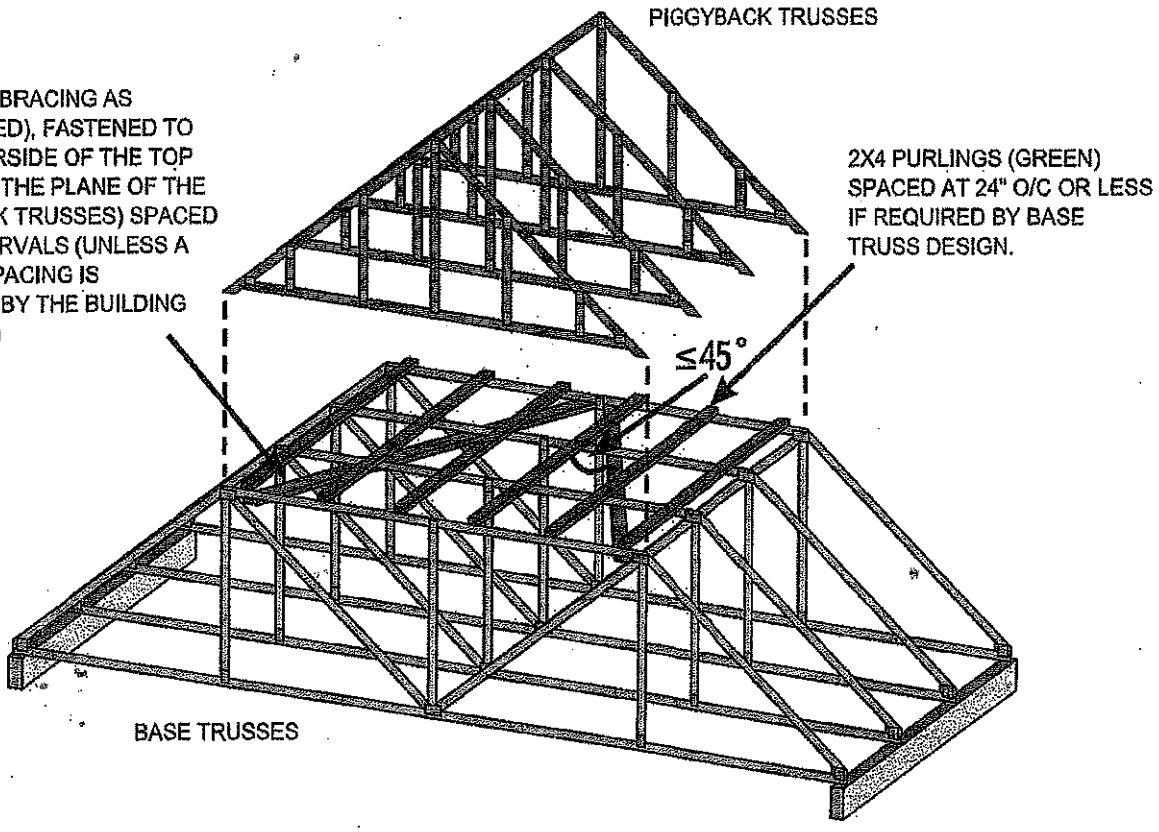
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

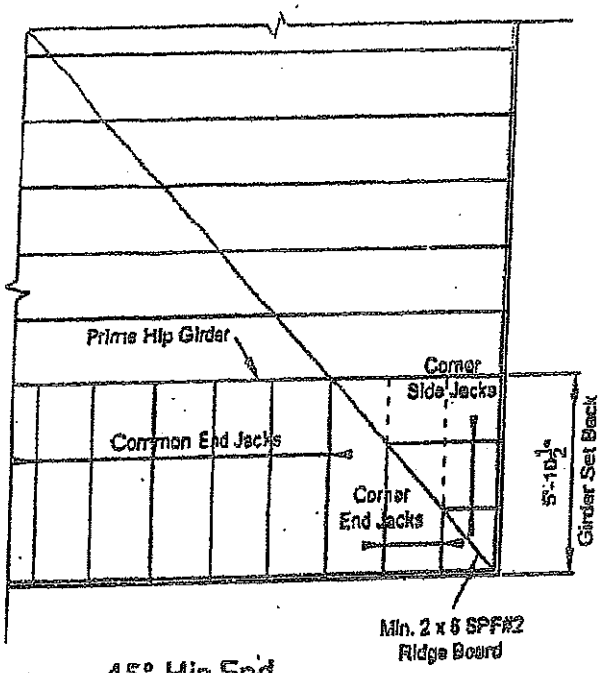
Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

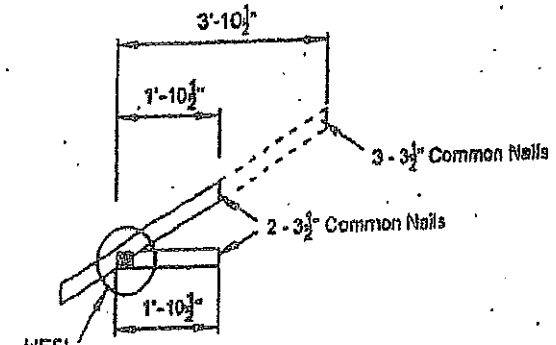
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

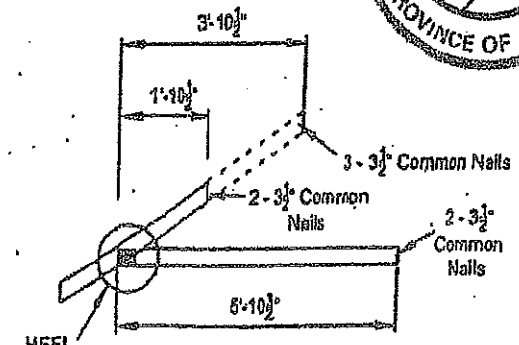
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495 14 R1
STRUCTURAL COMPONENT ONLY

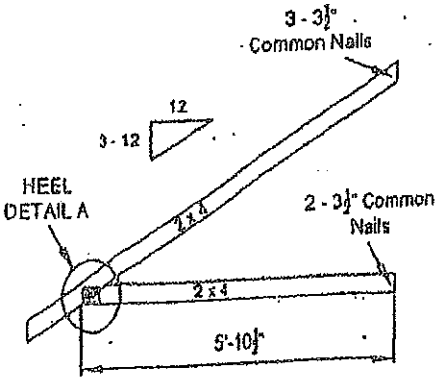
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



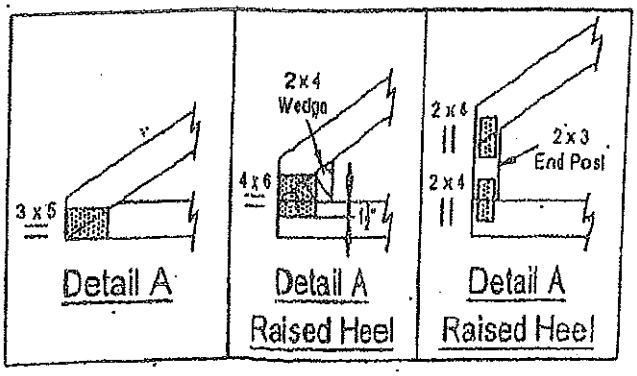
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

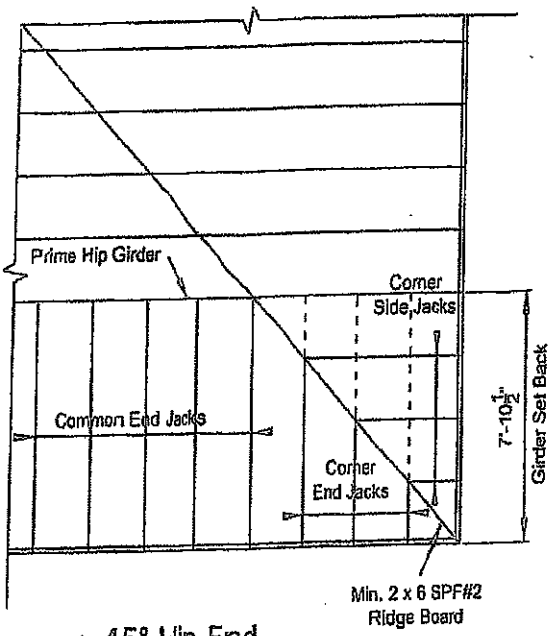
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 350314
STRUCTURAL COMPONENT ONLY

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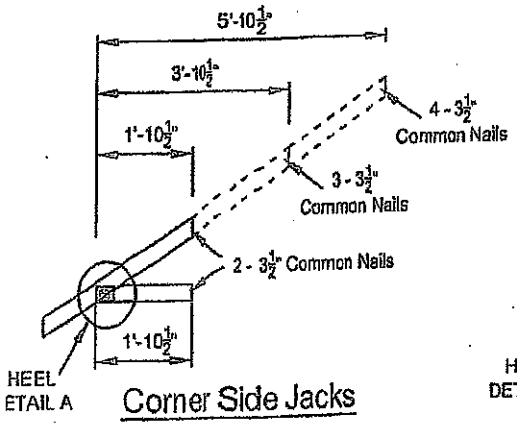
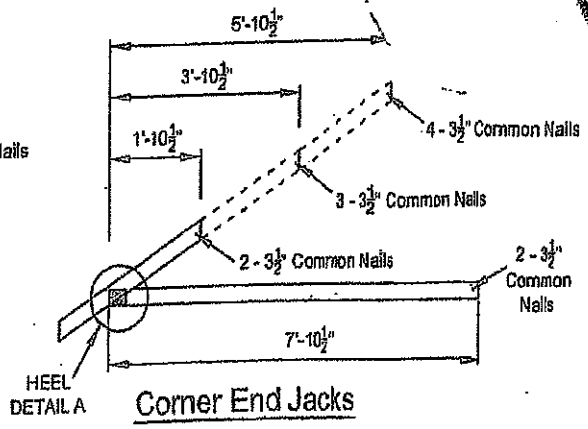
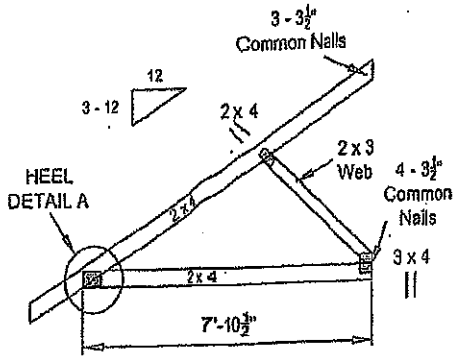
S. KATSOULAKOS
PROVINCE OF ONTARIO



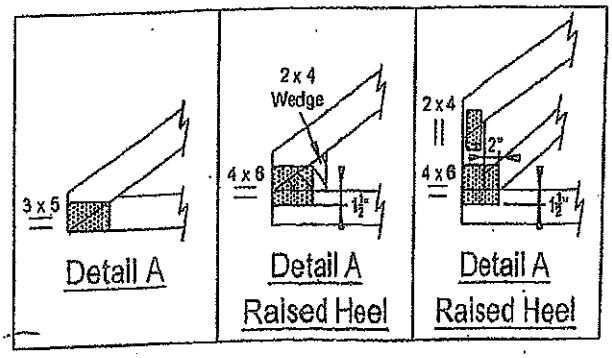
Corner Side Jacks



Corner End Jacks



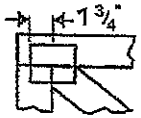
Common End Jacks



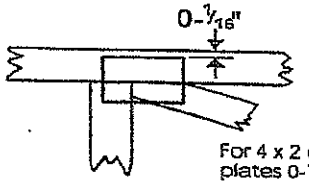
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

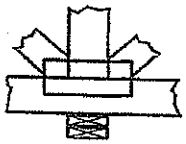
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

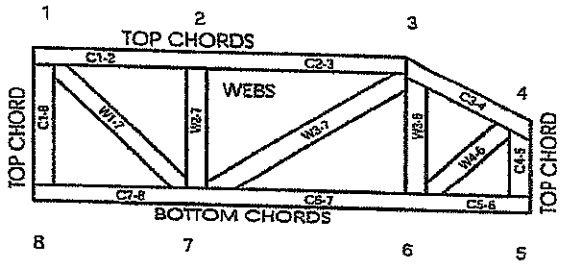
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12681-R

MiTek®

POWER TO PERFORM.™

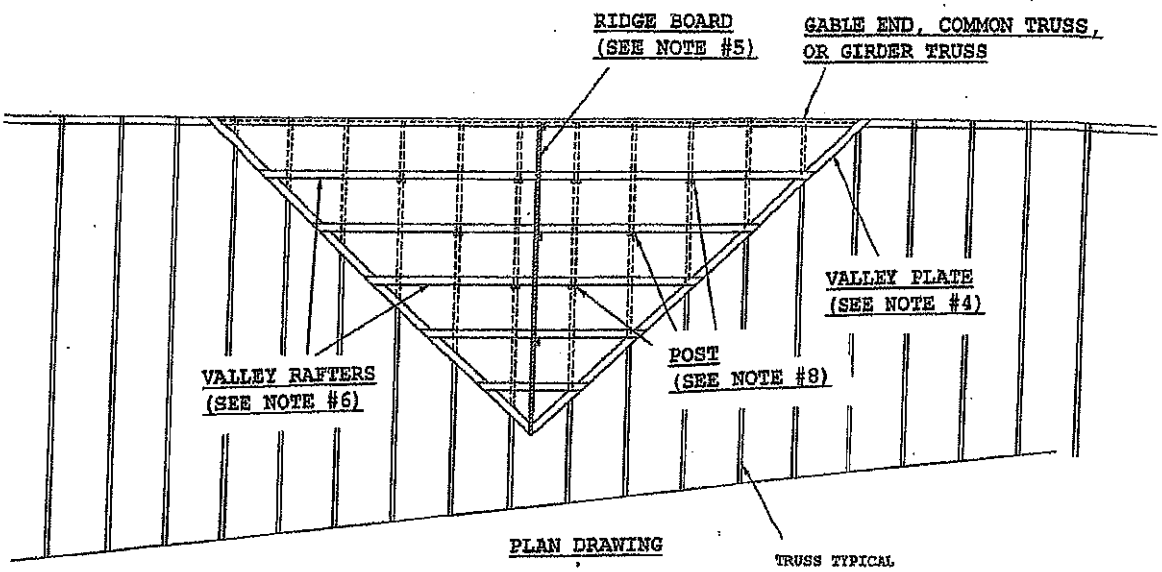
General Safety Notes

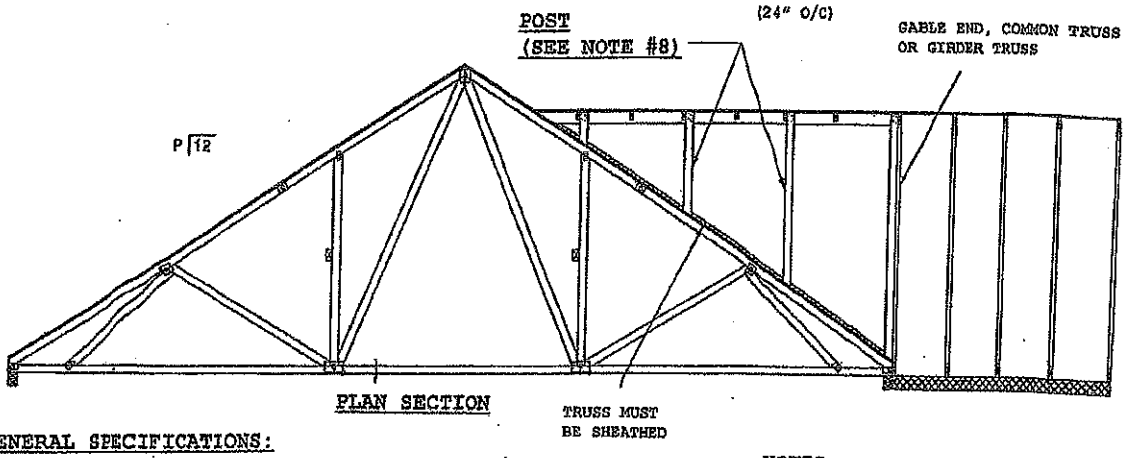
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



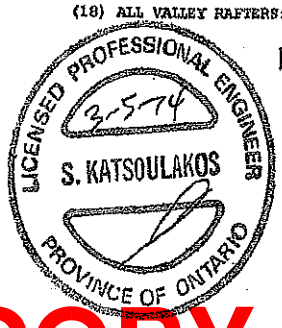


GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

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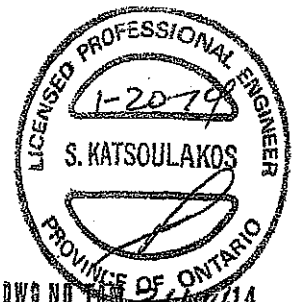

DWG NO [illegible] 7/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.