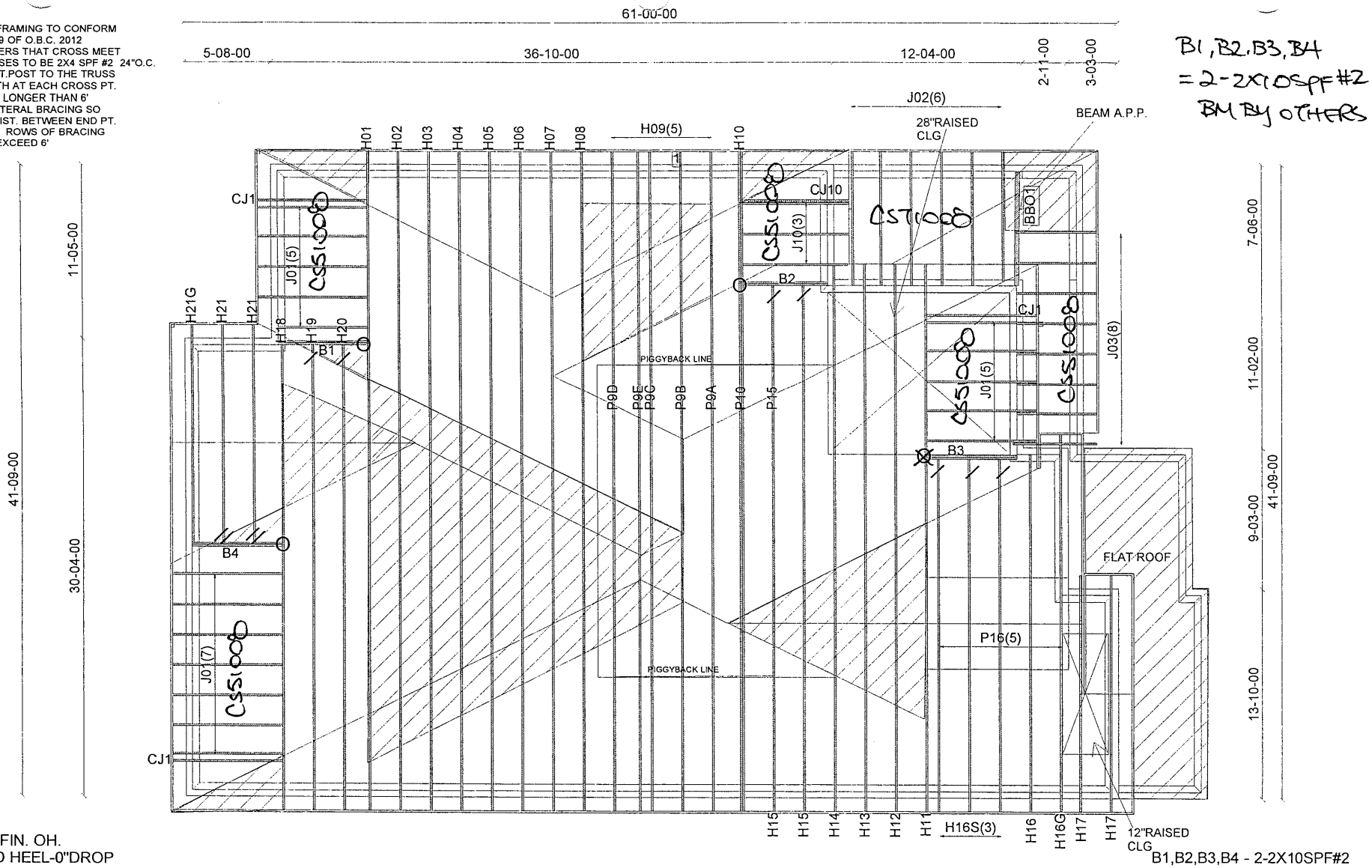
SITE COPY

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

B1, B2, B3, B4 = 2-2X10SPF#2 BM BY OTHERS



12" FIN. OH.
RSD HEEL-0"DROP
(2"X6" FASCIA)-FM
ASPHALT SHINGLES
2"X6"BRG./BRK.

B1,B2,B3,B4 - 2-2X10SPF#2
/ LJS26DS
O HHUS26-2
// LUS24
⊗ HUC26-2

CONVENTIONAL FRAMING BY OTHERS

A.18012412 - A.18012443

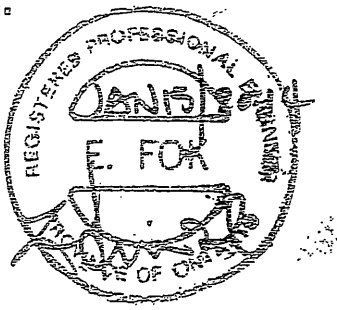
PL#: 96227

Mitek Ver:7.5.0

Job Track: 38514	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S50-2 / A
Layout ID: 264490	Project: GREEN VALLEY ESTATES		
Plan Log: 84728	Date: 2/9/2016	Designer: jerzy	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H18	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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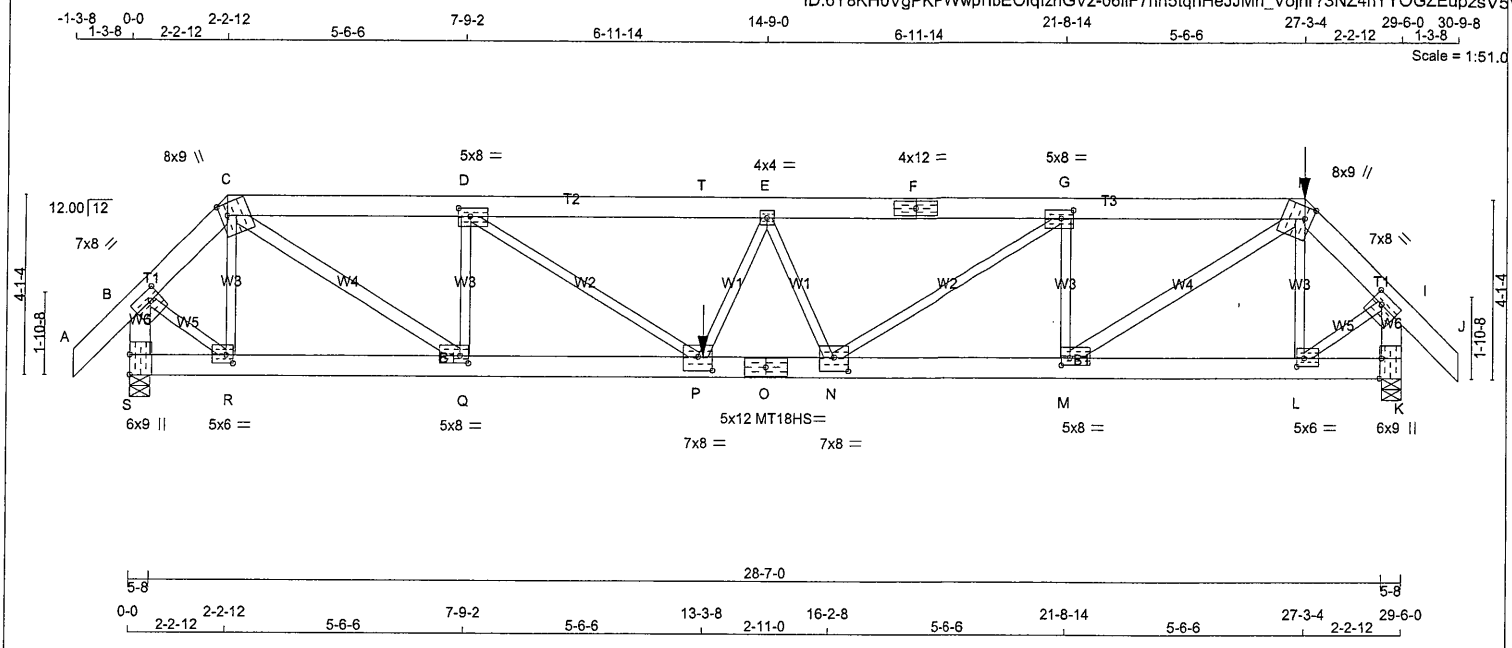

TOTAL WEIGHT = 165 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - H	2x6	DRY	1650F 1.5E	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - O	2x6	DRY	1650F 1.5E	SPF
O - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - Q	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.75	3.00
C	TTWW+m	MT20	8.0	9.0	3.25	2.00
D	TMWW-t	MT20	5.0	8.0	2.25	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	4.0	12.0		
G	TMWW-t	MT20	5.0	8.0	2.25	3.25
H	TTWW+m	MT20	8.0	9.0	3.25	2.00
I	TMVW-t	MT20	7.0	8.0	2.75	3.00
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	5.0	6.0	2.25	2.00
M	BMWW-t	MT20	5.0	8.0	2.00	2.25
N	BMWW-t	MT20	7.0	8.0	3.75	4.00
O	BS-t	MT18HS	5.0	12.0		
P	BMWW-t	MT20	7.0	8.0	3.75	4.00
Q	BMWW-t	MT20	5.0	8.0	2.00	2.25
R	BMWW-t	MT20	5.0	6.0	2.25	2.00
S	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3272	0	3272	0	0	5-8	5-2
K	3934	0	3934	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2270	1736 / 0	0 / 0	0 / 0	0 / 0	535 / 0	0 / 0
K	2732	2078 / 0	0 / 0	0 / 0	0 / 0	653 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 54	-104.9	-104.9	0.09 (1)	10.00	R- C	-947 / 0	0.23 (1)
B- C	-2723 / 0	-104.9	-104.9	0.09 (1)	4.97	C- Q	0 / 4567	0.81 (1)
C- D	-5705 / 0	-104.9	-104.9	0.26 (1)	4.01	Q- D	-2369 / 0	0.57 (1)
D- T	-8273 / 0	-104.9	-104.9	0.54 (1)	3.19	D- P	0 / 3093	0.77 (1)
T- E	-8273 / 0	-206.6	-206.6	0.54 (1)	3.19	P- E	-352 / 2	0.10 (1)
E- F	-8026 / 0	-206.6	-206.6	0.66 (1)	3.07	E- N	-978 / 0	0.27 (1)
F- G	-8026 / 0	-206.6	-206.6	0.66 (1)	3.07	N- G	0 / 1958	0.48 (1)
G- H	-6400 / 0	-206.6	-206.6	0.53 (1)	3.49	M- G	-2500 / 0	0.60 (1)
H- I	-3321 / 0	-104.9	-104.9	0.10 (1)	4.57	M- H	0 / 4902	0.87 (1)
I- J	0 / 54	-104.9	-104.9	0.09 (1)	10.00	L- H	-1078 / 0	0.26 (1)
S- B	-3294 / 0	0.0	0.0	0.24 (1)	5.80	B- R	0 / 2189	0.54 (1)
K- I	-3958 / 0	0.0	0.0	0.29 (1)	5.34	L- I	0 / 2670	0.66 (1)
S- R	0 / 0	-18.5	-18.5	0.05 (1)	10.00			
R- Q	0 / 1878	-18.5	-18.5	0.21 (1)	10.00			
Q- P	0 / 5705	-18.5	-18.5	0.56 (1)	10.00			
P- O	0 / 8411	-36.4	-36.4	0.78 (1)	10.00			
O- N	0 / 8411	-36.4	-36.4	0.78 (1)	10.00			
N- M	0 / 6401	-36.4	-36.4	0.61 (1)	10.00			
M- L	0 / 2292	-36.4	-36.4	0.27 (1)	10.00			
L- K	0 / 0	-36.4	-36.4	0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	27-3-4	-171	-171	---	FRONT	VERT	TOTAL
P	13-3-8	-1376	-1376	---	FRONT	VERT	TOTAL

A18012412

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-2-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 664 (0.53")

CSI: TC=0.66/1.00 (E-G:1) , BC=0.78/1.00 (N-P:1) , WB=0.87/1.00 (H-M:1) , SSI=0.50/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H18	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 170.6 lbs FACTORED DOWN AT 27-3-4 ON TOP CHORD, AND 1376.0 lbs FACTORED DOWN AT 13-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

MT18HS 580 354 2729 1405 4191 2010

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.77 (M) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LICENSED PROFESSIONAL ENGINEER
D. O'CONNOR
100009580
01/23/2018
PROVINCE OF ONTARIO

A18012412(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H19	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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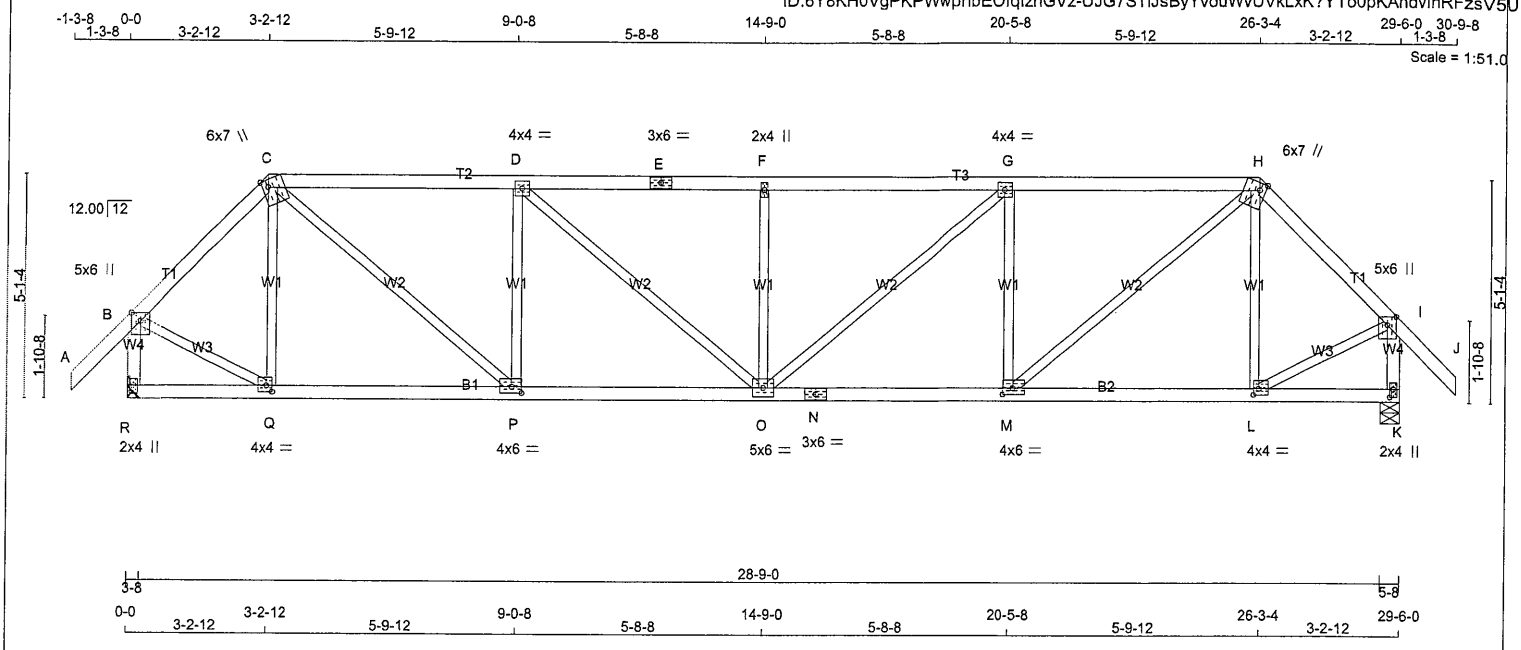

TOTAL WEIGHT = 125 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.50
C	TTWW+m	MT20	6.0	7.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	2.25	2.50
K	BMV1+p	MT20	2.0	4.0	2.25	1.00
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BMWW-t	MT20	4.0	6.0	1.75	2.75
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	6.0	1.75	2.75
Q	BMWW-t	MT20	4.0	4.0	1.75	1.50
R	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1967	0	1967	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-14	
K	1967	0	1967	0	0	5-8	2-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1364	1049 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
K	1364	1049 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	Q- C	-411 / 0	0.16 (1)
B- C	-1583 / 0	-104.9	-104.9	0.23 (1)	5.00	C- P	0 / 1748	0.39 (1)
C- D	-2455 / 0	-104.9	-104.9	0.69 (1)	3.64	P- D	-1005 / 0	0.39 (1)
D- E	-2844 / 0	-104.9	-104.9	0.74 (1)	3.36	D- O	0 / 508	0.11 (1)
E- F	-2844 / 0	-104.9	-104.9	0.74 (1)	3.36	O- F	-552 / 0	0.21 (1)
F- G	-2844 / 0	-104.9	-104.9	0.74 (1)	3.36	O- G	0 / 508	0.11 (1)
G- H	-2455 / 0	-104.9	-104.9	0.69 (1)	3.64	M- G	-1005 / 0	0.39 (1)
H- I	-1583 / 0	-104.9	-104.9	0.23 (1)	5.00	M- H	0 / 1748	0.39 (1)
I- J	0 / 52	-104.9	-104.9	0.15 (1)	10.00	L- H	-411 / 0	0.16 (1)
R- B	-1950 / 0	0.0	0.0	0.22 (1)	6.03	B- Q	0 / 1225	0.28 (1)
K- I	-1950 / 0	0.0	0.0	0.22 (1)	6.03	L- I	0 / 1225	0.28 (1)
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
Q- P	0 / 1105	-18.5	-18.5	0.24 (1)	10.00			
P- O	0 / 2455	-18.5	-18.5	0.45 (1)	10.00			
O- N	0 / 2455	-18.5	-18.5	0.45 (1)	10.00			
N- M	0 / 2455	-18.5	-18.5	0.45 (1)	10.00			
M- L	0 / 1105	-18.5	-18.5	0.24 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

A18012413

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.74/1.00 (F-G:1) , BC=0.45/1.00 (M-O:1) , WB=0.39/1.00 (C-P:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.64 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H20	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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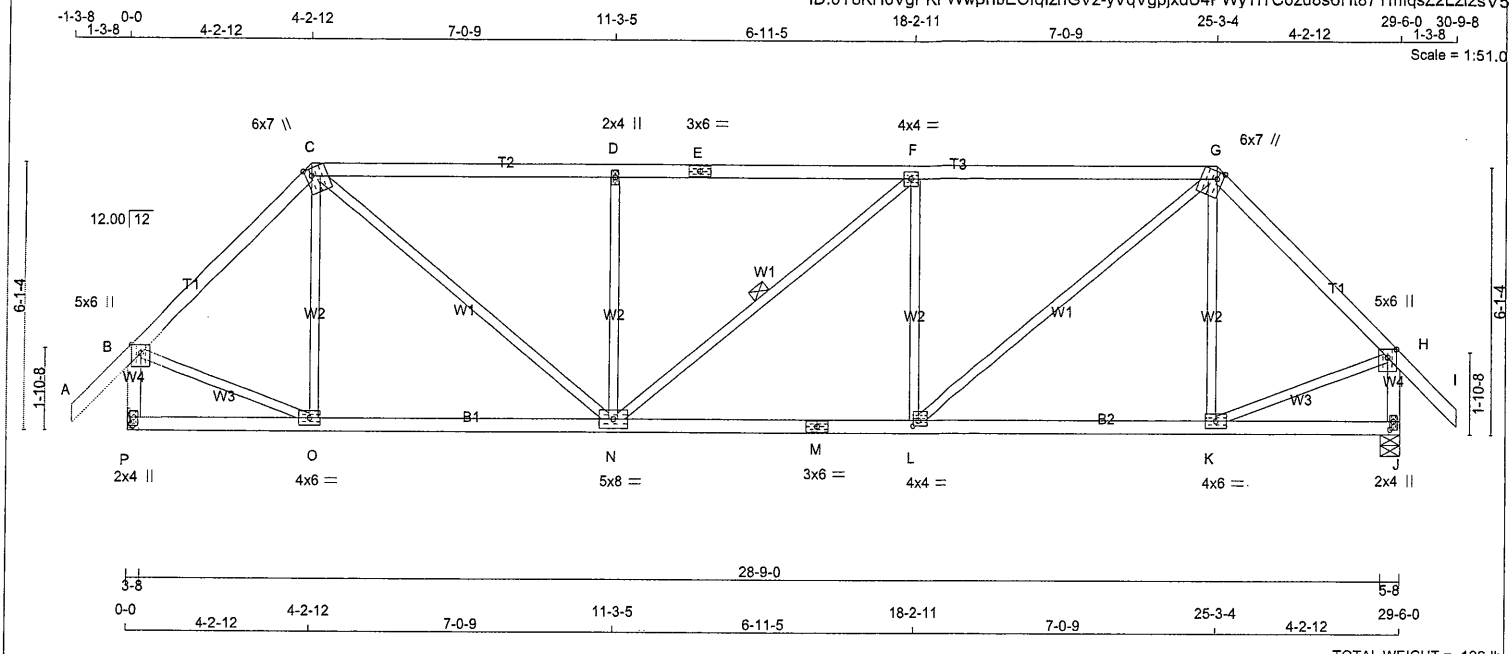

TOTAL WEIGHT = 126 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.50
C	TTWW+m	MT20	6.0	7.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	2.25	2.50
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1967	0	1967	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 2-14	
J	1967	0	1967	0	0	5-8	2-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1364	1049 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
J	1364	1049 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	O- C	-288 / 32	0.17 (1)
B- C	-1644 / 0	-104.9	-104.9	0.41 (1)	4.73	C- N	0 / 1456	0.33 (1)
C- D	-2278 / 0	-104.9	-104.9	0.99 (1)	3.28	N- D	-795 / 0	0.47 (1)
D- E	-2279 / 0	-104.9	-104.9	0.98 (1)	3.26	N- F	-2 / 0	0.00 (1)
E- F	-2279 / 0	-104.9	-104.9	0.98 (1)	3.26	L- F	-795 / 0	0.47 (1)
F- G	-2280 / 0	-104.9	-104.9	1.00 (1)	3.26	L- G	0 / 1458	0.33 (1)
G- H	-1643 / 0	-104.9	-104.9	0.41 (1)	4.73	K- G	-289 / 32	0.17 (1)
H- I	0 / 52	-104.9	-104.9	0.15 (1)	10.00	B- O	0 / 1228	0.28 (1)
P- B	-1942 / 0	0.0	0.0	0.22 (1)	6.04	K- H	0 / 1227	0.28 (1)
J- H	-1941 / 0	0.0	0.0	0.22 (1)	6.04			
P- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
O- N	0 / 1153	-18.5	-18.5	0.27 (1)	10.00			
N- M	0 / 2280	-18.5	-18.5	0.46 (1)	10.00			
M- L	0 / 2280	-18.5	-18.5	0.46 (1)	10.00			
L- K	0 / 1153	-18.5	-18.5	0.28 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=1.00/1.00 (F-G:1) , BC=0.46/1.00 (L-N:1) , WB=0.47/1.00 (F-L:1) , SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

A18012414

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H11	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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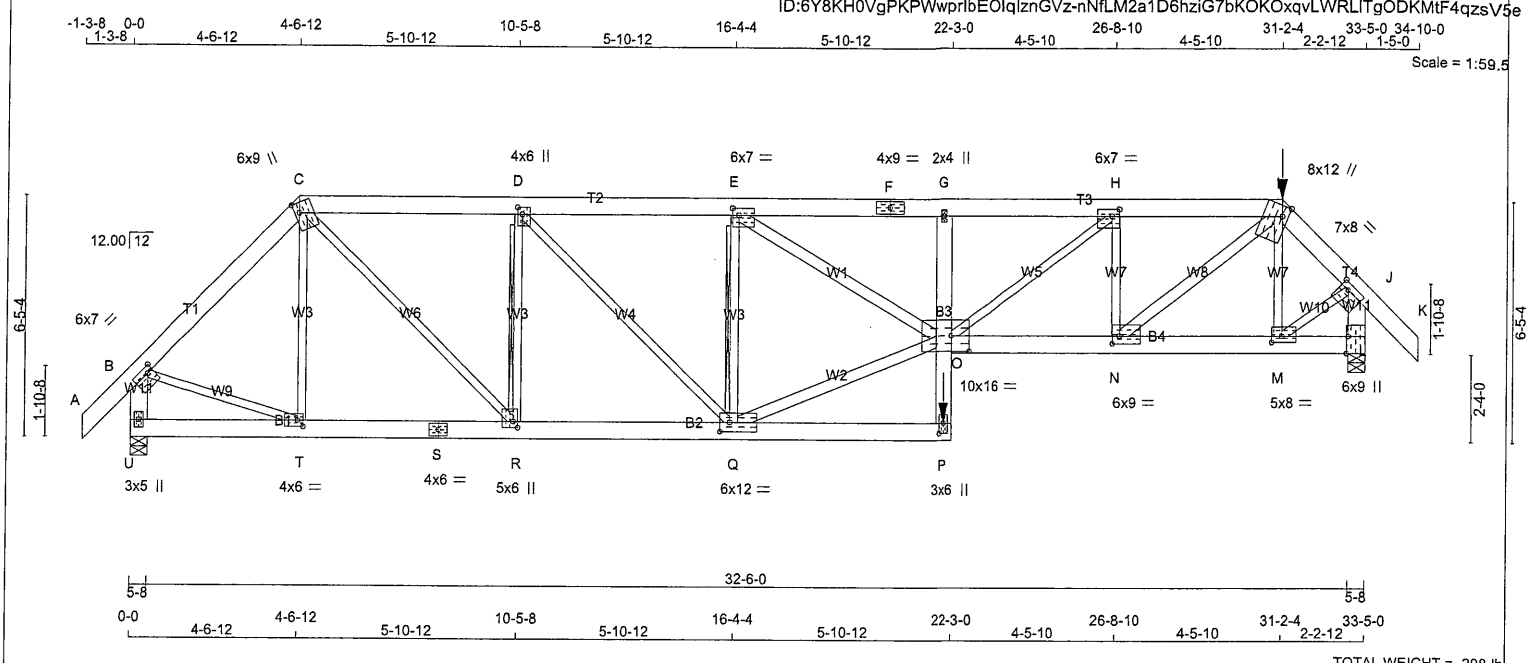

TOTAL WEIGHT = 208 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - I	2x6	DRY	1650F 1.5E	SPF
I - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	1650F 1.5E	SPF
E - O	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	3.25	1.50
D	TMWW+t	MT20	4.0	6.0	2.25	1.50
E	TMWW-t	MT20	6.0	7.0	2.25	2.00
F	TS-t	MT20	4.0	9.0		
G	TMV+p	MT20	2.0	4.0		
H	TMWW-t	MT20	6.0	7.0	2.25	2.25
I	TTWW+m	MT20	8.0	12.0	Edge	2.00
J	TMVW-t	MT20	7.0	8.0	2.00	2.75
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW-t	MT20	5.0	8.0	2.00	3.25
N	BMWW-t	MT20	6.0	9.0	2.50	2.25
O	BVMWWW-l	MT20	10.0	16.0	5.00	5.75
P	BMV+p	MT20	3.0	6.0	3.25	1.50
Q	BMWWW-t	MT20	6.0	12.0	3.00	3.25
R	BMWW+t	MT20	5.0	6.0	2.00	1.50
S	BS-t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	6.0	2.00	1.75
U	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3191	0	3191	0	0	5-8	4-14
L	4728	0	4728	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2214	1694 / 0	0 / 0	0 / 0	0 / 0	520 / 0	0 / 0
L	3283	2498 / 0	0 / 0	0 / 0	0 / 0	785 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L
BEARING SIZE FACTOR = 1.15 AT JNT(S) L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-R, E-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 54	-104.9	-104.9	0.09 (1)	10.00
B- C	-3047 / 0	-104.9	-104.9	0.27 (1)	4.56
C- D	-4245 / 0	-104.9	-104.9	0.24 (1)	4.53
D- E	-5595 / 0	-104.9	-104.9	0.29 (1)	4.02
E- F	-9985 / 0	-104.9	-104.9	0.48 (1)	2.97
F- G	-9985 / 0	-104.9	-104.9	0.48 (1)	2.97
G- H	-10138 / 0	-206.6	-206.6	0.52 (1)	2.88
H- I	-7128 / 0	-206.6	-206.6	0.36 (1)	3.54
I- J	-4016 / 0	-104.9	-104.9	0.13 (1)	4.19
J- K	0 / 59	-104.9	-104.9	0.10 (1)	10.00
U- B	-3165 / 0	0.0	0.0	0.23 (1)	5.90
L- J	-4743 / 0	0.0	0.0	0.35 (1)	4.90
U- T	0 / 0	-18.5	-18.5	0.07 (4)	10.00
T- S	0 / 2135	-18.5	-18.5	0.34 (1)	10.00
S- R	0 / 2135	-18.5	-18.5	0.34 (1)	10.00
R- Q	0 / 4245	-18.5	-18.5	0.60 (1)	10.00
Q- P	0 / 240	-18.5	-18.5	0.08 (4)	10.00
P- O	0 / 2217	0.0	0.0	0.70 (1)	10.00
O- G	-647 / 0	0.0	0.0	0.43 (1)	7.81
O- N	0 / 7129	-36.4	-36.4	0.64 (1)	10.00
N- M	0 / 2770	-36.4	-36.4	0.29 (1)	10.00
M- L	0 / 0	-36.4	-36.4	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-481 / 0	0.29 (1)
C- R	0 / 2993	0.74 (1)
R- D	-2030 / 0	0.57 (1)
D- Q	0 / 1922	0.48 (1)
Q- E	-3459 / 0	0.97 (1)
Q- O	0 / 5791	0.74 (1)
E- O	0 / 5221	0.92 (1)
O- H	0 / 3809	0.94 (1)
N- H	-3388 / 0	0.81 (1)
N- I	0 / 5597	0.99 (1)
M- I	-1375 / 0	0.33 (1)
B- T	0 / 2228	0.55 (1)
M- J	0 / 3229	0.80 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	31-2-4	-171	-171	---	FRONT	VERT	TOTAL
P	22-0-4	-2169	-2169	---	FRONT	VERT	TOTAL

A18012415

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-6-12
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 11-2-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 611 (0.66")

CSI: TC=0.52/1.00 (G-H:1) , BC=0.70/1.00 (O-P:1) , WB=0.99/1.00 (I-N:1) , SSI=0.38/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H11	1	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 170.6 lbs FACTORED DOWN AT 31-2-4 ON TOP CHORD, AND 2169.1 lbs FACTORED DOWN AT 22-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.94 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A18012415(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H12	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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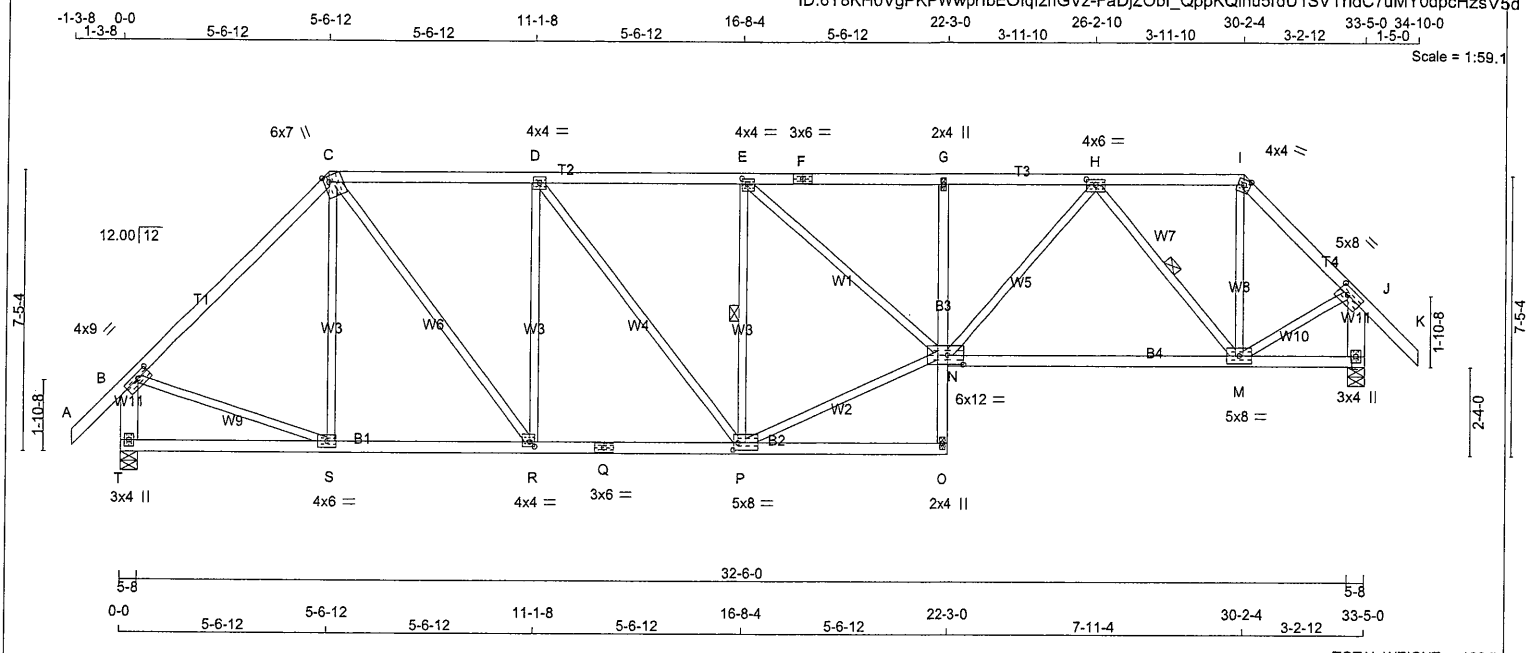

TOTAL WEIGHT = 160 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.50	4.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	1.75	1.75
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0	1.75	3.00
I	TTW-m	MT20	4.0	4.0	1.75	2.00
J	TMVW-t	MT20	5.0	8.0	2.25	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BVMWWW-t	MT20	6.0	12.0	3.00	5.00
O	BMV+p	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.25	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	1.50	1.50
S	BMWW-t	MT20	4.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2209	0	2209	0	0	5-8	2-6
L	2222	0	2222	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1532	1176 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0
L	1541	1184 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.12 (1)	10.00	S- C	-246 / 44	0.24 (1)
B- C	-1924 / 0	-104.9	-104.9	0.61 (1)	4.74	C- R	0 / 1363	0.31 (1)
C- D	-2192 / 0	-104.9	-104.9	0.60 (1)	3.93	R- D	-981 / 0	0.97 (1)
D- E	-2454 / 0	-104.9	-104.9	0.63 (1)	3.72	D- P	0 / 425	0.10 (1)
E- F	-3232 / 0	-104.9	-104.9	0.60 (1)	3.39	P- E	-1267 / 0	0.44 (1)
F- G	-3232 / 0	-104.9	-104.9	0.60 (1)	3.39	P- N	0 / 2645	0.60 (1)
G- H	-3244 / 0	-104.9	-104.9	0.42 (1)	3.54	E- N	0 / 1041	0.23 (1)
H- I	-1311 / 0	-104.9	-104.9	0.24 (1)	5.36	N- H	0 / 1249	0.28 (1)
I- J	-1834 / 0	-104.9	-104.9	0.24 (1)	4.71	H- M	-1789 / 0	0.49 (1)
J- K	0 / 56	-104.9	-104.9	0.17 (1)	10.00	M- I	0 / 954	0.21 (1)
T- B	-2167 / 0	0.0	0.0	0.16 (1)	6.92	B- S	0 / 1405	0.32 (1)
L- J	-2222 / 0	0.0	0.0	0.16 (1)	6.85	M- J	0 / 1419	0.32 (1)
T- S	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
S- R	0 / 1353	-18.5	-18.5	0.30 (1)	10.00			
R- Q	0 / 2193	-18.5	-18.5	0.42 (1)	10.00			
Q- P	0 / 2193	-18.5	-18.5	0.42 (1)	10.00			
P- O	0 / 24	-18.5	-18.5	0.14 (4)	10.00			
O- N	0 / 44	0.0	0.0	0.08 (1)	10.00			
N- G	-493 / 0	0.0	0.0	0.14 (1)	7.81			
N- M	0 / 2432	-18.5	-18.5	0.53 (1)	10.00			
M- L	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.53/1.00 (M-N:1) , WB=0.97/1.00 (D-R:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.60 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H13	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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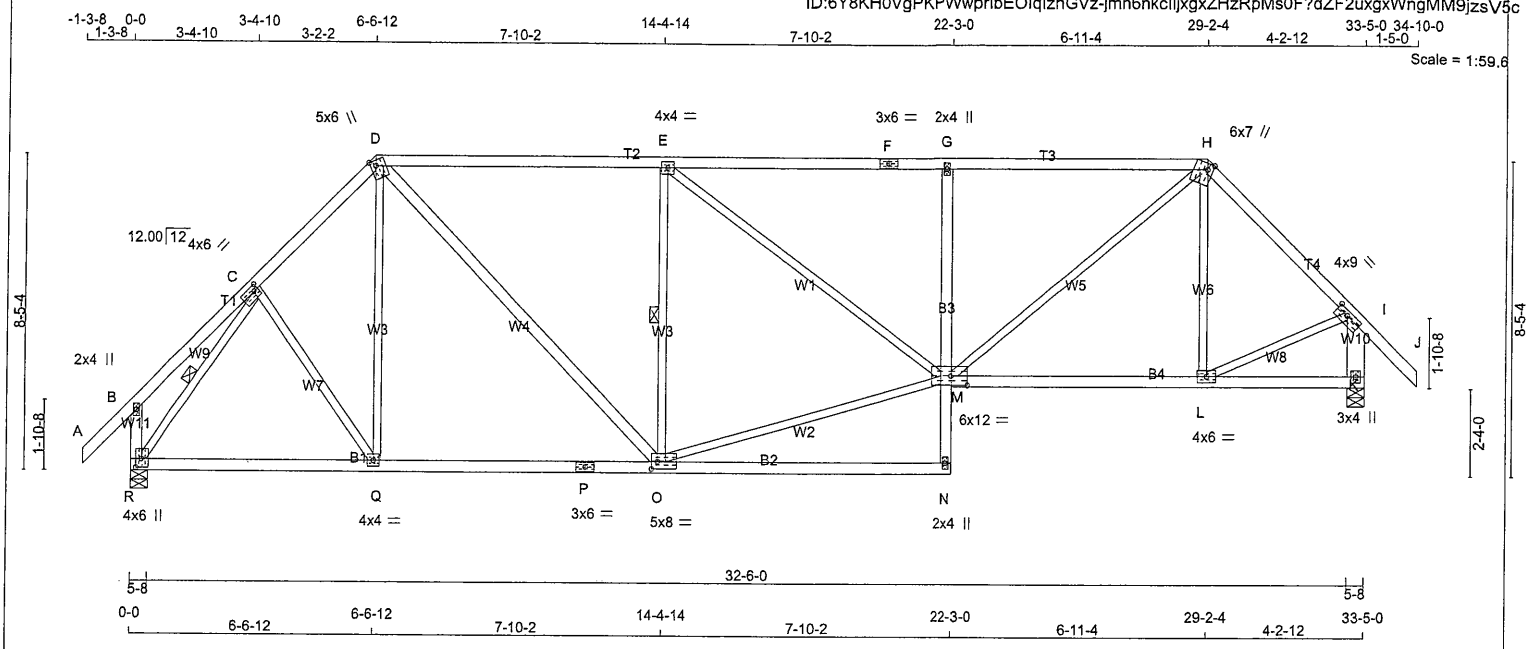

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	1.75	1.50
D	TTWW+m	MT20	5.0	6.0	1.75	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.75
I	TMVW-t	MT20	4.0	9.0	1.50	4.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BVMWWW-I	MT20	6.0	12.0	3.00	5.25
N	BMV+p	MT20	2.0	4.0		
O	BMWWW-t	MT20	5.0	8.0	2.25	2.00
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1+p	MT20	4.0	6.0	2.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	2222	0	2222	0	0	5-8	2-7
R	2209	0	2209	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1541	1184 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0
R	1532	1176 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.16 (1)	10.00
C- D	-1944 / 0	-104.9	-104.9	0.22 (1)	4.63
D- E	-2158 / 0	-104.9	-104.9	0.74 (1)	4.57
E- F	-2737 / 0	-104.9	-104.9	0.80 (1)	4.11
F- G	-2737 / 0	-104.9	-104.9	0.80 (1)	4.11
G- H	-2745 / 0	-104.9	-104.9	0.63 (1)	4.36
H- I	-1901 / 0	-104.9	-104.9	0.43 (1)	4.44
I- J	0 / 56	-104.9	-104.9	0.17 (1)	10.00
R- B	-280 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-2201 / 0	0.0	0.0	0.16 (1)	6.88
R- Q	0 / 1278	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 1351	-18.5	-18.5	0.37 (1)	10.00
P- O	0 / 1351	-18.5	-18.5	0.37 (1)	10.00
O- N	0 / 20	-18.5	-18.5	0.27 (4)	10.00
N- M	0 / 64	0.0	0.0	0.07 (1)	10.00
M- G	-822 / 0	0.0	0.0	0.53 (1)	7.81
M- L	0 / 1334	-18.5	-18.5	0.33 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.20 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 124	0.03 (1)
Q- D	0 / 100	0.03 (4)
D- O	0 / 1170	0.19 (1)
O- E	-1330 / 0	0.60 (1)
O- M	0 / 2249	0.51 (1)
E- M	0 / 704	0.16 (1)
M- H	0 / 1822	0.41 (1)
L- H	-334 / 38	0.20 (1)
R- C	-2273 / 0	0.55 (1)
L- I	0 / 1420	0.32 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.80/1.00 (E-G:1) , BC=0.53/1.00 (G-M:1) , WB=0.60/1.00 (E-O:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H14	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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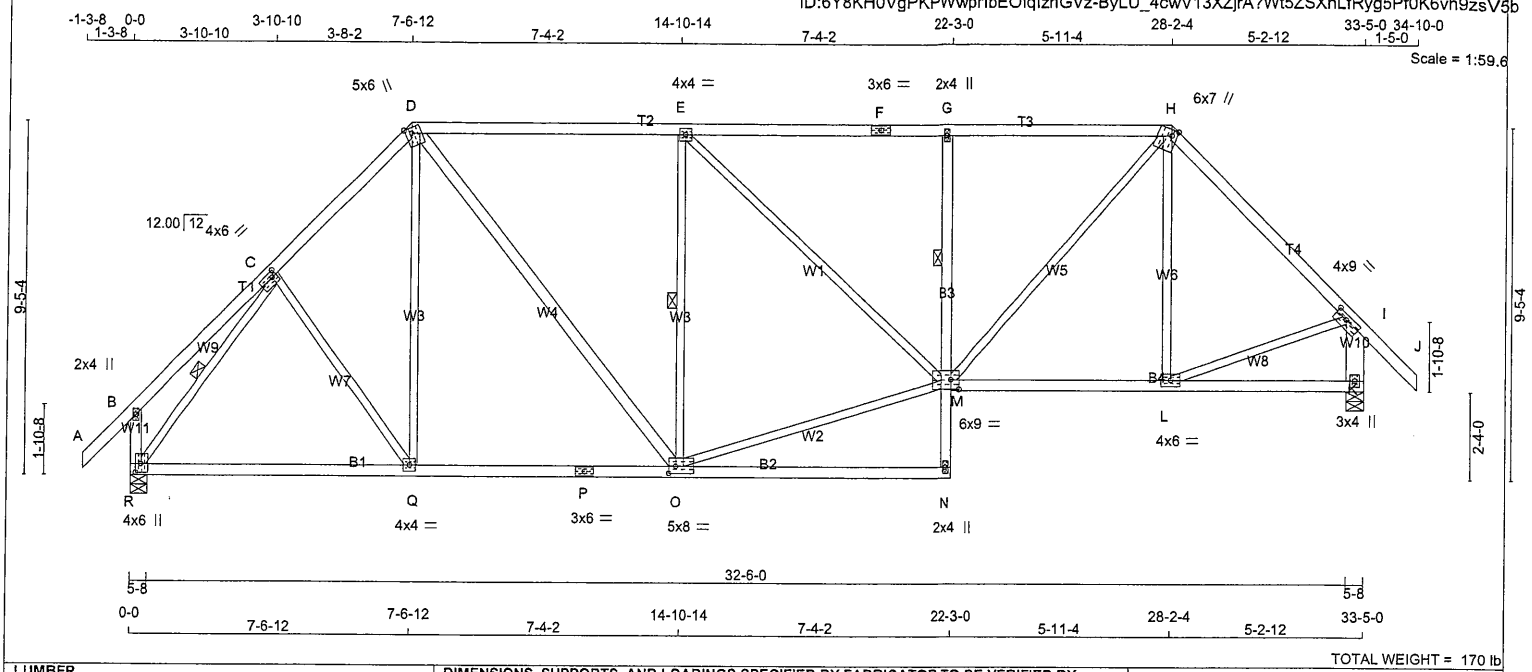

TOTAL WEIGHT = 170 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	1.75	1.50
D	TTWW+m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.75
I	TMVW-t	MT20	4.0	9.0	1.50	4.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BVMWWW-I	MT20	6.0	9.0	3.00	2.50
N	BMV+p	MT20	2.0	4.0		
O	BMWWW-t	MT20	5.0	8.0	2.25	2.25
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1+p	MT20	4.0	6.0	2.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	2222	0	2222	0	0	5-8	2-7
R	2209	0	2209	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1541	1184 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0
R	1532	1176 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, E-O, C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	C- Q	0 / 56	0.02 (4)
B- C	0 / 31	-104.9	-104.9	0.23 (1)	10.00	Q- D	0 / 127	0.04 (4)
C- D	-1925 / 0	-104.9	-104.9	0.31 (1)	4.56	D- O	0 / 953	0.15 (1)
D- E	-1928 / 0	-104.9	-104.9	0.82 (1)	4.24	O- E	-1217 / 0	0.72 (1)
E- F	-2321 / 0	-104.9	-104.9	0.86 (1)	3.89	O- M	0 / 2022	0.45 (1)
F- G	-2321 / 0	-104.9	-104.9	0.86 (1)	3.89	E- M	0 / 523	0.12 (1)
G- H	-2326 / 0	-104.9	-104.9	0.58 (1)	4.36	M- H	0 / 1457	0.33 (1)
H- I	-1924 / 0	-104.9	-104.9	0.70 (1)	4.11	L- H	-257 / 52	0.22 (1)
I- J	0 / 56	-104.9	-104.9	0.17 (1)	10.00	R- C	-2272 / 0	0.65 (1)
R- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	L- I	0 / 1411	0.32 (1)
K- I	-2186 / 0	0.0	0.0	0.16 (1)	6.90			
R- Q	0 / 1316	-18.5	-18.5	0.34 (1)	10.00			
Q- P	0 / 1335	-18.5	-18.5	0.35 (1)	10.00			
P- O	0 / 1335	-18.5	-18.5	0.35 (1)	10.00			
O- N	0 / 16	-18.5	-18.5	0.22 (4)	10.00			
N- M	0 / 61	0.0	0.0	0.06 (1)	10.00			
M- G	-730 / 0	0.0	0.0	0.08 (1)	6.25			
M- L	0 / 1353	-18.5	-18.5	0.32 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.86/1.00 (E-G:1) , BC=0.35/1.00 (O-Q:1) , WB=0.72/1.00 (E-O:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H15	2	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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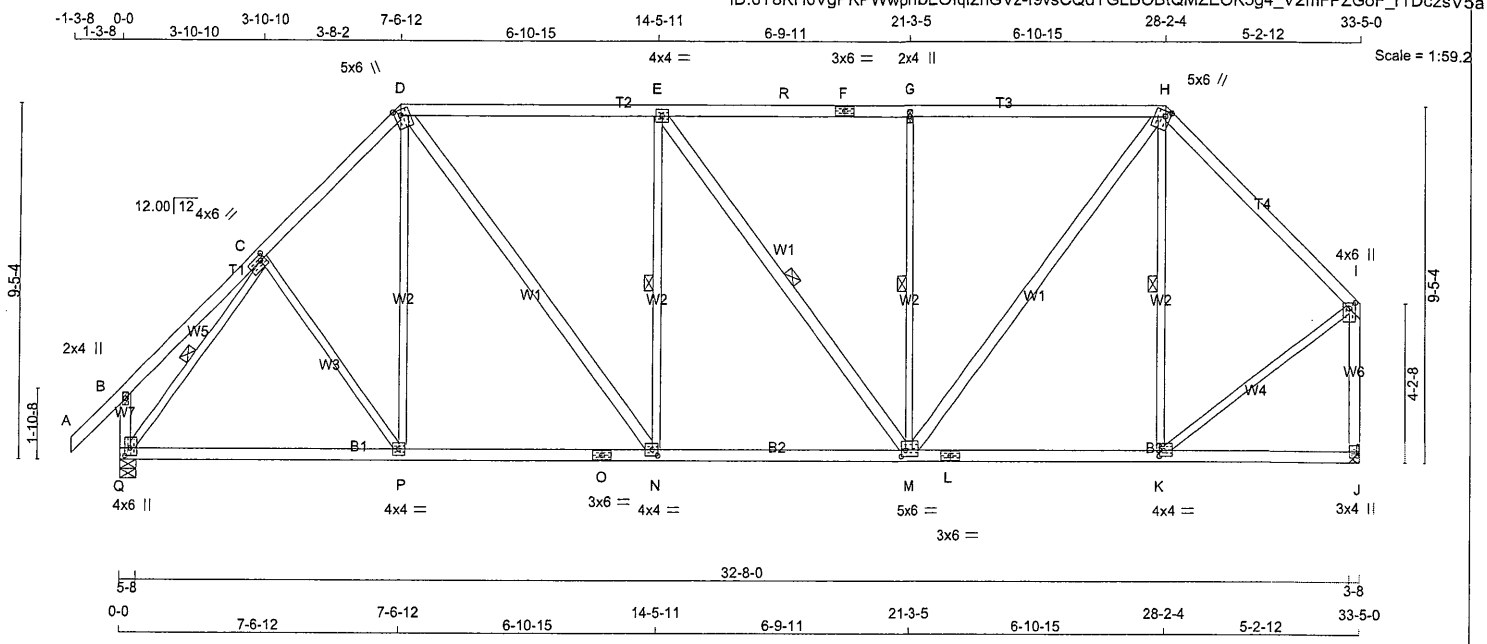


TOTAL WEIGHT = 2 X 173 = 346 lb [M][F]

LUMBER

N. L. G. A. RULES

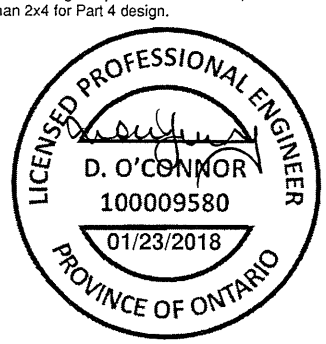
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	1.75	1.50
D	TTWW+m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	1.75	1.50
I	TMVW+p	MT20	4.0	6.0	1.75	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.75	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.00	1.50
N	BMWW-t	MT20	4.0	4.0	2.00	1.75
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1+p	MT20	4.0	6.0	2.50	1.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2281	0	2281	0	0	IN-SX	IN-SX
J	2145	0	2145	0	0	5-8	2-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1589	1176 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0
J	1499	1085 / 0	0 / 0	0 / 0	0 / 0	414 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

FOR SECTION D-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-M, G-M, H-K, C-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	C- P	0 / 73	0.03 (4)	
B- C	0 / 31	-104.9	-104.9	0.23 (1)	10.00	P- D	0 / 117	0.04 (4)	
C- D	-2010 / 0	-104.9	-104.9	0.24 (1)	4.58	D- N	0 / 1015	0.16 (1)	
D- E	-2008 / 0	-112.4	-112.4	0.71 (1)	2.00	N- E	-689 / 0	0.41 (1)	
E- R	-1897 / 0	-112.4	-112.4	0.63 (1)	2.00	E- M	-187 / 0	0.14 (1)	
R- F	-1897 / 0	-112.4	-112.4	0.63 (1)	2.00	M- G	-837 / 0	0.49 (1)	
F- G	-1897 / 0	-112.4	-112.4	0.63 (1)	2.00	M- H	0 / 1395	0.22 (1)	
G- H	-1896 / 0	-112.4	-112.4	0.70 (1)	2.00	K- H	-641 / 0	0.38 (1)	
H- I	-1505 / 0	-104.9	-104.9	0.43 (1)	4.89	Q- C	-2361 / 0	0.68 (1)	
Q- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	K- I	0 / 1309	0.29 (1)	
J- I	-2109 / 0	0.0	0.0	0.64 (1)	5.84				
Q- P	0 / 1367	-18.5	-18.5	0.36 (1)	10.00				
P- O	0 / 1395	-18.5	-18.5	0.36 (1)	10.00				
O- N	0 / 1395	-18.5	-18.5	0.36 (1)	10.00				
N- M	0 / 2008	-18.5	-18.5	0.41 (1)	10.00				
M- L	0 / 1054	-18.5	-18.5	0.27 (1)	10.00				
L- K	0 / 1054	-18.5	-18.5	0.27 (1)	10.00				
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00				

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.41/1.00 (M-N:1) , WB=0.68/1.00 (C-Q:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H21G	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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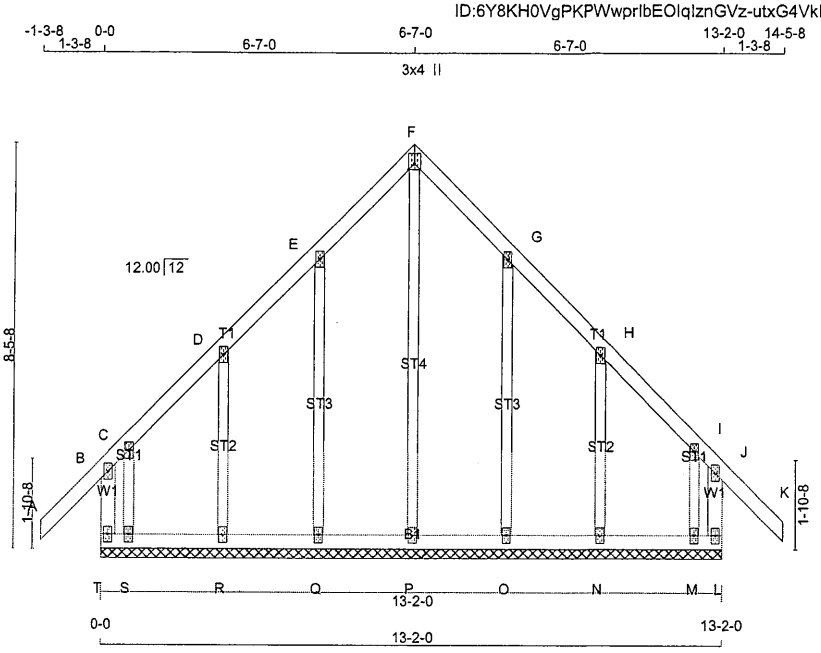

TOTAL WEIGHT = 70 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	3.0	4.0	Edge	
J	TMV+p	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-279 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	-71 / 0	-104.9	-104.9	0.11 (1)	6.25
C- D	0 / 13	-104.9	-104.9	0.05 (1)	10.00
D- E	0 / 19	-104.9	-104.9	0.06 (1)	10.00
E- F	0 / 22	-104.9	-104.9	0.06 (1)	10.00
F- G	0 / 22	-104.9	-104.9	0.06 (1)	10.00
G- H	0 / 19	-104.9	-104.9	0.06 (1)	10.00
H- I	0 / 13	-104.9	-104.9	0.05 (1)	10.00
I- J	-71 / 0	-104.9	-104.9	0.11 (1)	6.25
J- K	0 / 52	-104.9	-104.9	0.15 (1)	10.00
L- J	-279 / 0	0.0	0.0	0.03 (1)	7.81
T- S	-7 / 0	-18.5	-18.5	0.01 (1)	10.00
S- R	-7 / 0	-18.5	-18.5	0.02 (4)	10.00
R- Q	-13 / 0	-18.5	-18.5	0.02 (4)	6.25
Q- P	-16 / 0	-18.5	-18.5	0.01 (4)	6.25
P- O	-16 / 0	-18.5	-18.5	0.01 (4)	6.25
O- N	-13 / 0	-18.5	-18.5	0.02 (4)	6.25
N- M	-7 / 0	-18.5	-18.5	0.02 (4)	10.00
M- L	-7 / 0	-18.5	-18.5	0.01 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-250 / 0	0.34 (1)
Q- E	-215 / 0	0.14 (1)
R- D	-207 / 0	0.06 (1)
S- C	-12 / 0	0.00 (1)
O- G	-215 / 0	0.14 (1)
N- H	-207 / 0	0.06 (1)
M- I	-12 / 0	0.00 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (N-O:4) , WB=0.34/1.00 (F-P:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (F) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H21	2	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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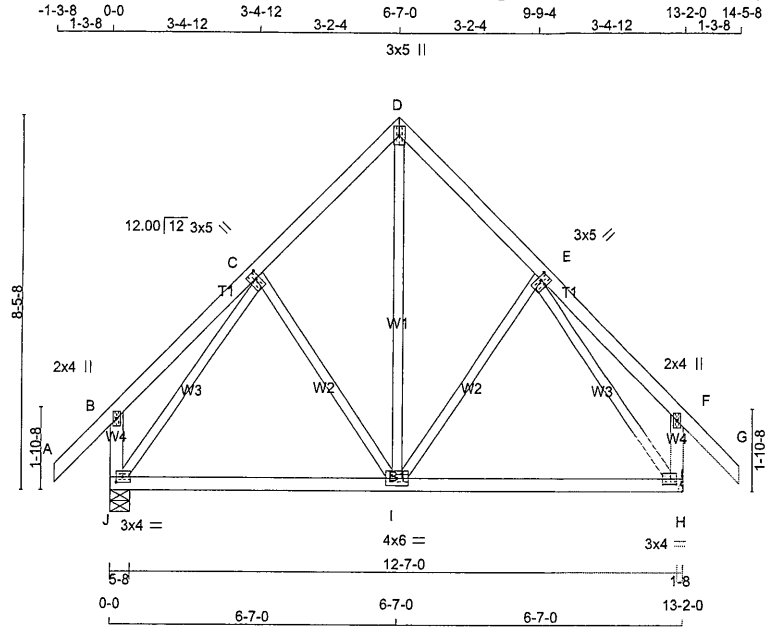

Scale = 1:50.4

TOTAL WEIGHT = 2 X 69 = 137 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+t	MT20	3.0	5.0	2.00	1.00
D	TTW+p	MT20	3.0	5.0		
E	TMWW+t	MT20	3.0	5.0	2.00	1.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	959	0	959	0	0	5-8	1-8
H	959	0	959	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	664	518 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	664	518 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	I- D	0 / 411	0.09 (1)
B- C	0 / 28	-104.9	-104.9	0.18 (1)	10.00	I- E	-159 / 6	0.09 (1)
C- D	-510 / 0	-104.9	-104.9	0.14 (1)	6.25	C- I	-159 / 6	0.09 (1)
D- E	-510 / 0	-104.9	-104.9	0.14 (1)	6.25	J- C	-769 / 0	0.43 (1)
E- F	0 / 28	-104.9	-104.9	0.18 (1)	10.00	E- H	-769 / 0	0.43 (1)
F- G	0 / 52	-104.9	-104.9	0.15 (1)	10.00			
J- B	-277 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-277 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 433	-18.5	-18.5	0.25 (4)	10.00			
I- H	0 / 433	-18.5	-18.5	0.25 (4)	10.00			

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (E-F:1) , BC=0.25/1.00 (H-I:4) , WB=0.43/1.00 (C-J:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H17	2	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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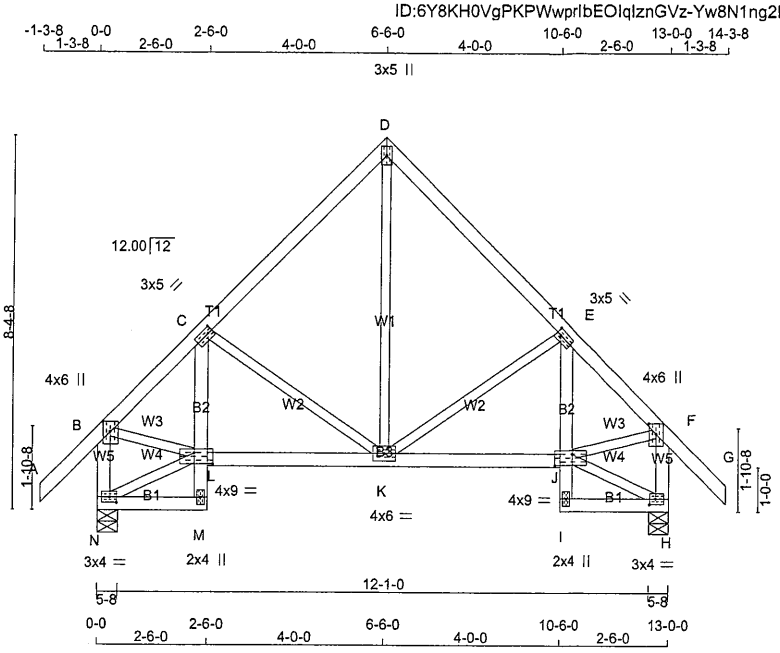

TOTAL WEIGHT = 2 X 72 = 144 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-t	MT20	3.0	5.0	1.50	1.25
D	TTW+p	MT20	3.0	5.0		
E	TMVW-t	MT20	3.0	5.0	1.50	1.25
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BMV+p	MT20	2.0	4.0		
J	BVMWW-l	MT20	4.0	9.0	3.00	5.00
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	4.0	9.0	3.00	5.00
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0	1.50	1.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	949	0	949	0	0	5-8	1-8
N	949	0	949	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	656	513 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
N	656	513 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 52	-104.9	-104.9	0.15 (1)	10.00	K- D		0 / 394	0.09 (1)
B- C		-813 / 0	-104.9	-104.9	0.17 (1)	6.25	K- E		-289 / 0	0.12 (1)
C- D		-559 / 0	-104.9	-104.9	0.21 (1)	6.25	J- H		-14 / 0	0.00 (1)
D- E		-559 / 0	-104.9	-104.9	0.21 (1)	6.25	J- F		0 / 619	0.14 (1)
E- F		-813 / 0	-104.9	-104.9	0.17 (1)	6.25	C- K		-289 / 0	0.12 (1)
F- G		0 / 52	-104.9	-104.9	0.15 (1)	10.00	N- L		-14 / 0	0.00 (1)
N- B		-921 / 0	0.0	0.0	0.10 (1)	7.81	B- L		0 / 619	0.14 (1)
H- F		-921 / 0	0.0	0.0	0.10 (1)	7.81				
N- M		0 / 13	-18.5	-18.5	0.03 (4)	10.00				
M- L		0 / 24	0.0	0.0	0.02 (1)	10.00				
L- C		-50 / 35	0.0	0.0	0.02 (4)	7.81				
L- K		0 / 614	-18.5	-18.5	0.15 (1)	10.00				
K- J		0 / 614	-18.5	-18.5	0.15 (1)	10.00				
I- J		0 / 24	0.0	0.0	0.02 (1)	10.00				
J- E		-50 / 35	0.0	0.0	0.02 (4)	7.81				
I- H		0 / 13	-18.5	-18.5	0.03 (4)	10.00				

A18012422

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.15/1.00 (J-K:1) , WB=0.14/1.00 (F-J:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H16	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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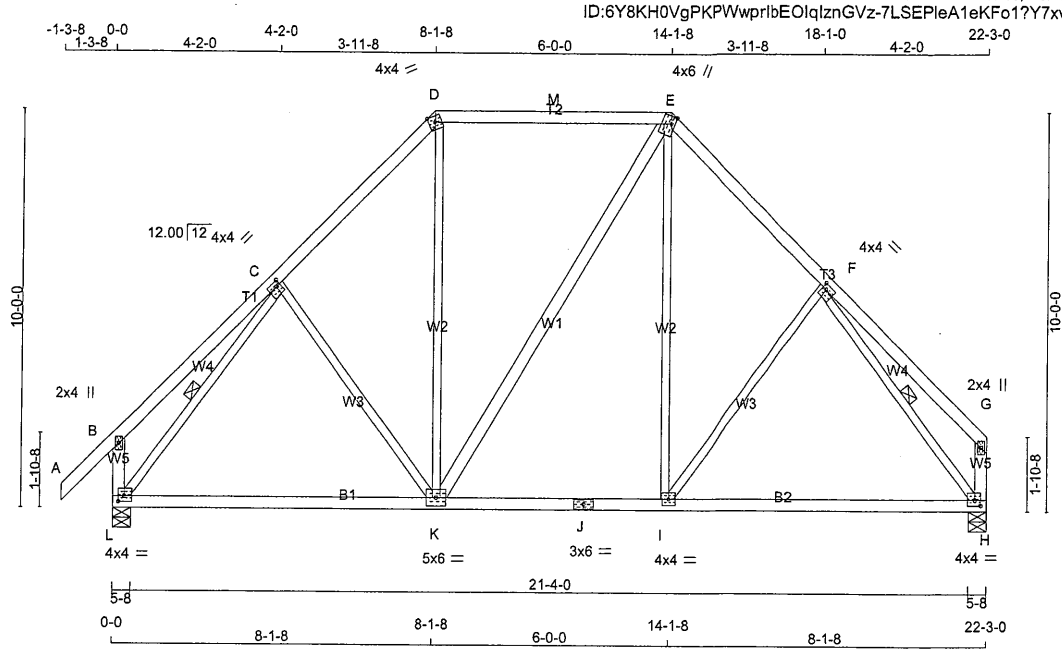

TOTAL WEIGHT = 114 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.25
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	1.75	1.75
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1542	0	1542	0	0	5-8	1-11
H	1396	0	1396	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1071	813 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
H	972	723 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
FOR SECTION D-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	0 / 36	-104.9	-104.9	0.28 (1)	10.00
C- D	-1112 / 0	-104.9	-104.9	0.23 (1)	5.75
D- M	-761 / 0	-112.4	-112.4	0.70 (1)	2.00
M- E	-761 / 0	-112.4	-112.4	0.70 (1)	2.00
E- F	-1111 / 0	-104.9	-104.9	0.23 (1)	5.75
F- G	0 / 36	-104.9	-104.9	0.28 (1)	10.00
L- B	-307 / 0	0.0	0.0	0.03 (1)	7.81
H- G	-161 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 849	-18.5	-18.5	0.30 (4)	10.00
K- J	0 / 761	-18.5	-18.5	0.29 (4)	10.00
J- I	0 / 761	-18.5	-18.5	0.29 (4)	10.00
I- H	0 / 849	-18.5	-18.5	0.31 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-150 / 22	0.14 (1)
K- D	0 / 261	0.06 (1)
K- E	0 / 0	0.00 (1)
I- E	0 / 260	0.06 (1)
I- F	-150 / 22	0.14 (1)
L- C	-1446 / 0	0.46 (1)
F- H	-1446 / 0	0.46 (1)

A18012423

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.31/1.00 (H-I:4) , WB=0.46/1.00 (C-L:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H16S	3	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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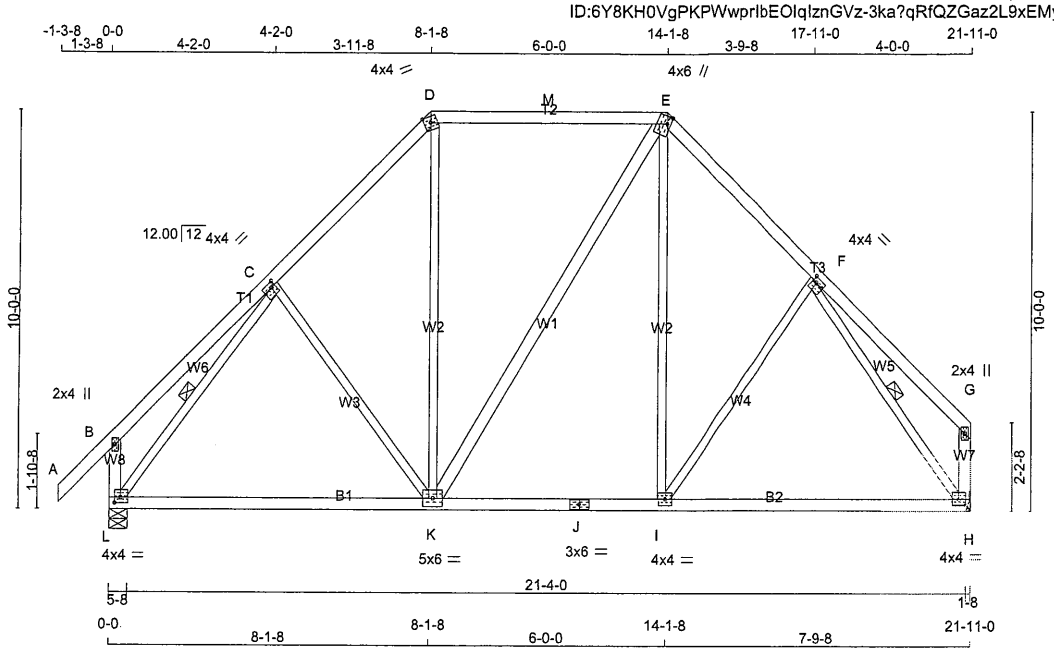

TOTAL WEIGHT = 3 X 113 = 340 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.25
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	1.75	2.00
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1521	0	1521	0	0	5-8	1-10
H	1375	0	1375	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1056	802 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0
H	958	712 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
FOR SECTION D-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	C- K	-154 / 21	0.14 (1)
B- C	0 / 36	-104.9	-104.9	0.28 (1)	10.00	K- D	0 / 243	0.05 (1)
C- D	-1087 / 0	-104.9	-104.9	0.23 (1)	5.80	K- E	0 / 27	0.00 (1)
D- M	-744 / 0	-112.4	-112.4	0.70 (1)	2.00	I- E	0 / 213	0.05 (1)
M- E	-744 / 0	-112.4	-112.4	0.70 (1)	2.00	I- F	-95 / 32	0.09 (1)
E- F	-1066 / 0	-104.9	-104.9	0.21 (1)	5.87	L- C	-1421 / 0	0.45 (1)
F- G	0 / 34	-104.9	-104.9	0.26 (1)	10.00	F- H	-1399 / 0	0.45 (1)
L- B	-307 / 0	0.0	0.0	0.03 (1)	7.81			
H- G	-154 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 834	-18.5	-18.5	0.30 (4)	10.00			
K- J	0 / 730	-18.5	-18.5	0.29 (4)	10.00			
J- I	0 / 730	-18.5	-18.5	0.29 (4)	10.00			
I- H	0 / 782	-18.5	-18.5	0.28 (4)	10.00			

A18012424

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.30/1.00 (K-L:4) , WB=0.45/1.00 (C-L:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H16G	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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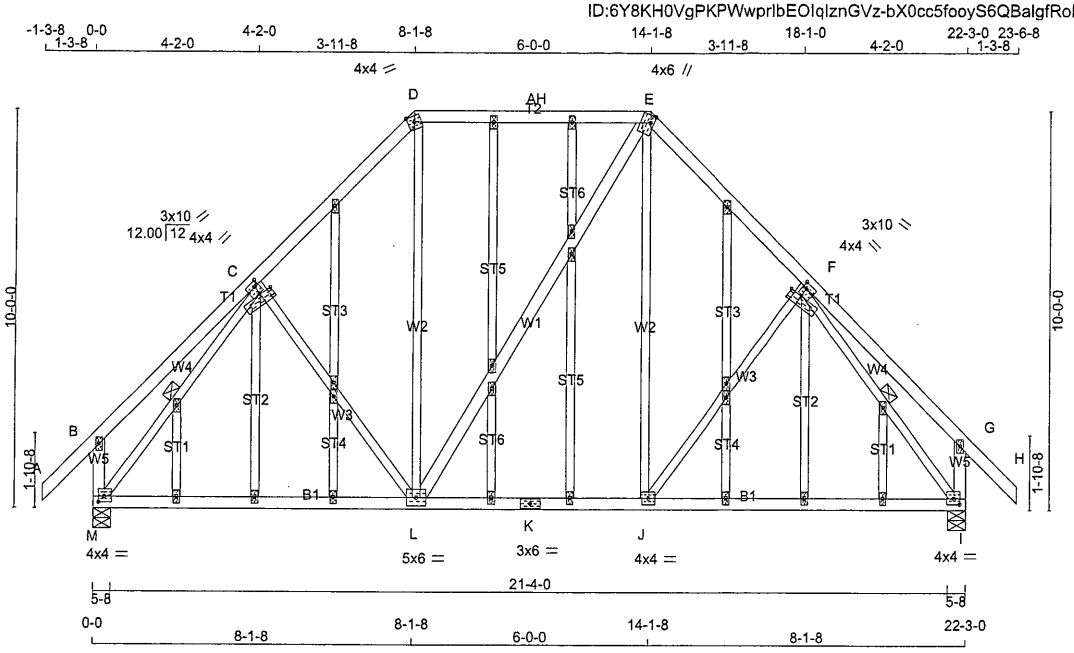

TOTAL WEIGHT = 155 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.25
C	NP+t	MT20	3.0	10.0	3.75	0.25
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	4.0	6.0	Edge	1.00
F	TMWW-t	MT20	4.0	4.0	1.50	1.25
F	NP+t	MT20	3.0	10.0	3.75	0.25
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0	1.75	1.75
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	4.0	4.0	1.75	1.75
N, O, P, Q, R, S, T, U, V, V, W, X, Y, Z, AA, AB, AC, AD, AD, AE, AF, AG						
N	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1542	0	1542	0	0	5-8	1-11
I	1542	0	1542	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1071	813 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
I	1071	813 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
FOR SECTION D-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	0 / 36	-104.9	-104.9	0.28 (1)	10.00
C- D	-1112 / 0	-104.9	-104.9	0.23 (1)	5.75
D-AH	-761 / 0	-112.4	-112.4	0.70 (1)	2.00
AH- E	-761 / 0	-112.4	-112.4	0.70 (1)	2.00
E- F	-1111 / 0	-104.9	-104.9	0.23 (1)	5.75
F- G	0 / 36	-104.9	-104.9	0.28 (1)	10.00
G- H	0 / 52	-104.9	-104.9	0.15 (1)	10.00
M- B	-307 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-307 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 849	-18.5	-18.5	0.30 (4)	10.00
L- K	0 / 761	-18.5	-18.5	0.29 (4)	10.00
K- J	0 / 761	-18.5	-18.5	0.29 (4)	10.00
J- I	0 / 849	-18.5	-18.5	0.31 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-150 / 22	0.14 (1)
L- D	0 / 261	0.06 (1)
L- E	0 / 0	0.00 (1)
J- E	0 / 260	0.06 (1)
J- F	-150 / 22	0.14 (1)
M- C	-1446 / 0	0.46 (1)
F- I	-1446 / 0	0.46 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.31/1.00 (I-J:4) , WB=0.46/1.00 (C-M:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P16	5	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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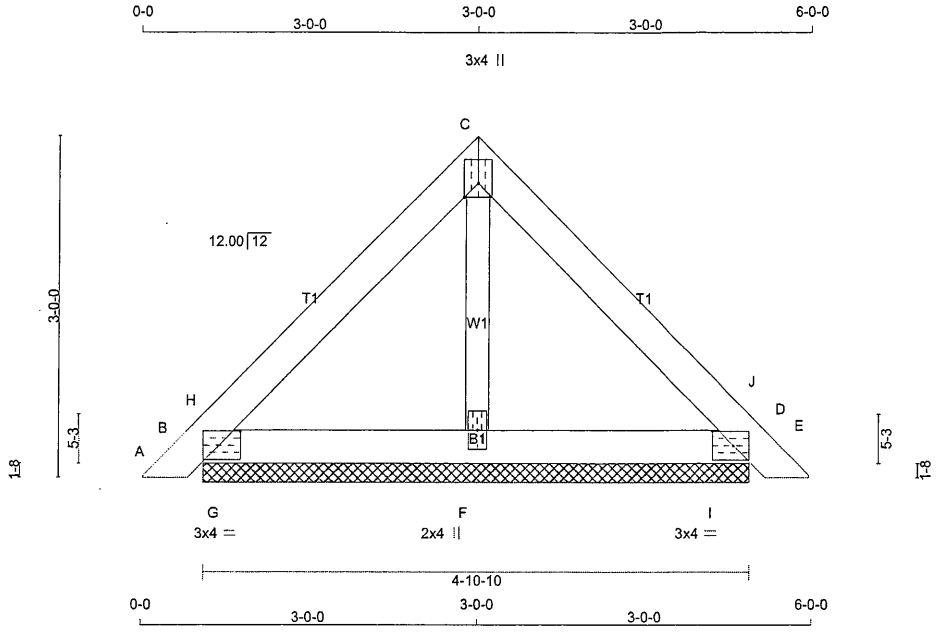

Scale = 1:19.6

TOTAL WEIGHT = 5 X 17 = 85 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW+p	MT20	3.0	4.0	Edge	
D	TMB1-I	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	251	0	251	0	0	4-10-10	4-4
D	251	0	251	0	0	4-10-10	4-4
F	193	0	193	0	0	4-10-10	4-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	173	143 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	173	143 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
F	137	89 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00	F- C	-73 / 0	0.01 (1)
B- H	-18 / 0	-104.9	-104.9	0.04 (1)	6.25	G- H	-255 / 0	0.00 (1)
H- C	-105 / 0	-104.9	-104.9	0.07 (1)	6.25	I- J	-255 / 0	0.00 (1)
C- J	-105 / 0	-104.9	-104.9	0.07 (1)	6.25			
J- D	-18 / 0	-104.9	-104.9	0.04 (1)	6.25			
D- E	0 / 15	-104.9	-104.9	0.02 (1)	10.00			
B- G	0 / 68	-18.5	-18.5	0.08 (1)	10.00			
G- F	0 / 68	-18.5	-18.5	0.08 (1)	10.00			
F- I	0 / 68	-18.5	-18.5	0.08 (1)	10.00			
I- D	0 / 68	-18.5	-18.5	0.08 (1)	10.00			

A18012426

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-H:1) , BC=0.08/1.00 (F-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.19/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P15	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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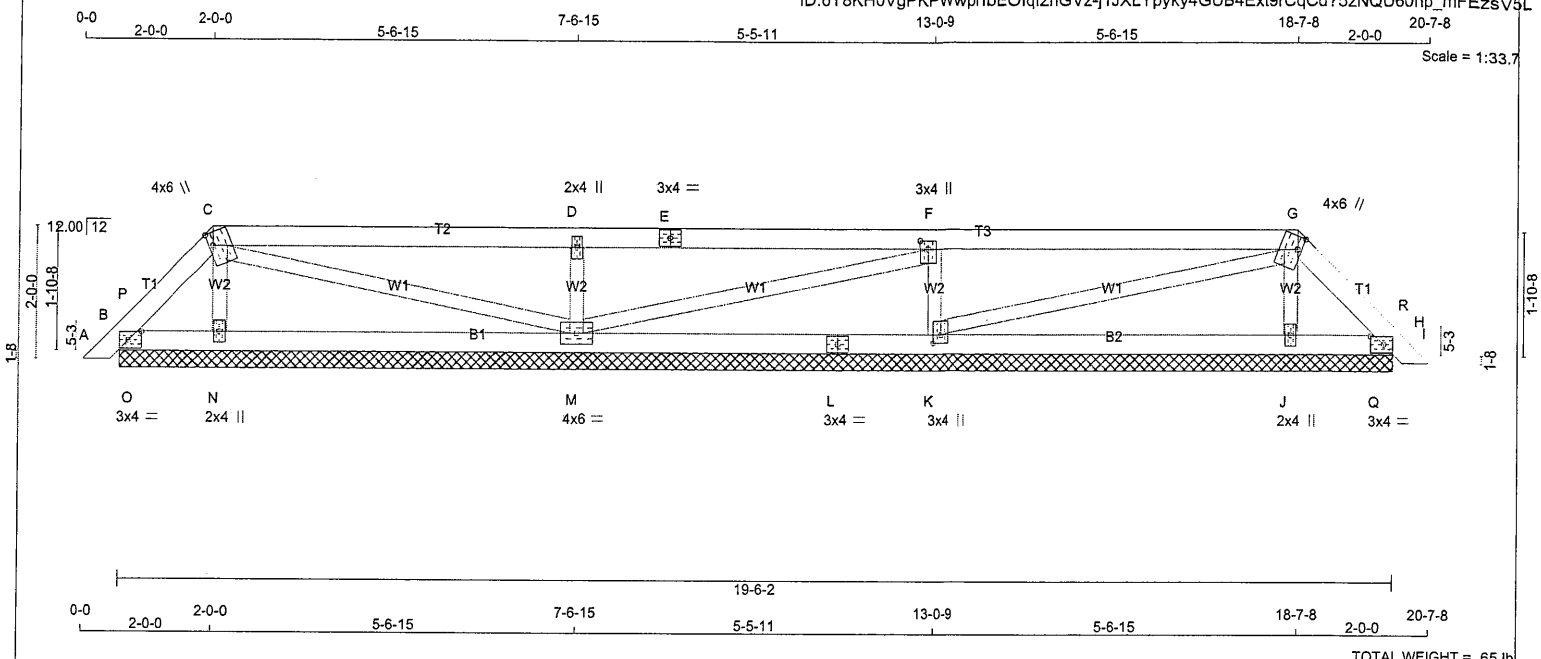

TOTAL WEIGHT = 65 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	4.0	6.0	2.25	0.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	4.0		
F	TMWW+t	MT20	3.0	4.0	1.50	1.50
G	TTWW+m	MT20	4.0	6.0	2.25	0.75
H	TMB1-I	MT20	3.0	4.0	1.50	2.25
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1+t	MT20	3.0	4.0	1.50	1.50
L	BS-t	MT20	3.0	4.0		
M	BMWWW1-t	MT20	4.0	6.0		
N	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	160	0	160	0	0	19-6-2 (8-6-2)	3-1-10
N	342	0	342	0	0	19-6-2 (8-6-2)	3-1-10
M	756	0	756	0	0	19-6-2 (8-6-2)	3-1-10
K	738	0	738	0	0	19-6-2 (8-6-2)	3-1-10
J	336	0	336	0	0	19-6-2 (8-6-2)	3-1-10
H	170	0	170	0	0	19-6-2 (8-6-2)	3-1-10

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	106	106 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
N	243	156 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
M	524	404 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
K	512	393 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
J	238	153 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
H	113	113 / 0	0 / 0	0 / 0	0 / 0	1 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00	N- C	-236 / 0	0.03 (1)
B- P	-24 / 26	-104.9	-104.9	0.02 (4)	6.25	C- M	-33 / 0	0.02 (1)
P- C	-70 / 0	-104.9	-104.9	0.02 (1)	6.25	M- D	-637 / 0	0.09 (1)
C- D	0 / 4	-104.9	-104.9	0.44 (1)	10.00	M- F	-25 / 0	0.01 (1)
D- E	0 / 4	-104.9	-104.9	0.44 (1)	10.00	K- F	-630 / 0	0.09 (1)
E- F	0 / 4	-104.9	-104.9	0.44 (1)	10.00	K- G	-19 / 0	0.01 (1)
F- G	-20 / 0	-104.9	-104.9	0.44 (1)	6.25	J- G	-230 / 0	0.03 (1)
G- R	-84 / 0	-104.9	-104.9	0.02 (1)	6.25	O- P	-134 / 0	0.00 (1)
R- H	-39 / 25	-104.9	-104.9	0.02 (4)	6.25	Q- R	-132 / 0	0.00 (1)
H- I	0 / 15	-104.9	-104.9	0.02 (1)	10.00			
B- O	0 / 44	-18.5	-18.5	0.04 (1)	10.00			
O- N	0 / 44	-18.5	-18.5	0.09 (4)	10.00			
N- M	0 / 28	-18.5	-18.5	0.11 (4)	10.00			
M- L	0 / 20	-18.5	-18.5	0.11 (4)	10.00			
L- K	0 / 20	-18.5	-18.5	0.11 (4)	10.00			
K- J	0 / 38	-18.5	-18.5	0.11 (4)	10.00			
J- Q	0 / 53	-18.5	-18.5	0.09 (4)	10.00			
Q- H	0 / 53	-18.5	-18.5	0.04 (1)	10.00			

A18012427

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44/1.00 (C-D:1) , BC=0.11/1.00 (K-M:4) , WB=0.09/1.00 (D-M:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P9A	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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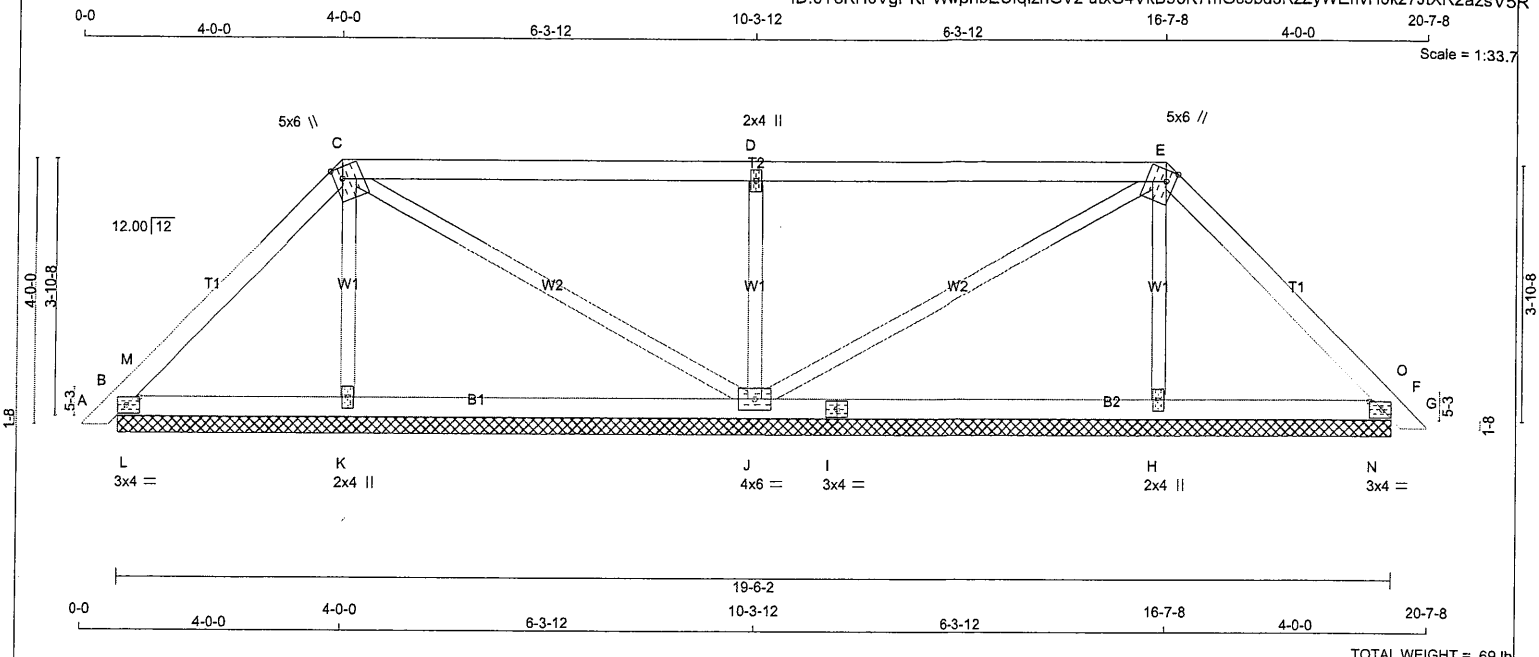

TOTAL WEIGHT = 69 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	357	0	357	0	0	19-6-2 (8-6-2)	7-4
K	387	0	387	0	0	19-6-2 (8-6-2)	7-4
J	1014	0	1014	0	0	19-6-2 (8-6-2)	7-4
H	387	0	387	0	0	19-6-2 (8-6-2)	7-4
F	357	0	357	0	0	19-6-2 (8-6-2)	7-4

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	244	208 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
K	274	179 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
J	701	552 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
H	274	179 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	244	208 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- M	-8 / 65	-104.9	-104.9	0.10 (1)	10.00
M- C	-172 / 0	-104.9	-104.9	0.15 (1)	6.25
C- D	-36 / 0	-104.9	-104.9	0.71 (1)	6.25
D- E	-36 / 0	-104.9	-104.9	0.71 (1)	6.25
E- O	-172 / 0	-104.9	-104.9	0.15 (1)	6.25
O- F	-8 / 65	-104.9	-104.9	0.10 (1)	10.00
F- G	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- L	0 / 111	-18.5	-18.5	0.14 (1)	10.00
L- K	0 / 111	-18.5	-18.5	0.14 (1)	10.00
K- J	0 / 104	-18.5	-18.5	0.16 (4)	10.00
J- I	0 / 104	-18.5	-18.5	0.16 (4)	10.00
I- H	0 / 104	-18.5	-18.5	0.16 (4)	10.00
H- N	0 / 111	-18.5	-18.5	0.14 (1)	10.00
N- F	0 / 111	-18.5	-18.5	0.14 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-249 / 0	0.06 (1)
C- J	-77 / 0	0.08 (1)
J- D	-827 / 0	0.19 (1)
J- E	-77 / 0	0.08 (1)
H- E	-249 / 0	0.06 (1)
L- M	-472 / 0	0.00 (1)
N- O	-472 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.71/1.00 (C-D:1) , BC=0.16/1.00 (J-K:4) , WB=0.19/1.00 (D-J:1) , SSI=0.36/1.00 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (E) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

A18012428

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P9B	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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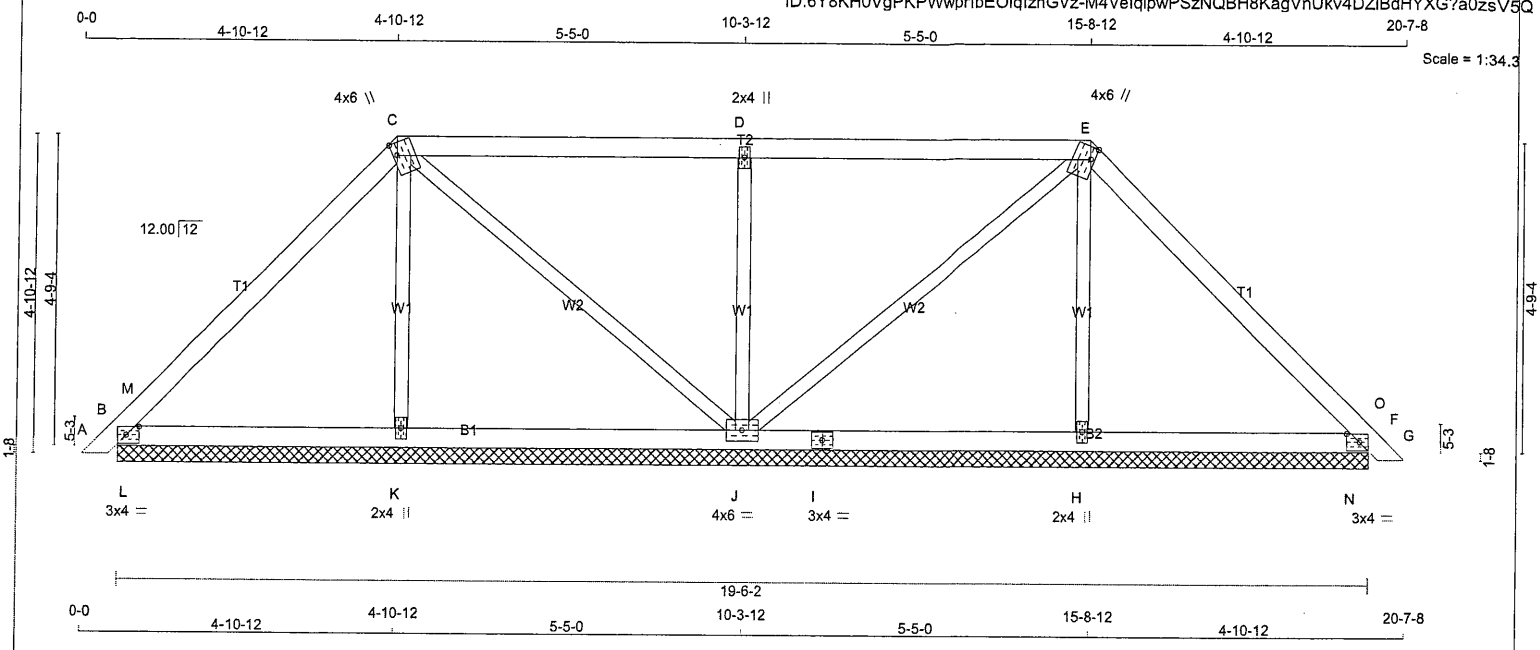

TOTAL WEIGHT = 71 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	4.0	6.0	2.25	0.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	4.0	6.0	2.25	0.75
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	449	0	449	0	0	19-6-2 (8-6-2)	5-10
K	345	0	345	0	0	19-6-2 (8-6-2)	5-10
J	914	0	914	0	0	19-6-2 (8-6-2)	5-10
H	345	0	345	0	0	19-6-2 (8-6-2)	5-10
F	449	0	449	0	0	19-6-2 (8-6-2)	5-10

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	308	254 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
K	245	157 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
J	631	504 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
H	245	157 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
F	308	254 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00	K- C	-192 / 0	0.06 (1)	
B- M	-42 / 169	-104.9	-104.9	0.17 (1)	6.25	C- J	-102 / 0	0.09 (1)	
M- C	-227 / 0	-104.9	-104.9	0.24 (1)	6.25	J- D	-709 / 0	0.24 (1)	
C- D	-64 / 0	-104.9	-104.9	0.52 (1)	6.25	J- E	-102 / 0	0.09 (1)	
D- E	-64 / 0	-104.9	-104.9	0.52 (1)	6.25	H- E	-192 / 0	0.06 (1)	
E- O	-227 / 0	-104.9	-104.9	0.24 (1)	6.25	L- M	-714 / 0	0.00 (1)	
O- F	-42 / 169	-104.9	-104.9	0.17 (1)	6.25	N- O	-714 / 0	0.00 (1)	
F- G	0 / 15	-104.9	-104.9	0.02 (1)	10.00				
B- L	0 / 147	-18.5	-18.5	0.22 (1)	10.00				
L- K	0 / 147	-18.5	-18.5	0.22 (1)	10.00				
K- J	0 / 143	-18.5	-18.5	0.14 (1)	10.00				
J- I	0 / 143	-18.5	-18.5	0.14 (1)	10.00				
I- H	0 / 143	-18.5	-18.5	0.14 (1)	10.00				
H- N	0 / 147	-18.5	-18.5	0.22 (1)	10.00				
N- F	0 / 147	-18.5	-18.5	0.22 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52/1.00 (C-D:1) , BC=0.22/1.00 (B-L:1) , WB=0.24/1.00 (D-J:1) , SSI=0.55/1.00 (F-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

A18012429

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P9C	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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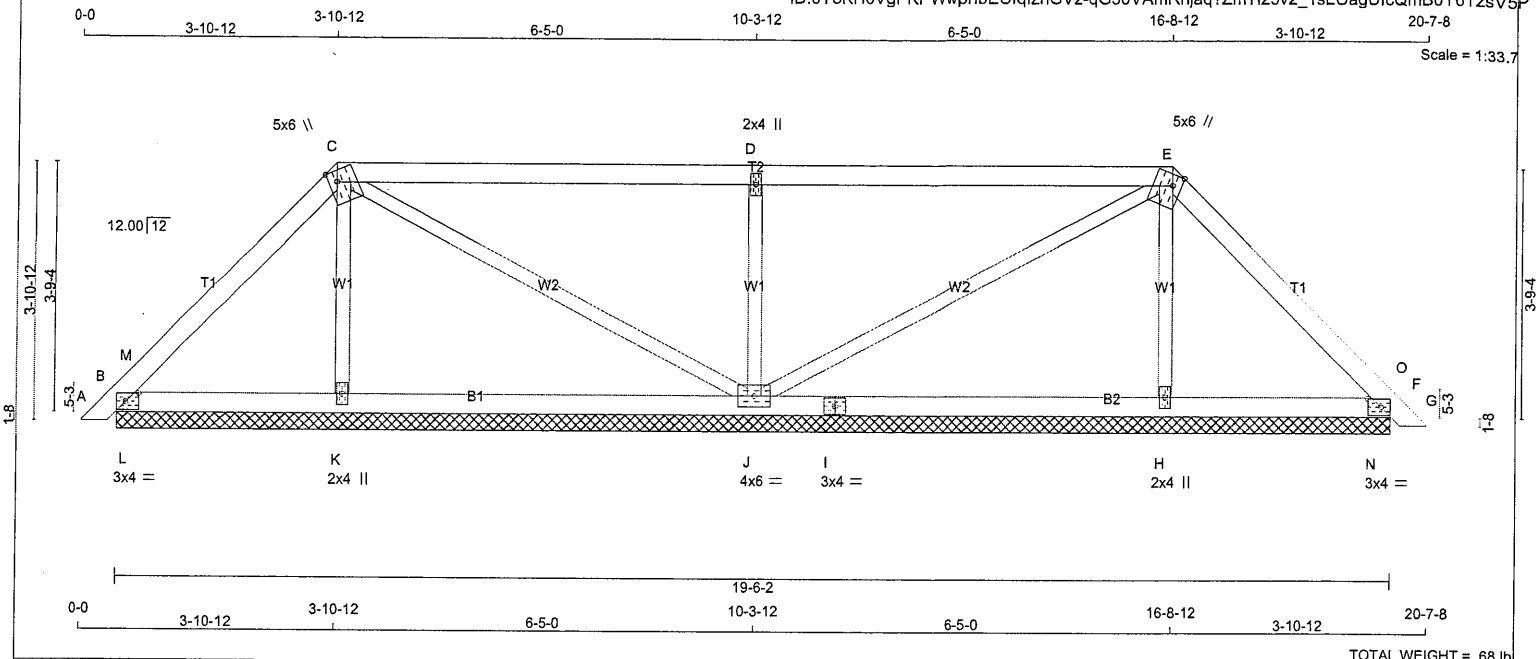

TOTAL WEIGHT = 68 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	346	0	346	0	0	19-6-2 (8-6-2)	7-8
K	392	0	392	0	0	19-6-2 (8-6-2)	7-8
J	1025	0	1025	0	0	19-6-2 (8-6-2)	7-8
H	392	0	392	0	0	19-6-2 (8-6-2)	7-8
F	346	0	346	0	0	19-6-2 (8-6-2)	7-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	237	202 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
K	277	181 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
J	709	558 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
H	277	181 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
F	237	202 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- M	-4 / 56	-104.9	-104.9	0.09 (1)	10.00
M- C	-166 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-34 / 0	-104.9	-104.9	0.73 (1)	6.25
D- E	-34 / 0	-104.9	-104.9	0.73 (1)	6.25
E- O	-166 / 0	-104.9	-104.9	0.14 (1)	6.25
O- F	-4 / 56	-104.9	-104.9	0.09 (1)	10.00
F- G	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- L	0 / 108	-18.5	-18.5	0.14 (1)	10.00
L- K	0 / 108	-18.5	-18.5	0.14 (1)	10.00
K- J	0 / 100	-18.5	-18.5	0.16 (4)	10.00
J- I	0 / 100	-18.5	-18.5	0.16 (4)	10.00
I- H	0 / 100	-18.5	-18.5	0.16 (4)	10.00
H- N	0 / 108	-18.5	-18.5	0.14 (1)	10.00
N- F	0 / 108	-18.5	-18.5	0.14 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-255 / 0	0.06 (1)
C- J	-74 / 0	0.07 (1)
J- D	-841 / 0	0.19 (1)
J- E	-74 / 0	0.07 (1)
H- E	-255 / 0	0.06 (1)
L- M	-448 / 0	0.00 (1)
N- O	-448 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.73/1.00 (C-D:1) , BC=0.16/1.00 (H-J:4) , WB=0.19/1.00 (D-J:1) , SSI=0.34/1.00 (F-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

A18012430

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P9D	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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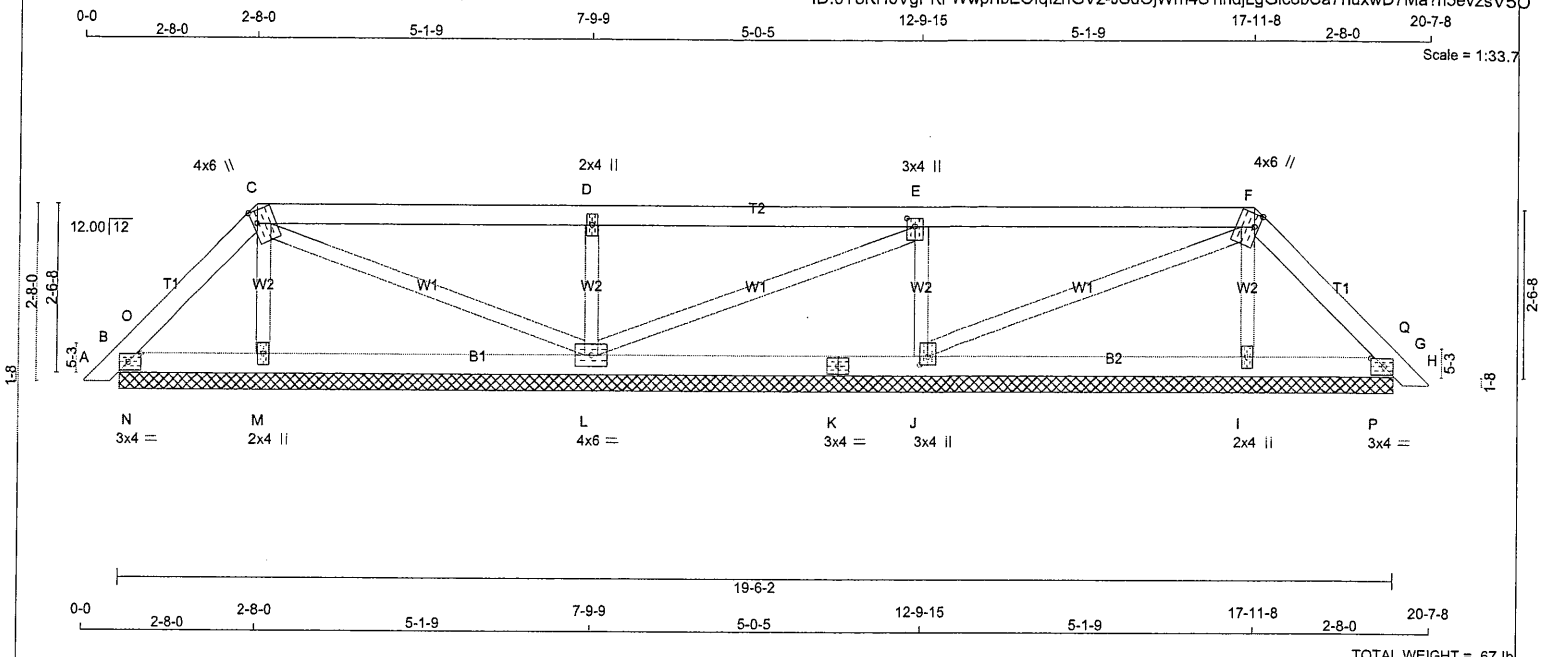

TOTAL WEIGHT = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	4.0	6.0	2.25	0.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW+t	MT20	3.0	4.0	1.50	1.50
F	TTWW+m	MT20	4.0	6.0	2.25	0.75
G	TMB1-I	MT20	3.0	4.0	1.50	2.50
I	BMW1+w	MT20	2.0	4.0		
J	BMWW1+t	MT20	3.0	4.0	1.50	1.50
K	BS-t	MT20	3.0	4.0		
L	BMWWW1-t	MT20	4.0	6.0		
M	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	221	0	221	0	0	19-6-2 (8-6-2)	3-0-1
M	326	0	326	0	0	19-6-2 (8-6-2)	3-0-1
L	725	0	725	0	0	19-6-2 (8-6-2)	3-0-1
J	671	0	671	0	0	19-6-2 (8-6-2)	3-0-1
I	320	0	320	0	0	19-6-2 (8-6-2)	3-0-1
G	239	0	239	0	0	19-6-2 (8-6-2)	3-0-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	150	133 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
M	230	153 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
L	502	390 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
J	465	356 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
I	226	149 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	163	144 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, J, I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00	M- C	-225 / 0	0.04 (1)
B- O	-13 / 7	-104.9	-104.9	0.03 (1)	6.25	C- L	-62 / 0	0.03 (1)
O- C	-87 / 0	-104.9	-104.9	0.05 (1)	6.25	L- D	-586 / 0	0.09 (1)
C- D	0 / 12	-104.9	-104.9	0.37 (1)	10.00	L- E	-51 / 0	0.03 (1)
D- E	0 / 12	-104.9	-104.9	0.37 (1)	10.00	J- E	-565 / 0	0.09 (1)
E- F	-34 / 0	-104.9	-104.9	0.37 (1)	6.25	J- F	-31 / 0	0.02 (1)
F- Q	-112 / 0	-104.9	-104.9	0.05 (1)	6.25	I- F	-219 / 0	0.03 (1)
Q- G	-41 / 5	-104.9	-104.9	0.03 (1)	6.25	N- O	-202 / 0	0.00 (1)
G- H	0 / 15	-104.9	-104.9	0.02 (1)	10.00	P- Q	-198 / 0	0.00 (1)
B- N	0 / 55	-18.5	-18.5	0.06 (1)	10.00			
N- M	0 / 55	-18.5	-18.5	0.07 (4)	10.00			
M- L	0 / 45	-18.5	-18.5	0.10 (4)	10.00			
L- K	0 / 34	-18.5	-18.5	0.10 (4)	10.00			
K- J	0 / 34	-18.5	-18.5	0.10 (4)	10.00			
J- I	0 / 63	-18.5	-18.5	0.10 (4)	10.00			
I- P	0 / 73	-18.5	-18.5	0.07 (4)	10.00			
P- G	0 / 73	-18.5	-18.5	0.06 (1)	10.00			

A18012431

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (D-E:1) , BC=0.10/1.00 (I-J:4) , WB=0.09/1.00 (D-L:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P9E	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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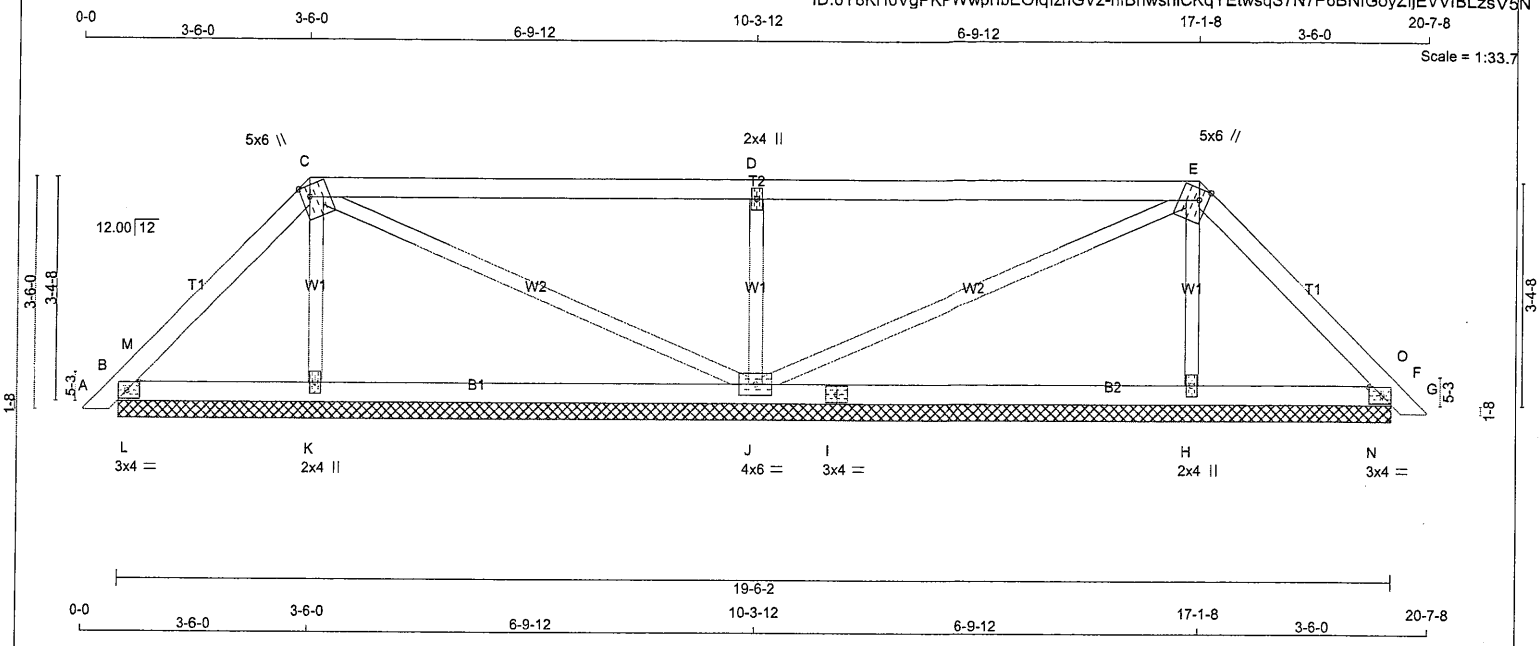

TOTAL WEIGHT = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	306	0	306	0	0	19-6-2 (8-6-2)	8-7
K	411	0	411	0	0	19-6-2 (8-6-2)	8-7
J	1069	0	1069	0	0	19-6-2 (8-6-2)	8-7
H	411	0	411	0	0	19-6-2 (8-6-2)	8-7
F	306	0	306	0	0	19-6-2 (8-6-2)	8-7

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	208	183 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
K	291	190 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
J	739	580 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
H	291	190 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
F	208	183 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- M	0 / 28	-104.9	-104.9	0.07 (1)	10.00
M- C	-145 / 0	-104.9	-104.9	0.10 (1)	6.25
C- D	-28 / 0	-104.9	-104.9	0.83 (1)	6.25
D- E	-28 / 0	-104.9	-104.9	0.83 (1)	6.25
E- O	-145 / 0	-104.9	-104.9	0.10 (1)	6.25
O- F	0 / 28	-104.9	-104.9	0.07 (1)	10.00
F- G	0 / 15	-104.9	-104.9	0.02 (1)	10.00
B- L	0 / 93	-18.5	-18.5	0.11 (1)	10.00
L- K	0 / 93	-18.5	-18.5	0.13 (4)	10.00
K- J	0 / 84	-18.5	-18.5	0.18 (4)	10.00
J- I	0 / 84	-18.5	-18.5	0.18 (4)	10.00
I- H	0 / 84	-18.5	-18.5	0.18 (4)	10.00
H- N	0 / 93	-18.5	-18.5	0.13 (4)	10.00
N- F	0 / 93	-18.5	-18.5	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-276 / 0	0.05 (1)
C- J	-61 / 0	0.07 (1)
J- D	-893 / 0	0.18 (1)
J- E	-61 / 0	0.07 (1)
H- E	-276 / 0	0.05 (1)
L- M	-368 / 0	0.00 (1)
N- O	-368 / 0	0.00 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.83/1.00 (C-D:1) , BC=0.18/1.00 (J-K:4) , WB=0.18/1.00 (D-J:1) , SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	P10	1	2	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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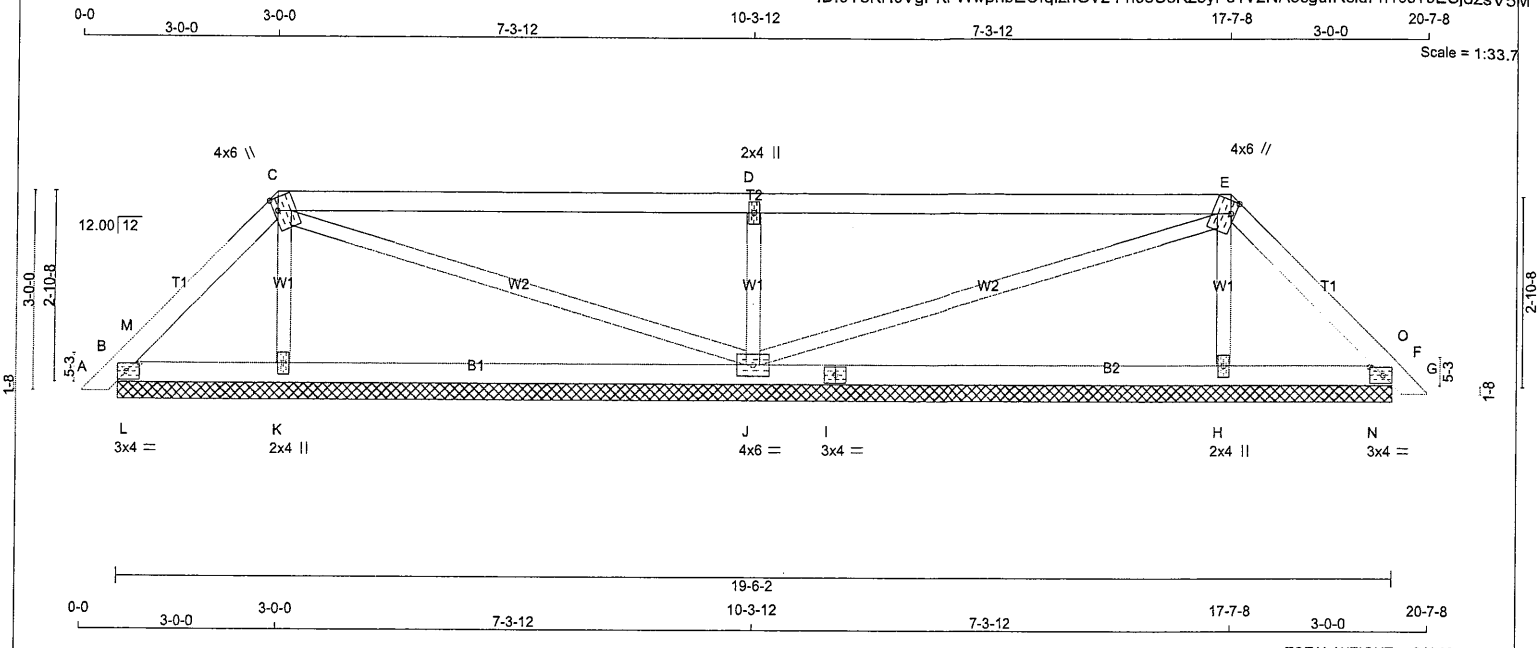


TOTAL WEIGHT = 2 X 66 = 132 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- I	1	12	TOP
I- F	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

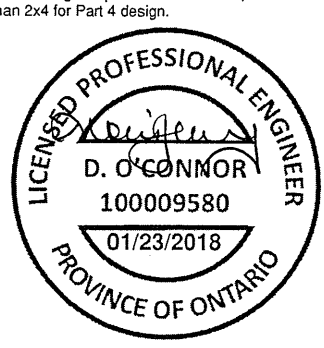
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	4.0	6.0	2.25	0.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	4.0	6.0	2.25	0.75
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	254	0	254	0	0	19-6-2 (8-6-2	2-4-0
K	433	0	433	0	0	19-6-2 (8-6-2	2-4-0
J	1127	0	1127	0	0	19-6-2 (8-6-2	2-4-0
H	433	0	433	0	0	19-6-2 (8-6-2	2-4-0
F	254	0	254	0	0	19-6-2 (8-6-2	2-4-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	171	159 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
K	307	198 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
J	779	610 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
H	307	198 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
F	171	159 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-104.9	-104.9	0.01 (1)	10.00
B- M	0 / 32	-104.9	-104.9	0.02 (1)	10.00
M- C	-122 / 0	-104.9	-104.9	0.03 (1)	6.25
C- D	-23 / 0	-104.9	-104.9	0.48 (1)	6.25
D- E	-23 / 0	-104.9	-104.9	0.48 (1)	6.25
E- O	-122 / 0	-104.9	-104.9	0.03 (1)	6.25
O- F	0 / 32	-104.9	-104.9	0.02 (1)	10.00
F- G	0 / 15	-104.9	-104.9	0.01 (1)	10.00
B- L	0 / 78	-18.5	-18.5	0.04 (1)	10.00
L- K	0 / 78	-18.5	-18.5	0.07 (4)	10.00
K- J	0 / 66	-18.5	-18.5	0.11 (4)	10.00
J- I	0 / 66	-18.5	-18.5	0.11 (4)	10.00
I- H	0 / 66	-18.5	-18.5	0.11 (4)	10.00
H- N	0 / 78	-18.5	-18.5	0.07 (4)	10.00
N- F	0 / 78	-18.5	-18.5	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-297 / 0	0.03 (1)
C- J	-45 / 0	0.03 (1)
J- D	-959 / 0	0.08 (1)
J- E	-45 / 0	0.03 (1)
H- E	-297 / 0	0.03 (1)
L- M	-278 / 0	0.00 (1)
N- O	-278 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-D:1) , BC=0.11/1.00 (J-K:4) , WB=0.08/1.00 (D-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H10	1	2	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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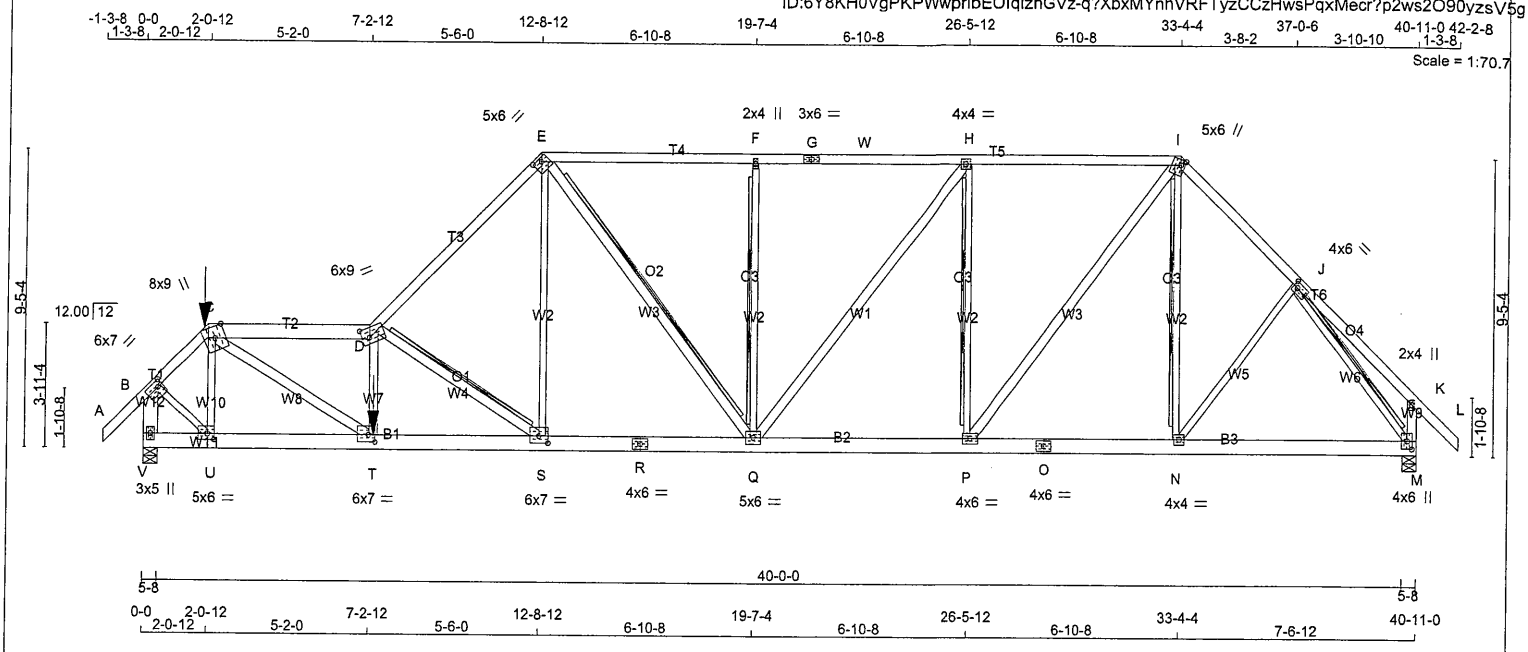

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - T	2x4	DRY	No.2	SPF
T - D	2x4	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
D- E	1	12	TOP
E- G	1	12	TOP
G- I	1	12	TOP
I- L	1	12	TOP
M- K	1	12	TOP
C- D	2	12	SIDE(50.8)
V- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- R	2	12	SIDE(174.2)
R- O	2	12	TOP
O- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
T- D	1	2	SIDE(663.9)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	6108	0	6108	0	0	5-8	4-7
M	3417	0	3417	0	0	5-8	1-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	4249	3186 / 0	0 / 0	0 / 0	0 / 0	1063 / 0	0 / 0
M	2381	1766 / 0	0 / 0	0 / 0	0 / 0	615 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-S, E-Q
2x4 DRY SPF No.2 T-BRACE AT F-Q, H-P, I-N, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.08 (1)	10.00	U- C	-1959 / 0	0.22 (1)
B- C	-5070 / 0	-104.9	-104.9	0.13 (1)	4.21	C- T	0 / 8620	0.76 (1)
C- D	-10741 / 0	-206.6	-206.6	0.43 (1)	3.38	T- D	-1615 / 0	0.15 (1)
D- E	-6477 / 0	-104.9	-104.9	0.82 (1)	3.39	D- S	-7199 / 0	0.59 (1)
E- F	-4583 / 0	-112.4	-112.4	0.62 (1)	2.00	S- E	0 / 4012	0.50 (1)
F- G	-4583 / 0	-112.4	-112.4	0.63 (1)	2.00	E- Q	-43 / 31	0.01 (1)
G- W	-4583 / 0	-112.4	-112.4	0.63 (1)	2.00	Q- F	-827 / 0	0.36 (1)
W- H	-4583 / 0	-112.4	-112.4	0.63 (1)	2.00	Q- H	0 / 1237	0.11 (1)
H- I	-3835 / 0	-112.4	-112.4	0.57 (1)	2.00	P- H	-1819 / 0	0.79 (1)
I- J	-3377 / 0	-104.9	-104.9	0.18 (1)	4.93	P- I	0 / 2439	0.22 (1)
J- K	0 / 30	-104.9	-104.9	0.12 (1)	10.00	N- I	-51 / 72	0.02 (1)
K- L	0 / 52	-104.9	-104.9	0.08 (1)	10.00	N- J	0 / 251	0.03 (1)
V- B	-6218 / 0	0.0	0.0	0.24 (1)	5.94	B- U	0 / 4316	0.53 (1)
M- K	-301 / 0	0.0	0.0	0.02 (1)	7.81	J- M	-3769 / 0	0.72 (1)
V- U	0 / 0	-36.4	-36.4	0.12 (1)	10.00			
U- T	0 / 3536	-36.4	-36.4	0.39 (1)	10.00			
T- S	0 / 10673	-18.5	-18.5	0.86 (1)	10.00			
S- R	0 / 4609	-18.5	-18.5	0.32 (1)	10.00			
R- Q	0 / 4609	-18.5	-18.5	0.32 (1)	10.00			
Q- P	0 / 3835	-18.5	-18.5	0.27 (1)	10.00			
P- O	0 / 2357	-18.5	-18.5	0.18 (1)	10.00			
O- N	0 / 2357	-18.5	-18.5	0.18 (1)	10.00			
N- M	0 / 2207	-18.5	-18.5	0.18 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-0-12	-158	-158	---	FRONT	VERT	TOTAL
T	7-4-8	-3213	-3213	---	FRONT	VERT	TOTAL

A18012434

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 12.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-0-12
RIGHT SETBACK = 7-6-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-4-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.86/1.00 (S-T:1) , WB=0.79/1.00 (H-P:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H10	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:6Y8KH0VgPKPWwprlbEOlqlznGVz-IB5z8iZPSoZ646YOmhp9OcN661y4kGl35i8iYOzsV5f

2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	2.25	2.00
C	TTWW+m	MT20	8.0	9.0	Edge	
D	TTWW-m	MT20	6.0	9.0	3.75	2.75
E	TTWW-h	MT20	5.0	6.0	1.50	3.25
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.00	1.50
J	TMWW-t	MT20	4.0	6.0	2.00	1.75
K	TMV+p	MT20	2.0	4.0		
M	BMVW1+p	MT20	4.0	6.0	3.00	1.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	4.0	6.0		
S	BMWW-t	MT20	6.0	7.0	2.75	3.50
T	BMWW-t	MT20	6.0	7.0	2.75	2.50
U	BMWW-t	MT20	5.0	6.0	2.25	2.50
V	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 157.8 lbs FACTORED DOWN AT 2-0-12 ON TOP CHORD, AND 3212.7 lbs FACTORED DOWN AT 7-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.97 (D) (INPUT = 1.00)

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H09	5	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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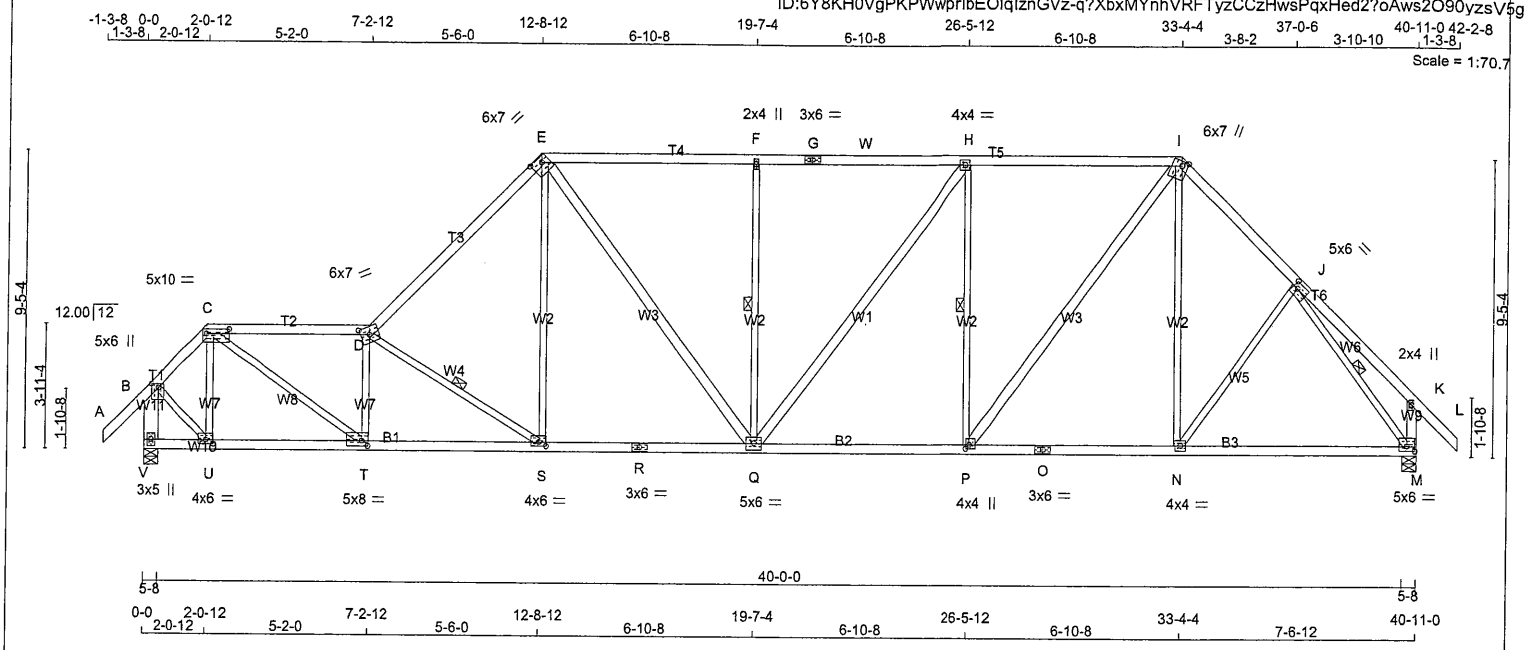

TOTAL WEIGHT = 5 X 204 = 1021 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.50	2.75
C	TTWW-I	MT20	5.0	10.0	1.75	8.25
D	TTWW-m	MT20	6.0	7.0	3.00	3.25
E	TTWW-h	MT20	6.0	7.0	2.00	4.25
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	1.75	3.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	4.0	1.50	1.50
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	6.0	1.50	2.75
T	BMWW-t	MT20	5.0	8.0	2.00	2.25
U	BMWW-t	MT20	4.0	6.0	1.50	2.75
V	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2739	0	2739	0	0	5-8	3-6
M	2759	0	2759	0	0	5-8	3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1907	1419 / 0	0 / 0	0 / 0	0 / 0	488 / 0	0 / 0
M	1923	1419 / 0	0 / 0	0 / 0	0 / 0	504 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-S, F-Q, H-P, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	-2069 / 0	-104.9	-104.9	0.10 (1)	4.65
C- D	-4229 / 0	-104.9	-104.9	0.62 (1)	2.91
D- E	-3559 / 0	-104.9	-104.9	0.78 (1)	3.17
E- F	-3008 / 0	-112.4	-112.4	0.82 (1)	2.00
F- G	-3009 / 0	-112.4	-112.4	0.76 (1)	2.00
G- W	-3009 / 0	-112.4	-112.4	0.76 (1)	2.00
W- H	-3009 / 0	-112.4	-112.4	0.76 (1)	2.00
H- I	-2761 / 0	-112.4	-112.4	0.79 (1)	2.00
I- J	-2573 / 0	-104.9	-104.9	0.27 (1)	4.10
J- K	0 / 30	-104.9	-104.9	0.22 (1)	10.00
K- L	0 / 52	-104.9	-104.9	0.15 (1)	10.00
V- B	-2749 / 0	0.0	0.0	0.20 (1)	6.30
M- K	-299 / 0	0.0	0.0	0.03 (1)	7.81
V- U	0 / 0	-18.5	-18.5	0.09 (1)	10.00
U- T	0 / 1426	-18.5	-18.5	0.32 (1)	10.00
T- S	0 / 4285	-18.5	-18.5	0.78 (1)	10.00
S- R	0 / 2531	-18.5	-18.5	0.50 (1)	10.00
R- Q	0 / 2531	-18.5	-18.5	0.50 (1)	10.00
Q- P	0 / 2762	-18.5	-18.5	0.53 (1)	10.00
P- O	0 / 1792	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 1792	-18.5	-18.5	0.42 (1)	10.00
N- M	0 / 1706	-18.5	-18.5	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-907 / 0	0.22 (1)
C- T	0 / 3444	0.77 (1)
T- D	-1937 / 0	0.46 (1)
D- S	-2104 / 0	0.65 (1)
S- E	0 / 1271	0.29 (1)
E- Q	0 / 792	0.13 (1)
Q- F	-834 / 0	0.49 (1)
Q- H	0 / 411	0.07 (1)
P- H	-1167 / 0	0.69 (1)
P- I	0 / 1611	0.26 (1)
N- I	0 / 93	0.03 (4)
N- J	0 / 142	0.03 (4)
B- U	0 / 1775	0.40 (1)
J- M	-2946 / 0	0.84 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 12.00/12 AND -12.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 12.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.82/1.00 (E-F:1) , BC=0.78/1.00 (S-T:1) , WB=0.84/1.00 (J-M:1) , SSI=0.36/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.72 (J) (INPUT = 1.00)

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H08	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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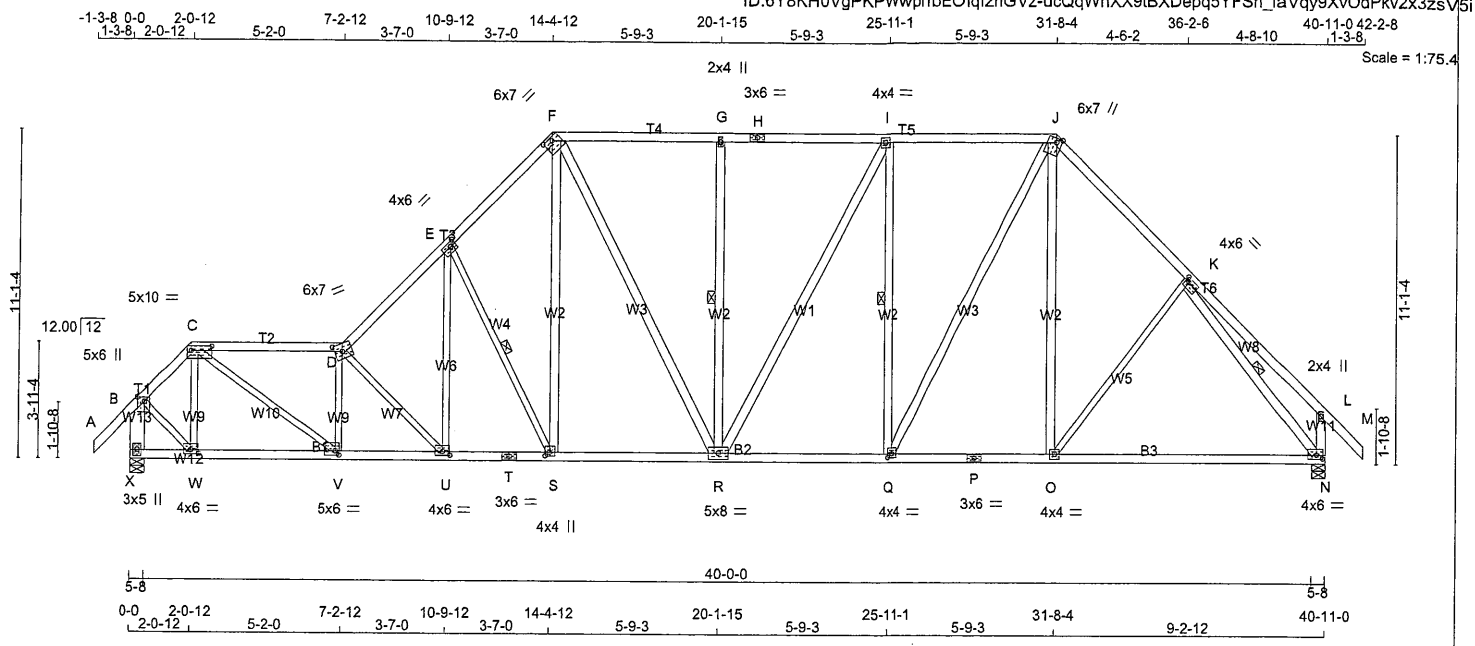

TOTAL WEIGHT = 242 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - T	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - C	2x3	DRY	No.2	SPF
C - V	2x3	DRY	No.2	SPF
V - D	2x3	DRY	No.2	SPF
D - U	2x3	DRY	No.2	SPF
U - E	2x3	DRY	No.2	SPF
E - S	2x3	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF
B - W	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW-l	MT20	5.0	10.0	1.75	8.25
D	TTWW-m	MT20	6.0	7.0	3.00	3.00
E	TMWW-t	MT20	4.0	6.0	1.75	2.25
F	TTWW-h	MT20	6.0	7.0	1.50	3.75
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW+m	MT20	6.0	7.0	Edge	2.25
K	TMWW-t	MT20	4.0	6.0	1.50	1.50
L	TMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	6.0	1.50	2.50
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	1.50	1.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2672	0	2672	0	0	5-8	3-3
N	2672	0	2672	0	0	5-8	2-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
N	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, G-R, I-Q, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00		W- C	-879 / 0	0.21 (1)
B- C	-2013 / 0	-104.9	-104.9	0.12 (1)	4.67		C- V	0 / 3326	0.75 (1)
C- D	-4095 / 0	-104.9	-104.9	0.84 (1)	2.67		V- D	-1874 / 0	0.45 (1)
D- E	-3942 / 0	-104.9	-104.9	0.47 (1)	3.22		D- U	-1887 / 0	0.80 (1)
E- F	-3115 / 0	-104.9	-104.9	0.38 (1)	3.67		U- E	0 / 1393	0.31 (1)
F- G	-2424 / 0	-104.9	-104.9	0.64 (1)	3.73		E- S	-1405 / 0	0.62 (1)
G- H	-2424 / 0	-104.9	-104.9	0.65 (1)	3.71		S- F	0 / 1350	0.22 (1)
H- I	-2424 / 0	-104.9	-104.9	0.65 (1)	3.71		F- R	0 / 496	0.08 (1)
I- J	-2252 / 0	-104.9	-104.9	0.63 (1)	3.86		R- G	-650 / 0	0.41 (1)
J- K	-2431 / 0	-104.9	-104.9	0.56 (1)	3.91		R- I	0 / 366	0.06 (1)
K- L	0 / 40	-104.9	-104.9	0.36 (1)	10.00		Q- I	-976 / 0	0.62 (1)
L- M	0 / 52	-104.9	-104.9	0.15 (1)	10.00		Q- J	0 / 1196	0.19 (1)
X- B	-2682 / 0	0.0	0.0	0.20 (1)	6.36		O- J	0 / 198	0.04 (4)
N- L	-329 / 0	0.0	0.0	0.04 (1)	7.81		O- K	-28 / 54	0.03 (1)
							B- W	0 / 1727	0.39 (1)
X- W	0 / 0	-18.5	-18.5	0.09 (1)	10.00		K- N	-2839 / 0	0.79 (1)
W- V	0 / 1388	-18.5	-18.5	0.31 (1)	10.00				
V- U	0 / 4149	-18.5	-18.5	0.74 (1)	10.00				
U- T	0 / 2820	-18.5	-18.5	0.52 (1)	10.00				
T- S	0 / 2820	-18.5	-18.5	0.52 (1)	10.00				
S- R	0 / 2190	-18.5	-18.5	0.42 (1)	10.00				
R- Q	0 / 2252	-18.5	-18.5	0.43 (1)	10.00				
Q- P	0 / 1688	-18.5	-18.5	0.47 (1)	10.00				
P- O	0 / 1688	-18.5	-18.5	0.47 (1)	10.00				
O- N	0 / 1703	-18.5	-18.5	0.47 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.84/1.00 (C-D:1) , BC=0.74/1.00 (U-V:1) , WB=0.80/1.00 (D-U:1) , SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.72 (T) (INPUT = 1.00)

A18012436

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H08	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:6Y8KH0VgPKPWwprlbEOlqlznGVz-ucQqWhXX9tBXDepq5YFSn_laVqy9XvOdPkv2x3zsV5i

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	4.0	1.50	1.75
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	6.0	1.50	3.00
V	BMWW-t	MT20	5.0	6.0	1.75	1.50
W	BMWW-t	MT20	4.0	6.0	1.50	3.00
X	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A18012436(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H07	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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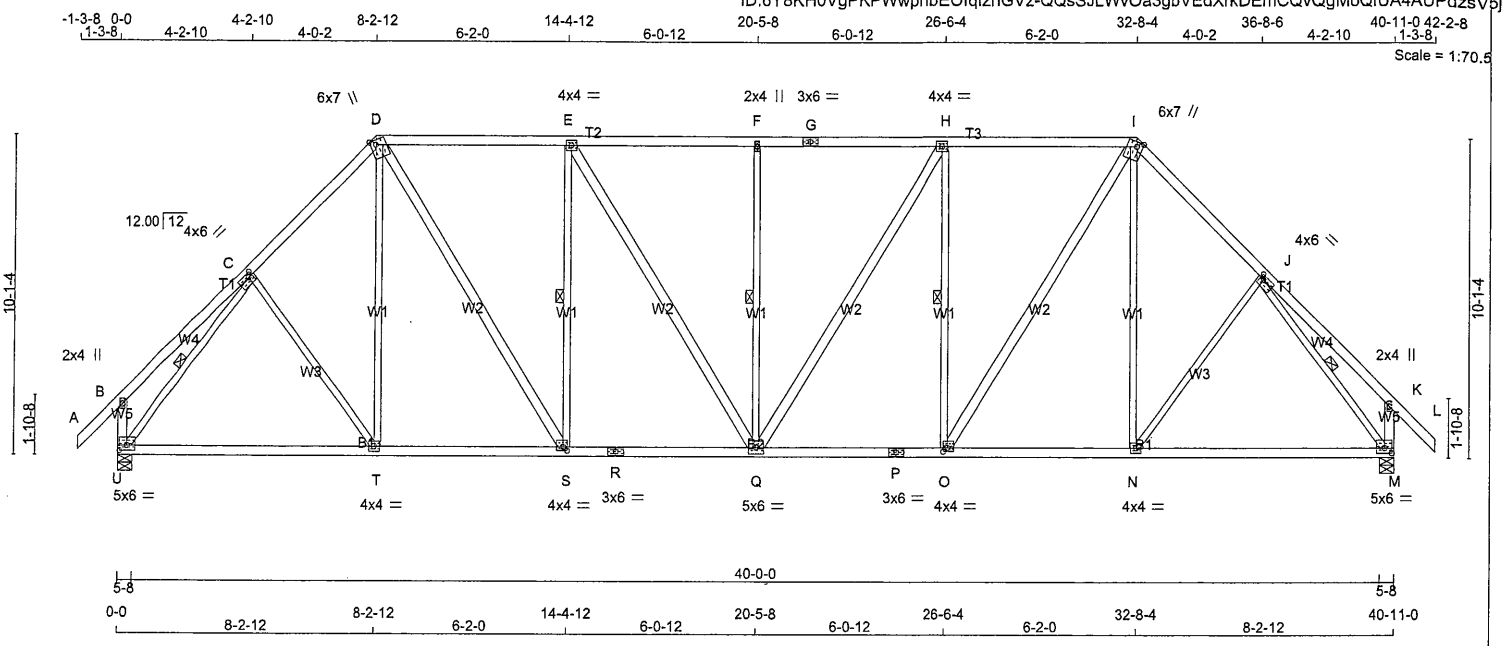

TOTAL WEIGHT = 221 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	1.50	1.50
D	TTWW+m	MT20	6.0	7.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	2.25
J	TMWW-t	MT20	4.0	6.0	1.50	1.50
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0	1.50	1.50
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	2.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2672	0	2672	0	0	5-8	3-10
M	2672	0	2672	0	0	5-8	3-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
M	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	1	0.15 (1)	10.00	C- T	0 / 68	0.02 (4)
B- C	0 / 34	-104.9	-104.9	1	0.27 (1)	10.00	T- D	0 / 125	0.04 (4)
C- D	-2458 / 0	-104.9	-104.9	1	0.42 (1)	4.03	D- S	0 / 1385	0.22 (1)
D- E	-2447 / 0	-104.9	-104.9	1	0.80 (1)	3.48	S- E	-1072 / 0	0.75 (1)
E- F	-2664 / 0	-104.9	-104.9	1	0.83 (1)	3.31	E- Q	0 / 411	0.07 (1)
F- G	-2664 / 0	-104.9	-104.9	1	0.83 (1)	3.31	Q- F	-585 / 0	0.41 (1)
G- H	-2664 / 0	-104.9	-104.9	1	0.83 (1)	3.31	Q- H	0 / 411	0.07 (1)
H- I	-2447 / 0	-104.9	-104.9	1	0.80 (1)	3.48	O- H	-1072 / 0	0.75 (1)
I- J	-2458 / 0	-104.9	-104.9	1	0.42 (1)	4.03	O- I	0 / 1385	0.22 (1)
J- K	0 / 34	-104.9	-104.9	1	0.27 (1)	10.00	N- I	0 / 125	0.04 (4)
K- L	0 / 52	-104.9	-104.9	1	0.15 (1)	10.00	N- J	0 / 68	0.02 (4)
U- B	-311 / 0	0.0	0.0	1	0.03 (1)	7.81	U- C	-2840 / 0	0.92 (1)
M- K	-311 / 0	0.0	0.0	1	0.03 (1)	7.81	J- M	-2840 / 0	0.92 (1)
U- T	0 / 1671	-18.5	-18.5	1	0.42 (1)	10.00			
T- S	0 / 1709	-18.5	-18.5	1	0.43 (1)	10.00			
S- R	0 / 2448	-18.5	-18.5	1	0.46 (1)	10.00			
R- Q	0 / 2448	-18.5	-18.5	1	0.46 (1)	10.00			
Q- P	0 / 2448	-18.5	-18.5	1	0.46 (1)	10.00			
P- O	0 / 2448	-18.5	-18.5	1	0.46 (1)	10.00			
O- N	0 / 1709	-18.5	-18.5	1	0.43 (1)	10.00			
N- M	0 / 1671	-18.5	-18.5	1	0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.83/1.00 (F-H:1) , BC=0.46/1.00 (O-Q:1) , WB=0.92/1.00 (J-M:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.70 (J) (INPUT = 1.00)

A18012437

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H06	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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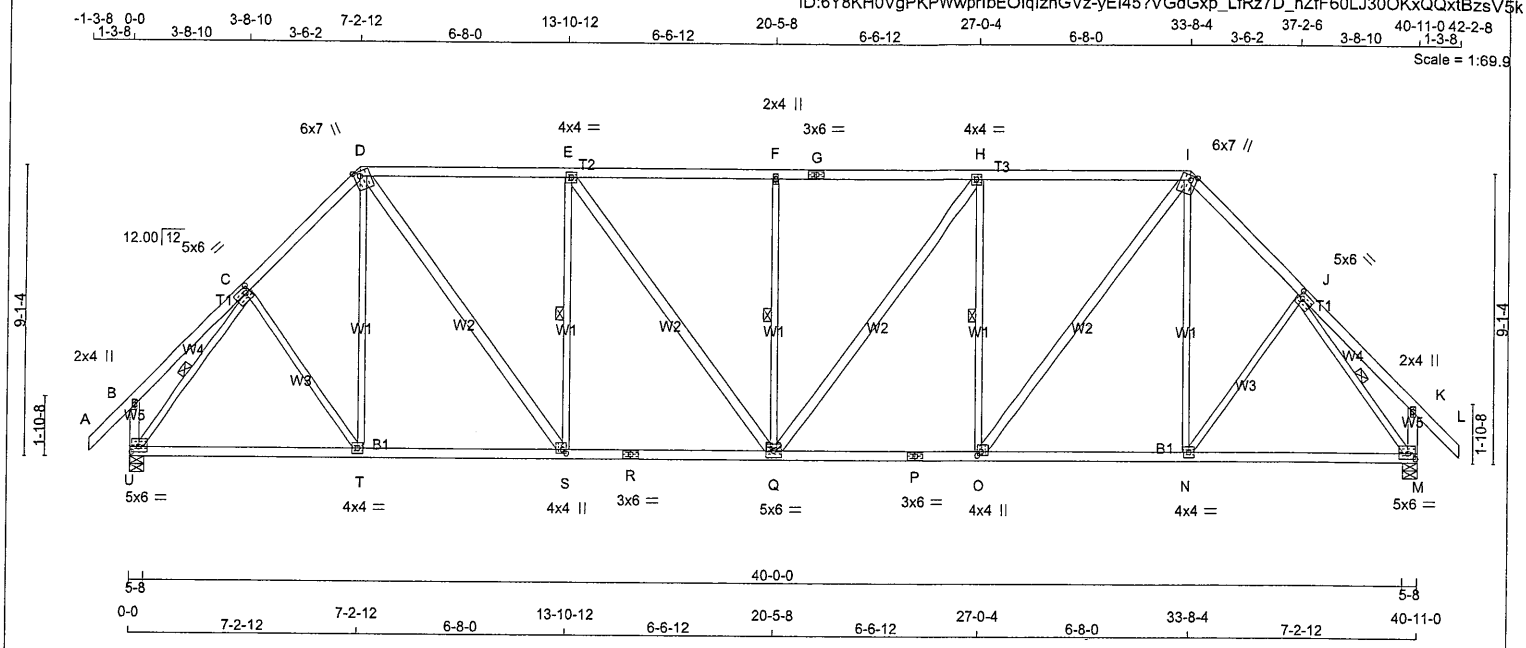

TOTAL WEIGHT = 211 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	6.0	7.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	4.0	1.50	1.50
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	4.0	1.50	1.50
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	2.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
U	2672	0	2672	0	0	5-8	3-10
M	2672	0	2672	0	0	5-8	3-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
M	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	C- T	0 / 158	0.04 (1)
B- C	0 / 29	-104.9	-104.9	0.20 (1)	10.00	T- D	0 / 98	0.03 (4)
C- D	-2474 / 0	-104.9	-104.9	0.31 (1)	4.11	D- S	0 / 1602	0.26 (1)
D- E	-2691 / 0	-104.9	-104.9	0.74 (1)	3.92	S- E	-1159 / 0	0.63 (1)
E- F	-2972 / 0	-104.9	-104.9	0.77 (1)	3.74	E- Q	0 / 471	0.08 (1)
F- G	-2972 / 0	-104.9	-104.9	0.77 (1)	3.74	Q- F	-634 / 0	0.34 (1)
G- H	-2972 / 0	-104.9	-104.9	0.77 (1)	3.74	Q- H	0 / 471	0.08 (1)
H- I	-2691 / 0	-104.9	-104.9	0.74 (1)	3.92	O- H	-1159 / 0	0.63 (1)
I- J	-2474 / 0	-104.9	-104.9	0.31 (1)	4.11	O- I	0 / 1602	0.26 (1)
J- K	0 / 29	-104.9	-104.9	0.20 (1)	10.00	N- I	0 / 98	0.03 (4)
K- L	0 / 52	-104.9	-104.9	0.15 (1)	10.00	N- J	0 / 158	0.04 (1)
U- B	-293 / 0	0.0	0.0	0.03 (1)	7.81	U- C	-2837 / 0	0.77 (1)
M- K	-293 / 0	0.0	0.0	0.03 (1)	7.81	J- M	-2837 / 0	0.77 (1)
U- T	0 / 1629	-18.5	-18.5	0.39 (1)	10.00			
T- S	0 / 1723	-18.5	-18.5	0.40 (1)	10.00			
S- R	0 / 2691	-18.5	-18.5	0.51 (1)	10.00			
R- Q	0 / 2691	-18.5	-18.5	0.51 (1)	10.00			
Q- P	0 / 2691	-18.5	-18.5	0.51 (1)	10.00			
P- O	0 / 2691	-18.5	-18.5	0.51 (1)	10.00			
O- N	0 / 1723	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 1629	-18.5	-18.5	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.77/1.00 (E-F:1) , BC=0.51/1.00 (O-Q:1) , WB=0.77/1.00 (J-M:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.73 (P) (INPUT = 1.00)

A18012438

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H05	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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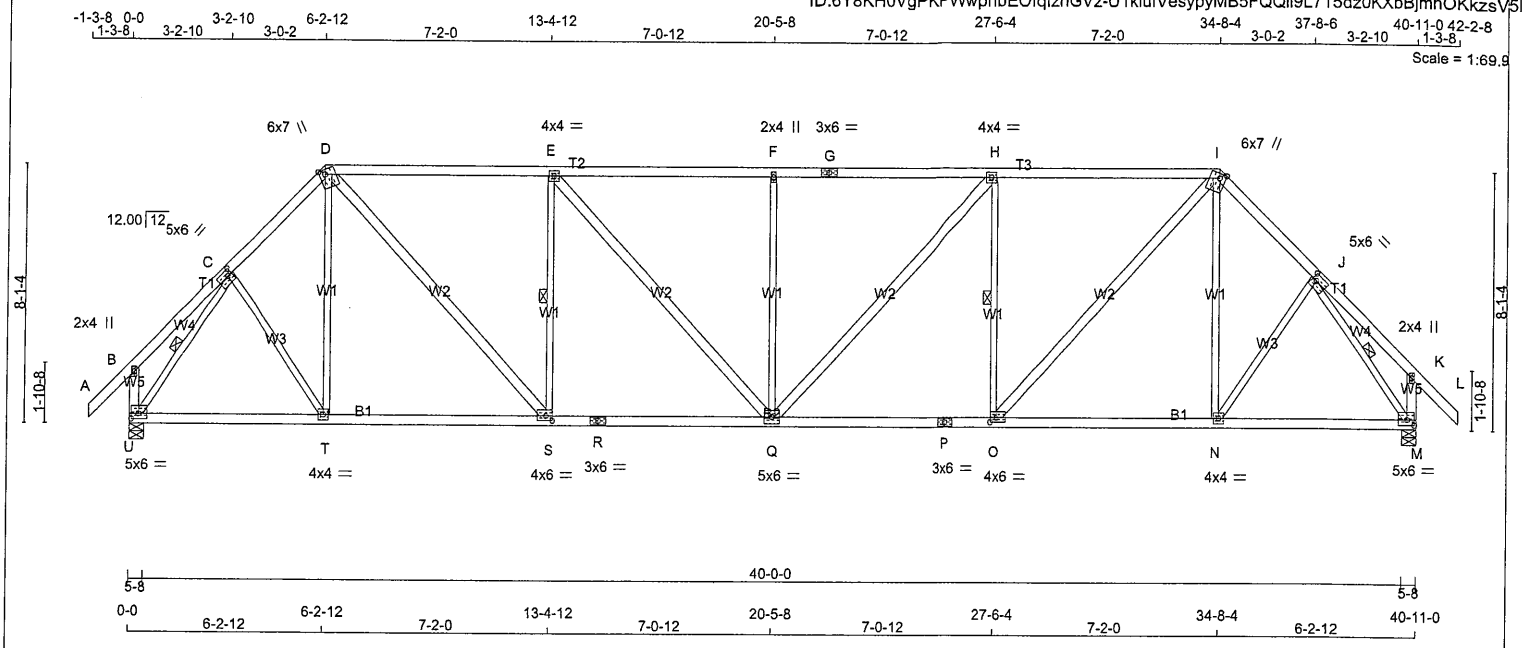


TOTAL WEIGHT = 202 lb [M]

LUMBER
N. L. G. A. RULES

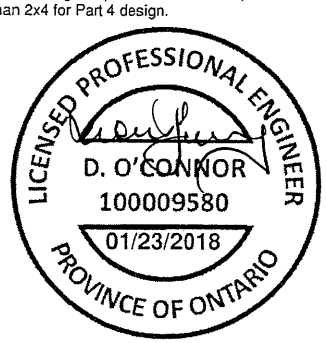
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	6.0	7.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	2.25
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0	1.75	2.50
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	6.0	1.75	2.50
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	2.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2672	0	2672	0	0	5-8	3-11
M	2672	0	2672	0	0	5-8	3-11

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
M	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	C- T	0 / 267	0.06 (1)
B- C	0 / 23	-104.9	-104.9	0.14 (1)	10.00	T- D	-81 / 70	0.10 (1)
C- D	-2474 / 0	-104.9	-104.9	0.23 (1)	4.20	D- S	0 / 1873	0.30 (1)
D- E	-2993 / 0	-104.9	-104.9	0.93 (1)	3.50	S- E	-1247 / 0	0.52 (1)
E- F	-3360 / 0	-104.9	-104.9	0.98 (1)	3.30	E- Q	0 / 547	0.09 (1)
F- G	-3360 / 0	-104.9	-104.9	0.98 (1)	3.30	Q- F	-683 / 0	0.87 (1)
G- H	-3360 / 0	-104.9	-104.9	0.98 (1)	3.30	Q- H	0 / 547	0.09 (1)
H- I	-2993 / 0	-104.9	-104.9	0.93 (1)	3.50	O- H	-1247 / 0	0.52 (1)
I- J	-2474 / 0	-104.9	-104.9	0.23 (1)	4.20	O- I	0 / 1873	0.30 (1)
J- K	0 / 23	-104.9	-104.9	0.14 (1)	10.00	N- I	-81 / 70	0.10 (1)
K- L	0 / 52	-104.9	-104.9	0.15 (1)	10.00	N- J	0 / 267	0.06 (1)
U- B	-275 / 0	0.0	0.0	0.03 (1)	7.81	U- C	-2827 / 0	0.65 (1)
M- K	-275 / 0	0.0	0.0	0.03 (1)	7.81	J- M	-2827 / 0	0.65 (1)
U- T	0 / 1571	-18.5	-18.5	0.36 (1)	10.00			
T- S	0 / 1724	-18.5	-18.5	0.38 (1)	10.00			
S- R	0 / 2993	-18.5	-18.5	0.58 (1)	10.00			
R- Q	0 / 2993	-18.5	-18.5	0.58 (1)	10.00			
Q- P	0 / 2993	-18.5	-18.5	0.58 (1)	10.00			
P- O	0 / 2993	-18.5	-18.5	0.58 (1)	10.00			
O- N	0 / 1724	-18.5	-18.5	0.38 (1)	10.00			
N- M	0 / 1571	-18.5	-18.5	0.36 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.98/1.00 (F-H:1) , BC=0.58/1.00 (Q-S:1) , WB=0.87/1.00 (F-Q:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.76 (P) (INPUT = 1.00)

A18012439

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H04	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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ID:6Y8KH0VgPKPWwprIbEOIqIznGVz-0rAKgJU05fh5k1W2siAWc8av5DcQb9S1U6xqolzsV5m

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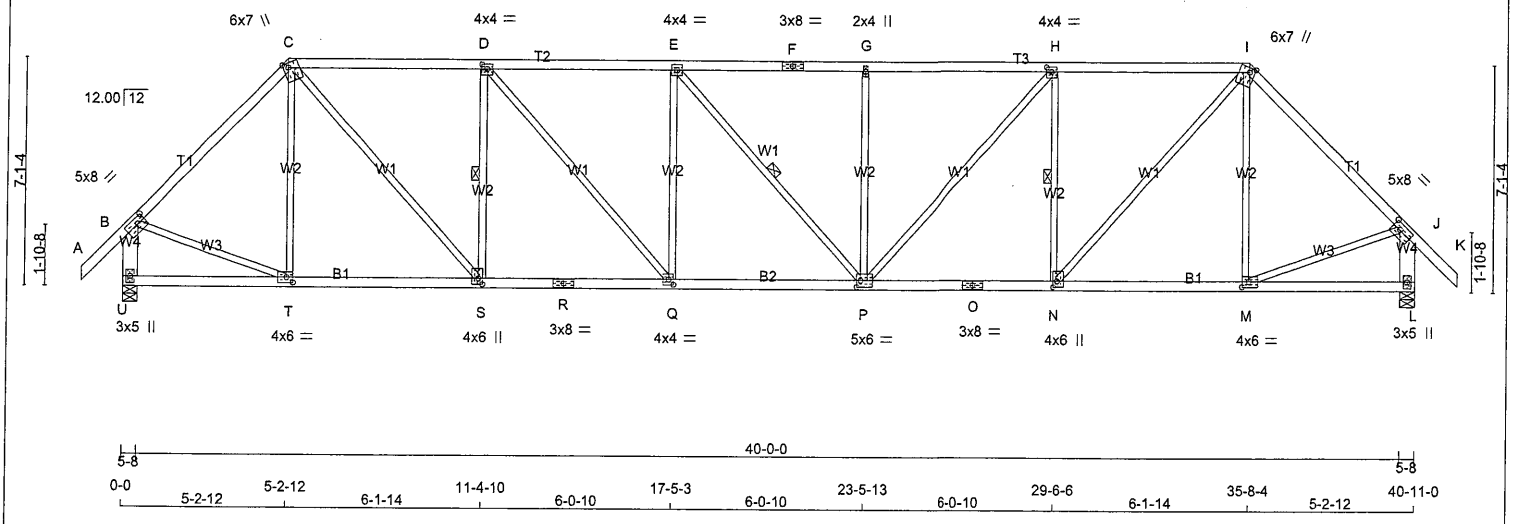
Scale = 1:69.6



5-8 | 40-0-0 | 5-8

0-0 | 5-2-12 | 5-2-12 | 6-1-14 | 11-4-10 | 6-0-10 | 17-5-3 | 6-0-10 | 23-5-13 | 6-0-10 | 29-6-6 | 6-1-14 | 35-8-4 | 5-2-12 | 40-11-0

TOTAL WEIGHT = 185 lb [M]

LUMBER
N. L. G. A. RULES

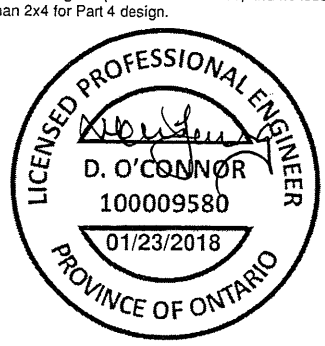
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	1650F 1.5E	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	6.0	7.0	1.75	2.00
D	TMWW-t	MT20	4.0	4.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0	1.75	2.00
I	TTWW+m	MT20	6.0	7.0	1.75	2.00
J	TMVW-t	MT20	5.0	8.0	2.25	3.00
L	BMV1+p	MT20	3.0	5.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW+t	MT20	4.0	6.0	2.25	1.50
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	5.0	6.0	2.50	1.50
Q	BMWW-t	MT20	4.0	4.0	2.00	1.75
R	BS-t	MT20	3.0	8.0		
S	BMWW+t	MT20	4.0	6.0	2.25	1.50
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2672	0	2672	0	0	5-8	3-3
L	2672	0	2672	0	0	5-8	3-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
L	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-S, E-P, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00
B- C	-2431 / 0	-104.9	-104.9	0.80 (1)	3.66
C- D	-3110 / 0	-104.9	-104.9	0.64 (1)	3.86
D- E	-3758 / 0	-104.9	-104.9	0.69 (1)	3.53
E- F	-3757 / 0	-104.9	-104.9	0.54 (1)	3.74
F- G	-3757 / 0	-104.9	-104.9	0.54 (1)	3.74
G- H	-3757 / 0	-104.9	-104.9	0.70 (1)	3.53
H- I	-3111 / 0	-104.9	-104.9	0.64 (1)	3.86
I- J	-2431 / 0	-104.9	-104.9	0.80 (1)	3.66
J- K	0 / 52	-104.9	-104.9	0.15 (1)	10.00
U- B	-2634 / 0	0.0	0.0	0.19 (1)	6.41
L- J	-2634 / 0	0.0	0.0	0.19 (1)	6.41
U- T	0 / 0	-18.5	-18.5	0.14 (4)	10.00
T- S	0 / 1708	-18.5	-18.5	0.36 (1)	10.00
S- R	0 / 3111	-18.5	-18.5	0.56 (1)	10.00
R- Q	0 / 3111	-18.5	-18.5	0.56 (1)	10.00
Q- P	0 / 3758	-18.5	-18.5	0.66 (1)	10.00
P- O	0 / 3111	-18.5	-18.5	0.56 (1)	10.00
O- N	0 / 3111	-18.5	-18.5	0.56 (1)	10.00
N- M	0 / 1708	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- C	-370 / 15	0.32 (1)
C- S	0 / 2087	0.47 (1)
S- D	-1436 / 0	0.46 (1)
D- Q	0 / 975	0.22 (1)
Q- E	-617 / 0	0.54 (1)
E- P	-2 / 0	0.00 (1)
P- G	-616 / 0	0.54 (1)
P- H	0 / 972	0.22 (1)
N- H	-1436 / 0	0.46 (1)
N- I	0 / 2088	0.47 (1)
M- I	-370 / 14	0.32 (1)
B- T	0 / 1782	0.40 (1)
M- J	0 / 1782	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.80/1.00 (B-C:1) , BC=0.66/1.00 (P-Q:1) , WB=0.54/1.00 (E-Q:1) , SSI=0.30/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 1.00)

A18012440

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H03	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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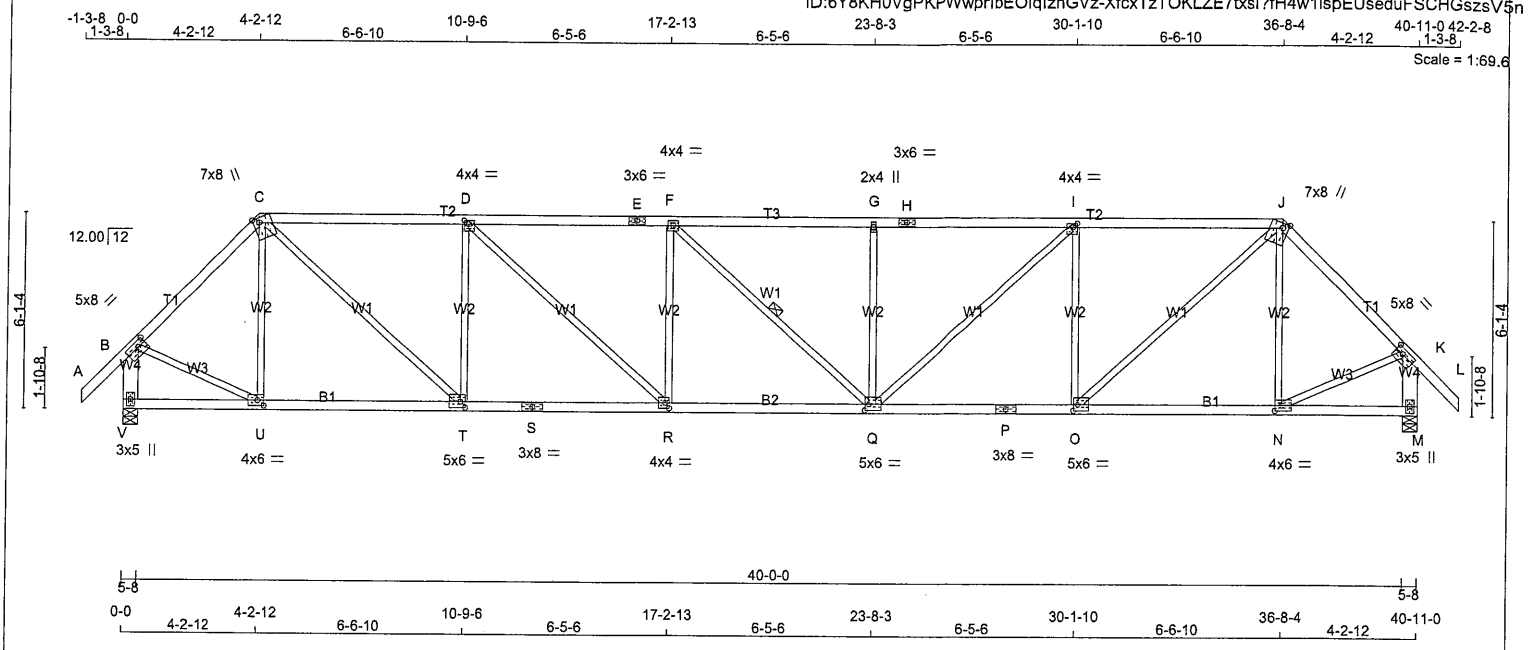

TOTAL WEIGHT = 176 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0	1.50	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	1.50	1.75
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW-t	MT20	5.0	8.0	2.00	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.50
O	BMWW-t	MT20	5.0	6.0	2.25	1.75
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	5.0	6.0	2.25	1.50
R	BMWW-t	MT20	4.0	4.0	2.00	1.50
S	BS-t	MT20	3.0	8.0		
T	BMWW-t	MT20	5.0	6.0	2.25	1.75
U	BMWW-t	MT20	4.0	6.0	2.00	2.50
V	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2672	0	2672	0	0	5-8	3-3
M	2672	0	2672	0	0	5-8	3-3

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
M	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	U- C	-474 / 0	0.28 (1)
B- C	-2379 / 0	-104.9	-104.9	0.48 (1)	4.01	C- T	0 / 2493	0.56 (1)
C- D	-3536 / 0	-104.9	-104.9	0.79 (1)	3.52	T- D	-1530 / 0	0.90 (1)
D- E	-4398 / 0	-104.9	-104.9	0.90 (1)	3.13	D- R	0 / 1160	0.26 (1)
E- F	-4398 / 0	-104.9	-104.9	0.90 (1)	3.13	R- F	-657 / 0	0.38 (1)
F- G	-4397 / 0	-104.9	-104.9	0.71 (1)	3.39	F- Q	-1 / 0	0.00 (1)
G- H	-4397 / 0	-104.9	-104.9	0.90 (1)	3.13	Q- G	-657 / 0	0.38 (1)
H- I	-4397 / 0	-104.9	-104.9	0.90 (1)	3.13	Q- I	0 / 1158	0.26 (1)
I- J	-3537 / 0	-104.9	-104.9	0.79 (1)	3.52	O- I	-1530 / 0	0.90 (1)
J- K	-2379 / 0	-104.9	-104.9	0.48 (1)	4.01	O- J	0 / 2493	0.56 (1)
K- L	0 / 52	-104.9	-104.9	0.15 (1)	10.00	N- J	-474 / 0	0.28 (1)
V- B	-2645 / 0	0.0	0.0	0.19 (1)	6.40	B- U	0 / 1777	0.40 (1)
M- K	-2645 / 0	0.0	0.0	0.19 (1)	6.40	N- K	0 / 1777	0.40 (1)
V- U	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
U- T	0 / 1668	-18.5	-18.5	0.35 (1)	10.00			
T- S	0 / 3537	-18.5	-18.5	0.64 (1)	10.00			
S- R	0 / 3537	-18.5	-18.5	0.64 (1)	10.00			
R- Q	0 / 4398	-18.5	-18.5	0.77 (1)	10.00			
Q- P	0 / 3537	-18.5	-18.5	0.64 (1)	10.00			
P- O	0 / 3537	-18.5	-18.5	0.64 (1)	10.00			
O- N	0 / 1667	-18.5	-18.5	0.35 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 900 (0.55")

CSI: TC=0.90/1.00 (G-I:1) , BC=0.77/1.00 (Q-R:1) , WB=0.90/1.00 (D-T:1) , SSI=0.32/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.95 (P) (INPUT = 1.00)

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H02	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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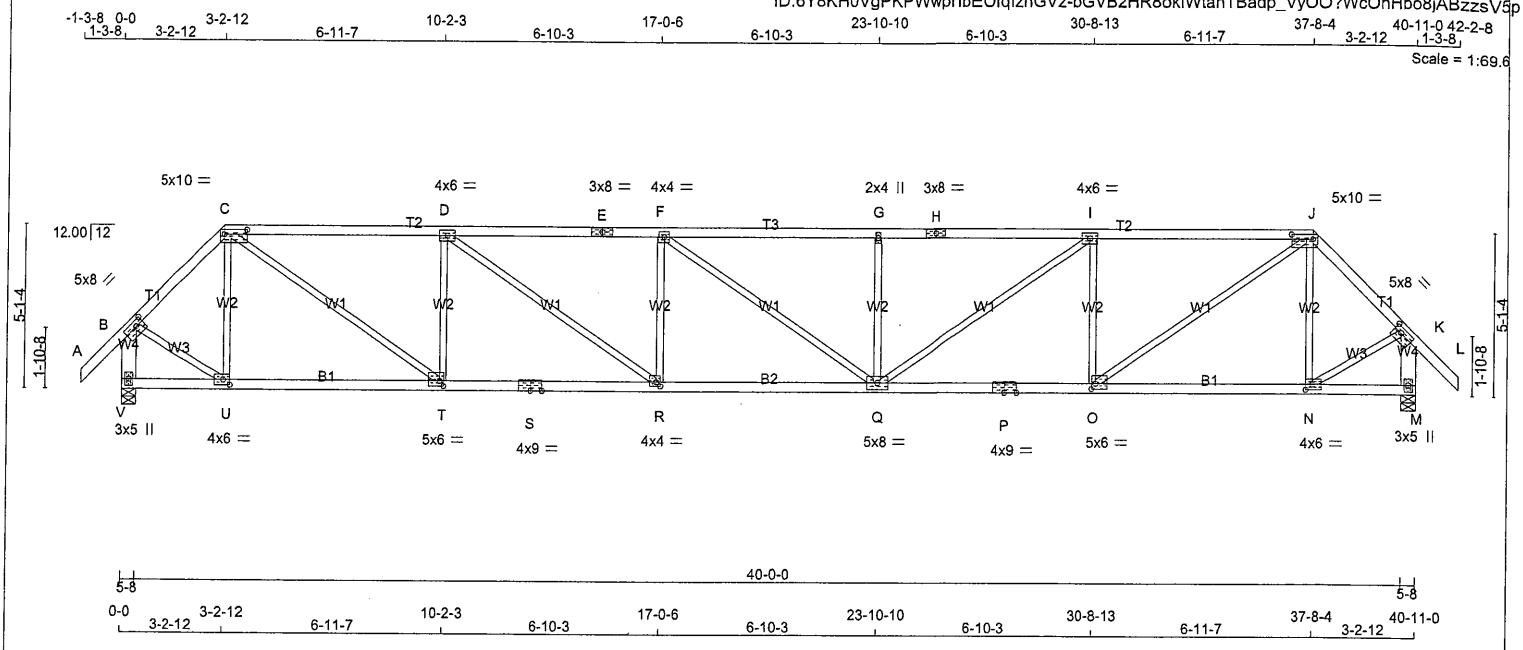

TOTAL WEIGHT = 167 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	2100F 1.8E	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.00
C	TTWW-I	MT20	5.0	10.0	1.75	8.25
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW-I	MT20	5.0	10.0	1.75	8.25
K	TMVW-t	MT20	5.0	8.0	2.00	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.50
O	BMWW-t	MT20	5.0	6.0	2.00	1.50
P	BS-t	MT20	4.0	9.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	4.0	4.0	1.50	1.50
S	BS-t	MT20	4.0	9.0		
T	BMWW-t	MT20	5.0	6.0	2.00	1.50
U	BMWW-t	MT20	4.0	6.0	2.00	2.50
V	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2672	0	2672	0	0	5-8	3-3
M	2672	0	2672	0	0	5-8	3-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
M	1853	1419 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	U- C	-614 / 0	0.24 (1)
B- C	-2273 / 0	-104.9	-104.9	0.27 (1)	4.29	C- T	0 / 3086	0.69 (1)
C- D	-4129 / 0	-104.9	-104.9	0.75 (1)	3.64	T- D	-1624 / 0	0.62 (1)
D- E	-5303 / 0	-104.9	-104.9	0.90 (1)	3.19	D- R	0 / 1435	0.32 (1)
E- F	-5303 / 0	-104.9	-104.9	0.90 (1)	3.19	R- F	-699 / 0	0.27 (1)
F- G	-5304 / 0	-104.9	-104.9	0.72 (1)	3.40	F- Q	0 / 0	0.00 (1)
G- H	-5304 / 0	-104.9	-104.9	0.90 (1)	3.19	Q- G	-699 / 0	0.27 (1)
H- I	-5304 / 0	-104.9	-104.9	0.90 (1)	3.19	Q- I	0 / 1435	0.32 (1)
I- J	-4129 / 0	-104.9	-104.9	0.75 (1)	3.64	O- I	-1624 / 0	0.62 (1)
J- K	-2273 / 0	-104.9	-104.9	0.27 (1)	4.29	O- J	0 / 3086	0.69 (1)
K- L	0 / 52	-104.9	-104.9	0.15 (1)	10.00	N- J	-614 / 0	0.24 (1)
V- B	-2661 / 0	0.0	0.0	0.20 (1)	6.39	B- U	0 / 1759	0.40 (1)
M- K	-2661 / 0	0.0	0.0	0.20 (1)	6.39	N- K	0 / 1759	0.40 (1)
V- U	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
U- T	0 / 1586	-18.5	-18.5	0.35 (1)	10.00			
T- S	0 / 4129	-18.5	-18.5	0.74 (1)	10.00			
S- R	0 / 4129	-18.5	-18.5	0.74 (1)	10.00			
R- Q	0 / 5303	-18.5	-18.5	0.93 (1)	10.00			
Q- P	0 / 4129	-18.5	-18.5	0.74 (1)	10.00			
P- O	0 / 4129	-18.5	-18.5	0.74 (1)	10.00			
O- N	0 / 1586	-18.5	-18.5	0.35 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 708 (0.69")

CSI: TC=0.90/1.00 (D-F:1) , BC=0.93/1.00 (Q-R:1) , WB=0.69/1.00 (C-T:1) , SSI=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.83 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H01	1	1	TRUSS DESC.	JT 38514

Alpa Roof Truss, Maple

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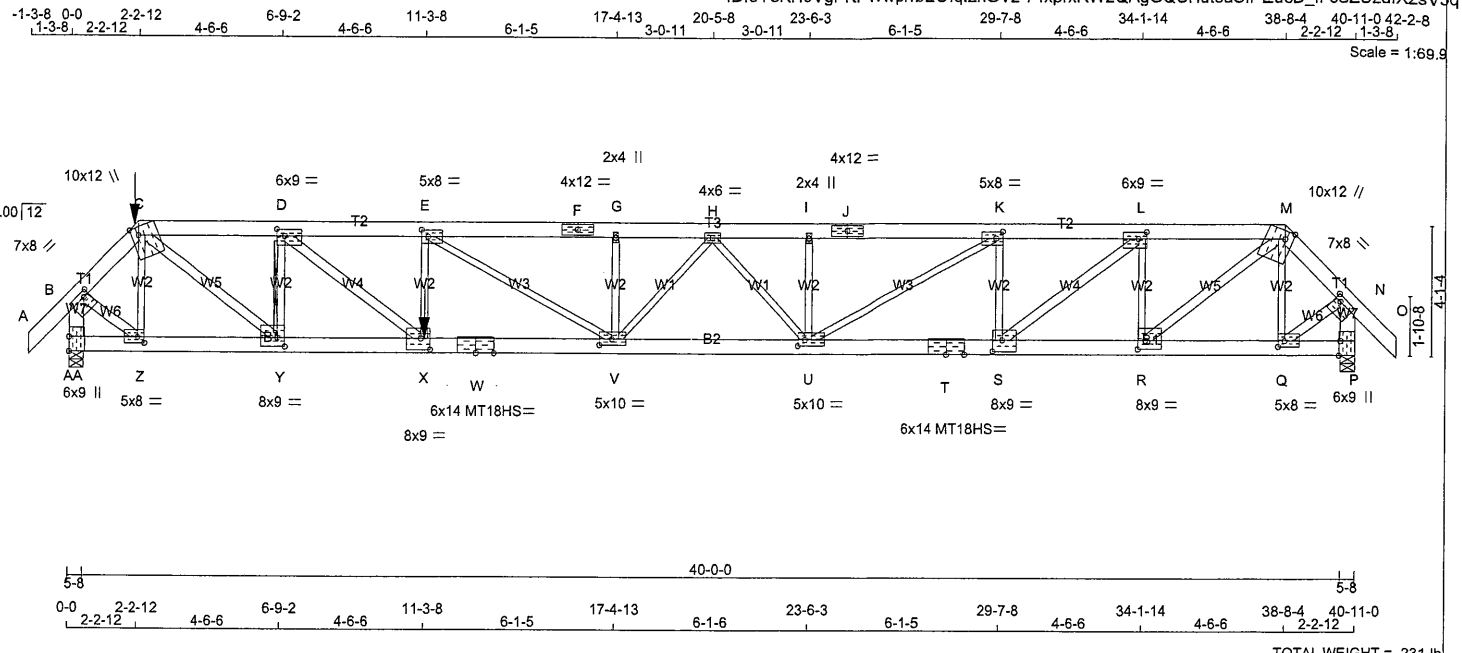

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - J	2x6	DRY	1650F 1.5E	SPF
J - M	2x6	DRY	1650F 1.5E	SPF
M - O	2x6	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- W	2x6	DRY	2100F 1.8E	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - Y	2x4	DRY	1650F 1.5E	SPF
D - X	2x4	DRY	No.2	SPF
S - L	2x4	DRY	No.2	SPF
R - M	2x4	DRY	1650F 1.5E	SPF
B - Z	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0	2.00	2.25
C	TTWW+m	MT20	10.0	12.0	Edge	3.00
D	TMWW-t	MT20	6.0	9.0	2.50	3.00
E	TMWW-t	MT20	5.0	8.0	2.50	2.50
F	TS-t	MT20	4.0	12.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TS-t	MT20	4.0	12.0		
K	TMWW-t	MT20	5.0	8.0	2.50	2.50
L	TMWW-t	MT20	6.0	9.0	2.50	3.00
M	TTWW+m	MT20	10.0	12.0	Edge	3.00
N	TMVW-t	MT20	7.0	8.0	2.00	2.25
P	BMV1+t	MT20	6.0	9.0	Edge	0.50
Q	BMWW-t	MT20	5.0	8.0	2.25	2.50
R	BMWW-t	MT20	8.0	9.0	3.25	2.75
S	BMWW-t	MT20	8.0	9.0	4.25	3.50
T	BS-t	MT18HS	6.0	14.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AA	5851	0	5851	0	0	5-8	5-8
P	3656	0	3656	0	0	5-8	4-3

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	4063	3088 / 0	0 / 0	0 / 0	0 / 0	976 / 0	0 / 0
P	2537	1938 / 0	0 / 0	0 / 0	0 / 0	599 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P
BEARING SIZE FACTOR = 1.15 AT JNT(S) AA, P (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-Y

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 54	-104.9	-104.9	0.09 (1)		10.00
B- C	-5060 / 0	-104.9	-104.9	0.18 (1)		3.74
C- D	-9259 / 0	-206.6	-206.6	0.47 (1)		3.06
D- E	-13568 / 0	-206.6	-206.6	0.74 (1)		2.34
E- F	-13252 / 0	-104.9	-104.9	0.82 (1)		2.35
F- G	-13252 / 0	-104.9	-104.9	0.82 (1)		2.35
G- H	-13252 / 0	-104.9	-104.9	0.57 (1)		2.52
H- I	-11562 / 0	-104.9	-104.9	0.46 (1)		2.79
I- J	-11562 / 0	-104.9	-104.9	0.65 (1)		2.66
J- K	-11562 / 0	-104.9	-104.9	0.65 (1)		2.66
K- L	-8760 / 0	-104.9	-104.9	0.37 (1)		3.25
L- M	-5737 / 0	-104.9	-104.9	0.21 (1)		4.05
M- N	-3078 / 0	-104.9	-104.9	0.10 (1)		4.72
N- O	0 / 54	-104.9	-104.9	0.09 (1)		10.00
AA- B	-5890 / 0	0.0	0.0	0.44 (1)		4.37
P- N	-3689 / 0	0.0	0.0	0.27 (1)		5.52
AA- Z	0 / 0	-36.4	-36.4	0.08 (1)		10.00
Z- Y	0 / 3489	-36.4	-36.4	0.26 (1)		10.00
Y- X	0 / 9259	-36.4	-36.4	0.60 (1)		10.00
X- W	0 / 13568	-18.5	-18.5	0.83 (1)		10.00
W- V	0 / 13568	-18.5	-18.5	0.83 (1)		10.00
V- U	0 / 12503	-18.5	-18.5	0.74 (1)		10.00
U- T	0 / 8760	-18.5	-18.5	0.54 (1)		10.00
T- S	0 / 8760	-18.5	-18.5	0.54 (1)		10.00
S- R	0 / 5737	-18.5	-18.5	0.33 (1)		10.00
R- Q	0 / 2123	-18.5	-18.5	0.16 (1)		10.00
Q- P	0 / 0	-18.5	-18.5	0.05 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Z- C	-1751 / 0	0.42 (1)
C- Y	0 / 7368	0.94 (1)
Y- D	-4415 / 0	0.79 (1)
D- X	0 / 5530	0.98 (1)
X- E	-562 / 0	0.13 (1)
E- V	-368 / 0	0.35 (1)
V- G	-532 / 0	0.13 (1)
V- H	0 / 1165	0.29 (1)
H- U	-1465 / 0	0.54 (1)
U- I	-488 / 0	0.12 (1)
U- K	0 / 3263	0.81 (1)
S- K	-2287 / 0	0.55 (1)
S- L	0 / 3880	0.69 (1)
R- L	-2847 / 0	0.68 (1)
R- M	0 / 4616	0.59 (1)
Q- M	-1060 / 0	0.25 (1)
B- Z	0 / 4068	0.72 (1)
Q- N	0 / 2475	0.44 (1)

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DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 11-3-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 647 (0.76")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 392 (1.25")

CSI: TC=0.82/1.00 (E-G:1) , BC=0.83/1.00 (V-X:1) , WB=0.98/1.00 (D-X:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
264490	H01	1	1	TRUSS DESC.	JT 38514

PLATES (table is in inches)

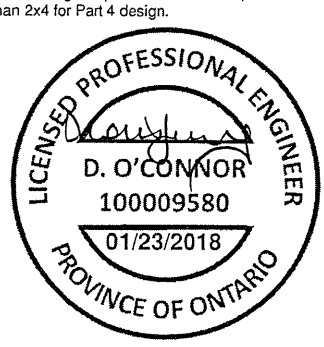
JT	TYPE	PLATES	W	LEN	Y	X
U	BMWWW-t	MT20	5.0	10.0	2.25	2.75
V	BMWWW-t	MT20	5.0	10.0	2.25	4.75
W	BS-t	MT18HS	6.0	14.0		
X	BMWW-t	MT20	8.0	9.0	4.25	3.50
Y	BMWW-t	MT20	8.0	9.0	3.25	2.75
Z	BMWW-t	MT20	5.0	8.0	2.25	2.50
AA	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 170.6 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD, AND 2857.1 lbs FACTORED DOWN AT 11-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-12	-171	-171	---	FRONT	VERT	TOTAL
X	11-3-8	-2857	-2857	---	FRONT	VERT	TOTAL

MT18HS 580 354 2729 1405 4191 2010

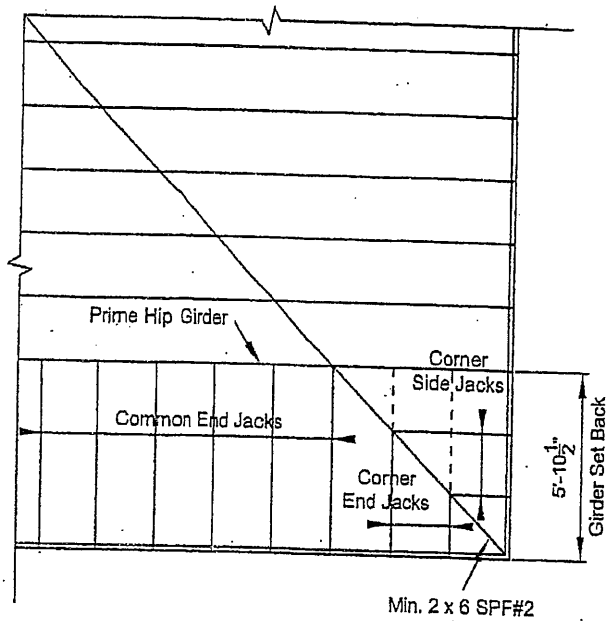
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 1.00 (W) (INPUT = 1.00)

A18012443(2)

STRACON ENGINEERING INC.



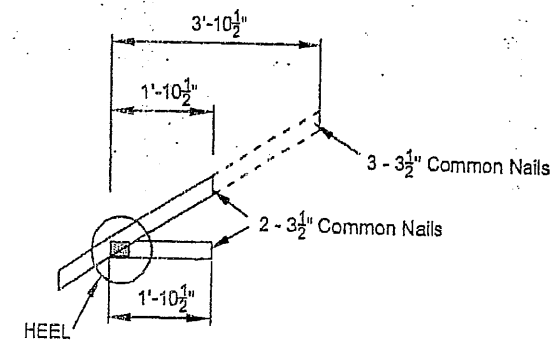
45° Hip End

LUMBER SPECIFICATION

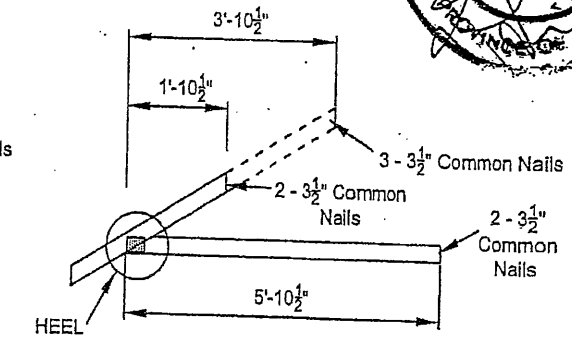
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

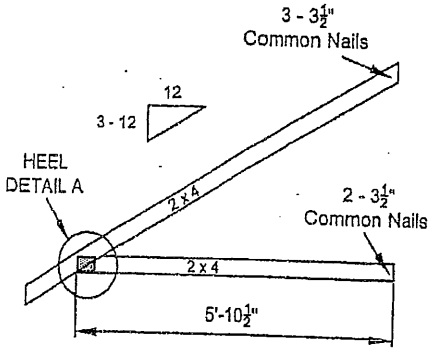
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



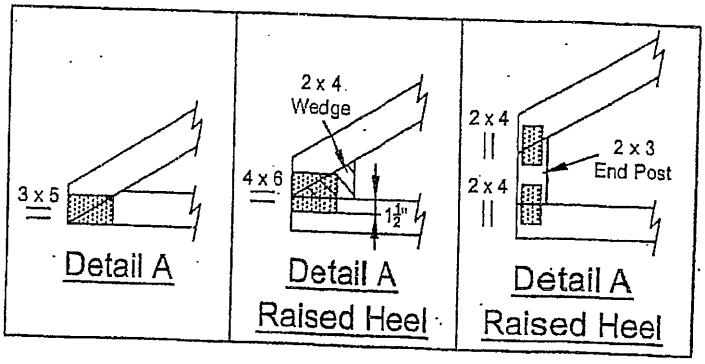
Corner Side Jacks



Corner End Jacks



Common End Jacks

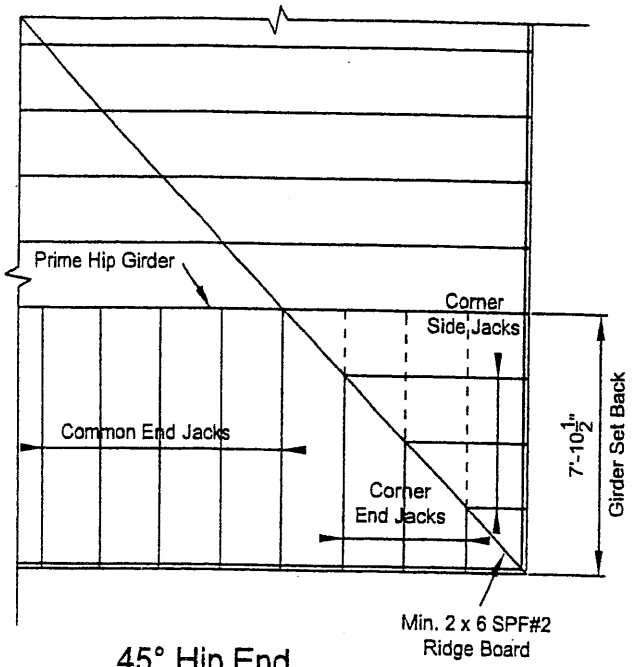


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

SITE COPY

STRACON ENGINEERING INC.



45° Hip End

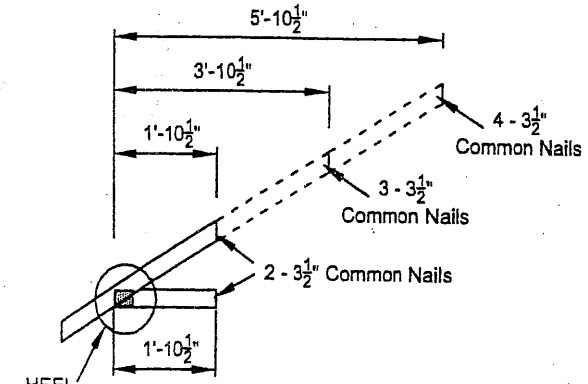
LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

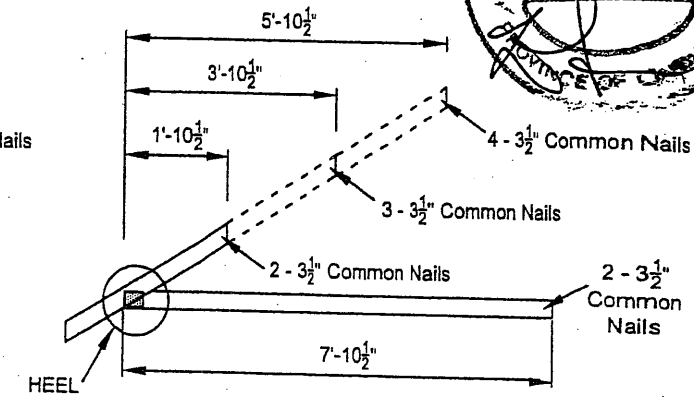
UNLESS OTHERWISE SHOWN

DESIGN LOAD

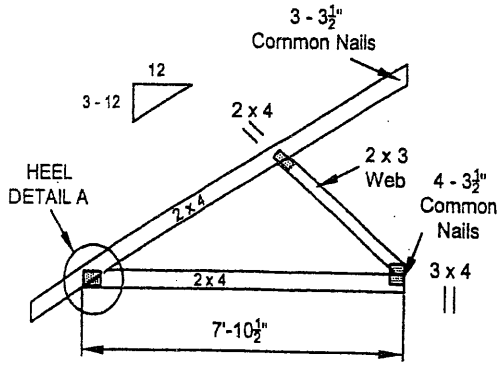
TOP CHORD SNOW LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F



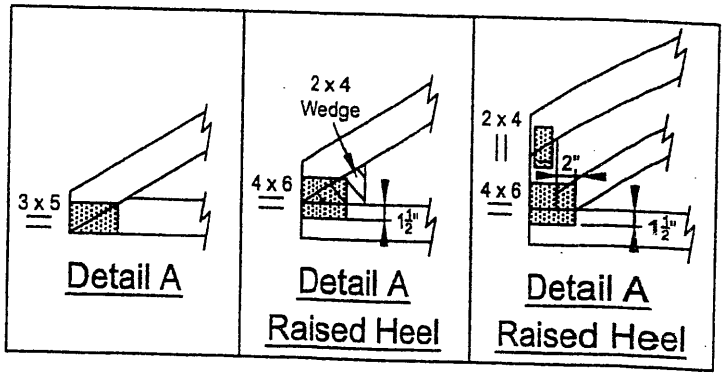
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N

SITE COPY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

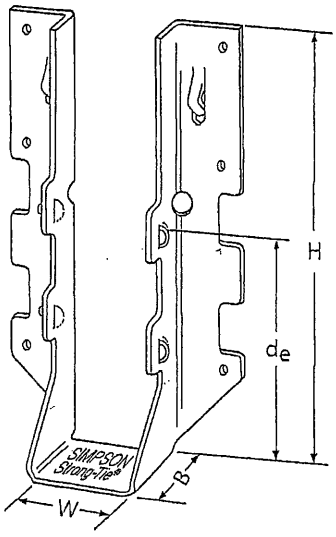
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

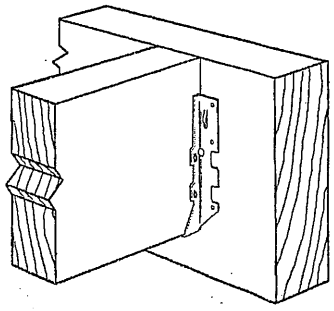
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

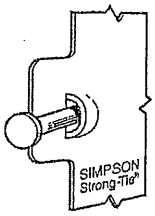


LUS28


Typical LUS Installation

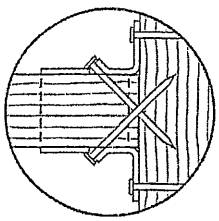
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3⅛	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4 13/16	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 13/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

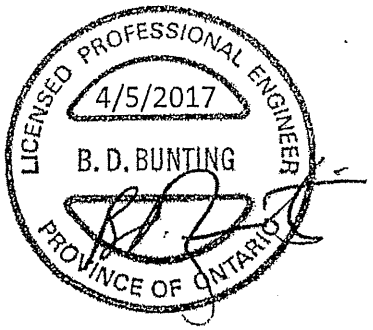
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

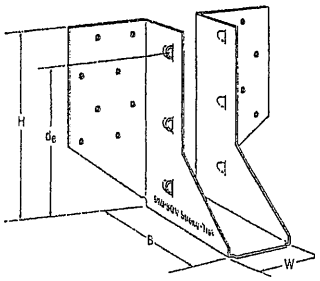
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

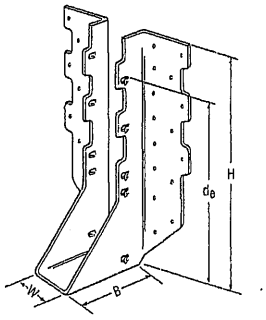
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

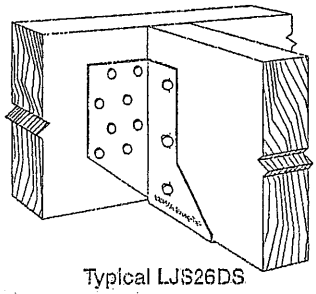
- See current catalogue for options



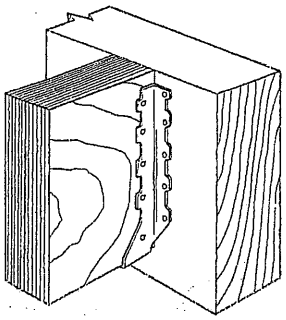
LJS26DS



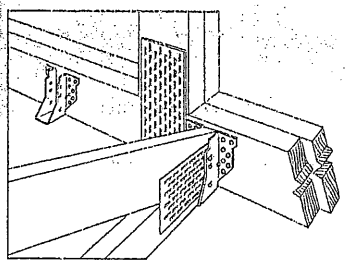
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



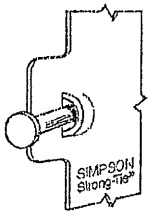
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

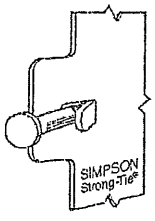
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

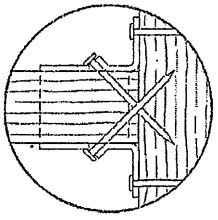


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

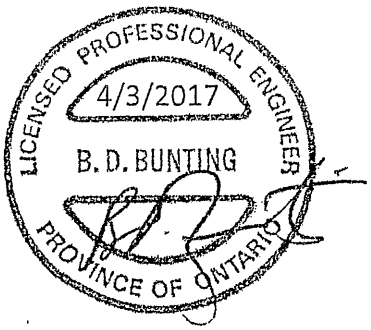
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

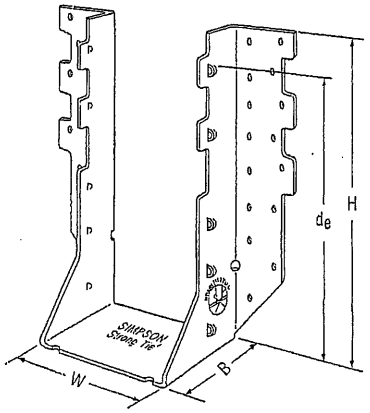
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

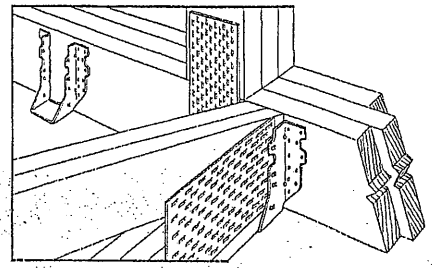
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

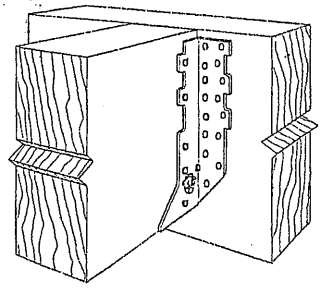

Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

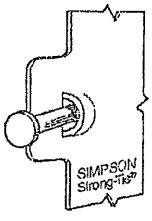
Options:

- See current catalogue for options

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

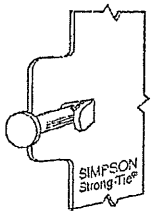
1. d_e is the distance from the seat of the hanger to the highest joist nail.


Typical HHUS Installation

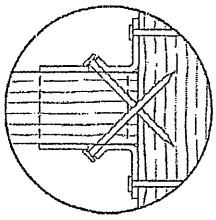


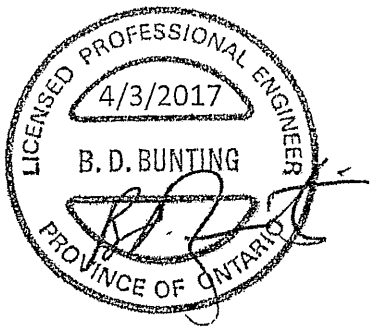
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

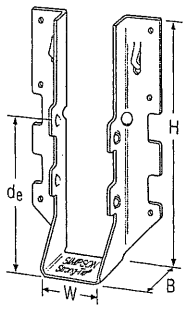
FACE MOUNT HANGERS – SOLID SAWN LUMBER

SIMPSON Strong-Tie®

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Joist Size	Model No.	Ga	Dimensions (in) W	H	B	d_e[6]	Fasteners Min/Max	Header	Joist	Factored Resistance D.Fir-L Uplift (K_D=1.15) lbs / kN	D.Fir-L Normal (K_D=1.00) lbs / kN	S-P-F Uplift (K_D=1.15) lbs / kN	S-P-F Normal (K_D=1.00) lbs / kN	Installed Cost Index
SAWN LUMBER SIZES														
2x4	LU24L	22	1 9/16	3 1/8	1 5/8	2 11/16	—	4-10d	2-10dx1 1/2	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22	Lowest
	LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	—	4-10d	2-10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14	+11%
	U24	16	1 9/16	3 1/8	1 1/2	1 13/16	—	4-10d	2-10dx1 1/2	450 / 2.00	1340 / 5.96	355 / 1.58	1030 / 4.58	+90%
	HU26	14	1 9/16	3 1/16	2 1/4	1 1/16	—	4-16d	2-10dx1 1/2	490 / 2.18	1525 / 6.78	450 / 2.00	1080 / 4.80	+490%
DBL 2x4	LUS24-2	18	3 1/8	3 1/8	2	1 17/32	—	4-16d	2-16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38	Lowest
	U24-2	16	3 1/8	3	2	1 15/16	—	4-10d	2-10d	480 / 2.14	1340 / 5.96	445 / 1.98	1030 / 4.58	+59%
	HU24-2/ HUC24-2	14	3 1/8	3 1/16	2 1/2	2 11/16	—	4-16d	2-10d	525 / 2.34	1710 / 7.61	490 / 2.18	1585 / 7.05	+244%
2x6	LU26L	22	1 9/16	5	1 5/8	4 19/32	—	6-10d	4-10dx1 1/2	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07	Lowest
	LUS26	18	1 9/16	4 3/4	1 3/4	3 25/32	—	4-10d	4-10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25	+10%
	U26	16	1 9/16	4 3/4	2	3 15/16	—	6-10d	4-10dx1 1/2	895 / 3.98	2005 / 8.92	780 / 3.47	1860 / 8.27	+70%
	LUC26Z	18	1 9/16	4 3/4	1 3/4	4	—	6-10d	4-10dx1 1/2	830 / 3.69	1605 / 7.14	710 / 3.16	1140 / 5.07	+74%
	HUS26	16	1 5/8	5 3/8	3	3 15/16	—	14-16d	6-16d	2705 / 12.03	4940 / 21.97	2065 / 9.20	3875 / 17.24	+320%
DBL 2x6	LU26-2L	20	3 1/8	5	1 5/8	4 19/32	—	6-10d	4-10dx1 1/2	760 / 3.38	1605 / 7.14	680 / 3.02	1140 / 5.07	Lowest
	LUS26-2	18	3 1/8	4 7/8	2	4 1/32	—	4-16d	4-16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54	+24%
	U26-2	16	3 1/8	5	2	3 13/16	—	8-10d	4-10d	960 / 4.27	2675 / 11.90	890 / 3.96	2475 / 11.01	+124%
	HU26-2/ HUC26-2	14	3 1/8	5 3/8	2 1/2	5	Min	8-16d	4-10d	1055 / 4.69	3420 / 15.21	980 / 4.36	2845 / 12.66	+358%
							Max	12-16d	6-10d	1580 / 7.03	4415 / 19.64	1470 / 6.54	3135 / 13.95	+372%
TPL 2x6	LUS26-3	18	4 5/8	4 1/8	2	3 9/32	—	4-16d	4-16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	2340 / 10.41	Lowest
	U26-3	16	4 5/8	4 1/4	2	3 11/16	—	8-10d	4-10d	960 / 4.27	2675 / 11.90	890 / 3.96	2475 / 11.01	+87%
	HU26-3/ HUC26-3	14	4 11/16	5 3/8	2 1/2	5	Min	8-16d	4-10d	1055 / 4.69	3420 / 15.21	980 / 4.36	2845 / 12.66	+193%
							Max	12-16d	6-10d	1580 / 7.03	4415 / 19.64	1470 / 6.54	3135 / 13.95	+198%
2x8	LU26L	22	1 9/16	5	1 5/8	4 19/32	—	6-10d	4-10dx1 1/2	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07	Lowest
	LUS26	18	1 9/16	4 3/4	1 3/4	3 25/32	—	4-10d	4-10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25	+10%
	LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	—	8-10d	6-10dx1 1/2	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89	+29%
	LUS28	18	1 9/16	6 5/8	1 3/4	3 25/32	—	6-10d	4-10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96	+42%
	U26	16	1 9/16	4 3/4	2	3 15/16	—	6-10d	4-10dx1 1/2	895 / 3.98	2005 / 8.92	780 / 3.47	1860 / 8.27	+70%
	LUC26Z	18	1 9/16	4 3/4	1 3/4	4	—	6-10d	4-10dx1 1/2	830 / 3.69	1605 / 7.14	710 / 3.16	1140 / 5.07	+70%
	HU28	14	1 9/16	5 1/4	2 1/4	4 7/8	—	6-16d	4-10dx1 1/2	980 / 4.36	2565 / 11.41	905 / 4.03	2380 / 10.59	+415%
	HUS28	16	1 5/8	7 3/32	3	6 3/32	—	22-16d	8-16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33	+457%



1. 10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.
2. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as in cantilever construction.
3. MIN nailing quantity and factored resistances—fill all round holes; MAX nailing quantity and factored resistances—fill all round and triangle holes.
4. D.Fir-L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.
5. See page 22 for hangers with reduced capacity due to installation with different nails.
6. d_e is the distance from the bearing seat to the top joist nail.
7. **NAILS:** 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long. See pages 22-23 for other nail sizes and information.

**Hangers do not have an Installed Cos*