ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

DENOTES:

ALL B -2-2X10 FLUSH

* - ALL FIRST FLOOR PLATE HEIGHT 7'-10"-0 U/S @ PLATE LEVEL

V -6/12 VAULT CEILING (1'-5"-0 HIGH A.P.P)

@ -1'-4"-0 HIGHER FASCIA ONLY

HARDWARE:

LUS26-2 -(X)
LJS26DS -(V)
LUS24 -(O)

T-180690

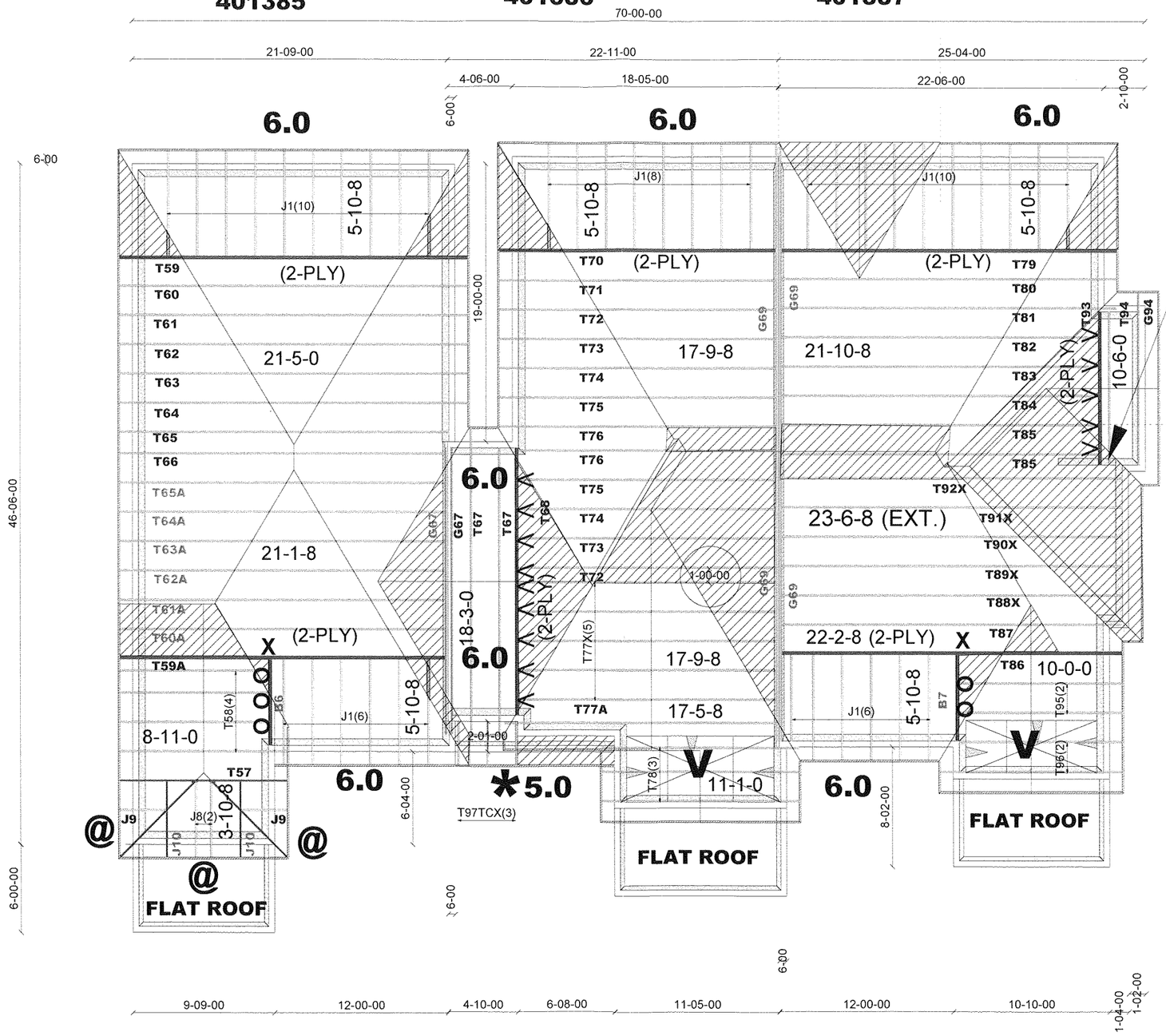
DENOTES:
CONVENTIONAL
FRAMING

TH-5E
EL:B UNIT21
401385

TH-2MOD
EL:B2(REV) UNIT22
401386

TH-8C
EL:B (REV) UNIT23
401387

10/12 PITCHES (TYP.)
UNLESS NOTED



2X6 LOAD
BRG. WALL

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACE DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track:	50264
Plan Log:	200510
Layout ID:	401384

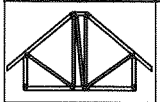
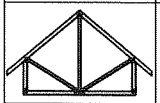
Builder / Location: **BAYVIEW WELLINGTON / ST CATHERINES**
Model / Elevation: FREEHOLD BLOCK 5 / UNITS 21 - 23
Mitek ver 8.2 3.229
Project: **PASSAGE ON THE CANAL**
Date: 2/12/2019 | Sales: Mario DiCano | Designer: AC

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401385
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 21	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

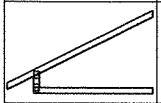
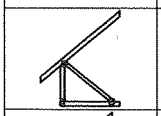
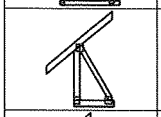
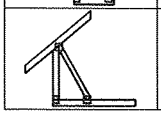
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T57 Hip Girder	10 /12	8-11-00	6-02-07	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	54.13 34.67		
	4	T58 Common	10 /12	8-11-00	6-08-04	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	185.24 116.67		
	1 2-ply	T59 Hip Girder	10 /12	21-05-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	203.37 126.67		
	1 2-ply	T59A Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	197.92 124.00		
	1	T60 Hip	10 /12	21-05-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	89.05 58.50		
	1	T60A Hip	10 /12	21-01-08	5-01-04	2 x 4	1-03-08	1-07-11 1-07-11	86.49 56.67		
	1	T61 Hip	10 /12	21-05-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.83 60.33		
	1	T61A Hip	10 /12	21-01-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	91.25 58.50		
	1	T62 Hip	10 /12	21-05-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	104.61 67.17		
	1	T62A Hip	10 /12	21-01-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	102.07 65.83		
	1	T63 Hip	10 /12	21-05-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	99.52 64.33		
	1	T63A Hip	10 /12	21-01-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	96.97 62.33		
	1	T64 Hip	10 /12	21-05-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.02 70.33		
	1	T64A Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 1-07-11	106.45 69.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401385
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 21	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T65A Hip	10 /12	21-01-08	10-01-04	2 x 4	1-03-08	1-07-11 1-07-11	112.3 73.33		
	1	T66 Common	10 /12	21-01-08	10-06-12	2 x 4	1-03-08	1-07-11 1-10-10	106.36 66.67		
	1	G67 GABLE	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	16	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J8 Jack-Open	10 /12	3-10-08	6-02-07	2 x 4	1-03-08	2-11-11 6-02-07	37.1 23.67		
	2	J9 Jack-Open	10 /12	1-09-07	4-05-09	2 x 4	1-03-08 1-01	2-11-11 4-05-09	24.58 16.33		
	2	J10 Jack-Open Girder	10 /12	1-09-07	4-05-09	2 x 4	1-03-08 2-01-01	2-11-11 4-05-09	29.13 19.00		

TOTAL # TRUSS= **44** TOTAL BFT OF ALL TRUSSES= **1453.67** BFT. TOTAL WEIGHT OF ALL TRSSES **2268.91** LBS

HARDWARE

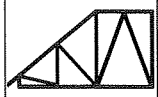
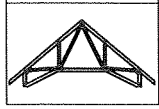
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **4**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401386
Location:	ST CATHERINES	Ref #	
Model:	TH-2MOD	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	A UNIT 22	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

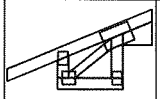
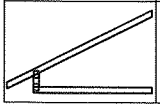
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T67 Common	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.14 94.67		
	1	G67 GABLE	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	1 2-ply	T68 Common Girder	6 /12	18-03-00	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	182.58 115.33		
	2	G69 GABLE	6 /12	19-09-00	11-00-08	2 x 4	1-03-08	1-02-00 11-00-08	204.67 128.67		
	1 2-ply	T70 Half Hip Girder	10 /12	17-09-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	148.81 94.33		
	1	T71 Half Hip	10 /12	17-09-08	5-02-00	2 x 4	1-03-08	1-07-11 5-02-00	74.52 46.83		
	2	T72 Half Hip	10 /12	17-09-08	6-02-00	2 x 4	1-03-08	1-07-11 6-02-00	157.86 100.00		
	2	T73 Half Hip	10 /12	17-09-08	7-02-00	2 x 4	1-03-08	1-07-11 7-02-00	172.72 110.00		
	2	T74 Half Hip	10 /12	17-09-08	8-02-00	2 x 4	1-03-08	1-07-11 8-02-00	183.15 114.00		
	2	T75 Half Hip	10 /12	17-09-08	9-02-00	2 x 4	1-03-08	1-07-11 9-02-00	198.73 124.33		
	2	T76 Half Hip	10 /12	17-09-08	10-02-00	2 x 4	1-03-08	1-07-11 10-02-00	208.2 130.33		
	1	T77A Common	10 /12	17-05-08	9-00-07	2 x 4	1-03-08	1-07-11 1-10-10	83.97 54.67		
	5	T77X Common	10 /12	17-09-08	9-00-07	2 x 4		1-04-06 1-10-10	413.96 266.67		
	3	T78 Roof Special	10 /12 6 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	162.57 110.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401386
Location:	ST CATHERINES	Ref #	
Model:	TH-2MOD	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	A UNIT 22	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T97TCX Monopitch	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	34.58 27.00		
	8	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	134.36 85.33		

TOTAL # TRUSS= **40** TOTAL BFT OF ALL TRUSSES= **1650.49** BFT. TOTAL WEIGHT OF ALL TRSSES **2576.59** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LJS26DS	

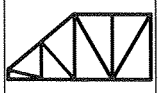
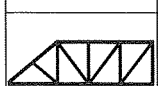
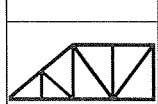
TOTAL NUMBER OF ITEMS= **9**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401387
Location:	ST CATHERINES	Ref #	
Model:	TH-8C	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 23	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

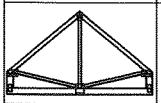
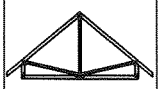
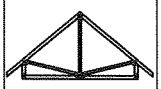
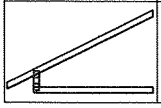
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	G69 GABLE	6 /12	**19-09-00**	11-00-08	2 x 4	1-03-08	1-02-00 11-00-08	204.67 128.67		
	1 2-ply	T79 Half Hip Girder	10 /12	**21-10-08**	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	181.67 115.00		
	1	T80 Half Hip	10 /12	**21-10-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	91.98 58.67		
	1	T81 Half Hip	10 /12	**21-10-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	97.87 61.33		
	1	T82 Half Hip	10 /12	**21-10-08**	7-01-04	2 x 4		1-07-11 7-01-04	95.74 60.00		
	1	T83 Half Hip	10 /12	**21-10-08**	8-01-04	2 x 4		1-07-11 8-01-04	107.2 68.67		
	1	T84 Half Hip	10 /12	**21-10-08**	9-01-04	2 x 4		1-07-11 9-01-04	114.93 72.17		
	2	T85 Half Hip	10 /12	**21-10-08**	10-01-04	2 x 4		1-07-11 10-01-04	241.59 151.00		
	1 2-ply	T86 Half Hip Girder	10 /12	**22-02-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	209.95 135.00		
	1	T87 Half Hip	10 /12	**22-02-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	92.96 60.00		
	1	T88X Half Hip	10 /12	**23-06-08**	6-01-04	2 x 4		6-06 6-01-04	95.17 59.00		
	1	T89X Half Hip	10 /12	**23-06-08**	7-01-04	2 x 4		6-06 7-01-04	104.5 67.67		
	1	T90X Half Hip	10 /12	**23-06-08**	8-01-04	2 x 4		6-06 8-01-04	109.36 70.33		
	1	T91X Half Hip	10 /12	**23-06-08**	9-01-04	2 x 4		6-06 9-01-04	114.48 72.33		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200510
Project:	PASSAGE ON THE CANAL	Layout ID:	401387
Location:	ST CATHERINES	Ref #	
Model:	TH-8C	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 23	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T92X Half Hip	10 /12	**23-06-08**	10-01-04	2 x 4		6-06 10-01-04	119.85 76.50		
	1 2-ply	T93 Common Girder	10 /12	**10-06-00**	6-00-03	2 x 4 2 x 6		1-07-11 1-07-11	102.58 69.33		
	1	T94 Common	10 /12	**10-06-00**	6-00-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	47.21 30.00		
	1	G94 GABLE	10 /12	**10-06-00**	6-00-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	51.04 34.00		
	2	T95 Common	10 /12	**10-00-00**	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.67 58.67		
	2	T96 Roof Special	10 /12 6 /12	**10-00-00**	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	100.63 69.33		
	16	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		

TOTAL # TRUSS= **43** TOTAL BFT OF ALL TRUSSES= **1688.34** BFT. TOTAL WEIGHT OF ALL TRSSES **2642.78** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
2	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **8**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T57	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:31:12 2019 Page 1

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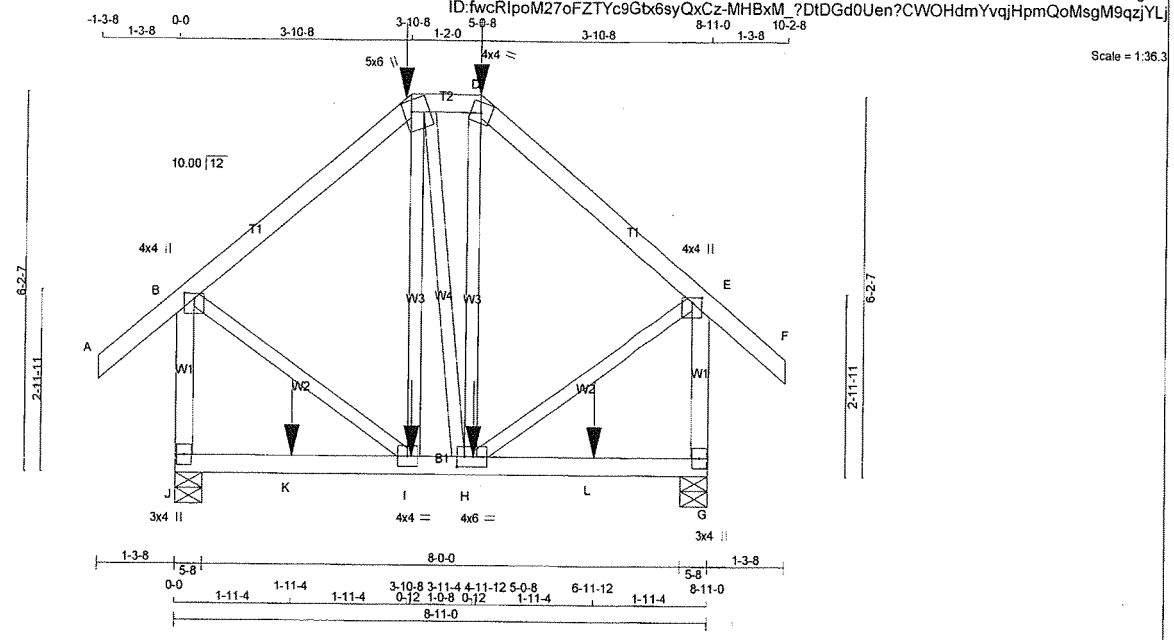

TOTAL WEIGHT = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	963	0	963	0	0	5-8	5-8
G	963	0	963	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	721	380 / 0	133 / 0	0 / 0	0 / 0	208 / 0	0 / 0
G	721	380 / 0	133 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.12 (1)	10.00	I- C	-97 / 74	0.06 (1)
B- C	-544 / 0	-77.7	-77.7	0.23 (1)	6.25	C- H	-8 / 6	0.00 (1)
C- D	-414 / 0	-77.7	-77.7	0.02 (1)	6.25	H- D	-106 / 80	0.07 (1)
D- E	-542 / 0	-77.7	-77.7	0.23 (1)	6.25	B- I	0 / 500	0.12 (1)
E- F	0 / 34	-77.7	-77.7	0.12 (1)	10.00	H- E	0 / 498	0.12 (1)
J- B	-884 / 0	0.0	0.0	0.15 (1)	7.81			
G- E	-882 / 0	0.0	0.0	0.15 (1)	7.81			
J- K	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
K- I	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
I- H	0 / 415	-39.5	-39.5	0.20 (2)	10.00			
H- L	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
L- G	0 / 0	-39.5	-39.5	0.16 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-259	-259	---	FRONT	VERT	TOTAL	---	---
D	5-0-8	-259	-259	---	FRONT	VERT	TOTAL	---	---
H	4-11-12	-37	-47	---	FRONT	VERT	TOTAL	---	---
I	3-11-4	-37	-47	---	FRONT	VERT	TOTAL	---	---
K	1-11-4	-37	-47	---	FRONT	VERT	TOTAL	---	---
L	6-11-12	-37	-47	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.20/1.00 (H-I:2) , WB=0.12/1.00 (B-I:1) , SSI=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/12/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1903643
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T58	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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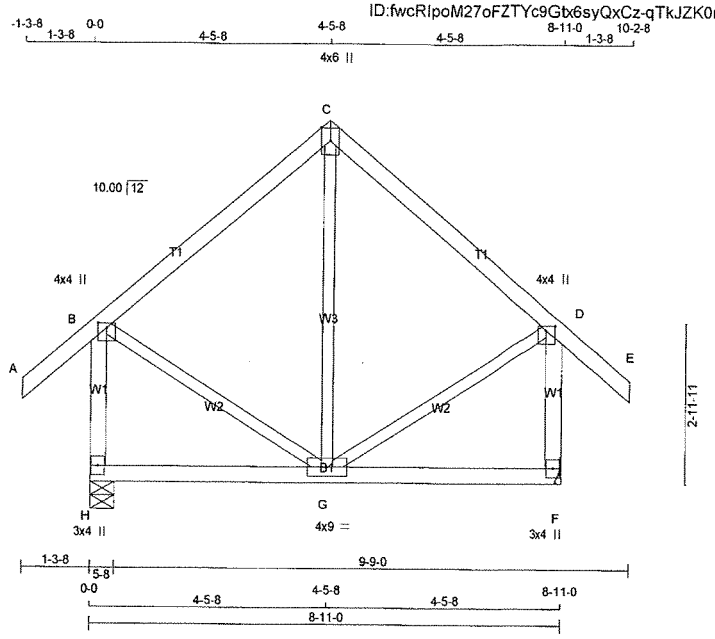

TOTAL WEIGHT = 4 X 46 = 185 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	630	0	630	0	0	5-8	5-8
F	630	0	630	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0
F	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)		10.00	G- C	-16 / 134	0.03 (3)
B- C	-258 / 0	-77.7	-77.7	0.20 (1)		6.25	B- G	0 / 230	0.05 (1)
C- D	-258 / 0	-77.7	-77.7	0.20 (1)		6.25	G- D	0 / 230	0.05 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)		10.00			
H- B	-563 / 0	0.0	0.0	0.09 (1)		7.81			
F- D	-563 / 0	0.0	0.0	0.09 (1)		7.81			
H- G	0 / 0	-39.5	-39.5	0.18 (3)		10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.18/1.00 (G-H:3) , WB=0.05/1.00 (B-G:1) , SSI=0.12/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

DWG NO. TAM TM03644
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T59	1	2	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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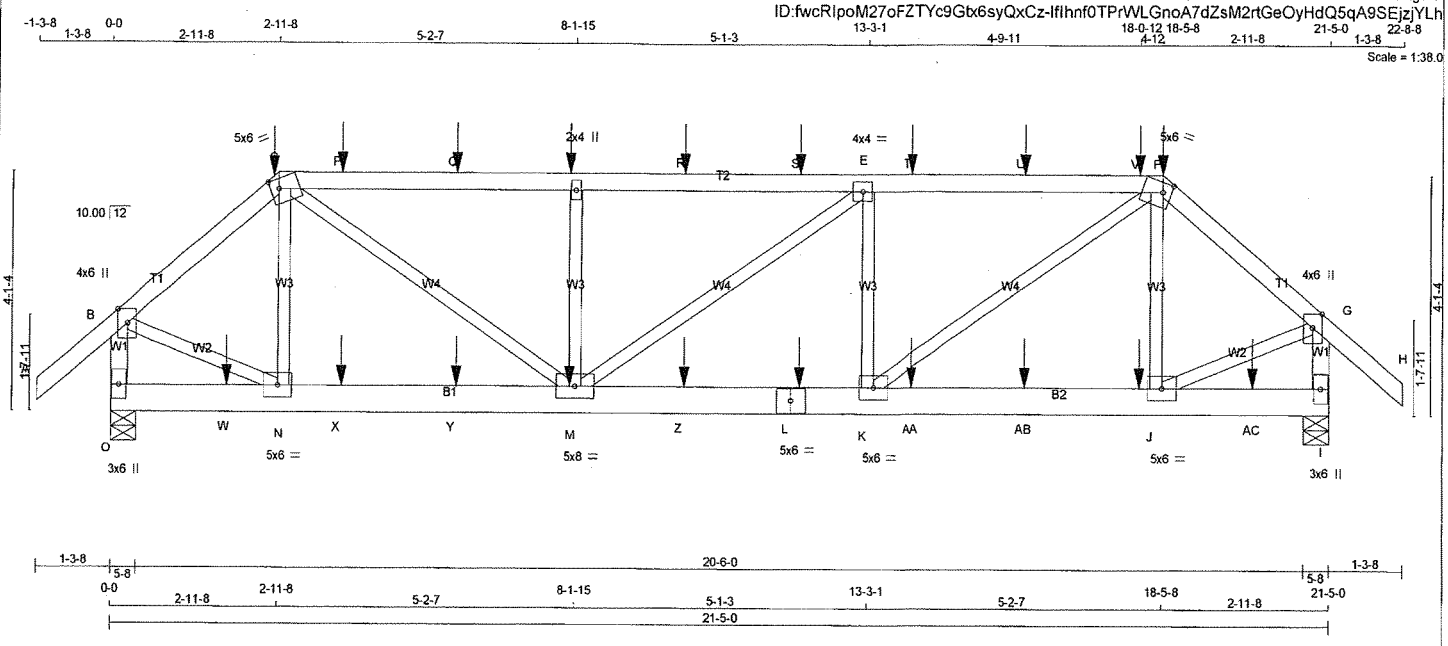


TOTAL WEIGHT = 2 X 102 = 203 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
O- B	1	12	TOP
I- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(183.1)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J, K, N						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2217	0	2217	0	0	5-8	5-8
I	2279	0	2279	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1683	822 / 0	369 / 0	0 / 0	0 / 0	492 / 0	0 / 0
I	1730	844 / 0	380 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	
B- C	-2067 / 0	-77.7	-77.7	0.08 (1)	6.07	
C- P	-3013 / 0	-77.7	-77.7	0.28 (1)	5.03	
P- Q	-3013 / 0	-77.7	-77.7	0.28 (1)	5.03	
Q- D	-3013 / 0	-77.7	-77.7	0.28 (1)	5.03	
D- R	-3013 / 0	-77.7	-77.7	0.30 (1)	4.99	
R- S	-3013 / 0	-77.7	-77.7	0.30 (1)	4.99	
S- E	-3013 / 0	-77.7	-77.7	0.30 (1)	4.99	
E- T	-3033 / 0	-77.7	-77.7	0.30 (1)	4.98	
T- U	-3033 / 0	-77.7	-77.7	0.30 (1)	4.98	
U- V	-3033 / 0	-77.7	-77.7	0.30 (1)	4.98	
V- F	-3033 / 0	-77.7	-77.7	0.30 (1)	4.98	
F- G	-2110 / 0	-77.7	-77.7	0.08 (1)	6.02	
G- H	0 / 34	-77.7	-77.7	0.06 (1)	10.00	
O- B	-2184 / 0	0.0	0.0	0.12 (1)	7.94	
I- G	-2225 / 0	0.0	0.0	0.13 (1)	7.48	
O- W	0 / 0	-39.5	-39.5	0.06 (2)	10.00	
W- N	0 / 0	-39.5	-39.5	0.06 (2)	10.00	
N- X	0 / 1573	-39.5	-39.5	0.15 (2)	10.00	
X- Y	0 / 1573	-39.5	-39.5	0.15 (2)	10.00	
Y- M	0 / 1573	-39.5	-39.5	0.15 (2)	10.00	
M- Z	0 / 3033	-39.5	-39.5	0.26 (1)	10.00	
Z- L	0 / 3033	-39.5	-39.5	0.26 (1)	10.00	
L- K	0 / 3033	-39.5	-39.5	0.26 (1)	10.00	
K-AA	0 / 1606	-39.5	-39.5	0.15 (2)	10.00	
AA-AB	0 / 1606	-39.5	-39.5	0.15 (2)	10.00	
AB- J	0 / 1606	-39.5	-39.5	0.15 (2)	10.00	
J-AC	0 / 0	-39.5	-39.5	0.05 (3)	10.00	
AC- I	0 / 0	-39.5	-39.5	0.05 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-299 / 48	0.04 (1)
M- C	0 / 1768	0.22 (1)
M- D	-687 / 0	0.09 (1)
M- E	-25 / 0	0.01 (1)
K- E	-678 / 0	0.08 (1)
K- F	0 / 1783	0.22 (1)
J- F	-1746 / 0	0.21 (1)
J- N	0 / 1763	0.21 (1)
J- L	0 / 1739	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	8-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
F	18-5-8	-38	-42	---	FRONT	VERT	DEAD	---	---
F	18-5-8	-180	-180	---	FRONT	VERT	SNOW	---	---
J	18-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
L	12-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.26/1.00 (K-M:1) , WB=0.22/1.00 (C-M:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

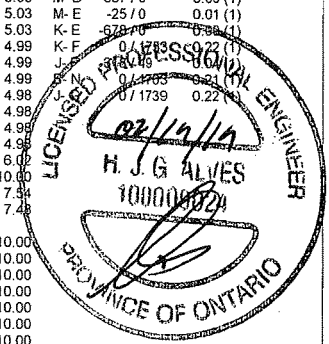
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.47 (G) (INPUT = 1.00)



DWG NO. TAM T1903645
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

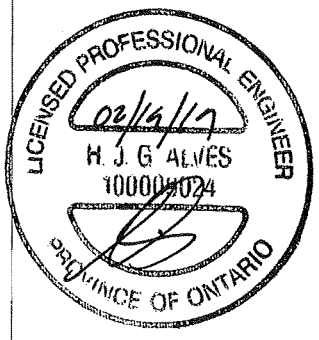
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T59	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:fwcRIpoM27oFZTYc9Gtx6syQxCz-IfIhnf0TPrWLGnoA7dZsM2rtGeOyHdQ5qA9SEjzjYLh

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	8-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
P	4-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	6-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	10-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	12-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
T	14-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
U	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
V	18-0-12	-117	-117	---	BACK	VERT	TOTAL	---	---
W	2-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
X	4-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Y	6-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Z	10-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AA	14-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AB	16-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AC	20-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

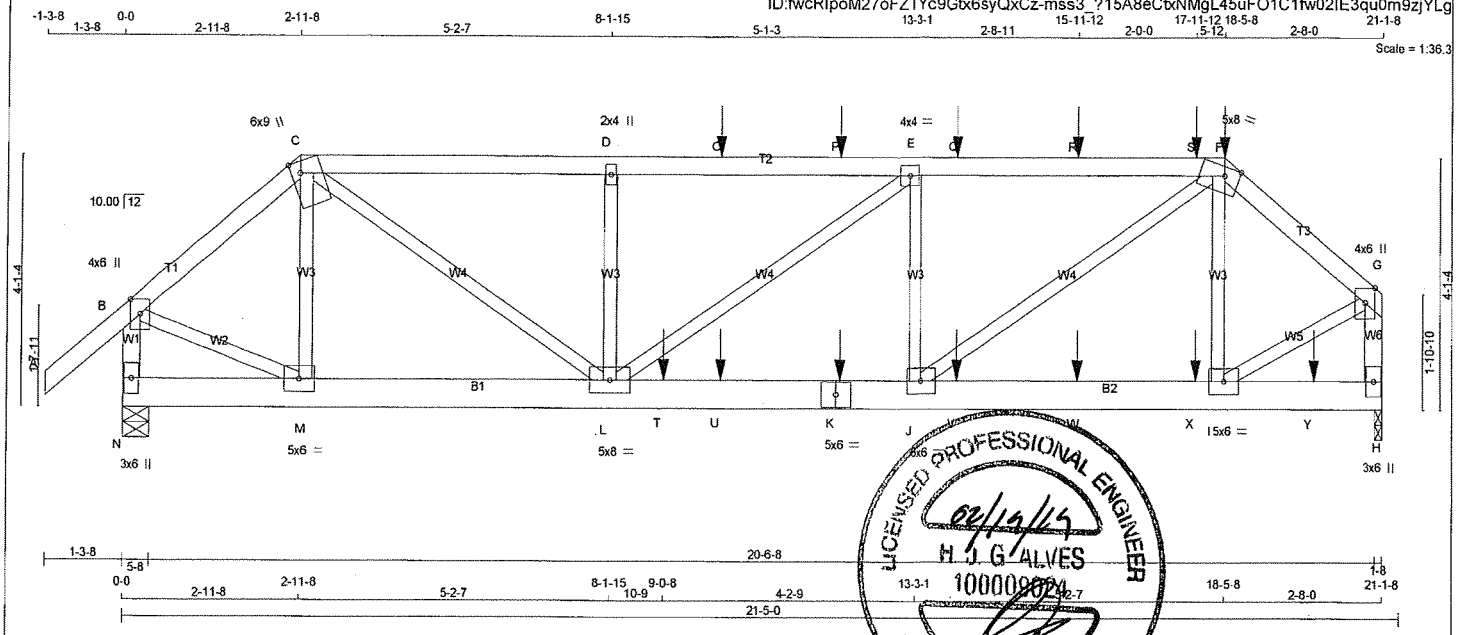


DWG NO. TAM T1903695
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T59A	1	2	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:fwcRIpoM27oFZTYc9Gtx6syQxCz-mss3_?15A8eCtxNMgL45uFO1C1fw02IE3qu0m9zjYLg



TOTAL WEIGHT = 2 X 99 = 198 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.,

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
N- B	1	12	TOP
H- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(0.0)
K- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2408	0	2408	0	0	5-8	5-8
H	2574	0	2574	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1817	918 / 0	373 / 0	0 / 0	0 / 0	526 / 0	0 / 0
H	1952	959 / 0	423 / 0	0 / 0	0 / 0	570 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00		M- C	-592 / 0	0.08 (1)
B- C	-2241 / 0	-77.7	-77.7	0.08 (1)	5.88		C- L	0 / 2996	0.37 (1)
C- D	-4139 / 0	-77.7	-77.7	0.24 (1)	4.48		L- D	-478 / 0	0.06 (1)
D- O	-4139 / 0	-77.7	-77.7	0.35 (1)	4.33		L- E	0 / 341	0.04 (1)
O- P	-4139 / 0	-77.7	-77.7	0.35 (1)	4.33		J- E	-916 / 0	0.12 (1)
P- E	-4139 / 0	-77.7	-77.7	0.35 (1)	4.33		J- F	0 / 2603	0.32 (1)
E- Q	-3863 / 0	-77.7	-77.7	0.34 (1)	4.47		I- F	-668 / 0	0.08 (1)
Q- R	-3863 / 0	-77.7	-77.7	0.34 (1)	4.47		B- M	0 / 1847	0.23 (1)
R- S	-3863 / 0	-77.7	-77.7	0.34 (1)	4.47		I- G	0 / 1993	0.25 (1)
S- F	-3863 / 0	-77.7	-77.7	0.34 (1)	4.47				
F- G	-2303 / 0	-77.7	-77.7	0.07 (1)	5.84				
N- B	-2350 / 0	0.0	0.0	0.13 (1)	7.32				
H- G	-2518 / 0	0.0	0.0	0.15 (1)	7.13				
N- M	0 / 0	-39.5	-39.5	0.02 (3)	10.00				
M- L	0 / 1699	-39.5	-39.5	0.25 (1)	10.00				
L- T	0 / 3863	-39.5	-39.5	0.53 (1)	10.00				
T- U	0 / 3863	-39.5	-39.5	0.53 (1)	10.00				
U- K	0 / 3863	-39.5	-39.5	0.53 (1)	10.00				
K- J	0 / 3863	-39.5	-39.5	0.53 (1)	10.00				
J- V	0 / 1743	-39.5	-39.5	0.21 (1)	10.00				
V- W	0 / 1743	-39.5	-39.5	0.21 (1)	10.00				
W- X	0 / 1743	-39.5	-39.5	0.21 (1)	10.00				
X- I	0 / 1743	-39.5	-39.5	0.21 (1)	10.00				
I- Y	0 / 0	-39.5	-39.5	0.05 (3)	10.00				
Y- H	0 / 0	-39.5	-39.5	0.05 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-5-8	-38	-42	---	FRONT	VERT	DEAD	---	---
F	18-5-8	-180	-180	---	FRONT	VERT	SNOW	---	---
K	11-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
O	9-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
P	11-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	13-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	15-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	17-11-12	-114	-114	---	FRONT	VERT	TOTAL	---	---
T	9-0-8	-1355	-1355	---	FRONT	VERT	TOTAL	---	---
U	9-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
V	13-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

DWG NO. TAM 71903646
STRUCTURAL COMPONENT ONLY

1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.53/1.00 (J-L:1) , WB=0.37/1.00 (C-L:1) , SSI=0.43/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

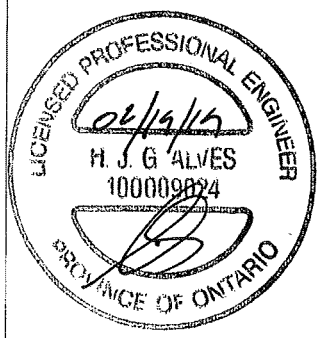
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T59A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:fwcRIpoM27oFZTYc9Gtx6syQxCz-mss3_?15A8eCtxNMgL45uFO1C1fw02IE3qu0m9zjYLg

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	15-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
X	17-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Y	19-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T1903646
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T60	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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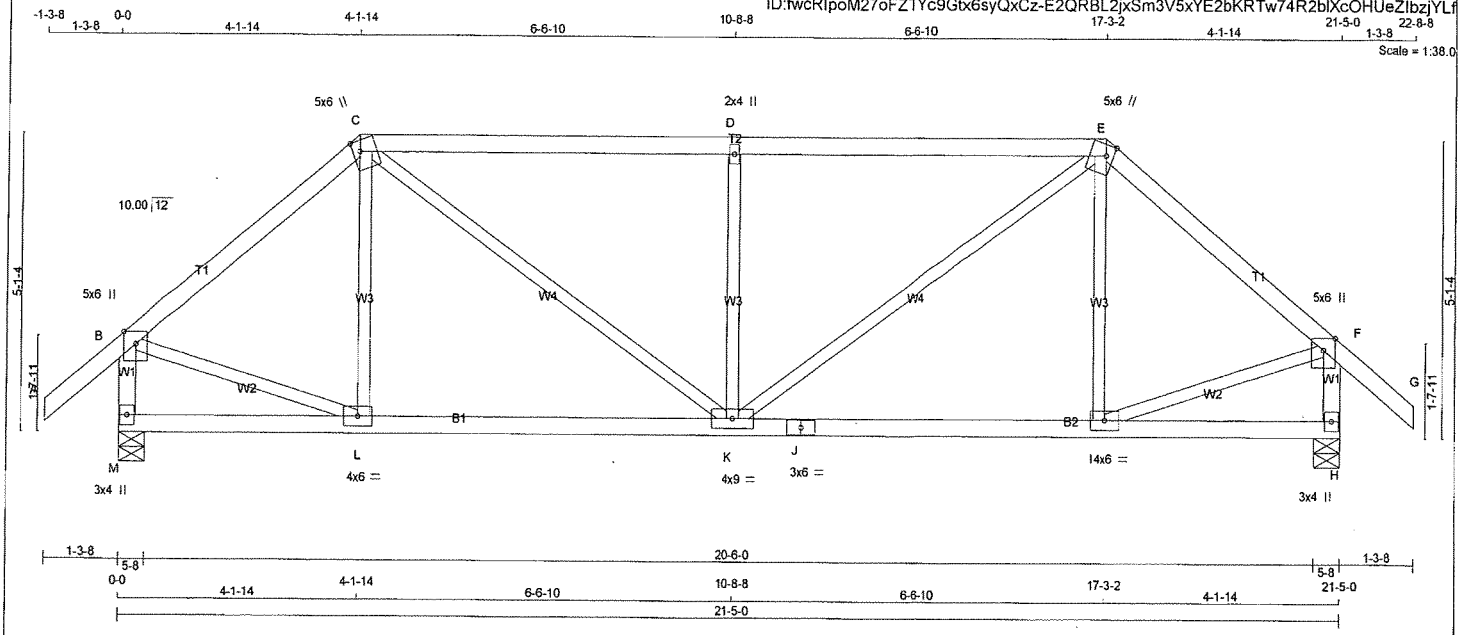


TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1363	0	1363	0	0	5-8	5-8
H	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
H	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

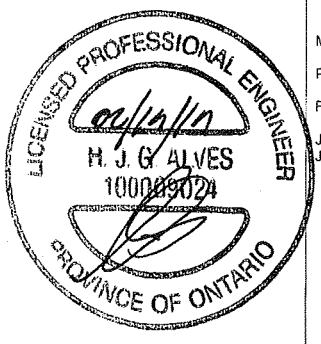
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-42 / 137	0.03 (3)
B- C	-1181 / 0	-77.7	-77.7	0.27 (1)	5.56	C- K	0 / 747	0.17 (1)
C- D	-1507 / 0	-77.7	-77.7	0.67 (1)	4.44	K- D	-621 / 0	0.24 (1)
D- E	-1507 / 0	-77.7	-77.7	0.67 (1)	4.44	K- E	0 / 747	0.17 (1)
E- F	-1181 / 0	-77.7	-77.7	0.27 (1)	5.56	I- E	-42 / 137	0.03 (3)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	B- L	0 / 947	0.21 (1)
M- B	-1305 / 0	0.0	0.0	0.14 (1)	7.06	I- F	0 / 947	0.21 (1)
H- F	-1305 / 0	0.0	0.0	0.14 (1)	7.06			
M- L	0 / 0	-39.5	-39.5	0.21 (3)	10.00			
L- K	0 / 904	-39.5	-39.5	0.37 (2)	10.00			
K- J	0 / 904	-39.5	-39.5	0.37 (2)	10.00			
J- I	0 / 904	-39.5	-39.5	0.37 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.37/1.00 (I-K:2) , WB=0.24/1.00 (D-K:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.45 (F) (INPUT = 1.00)

DWG NO. TAM 7190 3647
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T60A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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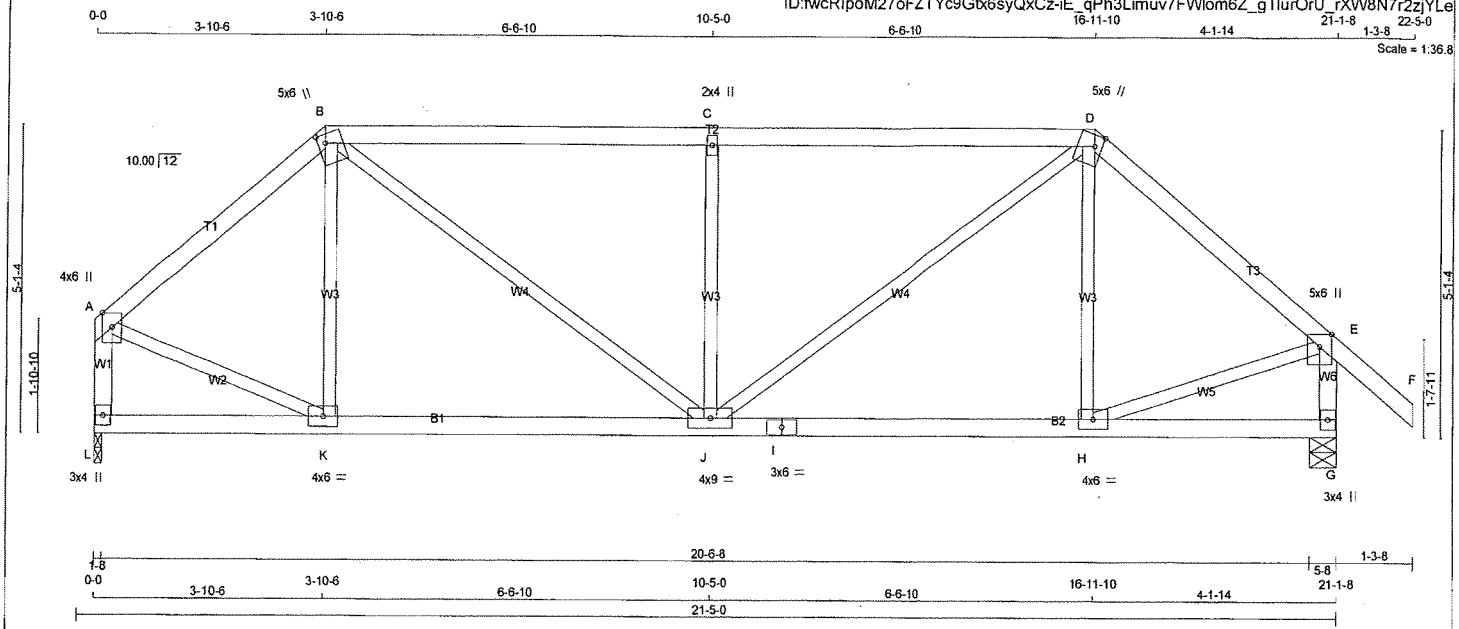


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	1.75	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1238	0	1238	0	0	1-8	1-8
G	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

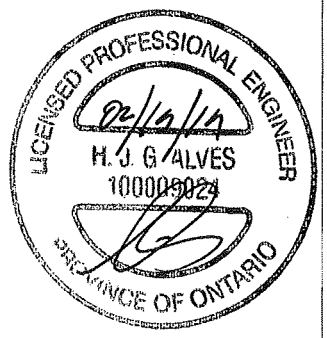
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1102 / 0	-77.7	-77.7	0.23 (1)	5.76	K- B	-99 / 100		0.04 (1)
B- C	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	B- J	0 / 778		0.17 (1)
C- D	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	J- C	-622 / 0		0.24 (1)
D- E	-1162 / 0	-77.7	-77.7	0.26 (1)	5.59	J- D	0 / 719		0.16 (1)
E- F	0 / 34	-77.7	-77.7	0.11 (1)	10.00	H- D	-38 / 139		0.03 (3)
L- A	-1188 / 0	0.0	0.0	0.13 (1)	7.32	A- K	0 / 906		0.20 (1)
G- E	-1288 / 0	0.0	0.0	0.14 (1)	7.11	H- E	0 / 931		0.21 (1)
L- K	0 / 0	-39.5	-39.5	0.20 (3)	10.00				
K- J	0 / 842	-39.5	-39.5	0.36 (2)	10.00				
J- I	0 / 889	-39.5	-39.5	0.37 (2)	10.00				
I- H	0 / 889	-39.5	-39.5	0.37 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.37/1.00 (H-J:2) , WB=0.24/1.00 (C-J:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

DWG NO. TAM T1903648
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T61	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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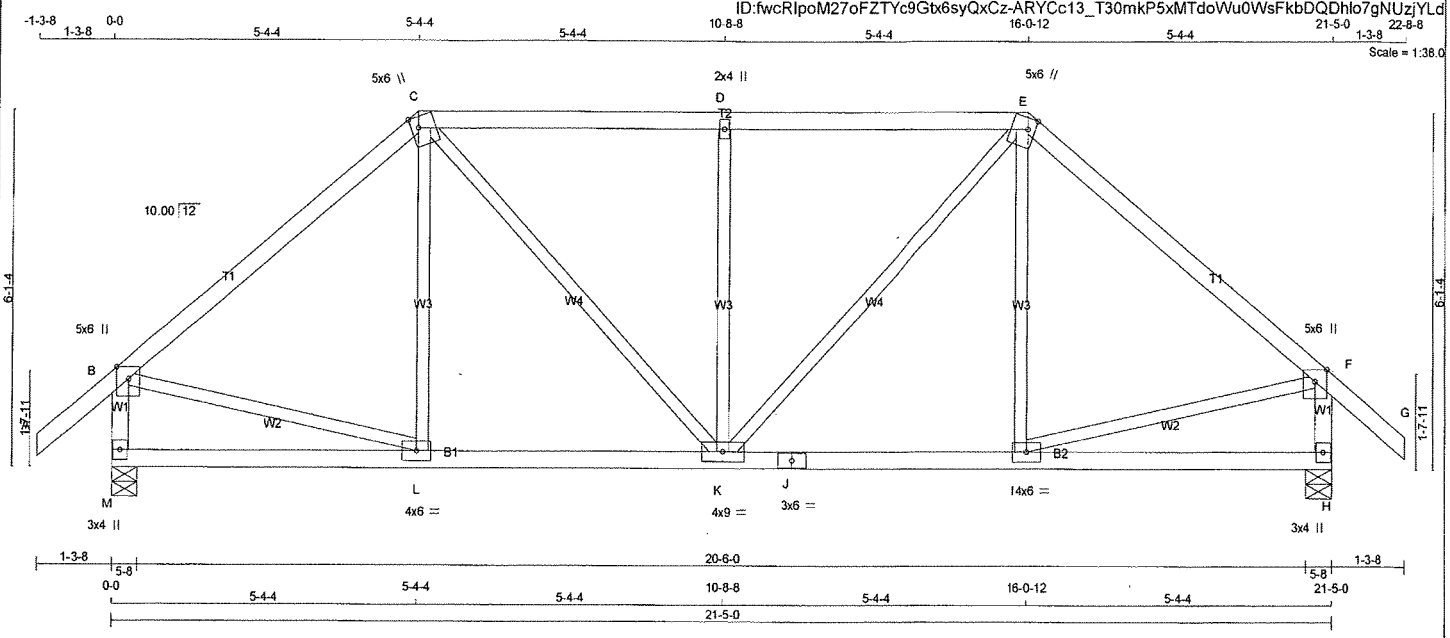


TOTAL WEIGHT = 94 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1363	0	1363	0	0	5-8	5-8
H	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
H	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

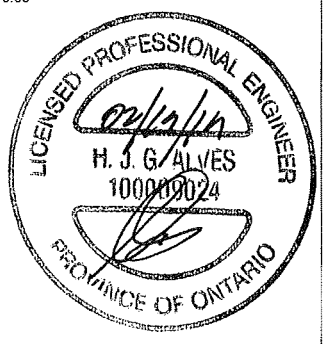
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
B- C	-1164 / 0	-77.7	-77.7	0.45 (1)	5.33	
C- D	-1213 / 0	-77.7	-77.7	0.41 (1)	5.27	
D- E	-1213 / 0	-77.7	-77.7	0.41 (1)	5.27	
E- F	-1164 / 0	-77.7	-77.7	0.45 (1)	5.33	
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
M- B	-1277 / 0	0.0	0.0	0.14 (1)	7.13	
H- F	-1277 / 0	0.0	0.0	0.14 (1)	7.13	
M- L	0 / 0	-39.5	-39.5	0.22 (3)	10.00	
L- K	0 / 893	-39.5	-39.5	0.34 (2)	10.00	
K- J	0 / 893	-39.5	-39.5	0.34 (2)	10.00	
J- I	0 / 893	-39.5	-39.5	0.34 (2)	10.00	
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	0 / 186	0.04 (3)
C- K	0 / 471	0.11 (1)
K- D	-505 / 0	0.30 (1)
K- E	0 / 471	0.11 (1)
I- E	0 / 186	0.04 (3)
B- L	0 / 918	0.21 (1)
I- F	0 / 918	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45/1.00 (E-F:1) , BC=0.34/1.00 (I-K:2) , WB=0.30/1.00 (D-K:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (L) (INPUT = 0.90)
JSI METAL= 0.45 (F) (INPUT = 1.00)

DWG NO. TAM T7903649
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T61A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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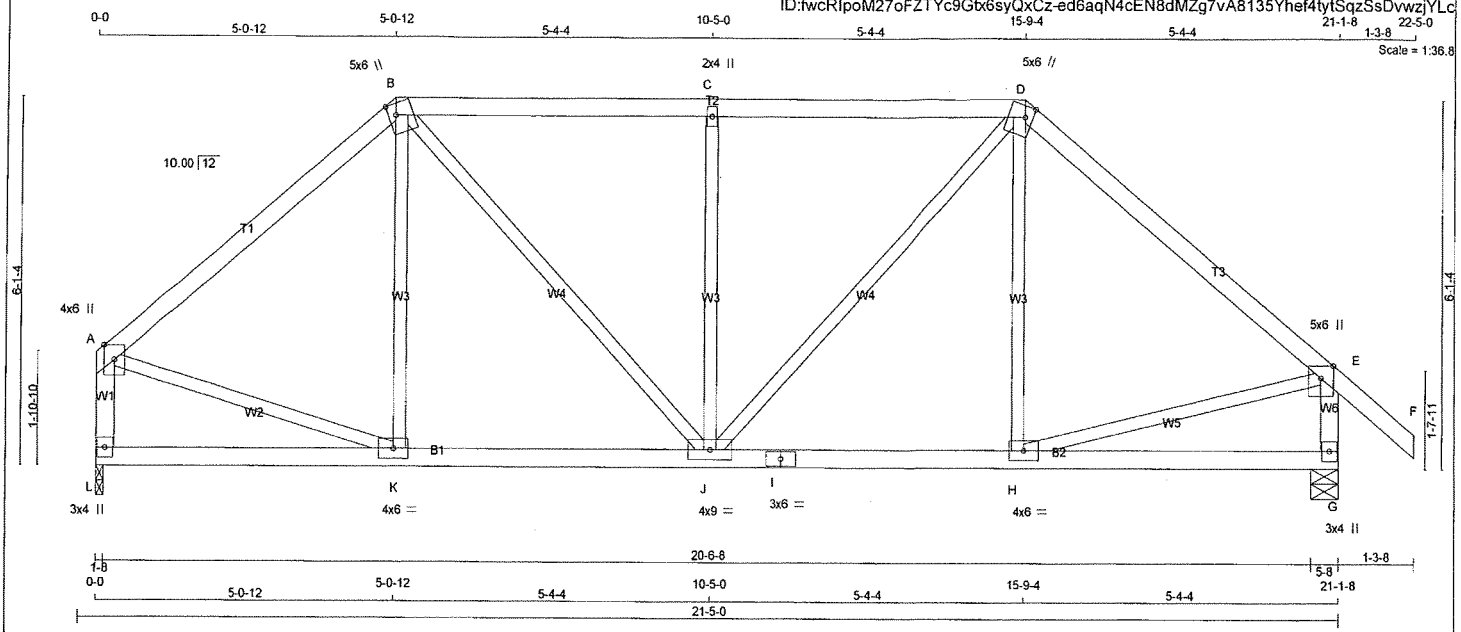


TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1238	0	1238	0	0	1-8	1-8
G	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

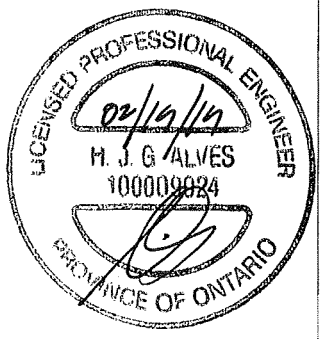
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1102 / 0	-77.7	-77.7	0.40 (1)	5.52	K- B	-17 / 153	0.04 (3)
B- C	-1182 / 0	-77.7	-77.7	0.41 (1)	5.33	B- J	0 / 497	0.11 (1)
C- D	-1182 / 0	-77.7	-77.7	0.41 (1)	5.33	J- C	-505 / 0	0.30 (1)
D- E	-1144 / 0	-77.7	-77.7	0.45 (1)	5.36	J- D	0 / 449	0.10 (1)
E- F	0 / 34	-77.7	-77.7	0.11 (1)	10.00	H- D	0 / 187	0.04 (3)
L- A	-1157 / 0	0.0	0.0	0.13 (1)	7.40	A- K	0 / 882	0.20 (1)
G- E	-1260 / 0	0.0	0.0	0.13 (1)	7.17	H- E	0 / 901	0.20 (1)
L- K	0 / 0	-39.5	-39.5	0.21 (3)	10.00			
K- J	0 / 844	-39.5	-39.5	0.32 (2)	10.00			
J- I	0 / 877	-39.5	-39.5	0.34 (2)	10.00			
I- H	0 / 877	-39.5	-39.5	0.34 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.34/1.00 (H-J:2) , WB=0.30/1.00 (C-J:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

DWG NO. TAM T1903850
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T62	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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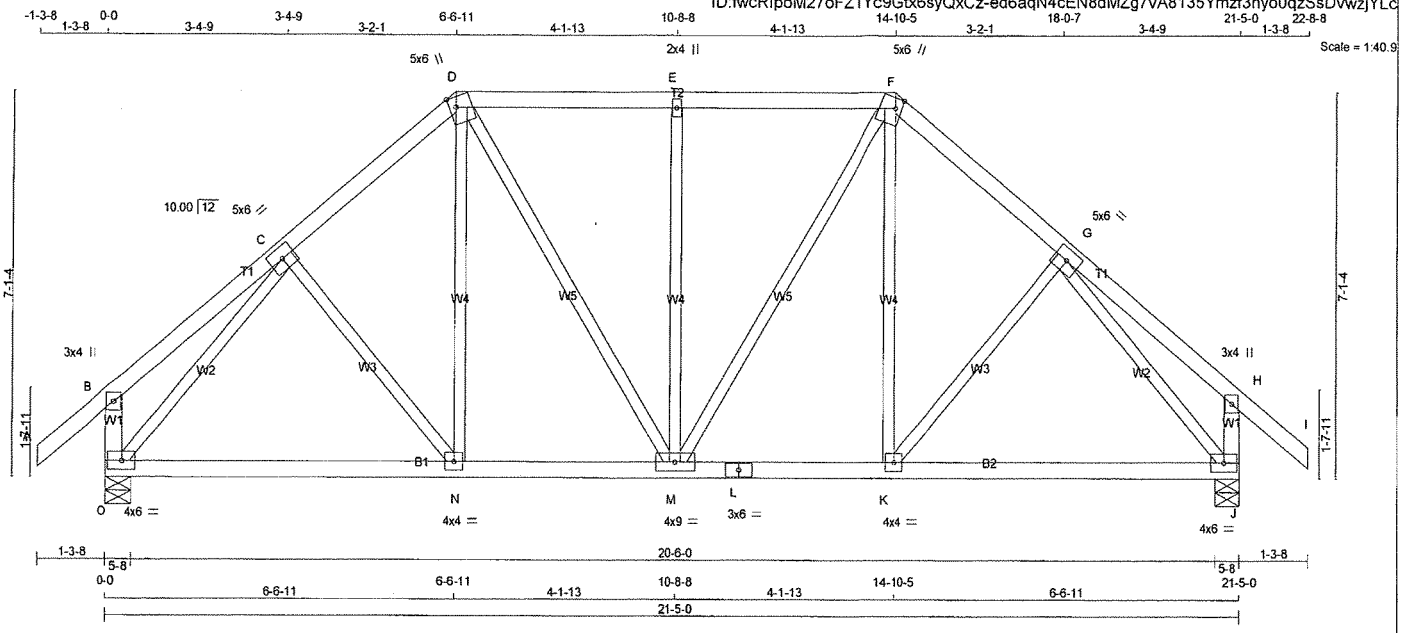

TOTAL WEIGHT = 105 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1363	0	1363	0	0	5-8	5-8
J	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
J	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 34		-77.7	-77.7	0.11 (1)		10.00	C- N	0 / 89		0.02 (3)
B- C	0 / 17		-77.7	-77.7	0.12 (1)		10.00	N- D	0 / 305		0.07 (2)
C- D	-1143 / 0		-77.7	-77.7	0.11 (1)		5.84	D- M	0 / 273		0.06 (1)
D- E	-1008 / 0		-77.7	-77.7	0.18 (1)		6.03	M- E	-387 / 0		0.34 (1)
E- F	-1008 / 0		-77.7	-77.7	0.18 (1)		6.03	M- F	0 / 273		0.06 (1)
F- G	-1143 / 0		-77.7	-77.7	0.11 (1)		5.84	K- F	0 / 305		0.07 (2)
G- H	0 / 17		-77.7	-77.7	0.12 (1)		10.00	K- G	0 / 89		0.02 (3)
H- I	0 / 34		-77.7	-77.7	0.11 (1)		10.00	O- C	-1349 / 0		0.58 (1)
O- B	-207 / 0		0.0	0.0	0.02 (1)		7.81	G- J	-1349 / 0		0.58 (1)
J- H	-207 / 0		0.0	0.0	0.02 (1)		7.81				
O- N	0 / 849		-39.5	-39.5	0.40 (2)		10.00				
N- M	0 / 866		-39.5	-39.5	0.41 (2)		10.00				
M- L	0 / 866		-39.5	-39.5	0.41 (2)		10.00				
L- K	0 / 866		-39.5	-39.5	0.41 (2)		10.00				
K- J	0 / 849		-39.5	-39.5	0.40 (2)		10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.41/1.00 (K-M:2) , WB=0.58/1.00 (C-O:1) , SSI=0.17/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.33 (G) (INPUT = 1.00)

DWG NO. TAM T1903651
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T62A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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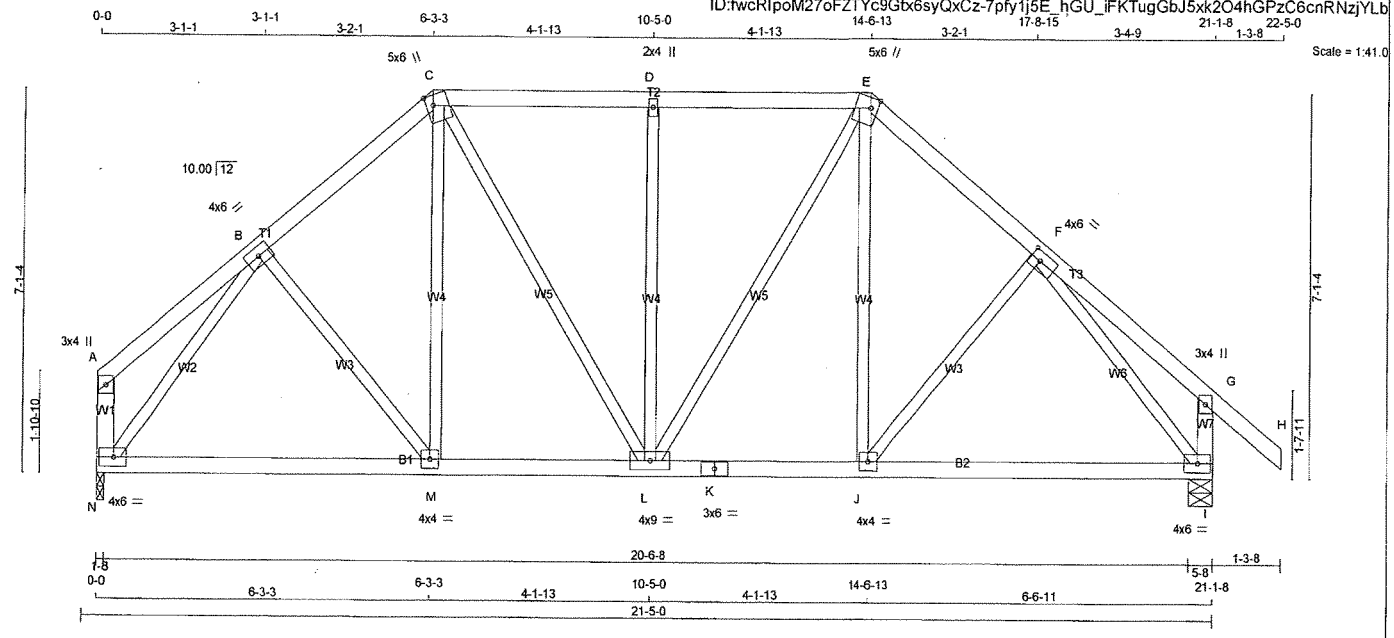

TOTAL WEIGHT = 102 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1238	0	1238	0	0	1-8	1-8
I	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
I	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 17	-77.7	-77.7	0.11 (1)	10.00	B- M	0 / 115	0.03 (3)
B- C	-1091 / 0	-77.7	-77.7	0.10 (1)	5.95	M- C	0 / 254	0.06 (3)
C- D	-982 / 0	-77.7	-77.7	0.17 (1)	6.09	C- L	0 / 298	0.07 (1)
D- E	-982 / 0	-77.7	-77.7	0.17 (1)	6.09	L- D	-387 / 0	0.34 (1)
E- F	-1121 / 0	-77.7	-77.7	0.11 (1)	5.88	L- E	0 / 254	0.06 (1)
F- G	0 / 17	-77.7	-77.7	0.12 (1)	10.00	J- E	0 / 308	0.07 (2)
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F	0 / 86	0.02 (3)
N- A	-90 / 0	0.0	0.0	0.01 (1)	7.81	N- B	-1301 / 0	0.52 (1)
I- G	-209 / 0	0.0	0.0	0.02 (1)	7.81	F- I	-1326 / 0	0.57 (1)
N- M	0 / 774	-39.5	-39.5	0.37 (2)	10.00			
M- L	0 / 827	-39.5	-39.5	0.38 (2)	10.00			
L- K	0 / 850	-39.5	-39.5	0.40 (2)	10.00			
K- J	0 / 850	-39.5	-39.5	0.40 (2)	10.00			
J- I	0 / 837	-39.5	-39.5	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.40/1.00 (J-L:2) , WB=0.57/1.00 (F-I:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

DWG NO. TAM T1903652
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T63	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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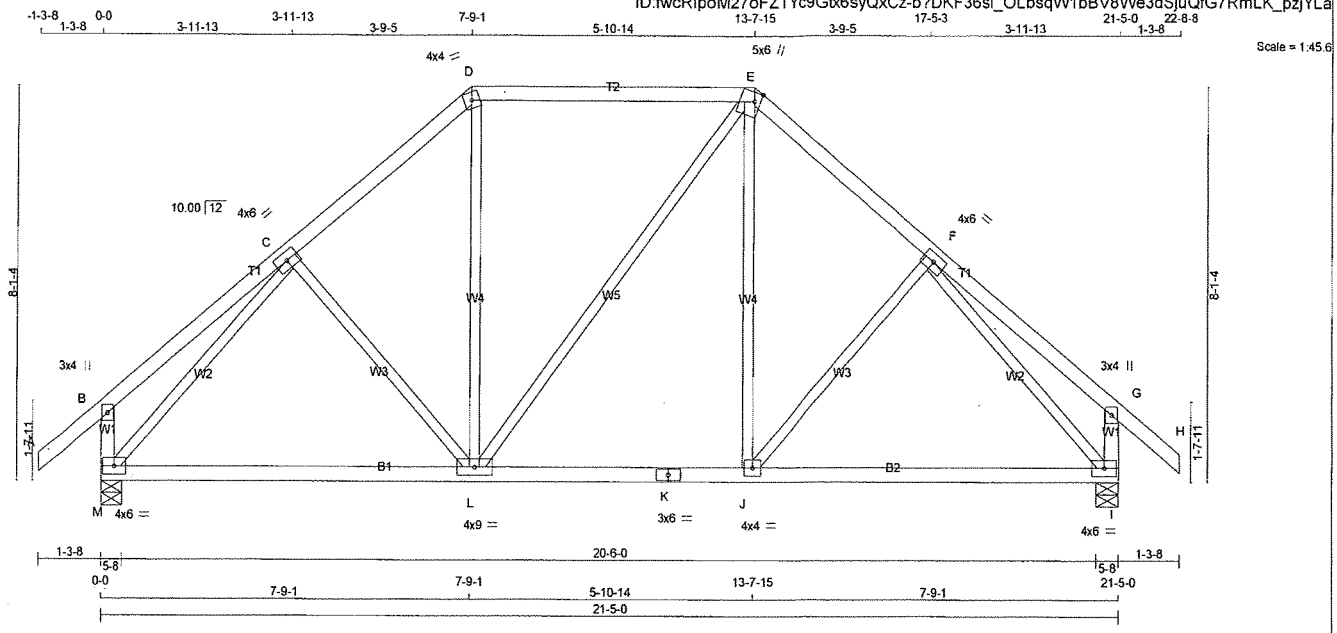

TOTAL WEIGHT = 100 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00
C- D	-1091 / 0	-77.7	-77.7	0.15 (1)	5.89
D- E	-824 / 0	-77.7	-77.7	0.36 (1)	6.21
E- F	-1091 / 0	-77.7	-77.7	0.15 (1)	5.89
F- G	0 / 22	-77.7	-77.7	0.18 (1)	10.00
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00
M- B	-223 / 0	0.0	0.0	0.02 (1)	7.81
I- G	-223 / 0	0.0	0.0	0.02 (1)	7.81
M- L	0 / 861	-39.5	-39.5	0.49 (2)	10.00
L- K	0 / 823	-39.5	-39.5	0.46 (2)	10.00
K- J	0 / 823	-39.5	-39.5	0.46 (2)	10.00
J- I	0 / 861	-39.5	-39.5	0.49 (2)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-63 / 58	0.04 (1)
L- D	0 / 379	0.09 (2)
L- E	0 / 0	0.00 (1)
J- E	0 / 378	0.09 (2)
J- F	-63 / 58	0.04 (1)
M- C	-1327 / 0	0.79 (1)
F- I	-1326 / 0	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 930 (0.28")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.49/1.00 (I-J:2) , WB=0.79/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.44 (K) (INPUT = 1.00)

DWG NO. TAM R7703653
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T63A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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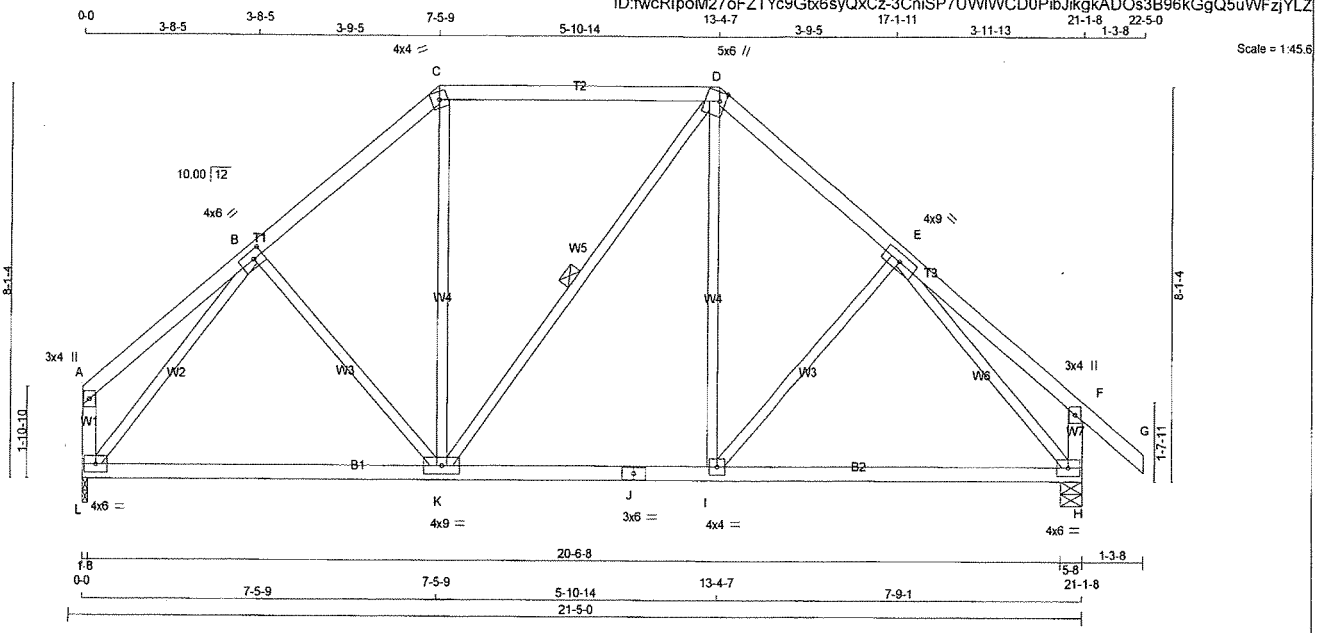


TOTAL WEIGHT = 97 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	9.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

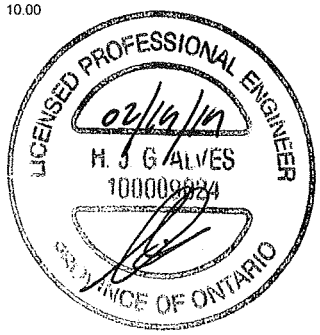
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-77.7	-77.7	0.17 (1)	10.00
B- C	-1049 / 0	-77.7	-77.7	0.14 (1)	5.99
C- D	-792 / 0	-77.7	-77.7	0.36 (1)	6.25
D- E	-1069 / 0	-77.7	-77.7	0.15 (1)	5.93
E- F	0 / 22	-77.7	-77.7	0.18 (1)	10.00
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00
L- A	-106 / 0	0.0	0.0	0.01 (1)	7.81
H- F	-225 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 796	-39.5	-39.5	0.46 (2)	10.00
K- J	0 / 807	-39.5	-39.5	0.46 (2)	10.00
J- I	0 / 807	-39.5	-39.5	0.46 (2)	10.00
I- H	0 / 847	-39.5	-39.5	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-13 / 80	0.02 (3)
K- C	0 / 354	0.08 (2)
K- D	-25 / 0	0.02 (1)
I- D	0 / 387	0.09 (2)
I- E	-68 / 55	0.04 (1)
L- B	-1283 / 0	0.71 (1)
E- H	-1304 / 0	0.77 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 928 (0.27")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.49/1.00 (H-I:2) , WB=0.77/1.00 (E-H:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)

DWG NO. TAM T1903654
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T64	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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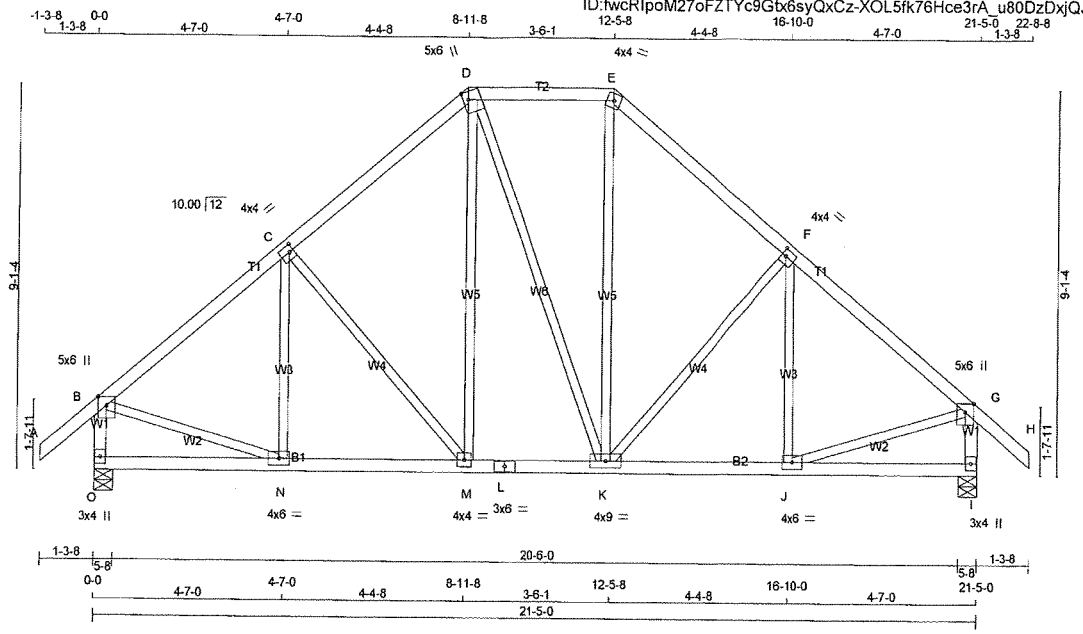


TOTAL WEIGHT = 109 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

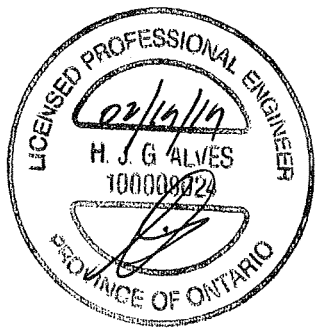
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- C		-61 / 115	0.03 (3)
B- C		-1190 / 0	-77.7	-77.7	0.22 (1)	5.63	C- M		-287 / 0	0.25 (1)
C- D		-999 / 0	-77.7	-77.7	0.21 (1)	6.01	M- D		0 / 374	0.08 (2)
D- E		-750 / 0	-77.7	-77.7	0.13 (1)	6.25	D- K		0 / 3	0.00 (2)
E- F		-1001 / 0	-77.7	-77.7	0.21 (1)	6.01	K- E		0 / 378	0.09 (2)
F- G		-1189 / 0	-77.7	-77.7	0.22 (1)	5.63	K- F		-285 / 0	0.25 (1)
G- H		0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F		-64 / 113	0.03 (1)
O- B		-1290 / 0	0.0	0.0	0.14 (1)	7.10	B- N		0 / 971	0.22 (1)
I- G		-1289 / 0	0.0	0.0	0.14 (1)	7.10	J- G		0 / 971	0.22 (1)
O- N		0 / 0	-39.5	-39.5	0.16 (3)	10.00				
N- M		0 / 934	-39.5	-39.5	0.28 (2)	10.00				
M- L		0 / 749	-39.5	-39.5	0.19 (2)	10.00				
L- K		0 / 749	-39.5	-39.5	0.19 (2)	10.00				
K- J		0 / 934	-39.5	-39.5	0.28 (2)	10.00				
J- I		0 / 0	-39.5	-39.5	0.16 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.28/1.00 (M-N:2) , WB=0.25/1.00 (C-M:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM 17903655
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T64A	1	1	UNITS 21 - 23 TRUSS DESC.	

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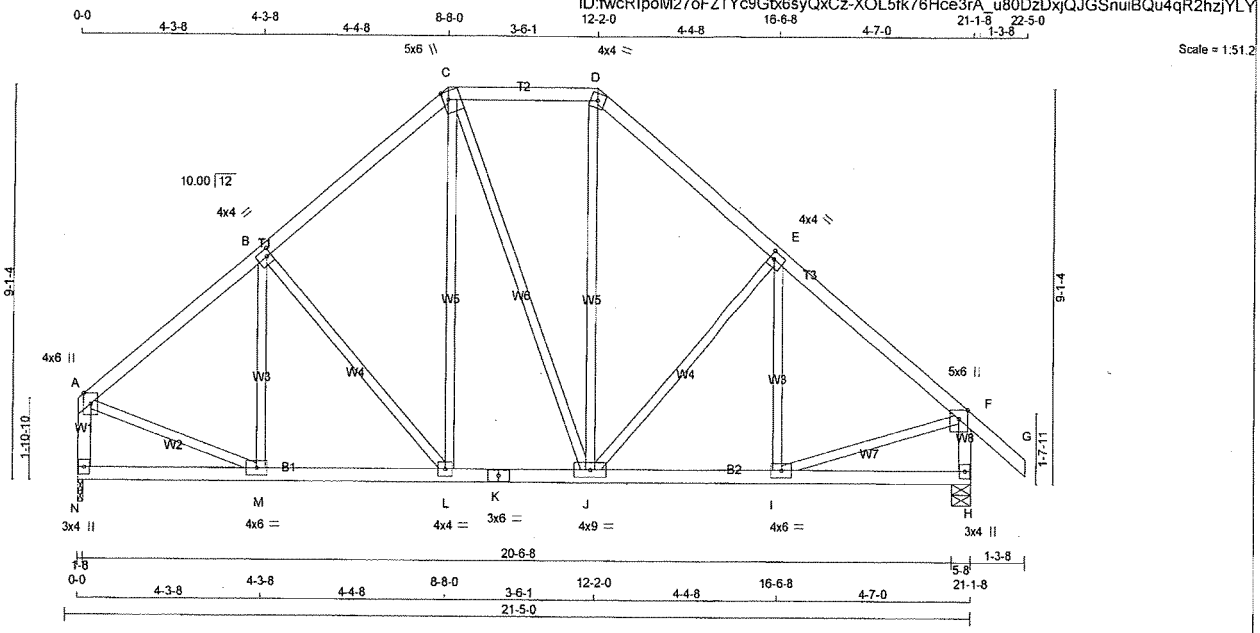


TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

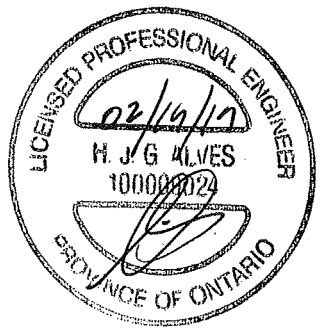
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		-1112 / 0	-77.7	-77.7	0.20 (1)	5.79
B- C		-967 / 0	-77.7	-77.7	0.20 (1)	6.10
C- D		-733 / 0	-77.7	-77.7	0.13 (1)	6.25
D- E		-978 / 0	-77.7	-77.7	0.21 (1)	6.06
E- F		-1170 / 0	-77.7	-77.7	0.22 (1)	5.66
F- G		0 / 34	-77.7	-77.7	0.11 (1)	10.00
N- A		-1170 / 0	0.0	0.0	0.13 (1)	7.37
H- F		-1272 / 0	0.0	0.0	0.14 (1)	7.14
N- M		0 / 0	-39.5	-39.5	0.14 (3)	10.00
M- L		0 / 874	-39.5	-39.5	0.26 (2)	10.00
L- K		0 / 725	-39.5	-39.5	0.19 (2)	10.00
K- J		0 / 725	-39.5	-39.5	0.19 (2)	10.00
J- I		0 / 918	-39.5	-39.5	0.27 (2)	10.00
I- H		0 / 0	-39.5	-39.5	0.15 (3)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B		-118 / 78	0.05 (1)
B- L		-232 / 0	0.20 (1)
L- C		0 / 340	0.08 (2)
C- J		0 / 20	0.00 (1)
J- D		0 / 365	0.08 (2)
J- E		-288 / 0	0.25 (1)
I- E		-59 / 115	0.03 (3)
A- M		0 / 929	0.21 (1)
I- F		0 / 955	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (E-F:1) , BC=0.27/1.00 (I-J:2) , WB=0.25/1.00 (E-J:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

DWG NO. TAM T1403656
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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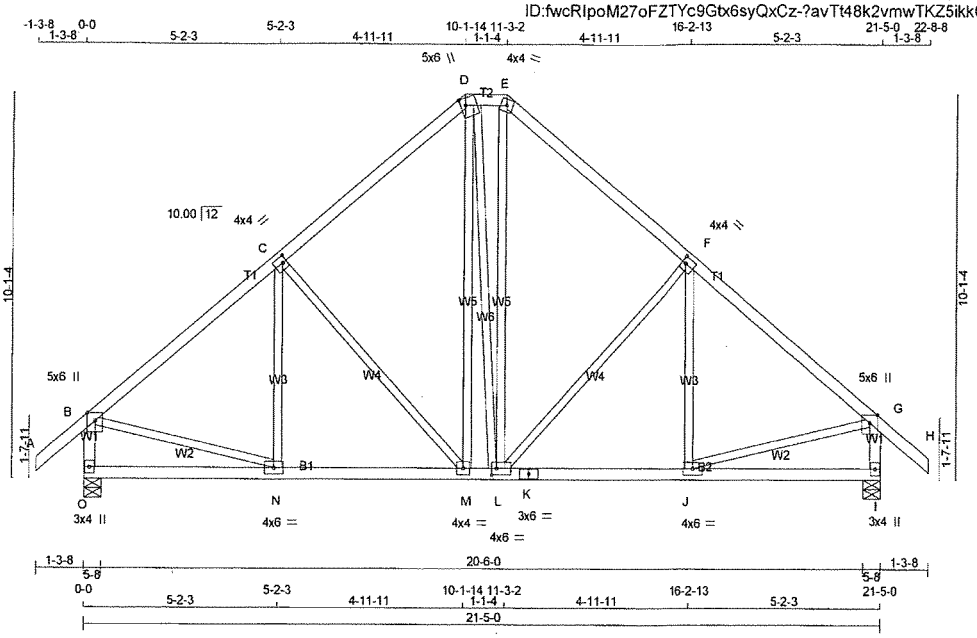


TOTAL WEIGHT = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

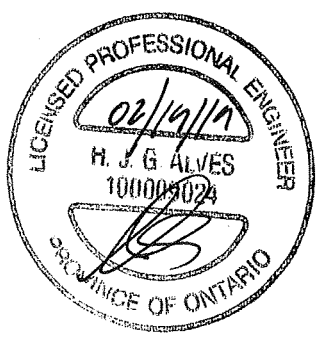
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- C	0 / 170	0.04 (3)	
B- C	-1193 / 0	-77.7	-77.7	0.29 (1)	5.54	C- M	-380 / 0	0.44 (1)	
C- D	-924 / 0	-77.7	-77.7	0.27 (1)	6.10	M- D	0 / 343	0.08 (2)	
D- E	-693 / 0	-77.7	-77.7	0.02 (1)	6.25	D- L	0 / 44	0.01 (2)	
E- F	-929 / 0	-77.7	-77.7	0.27 (1)	6.09	L- E	0 / 390	0.09 (2)	
F- G	-1190 / 0	-77.7	-77.7	0.29 (1)	5.54	L- F	-371 / 0	0.43 (1)	
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F	-10 / 162	0.04 (3)	
O- B	-1281 / 0	0.0	0.0	0.14 (1)	7.12	B- N	0 / 968	0.22 (1)	
I- G	-1280 / 0	0.0	0.0	0.14 (1)	7.13	J- G	0 / 967	0.22 (1)	
O- N	0 / 0	-39.5	-39.5	0.21 (3)	10.00				
N- M	0 / 939	-39.5	-39.5	0.33 (2)	10.00				
M- L	0 / 688	-39.5	-39.5	0.23 (2)	10.00				
L- K	0 / 937	-39.5	-39.5	0.32 (2)	10.00				
K- J	0 / 937	-39.5	-39.5	0.32 (2)	10.00				
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.33/1.00 (M-N:2) , WB=0.44/1.00 (C-M:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (L) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T1903657
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T65A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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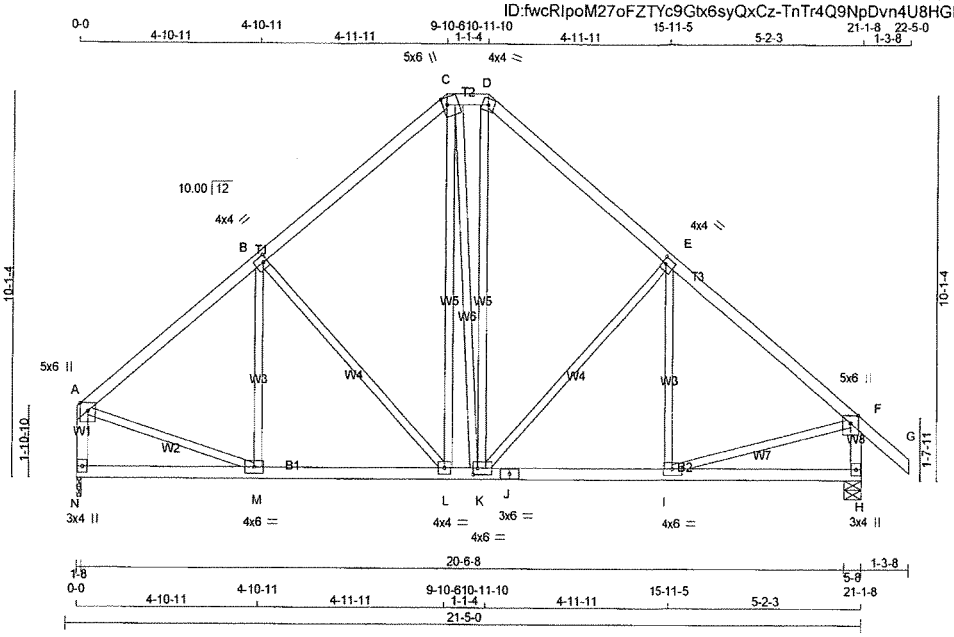

TOTAL WEIGHT = 112 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-1123 / 0	-77.7	-77.7	0.26 (1)	5.69	M- B	-51 / 135	0.03 (3)
B- C	-898 / 0	-77.7	-77.7	0.25 (1)	6.19	B- L	-328 / 0	0.38 (1)
C- D	-674 / 0	-77.7	-77.7	0.02 (1)	6.25	L- C	0 / 320	0.07 (2)
D- E	-906 / 0	-77.7	-77.7	0.27 (1)	6.15	C- K	0 / 53	0.01 (1)
E- F	-1170 / 0	-77.7	-77.7	0.28 (1)	5.58	K- D	0 / 376	0.08 (2)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- E	-375 / 0	0.43 (1)
N- A	-1162 / 0	0.0	0.0	0.13 (1)	7.39	I- E	-5 / 165	0.04 (3)
H- F	-1263 / 0	0.0	0.0	0.13 (1)	7.16	A- M	0 / 929	0.21 (1)
						I- F	0 / 951	0.21 (1)
N- M	0 / 0	-39.5	-39.5	0.19 (3)	10.00			
M- L	0 / 886	-39.5	-39.5	0.31 (2)	10.00			
L- K	0 / 669	-39.5	-39.5	0.22 (2)	10.00			
K- J	0 / 922	-39.5	-39.5	0.31 (2)	10.00			
J- I	0 / 922	-39.5	-39.5	0.31 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.31/1.00 (I-K:2) , WB=0.43/1.00 (E-K:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (K) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

DWG NO. TAM T1903253
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T66	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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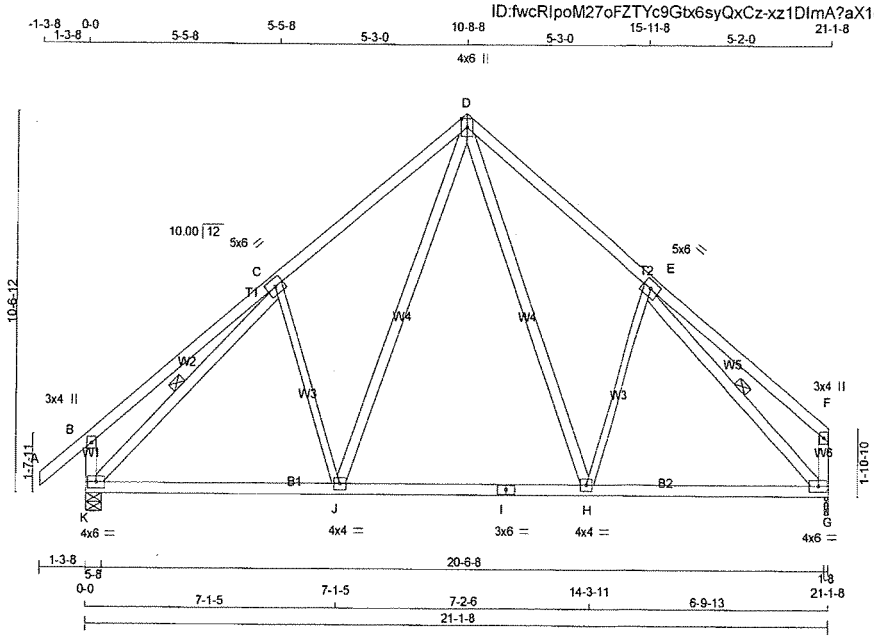

TOTAL WEIGHT = 111 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1346	0	1346	0	0	5-8	5-8
G	1238	0	1238	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-K, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)		10.00
B- C	0 / 32	-77.7	-77.7	0.37 (1)		10.00
C- D	-1101 / 0	-77.7	-77.7	0.31 (1)		5.67
D- E	-1074 / 0	-77.7	-77.7	0.30 (1)		5.74
E- F	0 / 32	-77.7	-77.7	0.35 (1)		10.00
K- B	-263 / 0	0.0	0.0	0.03 (1)		7.81
G- F	-143 / 0	0.0	0.0	0.02 (1)		7.81
K- J	0 / 897	-39.5	-39.5	0.47 (2)		10.00
J- I	0 / 633	-39.5	-39.5	0.44 (2)		10.00
I- H	0 / 633	-39.5	-39.5	0.44 (2)		10.00
H- G	0 / 862	-39.5	-39.5	0.44 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-243 / 16	0.16 (1)
J- D	0 / 574	0.09 (1)
K- C	-1321 / 0	0.38 (1)
H- E	-194 / 38	0.13 (1)
D- H	0 / 514	0.08 (1)
E- G	-1310 / 0	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.47/1.00 (J-K:2) , WB=0.38/1.00 (C-K:1) , SSI=0.18/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM T1903659
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T67	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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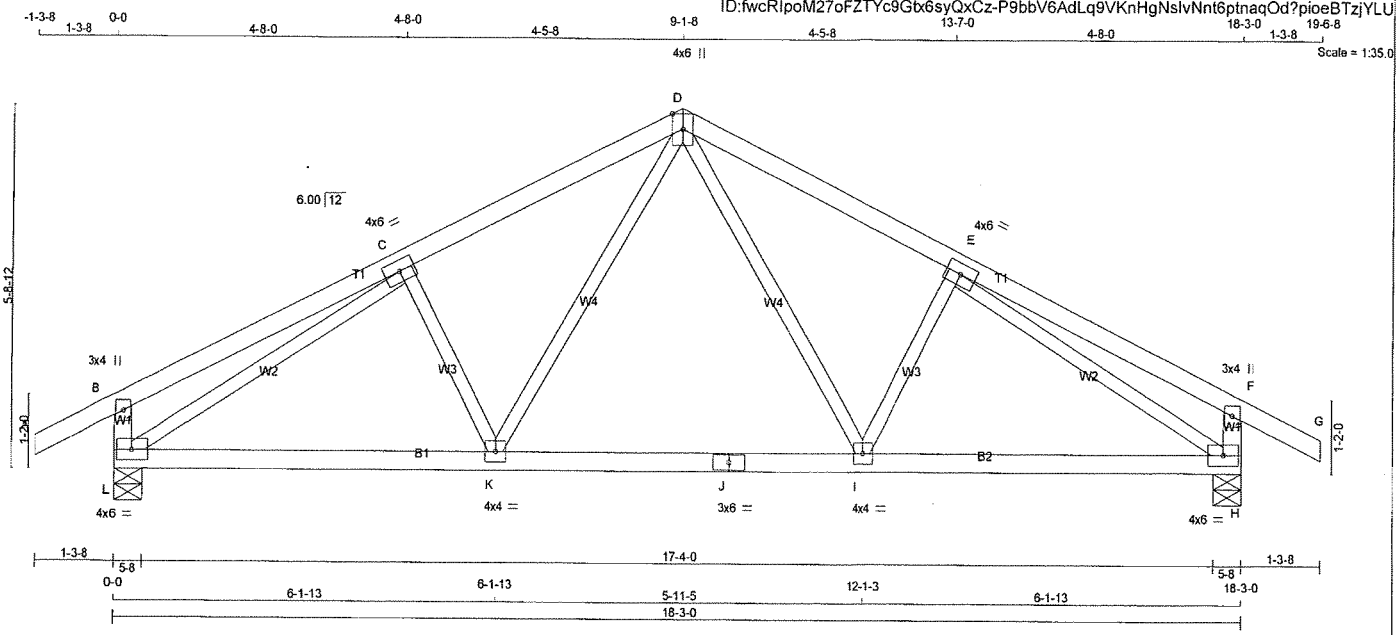


TOTAL WEIGHT = 2 X 73 = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1175	0	1175	0	0	5-8	5-8
H	1175	0	1175	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0
H	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

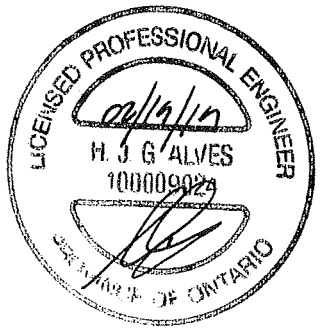
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	D- I	0 / 485	0.11 (1)	
B- C	0 / 18	-77.7	-77.7	0.25 (1)	10.00	I- E	-185 / 36	0.04 (1)	
C- D	-1277 / 0	-77.7	-77.7	0.21 (1)	5.48	K- D	0 / 485	0.11 (1)	
D- E	-1277 / 0	-77.7	-77.7	0.21 (1)	5.48	C- K	-185 / 36	0.04 (1)	
E- F	0 / 18	-77.7	-77.7	0.25 (1)	10.00	L- C	-1468 / 0	0.73 (1)	
F- G	0 / 24	-77.7	-77.7	0.10 (1)	10.00	E- H	-1468 / 0	0.73 (1)	
L- B	-243 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-243 / 0	0.0	0.0	0.02 (1)	7.81				
L- K	0 / 1209	-39.5	-39.5	0.41 (2)	10.00				
K- J	0 / 893	-39.5	-39.5	0.37 (2)	10.00				
J- I	0 / 893	-39.5	-39.5	0.37 (2)	10.00				
I- H	0 / 1209	-39.5	-39.5	0.41 (2)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.41/1.00 (H-I:2) , WB=0.73/1.00 (E-H:1) , SSI=0.15/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

DWG NO. TAM T1903660
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	G67	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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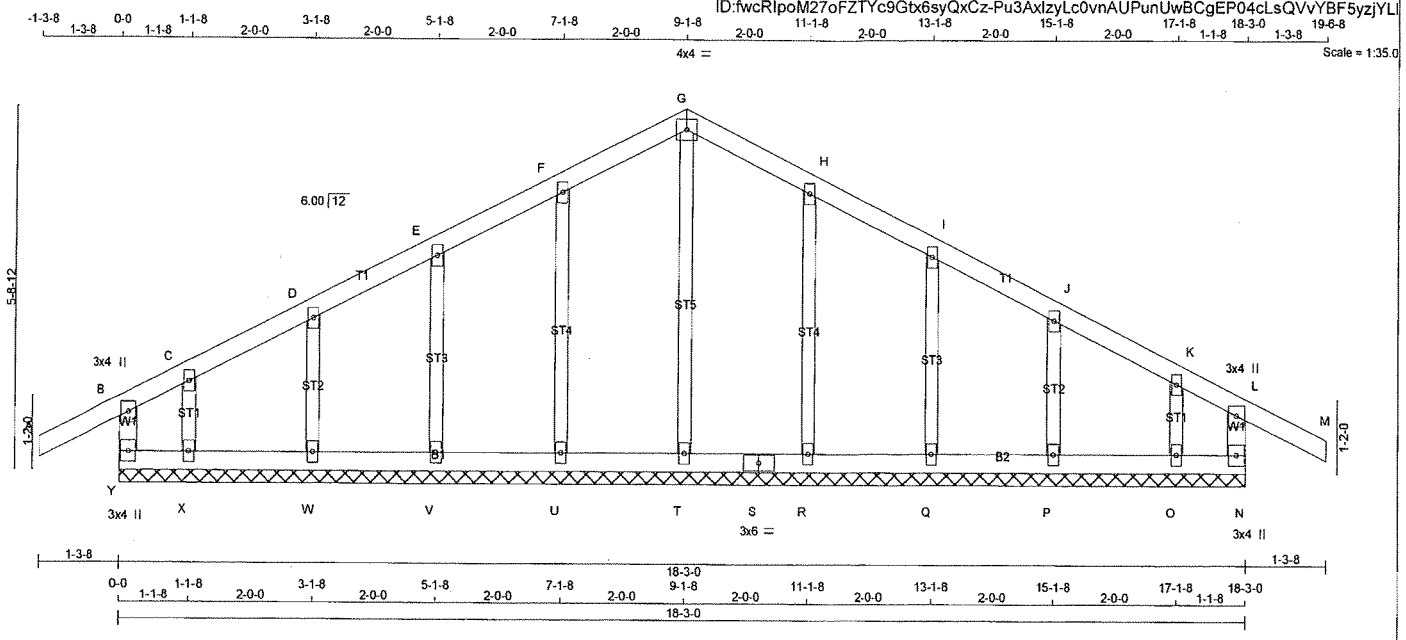


TOTAL WEIGHT = 2 X 71 = 142 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, R, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

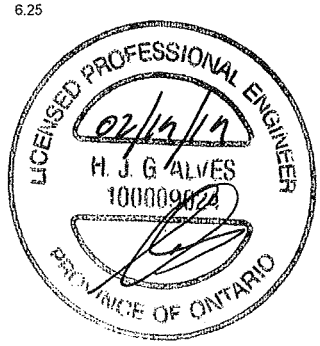
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	
B- C	0 / 0	-77.7	-77.7	0.07 (1)	10.00	
C- D	0 / 24	-77.7	-77.7	0.04 (1)	10.00	
D- E	0 / 24	-77.7	-77.7	0.04 (1)	10.00	
E- F	0 / 29	-77.7	-77.7	0.04 (1)	10.00	
F- G	0 / 31	-77.7	-77.7	0.04 (1)	10.00	
G- H	0 / 31	-77.7	-77.7	0.04 (1)	10.00	
H- I	0 / 29	-77.7	-77.7	0.04 (1)	10.00	
I- J	0 / 24	-77.7	-77.7	0.04 (1)	10.00	
J- K	0 / 24	-77.7	-77.7	0.04 (1)	10.00	
K- L	0 / 0	-77.7	-77.7	0.07 (1)	10.00	
L- M	0 / 24	-77.7	-77.7	0.10 (1)	10.00	
Y- B	-174 / 0	0.0	0.0	0.03 (1)	7.81	
N- L	-174 / 0	0.0	0.0	0.03 (1)	7.81	
Y- X	-15 / 0	-39.5	-39.5	0.02 (3)	6.25	
X- W	-18 / 0	-39.5	-39.5	0.03 (3)	6.25	
W- V	-23 / 0	-39.5	-39.5	0.03 (3)	6.25	
V- U	-25 / 0	-39.5	-39.5	0.02 (3)	6.25	
U- T	-28 / 0	-39.5	-39.5	0.02 (3)	6.25	
T- S	-28 / 0	-39.5	-39.5	0.02 (3)	6.25	
S- R	-28 / 0	-39.5	-39.5	0.02 (3)	6.25	
R- Q	-25 / 0	-39.5	-39.5	0.02 (3)	6.25	
Q- P	-23 / 0	-39.5	-39.5	0.03 (3)	6.25	
P- O	-18 / 0	-39.5	-39.5	0.03 (3)	6.25	
O- N	-15 / 0	-39.5	-39.5	0.02 (3)	6.25	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- G	-186 / 0	0.09 (1)
U- F	-160 / 0	0.05 (1)
V- E	-150 / 0	0.03 (1)
W- D	-164 / 0	0.03 (1)
X- C	-74 / 0	0.01 (1)
R- H	-160 / 0	0.05 (1)
Q- I	-150 / 0	0.03 (1)
P- J	-164 / 0	0.03 (1)
O- K	-74 / 0	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (V-W:3) , WB=0.09/1.00 (G-T:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (J) (INPUT = 0.90)
JSI METAL= 0.07 (J) (INPUT = 1.00)

DWG NO. TAM T903669
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T68	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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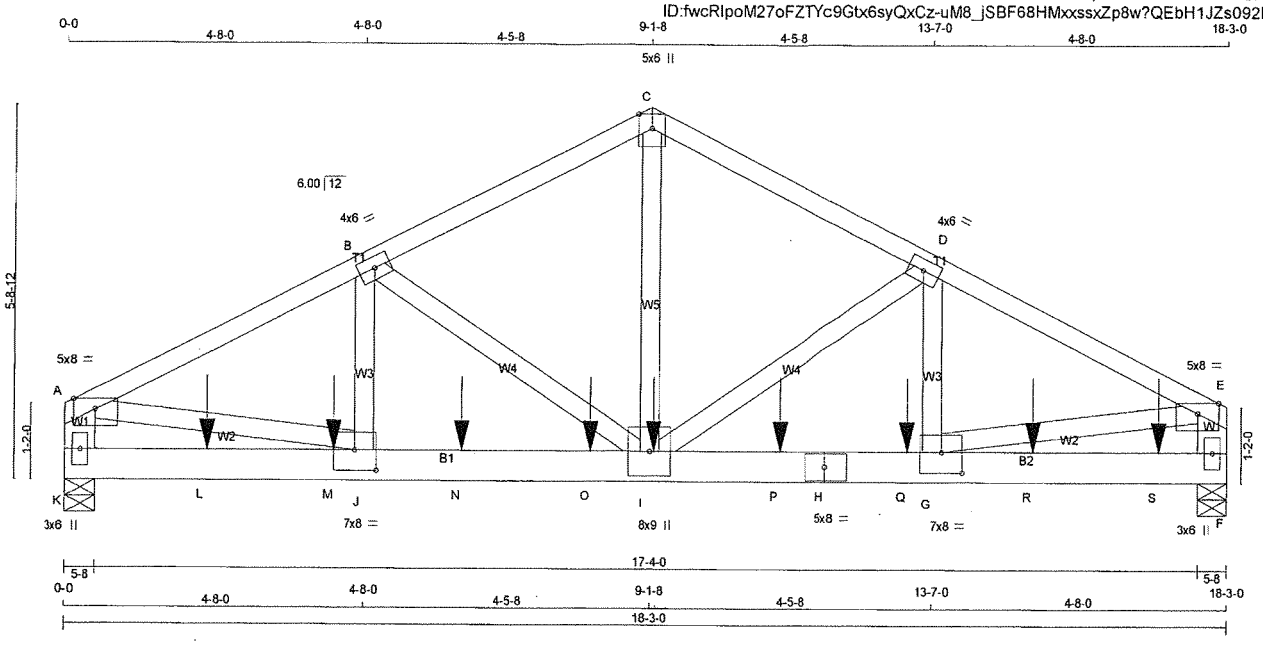


TOTAL WEIGHT = 2 X 91 = 183 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
I- C	1	6	SIDE(78.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	3.75	4.00
H	BS-t	MT20	5.0	8.0		
I	BMWWW+t	MT20	8.0	9.0		
J	BMWW-t	MT20	7.0	8.0	3.75	4.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	5644	0	5644	0	0	5-8	5-8
F	5953	0	5953	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4269	2129 / 0	897 / 0	0 / 0	0 / 0	1243 / 0	0 / 0
F	4516	2212 / 0	979 / 0	0 / 0	0 / 0	1325 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
A- B	-8205 / 0	-77.7	-77.7	0.44 (1)	3.17		I- C	0 / 5316	0.47 (1)
B- C	-6285 / 0	-77.7	-77.7	0.29 (1)	3.72		I- D	-1875 / 0	0.33 (1)
C- D	-6286 / 0	-77.7	-77.7	0.28 (1)	3.72		G- D	0 / 1436	0.13 (1)
D- E	-7982 / 0	-77.7	-77.7	0.42 (1)	3.23		B- I	-2117 / 0	0.37 (1)
K- A	-5051 / 0	0.0	0.0	0.18 (1)	6.48		J- B	0 / 1675	0.15 (1)
F- E	-4918 / 0	0.0	0.0	0.17 (1)	6.55		A- J	0 / 7348	0.66 (1)
							G- E	0 / 7247	0.64 (1)
K- L	0 / 0	-39.5	-39.5	0.44 (1)	10.00				
L- M	0 / 0	-39.5	-39.5	0.44 (1)	10.00				
M- J	0 / 0	-39.5	-39.5	0.44 (1)	10.00				
J- N	0 / 7348	-39.5	-39.5	0.83 (1)	10.00				
N- O	0 / 7348	-39.5	-39.5	0.83 (1)	10.00				
O- I	0 / 7348	-39.5	-39.5	0.83 (1)	10.00				
I- P	0 / 7150	-39.5	-39.5	0.77 (1)	10.00				
P- H	0 / 7150	-39.5	-39.5	0.77 (1)	10.00				
H- Q	0 / 7150	-39.5	-39.5	0.77 (1)	10.00				
Q- G	0 / 7150	-39.5	-39.5	0.77 (1)	10.00				
G- R	0 / 0	-39.5	-39.5	0.40 (1)	10.00				
R- S	0 / 0	-39.5	-39.5	0.40 (1)	10.00				
S- F	0 / 0	-39.5	-39.5	0.40 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	9-2-4	-1003	-1003	---	BACK	VERT	TOTAL	---	---
L	2-2-4	-1111	-1111	---	BACK	VERT	TOTAL	---	---
M	4-2-4	-1111	-1111	---	BACK	VERT	TOTAL	---	---
N	6-2-4	-1111	-1111	---	BACK	VERT	TOTAL	---	---
O	8-2-4	-1111	-1111	---	BACK	VERT	TOTAL	---	---
P	11-2-4	-1003	-1003	---	BACK	VERT	TOTAL	---	---
Q	13-2-4	-1003	-1003	---	BACK	VERT	TOTAL	---	---
R	15-2-4	-1003	-1003	---	BACK	VERT	TOTAL	---	---
S	17-2-4	-1003	-1003	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.44/1.00 (A-B:1) , BC=0.83/1.00 (I-J:1) , WB=0.66/1.00 (A-J:1) , SSI=0.60/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

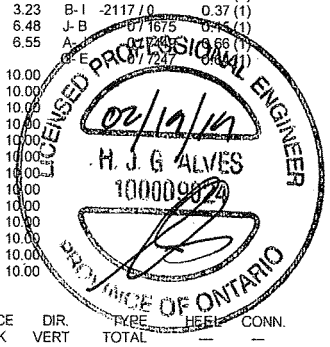
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)
JSI METAL= 0.93 (H) (INPUT = 1.00)



DWG NO. TAM T1903662
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	G69	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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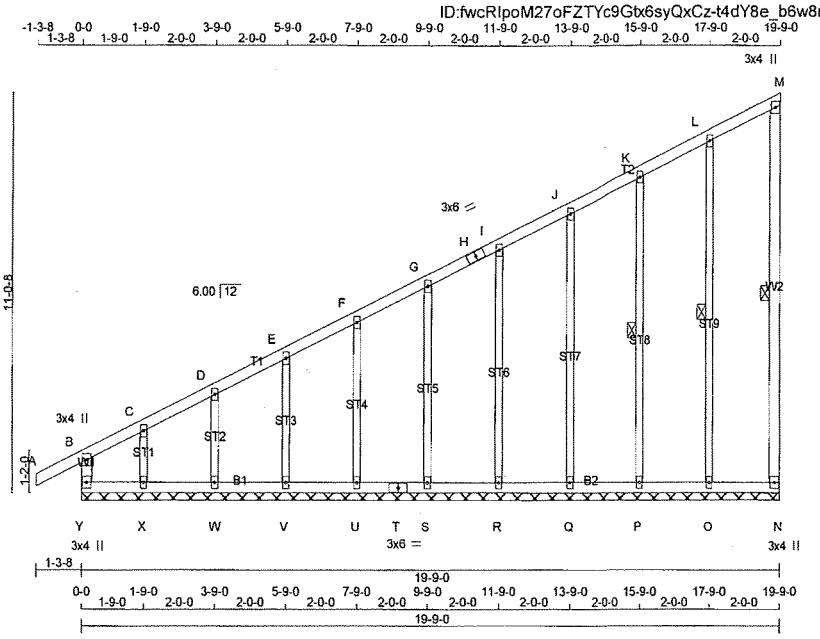


TOTAL WEIGHT = 4 X 102 = 409 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
Y - T	2x4	DRY	No.2	SPF
T - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G, I, J, K, L					
C	TMW+w	MT20	2.0	4.0	
H	TS-t	MT20	3.0	6.0	
M	TMV+p	MT20	3.0	4.0	
N	BMV1+p	MT20	3.0	4.0	
O, P, Q, R, S, U, V, W, X					
O	BMW1+w	MT20	2.0	4.0	
T	BS-t	MT20	3.0	6.0	
Y	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF M-N, L-O, K-P.

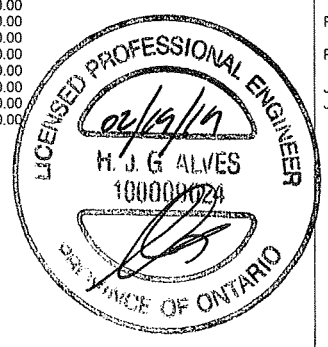
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	
B- C	-36 / 0	-77.7	-77.7	0.08 (1)	6.25	
C- D	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	
D- E	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	
E- F	-11 / 0	-77.7	-77.7	0.04 (1)	6.25	
F- G	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	
G- H	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	
H- I	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	
I- J	-5 / 0	-77.7	-77.7	0.04 (1)	10.00	
J- K	-4 / 0	-77.7	-77.7	0.04 (1)	10.00	
K- L	-2 / 0	-77.7	-77.7	0.04 (1)	10.00	
L- M	-6 / 0	-77.7	-77.7	0.04 (1)	10.00	
N- M	-70 / 0	0.0	0.0	0.01 (1)	6.25	
Y- B	-211 / 0	0.0	0.0	0.04 (1)	7.81	
Y- X	0 / 21	-39.5	-39.5	0.04 (1)	10.00	
X- W	0 / 17	-39.5	-39.5	0.03 (2)	10.00	
W- V	0 / 13	-39.5	-39.5	0.03 (2)	10.00	
V- U	0 / 10	-39.5	-39.5	0.02 (3)	10.00	
U- T	0 / 8	-39.5	-39.5	0.02 (3)	10.00	
T- S	0 / 8	-39.5	-39.5	0.02 (3)	10.00	
S- R	0 / 6	-39.5	-39.5	0.02 (3)	10.00	
R- Q	0 / 5	-39.5	-39.5	0.02 (3)	10.00	
Q- P	0 / 3	-39.5	-39.5	0.02 (3)	10.00	
P- O	0 / 2	-39.5	-39.5	0.03 (3)	10.00	
O- N	0 / 1	-39.5	-39.5	0.03 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- L	-168 / 0	0.12 (1)
P- K	-154 / 0	0.08 (1)
Q- J	-155 / 0	0.19 (1)
R- I	-155 / 0	0.13 (1)
S- G	-154 / 0	0.09 (1)
U- F	-155 / 0	0.06 (1)
V- E	-152 / 0	0.04 (1)
W- D	-164 / 0	0.03 (1)
X- C	-103 / 0	0.02 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.04/1.00 (X-Y:1) , WB=0.19/1.00 (J-Q:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM T903663
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T70	1	2	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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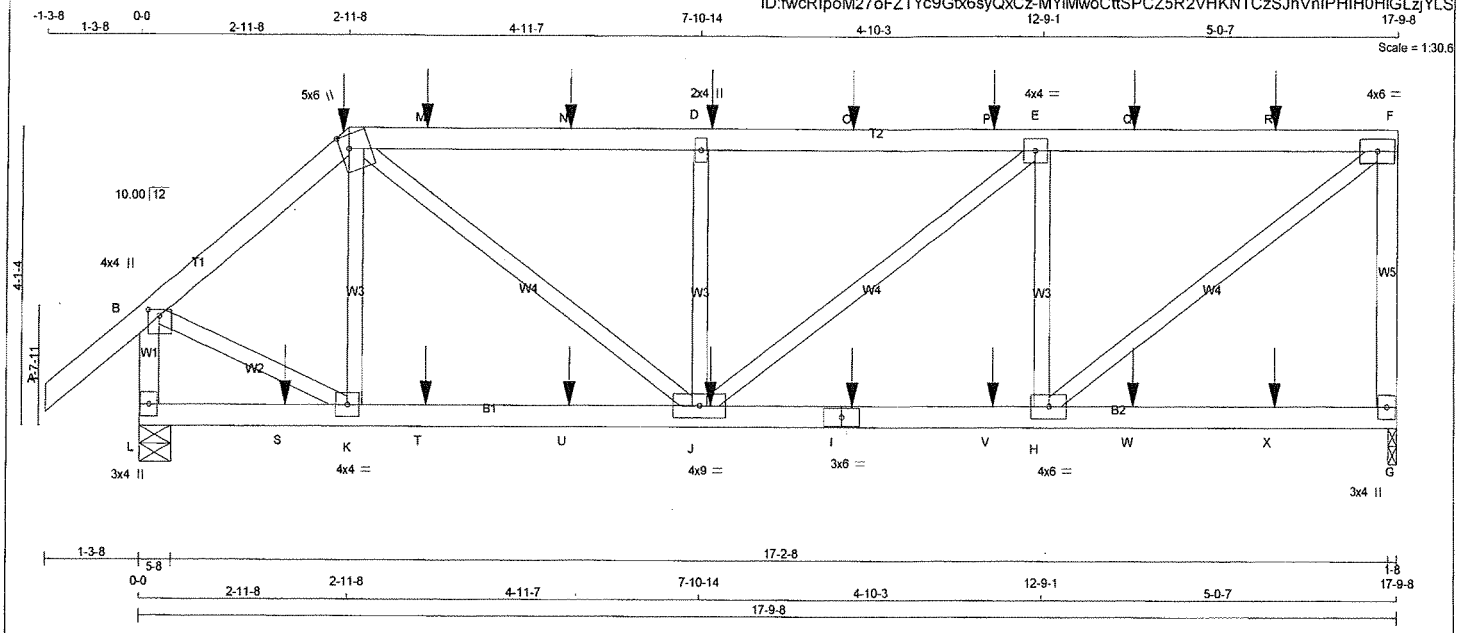


TOTAL WEIGHT = 2 X 74 = 149 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	1	12	SIDE(61.0)
I- G	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1677	0	1677	0	0	1-8	1-8
L	1839	0	1839	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1285	591 / 0	308 / 0	0 / 0	0 / 0	386 / 0	0 / 0
L	1395	683 / 0	305 / 0	0 / 0	0 / 0	407 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

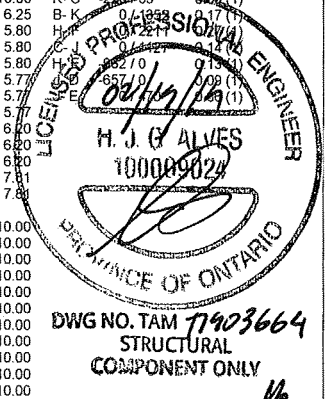
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00
B- C	-1625 / 0	-77.7	-77.7	0.07 (1)	6.25
C- M	-2132 / 0	-77.7	-77.7	0.23 (1)	5.80
M- N	-2132 / 0	-77.7	-77.7	0.23 (1)	5.80
N- D	-2132 / 0	-77.7	-77.7	0.23 (1)	5.80
D- O	-2132 / 0	-77.7	-77.7	0.25 (1)	5.77
O- P	-2132 / 0	-77.7	-77.7	0.25 (1)	5.77
P- E	-2132 / 0	-77.7	-77.7	0.25 (1)	5.77
E- Q	-1762 / 0	-77.7	-77.7	0.24 (1)	6.20
Q- R	-1762 / 0	-77.7	-77.7	0.24 (1)	6.20
R- F	-1762 / 0	-77.7	-77.7	0.24 (1)	6.20
G- F	-1558 / 0	0.0	0.0	0.21 (1)	7.81
L- B	-1798 / 0	0.0	0.0	0.10 (1)	7.81
L- S	0 / 0	-39.5	-39.5	0.12 (3)	10.00
S- K	0 / 0	-39.5	-39.5	0.12 (3)	10.00
K- T	0 / 1237	-39.5	-39.5	0.21 (2)	10.00
T- U	0 / 1237	-39.5	-39.5	0.21 (2)	10.00
U- J	0 / 1237	-39.5	-39.5	0.21 (2)	10.00
J- I	0 / 1762	-39.5	-39.5	0.30 (2)	10.00
I- V	0 / 1762	-39.5	-39.5	0.30 (2)	10.00
V- H	0 / 1762	-39.5	-39.5	0.30 (2)	10.00
H- W	0 / 0	-39.5	-39.5	0.17 (3)	10.00
W- X	0 / 0	-39.5	-39.5	0.17 (3)	10.00
X- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00

WEBS MEMB.	FORCE (LBS) FR-TO	MAX. FACTORED VERT. LOAD (PLF)	CSI (LC)
K- C	-228 / 85	0.03 (1)	
B- K	0 / 1858	0.17 (1)	
H- C	0 / 2211	0.20 (1)	
C- J	0 / 442	0.12 (3)	
H- J	382 / 0	0.13 (1)	
J- E	657 / 0	0.09 (1)	
H- E	0 / 1	0.00 (1)	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	8-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
I	10-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
J	8-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
M	4-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
N	6-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
O	10-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
P	12-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	14-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	2-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 77903664
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.30/1.00 (H-J:2) , WB=0.27/1.00 (F-H:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T70	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	4-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
U	6-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
V	12-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
W	14-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
X	16-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100004924
PROVINCE OF ONTARIO

DWG NO. TAM T903664
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T71	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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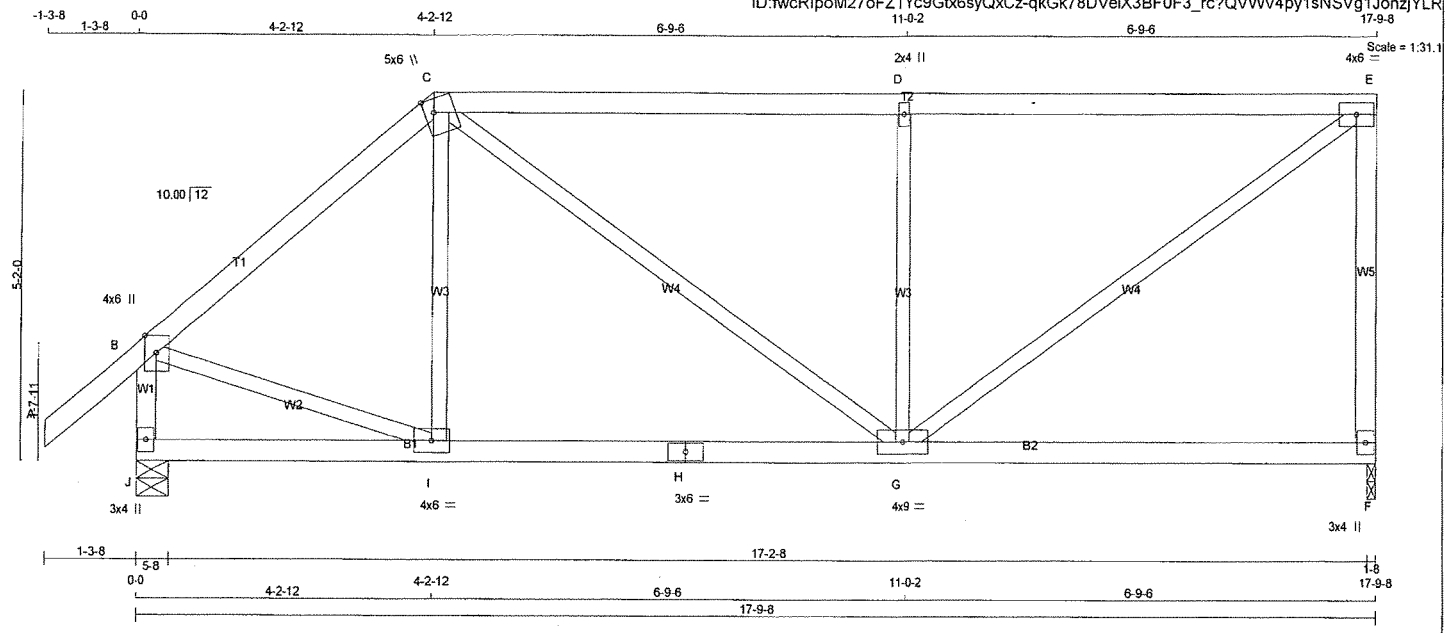


TOTAL WEIGHT = 75 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

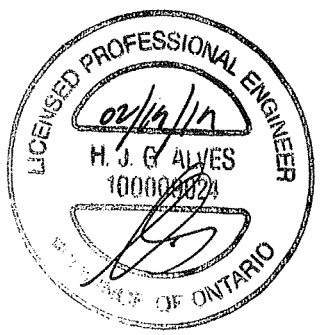
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 165	0.04 (3)
B- C	-936 / 0	-77.7	-77.7	0.26 (1)	6.06	C- G	0 / 380	0.09 (1)
C- D	-1026 / 0	-77.7	-77.7	0.67 (1)	5.11	G- D	-651 / 0	0.26 (1)
D- E	-1026 / 0	-77.7	-77.7	0.67 (1)	5.11	G- E	0 / 1261	0.28 (1)
F- E	-935 / 0	0.0	0.0	0.43 (1)	7.81	B- I	0 / 749	0.17 (1)
J- B	-1089 / 0	0.0	0.0	0.12 (1)	7.57			
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00			
I- H	0 / 718	-39.5	-39.5	0.43 (2)	10.00			
H- G	0 / 718	-39.5	-39.5	0.43 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.35 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.28/1.00 (E-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM P903665
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T72	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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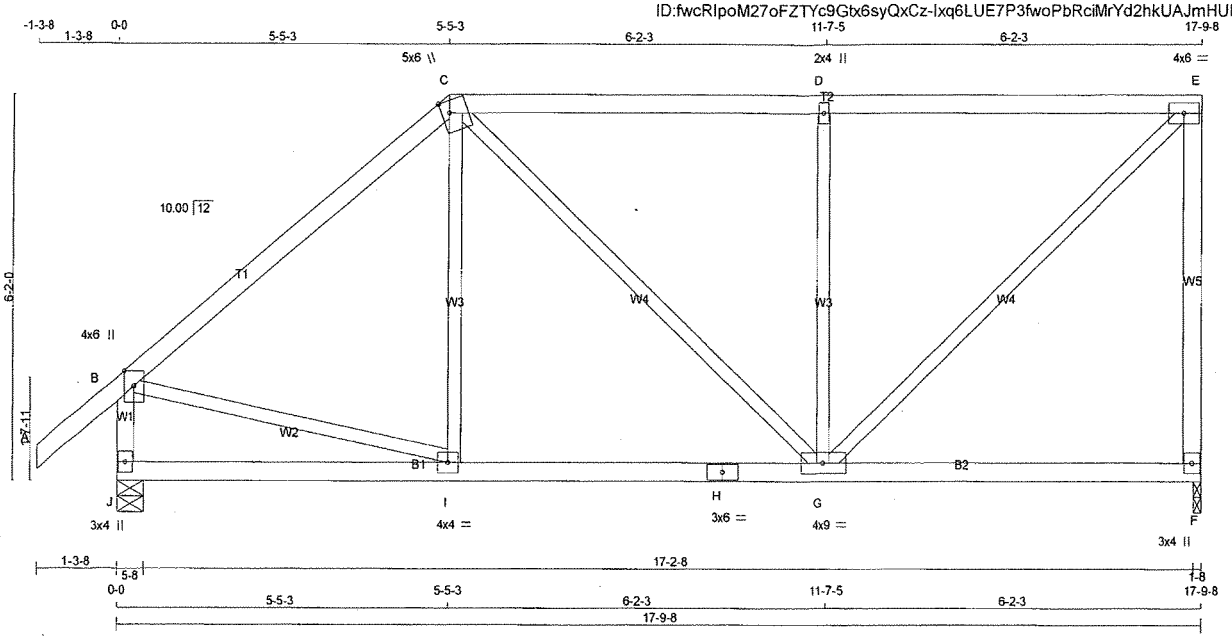

TOTAL WEIGHT = 2 X 79 = 158 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 218	0.05 (3)
B- C	-905 / 0	-77.7	-77.7	0.44 (1)	5.85	C- G	0 / 150	0.03 (1)
C- D	-804 / 0	-77.7	-77.7	0.53 (1)	5.90	G- D	-594 / 0	0.36 (1)
D- E	-804 / 0	-77.7	-77.7	0.53 (1)	5.90	G- E	0 / 1106	0.25 (1)
F- E	-943 / 0	0.0	0.0	0.70 (1)	7.81	B- I	0 / 713	0.16 (1)
J- B	-1063 / 0	0.0	0.0	0.11 (1)	7.64			
J- I	0 / 0	-39.5	-39.5	0.23 (3)	10.00			
I- H	0 / 695	-39.5	-39.5	0.36 (2)	10.00			
H- G	0 / 695	-39.5	-39.5	0.36 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.70/1.00 (E-F:1) , BC=0.36/1.00 (G-I:2) , WB=0.36/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T903666
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T73	2	1	UNITS 21 - 23 TRUSS DESC.	

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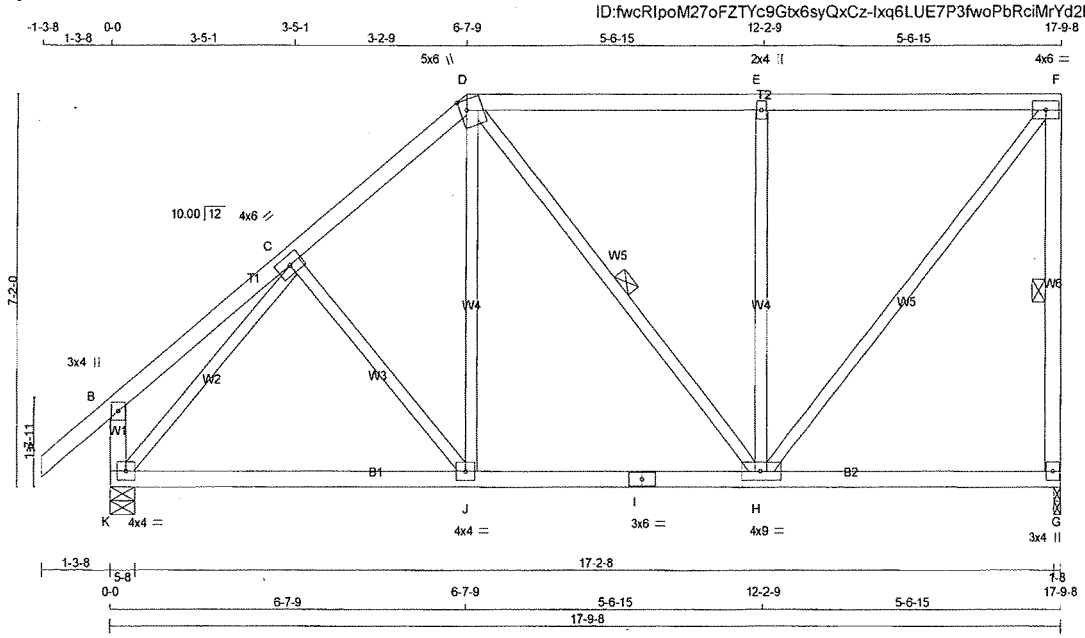

TOTAL WEIGHT = 2 X 86 = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 18	-77.7	-77.7	0.13 (1)	10.00
C- D	-871 / 0	-77.7	-77.7	0.14 (1)	6.25
D- E	-640 / 0	-77.7	-77.7	0.42 (1)	6.25
E- F	-641 / 0	-77.7	-77.7	0.42 (1)	6.25
G- F	-950 / 0	0.0	0.0	0.22 (1)	6.25
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 677	-39.5	-39.5	0.37 (2)	10.00
J- I	0 / 658	-39.5	-39.5	0.37 (2)	10.00
I- H	0 / 658	-39.5	-39.5	0.37 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-37 / 58	0.02 (1)
J- D	0 / 343	0.08 (2)
D- H	-51 / 0	0.03 (3)
H- E	-536 / 0	0.48 (1)
H- F	0 / 1012	0.23 (1)
K- C	-1074 / 0	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.42/1.00 (E-F:1) , BC=0.37/1.00 (H-J:2) , WB=0.48/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

DWG NO. TAM T1903667
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T74	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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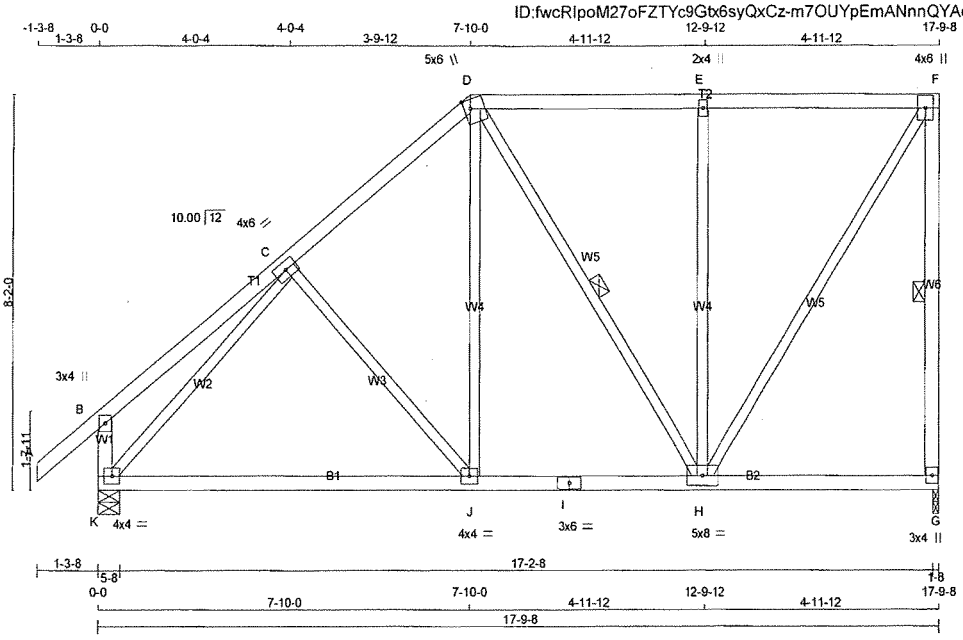


TOTAL WEIGHT = 2 X 92 = 183 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

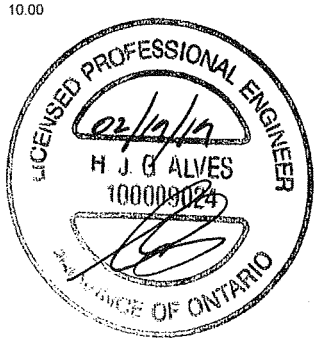
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-114 / 31	0.07 (1)
B- C	0 / 22	-77.7	-77.7	0.19 (1)	10.00	J- D	0 / 438	0.10 (2)
C- D	-813 / 0	-77.7	-77.7	0.20 (1)	6.25	D- H	-192 / 0	0.11 (2)
D- E	-514 / 0	-77.7	-77.7	0.32 (1)	6.25	H- E	-477 / 0	0.62 (1)
E- F	-514 / 0	-77.7	-77.7	0.32 (1)	6.25	H- F	0 / 956	0.22 (1)
G- F	-955 / 0	0.0	0.0	0.29 (1)	6.25	K- C	-1049 / 0	0.63 (1)
K- B	-224 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 682	-39.5	-39.5	0.47 (2)	10.00			
J- I	0 / 611	-39.5	-39.5	0.47 (2)	10.00			
I- H	0 / 611	-39.5	-39.5	0.47 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 797 (0.27")

CSI: TC=0.32/1.00 (E-F:1) , BC=0.47/1.00 (J-K:2) , WB=0.63/1.00 (C-K:1) , SSI=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

DWG NO. TAM T1903668
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T75	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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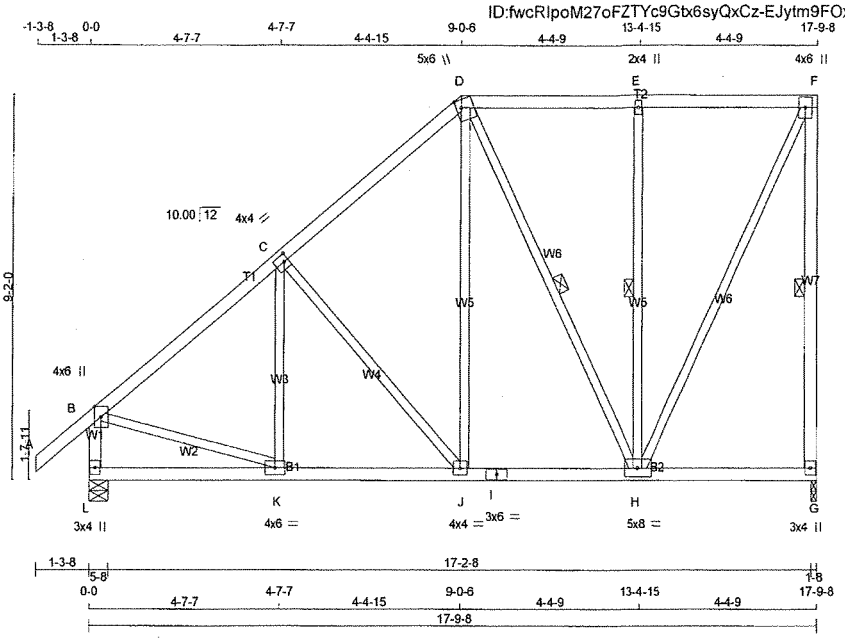


TOTAL WEIGHT = 2 X 99 = 199 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1043	0	1043	0	0	1-8	1-8
L	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
L	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

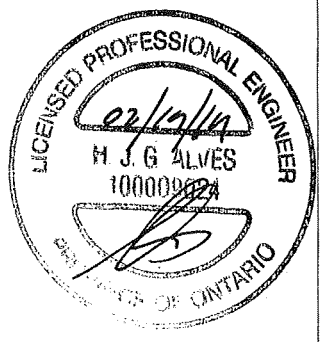
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-5 / 146		0.03 (3)
B- C	-942 / 0	-77.7	-77.7	0.21 (1)	6.14	C- J	-332 / 0		0.29 (1)
C- D	-713 / 0	-77.7	-77.7	0.21 (1)	6.25	J- D	0 / 427		0.10 (2)
D- E	-418 / 0	-77.7	-77.7	0.18 (1)	6.25	D- H	-255 / 0		0.18 (1)
E- F	-418 / 0	-77.7	-77.7	0.18 (1)	6.25	H- E	-418 / 0		0.23 (1)
G- F	-972 / 0	0.0	0.0	0.38 (1)	6.25	H- F	0 / 937		0.21 (1)
L- B	-1076 / 0	0.0	0.0	0.11 (1)	7.61	B- K	0 / 773		0.17 (1)
L- K	0 / 0	-39.5	-39.5	0.16 (3)	10.00				
K- J	0 / 744	-39.5	-39.5	0.25 (2)	10.00				
J- I	0 / 530	-39.5	-39.5	0.21 (2)	10.00				
I- H	0 / 530	-39.5	-39.5	0.21 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.14 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38/1.00 (F-G:1) , BC=0.25/1.00 (J-K:2) , WB=0.29/1.00 (C-J:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. 1AM T1903669
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T76	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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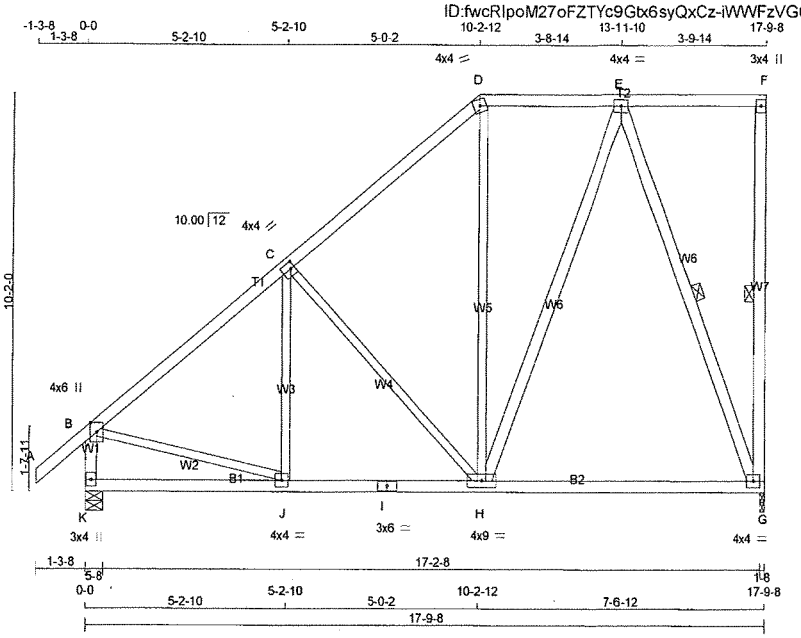


TOTAL WEIGHT = 2 X 104 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

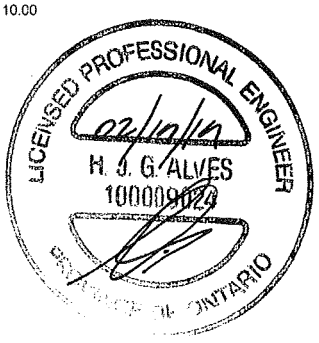
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- C	0 / 147	0.04 (3)
B- C	-929 / 0	-77.7	-77.7	0.28 (1)	6.08	C- H	-389 / 0	0.46 (1)
C- D	-652 / 0	-77.7	-77.7	0.27 (1)	6.25	H- D	0 / 181	0.04 (3)
D- E	-477 / 0	-77.7	-77.7	0.14 (1)	6.25	H- E	0 / 480	0.08 (2)
E- F	0 / 0	-77.7	-77.7	0.18 (1)	10.00	E- G	-867 / 0	0.51 (1)
G- F	-113 / 0	0.0	0.0	0.06 (1)	6.25	B- J	0 / 760	0.17 (1)
K- B	-1061 / 0	0.0	0.0	0.11 (1)	7.64			
K- J	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
J- I	0 / 737	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 737	-39.5	-39.5	0.45 (2)	10.00			
H- G	0 / 313	-39.5	-39.5	0.39 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.45/1.00 (H-J:2) , WB=0.51/1.00 (E-G:1) , SSI=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903670
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T77A	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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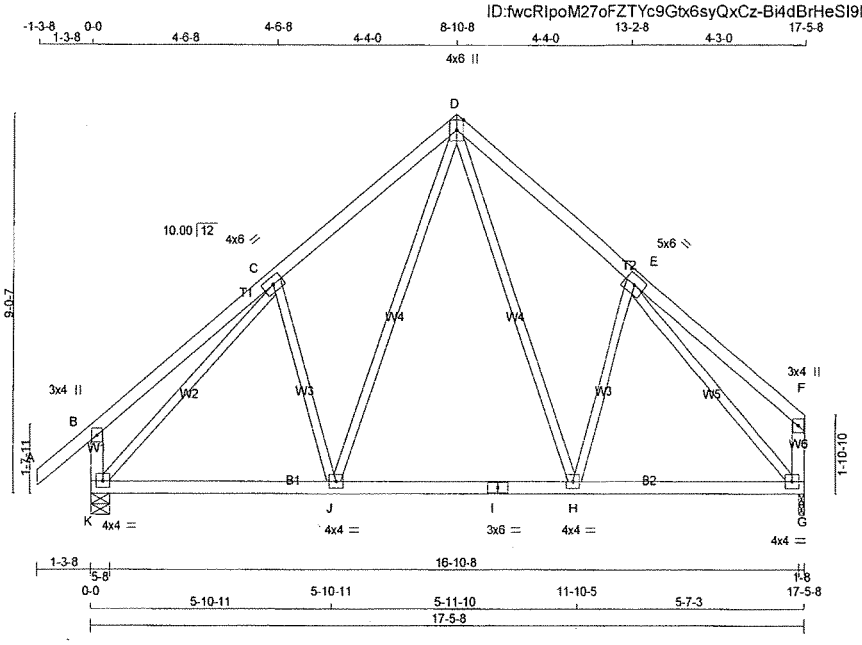

TOTAL WEIGHT = 84 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1131	0	1131	0	0	5-8	5-8
G	1023	0	1023	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	857	423 / 0	183 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	782	365 / 0	183 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 26	-77.7	-77.7	0.25 (1)	10.00
C- D	-875 / 0	-77.7	-77.7	0.20 (1)	6.25
D- E	-849 / 0	-77.7	-77.7	0.19 (1)	6.25
E- F	0 / 26	-77.7	-77.7	0.23 (1)	10.00
K- B	-238 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-119 / 0	0.0	0.0	0.01 (1)	7.81
K- J	0 / 706	-39.5	-39.5	0.33 (2)	10.00
J- I	0 / 508	-39.5	-39.5	0.31 (2)	10.00
I- H	0 / 508	-39.5	-39.5	0.31 (2)	10.00
H- G	0 / 673	-39.5	-39.5	0.31 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-179 / 26	0.08 (1)
J- D	0 / 452	0.10 (1)
K- C	-1066 / 0	0.84 (1)
H- E	-131 / 48	0.06 (1)
D- H	0 / 393	0.09 (1)
E- G	-1057 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.33/1.00 (J-K:2) , WB=0.84/1.00 (C-K:1) , SSI=0.15/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

DWG NO. TAM T7703671
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T77X	5	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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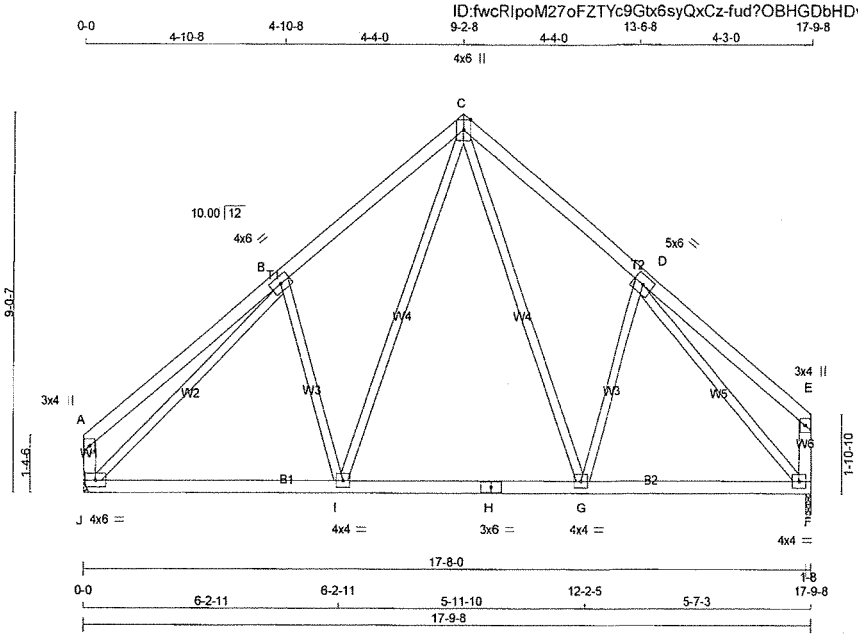

TOTAL WEIGHT = 5 X 83 = 414 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1043	0	1043	0	0	MECHANICAL	
F	1043	0	1043	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-77.7	-77.7	0.26 (1)	10.00
B- C	-929 / 0	-77.7	-77.7	0.22 (1)	6.17
C- D	-871 / 0	-77.7	-77.7	0.19 (1)	6.25
D- E	0 / 26	-77.7	-77.7	0.23 (1)	10.00
J- A	-143 / 0	0.0	0.0	0.01 (1)	7.81
F- E	-119 / 0	0.0	0.0	0.01 (1)	7.81
J- I	0 / 760	-39.5	-39.5	0.36 (2)	10.00
I- H	0 / 528	-39.5	-39.5	0.33 (2)	10.00
H- G	0 / 528	-39.5	-39.5	0.33 (2)	10.00
G- F	0 / 688	-39.5	-39.5	0.31 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	-229 / 4	0.11 (1)
G- D	-126 / 50	0.06 (1)
I- C	0 / 513	0.12 (1)
G- G	0 / 385	0.09 (1)
J- B	-1103 / 0	0.95 (1)
D- F	-1081 / 0	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.36/1.00 (I-J:2) , WB=0.95/1.00 (B-J:1) , SSI=0.15/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

DWG NO. TAM 7903672
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T78	3	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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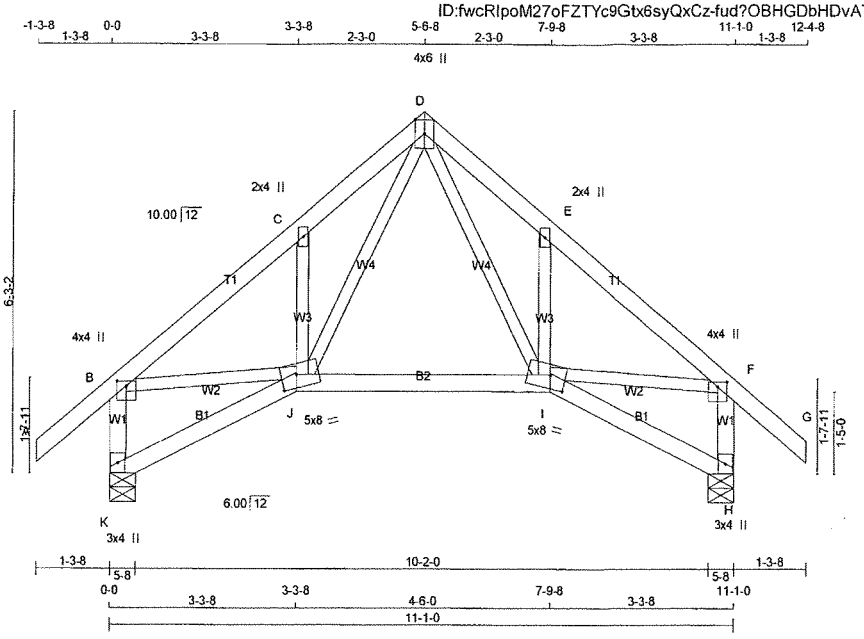


TOTAL WEIGHT = 3 X 54 = 163 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	757	0	757	0	0	5-8	5-8
H	757	0	757	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

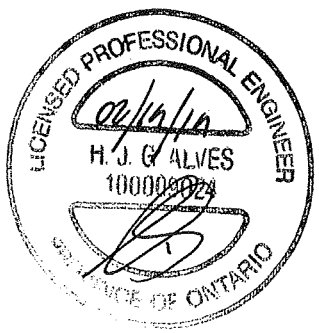
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	D- I	0 / 489	0.11 (1)
B- C	-763 / 0	-77.7	-77.7	0.11 (1)	6.25	I- E	-262 / 0	0.05 (1)
C- D	-794 / 0	-77.7	-77.7	0.07 (1)	6.25	J- D	0 / 489	0.11 (1)
D- E	-794 / 0	-77.7	-77.7	0.07 (1)	6.25	J- C	-262 / 0	0.05 (1)
E- F	-763 / 0	-77.7	-77.7	0.11 (1)	6.25	B- J	0 / 596	0.13 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- F	0 / 596	0.13 (1)
K- B	-692 / 0	0.0	0.0	0.08 (1)	7.81			
H- F	-692 / 0	0.0	0.0	0.08 (1)	7.81			
K- J	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
J- I	0 / 378	-39.5	-39.5	0.23 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.10 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.23/1.00 (I-J:2) , WB=0.13/1.00 (F-I:1) , SSI=0.10/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM T1903673
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T79	1	2	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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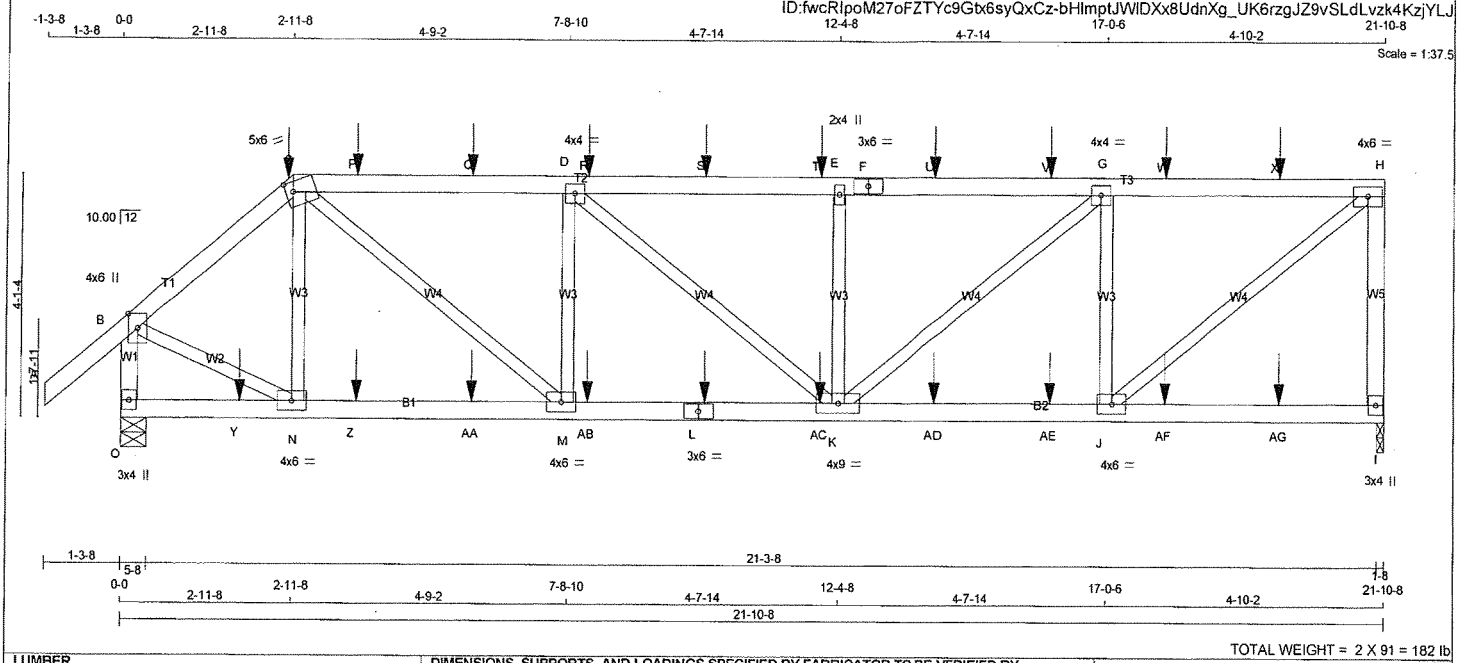


TOTAL WEIGHT = 2 X 91 = 182 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(0.1)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	1	12	SIDE(61.0)
L- I	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J, M, N						
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2060	0	2060	0	0	1-8	1-8
O	2234	0	2234	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1580	723 / 0	381 / 0	0 / 0	0 / 0	476 / 0	0 / 0
O	1698	822 / 0	378 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	
B- C	-2029 / 0	-77.7	-77.7	0.08 (1)	6.11	
C- P	-2926 / 0	-77.7	-77.7	0.26 (1)	5.11	
P- Q	-2926 / 0	-77.7	-77.7	0.26 (1)	5.11	
Q- D	-2926 / 0	-77.7	-77.7	0.26 (1)	5.11	
D- R	-3075 / 0	-77.7	-77.7	0.26 (1)	5.00	
R- S	-3075 / 0	-77.7	-77.7	0.26 (1)	5.00	
S- T	-3075 / 0	-77.7	-77.7	0.26 (1)	5.00	
T- E	-3075 / 0	-77.7	-77.7	0.26 (1)	5.00	
E- F	-3075 / 0	-77.7	-77.7	0.27 (1)	4.99	
F- U	-3075 / 0	-77.7	-77.7	0.27 (1)	4.99	
U- V	-3075 / 0	-77.7	-77.7	0.27 (1)	4.99	
V- G	-3075 / 0	-77.7	-77.7	0.27 (1)	4.99	
G- W	-2213 / 0	-77.7	-77.7	0.25 (1)	5.69	
W- X	-2213 / 0	-77.7	-77.7	0.25 (1)	5.69	
X- H	-2213 / 0	-77.7	-77.7	0.25 (1)	5.69	
I- H	-1948 / 0	0.0	0.0	0.26 (1)	7.81	
O- B	-2190 / 0	0.0	0.0	0.13 (1)	7.52	
O- Y	0 / 0	-39.5	-39.5	0.10 (3)	10.00	
Y- N	0 / 0	-39.5	-39.5	0.10 (3)	10.00	
N- Z	0 / 1542	-39.5	-39.5	0.23 (2)	10.00	
Z-AA	0 / 1542	-39.5	-39.5	0.23 (2)	10.00	
AA- M	0 / 1542	-39.5	-39.5	0.23 (2)	10.00	
M-AB	0 / 2926	-39.5	-39.5	0.35 (2)	10.00	
AB- L	0 / 2926	-39.5	-39.5	0.35 (2)	10.00	
L-AC	0 / 2926	-39.5	-39.5	0.35 (2)	10.00	
AC- K	0 / 2926	-39.5	-39.5	0.35 (2)	10.00	
K-AD	0 / 2213	-39.5	-39.5	0.33 (2)	10.00	
AD-AE	0 / 2213	-39.5	-39.5	0.33 (2)	10.00	
AE- J	0 / 2213	-39.5	-39.5	0.33 (2)	10.00	
J-AF	0 / 0	-39.5	-39.5	0.16 (3)	10.00	
AF-AG	0 / 0	-39.5	-39.5	0.16 (3)	10.00	
AG- I	0 / 0	-39.5	-39.5	0.16 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-373 / 0	0.05 (1)
B- N	0 / 1688	0.21 (1)
J- H	0 / 2816	0.35 (1)
C- M	0 / 1768	0.22 (1)
J- G	-1379 / 0	0.19 (1)
M- D	-778 / 0	0.10 (1)
K- G	0 / 1115	0.14 (1)
D- K	-607 / 133	0.02 (1)
K- E	-533 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
L	10-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.27/1.00 (E-G:1) , BC=0.35/1.00 (K-M:2) , WB=0.35/1.00 (H-J:1) , SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.54 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100019084
PROVINCE OF ONTARIO
02/19/19

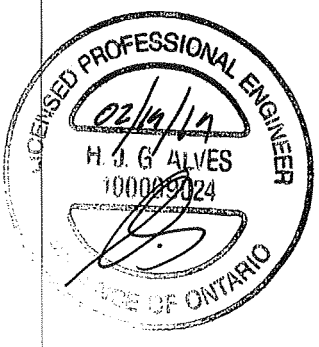
DWG NO. TAM 7903674
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T79	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	4-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	6-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	8-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	10-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
T	12-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
U	14-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
V	16-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
W	18-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
X	20-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Y	2-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Z	4-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AA	6-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AB	8-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AC	12-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AD	14-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AE	16-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AF	18-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AG	20-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM T1203674
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T80	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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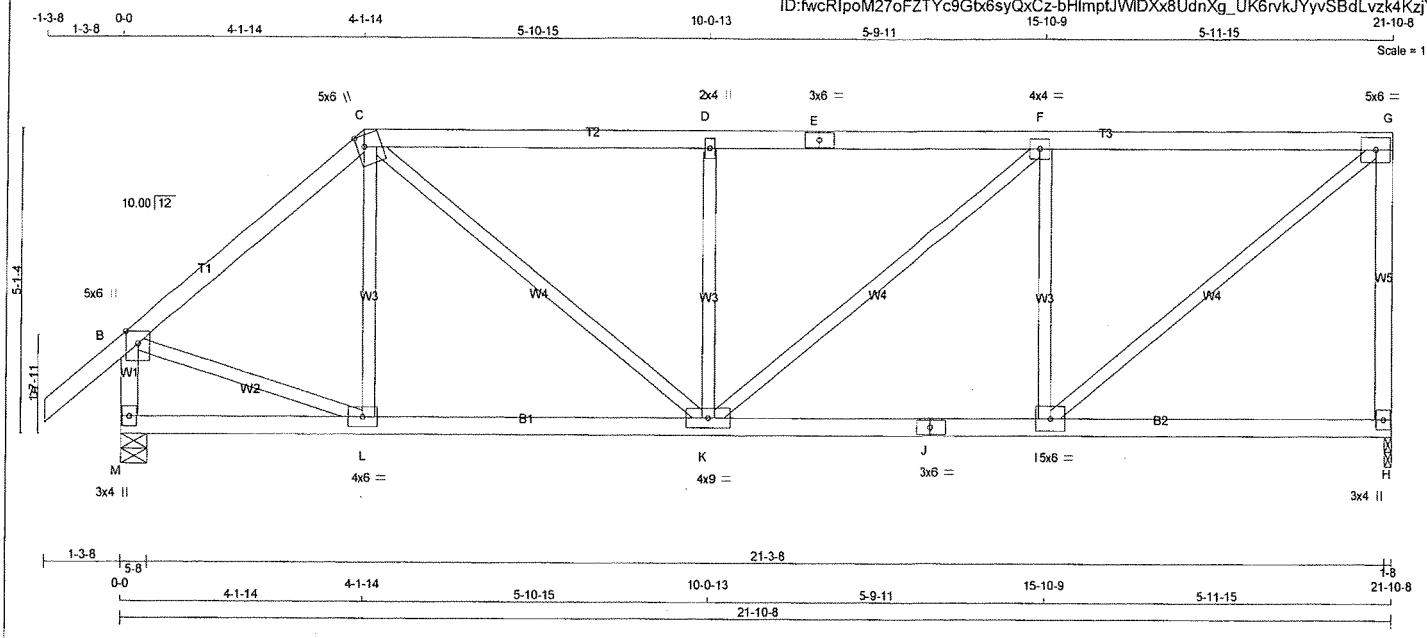


TOTAL WEIGHT = 92 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1282	0	1282	0	0	1-8	1-8
M	1389	0	1389	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
M	1055	515 / 0	230 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

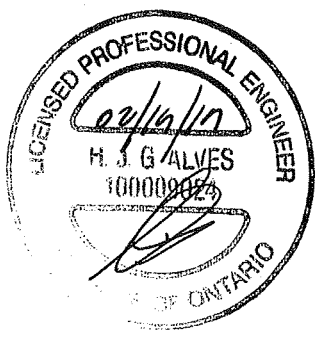
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-59 / 121	0.03 (3)	
B- C	-1209 / 0	-77.7	-77.7	0.27 (1)	5.51	B- L	0 / 969	0.22 (1)	
C- D	-1519 / 0	-77.7	-77.7	0.42 (1)	4.84	I- G	0 / 1594	0.36 (1)	
D- E	-1519 / 0	-77.7	-77.7	0.44 (1)	4.82	C- K	0 / 765	0.17 (1)	
E- F	-1519 / 0	-77.7	-77.7	0.44 (1)	4.82	I- F	-734 / 0	0.28 (1)	
F- G	-1243 / 0	-77.7	-77.7	0.42 (1)	5.23	K- D	-491 / 0	0.19 (1)	
H- G	-1186 / 0	0.0	0.0	0.52 (1)	7.33	K- F	0 / 359	0.08 (1)	
M- B	-1329 / 0	0.0	0.0	0.14 (1)	7.02				
M- L	0 / 0	-39.5	-39.5	0.19 (3)	10.00				
L- K	0 / 925	-39.5	-39.5	0.31 (2)	10.00				
K- J	0 / 1243	-39.5	-39.5	0.42 (2)	10.00				
J- I	0 / 1243	-39.5	-39.5	0.42 (2)	10.00				
I- H	0 / 0	-39.5	-39.5	0.26 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.52/1.00 (G-H:1) , BC=0.42/1.00 (I-K:2) , WB=0.36/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T7903675
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T81	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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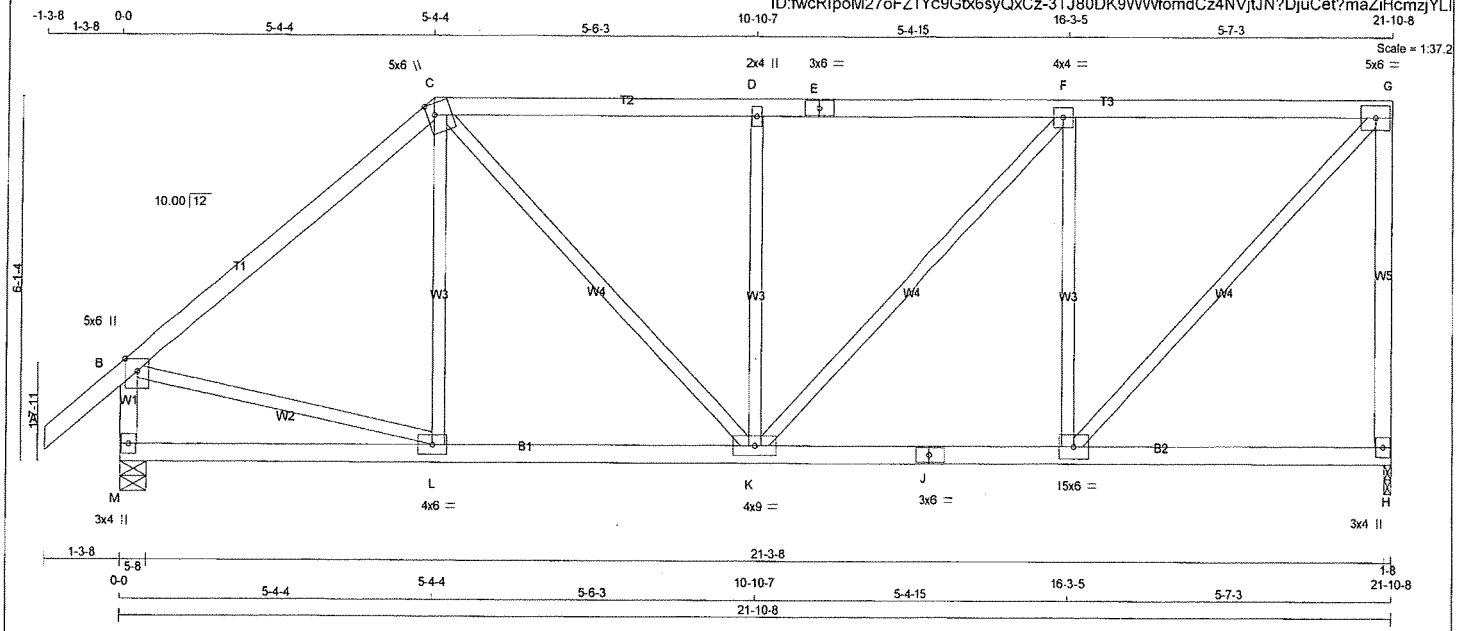


TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1282	0	1282	0	0	1-8	1-8
M	1389	0	1389	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
M	1055	515 / 0	230 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

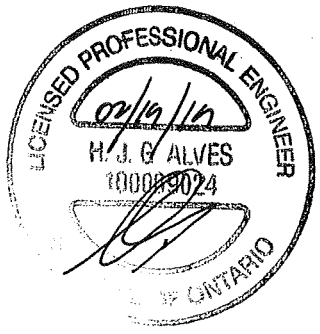
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	0 / 186	0.04 (3)
B- C	-1198 / 0	-77.7	-77.7	0.46 (1)	5.27	B- L	0 / 944	0.21 (1)
C- D	-1256 / 0	-77.7	-77.7	0.35 (1)	5.31	I- G	0 / 1412	0.32 (1)
D- E	-1256 / 0	-77.7	-77.7	0.36 (1)	5.29	C- K	0 / 490	0.11 (1)
E- F	-1256 / 0	-77.7	-77.7	0.36 (1)	5.29	I- F	-768 / 0	0.45 (1)
F- G	-979 / 0	-77.7	-77.7	0.35 (1)	5.82	K- D	-458 / 0	0.27 (1)
H- G	-1193 / 0	0.0	0.0	0.86 (1)	7.32	K- F	0 / 407	0.09 (1)
M- B	-1304 / 0	0.0	0.0	0.14 (1)	7.07			
M- L	0 / 0	-39.5	-39.5	0.23 (3)	10.00			
L- K	0 / 919	-39.5	-39.5	0.35 (2)	10.00			
K- J	0 / 979	-39.5	-39.5	0.36 (2)	10.00			
J- I	0 / 979	-39.5	-39.5	0.36 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.86/1.00 (G-H:1) , BC=0.36/1.00 (I-K:2) , WB=0.45/1.00 (F-I:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (I) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1703676
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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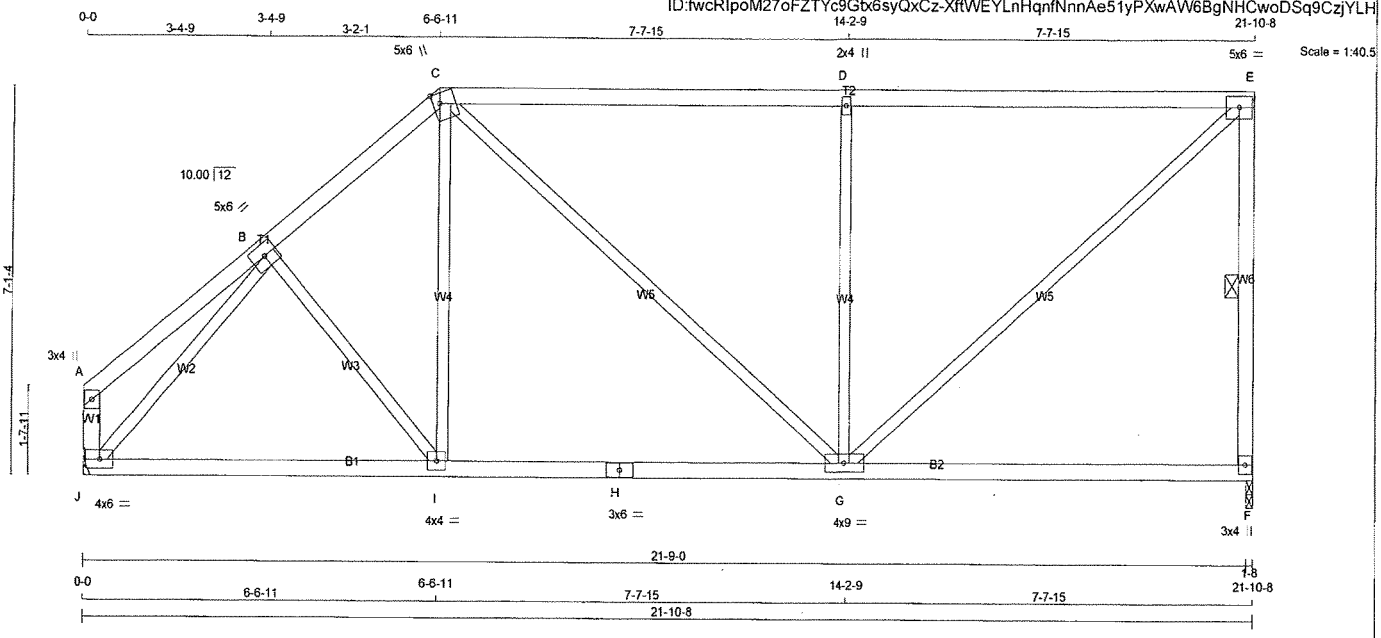


TOTAL WEIGHT = 96 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1282	0	1282	0	0	1-8	1-8
J	1282	0	1282	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
J	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

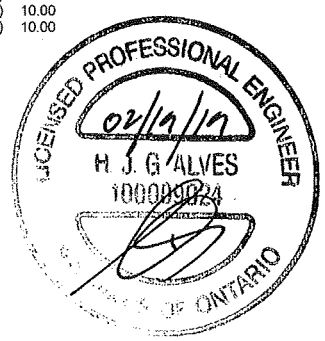
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-77.7	-77.7	0.12 (1)	10.00	B- I	0 / 94	0.02 (3)
B- C	-1181 / 0	-77.7	-77.7	0.14 (1)	5.72	I- C	0 / 331	0.07 (2)
C- D	-1054 / 0	-77.7	-77.7	0.89 (1)	4.44	C- G	0 / 213	0.05 (1)
D- E	-1055 / 0	-77.7	-77.7	0.89 (1)	4.44	G- D	-738 / 0	0.64 (1)
F- E	-1160 / 0	0.0	0.0	0.26 (1)	5.92	G- E	0 / 1408	0.32 (1)
J- A	-100 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1387 / 0	0.60 (1)
J- I	0 / 873	-39.5	-39.5	0.44 (2)	10.00			
I- H	0 / 896	-39.5	-39.5	0.53 (2)	10.00			
H- G	0 / 896	-39.5	-39.5	0.53 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.43 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.89/1.00 (D-E:1) , BC=0.53/1.00 (G-I:2) , WB=0.64/1.00 (D-G:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)

DWG NO. TAM 17703677
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T83	1	1	TRUSS DESC.	

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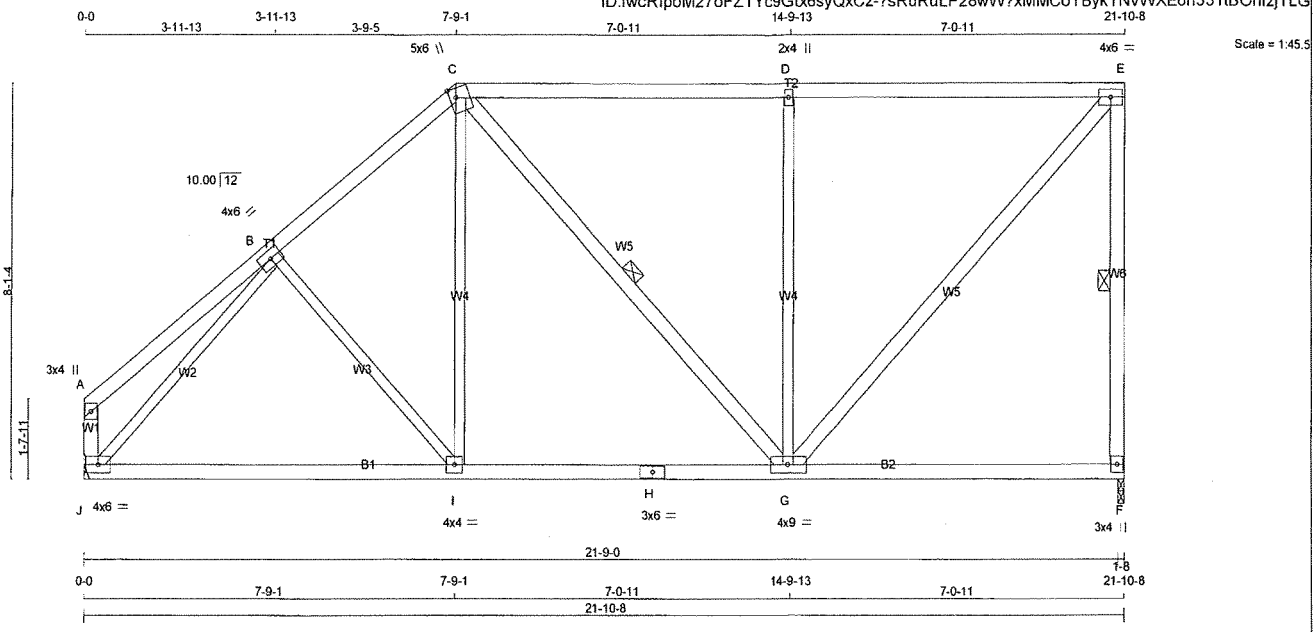


TOTAL WEIGHT = 107 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1282	0	1282	0	0	1-8	1-8
J	1282	0	1282	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
J	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

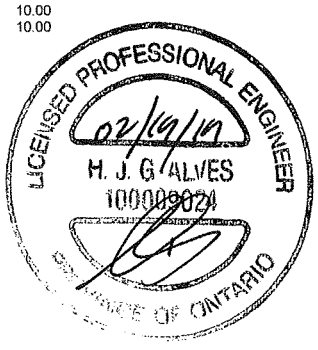
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-77.7	-77.7	0.18 (1)	10.00	B- I	-56 / 63	0.04 (1)
B- C	-1130 / 0	-77.7	-77.7	0.21 (1)	5.73	I- C	0 / 417	0.09 (2)
C- D	-871 / 0	-77.7	-77.7	0.72 (1)	5.27	C- G	-27 / 25	0.02 (3)
D- E	-871 / 0	-77.7	-77.7	0.72 (1)	5.27	G- D	-680 / 0	0.86 (1)
F- E	-1167 / 0	0.0	0.0	0.35 (1)	5.90	G- E	0 / 1295	0.21 (1)
J- A	-116 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1366 / 0	0.81 (1)
J- I	0 / 887	-39.5	-39.5	0.51 (2)	10.00			
I- H	0 / 854	-39.5	-39.5	0.51 (2)	10.00			
H- G	0 / 854	-39.5	-39.5	0.51 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.33 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.72/1.00 (D-E:1) , BC=0.51/1.00 (G-I:2) , WB=0.86/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

DWG NO. TAM 17903678
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T84	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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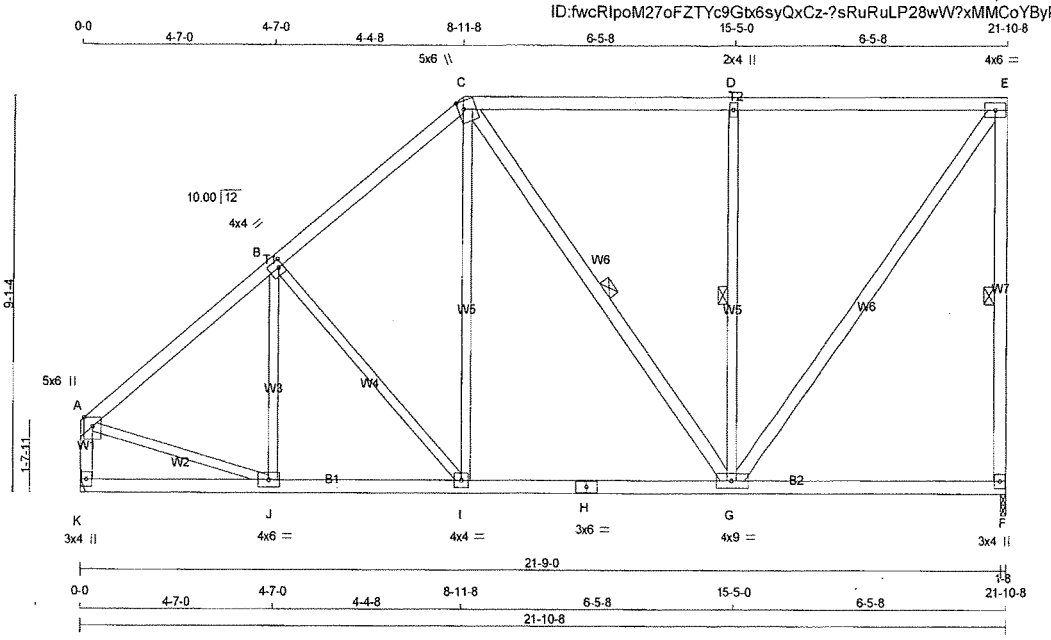


TOTAL WEIGHT = 115 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1282	0	1282	0	0	1-8	1-8
K	1282	0	1282	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
K	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

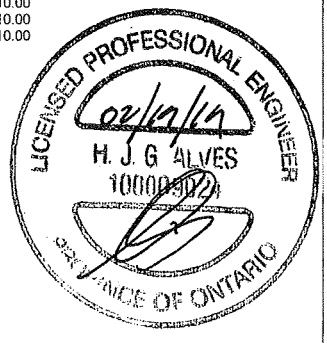
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1218 / 0	-77.7	-77.7	0.30 (1)	5.46
B- C	-1042 / 0	-77.7	-77.7	0.29 (1)	5.79
C- D	-729 / 0	-77.7	-77.7	0.58 (1)	5.99
D- E	-729 / 0	-77.7	-77.7	0.58 (1)	5.99
F- E	-1180 / 0	0.0	0.0	0.46 (1)	5.88
K- A	-1206 / 0	0.0	0.0	0.13 (1)	7.29
K- J	0 / 0	-39.5	-39.5	0.14 (3)	10.00
J- I	0 / 956	-39.5	-39.5	0.29 (2)	10.00
I- H	0 / 782	-39.5	-39.5	0.42 (2)	10.00
H- G	0 / 782	-39.5	-39.5	0.42 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.32 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-82 / 94	0.04 (1)
B- I	-271 / 0	0.23 (1)
I- C	0 / 436	0.10 (2)
C- G	-90 / 0	0.06 (2)
G- D	-622 / 0	0.34 (1)
G- E	0 / 1229	0.20 (1)
A- J	0 / 994	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.42/1.00 (G-I:2) , WB=0.34/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.44 (A) (INPUT = 1.00)

DWG NO. TAM 1703679
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T85	2	1	TRUSS DESC.	

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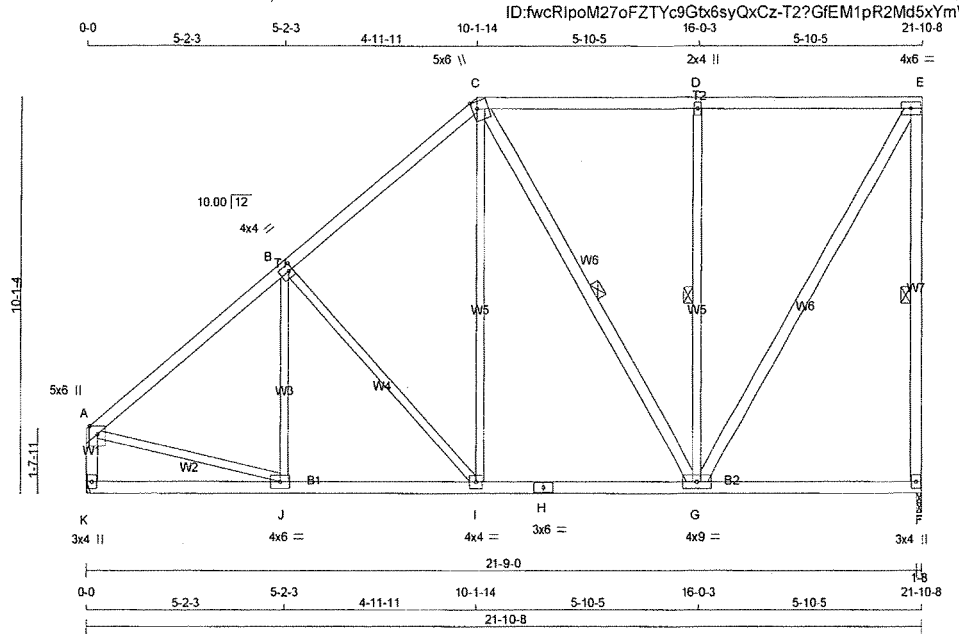


TOTAL WEIGHT = 2 X 121 = 242 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1282	0	1282	0	0	1-8	1-8
K	1282	0	1282	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0
K	980	457 / 0	230 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

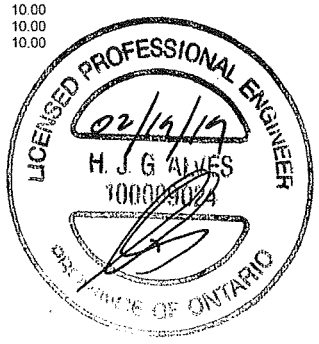
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1222 / 0	-77.7	-77.7	0.40 (1)		5.32
B- C	-966 / 0	-77.7	-77.7	0.37 (1)		5.83
C- D	-610 / 0	-77.7	-77.7	0.46 (1)		6.25
D- E	-610 / 0	-77.7	-77.7	0.46 (1)		6.25
F- E	-1189 / 0	0.0	0.0	0.59 (1)		5.86
K- A	-1199 / 0	0.0	0.0	0.13 (1)		7.30
K- J	0 / 0	-39.5	-39.5	0.19 (3)		10.00
J- I	0 / 962	-39.5	-39.5	0.32 (2)		10.00
I- H	0 / 722	-39.5	-39.5	0.35 (2)		10.00
H- G	0 / 722	-39.5	-39.5	0.35 (2)		10.00
G- F	0 / 0	-39.5	-39.5	0.26 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-16 / 155	0.04 (3)
B- I	-366 / 0	0.42 (1)
I- C	0 / 494	0.11 (2)
C- G	-218 / 0	0.16 (1)
G- D	-563 / 0	0.39 (1)
G- E	0 / 1184	0.19 (1)
A- J	0 / 992	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.59/1.00 (E-F:1) , BC=0.35/1.00 (G-I:2) , WB=0.42/1.00 (B-I:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T7203680
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T86	1	2	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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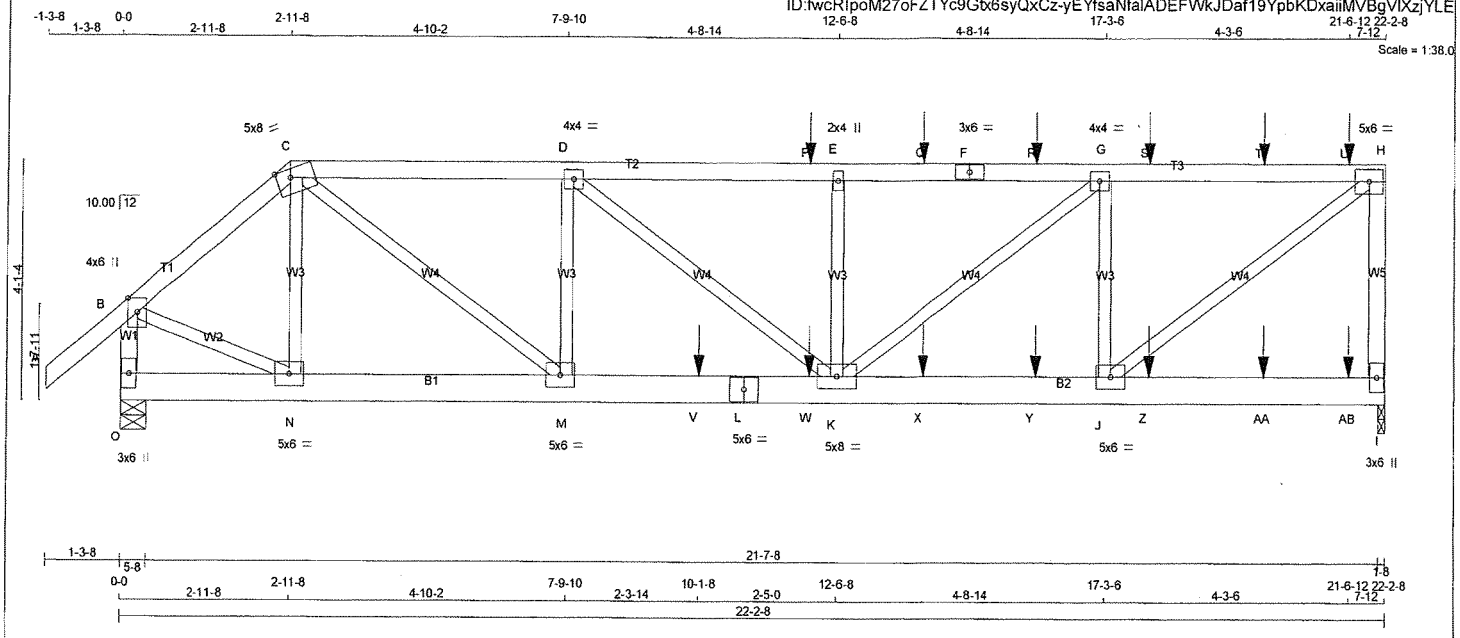


TOTAL WEIGHT = 2 X 105 = 210 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(0.0)
F- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(0.0)
L- I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	2447	0	2447	0	0	1-8	1-8
O	2143	0	2143	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1868	880 / 0	431 / 0	0 / 0	0 / 0	557 / 0	0 / 0
O	1622	805 / 0	344 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)		10.00
B- C	-1959 / 0	-77.7	-77.7	0.08 (1)		6.20
C- D	-3443 / 0	-77.7	-77.7	0.19 (1)		4.88
D- P	-3860 / 0	-77.7	-77.7	0.20 (1)		4.65
P- E	-3860 / 0	-77.7	-77.7	0.20 (1)		4.65
E- Q	-3860 / 0	-77.7	-77.7	0.32 (1)		4.49
Q- F	-3860 / 0	-77.7	-77.7	0.32 (1)		4.49
F- R	-3860 / 0	-77.7	-77.7	0.32 (1)		4.49
R- G	-3860 / 0	-77.7	-77.7	0.32 (1)		4.49
G- S	-2609 / 0	-77.7	-77.7	0.29 (1)		5.30
S- T	-2609 / 0	-77.7	-77.7	0.29 (1)		5.30
T- U	-2609 / 0	-77.7	-77.7	0.29 (1)		5.30
U- H	-2609 / 0	-77.7	-77.7	0.29 (1)		5.30
I- H	-2262 / 0	0.0	0.0	0.28 (1)		7.43
O- B	-2082 / 0	0.0	0.0	0.12 (1)		7.68
O- N	0 / 0	-39.5	-39.5	0.02 (2)		10.00
N- M	0 / 1485	-39.5	-39.5	0.23 (1)		10.00
M- V	0 / 3443	-39.5	-39.5	0.56 (1)		10.00
V- L	0 / 3443	-39.5	-39.5	0.56 (1)		10.00
L- W	0 / 3443	-39.5	-39.5	0.56 (1)		10.00
W- K	0 / 3443	-39.5	-39.5	0.56 (1)		10.00
K- X	0 / 2609	-39.5	-39.5	0.29 (1)		10.00
X- Y	0 / 2609	-39.5	-39.5	0.29 (1)		10.00
Y- J	0 / 2609	-39.5	-39.5	0.29 (1)		10.00
J- Z	0 / 0	-39.5	-39.5	0.09 (2)		10.00
Z-AA	0 / 0	-39.5	-39.5	0.09 (2)		10.00
AA-AB	0 / 0	-39.5	-39.5	0.09 (2)		10.00
AB- I	0 / 0	-39.5	-39.5	0.09 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-522 / 0	0.07 (1)
B- N	0 / 1614	0.20 (1)
J- H	0 / 3272	0.40 (1)
C- M	0 / 2466	0.31 (1)
J- G	-1695 / 0	0.22 (1)
M- D	-735 / 0	0.09 (1)
K- G	0 / 1591	0.20 (1)
D- K	0 / 530	0.07 (1)
K- E	-497 / 0	0.06 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	12-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	14-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	18-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
T	20-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
U	21-6-12	-107	-107	---	BACK	VERT	TOTAL	---	---
V	10-1-8	-960	-960	---	BACK	VERT	TOTAL	---	---
W	12-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.32/1.00 (E-G:1) , BC=0.56/1.00 (K-M:1) , WB=0.40/1.00 (H-J:1) , SSI=0.20/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

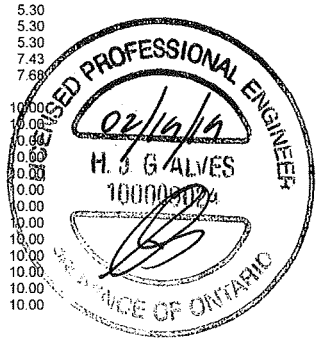
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)



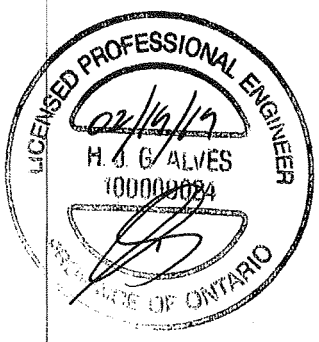
DWG NO. TAM T7903681
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T86	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	14-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
Y	16-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
Z	18-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AA	20-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AB	21-6-12	-63	-80	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM 77903681
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T87	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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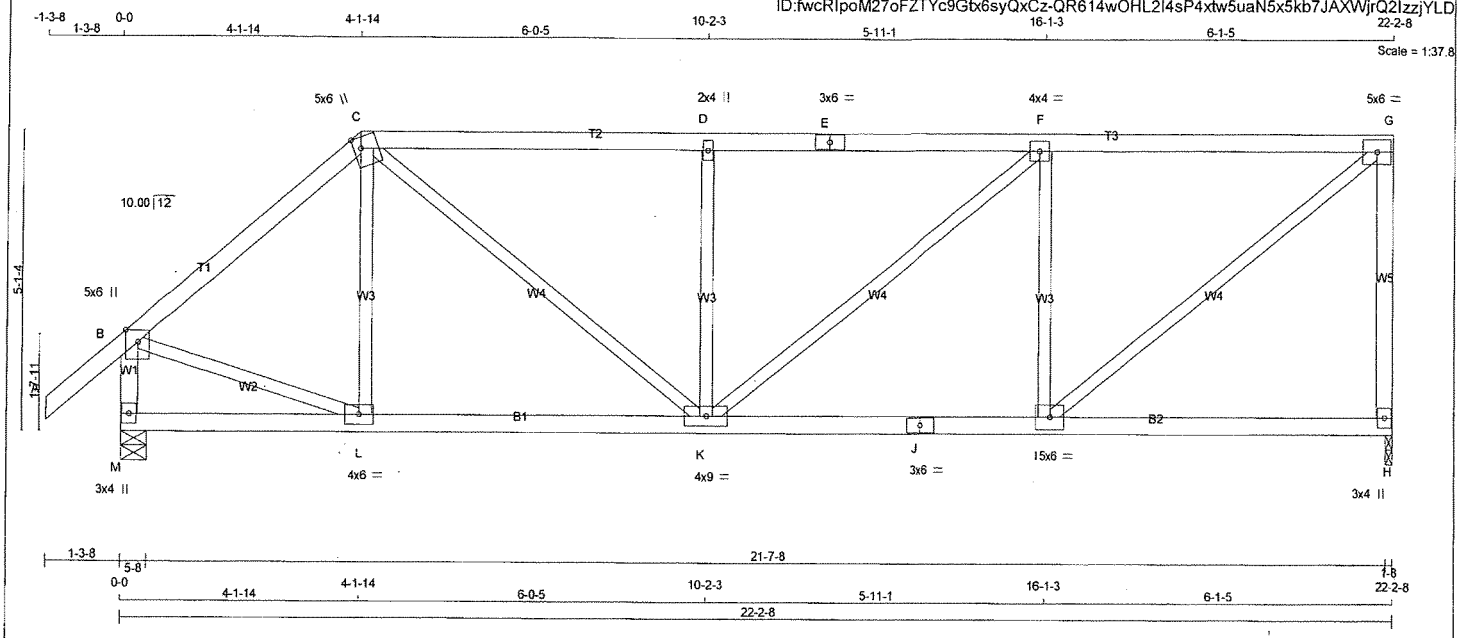


TOTAL WEIGHT = 93 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1301	0	1301	0	0	1-8	1-8
M	1409	0	1409	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	995	464 / 0	233 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1069	522 / 0	233 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

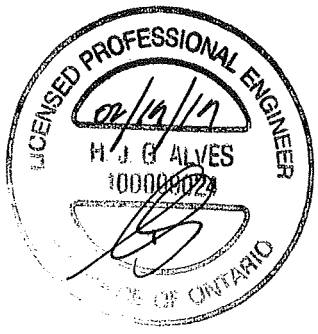
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	1	0.11 (1)	10.00	L- C	-62 / 121	0.03 (3)
B- C	-1232 / 0	-77.7	-77.7	1	0.27 (1)	5.47	B- L	0 / 987	0.22 (1)
C- D	-1566 / 0	-77.7	-77.7	1	0.45 (1)	4.75	I- G	0 / 1635	0.37 (1)
D- E	-1566 / 0	-77.7	-77.7	1	0.46 (1)	4.73	C- K	0 / 797	0.18 (1)
E- F	-1566 / 0	-77.7	-77.7	1	0.46 (1)	4.73	I- F	-743 / 0	0.29 (1)
F- G	-1284 / 0	-77.7	-77.7	1	0.44 (1)	5.13	K- D	-500 / 0	0.19 (1)
H- G	-1204 / 0	0.0	0.0	1	0.53 (1)	7.29	K- F	0 / 363	0.08 (1)
M- B	-1350 / 0	0.0	0.0	1	0.14 (1)	6.98			
M- L	0 / 0	-39.5	-39.5	3	0.19 (3)	10.00			
L- K	0 / 942	-39.5	-39.5	2	0.32 (2)	10.00			
K- J	0 / 1284	-39.5	-39.5	2	0.44 (2)	10.00			
J- I	0 / 1284	-39.5	-39.5	2	0.44 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	3	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.53/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.37/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903682
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T88X	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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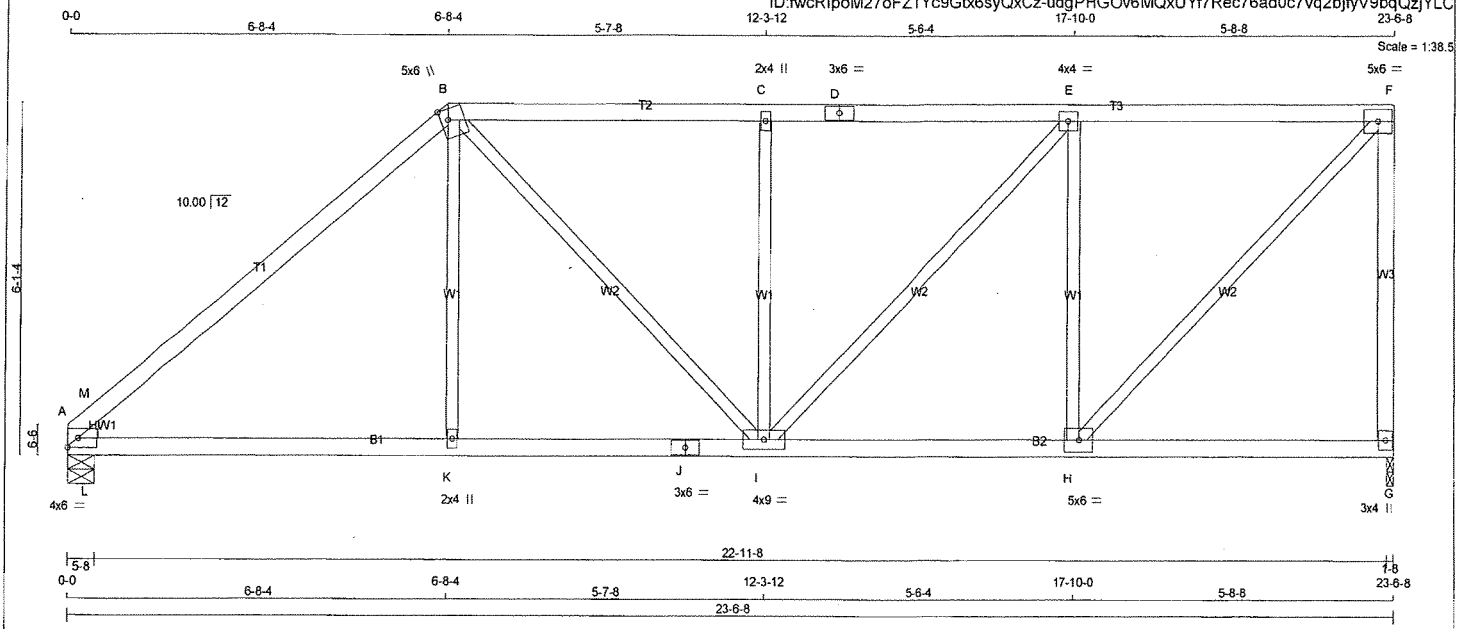

TOTAL WEIGHT = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	6.0		Edge
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
G	1380	0	1380	0	0	1-8	1-8	
A	1380	0	1380	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0
A	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- M	-1552 / 0	-77.7	-77.7	0.29 (1)	4.93		K- B	0 / 364	0.08 (2)
M- B	-1548 / 0	-77.7	-77.7	0.58 (1)	4.71		H- F	0 / 1556	0.35 (1)
B- C	-1443 / 0	-77.7	-77.7	0.38 (1)	5.00		B- I	0 / 381	0.09 (1)
C- D	-1443 / 0	-77.7	-77.7	0.39 (1)	4.98		H- E	-852 / 0	0.50 (1)
D- E	-1443 / 0	-77.7	-77.7	0.39 (1)	4.98		I- C	-466 / 0	0.27 (1)
E- F	-1090 / 0	-77.7	-77.7	0.37 (1)	5.57		I- E	0 / 513	0.12 (1)
G- F	-1290 / 0	0.0	0.0	0.93 (1)	7.10		L- M	-210 / 289	0.00 (1)
A- L	0 / 1172	-39.5	-39.5	0.35 (1)	10.00				
L- K	0 / 1172	-39.5	-39.5	0.47 (2)	10.00				
K- J	0 / 1177	-39.5	-39.5	0.47 (2)	10.00				
J- I	0 / 1177	-39.5	-39.5	0.47 (2)	10.00				
I- H	0 / 1090	-39.5	-39.5	0.39 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.93/1.00 (F-G:1) , BC=0.47/1.00 (I-K:2) , WB=0.50/1.00 (E-H:1) , SSI=0.24/1.00 (A-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)

DWG NO. TAM T1403683
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T89X	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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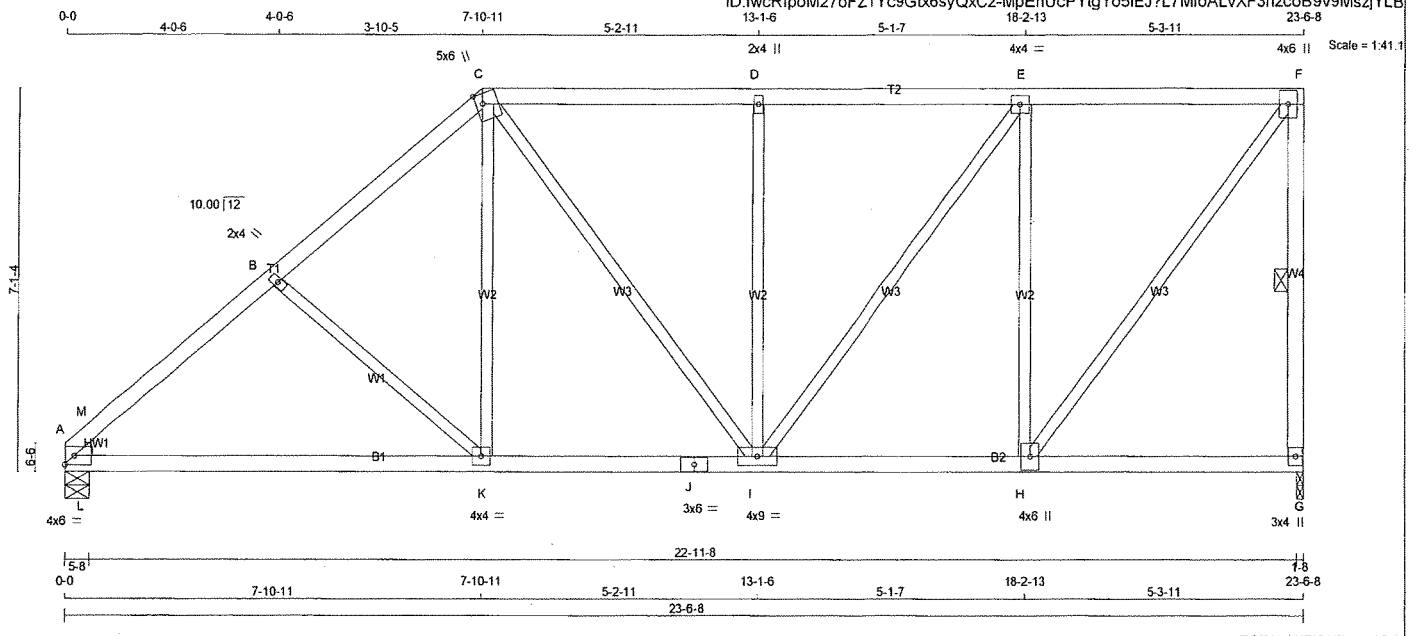


TOTAL WEIGHT = 105 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	6.0		Edge
B	TMW+w	MT20	2.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HEEL WEDGE
G	1380	0	1380	0	0	1-8	1-8	
A	1380	0	1380	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0
A	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

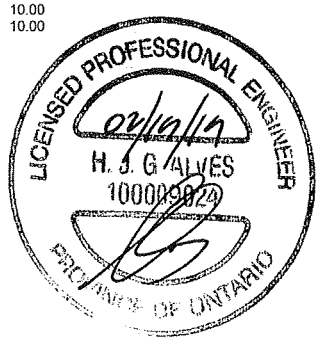
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- M	-2166 / 0	-77.7	-77.7	0.17 (3)	4.61	B- K	-284 / 0	0.14 (1)
M- B	-1669 / 0	-77.7	-77.7	0.22 (1)	4.92	K- C	0 / 497	0.11 (2)
B- C	-1435 / 0	-77.7	-77.7	0.21 (1)	5.23	H- F	0 / 1431	0.32 (1)
C- D	-1213 / 0	-77.7	-77.7	0.31 (1)	5.44	C- I	0 / 199	0.04 (1)
D- E	-1213 / 0	-77.7	-77.7	0.32 (1)	5.41	H- E	-889 / 0	0.78 (1)
E- F	-880 / 0	-77.7	-77.7	0.31 (1)	6.13	I- D	-433 / 0	0.38 (1)
G- F	-1296 / 0	0.0	0.0	0.30 (1)	5.67	I- E	0 / 555	0.12 (1)
						L- M	0 / 650	0.00 (1)
A- L	0 / 1300	-39.5	-39.5	0.43 (2)	10.00			
L- K	0 / 1300	-39.5	-39.5	0.47 (2)	10.00			
K- J	0 / 1091	-39.5	-39.5	0.45 (2)	10.00			
J- I	0 / 1091	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 880	-39.5	-39.5	0.33 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.47/1.00 (K-L:2) , WB=0.78/1.00 (E-H:1) , SSI=0.36/1.00 (A-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (H) (INPUT = 1.00)

DWG NO. TAM T203684
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T90X	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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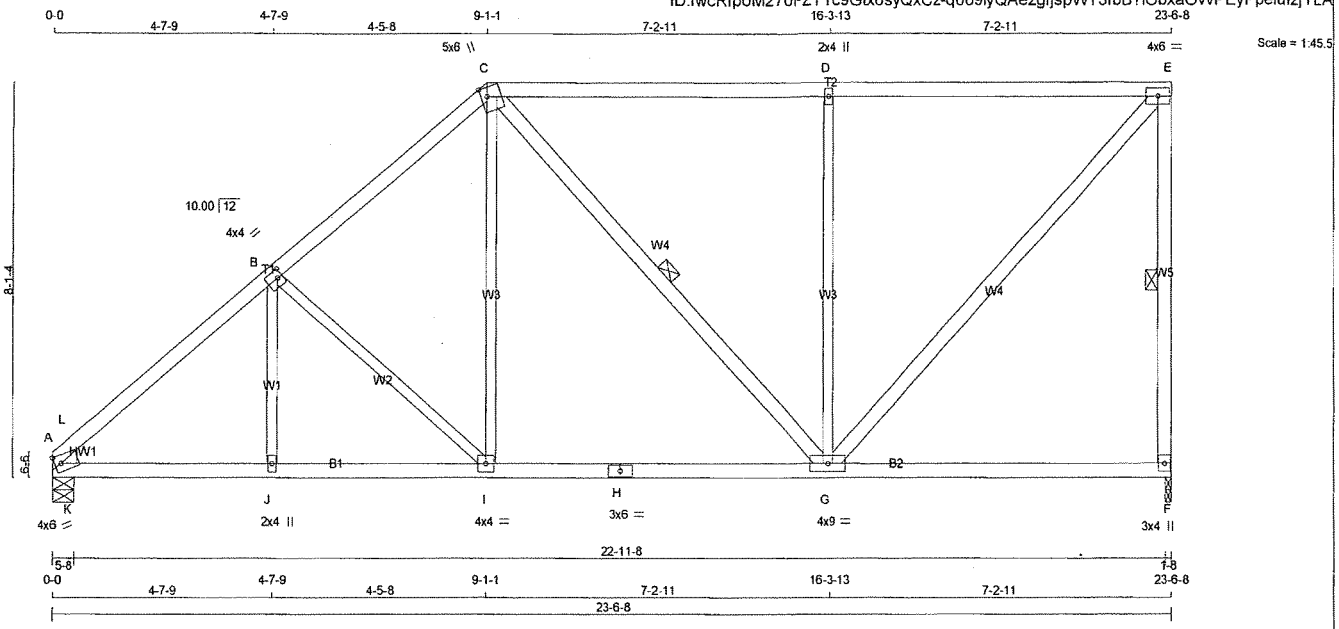

TOTAL WEIGHT = 109 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
F	1380	0	1380	0	0	1-8	1-8	
A	1380	0	1380	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0
A	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1994 / 0	-77.7	-77.7	0.16 (1)	4.63
L- B	-1698 / 0	-77.7	-77.7	0.26 (1)	4.85
B- C	-1323 / 0	-77.7	-77.7	0.25 (1)	5.36
C- D	-979 / 0	-77.7	-77.7	0.77 (1)	4.93
D- E	-979 / 0	-77.7	-77.7	0.77 (1)	4.92
F- E	-1266 / 0	0.0	0.0	0.38 (1)	5.73
A- K	0 / 1311	-39.5	-39.5	0.29 (1)	10.00
K- J	0 / 1311	-39.5	-39.5	0.29 (1)	10.00
J- I	0 / 1311	-39.5	-39.5	0.38 (2)	10.00
I- H	0 / 1004	-39.5	-39.5	0.53 (2)	10.00
H- G	0 / 1004	-39.5	-39.5	0.53 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.41 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 200	0.05 (3)
B- I	-419 / 0	0.27 (1)
I- C	0 / 539	0.12 (2)
C- G	-44 / 0	0.03 (2)
G- D	-696 / 0	0.88 (1)
G- E	0 / 1437	0.23 (1)
K- L	0 / 302	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.77/1.00 (D-E:1) , BC=0.53/1.00 (G-I:2) , WB=0.88/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)

DWG NO. TAM T20B685
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T91X	1	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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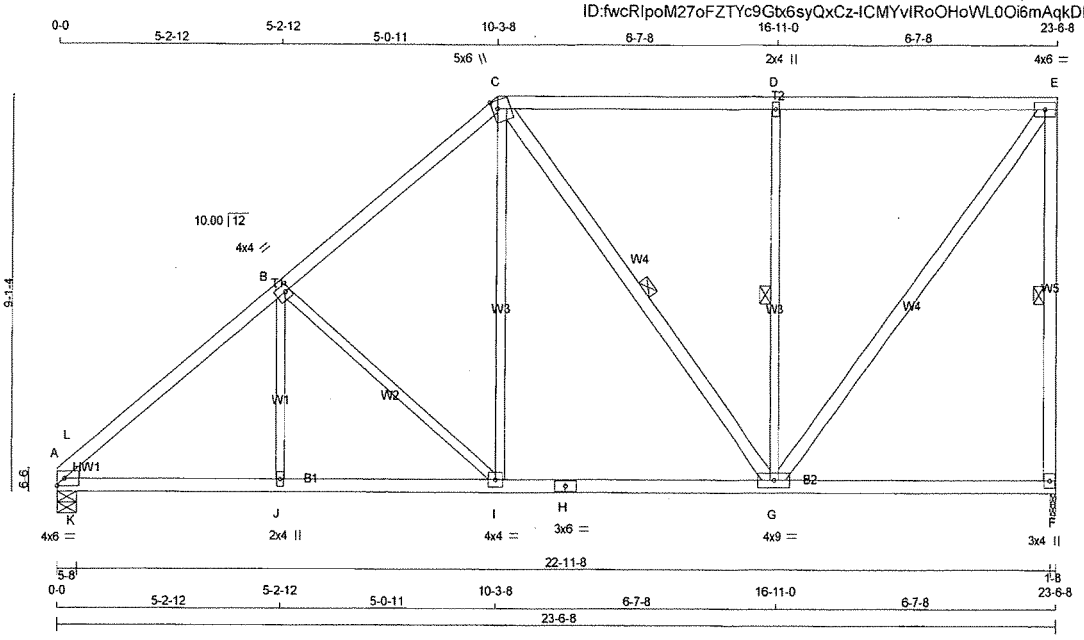


TOTAL WEIGHT = 114 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
F	1380	0	1380	0	0	1-8	1-8	
A	1380	0	1380	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0
A	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

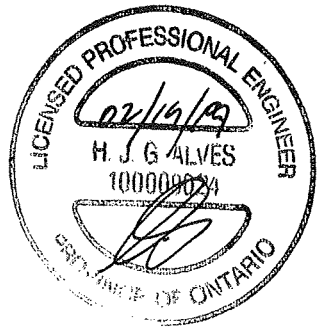
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1953 / 0	-77.7	-77.7	0.17 (1)	4.66	
L- B	-1669 / 0	-77.7	-77.7	0.36 (1)	4.77	
B- C	-1220 / 0	-77.7	-77.7	0.33 (1)	5.41	
C- D	-816 / 0	-77.7	-77.7	0.62 (1)	5.66	
D- E	-816 / 0	-77.7	-77.7	0.62 (1)	5.66	
F- E	-1275 / 0	0.0	0.0	0.49 (1)	5.71	
A- K	0 / 1293	-39.5	-39.5	0.27 (2)	10.00	
K- J	0 / 1293	-39.5	-39.5	0.32 (2)	10.00	
J- I	0 / 1293	-39.5	-39.5	0.36 (2)	10.00	
I- H	0 / 922	-39.5	-39.5	0.45 (2)	10.00	
H- G	0 / 922	-39.5	-39.5	0.45 (2)	10.00	
G- F	0 / 0	-39.5	-39.5	0.34 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 250	0.06 (3)
B- I	-503 / 0	0.45 (1)
I- C	0 / 580	0.13 (2)
C- G	-177 / 0	0.11 (1)
G- D	-638 / 0	0.34 (1)
G- E	0 / 1352	0.22 (1)
K- L	0 / 298	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.45/1.00 (G-I:2) , WB=0.45/1.00 (B-I:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.35 (A) (INPUT = 1.00)

DWG NO. TAM 119036B6
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T92X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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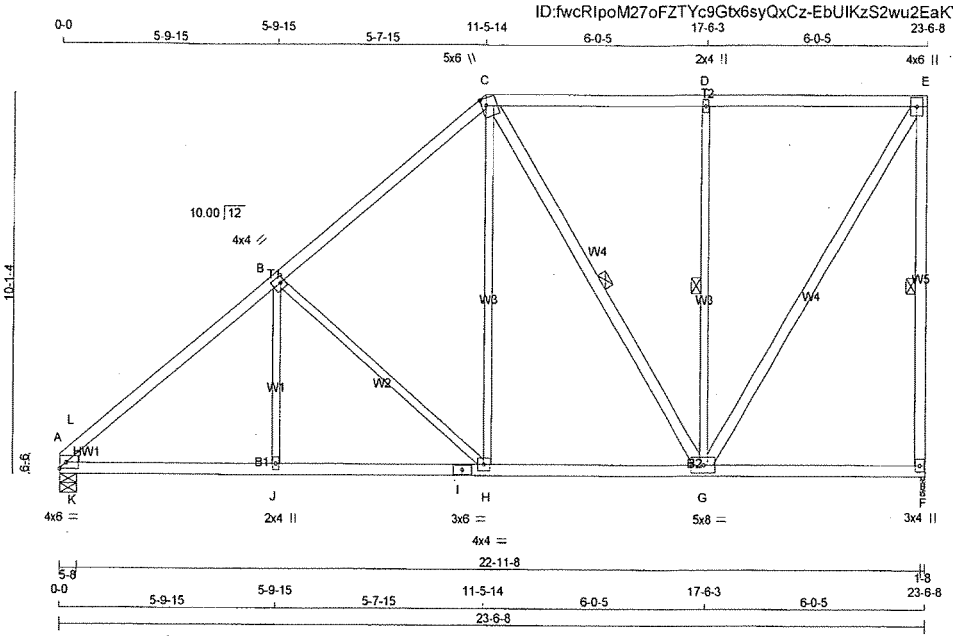


TOTAL WEIGHT = 120 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
F	1380	0	1380	0	0	1-8	1-8	
A	1380	0	1380	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0
A	1055	492 / 0	247 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

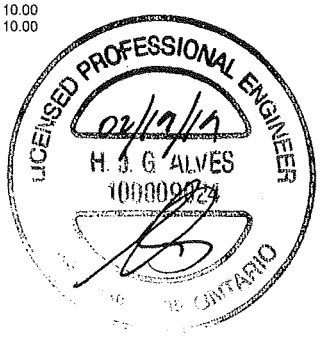
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1906 / 0	-77.7	-77.7	0.18 (1)	4.70	
L- B	-1636 / 0	-77.7	-77.7	0.49 (1)	4.67	
B- C	-1117 / 0	-77.7	-77.7	0.44 (1)	5.46	
C- D	-683 / 0	-77.7	-77.7	0.50 (1)	6.25	
D- E	-683 / 0	-77.7	-77.7	0.50 (1)	6.25	
F- E	-1284 / 0	0.0	0.0	0.64 (1)	5.69	
A- K	0 / 1272	-39.5	-39.5	0.26 (2)	10.00	
K- J	0 / 1272	-39.5	-39.5	0.38 (2)	10.00	
J- I	0 / 1272	-39.5	-39.5	0.38 (2)	10.00	
I- H	0 / 1272	-39.5	-39.5	0.38 (2)	10.00	
H- G	0 / 840	-39.5	-39.5	0.38 (2)	10.00	
G- F	0 / 0	-39.5	-39.5	0.27 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 299	0.07 (3)
B- H	-583 / 0	0.69 (1)
H- C	0 / 624	0.14 (2)
C- G	-301 / 0	0.22 (1)
G- D	-579 / 0	0.40 (1)
G- E	0 / 1299	0.21 (1)
K- L	0 / 311	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.64/1.00 (E-F:1) , BC=0.38/1.00 (G-H:2) , WB=0.69/1.00 (B-H:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

DWG NO. TAM T1903687
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T93	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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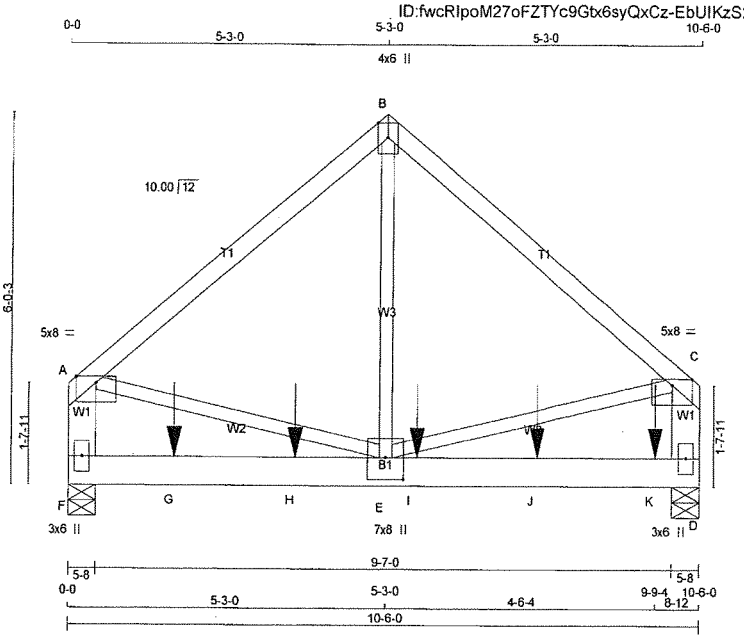


TOTAL WEIGHT = 2 X 51 = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
F- A	2	12	TOP
D- C	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TTW+p	MT20	4.0	6.0	Edge	
C	TMVW-p	MT20	5.0	8.0	Edge	
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW+t	MT20	7.0	8.0	4.25	3.50
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3413	0	3413	0	0	5-8	5-8
D	4034	0	4034	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2597	1249 / 0	580 / 0	0 / 0	0 / 0	768 / 0	0 / 0
D	3070	1476 / 0	686 / 0	0 / 0	0 / 0	907 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

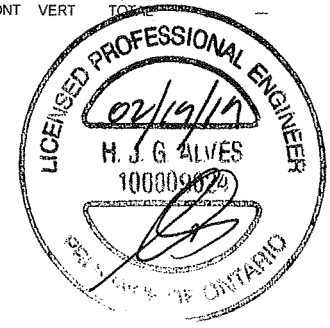
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-2854 / 0	-77.7	-77.7	0.28 (1)	5.17	E- B		0 / 3246	0.40 (1)
B- C	-2854 / 0	-77.7	-77.7	0.28 (1)	5.17	A- E		0 / 2251	0.28 (1)
F- A	-2541 / 0	0.0	0.0	0.09 (1)	7.81	E- C		0 / 2251	0.28 (1)
D- C	-2541 / 0	0.0	0.0	0.09 (1)	7.81				
F- G	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
G- H	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
H- E	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
E- I	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
I- J	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
J- K	0 / 0	-39.5	-39.5	0.78 (1)	10.00				
K- D	0 / 0	-39.5	-39.5	0.78 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-4	-1242	-1242	---	FRONT	VERT	TOTAL	---	---
H	3-9-4	-1242	-1242	---	FRONT	VERT	TOTAL	---	---
I	5-9-4	-1242	-1242	---	FRONT	VERT	TOTAL	---	---
J	7-9-4	-1242	-1242	---	FRONT	VERT	TOTAL	---	---
K	9-9-4	-1248	-1248	---	FRONT	VERT	TOTAL	---	---



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.78/1.00 (E-F:1) , WB=0.40/1.00 (B-E:1) , SSI=0.81/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

DWG NO. TAM T1703688
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	T94	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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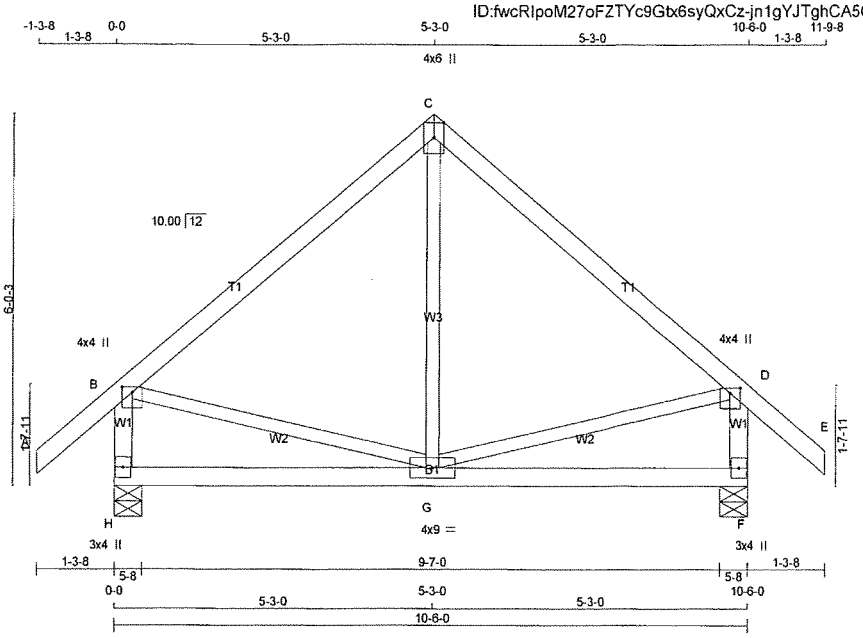

TOTAL WEIGHT = 47 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	723	0	723	0	0	5-8	5-8
F	723	0	723	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	545	277 / 0	110 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	545	277 / 0	110 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 233	0.06 (3)
B- C	-400 / 0	-77.7	-77.7	0.28 (1)	6.25	B- G	0 / 317	0.07 (1)
C- D	-400 / 0	-77.7	-77.7	0.28 (1)	6.25	G- D	0 / 317	0.07 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-644 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-644 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.25/1.00 (F-G:3) , WB=0.07/1.00 (D-G:1) , SSI=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

DWG NO. TAM T1903689
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 21 - 23	DRWG NO.
401384	G94	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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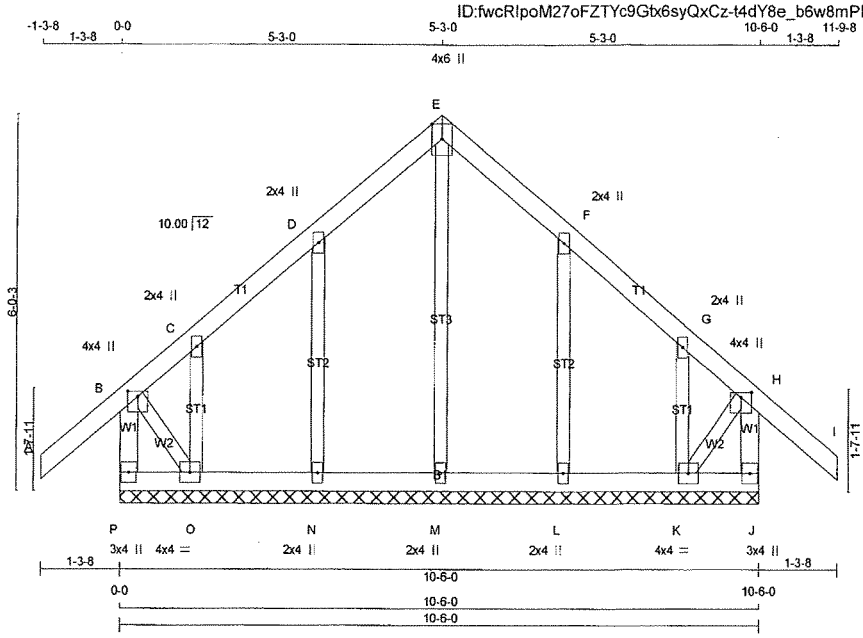


TOTAL WEIGHT = 51 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

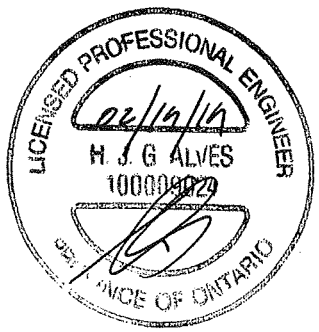
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
P- B	-230 / 0	0.0	0.0	0.02 (1)	7.81	M- E	-121 / 0	0.07 (1)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- D	-188 / 0	0.05 (1)
B- C	-51 / 0	-77.7	-77.7	0.10 (1)	6.25	O- C	-48 / 0	0.01 (1)
C- D	-1 / 0	-77.7	-77.7	0.05 (1)	10.00	L- F	-188 / 0	0.05 (1)
D- E	-16 / 0	-77.7	-77.7	0.05 (1)	6.25	K- G	-48 / 0	0.01 (1)
E- F	-16 / 0	-77.7	-77.7	0.05 (1)	6.25	B- O	0 / 15	0.00 (1)
F- G	-1 / 0	-77.7	-77.7	0.05 (1)	10.00	K- H	0 / 15	0.00 (1)
G- H	-51 / 0	-77.7	-77.7	0.10 (1)	6.25			
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
J- H	-230 / 0	0.0	0.0	0.02 (1)	7.81			
P- O	0 / 0	-39.5	-39.5	0.02 (3)	10.00			
O- N	0 / 8	-39.5	-39.5	0.03 (3)	10.00			
N- M	0 / 4	-39.5	-39.5	0.03 (3)	10.00			
M- L	0 / 4	-39.5	-39.5	0.03 (3)	10.00			
L- K	0 / 8	-39.5	-39.5	0.03 (3)	10.00			
K- J	0 / 0	-39.5	-39.5	0.02 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (K-L:3) , WB=0.07/1.00 (E-M:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (H) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM T1903690
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T95	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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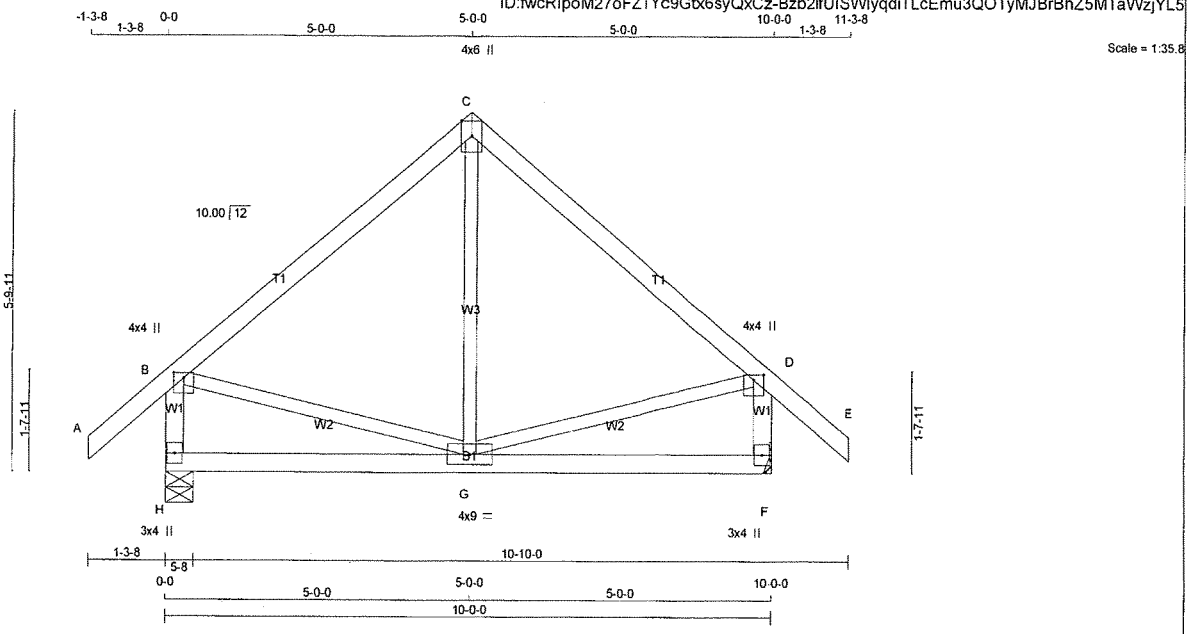

TOTAL WEIGHT = 2 X 45 = 91 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	694	0	694	0	0	5-8	5-8
F	694	0	694	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	523	267 / 0	105 / 0	0 / 0	0 / 0	151 / 0	0 / 0
F	523	267 / 0	105 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 219	0.05 (3)
B- C	-377 / 0	-77.7	-77.7	0.25 (1)	6.25	B- G	0 / 299	0.07 (1)
C- D	-377 / 0	-77.7	-77.7	0.25 (1)	6.25	G- D	0 / 299	0.07 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-618 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-618 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.23 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.23/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.13/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

DWG NO. TAM T1703691
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T96	2	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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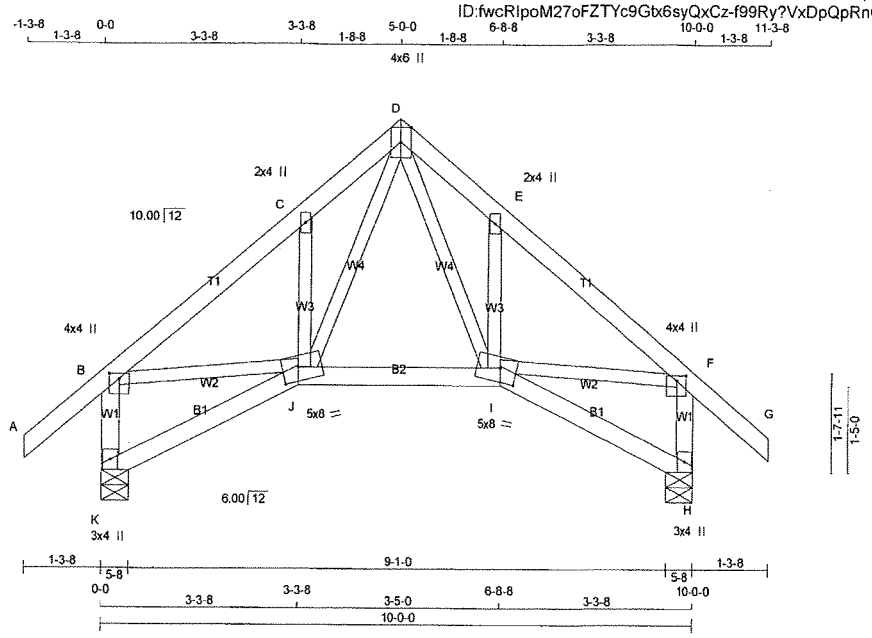


TOTAL WEIGHT = 2 X 50 = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	694	0	694	0	0	5-8	5-8
H	694	0	694	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	523	267 / 0	105 / 0	0 / 0	0 / 0	151 / 0	0 / 0
H	523	267 / 0	105 / 0	0 / 0	0 / 0	151 / 0	0 / 0

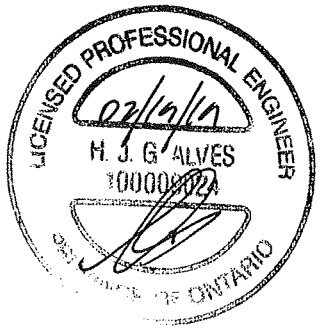
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	D- I	0 / 432	0.10 (1)
B- C	-659 / 0	-77.7	-77.7	0.11 (1)	6.25	I- E	-247 / 0	0.04 (1)
C- D	-694 / 0	-77.7	-77.7	0.07 (1)	6.25	J- D	0 / 432	0.10 (1)
D- E	-694 / 0	-77.7	-77.7	0.07 (1)	6.25	J- C	-247 / 0	0.04 (1)
E- F	-659 / 0	-77.7	-77.7	0.11 (1)	6.25	B- J	0 / 515	0.12 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- F	0 / 515	0.12 (1)
K- B	-629 / 0	0.0	0.0	0.07 (1)	7.81			
H- F	-629 / 0	0.0	0.0	0.07 (1)	7.81			
K- J	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
J- I	0 / 348	-39.5	-39.5	0.15 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.10 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.15/1.00 (I-J:2) , WB=0.12/1.00 (F-I:1) , SSI=0.09/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.21 (F) (INPUT = 1.00)

DWG NO. TAM T190 3692
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401384	T97TCX	3	1	UNITS 21 - 23 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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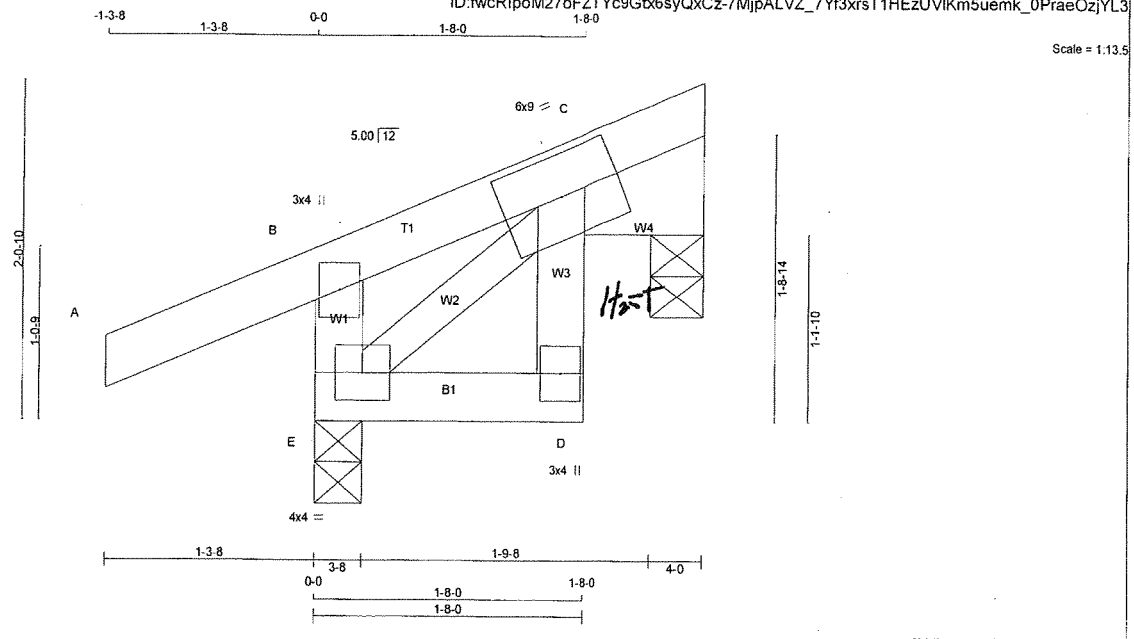


TOTAL WEIGHT = 3 X 12 = 35 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVWW1-t	MT20	6.0	9.0	
D	BMV+p	MT20	3.0	4.0	
E	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
C	55	0	58	0	-22	4-0	4-0
E	245	0	245	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT H25T

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	45	12 / -23	17 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	176	114 / 0	17 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 20	-77.7	-77.7	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	-16 / 0	-77.7	-77.7	0.10 (1)	6.25			
D- C	0 / 42	0.0	0.0	0.01 (3)	10.00			
E- B	-212 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-39.5	-39.5	0.03 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM T1903693
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

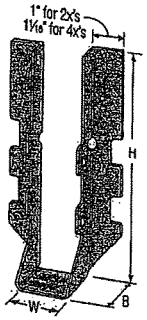
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

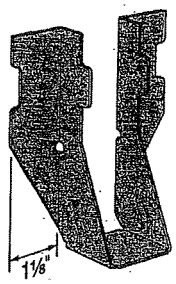
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

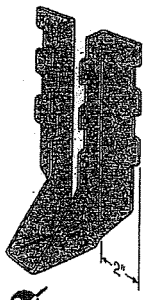
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



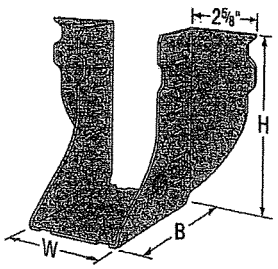
LUS28



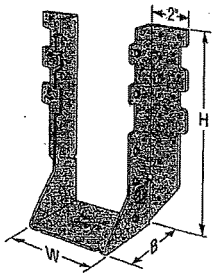
LU26L



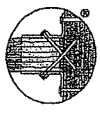
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

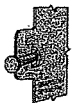


HHUS210-2



Double-Shear Nailing Top View

Double-Shear Nailing Side View; Do not bend tab

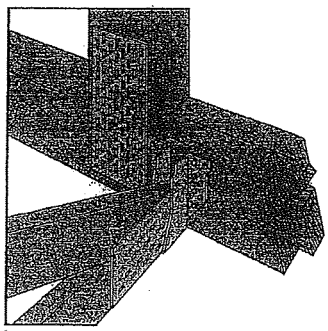


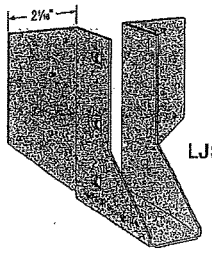
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

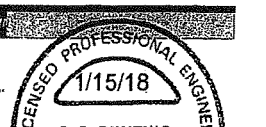
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

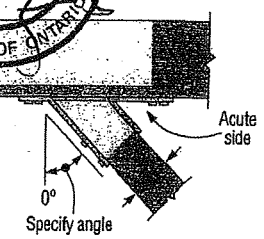
HHUS – Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS – Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅜	5⅞	3	3⅛	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.5	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.15	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7¹⁵⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

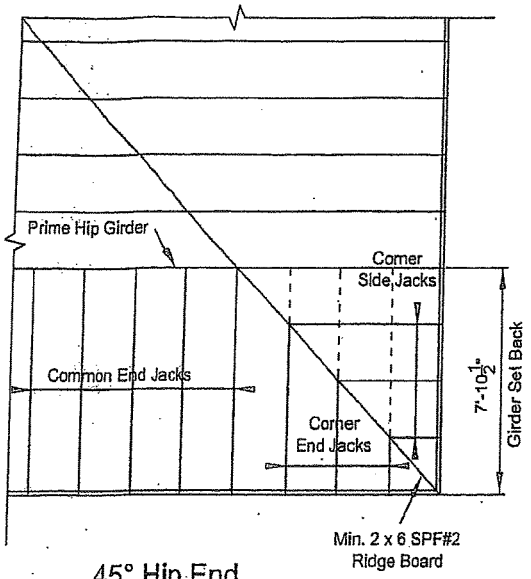
SIMPSON Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_B[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	9⅛	2	1½	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3⅝	5⅜	3	3⅛	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3⅝	9⅛	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4⅛	5½	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4⅛	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4⅛	9	3	7⅛	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4⅛	9¼	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅛	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6⅛	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7⅛	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6⅛	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6⅛	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6⅛	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3⅜	4¼	2	3⅛	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3⅝	5¼	3	3⅛	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3⅜	6¾	2	3⅛	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3⅜	8¾	2	5⅛	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80

See footnotes on p. 258.



45° Hip End

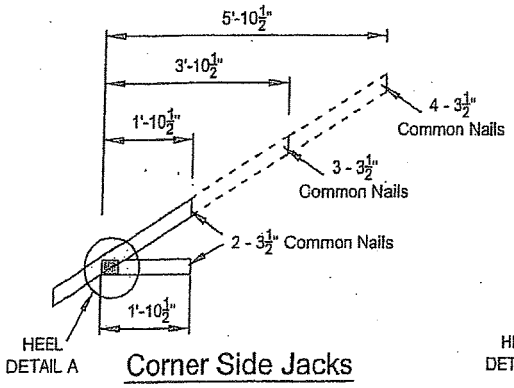
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

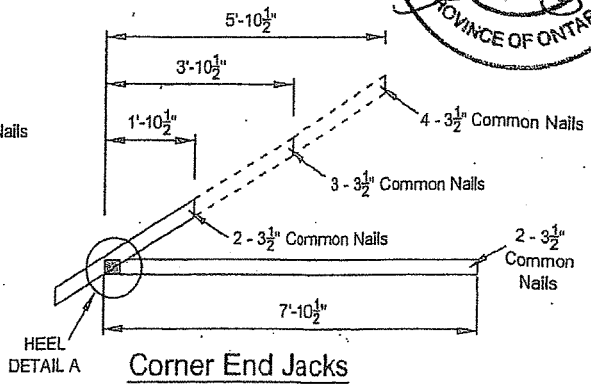
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

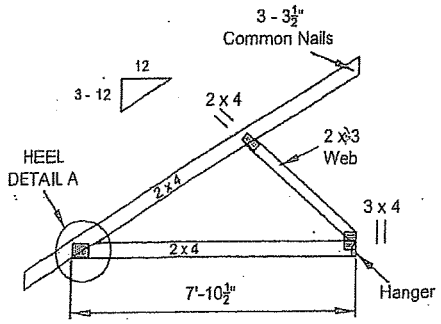
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



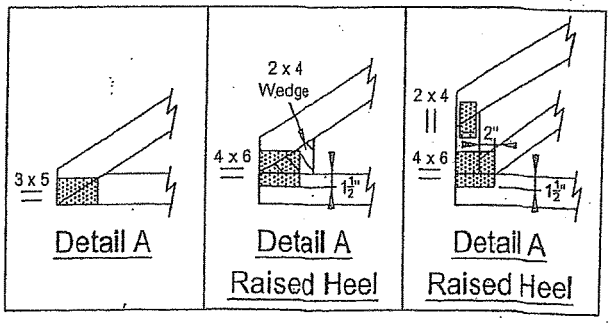
Corner Side Jacks



Corner End Jacks

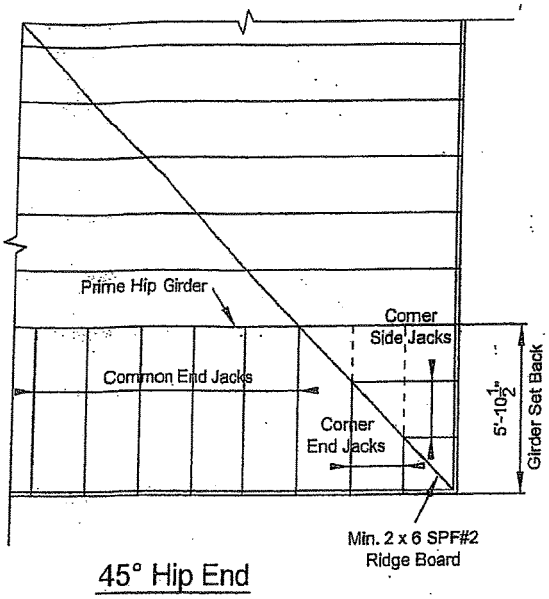


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

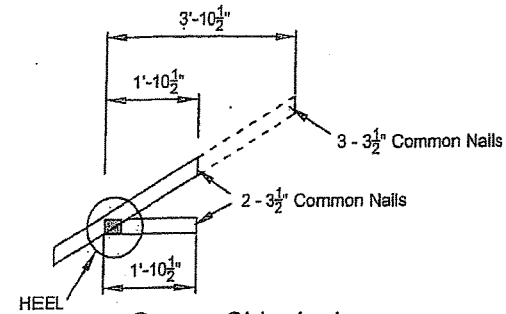
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

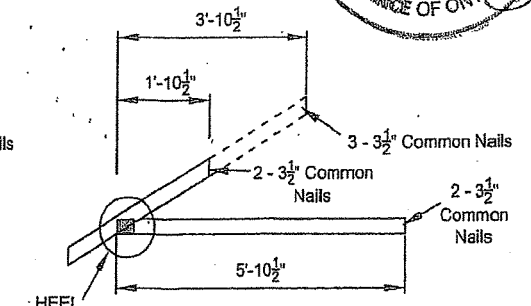
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

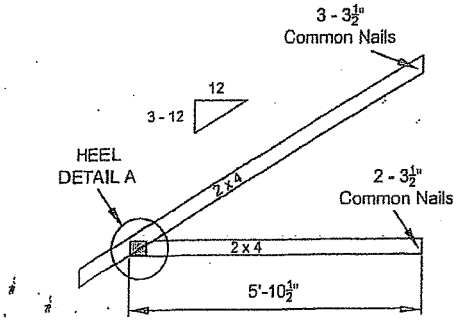
LICENSED PROFESSIONAL ENGINEER
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO



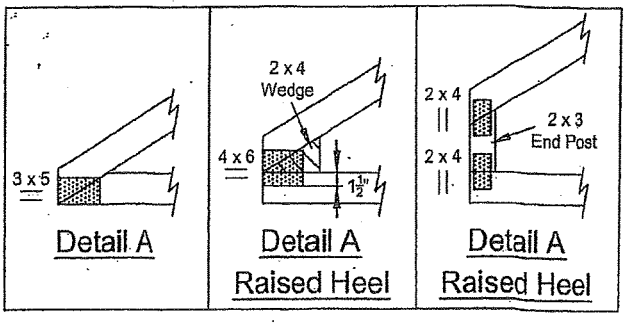
Corner Side Jacks



Corner End Jacks



Common End Jacks

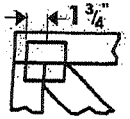


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

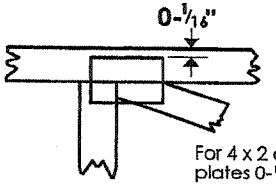
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

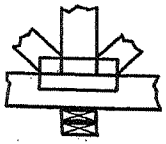
PLATE SIZE

4 x 4 — The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

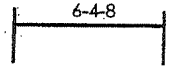
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

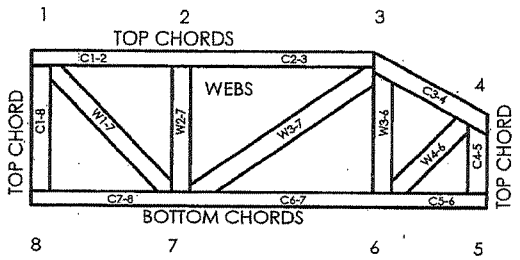
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

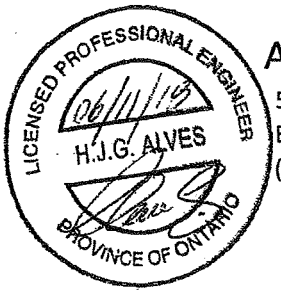
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
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RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218 Feb 09, 2018