ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

TH-9C
EL:B UNIT1
401346

TH-4
EL:B UNIT2
401347

TH-4
EL:B(REV) UNIT3
401348

TH-6E
EL:B(REV) UNIT4
401349

10/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

ALL B -2-2X10 FLUSH

* - ALL FIRST FLOOR PLATE HEIGHT 7'-10"-0 U/S @ PLATE LEVEL

C - 5/12 CATH. CEILING

H2 -12" RAISED CEILING

H -12" RAISED CEILING + 4" RAISED HEEL

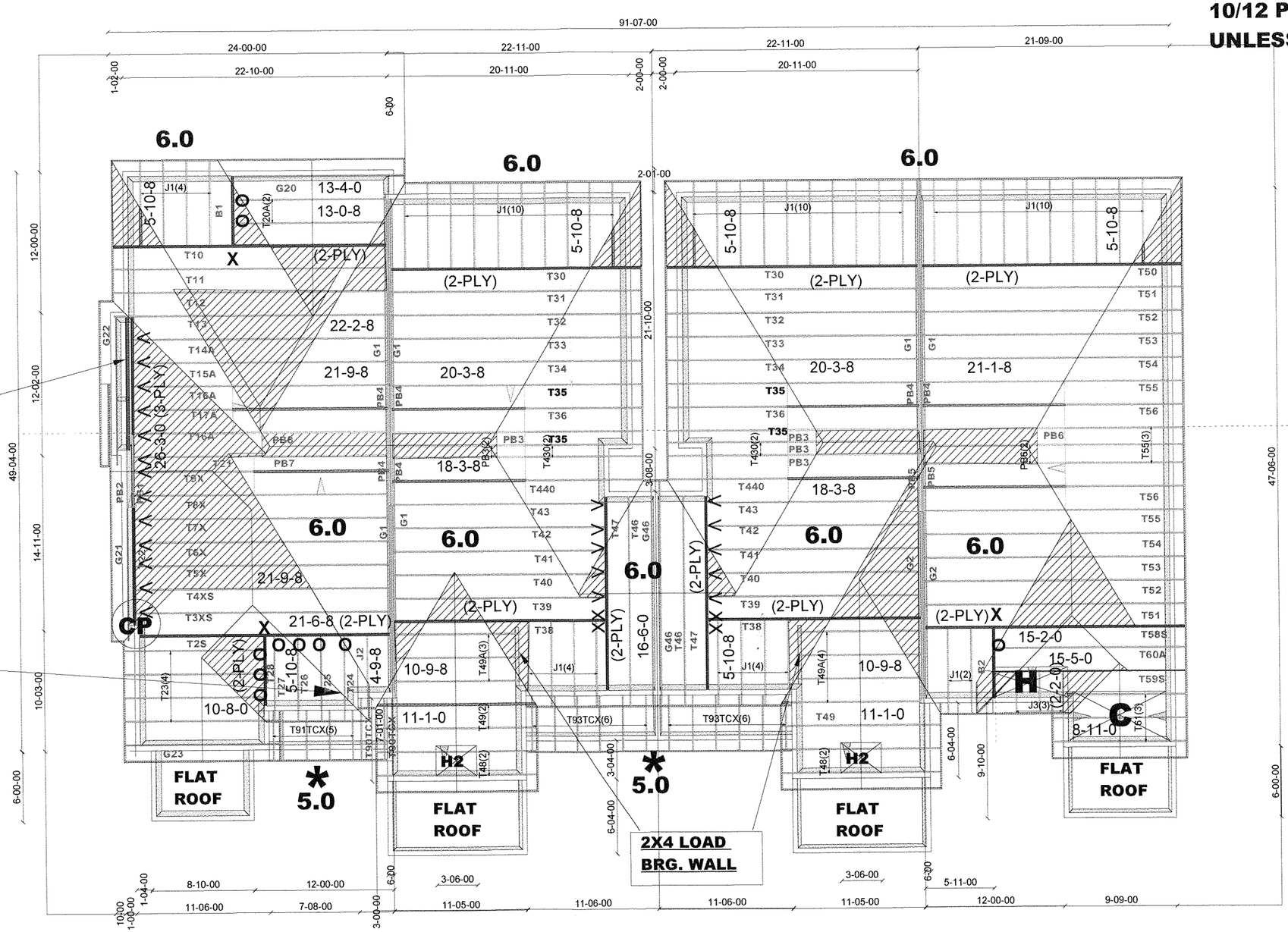
HARDWARE:

LUS26-2 -(X)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)
CP2-4 -(CP)

STEEL BEAM A.P.P

2X6 LOAD BRG. WALL

12" HIGHER FASCIA ONLY



T-180690

DENOTES:
CONVENTIONAL FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

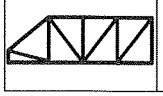
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 50264	Builder / Location: BAYVIEW WELLINGTON / ST CATHERINES	Model / Elevation: FREEHOLD BLOCK 1 / UNITS 1 - 4	Mitek ver 8.2.3.229
Plan Log: 200400	Project: PASSAGE ON THE CANAL		
Layout ID: 400998	Date: 2/11/2019 Sales: William Garcia Designer:		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401346
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	1 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 1	Designer:	Andrew Conway
		Sales Rep:	William Garcia

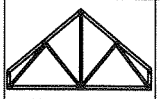
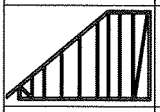
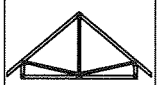
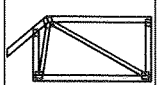
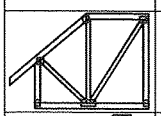
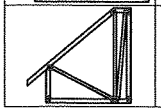
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	6 /12	21-00-00	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	240.42 158.67		
	1 2-ply	T2S Roof Special Girder	10 /12	21-06-08	7-06-07	2 x 4 2 x 6		1-07-11 3-06-12	223.46 140.33		
	1	T3XS Roof Special	10 /12	21-09-08	9-02-07	2 x 4		1-05-03 4-06-12	109.07 69.00		
	1	T4XS Roof Special	10 /12	21-09-08	9-09-06	2 x 4		1-05-03 5-06-12	99.27 61.50		
	1	T5X Half Hip	10 /12	21-09-08	6-06-12	2 x 4		1-05-03 6-06-12	98.41 60.67		
	1	T6X Half Hip	10 /12	21-09-08	7-06-12	2 x 4		1-05-03 7-06-12	107.47 68.00		
	1	T7X Half Hip	10 /12	21-09-08	8-06-12	2 x 4		1-05-03 8-06-12	111.25 70.50		
	1	T8X Half Hip	10 /12	21-09-08	9-06-12	2 x 4		1-05-03 9-06-12	117.02 71.50		
	1	T9X Half Hip	10 /12	21-09-08	10-06-12	2 x 4		1-05-03 10-06-12	123.03 78.00		
	1 2-ply	T10 Hip Girder	10 /12	22-02-08	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-10-10	221.8 142.67		
	1	T11 Hip	10 /12	22-02-08	5-01-04	2 x 4	1-03-08	1-07-11 1-10-10	89.69 59.00		
	1	T12 Hip	10 /12	22-02-08	6-01-04	2 x 4	1-03-08	1-07-11 1-10-10	94.33 59.83		
	1	T13 Half Hip	10 /12	22-02-08	6-00-12	2 x 4	1-03-08	1-07-11 6-00-12	98.56 62.00		
	1	T14A Half Hip	10 /12	21-09-08	7-00-12	2 x 4		1-11-14 7-00-12	101.85 64.83		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401346
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	2 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 1	Designer:	Andrew Conway
		Sales Rep:	William Garcia

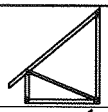
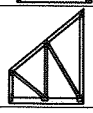
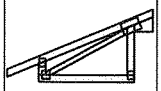
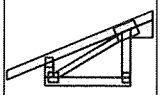
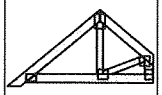
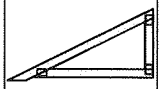
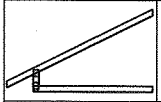
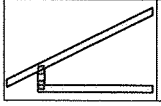
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T15A Half Hip	10 /12	21-09-08	8-00-12	2 x 4		1-11-14 8-00-12	107.3 68.00		
	2	T16A Half Hip	10 /12	21-09-08	9-00-12	2 x 4		1-11-14 9-00-12	230.1 145.33		
	1	T17A Half Hip	10 /12	21-09-08	10-00-12	2 x 4		1-11-14 10-00-12	120.86 76.83		
	2	T20A Common	10 /12	13-00-08	7-02-06	2 x 4		1-07-11 1-10-10	117.93 75.00		
	1	G20 GABLE	10 /12	13-04-00	7-02-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	67.11 44.17		
	1	T21 Piggyback Base	10 /12	21-09-08	9-06-12	2 x 4		1-11-14 9-06-12	117.92 72.00		
	1	G21 GABLE	10 /12	14-11-00	10-00-00	2 x 4	1-03-08	1-07-11 10-00-00	105.36 68.17		
	1 2-ply	T22 Piggyback Base Girder	10 /12	26-03-00	10-00-00	2 x 6		1-07-11 1-07-11	339.95 207.33		
	1	G22 GABLE	10 /12	11-04-00	6-04-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.08 37.67		
	4	T23 Common	10 /12	10-08-00	6-01-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	191.36 124.00		
	1	G23 GABLE	10 /12	10-08-00	6-01-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	51.84 34.00		
	1	T24 Half Hip	10 /12	5-10-08	3-02-06	2 x 4	1-03-08	2-07-11 3-02-06	27.88 18.50		
	1	T25 Half Hip	10 /12	5-10-08	4-10-06	2 x 4	1-03-08	2-07-11 4-10-06	34.17 23.50		
	1	T26 Half Hip	10 /12	5-10-08	6-06-06	2 x 4	1-03-08	2-07-11 6-06-06	40.21 25.17		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401346
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	3 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 1	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T27 Monopitch	10 /12	5-10-08	7-06-07	2 x 4	1-03-08	2-07-11 7-06-07	32.64 20.33		
	1 2-ply	T28 Monopitch Girder	10 /12	5-10-08	7-06-07	2 x 4 2 x 6		2-07-11 7-06-07	79.59 51.00		
	2	T90TCX Monopitch	5 /12	3-11-00	2-11-14	2 x 4	1-03-08	1-00-09 2-08-02	39.76 26.67		
	5	T91TCX Monopitch	5 /12	2-11-00	2-06-14	2 x 4	1-03-08	1-00-09 2-03-02	80.8 54.17		
	1 2-ply	PB1 Piggyback	10 /12	6-02-04	2-06-15	2 x 4			29.6 20.00		
	1	PB2 Piggyback	10 /12	4-10-10	2-06-15	2 x 4		1-01-00	15.66 11.33		
	2	PB4 Piggyback	6 /12	5-01-08	2-06-12	2 x 4		2-06-12	26.75 17.33		
	1	PB7 Piggyback	10 /12	12-08-07	1-06-00	2 x 4		1-06-00	39.46 25.67		
	1	PB8 Piggyback	10 /12	13-03-10	2-00-00	2 x 4		2-00-00	42.58 28.00		
	4	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	1	J2 Jack-Open	6 /12	4-09-08	3-06-12	2 x 4	1-03-08	1-02-00 3-06-12	14.18 8.67		

TOTAL # TRUSS= **59** TOTAL BFT OF ALL TRUSSES= **2492.01** BFT. TOTAL WEIGHT OF ALL TRSSES **3910.89** LBS

HARDWARE

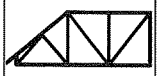
QTY	TYPE	MODEL	LENGTH
13	Hardware	LJS26DS	
9	Hardware	LUS24	
2	Hardware	LUS26-2	
1	Hardware	CP2-4	

TOTAL NUMBER OF ITEMS= **25**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401347
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	1 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 2	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

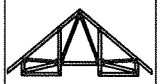
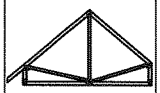
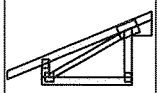
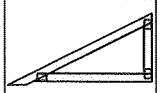
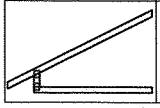
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	6 /12	21-00-00	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	240.42 158.67		
	1 2-ply	T30 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 111.00		
	1	T31 Half Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 55.83		
	1	T32 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.00		
	1	T33 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 58.67		
	1	T34 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	2	T35 Half Hip	10 /12	20-03-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	224.46 143.00		
	1	T36 Half Hip	10 /12	20-03-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	118.25 74.17		
	1 2-ply	T38 Flat Girder	0 /12	18-03-08	4-01-04	2 x 4 2 x 6		4-01-04 4-01-04	174.35 110.00		
	1	T39 Flat	0 /12	18-03-08	5-01-04	2 x 4		5-01-04 5-01-04	77.29 49.00		
	1	T40 Half Hip	10 /12	18-03-08	6-01-04	2 x 4		3-03-11 6-01-04	79.81 51.17		
	1	T41 Half Hip	10 /12	18-03-08	7-01-04	2 x 4		3-03-11 7-01-04	84.11 53.33		
	1	T42 Half Hip	10 /12	18-03-08	8-01-04	2 x 4		3-03-11 8-01-04	88.7 55.50		
	1	T43 Half Hip	10 /12	18-03-08	9-01-04	2 x 4		3-03-11 9-01-04	104.75 66.67		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401347
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	2 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 2	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T46 Common	6 /12	**16-06-00**	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	66.27 41.33		
	1	G46 GABLE	6 /12	**16-06-00**	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	62.19 38.83		
	1 2-ply	T47 Common Girder	6 /12	**16-06-00**	5-03-08	2 x 4 2 x 6		1-02-00 1-02-00	159.81 102.67		
	2	T48 Roof Special	10 /12	**11-01-00**	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.07 90.00		
	2	T49 Common	10 /12	**11-01-00**	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	98.81 67.33		
	3	T49A Common	10 /12	**10-09-08**	6-03-02	2 x 4	1-03-08	1-07-11 1-10-10	140.46 93.00		
	6	T93TCX Monopitch	5 /12	**3-05-00**	2-09-06	2 x 4	1-03-08	1-00-09 2-05-10	108.12 76.00		
	2	T430 Piggyback Base	10 /12	**18-03-08**	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	249.56 152.00		
	1	T440 Half Hip	10 /12	**18-03-08**	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	133.7 83.50		
	3	PB3 Piggyback	10 /12	**11-04-00**	2-00-00	2 x 4		2-00-00	109.68 73.00		
	2	PB4 Piggyback	6 /12	**5-01-08**	2-06-12	2 x 4		2-06-12	26.75 17.33		
	14	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		

TOTAL # TRUSS= **57** TOTAL BFT OF ALL TRUSSES= **2089** BFT. TOTAL WEIGHT OF ALL TRSSES **3252.65** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401347
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	3 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 2	Designer:	Andrew Conway
		Sales Rep:	William Garcia

HARDWARE

QTY	TYPE	MODEL	LENGTH

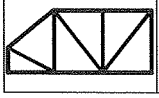
TOTAL NUMBER OF ITEMS= **6**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401348
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	1 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 3	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

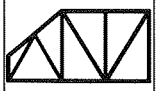
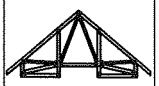
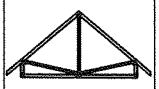
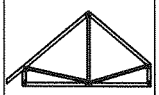
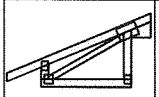
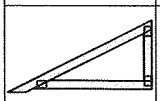
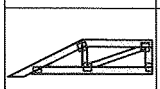
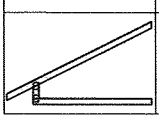
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G1 GABLE	6 /12	**21-00-00**	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	120.21 79.33		
	1	G2 GABLE	6 /12	**21-10-00**	9-01-04	2 x 4		1-02-00 9-01-04	116.24 73.50		
	1 2-ply	T30 Half Hip Girder	10 /12	**20-03-08**	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 111.00		
	1	T31 Half Hip	10 /12	**20-03-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 55.83		
	1	T32 Half Hip	10 /12	**20-03-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.00		
	1	T33 Half Hip	10 /12	**20-03-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 58.67		
	1	T34 Half Hip	10 /12	**20-03-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	2	T35 Half Hip	10 /12	**20-03-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	224.46 143.00		
	1	T36 Half Hip	10 /12	**20-03-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	118.25 74.17		
	1 2-ply	T38 Flat Girder	0 /12	**18-03-08**	4-01-04	2 x 4 2 x 6		4-01-04 4-01-04	174.35 110.00		
	1	T39 Flat	0 /12	**18-03-08**	5-01-04	2 x 4		5-01-04 5-01-04	77.29 49.00		
	1	T40 Half Hip	10 /12	**18-03-08**	6-01-04	2 x 4		3-03-11 6-01-04	79.81 51.17		
	1	T41 Half Hip	10 /12	**18-03-08**	7-01-04	2 x 4		3-03-11 7-01-04	84.11 53.33		
	1	T42 Half Hip	10 /12	**18-03-08**	8-01-04	2 x 4		3-03-11 8-01-04	88.7 55.50		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401348
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	2 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 3	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T43 Half Hip	10 /12	18-03-08	9-01-04	2 x 4		3-03-11 9-01-04	104.75 66.67		
	1	T46 Common	6 /12	16-06-00	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	66.27 41.33		
	1	G46 GABLE	6 /12	16-06-00	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	62.19 38.83		
	1 2-ply	T47 Common Girder	6 /12	16-06-00	5-03-08	2 x 4 2 x 6		1-02-00 1-02-00	159.81 102.67		
	2	T48 Roof Special	10 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.07 90.00		
	1	T49 Common	10 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.4 33.67		
	4	T49A Common	10 /12	10-09-08	6-03-02	2 x 4	1-03-08	1-07-11 1-10-10	187.29 124.00		
	6	T93TCX Monopitch	5 /12	3-05-00	2-09-06	2 x 4	1-03-08	1-00-09 2-05-10	108.12 76.00		
	2	T430 Piggyback Base	10 /12	18-03-08	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	249.56 152.00		
	1	T440 Half Hip	10 /12	18-03-08	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	133.7 83.50		
	3	PB3 Piggyback	10 /12	11-04-00	2-00-00	2 x 4		2-00-00	109.68 73.00		
	1	PB4 Piggyback	6 /12	5-01-08	2-06-12	2 x 4		2-06-12	13.38 8.67		
	1	PB5 Piggyback	6 /12	5-11-08	1-06-06	2 x 4		1-06-06	16.6 11.33		
	14	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401348
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	3 of 3
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 3	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **57** TOTAL BFT OF ALL TRUSSES= **2083.17** BFT. TOTAL WEIGHT OF ALL TRSSES **3249.32** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

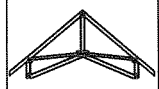
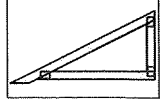
TOTAL NUMBER OF ITEMS= **6**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401349
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	1 of 2
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 4	Designer:	Andrew Conway
		Sales Rep:	William Garcia

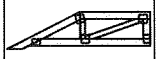
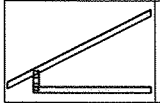
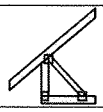
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G1 GABLE	6 /12	**21-00-00**	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	120.21 79.33		
	1	G2 GABLE	6 /12	**21-10-00**	9-01-04	2 x 4		1-02-00 9-01-04	116.24 73.50		
	1 2-ply	T50 Half Hip Girder	10 /12	**21-01-08**	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	177.3 116.33		
	2	T51 Half Hip	10 /12	**21-01-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	179.6 117.33		
	2	T52 Half Hip	10 /12	**21-01-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	191.49 122.67		
	2	T53 Half Hip	10 /12	**21-01-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	2	T54 Half Hip	10 /12	**21-01-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	5	T55 Half Hip	10 /12	**21-01-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	573.25 360.83		
	2	T56 Half Hip	10 /12	**21-01-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	241.17 151.00		
	1 2-ply	T58S Roof Special Girder	10 /12	**21-01-08**	7-10-07	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	250.2 165.67		
	1	T59S Hip Girder	10 /12	**15-05-00**	4-09-06	2 x 4	1-03-08	1-07-11 1-11-11	72.86 48.00		
	1	T60A Hip	10 /12	**15-02-00**	6-05-06	2 x 4	1-03-08	1-07-11 3-02-03	69.05 44.17		
	3	T61 Scissor	10 /12 5 /12	**8-11-00**	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	125.83 83.00		
	1	PB4 Piggyback	6 /12	**5-01-08**	2-06-12	2 x 4		2-06-12	13.38 8.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON	PlanLog:	200400
Project:	PASSAGE ON THE CANAL	Layout ID:	401349
Location:	ST CATHERINES	Ref #	
Model:	BLOCK 1 FREEHOLD	Page:	2 of 2
Lot #:		Date:	02/11/2019
Elevation:	B UNIT 4	Designer:	Andrew Conway
		Sales Rep:	William Garcia

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	PB5 Piggyback	6 /12	**5-11-08**	1-06-06	2 x 4		1-06-06	16.6 11.33		
	3	PB6 Piggyback	10 /12	**12-02-00**	2-00-00	2 x 4		2-00-00	117.31 79.00		
	12	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	3	J3 Jack-Open	10 /12	**2-02-00**	3-09-06	2 x 4	1-03-08	1-11-11 3-09-06	34.05 24.50		

TOTAL # TRUSS= **46** TOTAL BFT OF ALL TRUSSES= **1864** BFT. TOTAL WEIGHT OF ALL TRSSES **2891.94** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **2**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G1	6	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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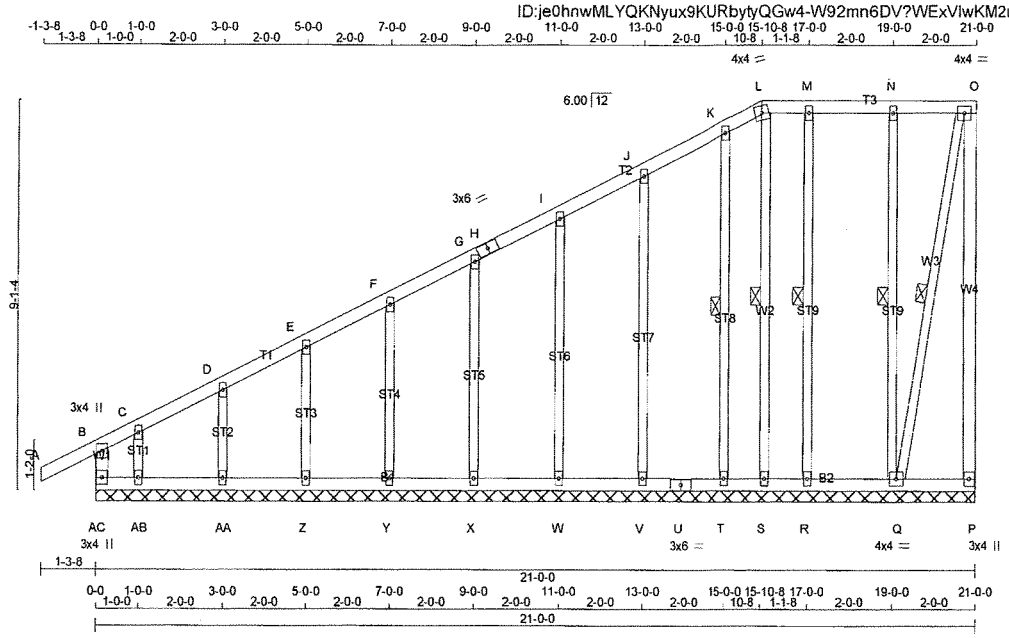


TOTAL WEIGHT = 6 X 120 = 721 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AC- B	2x4	DRY	No.2	SPF
AC- U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, M, N						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
L	TTW-m	MT20	4.0	4.0		
O	TMVW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q	BMWW1-t	MT20	4.0	4.0		
R, S, T, V, W, X, Y, Z, AA, AB						
R	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	6.0		
AC	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT P FOR 150 LBS FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF N-Q, M-R, K-T, L-S, O-Q.

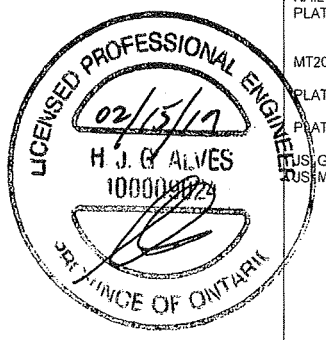
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-78.0	-78.0	0.10 (1)	10.00
B- C	-31 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	0 / 7	-78.0	-78.0	0.04 (1)	10.00
D- E	0 / 6	-78.0	-78.0	0.04 (1)	10.00
E- F	0 / 11	-78.0	-78.0	0.04 (1)	10.00
F- G	0 / 13	-78.0	-78.0	0.04 (1)	10.00
G- H	0 / 15	-78.0	-78.0	0.04 (1)	6.25
H- I	0 / 15	-78.0	-78.0	0.04 (1)	6.25
I- J	0 / 17	-78.0	-78.0	0.04 (1)	10.00
J- K	0 / 16	-78.0	-78.0	0.04 (1)	10.00
K- L	0 / 12	-78.0	-78.0	0.02 (1)	10.00
L- M	0 / 18	-78.0	-78.0	0.03 (1)	10.00
M- N	0 / 18	-78.0	-78.0	0.04 (1)	10.00
N- O	0 / 22	-78.0	-78.0	0.04 (1)	10.00
P- O	0 / 41	0.0	0.0	0.01 (1)	10.00
AC- B	-213 / 0	0.0	0.0	0.02 (1)	7.81
AC-AB	0 / 0	-38.5	-38.5	0.02 (3)	10.00
AB-AA	-2 / 0	-38.5	-38.5	0.02 (3)	10.00
AA- Z	-6 / 0	-38.5	-38.5	0.02 (3)	10.00
Z- Y	-9 / 0	-38.5	-38.5	0.02 (3)	10.00
Y- X	-12 / 0	-38.5	-38.5	0.02 (3)	6.25
X- W	-13 / 0	-38.5	-38.5	0.02 (3)	6.25
W- V	-15 / 0	-38.5	-38.5	0.02 (3)	6.25
V- U	-17 / 0	-38.5	-38.5	0.02 (3)	6.25
U- T	-17 / 0	-38.5	-38.5	0.02 (3)	6.25
T- S	-17 / 0	-38.5	-38.5	0.02 (3)	6.25
S- R	-18 / 0	-38.5	-38.5	0.02 (3)	6.25
R- Q	-18 / 0	-38.5	-38.5	0.03 (3)	6.25
Q- P	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- N	-173 / 0	0.09 (1)
R- M	-135 / 0	0.07 (1)
T- K	-125 / 0	0.06 (1)
V- J	-162 / 0	0.18 (1)
W- I	-154 / 0	0.11 (1)
X- G	-155 / 0	0.08 (1)
Y- F	-156 / 0	0.05 (1)
Z- E	-151 / 0	0.03 (1)
AA- D	-169 / 0	0.03 (1)
AB- C	-31 / 0	0.00 (1)
S- L	-56 / 0	0.03 (1)
Q- O	-108 / 0	0.06 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (Q-R:3) , WB=0.18/1.00 (J-V:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUS GRIP= 0.60 (L) (INPUT = 0.90)
LUS METAL= 0.07 (D) (INPUT = 1.00)

BWB NO. TAM 1703432
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T2S	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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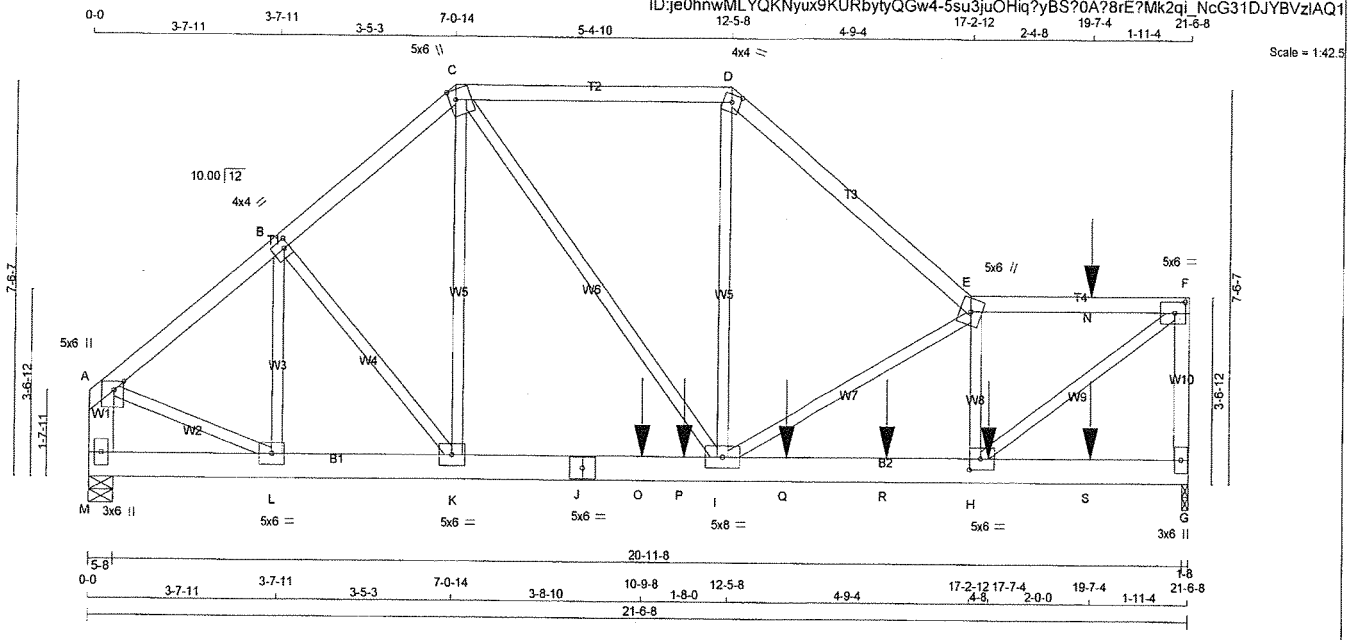

TOTAL WEIGHT = 2 X 112 = 223 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
A- C	1	12		TOP
C- D	1	12		TOP
D- E	1	12		TOP
E- F	1	12		SIDE(0.0)
F- G	1	12		TOP
M- A	2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
M- J	2	12		TOP
J- G	2	12		SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS				
2x3	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0	1.75	2.00
E	TTWW+m	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.50
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0	2.50	2.50
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2604	0	2804	0	0	1-8	1-8
M	2301	0	2301	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2116	1074 / 0	441 / 0	0 / 0	0 / 0	601 / 0	0 / 0
M	1739	877 / 0	366 / 0	0 / 0	0 / 0	495 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2309 / 0	-78.0	-78.0	0.10 (1)	5.80
B- C	-2544 / 0	-78.0	-78.0	0.11 (1)	5.58
C- D	-2543 / 0	-78.0	-78.0	0.26 (1)	5.40
D- E	-3278 / 0	-78.0	-78.0	0.24 (1)	4.93
E- N	-3298 / 0	-78.0	-78.0	0.24 (1)	4.89
N- F	-3298 / 0	-78.0	-78.0	0.24 (1)	4.89
G- F	-2682 / 0	0.0	0.0	0.26 (1)	6.96
M- A	-2203 / 0	0.0	0.0	0.08 (1)	7.81
M- L	0 / 0	-38.5	-38.5	0.05 (1)	10.00
L- K	0 / 1788	-38.5	-38.5	0.28 (1)	10.00
K- J	0 / 1940	-38.5	-38.5	0.47 (1)	10.00
J- O	0 / 1940	-38.5	-38.5	0.47 (1)	10.00
O- P	0 / 1940	-38.5	-38.5	0.47 (1)	10.00
P- I	0 / 1940	-38.5	-38.5	0.47 (1)	10.00
I- Q	0 / 3362	-38.5	-38.5	0.54 (1)	10.00
Q- R	0 / 3362	-38.5	-38.5	0.54 (1)	10.00
R- H	0 / 3362	-38.5	-38.5	0.54 (1)	10.00
H- S	0 / 0	-38.5	-38.5	0.07 (2)	10.00
S- G	0 / 0	-38.5	-38.5	0.07 (2)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-643 / 0	0.10 (1)
B- K	0 / 227	0.03 (1)
K- C	0 / 495	0.06 (2)
C- I	0 / 1014	0.13 (1)
I- D	0 / 1703	0.21 (1)
I- E	-1006 / 0	0.30 (1)
H- E	-1551 / 0	0.51 (1)
H- S	0 / 1886	0.24 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEIGHT	COMB.
H	17-7-4	-291	-291	---	FRONT	VERT	TOTAL	---	---
N	19-7-4	-62	-62	---	FRONT	VERT	TOTAL	---	---
O	10-9-8	-1292	-1292	---	FRONT	VERT	TOTAL	---	---
P	11-7-4	-304	-304	---	FRONT	VERT	TOTAL	---	---
Q	13-7-4	-304	-304	---	FRONT	VERT	TOTAL	---	---
R	15-7-4	-304	-304	---	FRONT	VERT	TOTAL	---	---
S	19-7-4	-39	-49	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.26/1.00 (F-G:1) , BC=0.54/1.00 (H-I:1) , WB=0.51/1.00 (F-H:1) , SSI=0.45/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H J G ALVES
100009977
PROVINCE OF ONTARIO

DWG NO. TAM T1903362
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G2	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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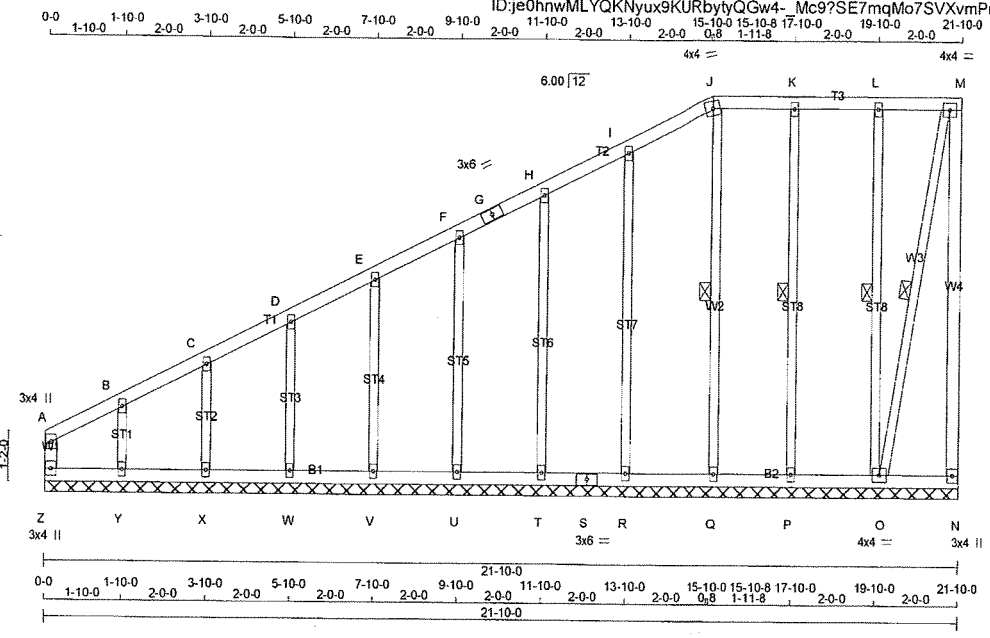


TOTAL WEIGHT = 2 X 116 = 232 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Z - A	2x4	DRY	No.2	SPF
Z - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMV+p	MT20	3.0	4.0	
B, C, D, E, F, H, I, K, L					
B	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
J	TTW-m	MT20	4.0	4.0	
M	TMVW-t	MT20	4.0	4.0	
N	BMV1+p	MT20	3.0	4.0	
O	BMWW1-t	MT20	4.0	4.0	
P, Q, R, T, U, V, W, X, Y					
P	BMW1+w	MT20	2.0	4.0	
S	BS-t	MT20	3.0	6.0	
Z	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 150 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

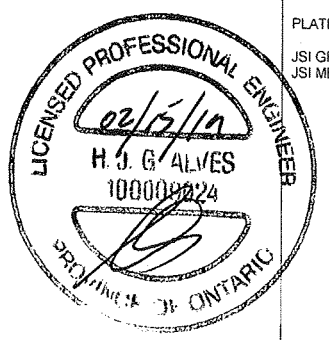
1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-O, K-P, J-Q, M-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 7	-78.0	-78.0	0.04 (1)	10.00	O- L	-164 / 0	0.09 (1)
B- C	0 / 7	-78.0	-78.0	0.04 (1)	10.00	P- K	-174 / 0	0.09 (1)
C- D	0 / 11	-78.0	-78.0	0.04 (1)	10.00	R- I	-179 / 0	0.23 (1)
D- E	0 / 14	-78.0	-78.0	0.04 (1)	10.00	T- H	-150 / 0	0.13 (1)
E- F	0 / 16	-78.0	-78.0	0.04 (1)	10.00	U- F	-156 / 0	0.09 (1)
F- G	0 / 18	-78.0	-78.0	0.04 (1)	6.25	V- E	-155 / 0	0.06 (1)
G- H	0 / 18	-78.0	-78.0	0.04 (1)	6.25	W- D	-155 / 0	0.04 (1)
H- I	0 / 22	-78.0	-78.0	0.05 (1)	10.00	X- C	-154 / 0	0.03 (1)
I- J	0 / 14	-78.0	-78.0	0.05 (1)	10.00	Y- B	-159 / 0	0.02 (1)
J- K	0 / 21	-78.0	-78.0	0.05 (1)	10.00	Q- J	-136 / 0	0.07 (1)
K- L	0 / 21	-78.0	-78.0	0.05 (1)	10.00	O- M	-120 / 0	0.07 (1)
L- M	0 / 24	-78.0	-78.0	0.04 (1)	10.00			
N- M	0 / 52	0.0	0.0	0.01 (1)	10.00			
Z- A	-54 / 0	0.0	0.0	0.01 (1)	7.81			
Z- Y	0 / 0	-38.5	-38.5	0.03 (3)	10.00			
Y- X	-6 / 0	-38.5	-38.5	0.03 (3)	10.00			
X- W	-10 / 0	-38.5	-38.5	0.02 (3)	10.00			
W- V	-12 / 0	-38.5	-38.5	0.02 (3)	6.25			
V- U	-14 / 0	-38.5	-38.5	0.02 (3)	6.25			
U- T	-16 / 0	-38.5	-38.5	0.02 (3)	6.25			
T- S	-18 / 0	-38.5	-38.5	0.03 (3)	6.25			
S- R	-18 / 0	-38.5	-38.5	0.03 (3)	6.25			
R- Q	-19 / 0	-38.5	-38.5	0.03 (3)	6.25			
Q- P	-21 / 0	-38.5	-38.5	0.02 (3)	6.25			
P- O	-21 / 0	-38.5	-38.5	0.03 (3)	6.25			
O- N	0 / 0	-38.5	-38.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (H-I:1) , BC=0.03/1.00 (O-P:3) , WB=0.23/1.00 (I-R:1) , SSI=0.07/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.07 (I) (INPUT = 1.00)

DWG NO. TAM 1400343 3
STRUCTURAL
COMPONENTS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T3XS	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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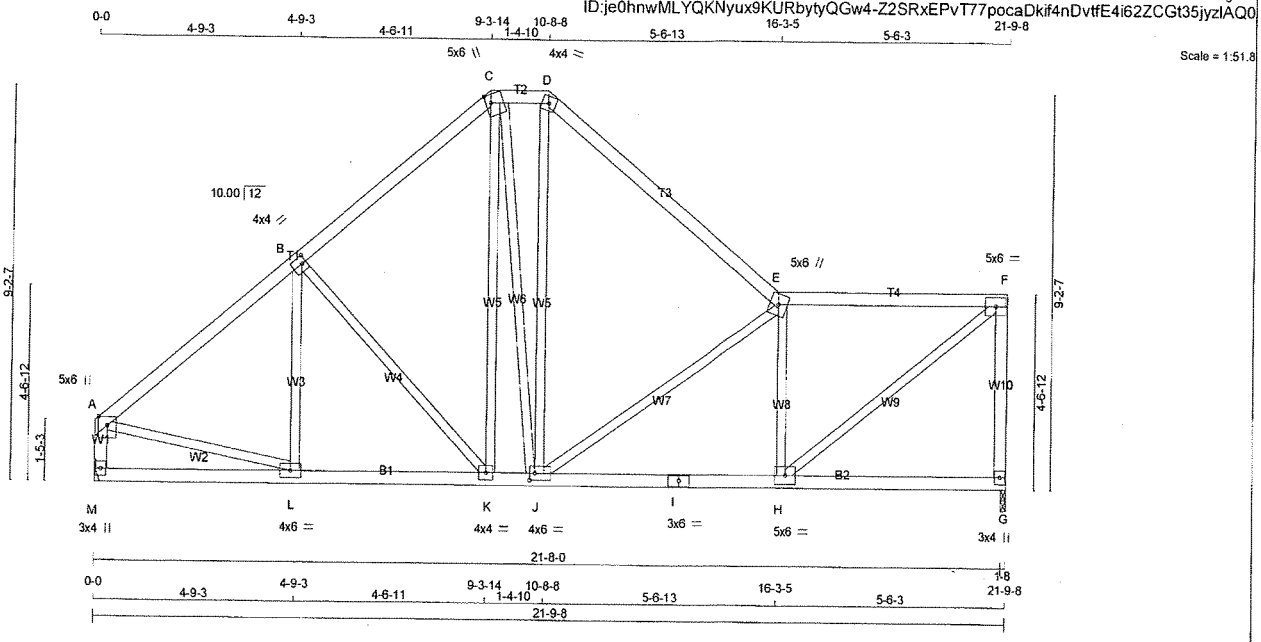

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1269	0	1269	0	0	1-8	1-8
M	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
M	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1261 / 0	-78.0	-78.0	0.24 (1)	5.47
B- C	-1022 / 0	-78.0	-78.0	0.23 (1)	5.93
C- D	-787 / 0	-78.0	-78.0	0.03 (1)	6.25
D- E	-1018 / 0	-78.0	-78.0	0.33 (1)	5.80
E- F	-1236 / 0	-78.0	-78.0	0.33 (1)	5.39
G- F	-1186 / 0	0.0	0.0	0.39 (1)	7.33
M- A	-1196 / 0	0.0	0.0	0.12 (1)	7.31
M- L	0 / 0	-38.5	-38.5	0.17 (3)	10.00
L- K	0 / 990	-38.5	-38.5	0.30 (2)	10.00
K- J	0 / 765	-38.5	-38.5	0.26 (2)	10.00
J- I	0 / 1254	-38.5	-38.5	0.39 (2)	10.00
I- H	0 / 1254	-38.5	-38.5	0.39 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-17 / 146	0.03 (3)
B- K	-337 / 0	0.30 (1)
K- C	0 / 303	0.07 (1)
C- J	0 / 149	0.03 (1)
J- D	0 / 397	0.09 (2)
J- E	-590 / 0	0.57 (1)
H- E	-736 / 0	0.23 (1)
H- F	0 / 1575	0.35 (1)
A- L	0 / 1015	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (F-G:1) , BC=0.39/1.00 (H-J:2) , WB=0.57/1.00 (E-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (F) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)

DWG NO. TAM T1903363
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T4XS	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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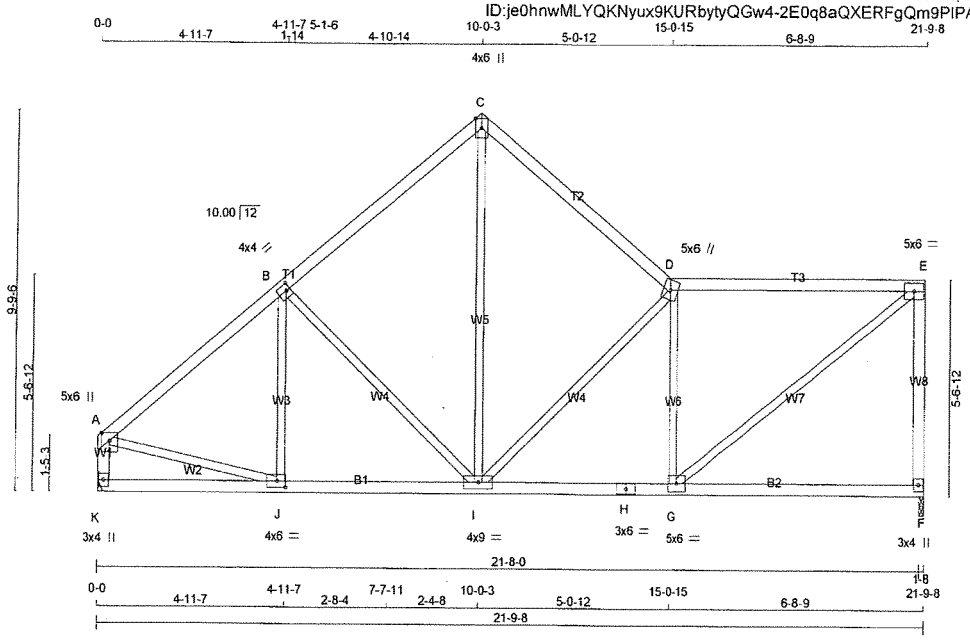


TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	4.0	6.0	Edge	
D	TTWW+m	MT20	5.0	6.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

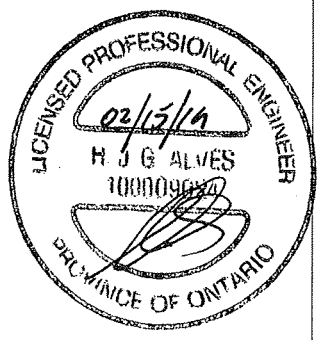
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1264 / 0	-78.0	-78.0	0.27 (1)	5.43
B- C	-985 / 0	-78.0	-78.0	0.27 (1)	5.96
C- D	-954 / 0	-78.0	-78.0	0.27 (1)	6.03
D- E	-1136 / 0	-78.0	-78.0	0.48 (1)	5.32
F- E	-1164 / 0	0.0	0.0	0.65 (1)	7.39
K- A	-1195 / 0	0.0	0.0	0.12 (1)	7.31
K- J	0 / 0	-38.5	-38.5	0.18 (3)	10.00
J- I	0 / 995	-38.5	-38.5	0.31 (2)	10.00
I- H	0 / 1149	-38.5	-38.5	0.43 (2)	10.00
H- G	0 / 1149	-38.5	-38.5	0.43 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.28 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	-641 / 0	0.30 (1)
G- E	0 / 1453	0.33 (1)
I- C	0 / 867	0.20 (1)
I- D	-598 / 0	0.62 (1)
J- B	-7 / 157	0.04 (3)
B- I	-368 / 0	0.38 (1)
A- J	0 / 1020	0.23 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.43/1.00 (G-I:2) , WB=0.62/1.00 (D-I:1) . SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.48 (A) (INPUT = 1.00)

DWG NO. TAM T1903364
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T5X	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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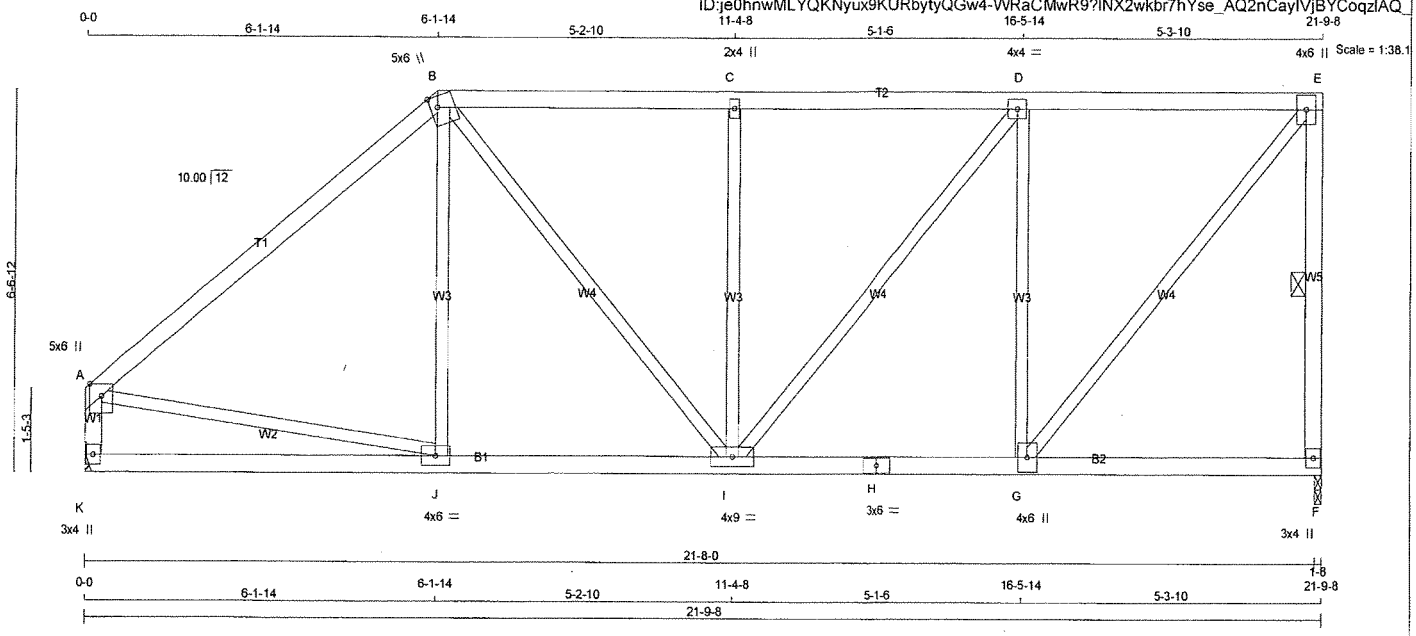


TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

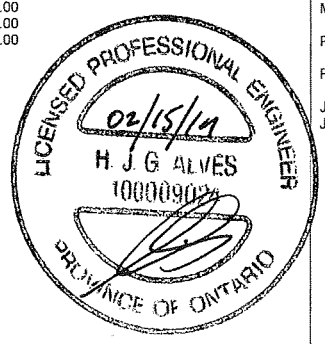
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1200 / 0	-78.0	-78.0	0.63 (1)	5.00
B- C	-1140 / 0	-78.0	-78.0	0.30 (1)	5.57
C- D	-1141 / 0	-78.0	-78.0	0.32 (1)	5.55
D- E	-863 / 0	-78.0	-78.0	0.31 (1)	6.17
F- E	-1188 / 0	0.0	0.0	0.24 (1)	5.86
K- A	-1172 / 0	0.0	0.0	0.12 (1)	7.37
K- J	0 / 0	-38.5	-38.5	0.26 (3)	10.00
J- I	0 / 922	-38.5	-38.5	0.38 (2)	10.00
I- H	0 / 863	-38.5	-38.5	0.32 (2)	10.00
H- G	0 / 863	-38.5	-38.5	0.32 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 238	0.06 (3)
A- J	0 / 934	0.21 (1)
G- E	0 / 1336	0.30 (1)
B- I	0 / 341	0.08 (1)
G- D	-788 / 0	0.56 (1)
I- C	-434 / 0	0.31 (1)
I- D	0 / 440	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.63/1.00 (A-B:1) , BC=0.38/1.00 (I-J:2) , WB=0.56/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T1903368
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BLOCK 1 FREEHOLDUNITS 1 - 4	DRWG NO.
400998	T6X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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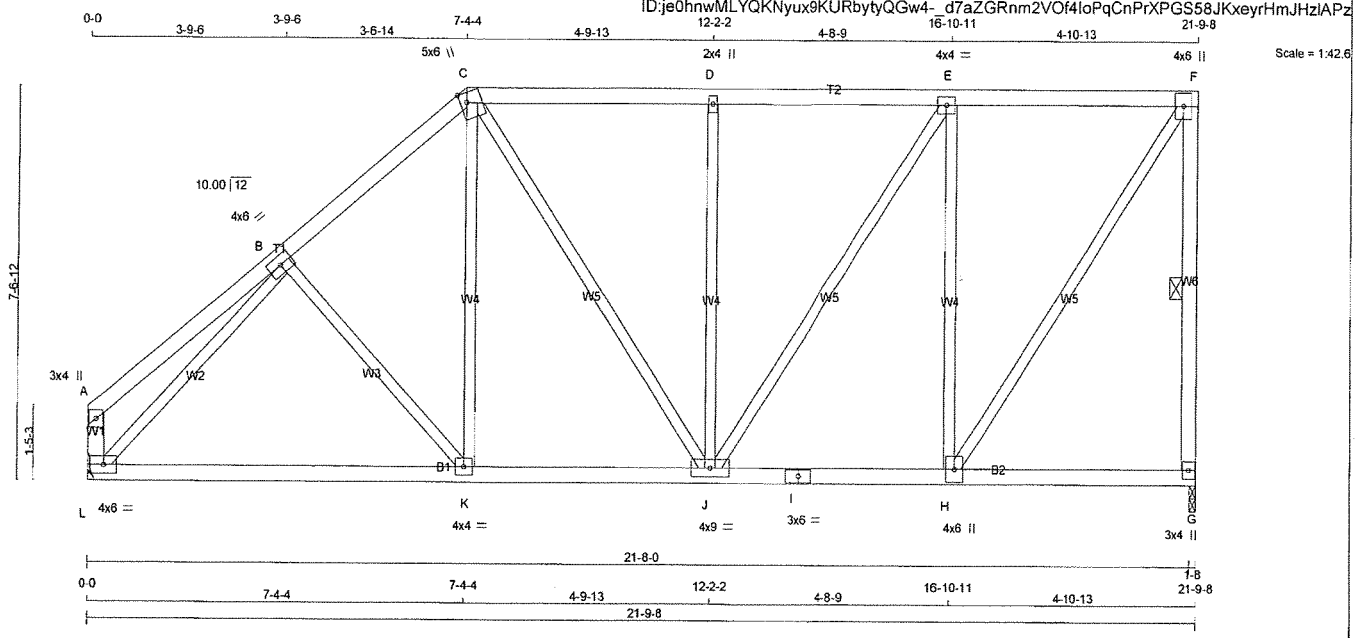


TOTAL WEIGHT = 107 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1269	0	1269	0	0	1-8	1-8
L	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

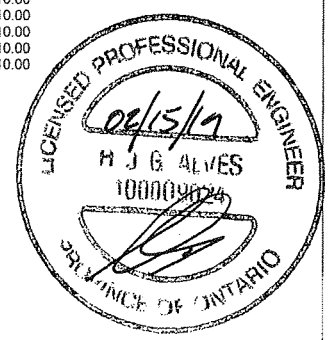
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-78.0	-78.0	0.16 (1)	10.00	B- K	-61 / 55	0.03 (1)
B- C	-1165 / 0	-78.0	-78.0	0.18 (1)	5.69	K- C	0 / 384	0.09 (2)
C- D	-964 / 0	-78.0	-78.0	0.25 (1)	6.01	L- B	-1388 / 0	0.69 (1)
D- E	-965 / 0	-78.0	-78.0	0.26 (1)	5.99	H- F	0 / 1254	0.28 (1)
E- F	-701 / 0	-78.0	-78.0	0.25 (1)	6.25	C- J	0 / 150	0.03 (1)
G- F	-1195 / 0	0.0	0.0	0.31 (1)	5.85	H- E	-819 / 0	0.85 (1)
L- A	-111 / 0	0.0	0.0	0.01 (1)	7.81	J- D	-400 / 0	0.42 (1)
						J- E	0 / 484	0.11 (1)
L- K	0 / 918	-38.5	-38.5	0.47 (2)	10.00			
K- J	0 / 882	-38.5	-38.5	0.47 (2)	10.00			
J- I	0 / 701	-38.5	-38.5	0.27 (2)	10.00			
I- H	0 / 701	-38.5	-38.5	0.27 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.31/1.00 (F-G:1) , BC=0.47/1.00 (K-L:2) , WB=0.85/1.00 (E-H:1) , SSI=0.18/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

DWG NO. TAM T1903366
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T7X	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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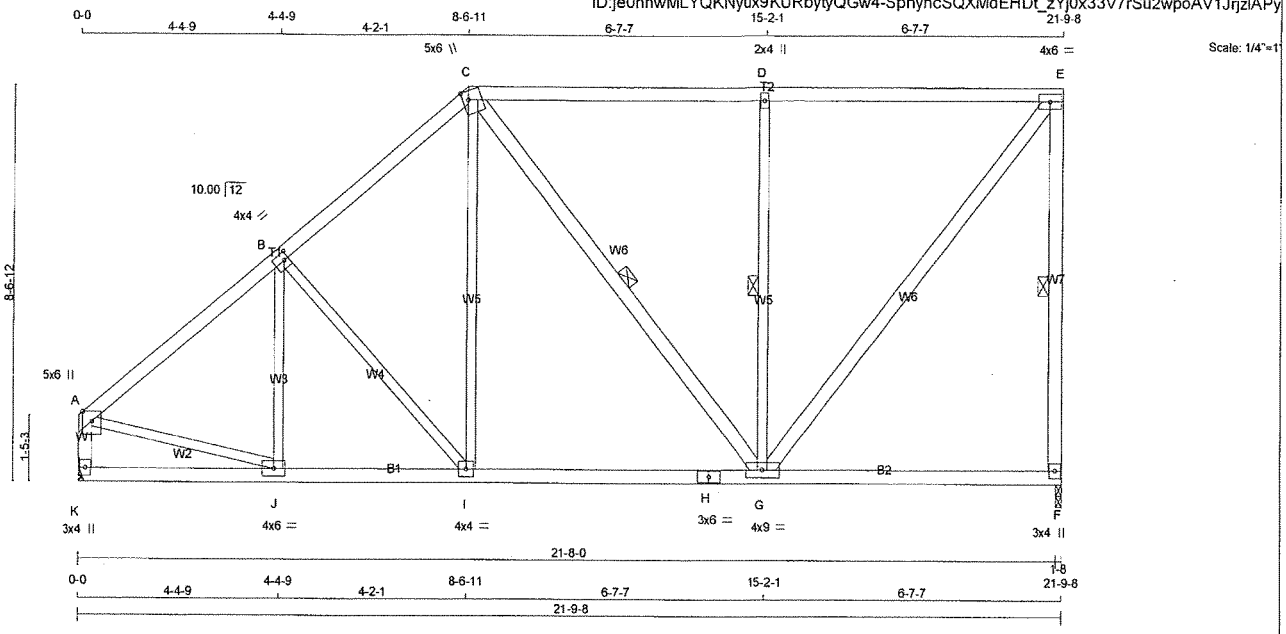


TOTAL WEIGHT = 111 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

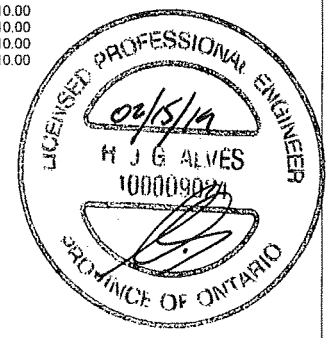
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1255 / 0	-78.0	-78.0	0.27 (1)		5.43
B- C	-1079 / 0	-78.0	-78.0	0.26 (1)		5.76
C- D	-782 / 0	-78.0	-78.0	0.62 (1)		5.75
D- E	-782 / 0	-78.0	-78.0	0.62 (1)		5.75
F- E	-1168 / 0	0.0	0.0	0.39 (1)		5.90
K- A	-1198 / 0	0.0	0.0	0.12 (1)		7.31
K- J	0 / 0	-38.5	-38.5	0.12 (3)		10.00
J- I	0 / 983	-38.5	-38.5	0.29 (2)		10.00
I- H	0 / 812	-38.5	-38.5	0.43 (2)		10.00
H- G	0 / 812	-38.5	-38.5	0.43 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.33 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-73 / 89	0.03 (1)
B- I	-264 / 0	0.19 (1)
I- C	0 / 425	0.10 (2)
C- G	-52 / 0	0.03 (2)
G- D	-640 / 0	0.30 (1)
G- E	0 / 1247	0.20 (1)
A- J	0 / 1013	0.23 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.30/1.00 (D-G:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T1903367
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T8X	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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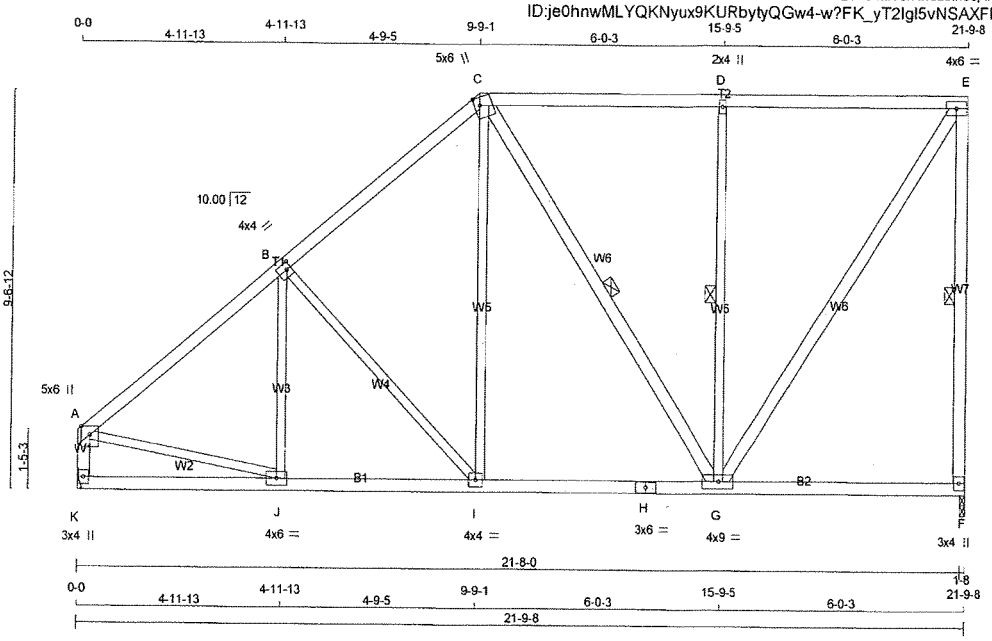


TOTAL WEIGHT = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1269	0	1269	0	0	IN-SX	IN-SX
K	1269	0	1269	0	0	1-8	1-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

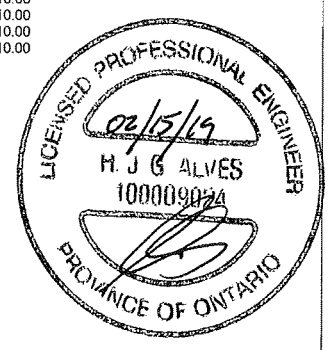
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1258 / 0	-78.0	-78.0	0.37 (1)	5.30
B- C	-1001 / 0	-78.0	-78.0	0.34 (1)	5.80
C- D	-652 / 0	-78.0	-78.0	0.50 (1)	6.25
D- E	-652 / 0	-78.0	-78.0	0.50 (1)	6.25
F- E	-1177 / 0	0.0	0.0	0.51 (1)	5.88
K- A	-1191 / 0	0.0	0.0	0.12 (1)	7.32
K- J	0 / 0	-38.5	-38.5	0.17 (3)	10.00
J- I	0 / 988	-38.5	-38.5	0.30 (2)	10.00
I- H	0 / 749	-38.5	-38.5	0.37 (2)	10.00
H- G	0 / 749	-38.5	-38.5	0.37 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.27 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-9 / 150	0.04 (3)
B- I	-359 / 0	0.35 (1)
I- C	0 / 480	0.11 (2)
C- G	-180 / 0	0.12 (1)
G- D	-581 / 0	0.35 (1)
G- E	0 / 1191	0.19 (1)
A- J	0 / 1012	0.23 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.37/1.00 (G-I:2) , WB=0.35/1.00 (B-I:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM T1903368
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T9X	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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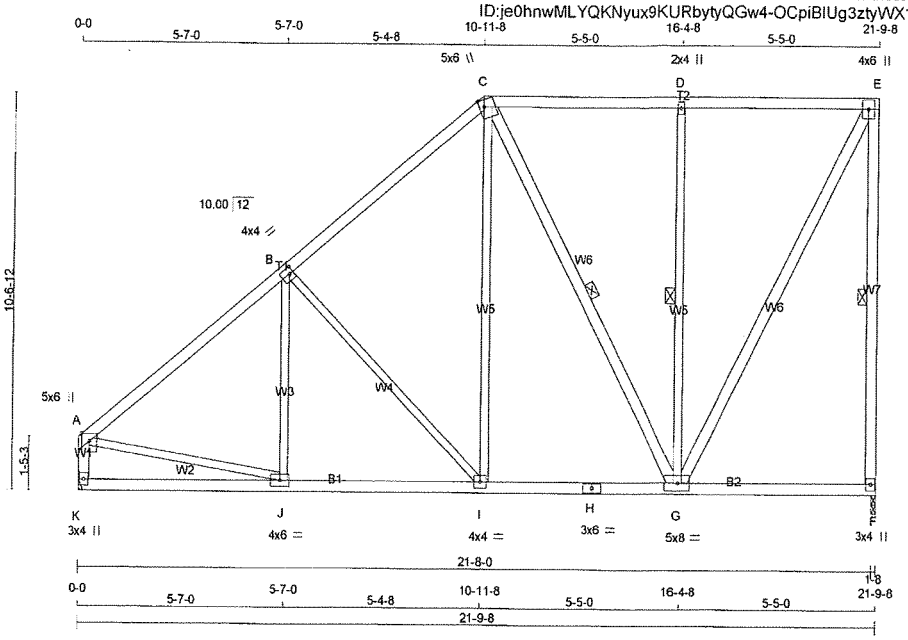


TOTAL WEIGHT = 123 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

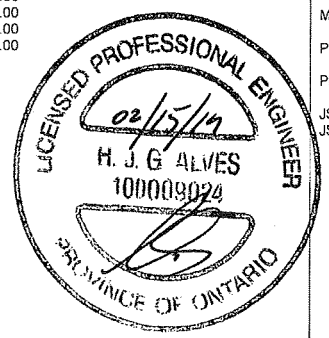
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1252 / 0	-78.0	-78.0	0.48 (1)	5.16
B- C	-918 / 0	-78.0	-78.0	0.44 (1)	5.83
C- D	-543 / 0	-78.0	-78.0	0.39 (1)	6.25
D- E	-544 / 0	-78.0	-78.0	0.39 (1)	6.25
F- E	-1185 / 0	0.0	0.0	0.66 (1)	5.87
K- A	-1183 / 0	0.0	0.0	0.12 (1)	7.34
K- J	0 / 0	-38.5	-38.5	0.22 (3)	10.00
J- I	0 / 987	-38.5	-38.5	0.35 (2)	10.00
I- H	0 / 683	-38.5	-38.5	0.30 (2)	10.00
H- G	0 / 683	-38.5	-38.5	0.30 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 204	0.05 (3)
B- I	-447 / 0	0.59 (1)
I- C	0 / 534	0.12 (2)
C- G	-300 / 0	0.22 (1)
G- D	-522 / 0	0.41 (1)
G- E	0 / 1158	0.19 (1)
A- J	0 / 1005	0.23 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.66/1.00 (E-F:1) , BC=0.35/1.00 (I-J:2) , WB=0.59/1.00 (B-I:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (A) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM 07903369
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T10	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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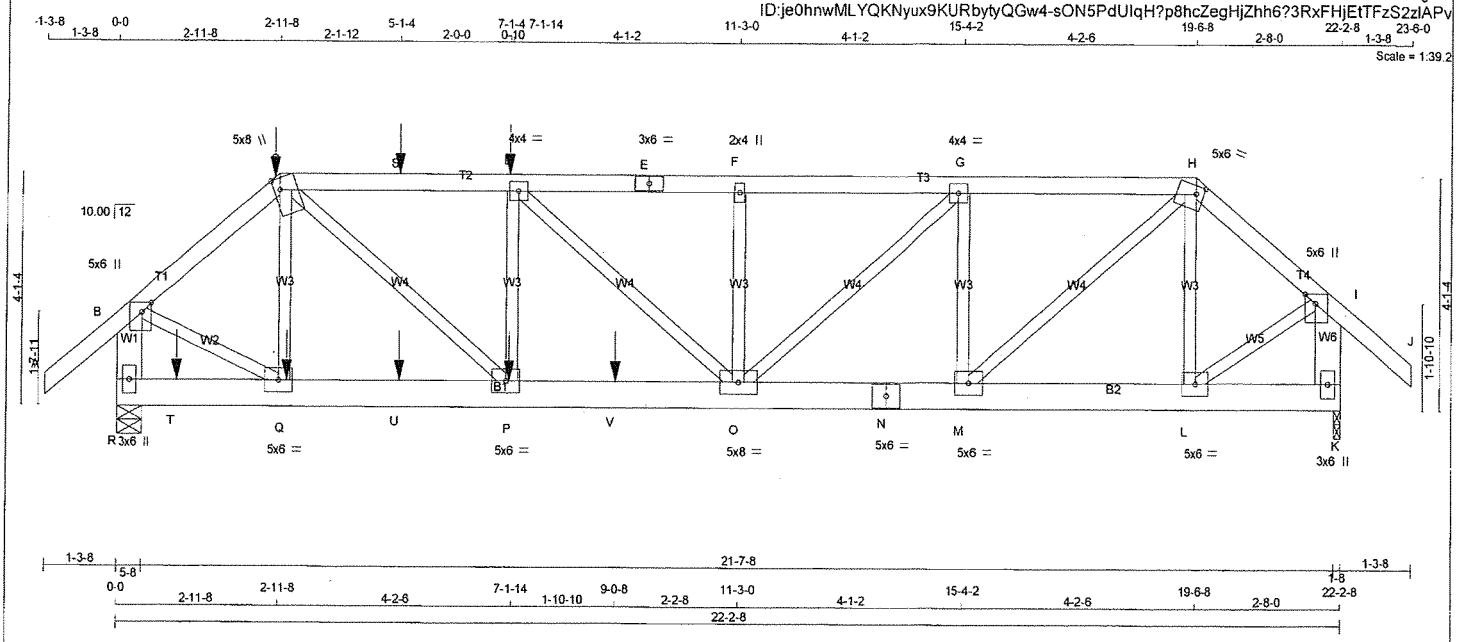

TOTAL WEIGHT = 2 X 111 = 222 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	TOP
H- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2430	0	2430	0	0	5-8	5-8
K	1937	0	1937	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1852	884 / 0	427 / 0	0 / 0	0 / 0	541 / 0	0 / 0
K	1469	726 / 0	321 / 0	0 / 0	0 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.06 (1)	10.00	Q- C	-510 / 0	0.06 (1)
B- C	-2232 / 0	-78.0	-78.0	0.08 (1)	5.90	C- P	0 / 2328	0.29 (1)
C- S	-3439 / 0	-78.0	-78.0	0.20 (1)	4.86	P- D	-644 / 0	0.08 (1)
S- D	-3439 / 0	-78.0	-78.0	0.20 (1)	4.86	D- O	0 / 199	0.02 (1)
D- E	-3586 / 0	-78.0	-78.0	0.17 (1)	4.81	O- F	-276 / 0	0.04 (1)
E- F	-3586 / 0	-78.0	-78.0	0.17 (1)	4.81	O- G	0 / 1268	0.16 (1)
F- G	-3586 / 0	-78.0	-78.0	0.15 (1)	4.83	M- G	-1214 / 0	0.15 (1)
G- H	-2649 / 0	-78.0	-78.0	0.13 (1)	5.47	M- H	0 / 1689	0.22 (1)
H- I	-1628 / 0	-78.0	-78.0	0.06 (1)	6.25	L- H	0 / 0	0.06 (1)
I- J	0 / 35	-78.0	-78.0	0.06 (1)	10.00	B- Q	1536 / 0	0.19 (1)
R- B	-2342 / 0	0.0	0.0	0.09 (1)	7.81	L- P	67 / 1408	0.17 (1)
K- I	-1920 / 0	0.0	0.0	0.07 (1)	7.81			
R- T	0 / 0	-38.5	-38.5	0.03 (2)	10.00			
T- Q	0 / 0	-38.5	-38.5	0.03 (2)	10.00			
Q- U	0 / 1694	-38.5	-38.5	0.23 (1)	10.00			
U- P	0 / 1694	-38.5	-38.5	0.23 (1)	10.00			
P- V	0 / 3439	-38.5	-38.5	0.54 (1)	10.00			
V- O	0 / 3439	-38.5	-38.5	0.54 (1)	10.00			
O- N	0 / 2649	-38.5	-38.5	0.27 (1)	10.00			
N- M	0 / 2649	-38.5	-38.5	0.27 (1)	10.00			
M- L	0 / 1233	-38.5	-38.5	0.12 (1)	10.00			
L- K	0 / 0	-38.5	-38.5	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-100	-100	---	BACK	VERT	TOTAL	---	---
D	7-1-4	-94	-94	---	BACK	VERT	TOTAL	---	---
P	7-1-4	-55	-70	---	BACK	VERT	TOTAL	---	---
Q	3-1-4	-55	-70	---	BACK	VERT	TOTAL	---	---
S	5-1-4	-94	-94	---	BACK	VERT	TOTAL	---	---
T	1-1-4	-55	-70	---	BACK	VERT	TOTAL	---	---
U	5-1-4	-55	-70	---	BACK	VERT	TOTAL	---	---
V	9-0-8	-1056	-1056	---	TOP	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA.086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.54/1.00 (O-P:1) , WB=0.29/1.00 (C-P:1) , SSI=0.22/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.27 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H J G ALVES
100009274
PROVINCE OF ONTARIO

DWG NO. TAM T1903370
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T11	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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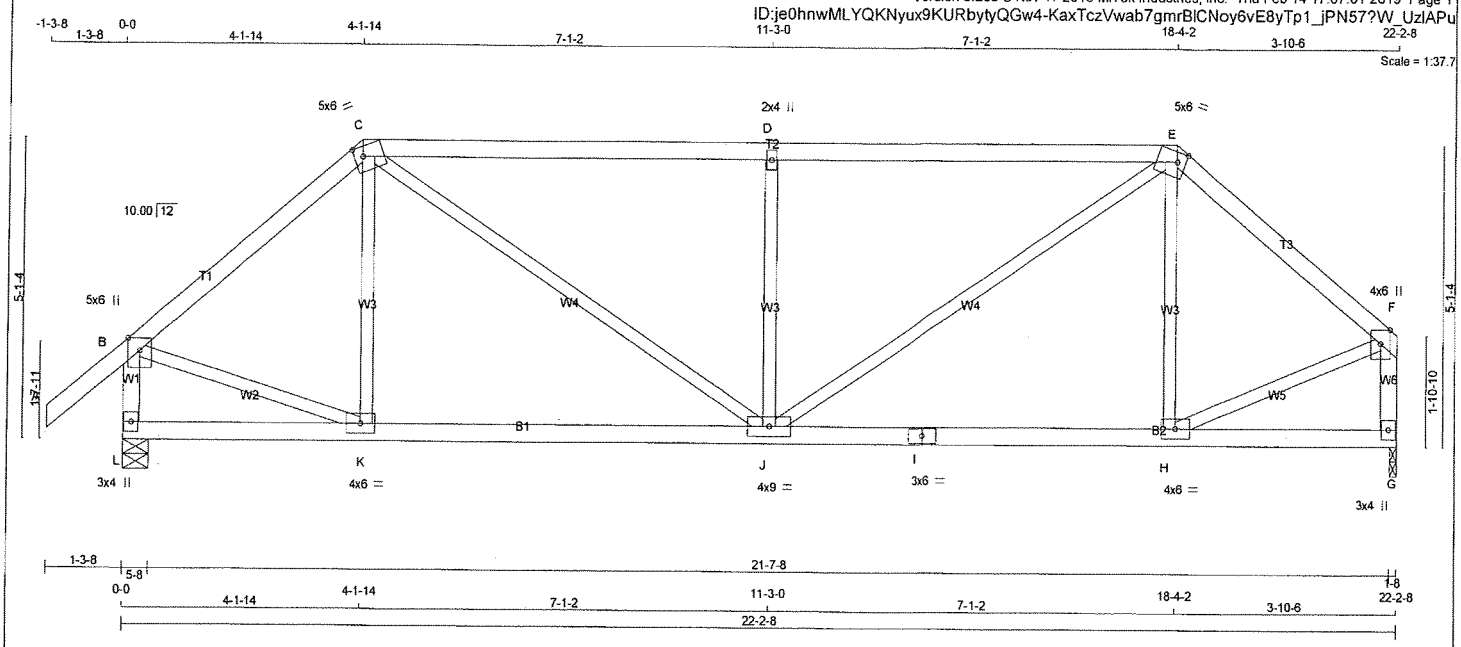

TOTAL WEIGHT = 90 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1402	0	1402	0	0	5-8	5-8
G	1294	0	1294	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1063	525 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	988	466 / 0	233 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	K- C	-47 / 141	0.03 (3)
B- C	-1230 / 0	-78.0	-78.0	0.27 (1)	5.47	C- J	0 / 823	0.19 (1)
C- D	-1623 / 0	-78.0	-78.0	0.82 (1)	4.03	J- D	-677 / 0	0.26 (1)
D- E	-1623 / 0	-78.0	-78.0	0.82 (1)	4.03	J- E	0 / 884	0.20 (1)
E- F	-1166 / 0	-78.0	-78.0	0.23 (1)	5.63	H- E	-111 / 101	0.04 (1)
L- B	-1349 / 0	0.0	0.0	0.14 (1)	6.98	B- K	0 / 986	0.22 (1)
G- F	-1249 / 0	0.0	0.0	0.14 (1)	7.19	H- F	0 / 959	0.22 (1)
L- K	0 / 0	-38.5	-38.5	0.23 (3)	10.00			
K- J	0 / 942	-38.5	-38.5	0.42 (2)	10.00			
J- I	0 / 891	-38.5	-38.5	0.41 (2)	10.00			
I- H	0 / 891	-38.5	-38.5	0.41 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.42/1.00 (J-K:2) , WB=0.26/1.00 (D-J:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.53 (F) (INPUT = 1.00)

DWG NO. TAM P2003371
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T12	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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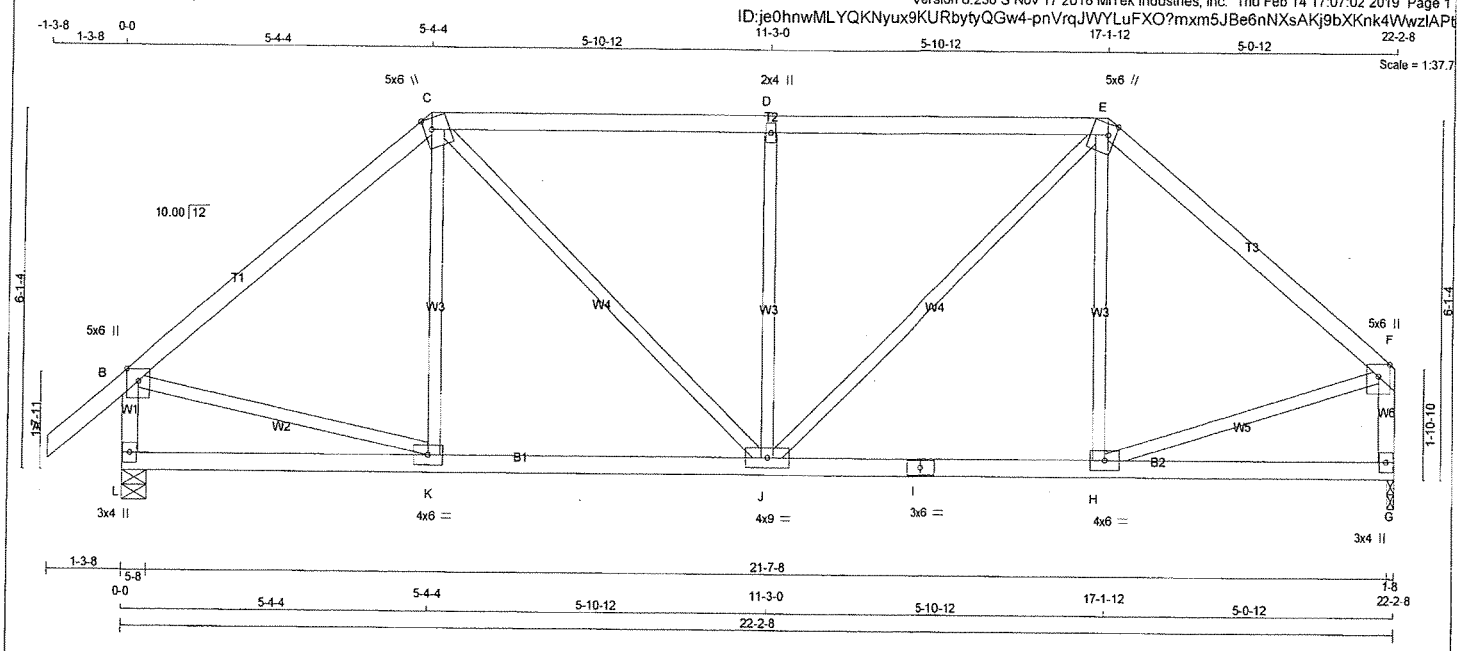

TOTAL WEIGHT = 94 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1402	0	1402	0	0	5-8	5-8
G	1294	0	1294	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1063	525 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	988	466 / 0	233 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-1215 / 0	-78.0	-78.0	0.46 (1)	5.24
C- D	-1307 / 0	-78.0	-78.0	0.51 (1)	4.96
D- E	-1307 / 0	-78.0	-78.0	0.51 (1)	4.96
E- F	-1169 / 0	-78.0	-78.0	0.40 (1)	5.39
L- B	-1319 / 0	0.0	0.0	0.14 (1)	7.04
G- F	-1217 / 0	0.0	0.0	0.14 (1)	7.26
L- K	0 / 0	-38.5	-38.5	0.23 (3)	10.00
K- J	0 / 932	-38.5	-38.5	0.35 (2)	10.00
J- I	0 / 896	-38.5	-38.5	0.34 (2)	10.00
I- H	0 / 896	-38.5	-38.5	0.34 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 185	0.04 (3)
C- J	0 / 527	0.12 (1)
J- D	-561 / 0	0.33 (1)
J- E	0 / 577	0.13 (1)
H- E	-30 / 150	0.04 (3)
B- K	0 / 957	0.22 (1)
H- F	0 / 936	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.35/1.00 (J-K:2) , WB=0.33/1.00 (D-J:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (K) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903372
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T13	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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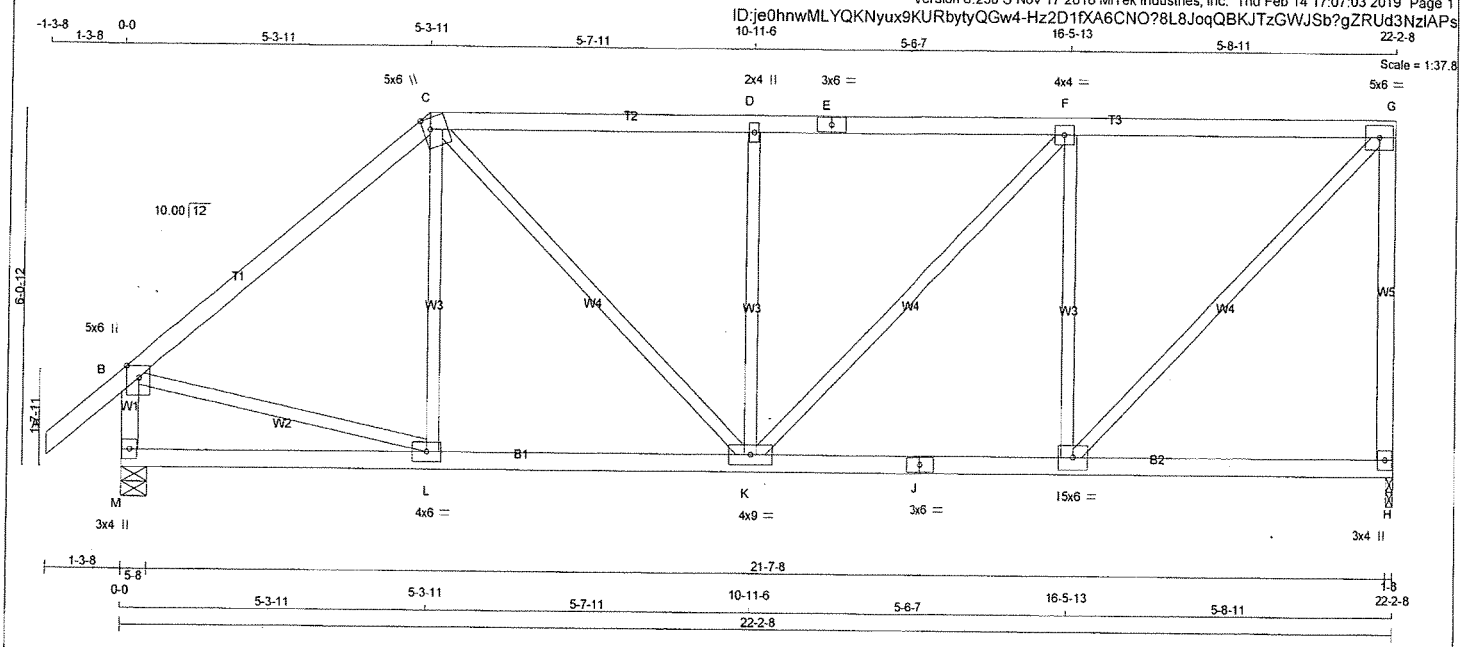


TOTAL WEIGHT = 99 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1294	0	1294	0	0	1-8	1-8
M	1402	0	1402	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	988	466 / 0	233 / 0	0 / 0	0 / 0	289 / 0	0 / 0
M	1063	525 / 0	233 / 0	0 / 0	0 / 0	305 / 0	0 / 0

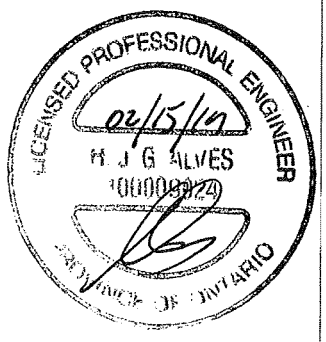
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	L- C	0 / 179	0.04 (3)
B- C	-1215 / 0	-78.0	-78.0	0.45 (1)	5.25	B- L	0 / 958	0.22 (1)
C- D	-1297 / 0	-78.0	-78.0	0.37 (1)	5.21	I- G	0 / 1443	0.32 (1)
D- E	-1298 / 0	-78.0	-78.0	0.38 (1)	5.19	C- K	0 / 522	0.12 (1)
E- F	-1298 / 0	-78.0	-78.0	0.38 (1)	5.19	I- F	-777 / 0	0.45 (1)
F- G	-1016 / 0	-78.0	-78.0	0.37 (1)	5.71	K- D	-470 / 0	0.27 (1)
H- G	-1205 / 0	0.0	0.0	0.85 (1)	7.29	K- F	0 / 407	0.09 (1)
M- B	-1320 / 0	0.0	0.0	0.14 (1)	7.04			
M- L	0 / 0	-38.5	-38.5	0.22 (3)	10.00			
L- K	0 / 932	-38.5	-38.5	0.35 (2)	10.00			
K- J	0 / 1016	-38.5	-38.5	0.37 (2)	10.00			
J- I	0 / 1016	-38.5	-38.5	0.37 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.85/1.00 (G-H:1) , BC=0.37/1.00 (I-K:2) , WB=0.45/1.00 (F-I:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (G) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM1903373
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T14A	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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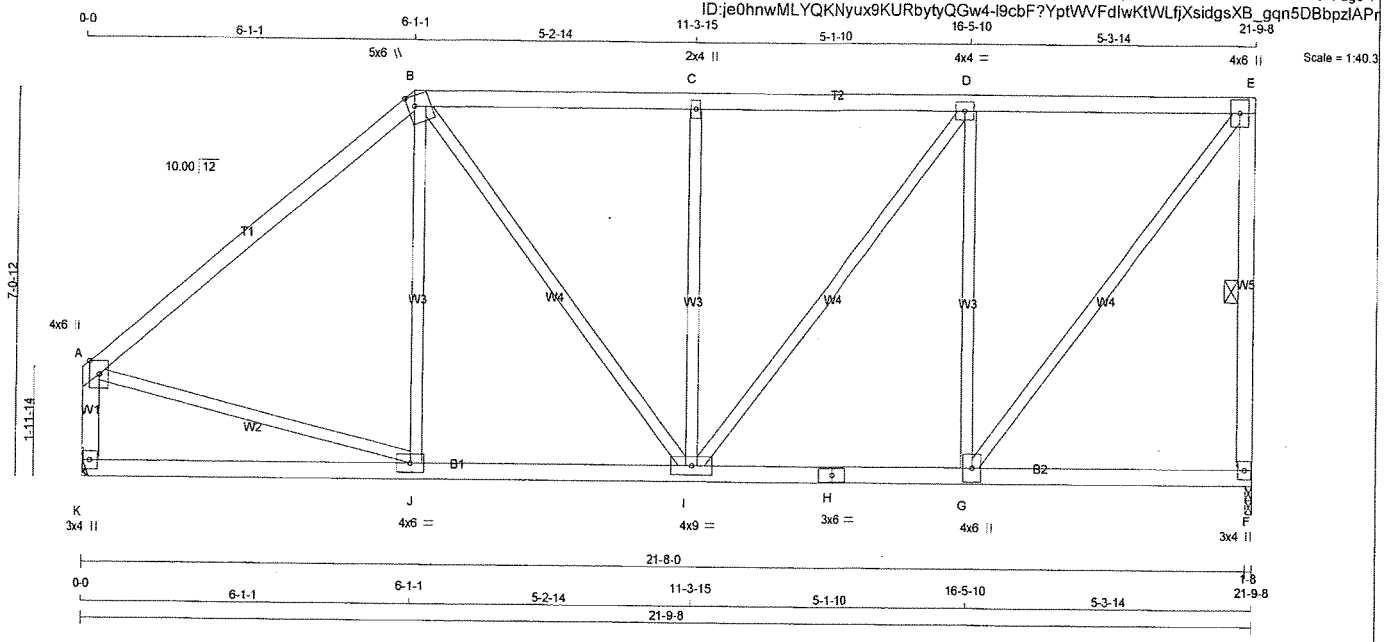


TOTAL WEIGHT = 102 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

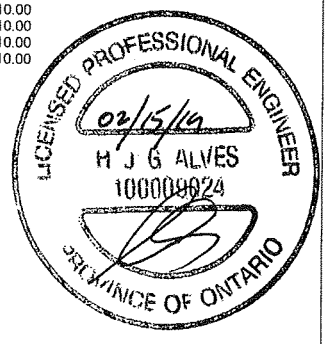
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1105 / 0	-78.0	-78.0	0.60 (1)		5.20
B- C	-1057 / 0	-78.0	-78.0	0.30 (1)		5.73
C- D	-1057 / 0	-78.0	-78.0	0.32 (1)		5.71
D- E	-802 / 0	-78.0	-78.0	0.31 (1)		6.25
F- E	-1188 / 0	0.0	0.0	0.27 (1)		5.86
K- A	-1173 / 0	0.0	0.0	0.13 (1)		7.36
K- J	0 / 0	-38.5	-38.5	0.26 (3)		10.00
J- I	0 / 848	-38.5	-38.5	0.37 (2)		10.00
I- H	0 / 802	-38.5	-38.5	0.31 (2)		10.00
H- G	0 / 802	-38.5	-38.5	0.31 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.21 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J- B	0 / 199	0.05 (3)
A- J	0 / 877	0.20 (1)
G- E	0 / 1297	0.29 (1)
B- I	0 / 341	0.08 (1)
G- D	-786 / 0	0.68 (1)
I- C	-437 / 0	0.38 (1)
I- D	0 / 423	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.60/1.00 (A-B:1) , BC=0.37/1.00 (I-J:2) , WB=0.68/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)

DWG NO. TAM T1203374
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T15A	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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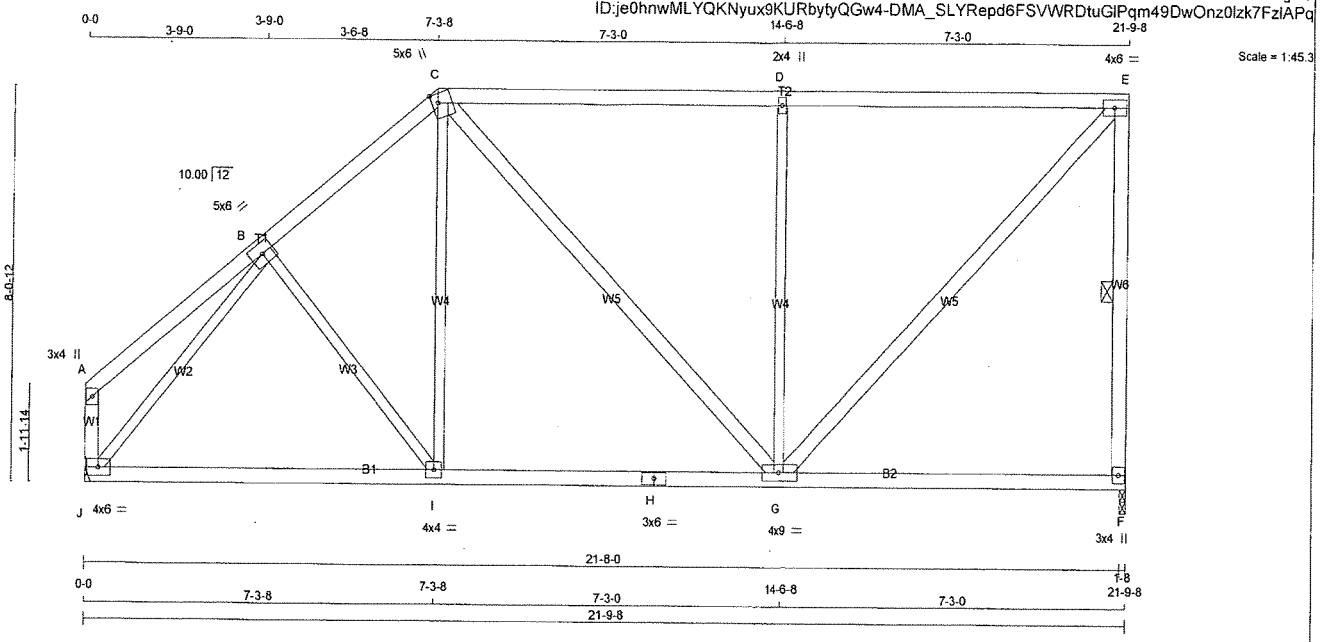


TOTAL WEIGHT = 107 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1269	0	1269	0	0	1-8	1-8
J	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
J	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

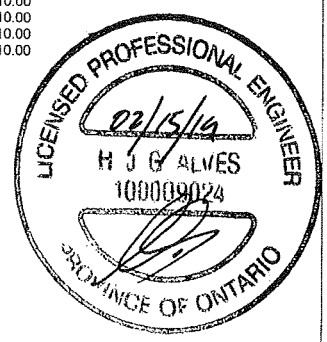
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-78.0	-78.0	0.16 (1)	10.00	B- I	0 / 93	0.02 (3)
B- C	-1088 / 0	-78.0	-78.0	0.18 (1)	5.85	I- C	0 / 348	0.08 (2)
C- D	-882 / 0	-78.0	-78.0	0.77 (1)	5.11	C- G	0 / 88	0.01 (1)
D- E	-882 / 0	-78.0	-78.0	0.77 (1)	5.11	G- D	-702 / 0	0.88 (1)
F- E	-1156 / 0	0.0	0.0	0.34 (1)	5.92	G- E	0 / 1289	0.21 (1)
J- A	-109 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1322 / 0	0.78 (1)
J- I	0 / 810	-38.5	-38.5	0.46 (2)	10.00			
I- H	0 / 822	-38.5	-38.5	0.47 (2)	10.00			
H- G	0 / 822	-38.5	-38.5	0.47 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.36 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.77/1.00 (D-E:1) , BC=0.47/1.00 (G-I:2) , WB=0.88/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)

DWG NO. TAM T1903576
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T16A	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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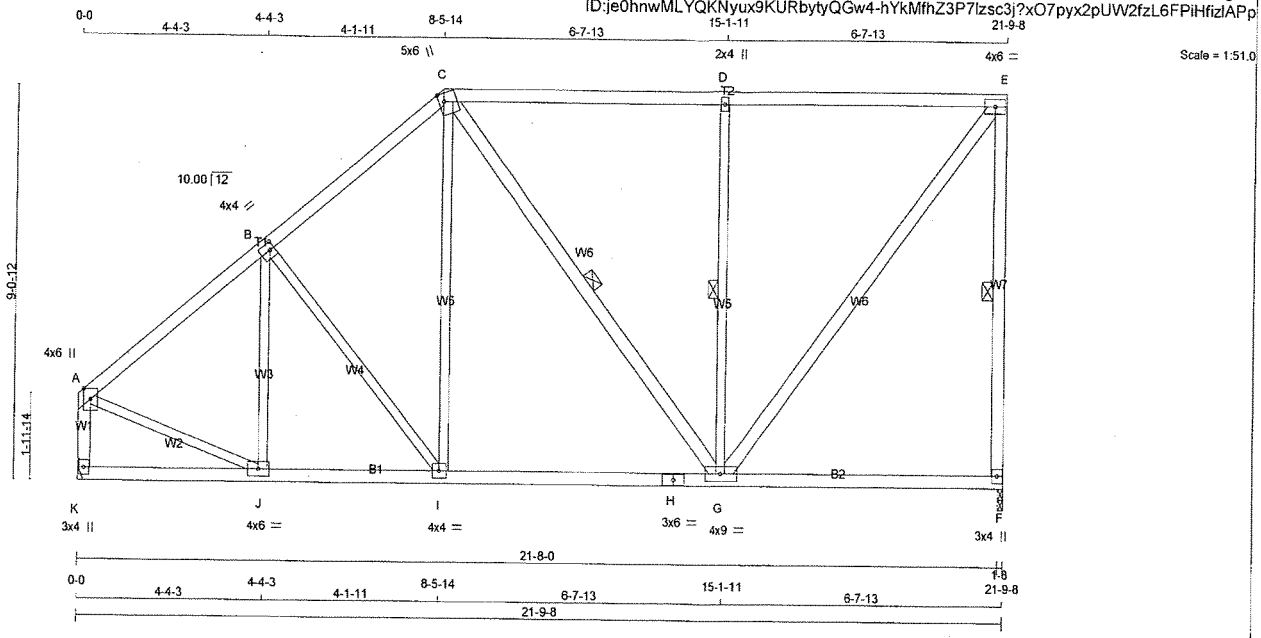


TOTAL WEIGHT = 2 X 115 = 230 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

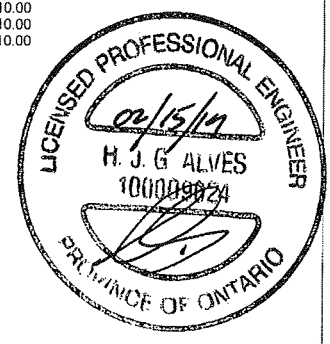
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1121 / 0	-78.0	-78.0	0.26 (1)	5.68
B- C	-1016 / 0	-78.0	-78.0	0.26 (1)	5.89
C- D	-740 / 0	-78.0	-78.0	0.62 (1)	5.86
D- E	-740 / 0	-78.0	-78.0	0.62 (1)	5.86
F- E	-1168 / 0	0.0	0.0	0.45 (1)	5.90
K- A	-1199 / 0	0.0	0.0	0.14 (1)	7.30
K- J	0 / 0	-38.5	-38.5	0.11 (3)	10.00
J- I	0 / 880	-38.5	-38.5	0.28 (2)	10.00
I- H	0 / 763	-38.5	-38.5	0.43 (2)	10.00
H- G	0 / 763	-38.5	-38.5	0.43 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.34 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-162 / 39	0.08 (1)
B- I	-193 / 0	0.16 (1)
I- C	0 / 383	0.09 (2)
C- G	-43 / 0	0.03 (2)
G- D	-643 / 0	0.34 (1)
G- E	0 / 1220	0.20 (1)
A- J	0 / 941	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.34/1.00 (D-G:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

DWG NO. TAM 77403376
STRUCTURAL
COMPONENTS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T17A	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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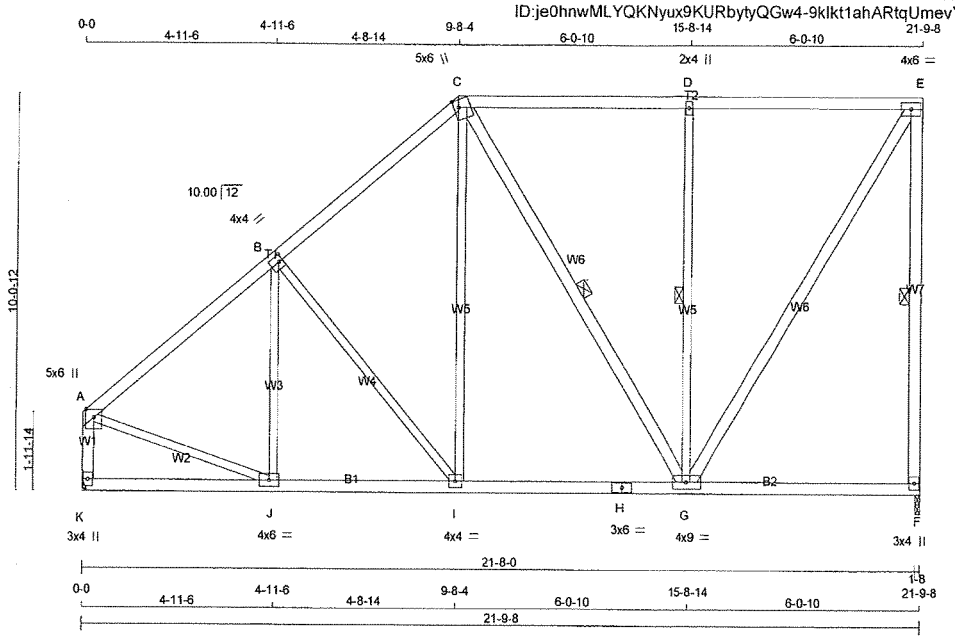


TOTAL WEIGHT = 121 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

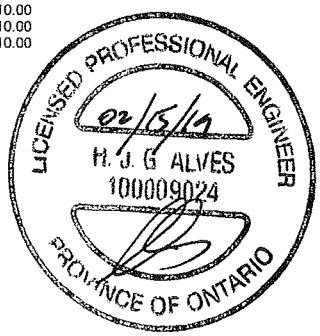
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1135 / 0	-78.0	-78.0	0.35 (1)	5.53
B- C	-950 / 0	-78.0	-78.0	0.34 (1)	5.92
C- D	-621 / 0	-78.0	-78.0	0.50 (1)	6.25
D- E	-621 / 0	-78.0	-78.0	0.50 (1)	6.25
F- E	-1176 / 0	0.0	0.0	0.58 (1)	5.88
K- A	-1192 / 0	0.0	0.0	0.14 (1)	7.32
K- J	0 / 0	-38.5	-38.5	0.17 (3)	10.00
J- I	0 / 894	-38.5	-38.5	0.28 (2)	10.00
I- H	0 / 710	-38.5	-38.5	0.36 (2)	10.00
H- G	0 / 710	-38.5	-38.5	0.36 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.28 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-91 / 104	0.05 (1)
B- I	-293 / 0	0.33 (1)
I- C	0 / 445	0.10 (2)
C- G	-170 / 0	0.12 (1)
G- D	-585 / 0	0.40 (1)
G- E	0 / 1173	0.19 (1)
A- J	0 / 942	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	44.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.36/1.00 (G-I:2) , WB=0.40/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 1.00)

DWG NO. TAM 7/1903377
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T20A	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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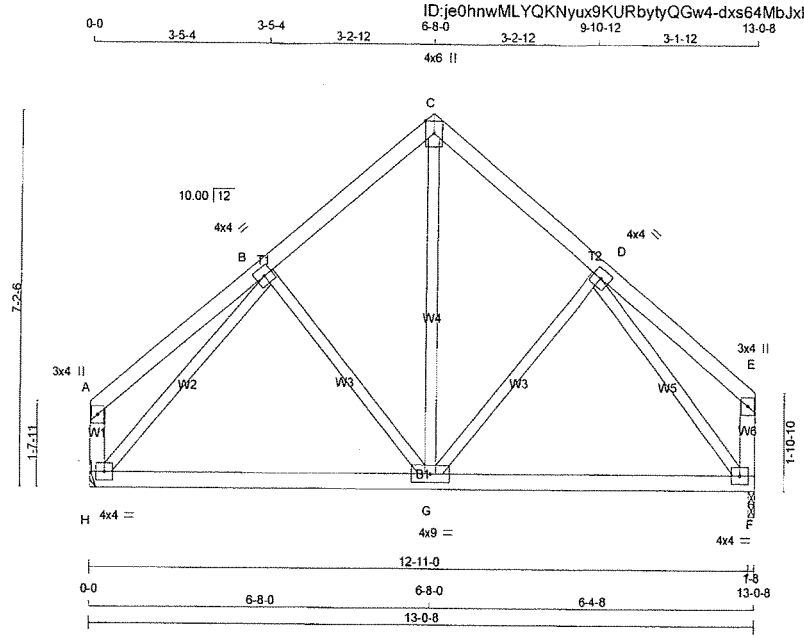


TOTAL WEIGHT = 2 X 59 = 118 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	760	0	760	0	0	MECHANICAL	
F	760	0	760	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	580	274 / 0	137 / 0	0 / 0	0 / 0	170 / 0	0 / 0
F	580	274 / 0	137 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

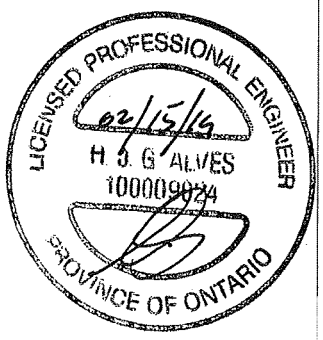
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-78.0	-78.0	0.13 (1)	10.00
B- C	-526 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	-524 / 0	-78.0	-78.0	0.10 (1)	6.25
D- E	0 / 19	-78.0	-78.0	0.12 (1)	10.00
H- A	-100 / 0	0.0	0.0	0.01 (1)	7.81
F- E	-88 / 0	0.0	0.0	0.01 (1)	7.81
H- G	0 / 457	-38.5	-38.5	0.41 (2)	10.00
G- F	0 / 430	-38.5	-38.5	0.41 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	-106 / 22	0.05 (1)
G- C	0 / 456	0.10 (2)
G- D	-64 / 40	0.03 (1)
H- B	-724 / 0	0.32 (1)
D- F	-722 / 0	0.30 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.41/1.00 (G-H:2) , WB=0.32/1.00 (B-H:1) , SSI=0.17/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

DWG NO. TAM T1403378
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G20	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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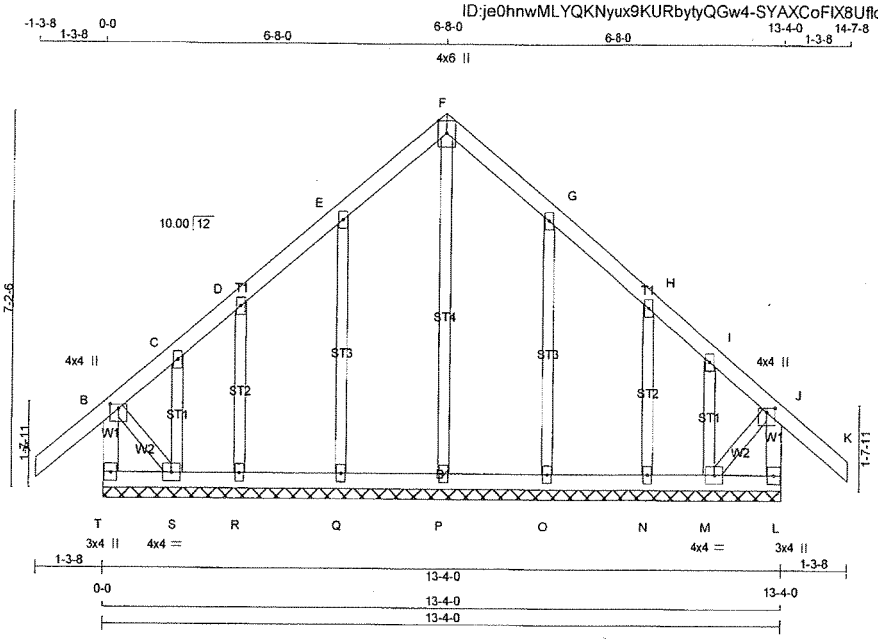


TOTAL WEIGHT = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
T- B	-246 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-58 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	-7 / 0	-78.0	-78.0	0.03 (1)	10.00
D- E	-9 / 0	-78.0	-78.0	0.05 (1)	10.00
E- F	-20 / 0	-78.0	-78.0	0.05 (1)	6.25
F- G	-20 / 0	-78.0	-78.0	0.05 (1)	6.25
G- H	-9 / 0	-78.0	-78.0	0.05 (1)	10.00
H- I	-7 / 0	-78.0	-78.0	0.03 (1)	10.00
I- J	-58 / 0	-78.0	-78.0	0.10 (1)	6.25
J- K	0 / 35	-78.0	-78.0	0.11 (1)	10.00
L- J	-246 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-38.5	-38.5	0.01 (3)	10.00
S- R	0 / 15	-38.5	-38.5	0.02 (2)	10.00
R- Q	0 / 11	-38.5	-38.5	0.03 (3)	10.00
Q- P	0 / 8	-38.5	-38.5	0.03 (3)	10.00
P- O	0 / 8	-38.5	-38.5	0.03 (3)	10.00
O- N	0 / 11	-38.5	-38.5	0.03 (3)	10.00
N- M	0 / 15	-38.5	-38.5	0.02 (2)	10.00
M- L	0 / 0	-38.5	-38.5	0.01 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
P- F	-118 / 0	0.10 (1)
Q- E	-181 / 0	0.08 (1)
R- D	-134 / 0	0.03 (1)
S- C	-22 / 0	0.00 (1)
O- G	-181 / 0	0.08 (1)
N- H	-134 / 0	0.03 (1)
M- I	-22 / 0	0.00 (1)
B- S	0 / 21	0.00 (1)
M- J	0 / 21	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (Q-R:3) , WB=0.10/1.00 (F-P:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

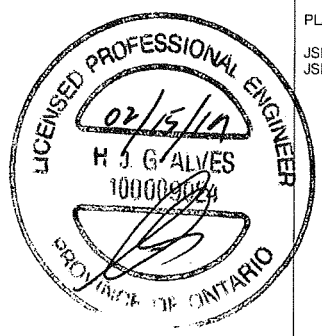
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)



BMS NO: TAM 7190343
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T21	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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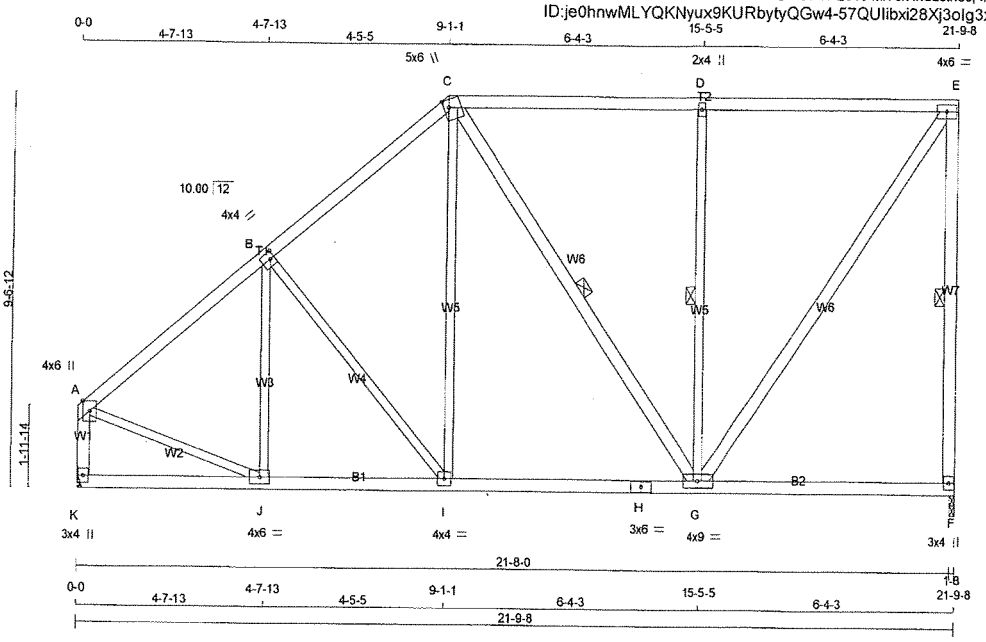


TOTAL WEIGHT = 118 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1269	0	1269	0	0	1-8	1-8
K	1269	0	1269	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	970	458 / 0	229 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

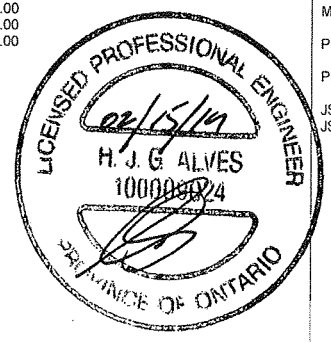
LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1129 / 0	-78.0	-78.0	0.31 (1)	5.60
B- C	-984 / 0	-78.0	-78.0	0.29 (1)	5.90
C- D	-677 / 0	-78.0	-78.0	0.56 (1)	6.20
D- E	-678 / 0	-78.0	-78.0	0.56 (1)	6.20
F- E	-1172 / 0	0.0	0.0	0.51 (1)	5.90
K- A	-1195 / 0	0.0	0.0	0.14 (1)	7.31
K- J	0 / 0	-38.5	-38.5	0.14 (3)	10.00
J- I	0 / 888	-38.5	-38.5	0.27 (2)	10.00
I- H	0 / 737	-38.5	-38.5	0.40 (2)	10.00
H- G	0 / 737	-38.5	-38.5	0.40 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.31 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-125 / 72	0.07 (1)
B- I	-244 / 0	0.24 (1)
I- C	0 / 414	0.09 (2)
C- G	-106 / 0	0.07 (1)
G- D	-614 / 0	0.37 (1)
G- E	0 / 1194	0.19 (1)
A- J	0 / 942	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.40/1.00 (G-I:2) , WB=0.37/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)

DWG NO. TAM T1903374
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G21	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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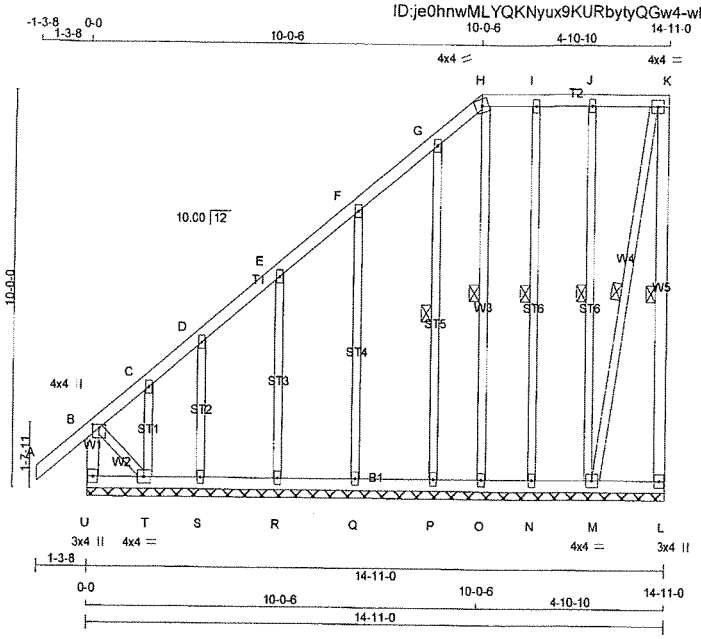


Scale = 1:55.8

TOTAL WEIGHT = 105 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
U - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW-m	MT20	4.0	4.0		
K	TMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF K-L, J-M, I-N, G-P, H-O, K-M.

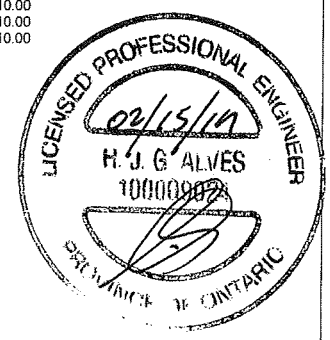
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U- B	-235 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-47 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	0 / 0	-78.0	-78.0	0.03 (1)	10.00
D- E	-5 / 0	-78.0	-78.0	0.04 (1)	10.00
E- F	-3 / 0	-78.0	-78.0	0.04 (1)	10.00
F- G	-3 / 0	-78.0	-78.0	0.04 (1)	10.00
G- H	-8 / 0	-78.0	-78.0	0.03 (1)	10.00
H- I	0 / 2	-78.0	-78.0	0.02 (1)	10.00
I- J	0 / 2	-78.0	-78.0	0.03 (1)	10.00
J- K	0 / 5	-78.0	-78.0	0.04 (1)	10.00
L- K	-41 / 0	0.0	0.0	0.02 (1)	6.25
U- T	0 / 0	-38.5	-38.5	0.02 (3)	10.00
T- S	0 / 9	-38.5	-38.5	0.02 (3)	10.00
S- R	0 / 5	-38.5	-38.5	0.02 (3)	10.00
R- Q	0 / 2	-38.5	-38.5	0.02 (3)	10.00
Q- P	0 / 0	-38.5	-38.5	0.02 (3)	10.00
P- O	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
O- N	-2 / 0	-38.5	-38.5	0.01 (3)	10.00
N- M	-2 / 0	-38.5	-38.5	0.02 (3)	10.00
M- L	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- J	-152 / 0	0.10 (1)
N- I	-118 / 0	0.08 (1)
P- G	-135 / 0	0.07 (1)
Q- F	-159 / 0	0.16 (1)
R- E	-156 / 0	0.08 (1)
S- D	-143 / 0	0.04 (1)
T- C	-39 / 0	0.01 (1)
O- H	-77 / 0	0.05 (1)
M- K	-27 / 0	0.02 (1)
B- T	0 / 13	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.02/1.00 (R-S:3) , WB=0.16/1.00 (F-Q:1) , SSI=0.07/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

DWG NO. TAM 719034351
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T22	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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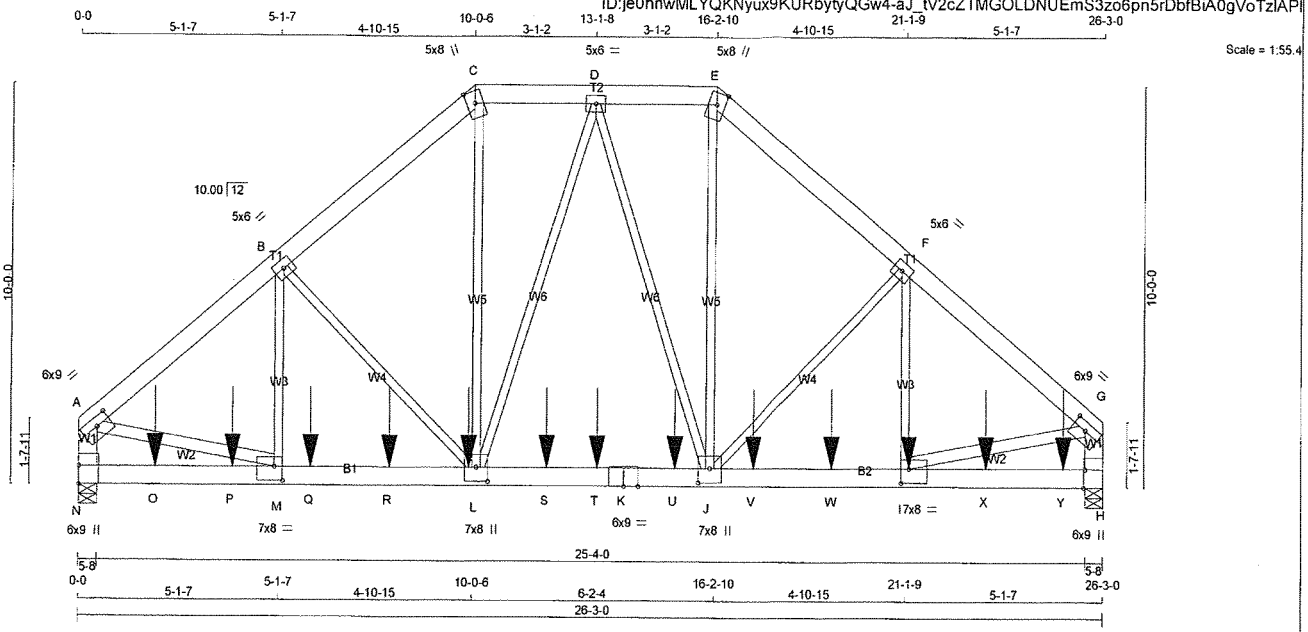


TOTAL WEIGHT = 2 X 170 = 340 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	2100F 1.8E	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- G	2	12	TOP
N- A	2	12	TOP
H- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(197.8)
K- H	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
L- C	1	6	SIDE(123.4)
F- I	1	6	SIDE(123.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.50	4.25
B, D, F						
B	TMWW-t	MT20	5.0	6.0		
C	TTW+m	MT20	5.0	8.0	Edge	
E	TTW+m	MT20	5.0	8.0	Edge	
G	TMVW-t	MT20	6.0	9.0	2.50	4.25
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW-t	MT20	7.0	8.0	4.25	2.50
J	BMWWW+t	MT20	7.0	8.0	4.25	3.50
K	BS-t	MT20	6.0	9.0		
L	BMWWW+t	MT20	7.0	8.0	4.25	3.50
M	BMWW-t	MT20	7.0	8.0	4.25	2.50
N	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	9263	0	9263	0	0	5-8	5-8
H	9798	0	9798	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	7042	3427 / 0	1581 / 0	0 / 0	0 / 0	2033 / 0	0 / 0
H	7448	3626 / 0	1672 / 0	0 / 0	0 / 0	2150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-10119 / 0	-78.0	-78.0	0.27 (1)	3.69	M- B	0 / 1652	0.20 (1)
B- C	-8722 / 0	-78.0	-78.0	0.22 (1)	3.99	B- L	-1608 / 0	0.82 (1)
C- D	-6735 / 0	-78.0	-78.0	0.24 (1)	4.38	L- C	0 / 5063	0.63 (1)
D- E	-6728 / 0	-78.0	-78.0	0.24 (1)	4.38	L- D	0 / 147	0.02 (2)
E- F	-8713 / 0	-78.0	-78.0	0.22 (1)	3.99	D- J	0 / 126	0.02 (2)
F- G	-10189 / 0	-78.0	-78.0	0.28 (1)	3.68	J- E	0 / 5057	0.63 (1)
N- A	-8368 / 0	0.0	0.0	0.30 (1)	5.20	J- F	-1697 / 0	0.87 (1)
H- G	-8424 / 0	0.0	0.0	0.30 (1)	5.19	I- F	0 / 1767	0.22 (1)
						A- M	0 / 7956	0.70 (1)
N- O	0 / 0	-38.5	-38.5	0.33 (1)	10.00	I- G	0 / 8010	0.71 (1)
O- P	0 / 0	-38.5	-38.5	0.33 (1)	10.00			
P- M	0 / 0	-38.5	-38.5	0.33 (1)	10.00			
M- Q	0 / 7774	-38.5	-38.5	0.55 (1)	10.00			
Q- R	0 / 7774	-38.5	-38.5	0.55 (1)	10.00			
R- L	0 / 7774	-38.5	-38.5	0.55 (1)	10.00			
L- S	0 / 6693	-38.5	-38.5	0.58 (1)	10.00			
S- T	0 / 6693	-38.5	-38.5	0.58 (1)	10.00			
T- K	0 / 6693	-38.5	-38.5	0.58 (1)	10.00			
K- U	0 / 6693	-38.5	-38.5	0.58 (1)	10.00			
U- J	0 / 6693	-38.5	-38.5	0.58 (1)	10.00			
J- V	0 / 7827	-38.5	-38.5	0.61 (1)	10.00			
V- W	0 / 7827	-38.5	-38.5	0.61 (1)	10.00			
W- I	0 / 7827	-38.5	-38.5	0.61 (1)	10.00			
I- X	0 / 0	-38.5	-38.5	0.30 (1)	10.00			
X- Y	0 / 0	-38.5	-38.5	0.30 (1)	10.00			
Y- H	0 / 0	-38.5	-38.5	0.30 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	21-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---
L	9-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
O	1-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
P	3-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
Q	5-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
R	7-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
S	11-11-4	-1231	-1231	---	BACK	VERT	TOTAL	---	---
T	13-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---
U	15-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---
V	17-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---
W	19-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/15/19
R. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.30/1.00 (G-H;1) , BC=0.61/1.00 (I-J;1) , WB=0.87/1.00 (F-J;1) , SSI=0.74/1.00 (J-L;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.99 (K) (INPUT = 1.00)

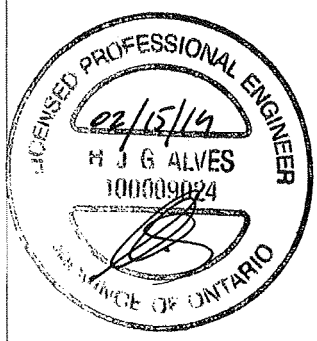
DWG NO. TAM T1903380
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BLOCK 1 FREEHOLDUNITS 1 - 4	DRWG NO.
400998	T22	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	23-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---
Y	25-2-12	-1231	-1231	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM T1903380
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G22	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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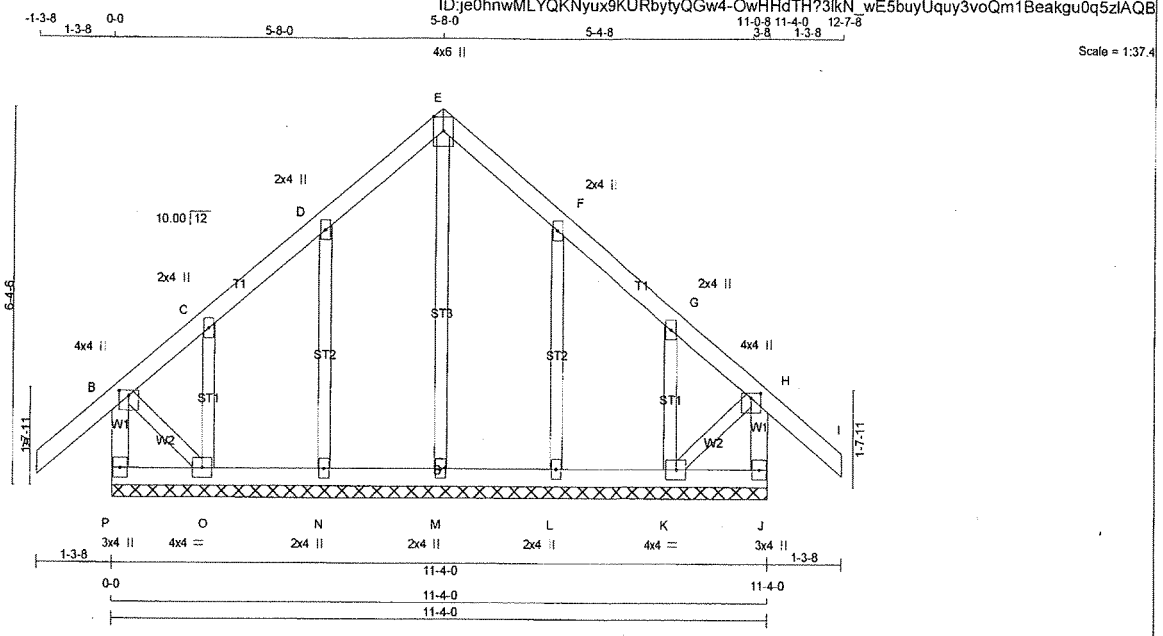


TOTAL WEIGHT = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

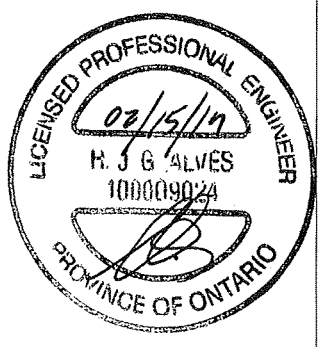
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-235 / 0	0.0	0.0	0.03	(1)	7.81	M- E	-116 / 0	0.07 (1)
A- B	0 / 35	-78.0	-78.0	0.11	(1)	10.00	N- D	-189 / 0	0.06 (1)
B- C	-46 / 0	-78.0	-78.0	0.10	(1)	6.25	O- C	-80 / 0	0.01 (1)
C- D	-5 / 0	-78.0	-78.0	0.05	(1)	10.00	L- F	-189 / 0	0.06 (1)
D- E	-21 / 0	-78.0	-78.0	0.05	(1)	6.25	K- G	-80 / 0	0.01 (1)
E- F	-21 / 0	-78.0	-78.0	0.05	(1)	6.25	B- O	0 / 18	0.00 (1)
F- G	-5 / 0	-78.0	-78.0	0.05	(1)	10.00	K- H	0 / 18	0.00 (1)
G- H	-46 / 0	-78.0	-78.0	0.10	(1)	6.25			
H- I	0 / 35	-78.0	-78.0	0.11	(1)	10.00			
J- H	-235 / 0	0.0	0.0	0.03	(1)	7.81			
P- O	0 / 0	-38.5	-38.5	0.02	(3)	10.00			
O- N	0 / 11	-38.5	-38.5	0.02	(3)	10.00			
N- M	0 / 7	-38.5	-38.5	0.02	(3)	10.00			
M- L	0 / 7	-38.5	-38.5	0.02	(3)	10.00			
L- K	0 / 11	-38.5	-38.5	0.02	(3)	10.00			
K- J	0 / 0	-38.5	-38.5	0.02	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.02/1.00 (N-O:3) , WB=0.07/1.00 (E-M:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.10 (F) (INPUT = 1.00)

BCIN NO. TAM 7903403
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T23	4	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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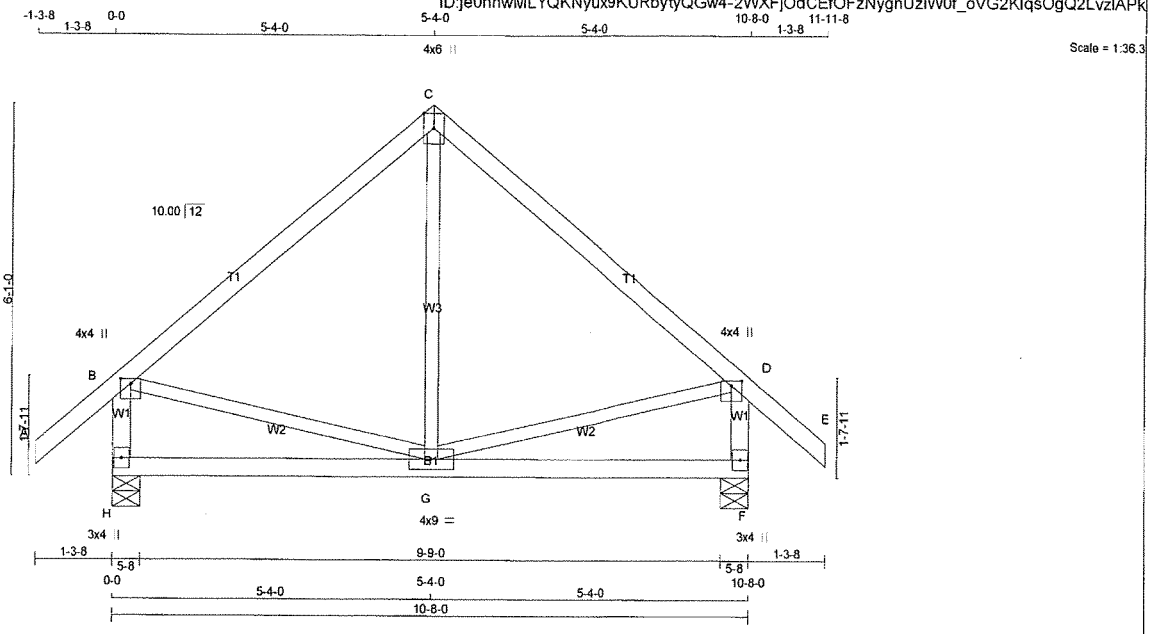


TOTAL WEIGHT = 4 X 48 = 191 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	729	0	729	0	0	5-8	5-8
F	729	0	729	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	550	282 / 0	112 / 0	0 / 0	0 / 0	155 / 0	0 / 0
F	550	282 / 0	112 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

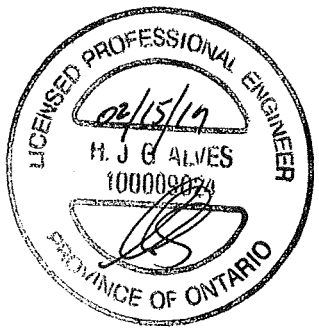
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	G- C	0 / 233	0.06 (3)
B- C	-405 / 0	-78.0	-78.0	0.29 (1)	6.25	B- G	0 / 320	0.07 (1)
C- D	-405 / 0	-78.0	-78.0	0.29 (1)	6.25	G- D	0 / 320	0.07 (1)
D- E	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
H- B	-651 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-651 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-38.5	-38.5	0.25 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.25 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.25/1.00 (F-G:3) , WB=0.07/1.00 (B-G:1) , SSI=0.14/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM T1203381
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G23	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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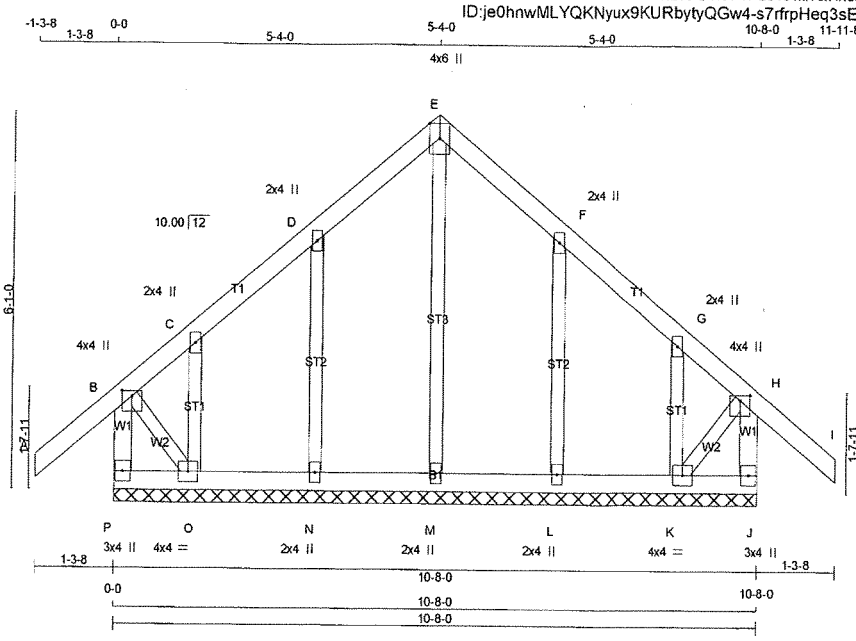


TOTAL WEIGHT = 52 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

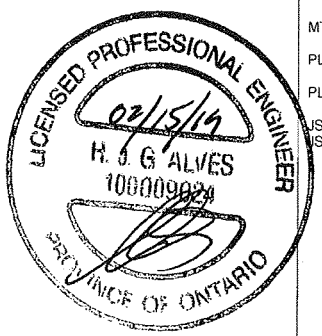
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-232 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-50 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	-2 / 0	-78.0	-78.0	0.05 (1)	10.00
D- E	-17 / 0	-78.0	-78.0	0.05 (1)	6.25
E- F	-17 / 0	-78.0	-78.0	0.05 (1)	6.25
F- G	-2 / 0	-78.0	-78.0	0.05 (1)	10.00
G- H	-50 / 0	-78.0	-78.0	0.10 (1)	6.25
H- I	0 / 35	-78.0	-78.0	0.11 (1)	10.00
J- H	-232 / 0	0.0	0.0	0.02 (1)	7.81
P- O	0 / 0	-38.5	-38.5	0.02 (3)	10.00
O- N	0 / 9	-38.5	-38.5	0.03 (3)	10.00
N- M	0 / 5	-38.5	-38.5	0.02 (3)	10.00
M- L	0 / 5	-38.5	-38.5	0.02 (3)	10.00
L- K	0 / 9	-38.5	-38.5	0.03 (3)	10.00
K- J	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-120 / 0	0.07 (1)
N- D	-189 / 0	0.05 (1)
O- C	-54 / 0	0.01 (1)
L- F	-189 / 0	0.05 (1)
K- G	-54 / 0	0.01 (1)
B- O	0 / 16	0.00 (1)
K- H	0 / 16	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (N-O:3) , WB=0.07/1.00 (E-M:1) , SSI=0.06/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.17 (B) (INPUT = 0.90)
SI METAL= 0.10 (D) (INPUT = 1.00)

DESIGNER TAM 7903404
STRUCTURAL
CERTIFICATE NUMBER

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T24	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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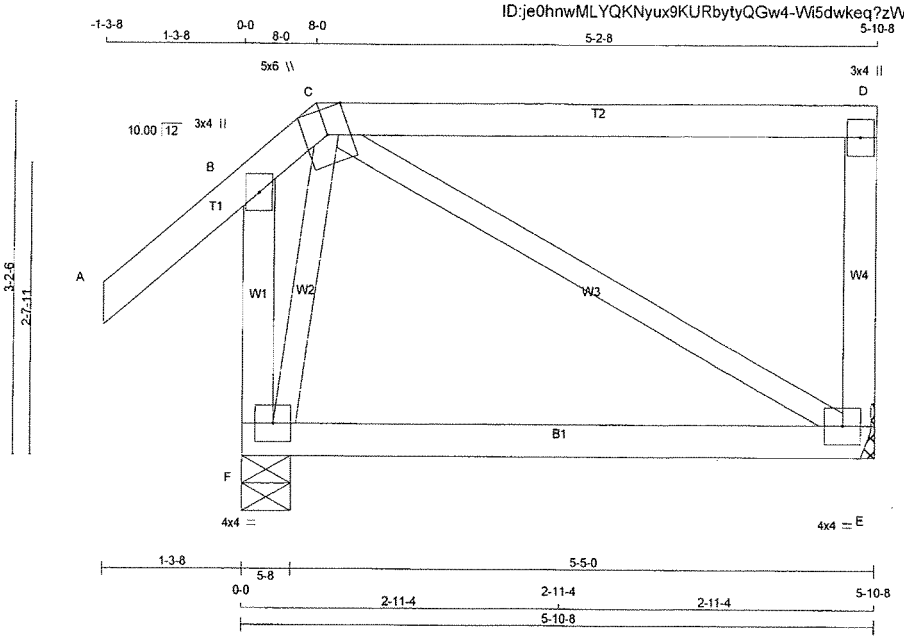


TOTAL WEIGHT = 28 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	330	0	330	0	0	MECHANICAL	
F	463	0	463	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	253	117 / 0	62 / 0	0 / 0	0 / 0	74 / 0	0 / 0
F	345	188 / 0	62 / 0	0 / 0	0 / 0	95 / 0	0 / 0

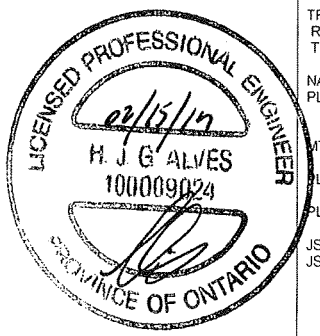
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	F- C	-113 / 0	0.02 (1)
B- C	-67 / 0	-78.0	-78.0	0.10 (1)	6.25	C- E	-31 / 0	0.02 (1)
C- D	0 / 0	-78.0	-78.0	0.35 (1)	10.00			
E- D	-201 / 0	0.0	0.0	0.03 (1)	7.81			
F- B	-240 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	0 / 27	-38.5	-38.5	0.31 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.31/1.00 (E-F:3) , WB=0.02/1.00 (C-F:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

DWG NO. TAM T1903362
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T25	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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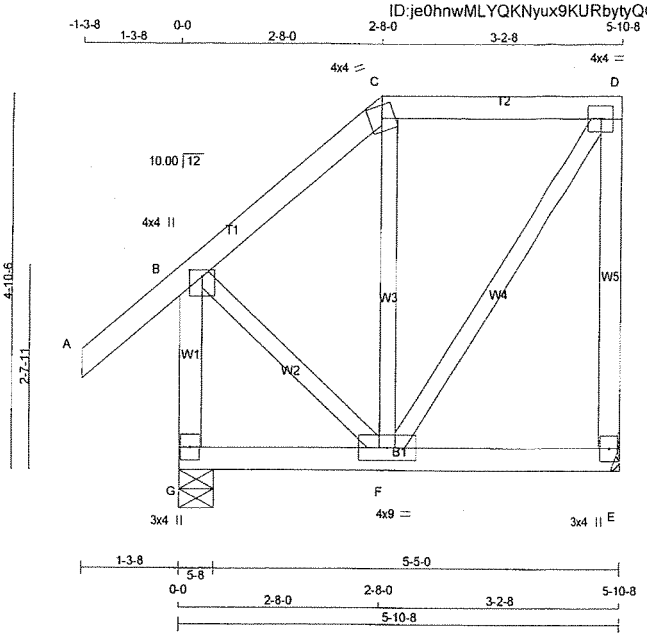

Scale = 1:28.8

TOTAL WEIGHT = 34 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	342	0	342	0	0	MECHANICAL	
G	450	0	450	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	261	123 / 0	62 / 0	0 / 0	0 / 0	76 / 0	0 / 0
G	336	182 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)		10.00	F- C	-131 / 15	0.05 (1)
B- C	-154 / 0	-78.0	-78.0	0.09 (1)		6.25	F- D	0 / 205	0.05 (1)
C- D	-115 / 0	-78.0	-78.0	0.14 (1)		6.25	B- F	0 / 154	0.03 (1)
E- D	-294 / 0	0.0	0.0	0.11 (1)		7.81			
G- B	-410 / 0	0.0	0.0	0.06 (1)		7.81			
G- F	0 / 0	-38.5	-38.5	0.07 (3)		10.00			
F- E	0 / 0	-38.5	-38.5	0.07 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (C-D:1) , BC=0.07/1.00 (E-F:3) , WB=0.05/1.00 (D-F:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

DWG NO. TAM 7403383
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T26	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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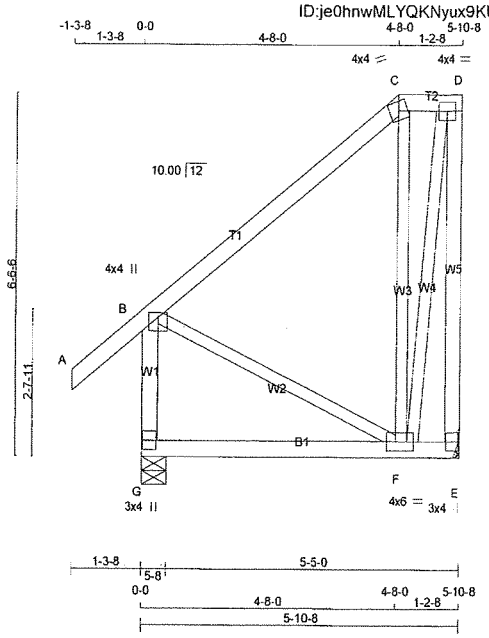


Scale = 1:39.9

TOTAL WEIGHT = 40 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	342	0	342	0	0	MECHANICAL	
G	450	0	450	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	261	123 / 0	62 / 0	0 / 0	0 / 0	76 / 0	0 / 0
G	336	182 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

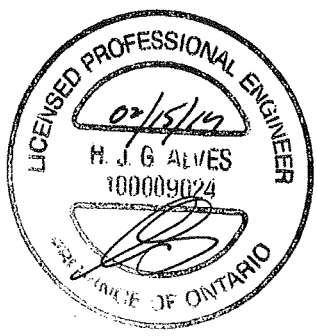
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-82 / 0	-78.0	-78.0	0.29 (1)	6.25
C- D	-60 / 0	-78.0	-78.0	0.02 (1)	6.25
E- D	-386 / 0	0.0	0.0	0.34 (1)	7.81
G- B	-373 / 0	0.0	0.0	0.05 (1)	7.81
G- F	0 / 0	-38.5	-38.5	0.15 (3)	10.00
F- E	0 / 0	-38.5	-38.5	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-177 / 0	0.12 (1)
B- F	0 / 70	0.02 (1)
F- D	0 / 345	0.08 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.34/1.00 (D-E:1) , BC=0.15/1.00 (F-G:3) , WB=0.12/1.00 (C-F:1) , SSI=0.12/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (F) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)

DWG NO. TAM7740338A
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BLOCK 1 FREEHOLDUNITS 1 - 4	DRWG NO.
400998	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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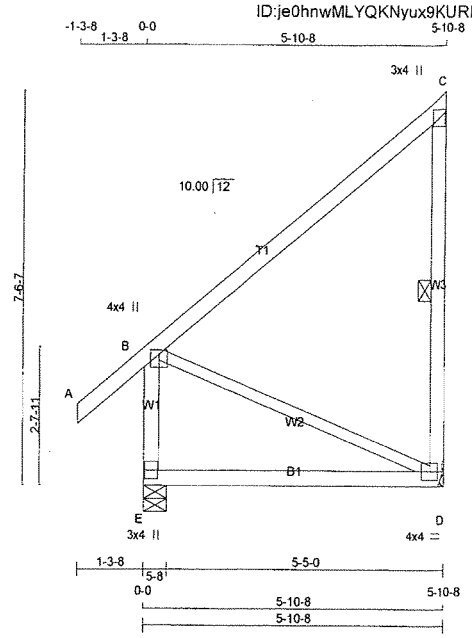


Scale = 1:42.4

TOTAL WEIGHT = 33 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	342	0	342	0	0	MECHANICAL	
E	450	0	450	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	261	123 / 0	62 / 0	0 / 0	0 / 0	76 / 0	0 / 0
E	336	182 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

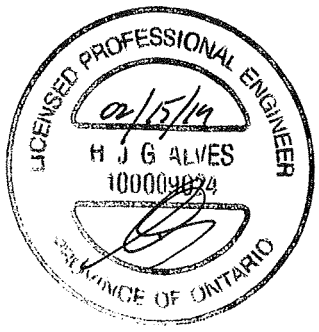
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11	(1)	10.00	B- D	0 / 0	0.00 (1)
B- C	0 / 0	-78.0	-78.0	0.46	(1)	10.00			
D- C	-229 / 0	0.0	0.0	0.06	(1)	6.25			
E- B	-337 / 0	0.0	0.0	0.05	(1)	7.81			
E- D	0 / 0	-38.5	-38.5	0.31	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.31/1.00 (D-E:3) , WB=0.00/1.00 (B-D:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 .

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

DWG NO. TAM T2403385
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T28	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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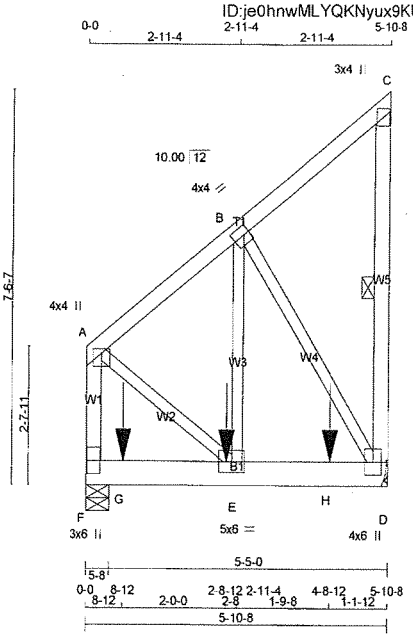

TOTAL WEIGHT = 2 X 40 = 80 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1330	0	1330	0	0	MECHANICAL	
F	1432	0	1432	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	997	527 / 0	192 / 0	0 / 0	0 / 0	279 / 0	0 / 0
F	1074	567 / 0	207 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-665 / 0	-78.0	-78.0	0.06 (1)	6.25	E- B	0 / 988	0.12 (1)	
B- C	-16 / 0	-78.0	-78.0	0.06 (1)	6.25	B- D	-984 / 0	0.24 (1)	
D- C	-89 / 0	0.0	0.0	0.01 (1)	6.25	A- E	0 / 657	0.08 (1)	
F- A	-921 / 0	0.0	0.0	0.07 (1)	7.81				
F- G	0 / 0	-38.5	-38.5	0.13 (1)	10.00				
G- E	0 / 0	-38.5	-38.5	0.13 (1)	10.00				
E- H	0 / 524	-38.5	-38.5	0.20 (1)	10.00				
H- D	0 / 524	-38.5	-38.5	0.20 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-8-12	-691	-691	---	BACK	VERT	TOTAL	---	---
G	8-12	-696	-696	---	BACK	VERT	TOTAL	---	---
H	4-8-12	-691	-691	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.07/1.00 (A-F:1) , BC=0.20/1.00 (D-E:1) , WB=0.24/1.00 (B-D:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (B) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

DWG NO. TAM T1903386
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T30	2	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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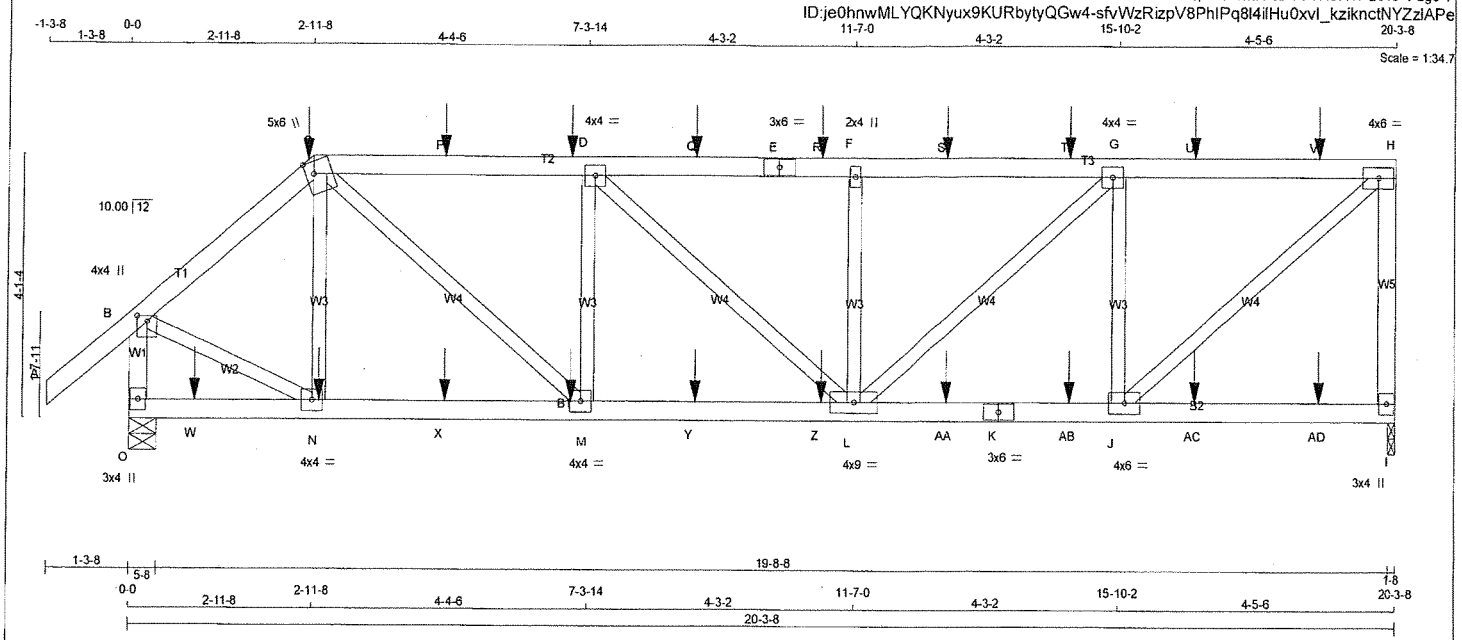

TOTAL WEIGHT = 4 X 86 = 345 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	1	12	SIDE(61.0)
K- I	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1918	0	1918	0	0	1-8	1-8
O	1956	0	1956	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1472	675 / 0	362 / 0	0 / 0	0 / 0	435 / 0	0 / 0
O	1499	691 / 0	364 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 35	-78.0	-78.0	0.06 (1)	10.00
B- C	-1715 / 0	-78.0	-78.0	0.08 (1)	6.25
C- P	-2450 / 0	-78.0	-78.0	0.19 (1)	5.56
P- D	-2450 / 0	-78.0	-78.0	0.19 (1)	5.56
D- Q	-2579 / 0	-78.0	-78.0	0.19 (1)	5.44
Q- E	-2579 / 0	-78.0	-78.0	0.19 (1)	5.44
E- R	-2579 / 0	-78.0	-78.0	0.19 (1)	5.44
R- F	-2579 / 0	-78.0	-78.0	0.19 (1)	5.44
F- S	-2579 / 0	-78.0	-78.0	0.22 (1)	5.41
S- T	-2579 / 0	-78.0	-78.0	0.22 (1)	5.41
T- G	-2579 / 0	-78.0	-78.0	0.22 (1)	5.41
G- U	-1859 / 0	-78.0	-78.0	0.20 (1)	6.13
U- V	-1859 / 0	-78.0	-78.0	0.20 (1)	6.13
V- H	-1859 / 0	-78.0	-78.0	0.20 (1)	6.13
I- H	-1805 / 0	0.0	0.0	0.24 (1)	7.81
O- B	-1887 / 0	0.0	0.0	0.11 (1)	7.81
O- W	0 / 0	-38.5	-38.5	0.08 (3)	10.00
W- N	0 / 0	-38.5	-38.5	0.08 (3)	10.00
N- X	0 / 1305	-38.5	-38.5	0.19 (2)	10.00
X- M	0 / 1305	-38.5	-38.5	0.19 (2)	10.00
M- Y	0 / 2450	-38.5	-38.5	0.28 (2)	10.00
Y- Z	0 / 2450	-38.5	-38.5	0.28 (2)	10.00
Z- L	0 / 2450	-38.5	-38.5	0.28 (2)	10.00
L-AA	0 / 1859	-38.5	-38.5	0.27 (2)	10.00
AA- K	0 / 1859	-38.5	-38.5	0.27 (2)	10.00
K-AB	0 / 1859	-38.5	-38.5	0.27 (2)	10.00
AB- J	0 / 1859	-38.5	-38.5	0.27 (2)	10.00
J-AC	0 / 0	-38.5	-38.5	0.13 (3)	10.00
AC-AD	0 / 0	-38.5	-38.5	0.13 (3)	10.00
AD- I	0 / 0	-38.5	-38.5	0.13 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- C	-289 / 14	0.04 (1)
B- N	0 / 1427	0.18 (1)
J- H	0 / 2448	0.30 (1)
C- M	0 / 1516	0.19 (1)
J- G	-1267 / 0	0.17 (1)
M- D	-709 / 0	0.10 (1)
L- G	0 / 967	0.12 (1)
D- L	0 / 174	0.02 (1)
L- F	-493 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-101	-101	---	FRONT	VERT	TOTAL	---	---
D	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
M	7-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
N	3-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
P	5-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.24/1.00 (H-I:1) , BC=0.28/1.00 (L-M:2) , WB=0.30/1.00 (H-J:1) , SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM T1403387
STRUCTURAL
CONSTRUCTION ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BLOCK 1 FREEHOLDUNITS 1 - 4	DRWG NO.
400998	T30	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	11-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
S	13-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
T	15-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
U	17-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
V	19-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
W	1-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	5-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	9-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	11-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	13-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	15-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC	17-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AD	19-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. GALVES
100009624
PROVINCE OF ONTARIO

DWG NO. TAM T1903387
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T31	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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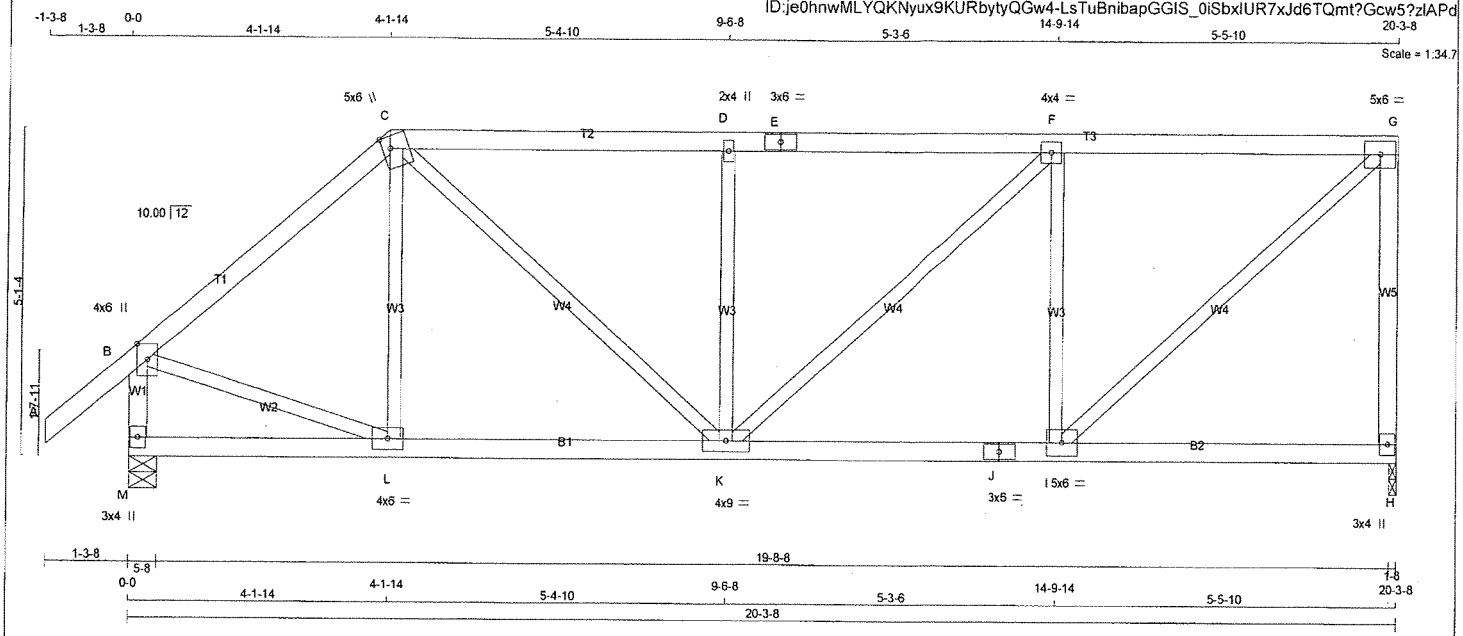


TOTAL WEIGHT = 2 X 87 = 175 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1182	0	1182	0	0	1-8	1-8
M	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
M	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

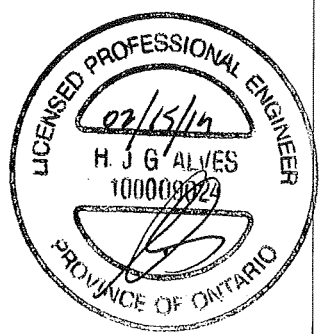
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	L- C	-49 / 118	0.03 (3)	
B- C	-1093 / 0	-78.0	-78.0	0.26 (1)	5.73	B- L	0 / 876	0.20 (1)	
C- D	-1300 / 0	-78.0	-78.0	0.33 (1)	5.26	I- G	0 / 1399	0.31 (1)	
D- E	-1300 / 0	-78.0	-78.0	0.35 (1)	5.24	C- K	0 / 621	0.14 (1)	
E- F	-1300 / 0	-78.0	-78.0	0.35 (1)	5.24	I- F	-689 / 0	0.26 (1)	
F- G	-1050 / 0	-78.0	-78.0	0.33 (1)	5.69	K- D	-448 / 0	0.17 (1)	
H- G	-1097 / 0	0.0	0.0	0.48 (1)	7.55	K- F	0 / 338	0.08 (1)	
M- B	-1229 / 0	0.0	0.0	0.13 (1)	7.23				
M- L	0 / 0	-38.5	-38.5	0.16 (3)	10.00				
L- K	0 / 836	-38.5	-38.5	0.28 (2)	10.00				
K- J	0 / 1050	-38.5	-38.5	0.35 (2)	10.00				
J- I	0 / 1050	-38.5	-38.5	0.35 (2)	10.00				
I- H	0 / 0	-38.5	-38.5	0.21 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.48/1.00 (G-H:1) , BC=0.35/1.00 (I-K:2) , WB=0.31/1.00 (G-I:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T1903389
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T32	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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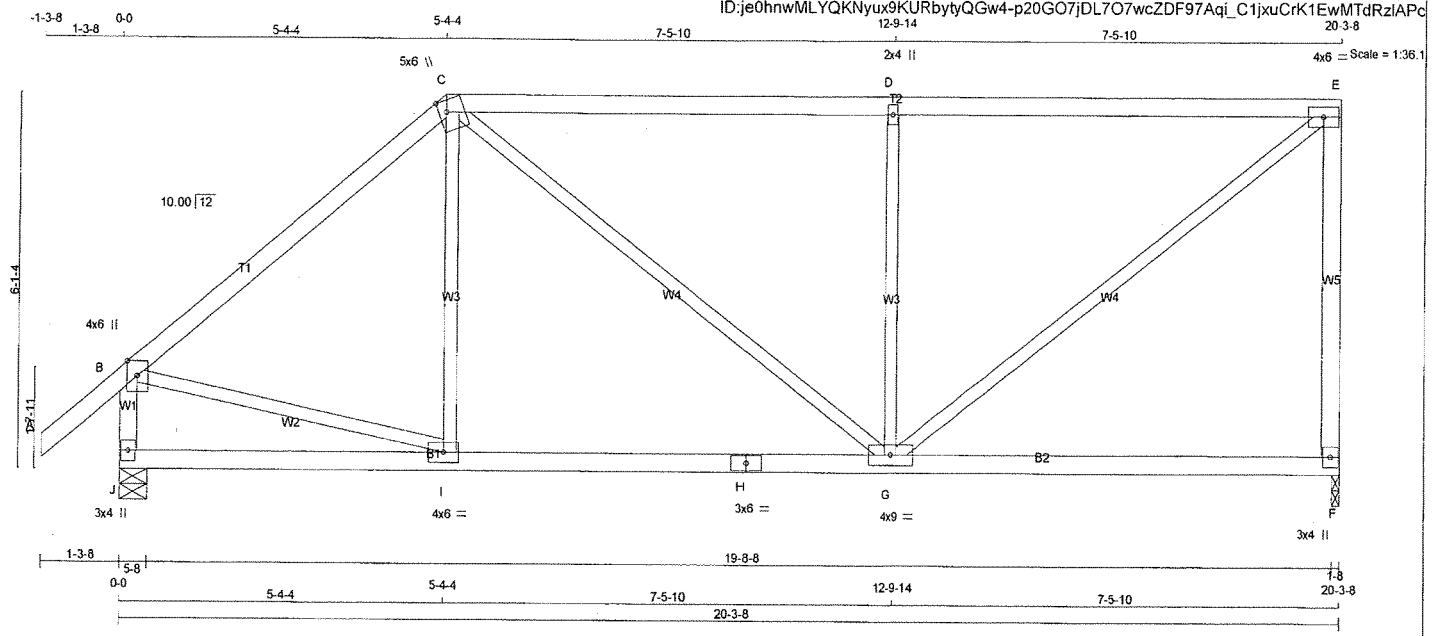

TOTAL WEIGHT = 2 X 86 = 172 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1182	0	1182	0	0	1-8	1-8
J	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	I- C	0 / 219	0.05 (3)
B- C	-1083 / 0	-78.0	-78.0	0.45 (1)	5.48	C- G	0 / 329	0.07 (1)
C- D	-1091 / 0	-78.0	-78.0	0.84 (1)	4.54	G- D	-721 / 0	0.42 (1)
D- E	-1091 / 0	-78.0	-78.0	0.85 (1)	4.54	G- E	0 / 1379	0.31 (1)
F- E	-1067 / 0	0.0	0.0	0.77 (1)	7.63	B- I	0 / 854	0.19 (1)
J- B	-1211 / 0	0.0	0.0	0.13 (1)	7.28			
J- I	0 / 0	-38.5	-38.5	0.26 (3)	10.00			
I- H	0 / 832	-38.5	-38.5	0.51 (2)	10.00			
H- G	0 / 832	-38.5	-38.5	0.51 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.41 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.85/1.00 (D-E:1) , BC=0.51/1.00 (G-I:2) , WB=0.42/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. T400390
STRUCTURAL
CROSS CHECK ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T33	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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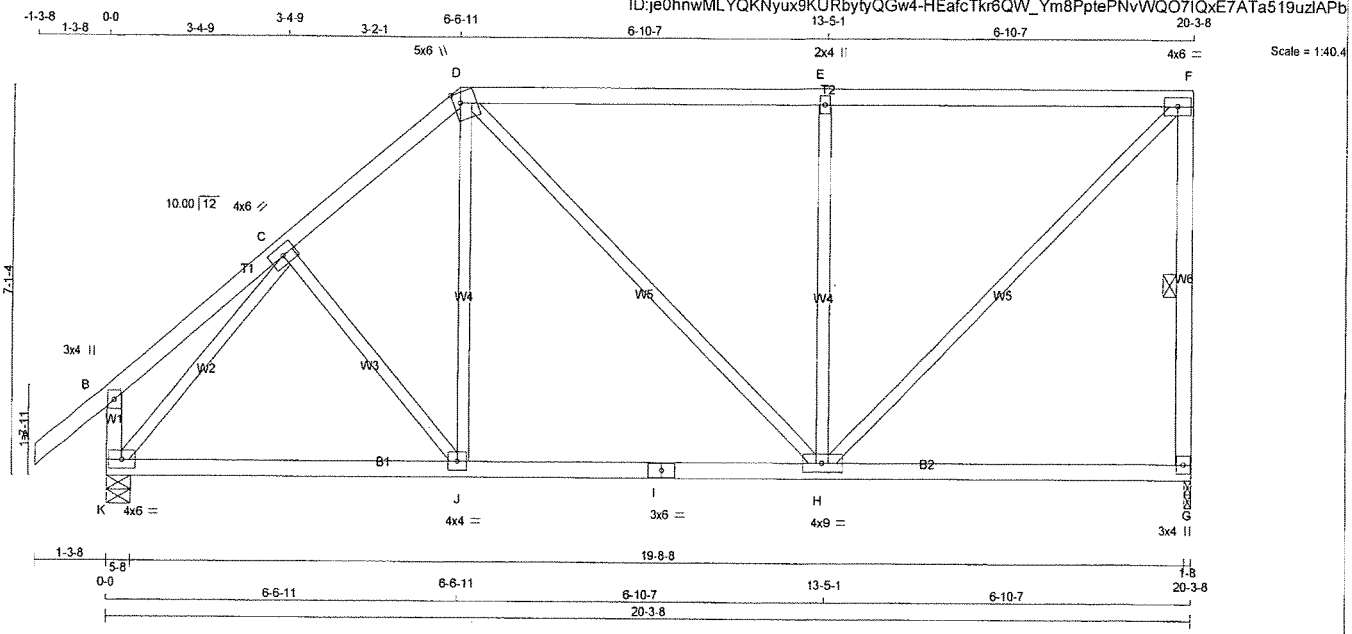


TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1182	0	1182	0	0	1-8	1-8
K	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
K	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

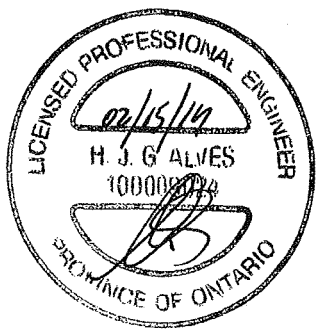
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	
B- C	0 / 17	-78.0	-78.0	0.12 (1)	10.00	
C- D	-1055 / 0	-78.0	-78.0	0.14 (1)	5.97	
D- E	-882 / 0	-78.0	-78.0	0.68 (1)	5.36	
E- F	-882 / 0	-78.0	-78.0	0.68 (1)	5.36	
G- F	-1074 / 0	0.0	0.0	0.24 (1)	6.09	
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81	
K- J	0 / 793	-38.5	-38.5	0.40 (2)	10.00	
J- I	0 / 799	-38.5	-38.5	0.42 (2)	10.00	
I- H	0 / 799	-38.5	-38.5	0.42 (2)	10.00	
H- G	0 / 0	-38.5	-38.5	0.33 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 79	0.02 (3)
J- D	0 / 326	0.07 (2)
D- H	0 / 117	0.03 (1)
H- E	-664 / 0	0.58 (1)
H- F	0 / 1239	0.28 (1)
K- C	-1260 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.42/1.00 (H-J:2) , WB=0.58/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

DWG NO. TAM 77903391
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T34	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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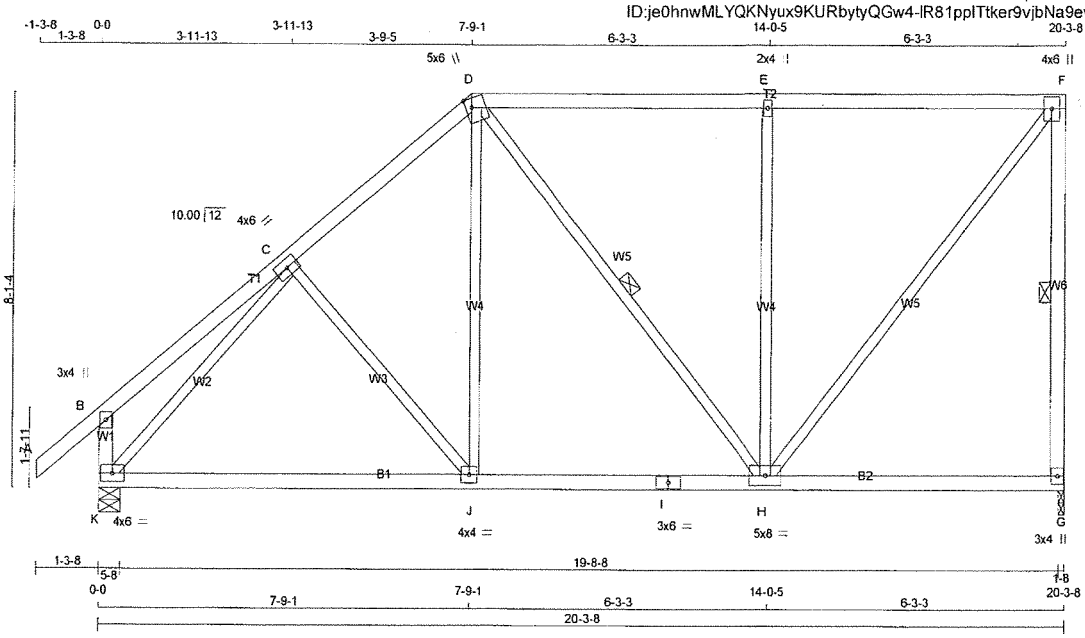

TOTAL WEIGHT = 2 X 98 = 196 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1182	0	1182	0	0	1-8	1-8
K	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
K	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	C- J	-79 / 49	0.05 (1)
B- C	0 / 22	-78.0	-78.0	0.18 (1)	10.00	J- D	0 / 413	0.09 (2)
C- D	-1002 / 0	-78.0	-78.0	0.20 (1)	5.99	D- H	-72 / 0	0.05 (3)
D- E	-721 / 0	-78.0	-78.0	0.54 (1)	6.10	H- E	-605 / 0	0.77 (1)
E- F	-722 / 0	-78.0	-78.0	0.54 (1)	6.10	H- F	0 / 1148	0.26 (1)
G- F	-1081 / 0	0.0	0.0	0.32 (1)	6.08	K- C	-1238 / 0	0.73 (1)
K- B	-224 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 803	-38.5	-38.5	0.48 (2)	10.00			
J- I	0 / 756	-38.5	-38.5	0.48 (2)	10.00			
I- H	0 / 756	-38.5	-38.5	0.48 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 960 (0.25")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.48/1.00 (J-K:2) , WB=0.77/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

DWG NO. TAM T1903392
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BLOCK 1 FREEHOLDUNITS 1 - 4	DRWG NO.
400998	T35	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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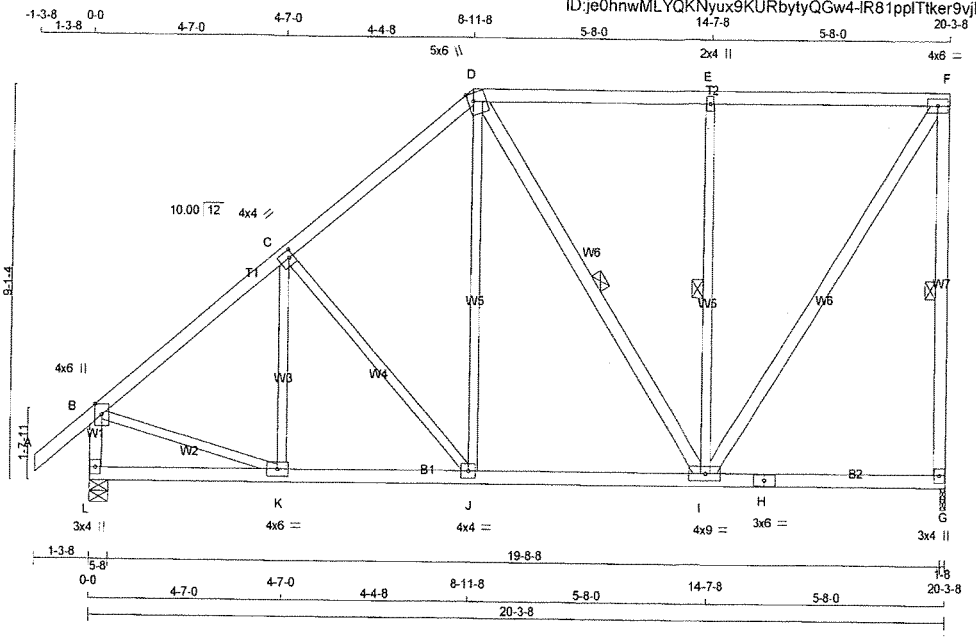

TOTAL WEIGHT = 4 X 112 = 449 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1182	0	1182	0	0	1-8	1-8
L	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
L	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	-1105 / 0	-78.0	-78.0	0.29 (1)	5.66
C- D	-911 / 0	-78.0	-78.0	0.28 (1)	6.09
D- E	-599 / 0	-78.0	-78.0	0.43 (1)	6.25
E- F	-599 / 0	-78.0	-78.0	0.43 (1)	6.25
G- F	-1095 / 0	0.0	0.0	0.42 (1)	6.04
L- B	-1217 / 0	0.0	0.0	0.13 (1)	7.26
L- K	0 / 0	-38.5	-38.5	0.14 (3)	10.00
K- J	0 / 869	-38.5	-38.5	0.25 (2)	10.00
J- I	0 / 682	-38.5	-38.5	0.33 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.24 (3)	10.00
H- G	0 / 0	-38.5	-38.5	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-57 / 111	0.03 (3)
C- J	-291 / 0	0.25 (1)
J- D	0 / 424	0.10 (2)
D- I	-153 / 0	0.09 (1)
I- E	-547 / 0	0.30 (1)
I- F	0 / 1102	0.18 (1)
B- K	0 / 903	0.20 (1)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.33/1.00 (I-J:2) , WB=0.30/1.00 (E-I:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1903393
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T36	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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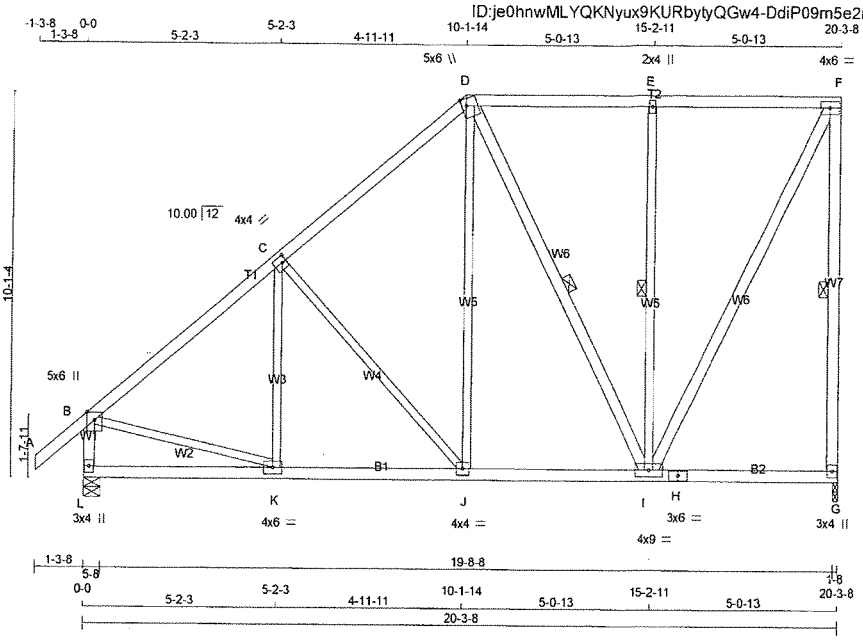


TOTAL WEIGHT = 2 X 118 = 236 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1182	0	1182	0	0	1-8	1-8
L	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
L	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

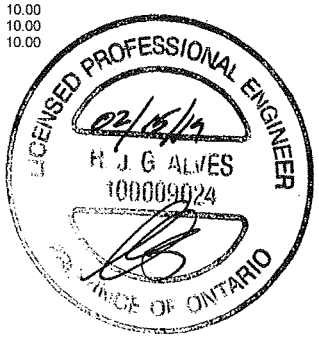
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	K- C	0 / 167	0.04 (3)
B- C	-1105 / 0	-78.0	-78.0	0.39 (1)	5.53	C- J	-383 / 0	0.44 (1)
C- D	-834 / 0	-78.0	-78.0	0.37 (1)	6.16	J- D	0 / 482	0.11 (2)
D- E	-495 / 0	-78.0	-78.0	0.34 (1)	6.25	D- I	-274 / 0	0.18 (1)
E- F	-495 / 0	-78.0	-78.0	0.34 (1)	6.25	I- E	-488 / 0	0.34 (1)
G- F	-1103 / 0	0.0	0.0	0.55 (1)	6.03	I- F	0 / 1072	0.17 (1)
L- B	-1210 / 0	0.0	0.0	0.13 (1)	7.28	B- K	0 / 899	0.20 (1)
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
K- J	0 / 872	-38.5	-38.5	0.31 (2)	10.00			
J- I	0 / 620	-38.5	-38.5	0.27 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
H- G	0 / 0	-38.5	-38.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.31/1.00 (J-K:2) , WB=0.44/1.00 (C-J:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO: TAM T1903394
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T38	2	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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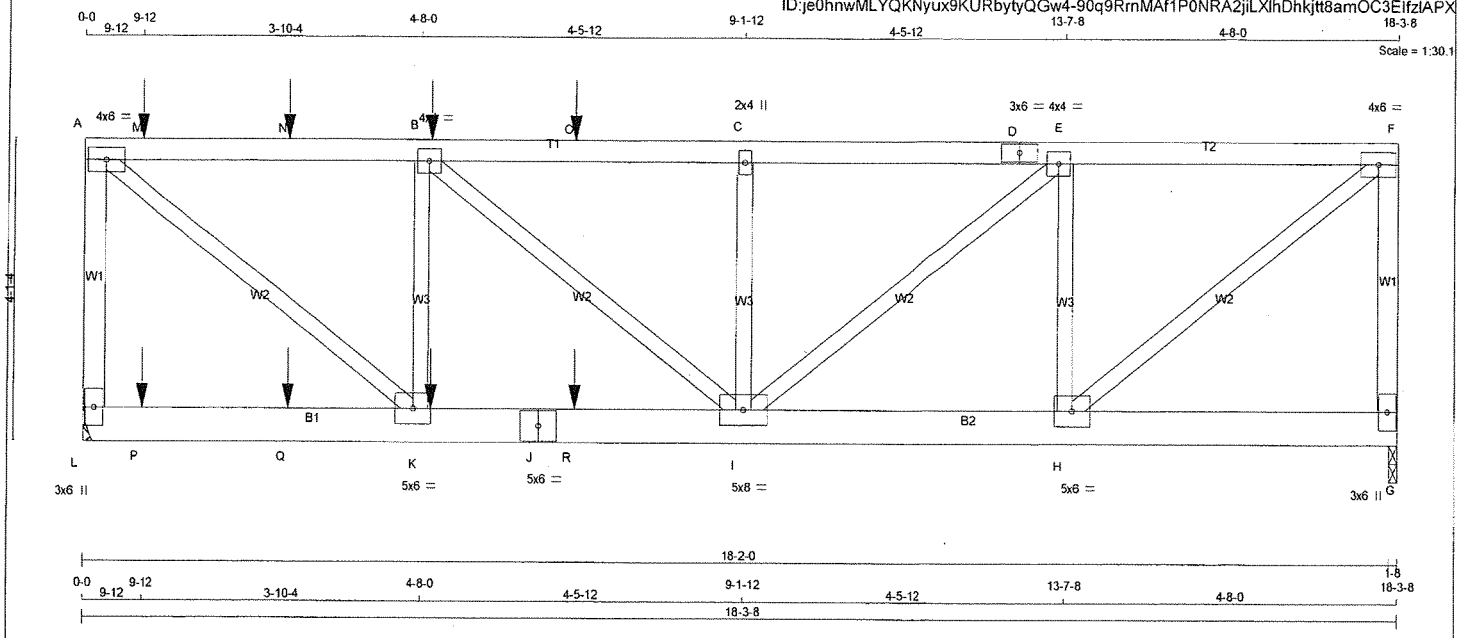

TOTAL WEIGHT = 4 X 87 = 349 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L- A	1	12	TOP
A- D	1	12	SIDE(61.0)
D- F	1	12	TOP
F- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	2	12	SIDE(183.1)
J- G	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1548	0	1548	0	0	MECHANICAL	
G	1189	0	1189	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1187	548 / 0	289 / 0	0 / 0	0 / 0	349 / 0	0 / 0
G	909	426 / 0	217 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- A	-1407 / 0	0.0	0.0	0.18 (1)	7.81	H- F	0 / 1543	0.19 (1)
A- M	-1452 / 0	-78.0	-78.0	0.21 (1)	6.25	A- K	0 / 1859	0.23 (1)
M- N	-1452 / 0	-78.0	-78.0	0.21 (1)	6.25	H- E	-767 / 0	0.10 (1)
N- B	-1452 / 0	-78.0	-78.0	0.21 (1)	6.25	K- B	-821 / 0	0.10 (1)
B- O	-1642 / 0	-78.0	-78.0	0.21 (1)	6.25	I- E	0 / 568	0.07 (1)
O- C	-1642 / 0	-78.0	-78.0	0.21 (1)	6.25	B- I	0 / 1	0.00 (1)
C- D	-1642 / 0	-78.0	-78.0	0.14 (1)	6.25	I- G	909 / 0	0.04 (1)
D- E	-1642 / 0	-78.0	-78.0	0.14 (1)	6.25			
E- F	-1205 / 0	-78.0	-78.0	0.13 (1)	6.25			
G- F	-1110 / 0	0.0	0.0	0.14 (1)	7.81			
L- P	0 / 0	-38.5	-38.5	0.06 (2)	10.00			
P- Q	0 / 0	-38.5	-38.5	0.06 (2)	10.00			
Q- K	0 / 0	-38.5	-38.5	0.06 (2)	10.00			
K- J	0 / 1452	-38.5	-38.5	0.13 (2)	10.00			
J- R	0 / 1452	-38.5	-38.5	0.13 (2)	10.00			
R- I	0 / 1452	-38.5	-38.5	0.13 (2)	10.00			
I- H	0 / 1205	-38.5	-38.5	0.10 (1)	10.00			
H- G	0 / 0	-38.5	-38.5	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	WHEEL	CONN.
B	4-9-12	-94	-94	---	BACK	VERT	TOTAL	---	---
K	4-9-12	-55	-70	---	BACK	VERT	TOTAL	---	---
M	9-12	-101	-101	---	BACK	VERT	TOTAL	---	---
N	2-9-12	-94	-94	---	BACK	VERT	TOTAL	---	---
O	6-9-12	-94	-94	---	BACK	VERT	TOTAL	---	---
P	9-12	-59	-75	---	BACK	VERT	TOTAL	---	---
Q	2-9-12	-55	-70	---	BACK	VERT	TOTAL	---	---
R	6-9-12	-55	-70	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.13/1.00 (I-K:2) , WB=0.23/1.00 (A-K:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1903398
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T39	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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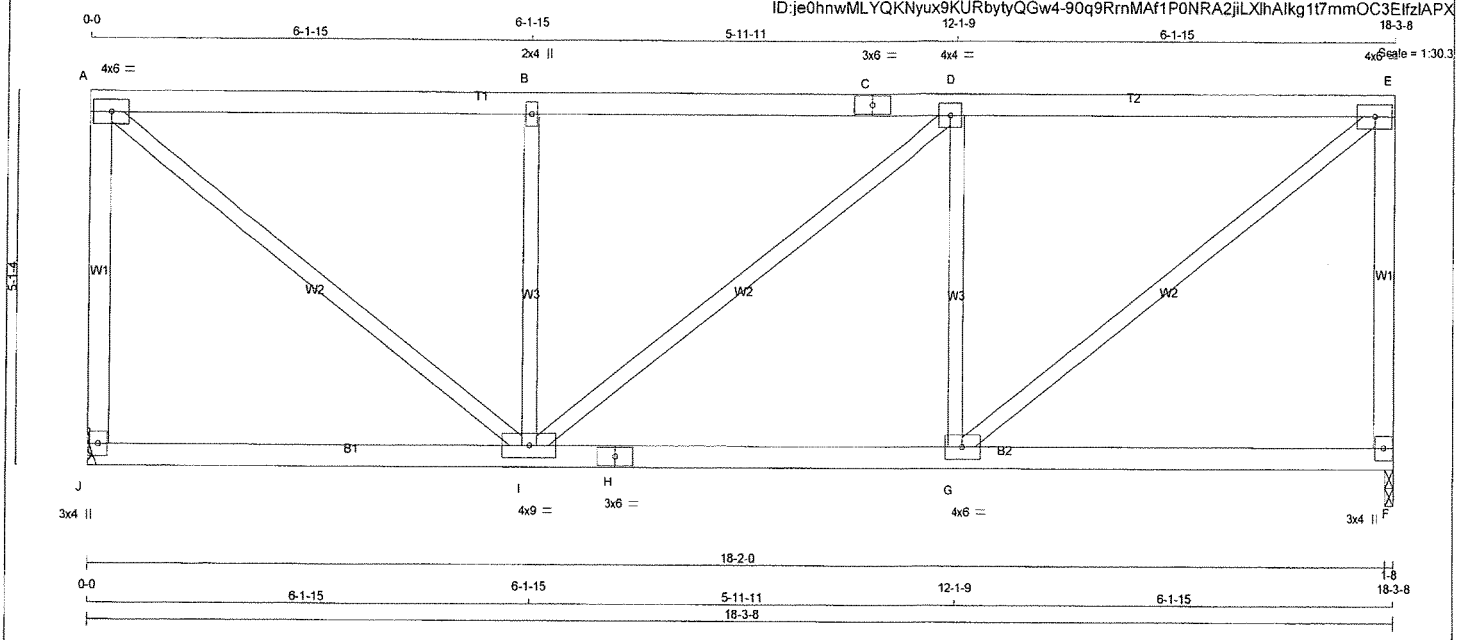


TOTAL WEIGHT = 2 X 77 = 155 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW-t	MT20	4.0	6.0	
H	BS-t	MT20	3.0	6.0	
I	BMWWW-t	MT20	4.0	9.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1066	0	1066	0	0	MECHANICAL	
F	1066	0	1066	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
F	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

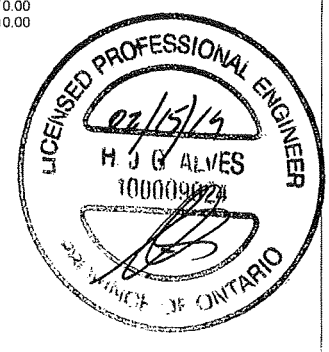
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- A	-968 / 0	0.0	0.0	0.43 (1)	7.81	G- E	0 / 1257	0.28 (1)
A- B	-990 / 0	-78.0	-78.0	0.42 (1)	5.68	A- I	0 / 1256	0.28 (1)
B- C	-990 / 0	-78.0	-78.0	0.43 (1)	5.68	G- D	-518 / 0	0.20 (1)
C- D	-990 / 0	-78.0	-78.0	0.43 (1)	5.68	I- B	-518 / 0	0.20 (1)
D- E	-991 / 0	-78.0	-78.0	0.43 (1)	5.67	I- D	-1 / 0	0.00 (1)
F- E	-969 / 0	0.0	0.0	0.43 (1)	7.81			
J- I	0 / 0	-38.5	-38.5	0.25 (3)	10.00			
I- H	0 / 991	-38.5	-38.5	0.38 (2)	10.00			
H- G	0 / 991	-38.5	-38.5	0.38 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.25 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.38/1.00 (G-I:2) , WB=0.28/1.00 (E-G:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

DWG NO. TAM T1903396
STRUCTURAL
CONNECTORS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T40	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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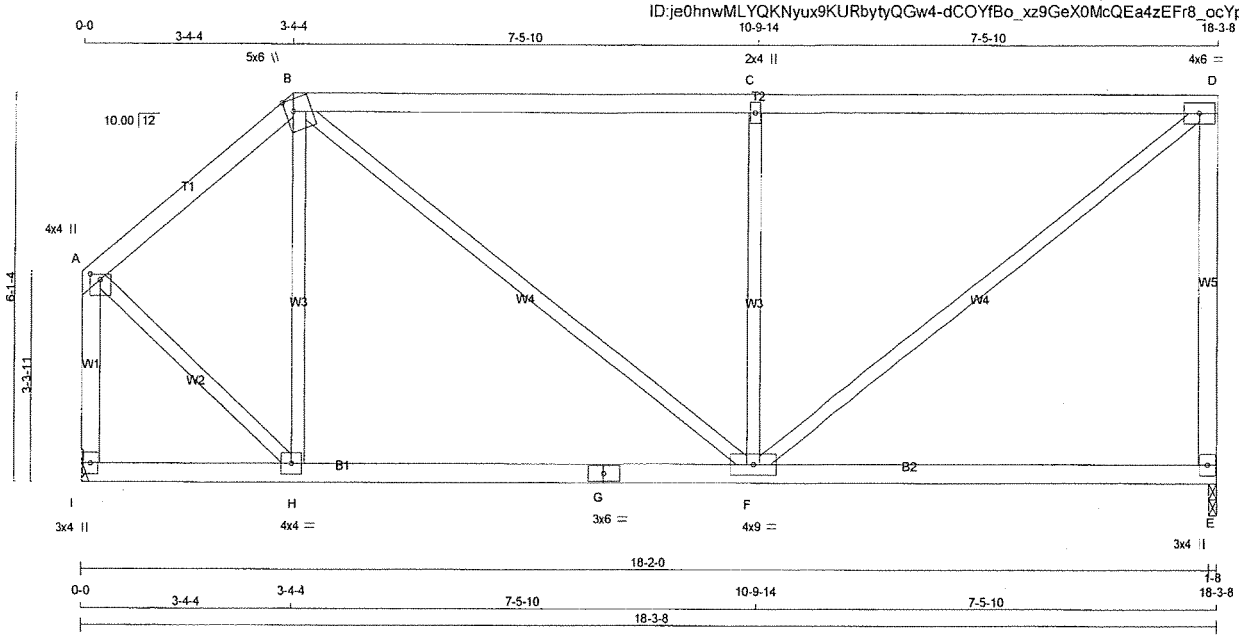


TOTAL WEIGHT = 2 X 80 = 160 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1066	0	1066	0	0	1-8	1-8
I	1066	0	1066	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

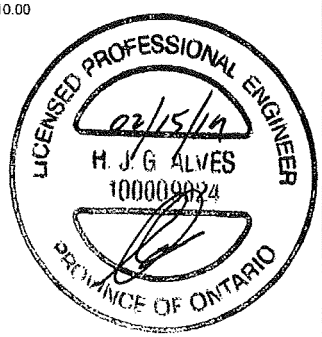
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-693 / 0	-78.0	-78.0	0.16 (1)	6.25
B- C	-941 / 0	-78.0	-78.0	0.82 (1)	4.83
C- D	-941 / 0	-78.0	-78.0	0.83 (1)	4.83
E- D	-951 / 0	0.0	0.0	0.69 (1)	7.81
I- A	-1032 / 0	0.0	0.0	0.19 (1)	7.72
I- H	0 / 0	-38.5	-38.5	0.21 (3)	10.00
H- G	0 / 527	-38.5	-38.5	0.47 (3)	10.00
G- F	0 / 527	-38.5	-38.5	0.47 (3)	10.00
F- E	0 / 0	-38.5	-38.5	0.42 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-230 / 24	0.13 (1)
B- F	0 / 526	0.12 (1)
F- C	-722 / 0	0.42 (1)
F- D	0 / 1191	0.27 (1)
A- H	0 / 699	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.83/1.00 (C-D:1) , BC=0.47/1.00 (F-H:3) , WB=0.42/1.00 (C-F:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

DWG NO. TAM T1903397
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T41	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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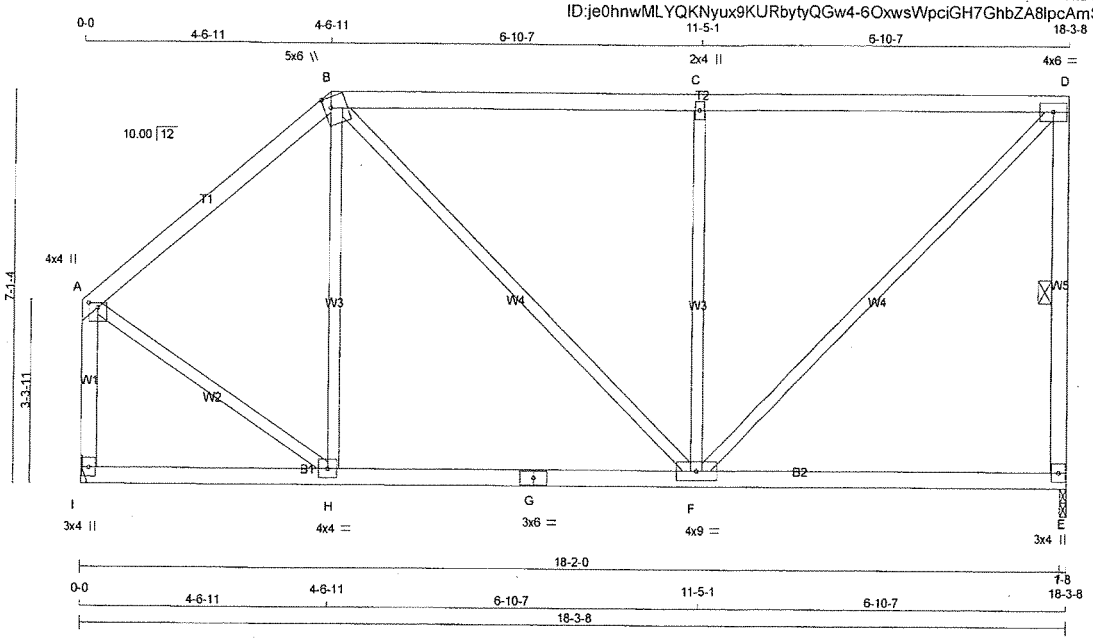


TOTAL WEIGHT = 2 X 84 = 168 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1066	0	1066	0	0	1-8	1-8
I	1066	0	1066	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

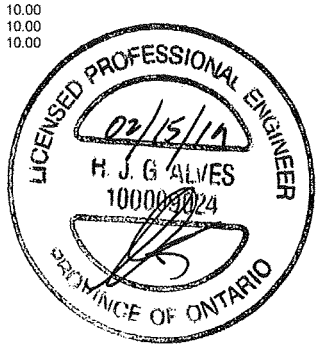
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-727 / 0	-78.0	-78.0	0.30 (1)	6.25	H- B	-123 / 92	0.11 (1)
B- C	-765 / 0	-78.0	-78.0	0.67 (1)	5.66	B- F	0 / 297	0.07 (1)
C- D	-766 / 0	-78.0	-78.0	0.67 (1)	5.66	F- C	-664 / 0	0.58 (1)
E- D	-959 / 0	0.0	0.0	0.22 (1)	6.25	F- D	0 / 1075	0.24 (1)
I- A	-1000 / 0	0.0	0.0	0.19 (1)	7.81	A- H	0 / 661	0.15 (1)
I- H	0 / 0	-38.5	-38.5	0.20 (3)	10.00			
H- G	0 / 555	-38.5	-38.5	0.41 (2)	10.00			
G- F	0 / 555	-38.5	-38.5	0.41 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.35 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.41/1.00 (F-H:2) , WB=0.58/1.00 (C-F:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)

DWG NO. TAM T1903398
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T42	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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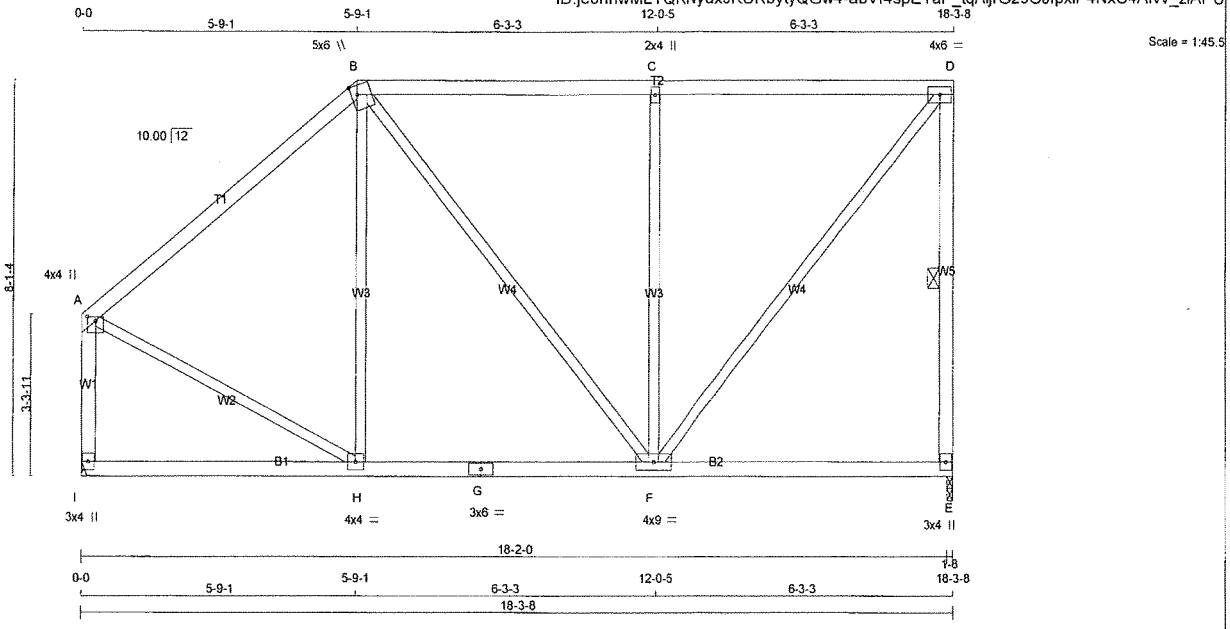


TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1066	0	1066	0	0	1-8	1-8
I	1066	0	1066	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

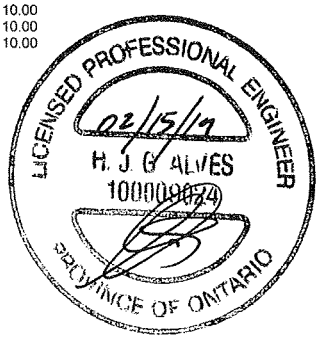
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-728 / 0	-78.0	-78.0	0.49 (1)	6.24	H- B	-32 / 157	0.04 (1)
B- C	-630 / 0	-78.0	-78.0	0.54 (1)	6.25	B- F	0 / 115	0.03 (1)
C- D	-630 / 0	-78.0	-78.0	0.54 (1)	6.25	F- C	-606 / 0	0.77 (1)
E- D	-967 / 0	0.0	0.0	0.29 (1)	6.25	F- D	0 / 1002	0.23 (1)
I- A	-975 / 0	0.0	0.0	0.18 (1)	7.81	A- H	0 / 626	0.14 (1)
I- H	0 / 0	-38.5	-38.5	0.24 (3)	10.00			
H- G	0 / 558	-38.5	-38.5	0.34 (2)	10.00			
G- F	0 / 558	-38.5	-38.5	0.34 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.34/1.00 (F-H:2) , WB=0.77/1.00 (C-F:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

DWG NO. TAM 71903399
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T43	2	1	TRUSS DESC.	

BLOCK 1 FREEHOLDUNITS 1 - 4

Tamarack Roof Truss, Burlington

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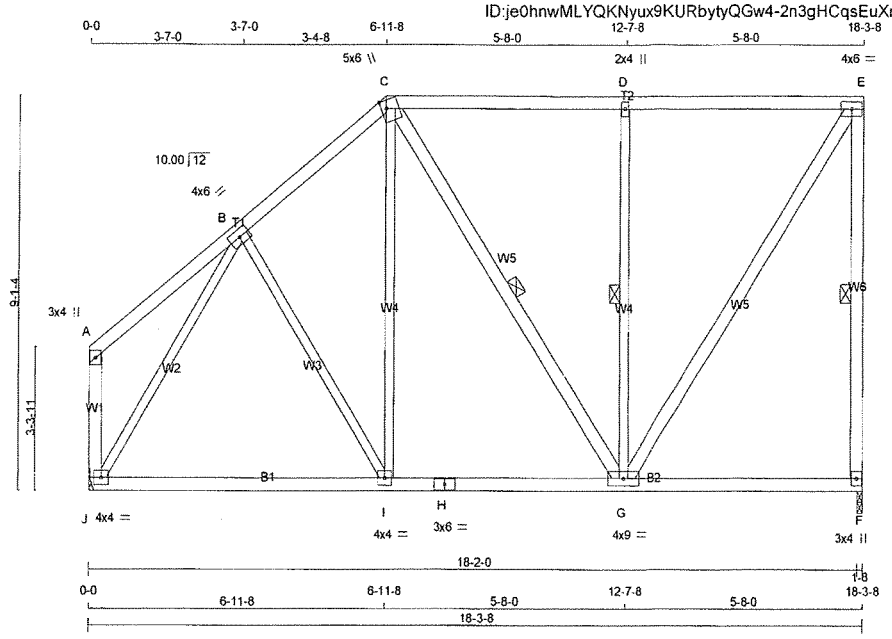


TOTAL WEIGHT = 2 X 105 = 209 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
F	1066	0	1066	0	0	1-8	1-8
J	1066	0	1066	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

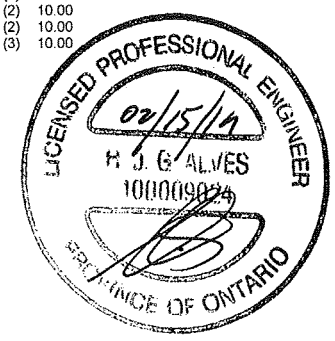
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-78.0	-78.0	0.15 (1)	10.00	B- I	0 / 116	0.03 (3)
B- C	-726 / 0	-78.0	-78.0	0.16 (1)	6.25	I- C	0 / 266	0.06 (3)
C- D	-521 / 0	-78.0	-78.0	0.43 (1)	6.25	C- G	-57 / 0	0.03 (3)
D- E	-521 / 0	-78.0	-78.0	0.43 (1)	6.25	G- D	-547 / 0	0.30 (1)
F- E	-974 / 0	0.0	0.0	0.38 (1)	6.25	G- E	0 / 958	0.15 (1)
J- A	-106 / 0	0.0	0.0	0.02 (1)	7.81	J- B	-986 / 0	0.83 (1)
J- I	0 / 504	-38.5	-38.5	0.37 (2)	10.00			
I- H	0 / 543	-38.5	-38.5	0.38 (2)	10.00			
H- G	0 / 543	-38.5	-38.5	0.38 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.38/1.00 (G-I:2) , WB=0.83/1.00 (B-J:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (J) (INPUT = 0.90)
JSI METAL= 0.31 (H) (INPUT = 1.00)

BWS NO. TAM 21903400
STRUCTURAL
CONFIGURATION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T46	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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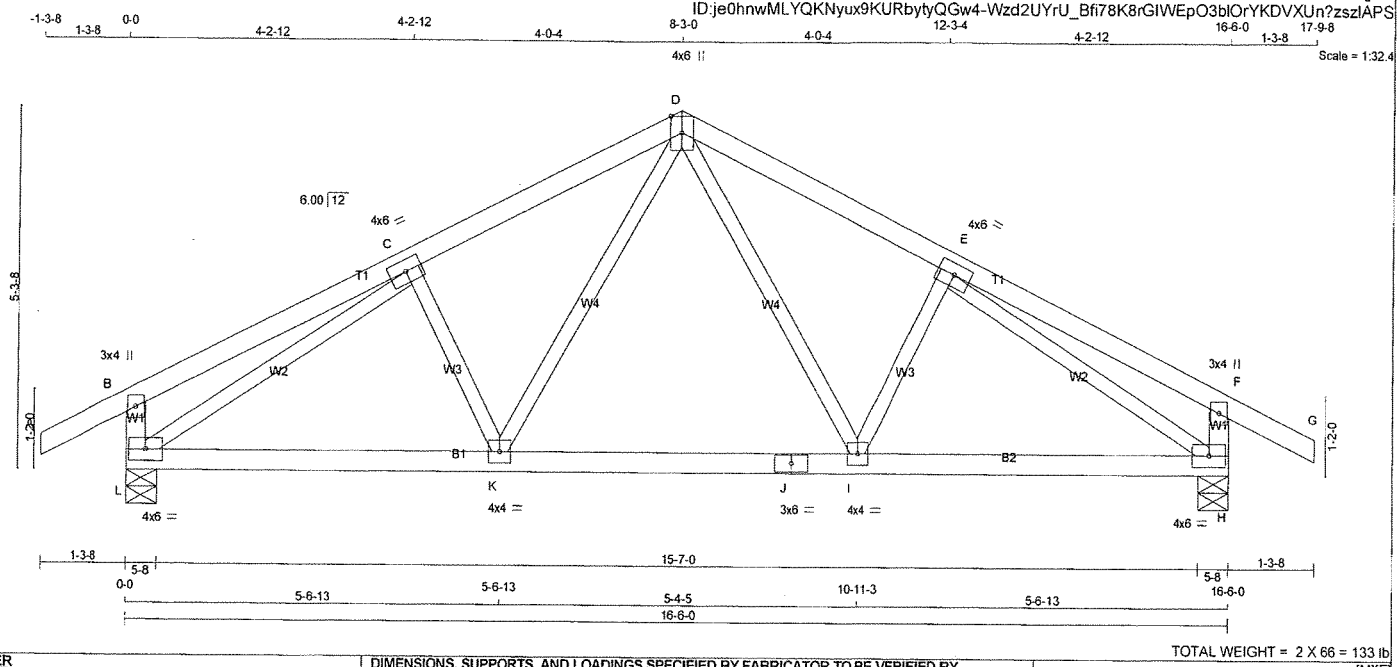


TOTAL WEIGHT = 2 X 66 = 133 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1067	0	1067	0	0	5-8	5-8
H	1067	0	1067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	808	404 / 0	173 / 0	0 / 0	0 / 0	231 / 0	0 / 0
H	808	404 / 0	173 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

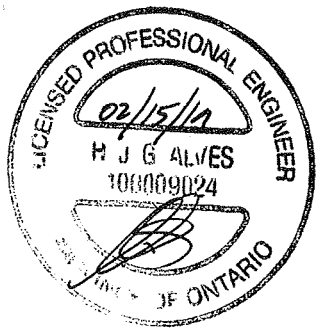
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-78.0	-78.0	0.10 (1)	10.00
B- C	0 / 16	-78.0	-78.0	0.20 (1)	10.00
C- D	-1119 / 0	-78.0	-78.0	0.17 (1)	5.82
D- E	-1119 / 0	-78.0	-78.0	0.17 (1)	5.82
E- F	0 / 16	-78.0	-78.0	0.20 (1)	10.00
F- G	0 / 24	-78.0	-78.0	0.10 (1)	10.00
L- B	-232 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 1055	-38.5	-38.5	0.34 (2)	10.00
K- J	0 / 789	-38.5	-38.5	0.30 (2)	10.00
J- I	0 / 789	-38.5	-38.5	0.30 (2)	10.00
I- H	0 / 1055	-38.5	-38.5	0.34 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 418	0.09 (1)
I- E	-155 / 38	0.03 (1)
K- D	0 / 418	0.09 (1)
C- K	-155 / 38	0.03 (1)
L- C	-1293 / 0	0.52 (1)
E- H	-1293 / 0	0.52 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.34/1.00 (K-L:2) , WB=0.52/1.00 (C-L:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

DWG NO. TAM T1903401
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	G46	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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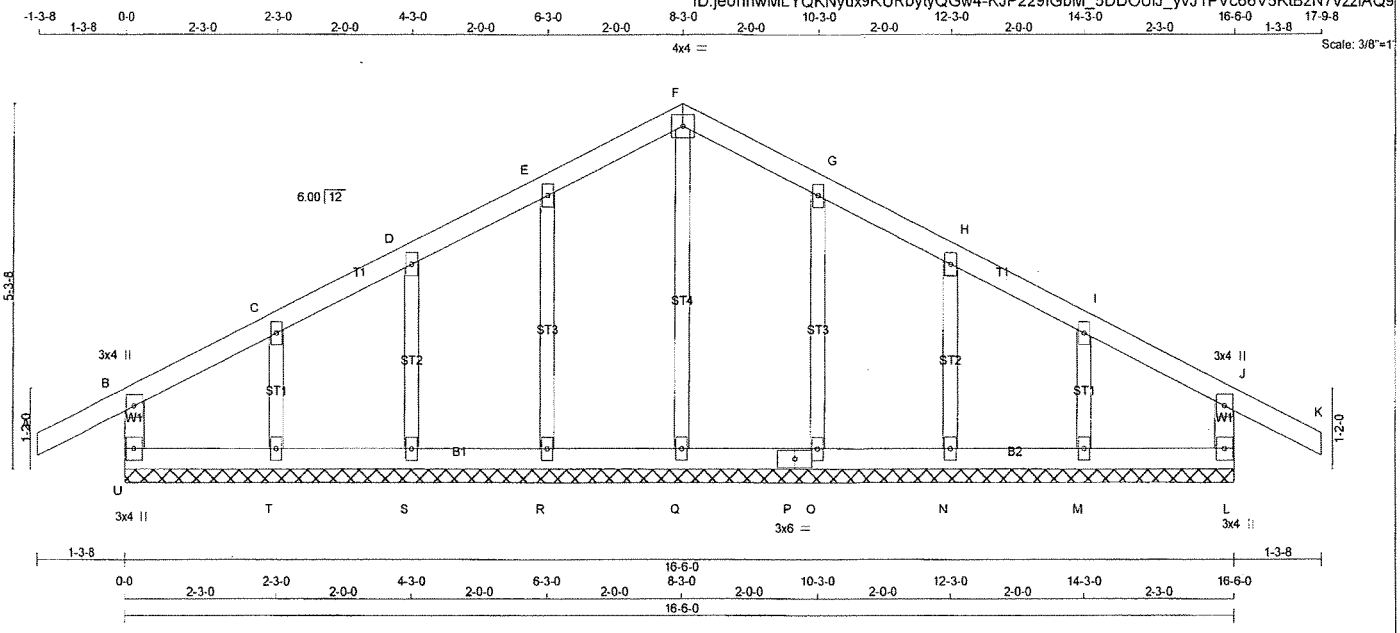


TOTAL WEIGHT = 2 X 62 = 124 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, Q, R, S, T						
M	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

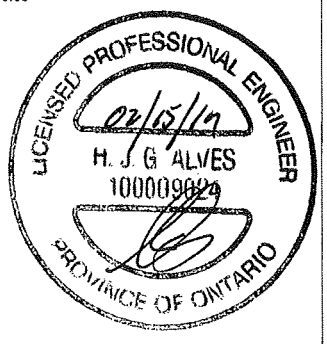
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 24	-78.0	-78.0	0.10 (1)	10.00	Q- F	-148 / 0	0.06 (1)
B- C	-29 / 0	-78.0	-78.0	0.05 (1)	6.25	R- E	-158 / 0	0.04 (1)
C- D	-23 / 0	-78.0	-78.0	0.04 (1)	6.25	S- D	-152 / 0	0.03 (1)
D- E	-19 / 0	-78.0	-78.0	0.04 (1)	6.25	T- C	-160 / 0	0.02 (1)
E- F	-17 / 0	-78.0	-78.0	0.04 (1)	6.25	O- G	-158 / 0	0.04 (1)
F- G	-17 / 0	-78.0	-78.0	0.04 (1)	6.25	N- H	-152 / 0	0.03 (1)
G- H	-19 / 0	-78.0	-78.0	0.04 (1)	6.25	M- I	-160 / 0	0.02 (1)
H- I	-23 / 0	-78.0	-78.0	0.04 (1)	6.25			
I- J	-29 / 0	-78.0	-78.0	0.05 (1)	6.25			
J- K	0 / 24	-78.0	-78.0	0.10 (1)	10.00			
U- B	-206 / 0	0.0	0.0	0.02 (2)	7.81			
L- J	-206 / 0	0.0	0.0	0.02 (2)	7.81			
U- T	0 / 25	-38.5	-38.5	0.03 (3)	10.00			
T- S	0 / 20	-38.5	-38.5	0.03 (3)	10.00			
S- R	0 / 17	-38.5	-38.5	0.03 (3)	10.00			
R- Q	0 / 15	-38.5	-38.5	0.03 (3)	10.00			
Q- P	0 / 15	-38.5	-38.5	0.03 (3)	10.00			
P- O	0 / 15	-38.5	-38.5	0.03 (3)	10.00			
O- N	0 / 17	-38.5	-38.5	0.03 (3)	10.00			
N- M	0 / 20	-38.5	-38.5	0.03 (3)	10.00			
M- L	0 / 25	-38.5	-38.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (J-K:1) , BC=0.03/1.00 (T-U:3) , WB=0.06/1.00 (F-Q:1) , SSI=0.07/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (G) (INPUT = 0.90)
JSI METAL= 0.07 (J) (INPUT = 1.00)

BCIS NO. TAM T1903388
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T47	2	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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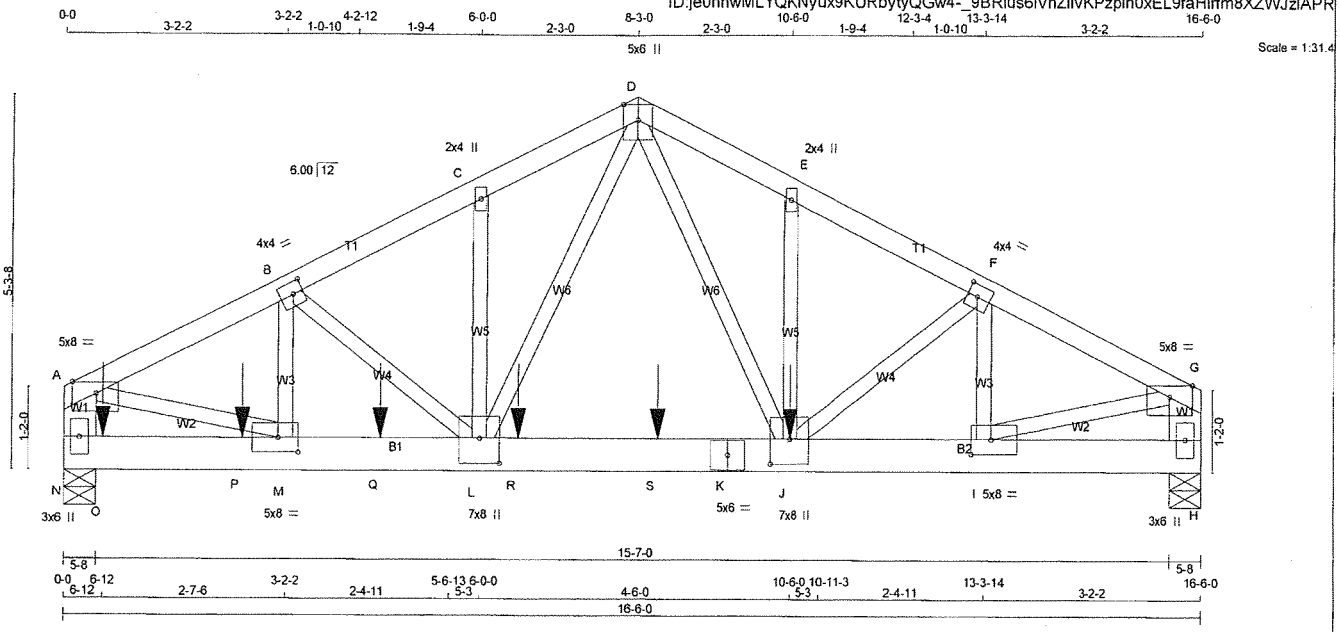


TOTAL WEIGHT = 4 X 80 = 320 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
N- A	3	2	SIDE(87.1)
H- G	3	2	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	SIDE(183.1)
K- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
J- E	1	6	SIDE(210.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	8.0	2.50	3.50
J	BMWWW+t	MT20	7.0	8.0	4.25	3.50
K	BS-t	MT20	5.0	6.0		
L	BMWWW+t	MT20	7.0	8.0	4.25	3.50
M	BMWW-t	MT20	5.0	8.0	2.50	3.50
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION				INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		IN-SX	IN-SX
N	5234	0	5234	0	0		5-8	5-8
H	3343	0	3343	0	0		5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3979	1936 / 0	894 / 0	0 / 0	0 / 0	1149 / 0	0 / 0
H	2545	1227 / 0	581 / 0	0 / 0	0 / 0	737 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

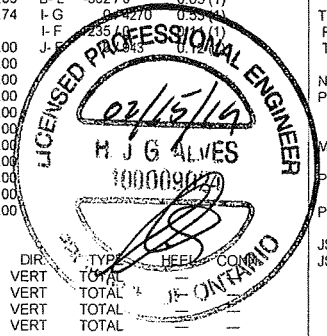
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-6079 / 0	-78.0	-78.0	0.20 (1)	3.83
B- C	-5751 / 0	-78.0	-78.0	0.17 (1)	3.94
C- D	-5736 / 0	-78.0	-78.0	0.16 (1)	3.96
D- E	-5437 / 0	-78.0	-78.0	0.15 (1)	4.06
E- F	-5457 / 0	-78.0	-78.0	0.15 (1)	4.05
F- G	-4632 / 0	-78.0	-78.0	0.14 (1)	4.37
N- A	-4166 / 0	0.0	0.0	0.15 (1)	6.99
H- G	-3197 / 0	0.0	0.0	0.11 (1)	7.74
N- O	0 / 0	-38.5	-38.5	0.22 (1)	10.00
O- P	0 / 0	-38.5	-38.5	0.22 (1)	10.00
P- M	0 / 0	-38.5	-38.5	0.22 (1)	10.00
M- Q	0 / 5439	-38.5	-38.5	0.63 (1)	10.00
Q- L	0 / 5439	-38.5	-38.5	0.63 (1)	10.00
L- R	0 / 3900	-38.5	-38.5	0.58 (1)	10.00
R- S	0 / 3900	-38.5	-38.5	0.58 (1)	10.00
S- K	0 / 3900	-38.5	-38.5	0.58 (1)	10.00
K- J	0 / 3900	-38.5	-38.5	0.58 (1)	10.00
J- I	0 / 4148	-38.5	-38.5	0.44 (1)	10.00
I- H	0 / 0	-38.5	-38.5	0.10 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- E	-158 / 0	0.02 (1)
L- C	-172 / 0	0.02 (1)
L- D	0 / 2959	0.37 (1)
D- J	0 / 2320	0.29 (1)
M- B	0 / 176	0.02 (2)
A- M	0 / 5598	0.69 (1)
B- L	-392 / 0	0.05 (1)
I- G	0 / 4270	0.53 (1)
I- F	-235 / 0	0.03 (1)
J- E	-2843 / 0	0.12 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	COMMO
J	10-6-0	-1510	-1510	---	FRONT	VERT	TOTAL	---	
O	6-12	-1036	-1036	---	FRONT	VERT	TOTAL	---	
P	2-6-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	
Q	4-6-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	
R	6-6-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	
S	8-6-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	44.5	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.63/1.00 (L-M:1) , WB=0.69/1.00 (A-M:1) , SSI=0.49/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.51 (M) (INPUT = 1.00)

BWS NO. TAM 11903402
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T48	4	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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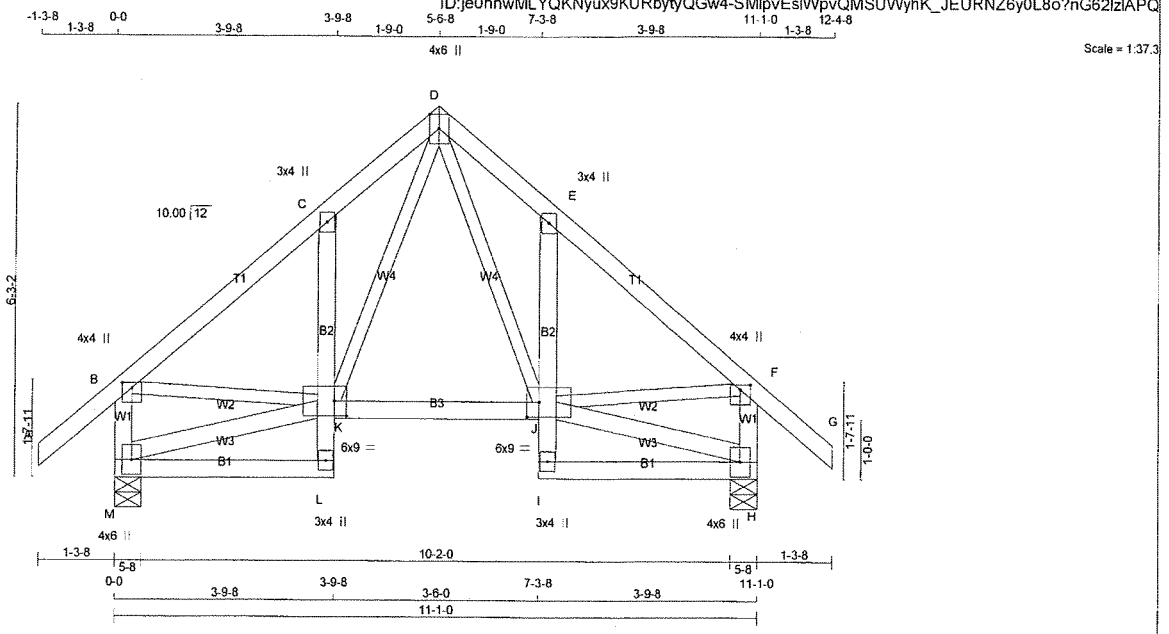


TOTAL WEIGHT = 4 X 67 = 266 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMVW1+p	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	6.0	9.0	3.00	2.50
K	BVMWWW-I	MT20	6.0	9.0	3.00	2.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	754	0	754	0	0	5-8	5-8
H	754	0	754	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	568	291 / 0	116 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	568	291 / 0	116 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

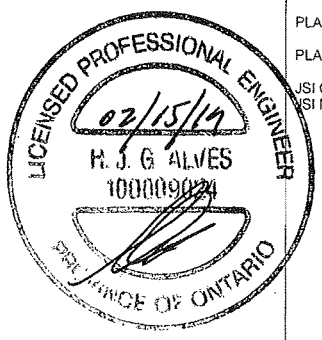
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	D- J	0 / 397	0.09 (1)
B- C	-633 / 0	-78.0	-78.0	0.12 (1)	6.25	K- D	0 / 397	0.09 (1)
C- D	-665 / 0	-78.0	-78.0	0.08 (1)	6.25	M- K	-10 / 0	0.00 (1)
D- E	-665 / 0	-78.0	-78.0	0.08 (1)	6.25	B- K	0 / 499	0.11 (1)
E- F	-633 / 0	-78.0	-78.0	0.12 (1)	6.25	J- H	-10 / 0	0.00 (1)
F- G	0 / 35	-78.0	-78.0	0.11 (1)	10.00	J- F	0 / 499	0.11 (1)
M- B	-681 / 0	0.0	0.0	0.07 (1)	7.81			
H- F	-681 / 0	0.0	0.0	0.07 (1)	7.81			
M- L	0 / 9	-38.5	-38.5	0.12 (3)	10.00			
L- K	0 / 89	0.0	0.0	0.03 (2)	10.00			
K- C	-272 / 0	0.0	0.0	0.01 (1)	7.81			
K- J	0 / 349	-38.5	-38.5	0.17 (2)	10.00			
I- J	0 / 89	0.0	0.0	0.03 (2)	10.00			
J- E	-272 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 9	-38.5	-38.5	0.12 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (E-F:1) , BC=0.17/1.00 (J-K:2) , WB=0.11/1.00 (B-K:1) , SSI=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

DWG NO. TAM T1903405
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T49	3	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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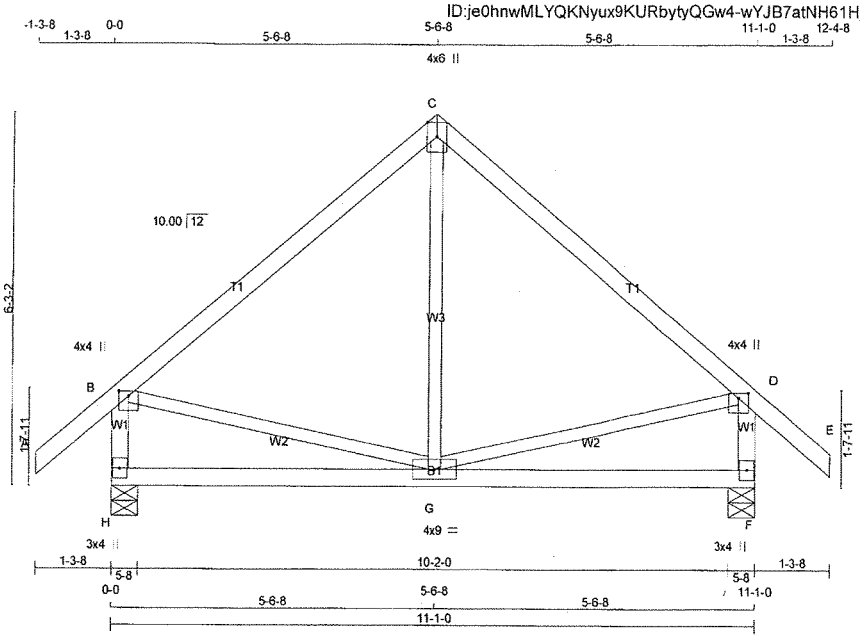

TOTAL WEIGHT = 3 X 49 = 148 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	754	0	754	0	0	5-8	5-8
F	754	0	754	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	568	291 / 0	116 / 0	0 / 0	0 / 0	161 / 0	0 / 0
F	568	291 / 0	116 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	G- C	0 / 245	0.06 (3)
B- C	-424 / 0	-78.0	-78.0	0.31 (1)	6.25	B- G	0 / 335	0.08 (1)
C- D	-424 / 0	-78.0	-78.0	0.31 (1)	6.25	G- D	0 / 335	0.08 (1)
D- E	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
H- B	-673 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-673 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-38.5	-38.5	0.27 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.27/1.00 (F-G:3) , WB=0.08/1.00 (B-G:1) , SSI=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

DWG NO. TAM T1903406
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T49A	7	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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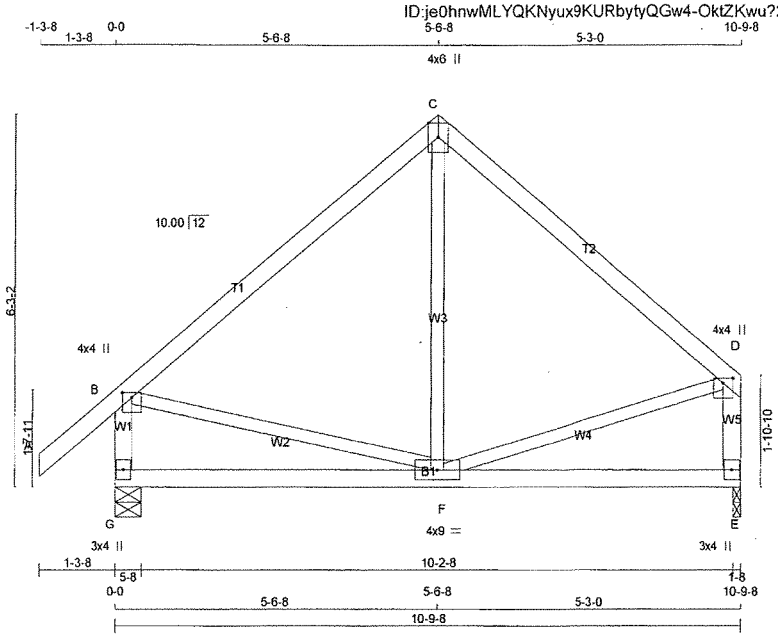

TOTAL WEIGHT = 7 X 47 = 328 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	737	0	737	0	0	5-8	5-8
E	629	0	629	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	555	285 / 0	113 / 0	0 / 0	0 / 0	157 / 0	0 / 0
E	480	227 / 0	113 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	F- C	0 / 230	0.06 (3)
B- C	-400 / 0	-78.0	-78.0	0.31 (1)	6.25	B- F	0 / 315	0.07 (1)
C- D	-404 / 0	-78.0	-78.0	0.28 (1)	6.25	F- D	0 / 324	0.07 (1)
G- B	-650 / 0	0.0	0.0	0.07 (1)	7.81			
E- D	-558 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-38.5	-38.5	0.26 (3)	10.00			
F- E	0 / 0	-38.5	-38.5	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.26/1.00 (E-F:3) , WB=0.07/1.00 (D-F:1) , SSI=0.14/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM 71903407
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T50	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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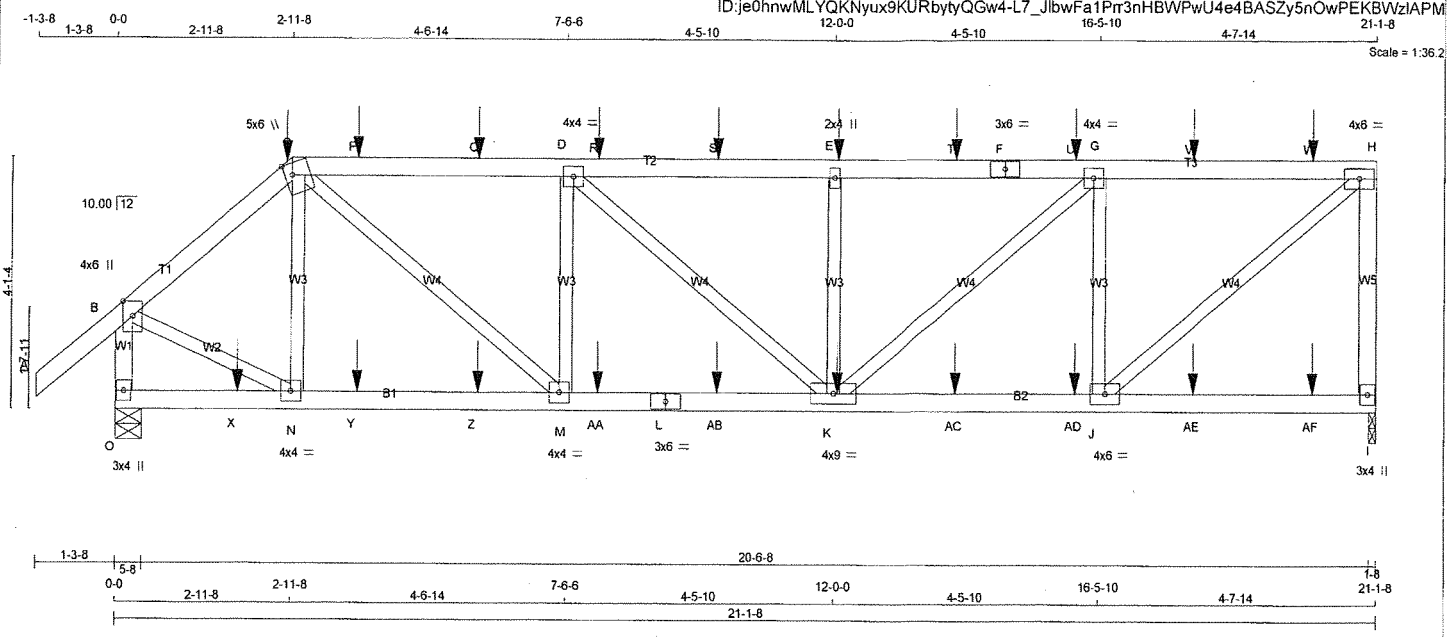


TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	1	12	SIDE(0.0)
L- I	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2026	0	2026	0	0	1-8	1-8
O	2120	0	2120	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1553	718 / 0	379 / 0	0 / 0	0 / 0	456 / 0	0 / 0
O	1612	788 / 0	365 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 35	-78.0	-78.0	0.06 (1)	10.00	N- C	-349 / 1	0.05 (1)
B- C	-1909 / 0	-78.0	-78.0	0.08 (1)	6.25	B- N	0 / 1589	0.20 (1)
C- P	-2712 / 0	-78.0	-78.0	0.24 (1)	5.28	J- H	0 / 2642	0.33 (1)
P- Q	-2712 / 0	-78.0	-78.0	0.24 (1)	5.28	C- M	0 / 1637	0.20 (1)
Q- D	-2712 / 0	-78.0	-78.0	0.24 (1)	5.28	J- G	-1332 / 0	0.18 (1)
D- R	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18	M- D	-743 / 0	0.10 (1)
R- S	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18	K- G	0 / 1046	0.13 (1)
S- E	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18	D- K	0 / 169	0.02 (1)
E- T	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18	K- F	0 / 1046	0.13 (1)
T- F	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18			
F- U	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18			
U- G	-2841 / 0	-78.0	-78.0	0.24 (1)	5.18			
G- V	-2044 / 0	-78.0	-78.0	0.22 (1)	5.90			
V- W	-2044 / 0	-78.0	-78.0	0.22 (1)	5.90			
W- H	-2044 / 0	-78.0	-78.0	0.22 (1)	5.90			
I- H	-1903 / 0	0.0	0.0	0.26 (1)	7.81			
O- B	-2075 / 0	0.0	0.0	0.12 (1)	7.68			
O- X	0 / 0	-38.5	-38.5	0.10 (3)	10.00			
X- N	0 / 0	-38.5	-38.5	0.10 (3)	10.00			
N- Y	0 / 1452	-38.5	-38.5	0.22 (2)	10.00			
Y- Z	0 / 1452	-38.5	-38.5	0.22 (2)	10.00			
Z- M	0 / 1452	-38.5	-38.5	0.22 (2)	10.00			
M-AA	0 / 2712	-38.5	-38.5	0.32 (2)	10.00			
AA- L	0 / 2712	-38.5	-38.5	0.32 (2)	10.00			
L-AB	0 / 2712	-38.5	-38.5	0.32 (2)	10.00			
AB- K	0 / 2712	-38.5	-38.5	0.32 (2)	10.00			
K-AC	0 / 2044	-38.5	-38.5	0.29 (2)	10.00			
AC-AD	0 / 2044	-38.5	-38.5	0.29 (2)	10.00			
AD- J	0 / 2044	-38.5	-38.5	0.29 (2)	10.00			
J-AE	0 / 0	-38.5	-38.5	0.14 (3)	10.00			
AE-AF	0 / 0	-38.5	-38.5	0.14 (3)	10.00			
AF- I	0 / 0	-38.5	-38.5	0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-25	-28	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-158	-158	---	FRONT	VERT	SNOW	---	---
E	12-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
K	12-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.26/1.00 (H-I:1) , BC=0.32/1.00 (K-M:2) , WB=0.33/1.00 (H-J:1) , SSI=0.14/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

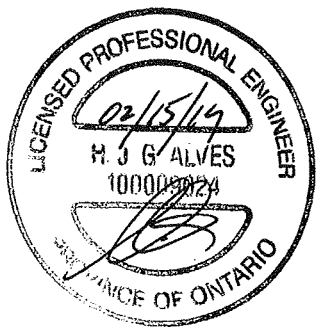
LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1903408
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	4-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	6-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	8-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	10-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
T	14-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
U	16-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
V	18-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
W	20-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
X	2-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
Y	4-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
Z	6-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AA	8-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	10-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	14-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	16-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	18-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	20-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM T1903408
STRUCTURAL
CONSENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T51	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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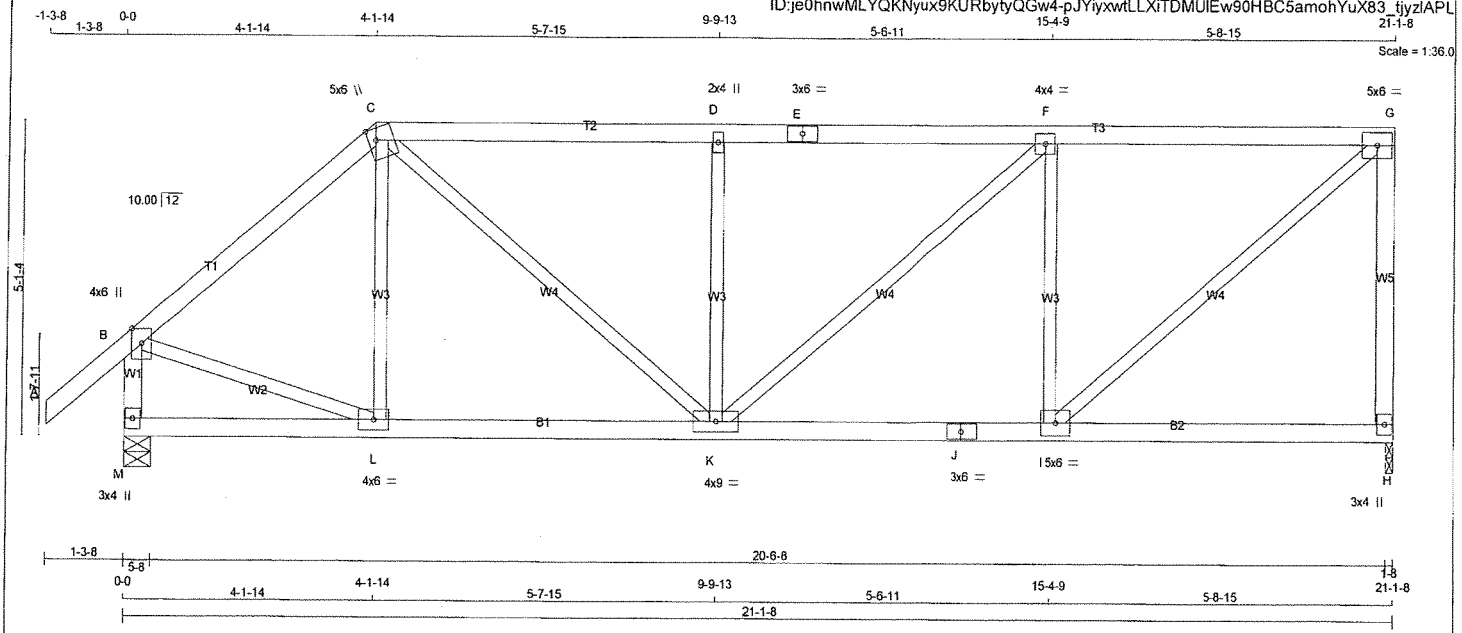


TOTAL WEIGHT = 2 X 90 = 180 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1231	0	1231	0	0	1-8	1-8
M	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
M	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

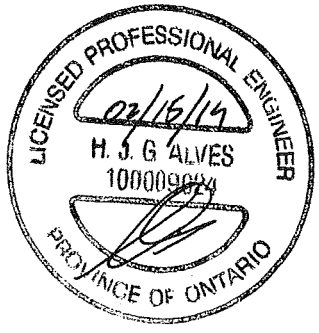
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	L- C	-56 / 118	0.03 (3)
B- C	-1150 / 0	-78.0	-78.0	0.26 (1)	5.62	B- L	0 / 921	0.21 (1)
C- D	-1409 / 0	-78.0	-78.0	0.38 (1)	5.04	I- G	0 / 1494	0.34 (1)
D- E	-1409 / 0	-78.0	-78.0	0.39 (1)	5.02	C- K	0 / 693	0.16 (1)
E- F	-1409 / 0	-78.0	-78.0	0.39 (1)	5.02	I- F	-713 / 0	0.27 (1)
F- G	-1146 / 0	-78.0	-78.0	0.38 (1)	5.45	K- D	-472 / 0	0.18 (1)
H- G	-1141 / 0	0.0	0.0	0.50 (1)	7.44	K- F	0 / 348	0.08 (1)
M- B	-1279 / 0	0.0	0.0	0.14 (1)	7.13			
M- L	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
L- K	0 / 880	-38.5	-38.5	0.29 (2)	10.00			
K- J	0 / 1146	-38.5	-38.5	0.38 (2)	10.00			
J- I	0 / 1146	-38.5	-38.5	0.38 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.50/1.00 (G-H:1) , BC=0.38/1.00 (I-K:2) , WB=0.34/1.00 (G-I:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

DWG NO. TAM 71903409
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T52	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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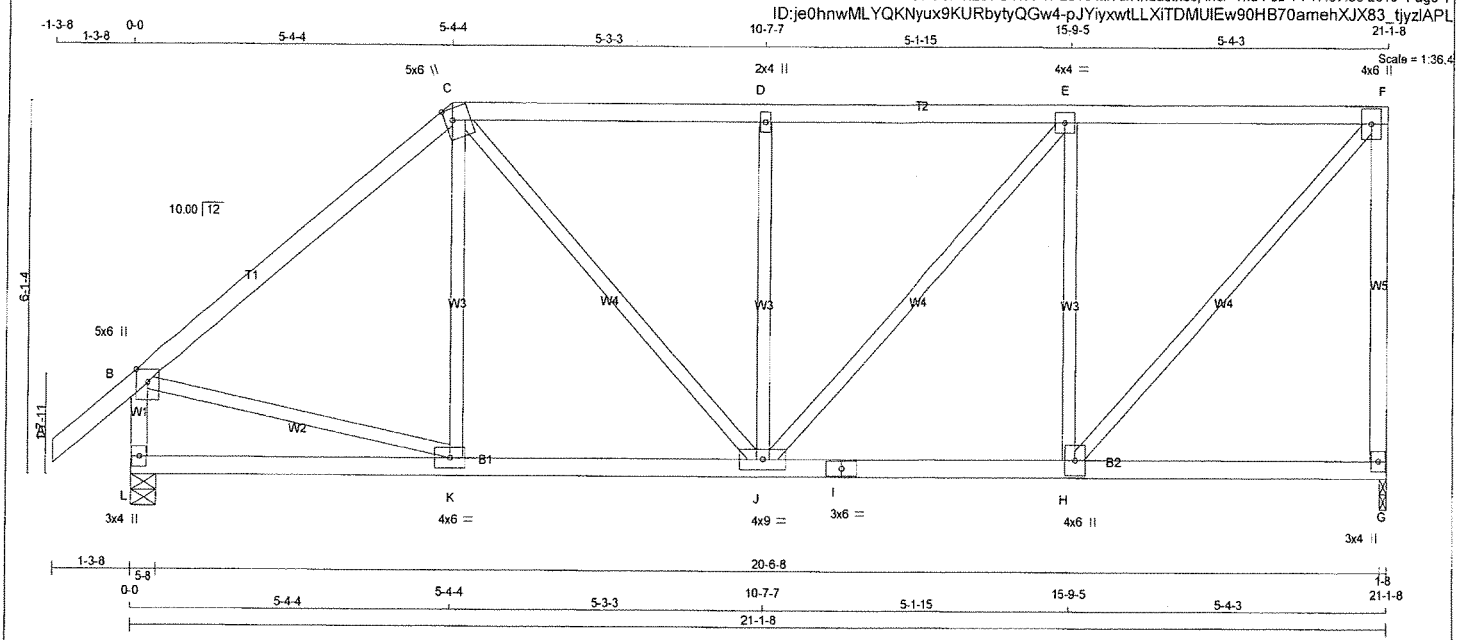

TOTAL WEIGHT = 2 X 96 = 191 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1231	0	1231	0	0	1-8	1-8
L	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
L	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO.	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO
A- B	0 / 35		-78.0	-78.0	0.11 (1)	10.00	K- C	0 / 182	0.04 (3)	
B- C	-1136 / 0		-78.0	-78.0	0.45 (1)	5.38	B- K	0 / 896	0.20 (1)	
C- D	-1163 / 0		-78.0	-78.0	0.31 (1)	5.52	H- F	0 / 1330	0.30 (1)	
D- E	-1163 / 0		-78.0	-78.0	0.32 (1)	5.50	C- J	0 / 433	0.10 (1)	
E- F	-901 / 0		-78.0	-78.0	0.31 (1)	6.06	H- E	-747 / 0	0.44 (1)	
G- F	-1148 / 0		0.0	0.0	0.83 (1)	7.42	J- D	-438 / 0	0.26 (1)	
L- B	-1255 / 0		0.0	0.0	0.13 (1)	7.18	J- E	0 / 396	0.09 (1)	
L- K	0 / 0		-38.5	-38.5	0.21 (3)	10.00				
K- J	0 / 872		-38.5	-38.5	0.33 (2)	10.00				
J- I	0 / 901		-38.5	-38.5	0.32 (2)	10.00				
I- H	0 / 901		-38.5	-38.5	0.32 (2)	10.00				
H- G	0 / 0		-38.5	-38.5	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.83/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.44/1.00 (E-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T1903410
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T53	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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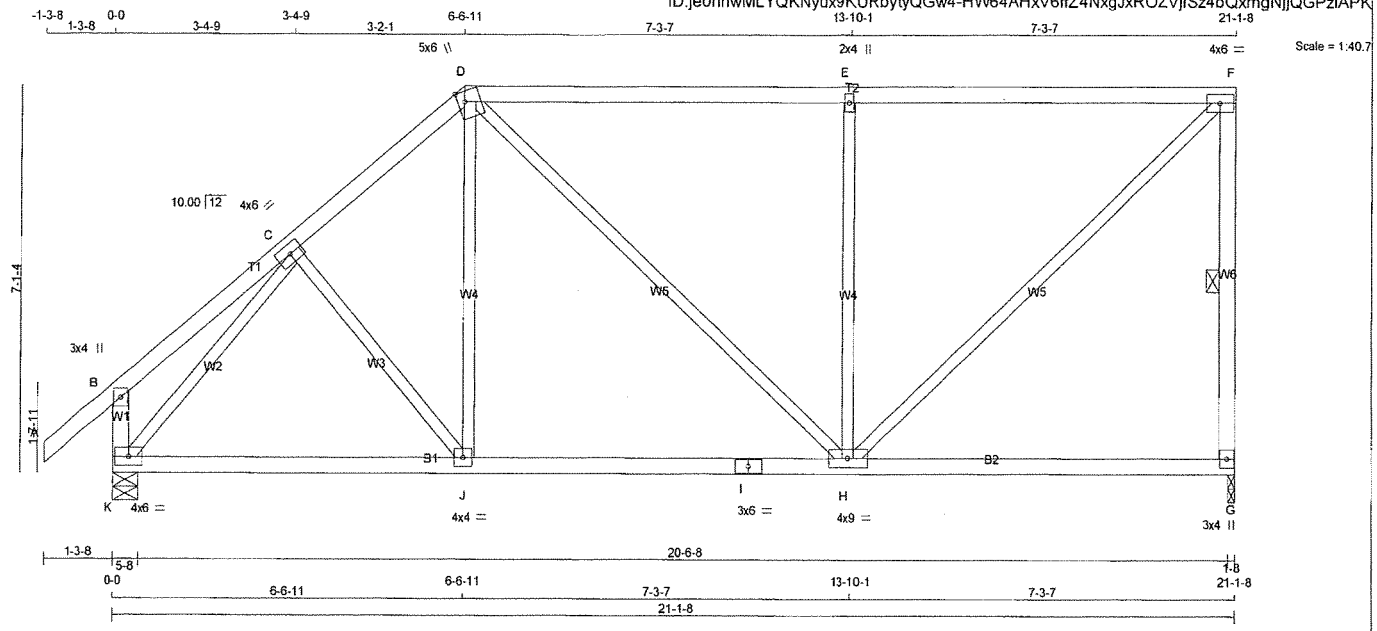


TOTAL WEIGHT = 2 X 95 = 191 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1231	0	1231	0	0	1-8	1-8
K	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
K	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

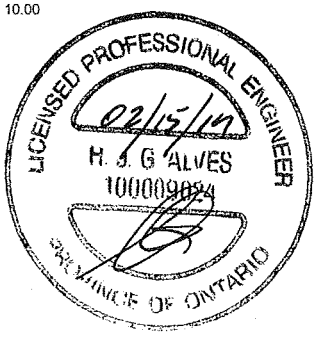
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00
B- C	0 / 17	-78.0	-78.0	0.12 (1)	10.00
C- D	-1117 / 0	-78.0	-78.0	0.14 (1)	5.84
D- E	-968 / 0	-78.0	-78.0	0.78 (1)	4.91
E- F	-968 / 0	-78.0	-78.0	0.78 (1)	4.91
G- F	-1117 / 0	0.0	0.0	0.25 (1)	6.00
K- B	-208 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 833	-38.5	-38.5	0.42 (2)	10.00
J- I	0 / 847	-38.5	-38.5	0.48 (2)	10.00
I- H	0 / 847	-38.5	-38.5	0.48 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.37 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 86	0.02 (3)
J- D	0 / 326	0.07 (2)
D- H	0 / 166	0.04 (1)
H- E	-704 / 0	0.62 (1)
H- F	0 / 1322	0.30 (1)
K- C	-1323 / 0	0.57 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78/1.00 (E-F:1) , BC=0.48/1.00 (H-J:2) , WB=0.62/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

BWS NO. TAM T1903411
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T54	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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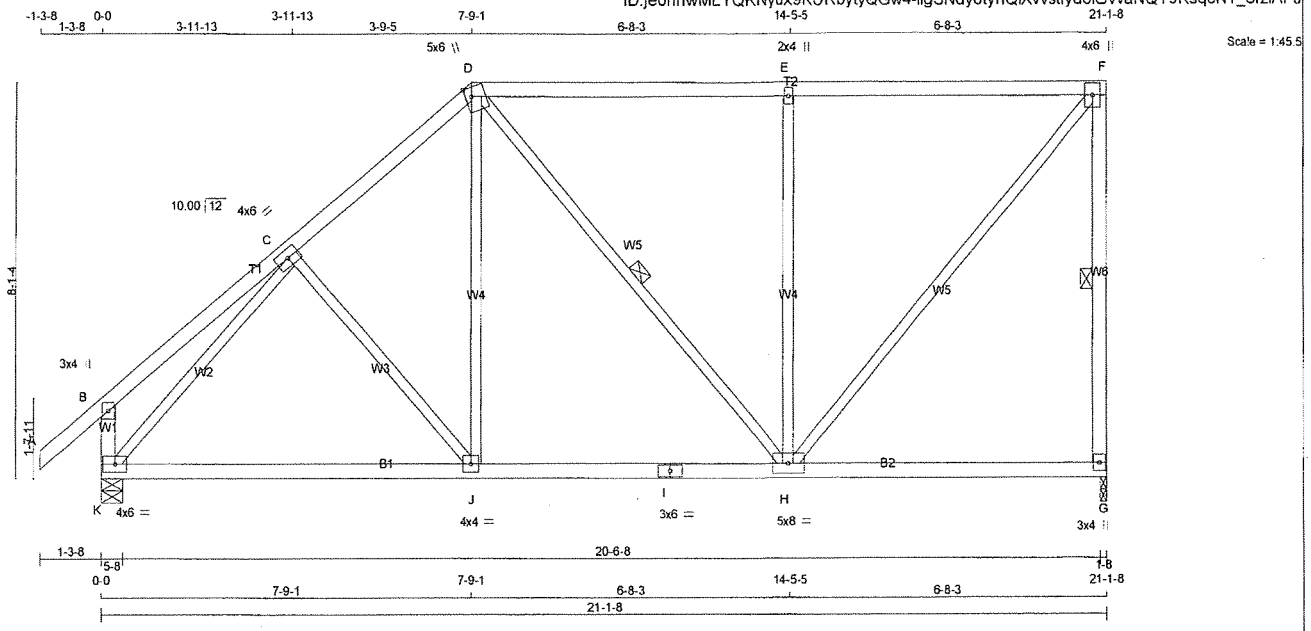


TOTAL WEIGHT = 2 X 100 = 201 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1231	0	1231	0	0	1-8	1-8
K	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
K	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

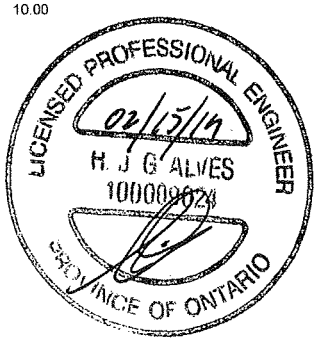
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	C- J	-68 / 55	0.04 (1)
B- C	0 / 22	-78.0	-78.0	0.18 (1)	10.00	J- D	0 / 411	0.09 (2)
C- D	-1066 / 0	-78.0	-78.0	0.20 (1)	5.86	D- H	-49 / 0	0.04 (3)
D- E	-796 / 0	-78.0	-78.0	0.63 (1)	5.68	H- E	-646 / 0	0.82 (1)
E- F	-796 / 0	-78.0	-78.0	0.63 (1)	5.68	H- F	0 / 1220	0.27 (1)
G- F	-1124 / 0	0.0	0.0	0.33 (1)	5.99	K- C	-1302 / 0	0.77 (1)
K- B	-224 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 845	-38.5	-38.5	0.49 (2)	10.00			
J- I	0 / 805	-38.5	-38.5	0.49 (2)	10.00			
I- H	0 / 805	-38.5	-38.5	0.49 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.49/1.00 (J-K:2) , WB=0.82/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM T1903412
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T55	5	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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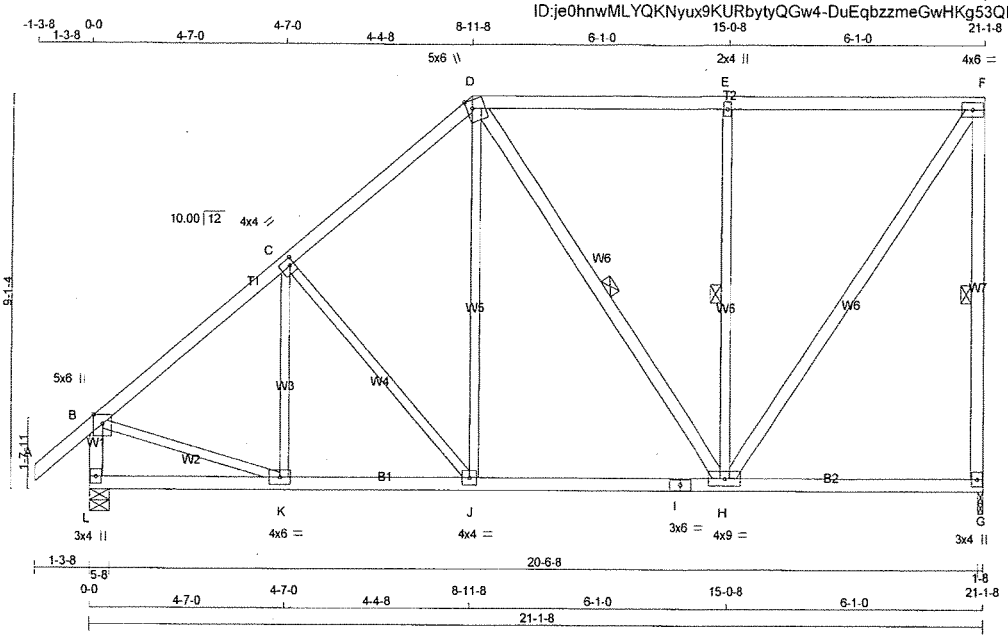


TOTAL WEIGHT = 5 X 115 = 573 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1231	0	1231	0	0	1-8	1-8
L	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
L	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

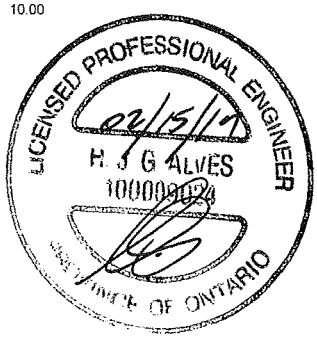
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	K- C	-72 / 101	0.03 (1)
B- C	-1161 / 0	-78.0	-78.0	0.30 (1)	5.55	C- J	-280 / 0	0.24 (1)
C- D	-976 / 0	-78.0	-78.0	0.28 (1)	5.94	J- D	0 / 427	0.10 (2)
D- E	-664 / 0	-78.0	-78.0	0.51 (1)	6.25	D- H	-120 / 0	0.07 (1)
E- F	-664 / 0	-78.0	-78.0	0.51 (1)	6.25	H- E	-588 / 0	0.32 (1)
G- F	-1137 / 0	0.0	0.0	0.44 (1)	5.96	H- F	0 / 1164	0.19 (1)
L- B	-1266 / 0	0.0	0.0	0.13 (1)	7.16	B- K	0 / 948	0.21 (1)
L- K	0 / 0	-38.5	-38.5	0.14 (3)	10.00			
K- J	0 / 912	-38.5	-38.5	0.26 (2)	10.00			
J- I	0 / 732	-38.5	-38.5	0.37 (2)	10.00			
I- H	0 / 732	-38.5	-38.5	0.37 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.37/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T7403413
STRUCTURAL
CITY OF [illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T56	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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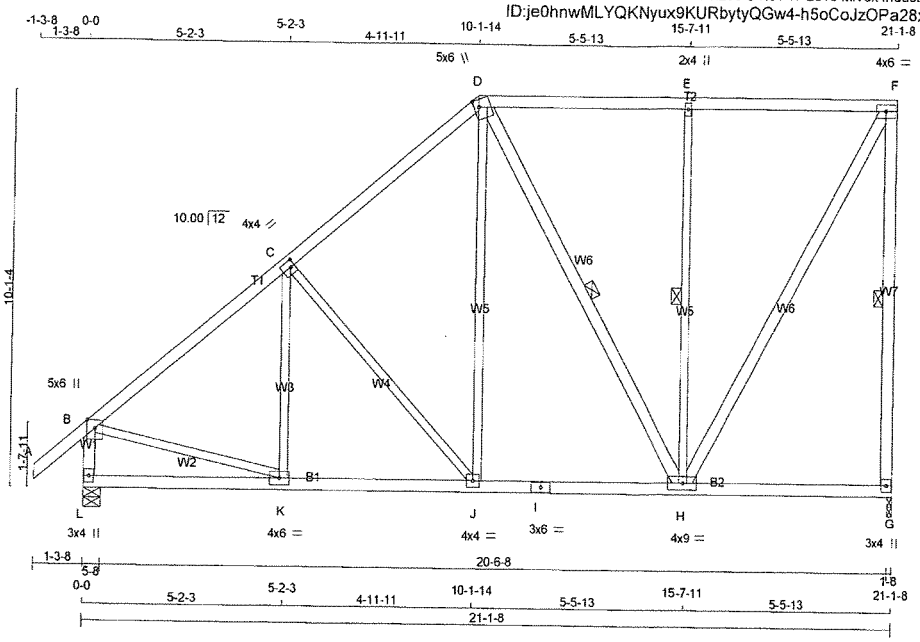


TOTAL WEIGHT = 2 X 121 = 241 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1231	0	1231	0	0	1-8	1-8
L	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
L	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

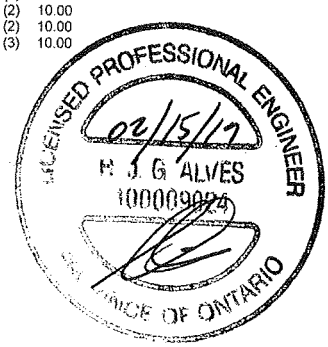
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	K- C	-9 / 159	0.04 (3)
B- C	-1163 / 0	-78.0	-78.0	0.39 (1)	5.42	C- J	-373 / 0	0.43 (1)
C- D	-900 / 0	-78.0	-78.0	0.37 (1)	5.98	J- D	0 / 484	0.11 (2)
D- E	-552 / 0	-78.0	-78.0	0.40 (1)	6.25	D- H	-244 / 0	0.17 (1)
E- F	-553 / 0	-78.0	-78.0	0.40 (1)	6.25	H- E	-529 / 0	0.37 (1)
G- F	-1146 / 0	0.0	0.0	0.57 (1)	5.94	H- F	0 / 1126	0.18 (1)
L- B	-1258 / 0	0.0	0.0	0.13 (1)	7.17	B- K	0 / 945	0.21 (1)
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
K- J	0 / 916	-38.5	-38.5	0.31 (2)	10.00			
J- I	0 / 671	-38.5	-38.5	0.31 (2)	10.00			
I- H	0 / 671	-38.5	-38.5	0.31 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.57/1.00 (F-G:1) , BC=0.31/1.00 (J-K:2) , WB=0.43/1.00 (C-J:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

BKS NO. TAM T1903414
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T58S	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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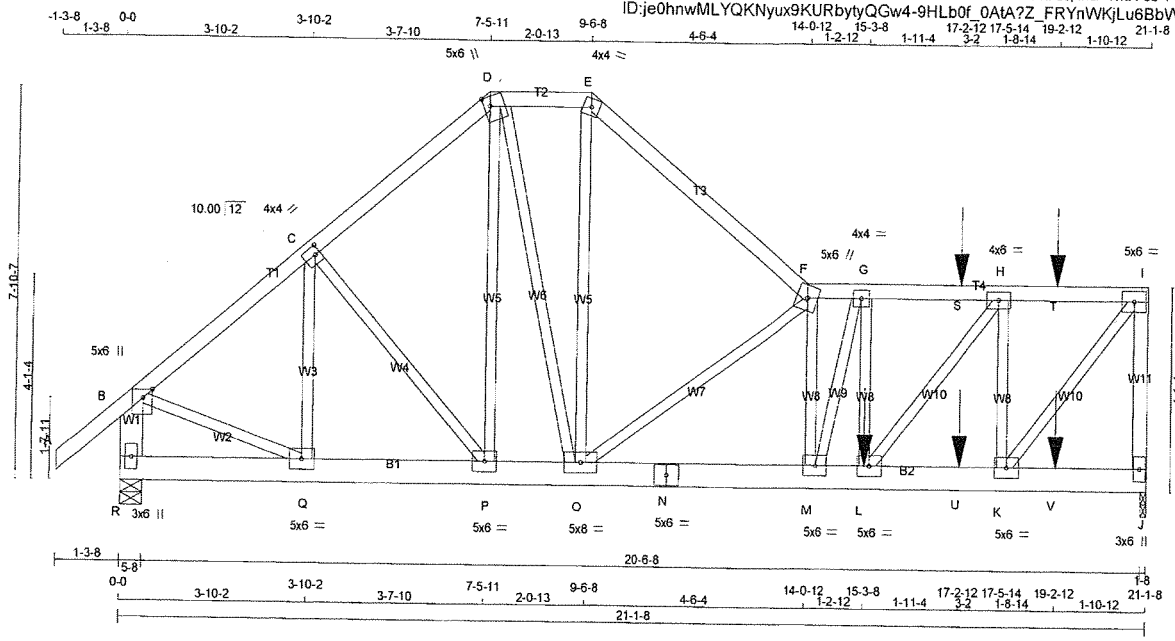


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- I	1	12	SIDE(0.0)
I- J	1	12	TOP
R- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- L	1	6	SIDE(136.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		
K, L, M, P, Q						
K	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2375	0	2375	0	0	1-8	1-8
R	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1811	864 / 0	420 / 0	0 / 0	0 / 0	528 / 0	0 / 0
R	1303	643 / 0	286 / 0	0 / 0	0 / 0	375 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FACTORED VERT. FORCE (LBS)	LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-78.0	-78.0	0.06 (1)	10.00
B- C	-1585 / 0	-78.0	-78.0	0.11 (1)	6.25
C- D	-1562 / 0	-78.0	-78.0	0.11 (1)	6.25
D- E	-1347 / 0	-78.0	-78.0	0.04 (1)	6.25
E- F	-1737 / 0	-78.0	-78.0	0.17 (1)	6.25
F- G	-2973 / 0	-78.0	-78.0	0.03 (1)	5.34
G- S	-2911 / 0	-78.0	-78.0	0.11 (1)	5.30
S- H	-2911 / 0	-78.0	-78.0	0.11 (1)	5.30
H- T	-1772 / 0	-78.0	-78.0	0.09 (1)	6.25
T- I	-1772 / 0	-78.0	-78.0	0.09 (1)	6.25
J- I	-2313 / 0	0.0	0.0	0.29 (1)	7.37
R- B	-1646 / 0	0.0	0.0	0.06 (1)	7.81
R- Q	0 / 0	-38.5	-38.5	0.03 (2)	10.00
Q- P	0 / 1234	-38.5	-38.5	0.10 (1)	10.00
P- O	0 / 1182	-38.5	-38.5	0.10 (1)	10.00
O- N	0 / 2975	-38.5	-38.5	0.24 (1)	10.00
N- M	0 / 2975	-38.5	-38.5	0.24 (1)	10.00
M- L	0 / 2911	-38.5	-38.5	0.25 (1)	10.00
L- U	0 / 1772	-38.5	-38.5	0.17 (1)	10.00
U- K	0 / 1772	-38.5	-38.5	0.17 (1)	10.00
K- V	0 / 0	-38.5	-38.5	0.03 (3)	10.00
V- J	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-257 / 0	0.04 (1)
C- P	-84 / 0	0.02 (1)
P- D	0 / 186	0.02 (2)
D- O	0 / 647	0.08 (1)
O- E	0 / 856	0.11 (1)
O- F	-2102 / 0	0.64 (1)
M- F	-68 / 23	0.01 (1)
K- H	-1923 / 0	0.24 (1)
K- I	0 / 2822	0.35 (1)
B- Q	0 / 1295	0.16 (1)
L- G	-2367 / 0	0.04 (1)
M- G	0 / 1295	0.16 (2)
L- H	-107 / 1885	0.23 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
L	15-3-8	-1226	-1226	---	BACK	VERT	TOTAL	---	---
S	17-2-12	-94	-94	---	BACK	VERT	TOTAL	---	---
T	19-2-12	-94	-94	---	BACK	VERT	TOTAL	---	---
U	17-2-12	-55	-70	---	BACK	VERT	TOTAL	---	---
V	19-2-12	-55	-70	---	BACK	VERT	TOTAL	---	---

TOTAL WEIGHT = 2 X 125 = 250 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.29/1.00 (I-J:1) , BC=0.25/1.00 (L-M:1) , WB=0.64/1.00 (F-O:1) , SSI=0.09/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

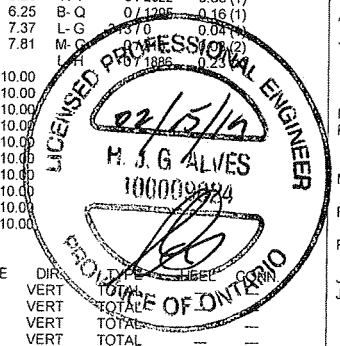
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)



02/6/17

DWG NO. T1903415
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T59S	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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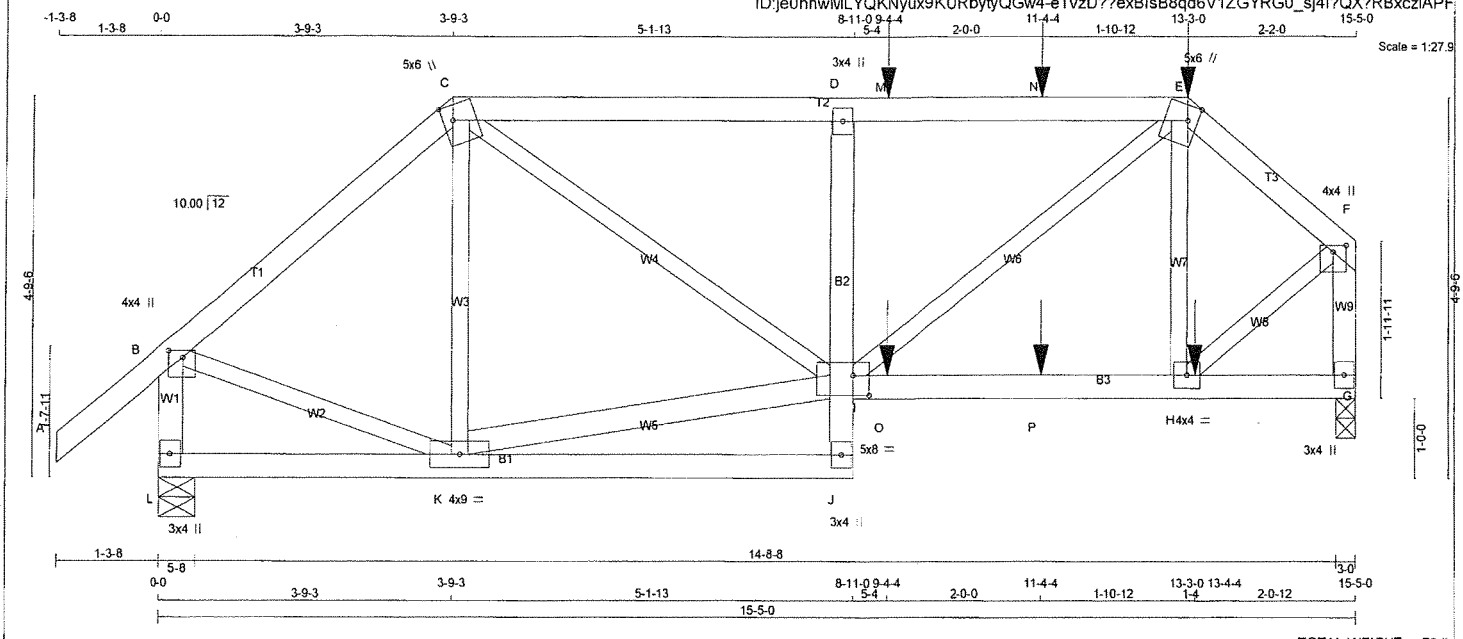

TOTAL WEIGHT = 73 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
J	BMV+p	MT20	3.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1018	0	1018	0	0	5-8	5-8
G	930	0	930	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	770	385 / 0	165 / 0	0 / 0	0 / 0	221 / 0	0 / 0
G	711	332 / 0	168 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
		(PLF)						
A- B	0 / 35	-78.0	-78.0	0.12 (1)	10.00	K- C	-112 / 83	0.04 (1)
B- C	-786 / 0	-78.0	-78.0	0.22 (1)	6.25	I- E	0 / 679	0.17 (1)
C- D	-1055 / 0	-78.0	-78.0	0.35 (1)	5.63	H- E	-202 / 8	0.05 (1)
D- M	-1063 / 0	-78.0	-78.0	0.34 (1)	5.62	B- K	0 / 639	0.16 (1)
M- N	-1063 / 0	-78.0	-78.0	0.34 (1)	5.62	H- F	0 / 657	0.16 (1)
N- E	-1063 / 0	-78.0	-78.0	0.34 (1)	5.62	K- I	0 / 549	0.14 (1)
E- F	-697 / 0	-78.0	-78.0	0.08 (1)	6.25	G- I	0 / 549	0.14 (1)
L- B	-972 / 0	0.0	0.0	0.11 (1)	7.81			
G- F	-918 / 0	0.0	0.0	0.11 (1)	7.81			
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
K- J	0 / 29	-38.5	-38.5	0.20 (3)	10.00			
J- I	0 / 100	0.0	0.0	0.06 (1)	10.00			
I- D	-457 / 0	0.0	0.0	0.06 (1)	7.81			
I- O	0 / 526	-38.5	-38.5	0.22 (2)	10.00			
O- P	0 / 526	-38.5	-38.5	0.22 (2)	10.00			
P- H	0 / 526	-38.5	-38.5	0.22 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	TOP
E	13-3-0	-13	-13	---	BACK	VERT	TOTAL	---	---
H	13-4-4	-3	-4	---	BACK	VERT	TOTAL	---	---
M	9-4-4	-7	-7	---	BACK	VERT	TOTAL	---	---
N	11-4-4	-7	-7	---	BACK	VERT	TOTAL	---	---
O	9-4-4	-11	-14	---	BACK	VERT	TOTAL	---	---
P	11-4-4	-3	-4	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.22/1.00 (H-I:2) , WB=0.17/1.00 (E-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM 7190341G
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T60A	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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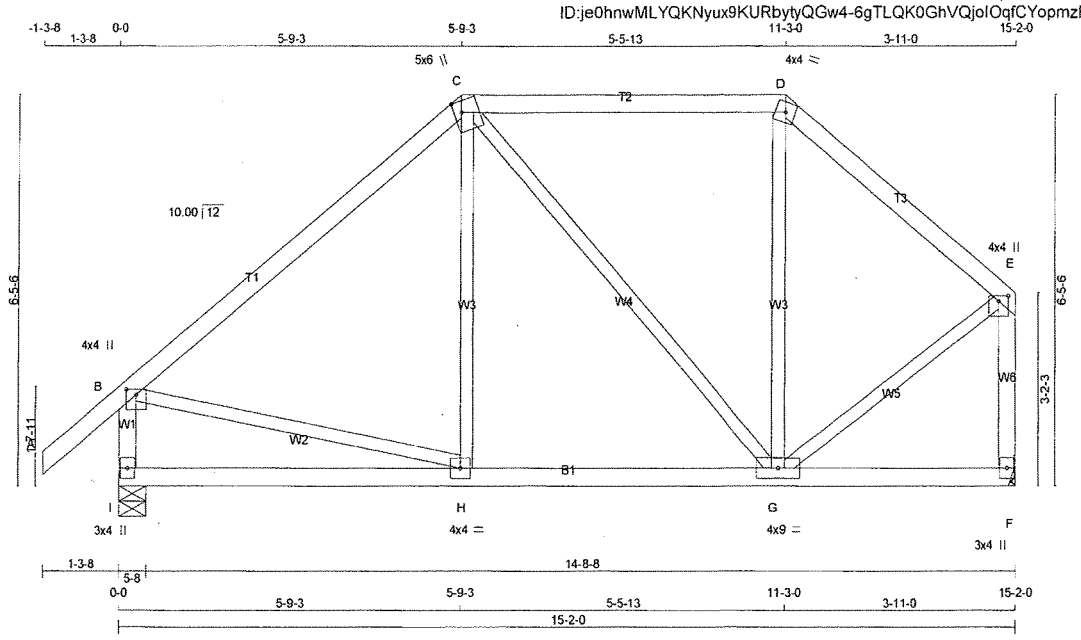

TOTAL WEIGHT = 69 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	992	0	992	0	0	5-8	5-8
F	884	0	884	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	750	377 / 0	159 / 0	0 / 0	0 / 0	214 / 0	0 / 0
F	675	319 / 0	159 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	H- C	0 / 247	0.06 (3)
B- C	-698 / 0	-78.0	-78.0	0.34 (1)	6.25	C- G	-158 / 0	0.22 (1)
C- D	-434 / 0	-78.0	-78.0	0.30 (1)	6.25	G- D	-3 / 136	0.03 (3)
D- E	-566 / 0	-78.0	-78.0	0.16 (1)	6.25	B- H	0 / 549	0.12 (1)
I- B	-900 / 0	0.0	0.0	0.10 (1)	7.81	G- E	0 / 531	0.12 (1)
F- E	-823 / 0	0.0	0.0	0.15 (1)	7.81			
I- H	0 / 0	-38.5	-38.5	0.24 (3)	10.00			
H- G	0 / 538	-38.5	-38.5	0.30 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.30/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM T1903417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T61	3	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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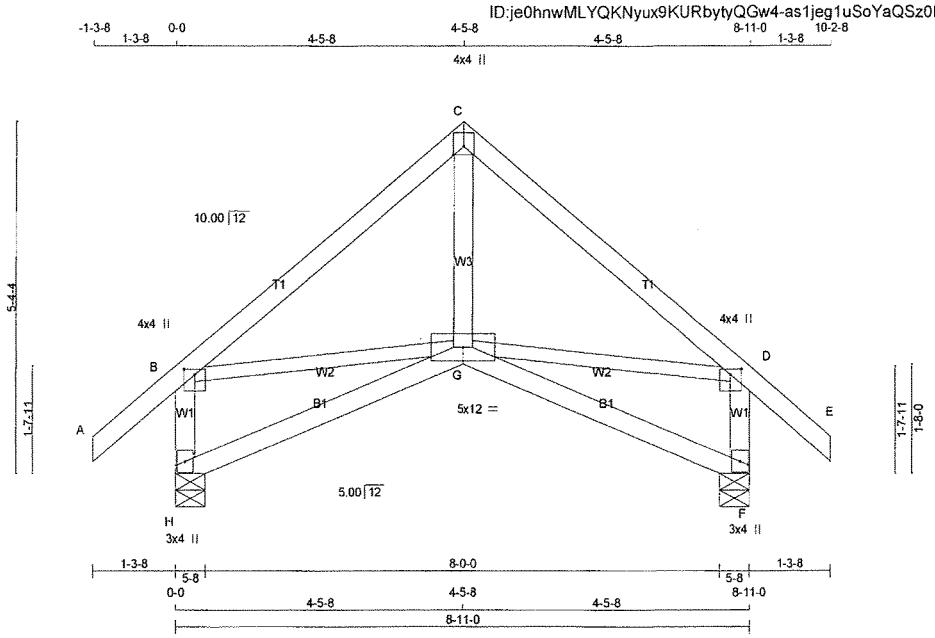


TOTAL WEIGHT = 3 X 42 = 126 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BBWWW-p	MT20	5.0	12.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	627	0	627	0	0	5-8	5-8
F	627	0	627	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	472	245 / 0	94 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	472	245 / 0	94 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	G- C	0 / 271	0.04 (3)
B- C	-451 / 0	-78.0	-78.0	0.20 (1)	6.25	B- G	0 / 348	0.08 (1)
C- D	-451 / 0	-78.0	-78.0	0.20 (1)	6.25	G- D	0 / 348	0.08 (1)
D- E	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
H- B	-542 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-542 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-38.5	-38.5	0.18 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.18/1.00 (F-G:3) , WB=0.08/1.00 (B-G:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

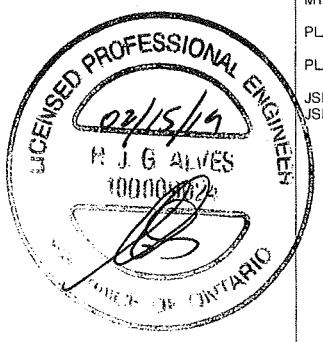
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



BWS NO. IAM T1903418
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T90TCX	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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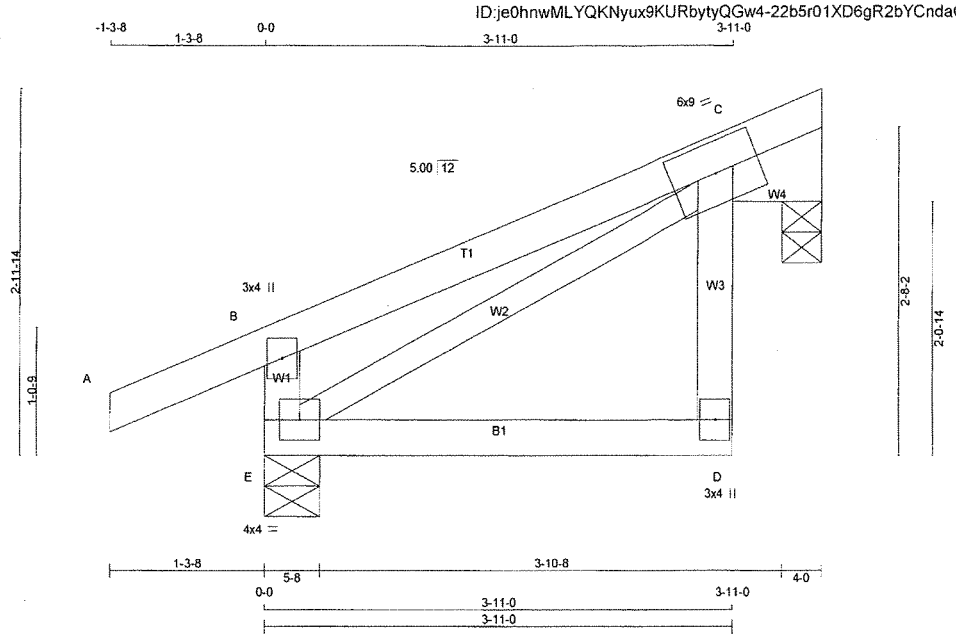

TOTAL WEIGHT = 2 X 20 = 40 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	228	0	228	0	0	4-0	4-0
E	333	0	333	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	174	82 / 0	41 / 0	0 / 0	0 / 0	51 / 0	0 / 0
E	247	139 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-78.0	-78.0	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-78.0	-78.0	0.20 (1)	10.00			
D- C	0 / 96	0.0	0.0	0.02 (3)	10.00			
E- B	-258 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-38.5	-38.5	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.14/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H J G ALVES
100009024
PROVINCE OF ONTARIO

BWS NO. TAM T1903419
STRUCTURAL
COPY DRAWN ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T91TCX	5	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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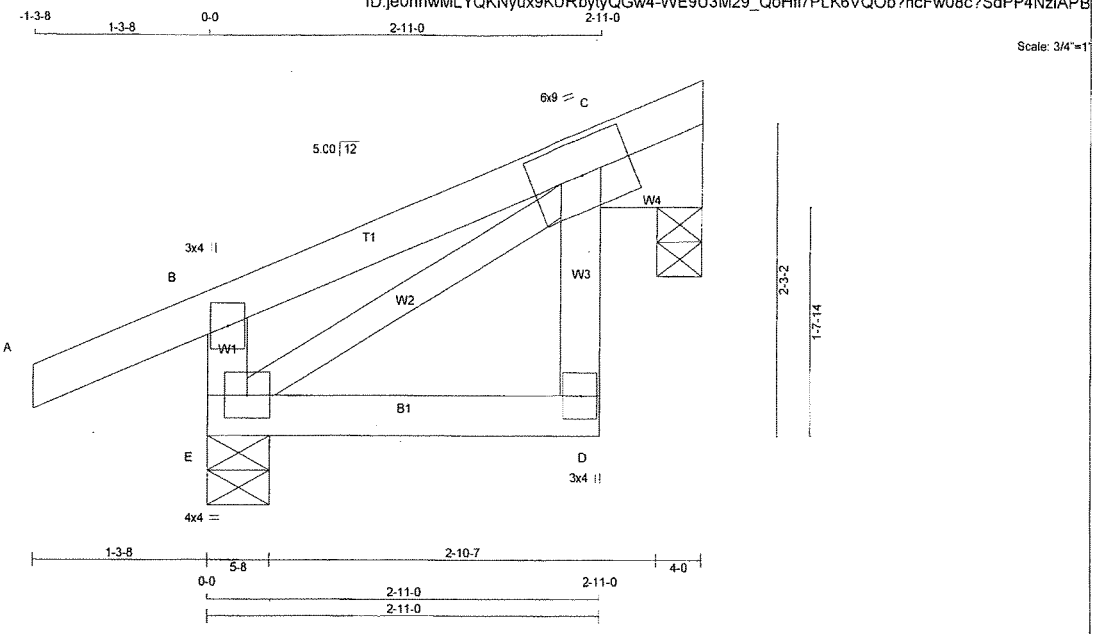


TOTAL WEIGHT = 5 X 16 = 81 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	170	0	170	0	0	4-0	4-0
E	275	0	275	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	130	61 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0
E	203	118 / 0	31 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

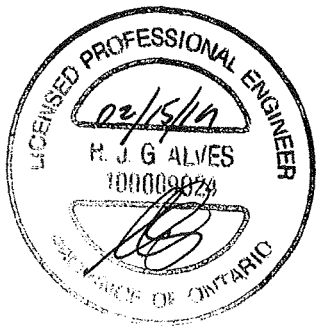
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-78.0	-78.0	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-78.0	-78.0	0.11 (1)	10.00			
D- C	0 / 71	0.0	0.0	0.01 (3)	10.00			
E- B	-219 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-38.5	-38.5	0.08 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (B-C:1) , BC=0.08/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM T1903420
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T93TCX	12	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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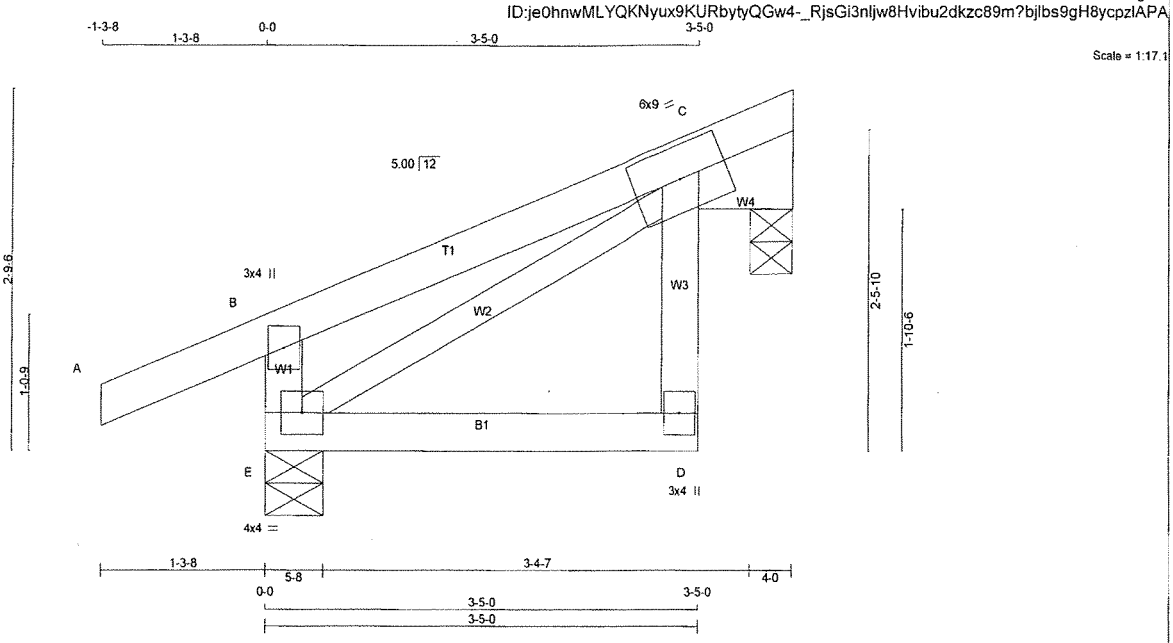


TOTAL WEIGHT = 12 X 18 = 216 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	199	0	199	0	0	4-0	4-0
E	304	0	304	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	152	72 / 0	36 / 0	0 / 0	0 / 0	44 / 0	0 / 0
E	225	128 / 0	36 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

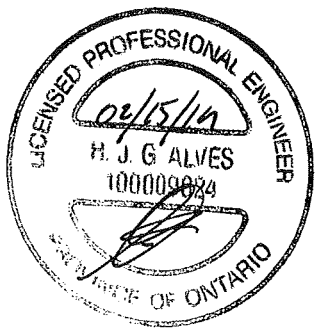
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-78.0	-78.0	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-78.0	-78.0	0.15 (1)	10.00			
D- C	0 / 84	0.0	0.0	0.01 (3)	10.00			
E- B	-238 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-38.5	-38.5	0.11 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.11/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

ENGINEERING BY 7903421
STRUCTURAL
CITY OF KINGSTON

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T430	4	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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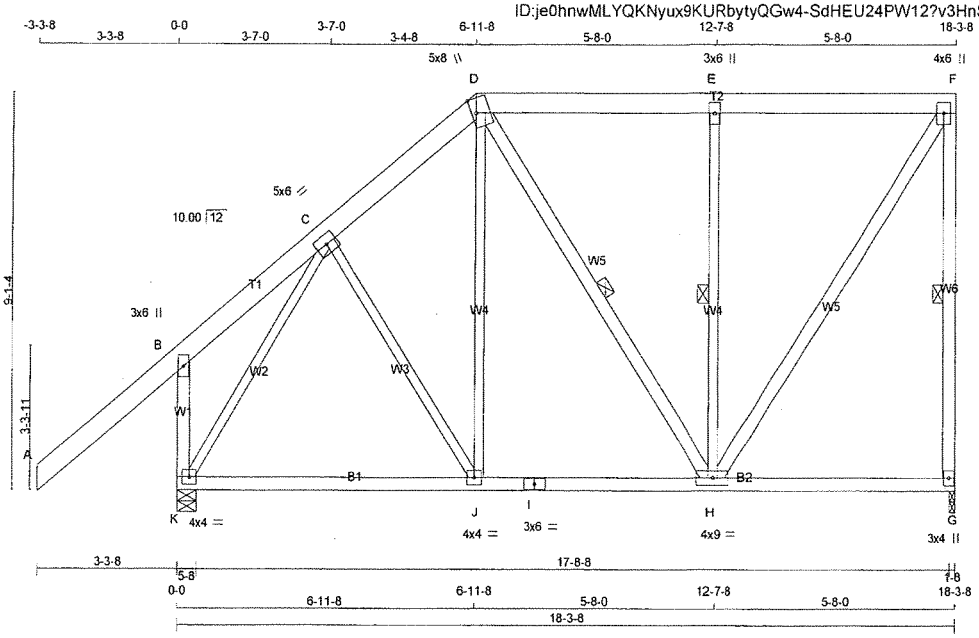


TOTAL WEIGHT = 4 X 125 = 499 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1040	0	1040	0	0	1-8	1-8
K	1359	0	1359	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	371 / 0	192 / 0	0 / 0	0 / 0	234 / 0	0 / 0
K	1017	542 / 0	192 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

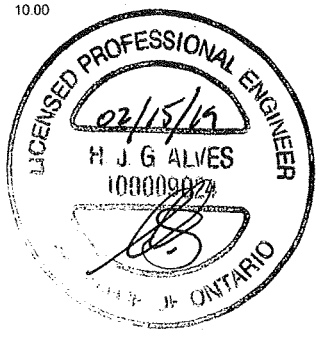
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 86	-78.0	-78.0	0.32 (1)	10.00	
B- C	-90 / 0	-78.0	-78.0	0.31 (1)	6.25	
C- D	-670 / 0	-78.0	-78.0	0.09 (1)	6.25	
D- E	-506 / 0	-78.0	-78.0	0.19 (1)	6.25	
E- F	-507 / 0	-78.0	-78.0	0.19 (1)	6.25	
G- F	-949 / 0	0.0	0.0	0.35 (1)	6.25	
K- B	-543 / 0	0.0	0.0	0.09 (1)	7.81	
K- J	0 / 424	-38.5	-38.5	0.36 (2)	10.00	
J- I	0 / 517	-38.5	-38.5	0.38 (2)	10.00	
I- H	0 / 517	-38.5	-38.5	0.38 (2)	10.00	
H- G	0 / 0	-38.5	-38.5	0.20 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 177	0.04 (2)
J- D	0 / 238	0.06 (3)
D- H	-55 / 0	0.03 (3)
H- E	-537 / 0	0.28 (1)
H- F	0 / 926	0.15 (1)
K- C	-822 / 0	0.64 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.38/1.00 (H-J:2) , WB=0.64/1.00 (C-K:1) , SSI=0.17/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (H) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

DWG NO. TAM 1903422
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	T440	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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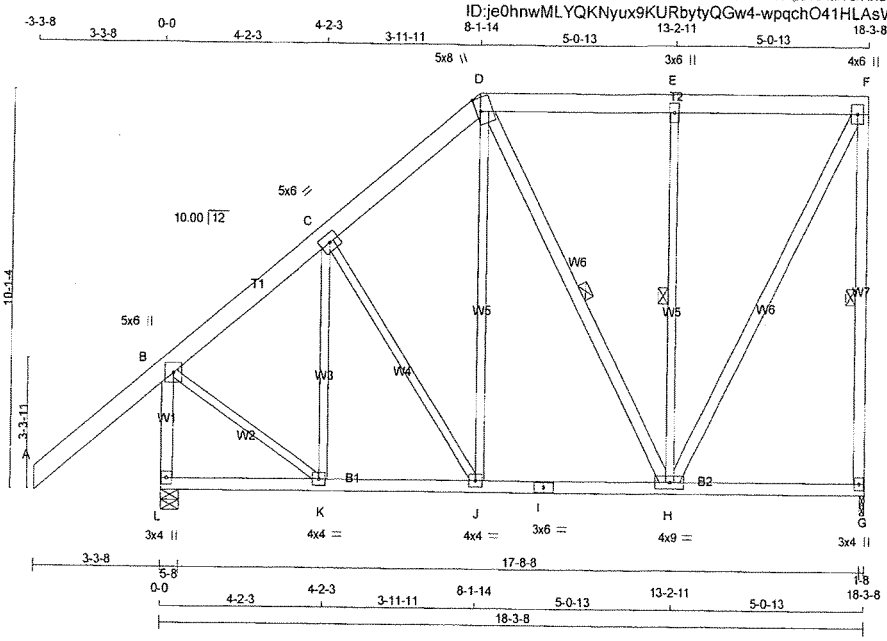


TOTAL WEIGHT = 2 X 134 = 267 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1040	0	1040	0	0	1-8	1-8
L	1359	0	1359	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	371 / 0	192 / 0	0 / 0	0 / 0	234 / 0	0 / 0
L	1017	542 / 0	192 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

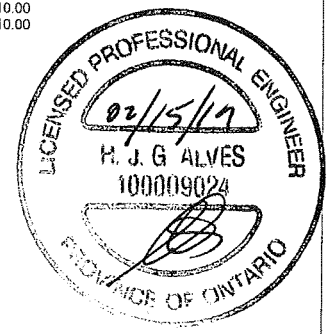
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 86	-78.0	-78.0	0.32 (1)	10.00		K- C	-165 / 21	0.12 (1)
B- C	-717 / 0	-78.0	-78.0	0.31 (1)	6.25		C- J	-52 / 0	0.06 (2)
C- D	-625 / 0	-78.0	-78.0	0.10 (1)	6.25		J- D	0 / 249	0.06 (3)
D- E	-422 / 0	-78.0	-78.0	0.14 (1)	6.25		D- H	-119 / 0	0.07 (1)
E- F	-422 / 0	-78.0	-78.0	0.14 (1)	6.25		H- E	-476 / 0	0.32 (1)
G- F	-962 / 0	0.0	0.0	0.46 (1)	6.25		H- F	0 / 908	0.15 (1)
L- B	-1294 / 0	0.0	0.0	0.22 (1)	7.10		B- K	0 / 605	0.14 (1)
L- K	0 / 0	-38.5	-38.5	0.12 (3)	10.00				
K- J	0 / 499	-38.5	-38.5	0.19 (2)	10.00				
J- I	0 / 477	-38.5	-38.5	0.25 (2)	10.00				
I- H	0 / 477	-38.5	-38.5	0.25 (2)	10.00				
H- G	0 / 0	-38.5	-38.5	0.19 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.46/1.00 (F-G:1) , BC=0.25/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM 7903423
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB1	1	2	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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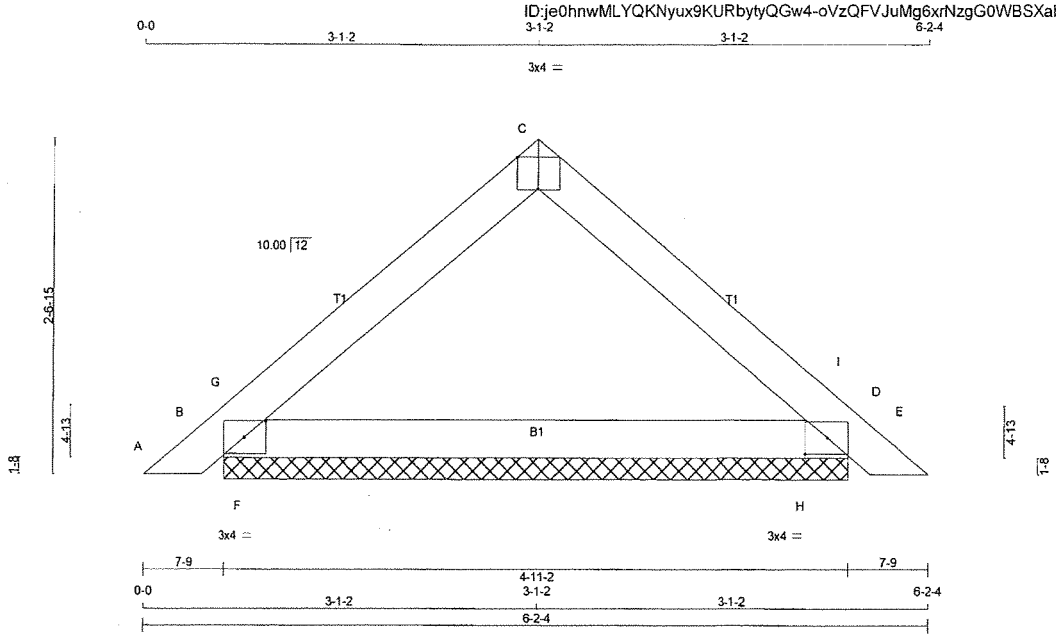


Scale = 1:17.1

TOTAL WEIGHT = 2 X 15 = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	329	0	329	0	0	4-11-2	4-11-2
D	329	0	329	0	0	4-11-2	4-11-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	248	126 / 0	52 / 0	0 / 0	0 / 0	70 / 0	0 / 0
D	248	126 / 0	52 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

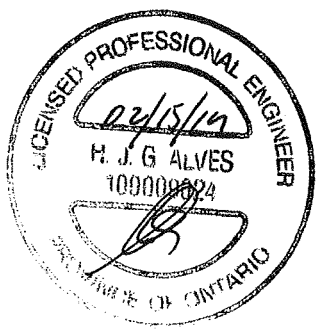
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 12	-78.0	-78.0	0.01 (1)	10.00	F- G	0 / 295	0.00 (1)
B- G	-362 / 0	-78.0	-78.0	0.05 (3)	6.25	H- I	0 / 295	0.00 (1)
G- C	-156 / 0	-78.0	-78.0	0.05 (1)	6.25			
C- I	-156 / 0	-78.0	-78.0	0.05 (1)	6.25			
I- D	-362 / 0	-78.0	-78.0	0.05 (3)	6.25			
D- E	0 / 12	-78.0	-78.0	0.01 (1)	10.00			
B- F	0 / 131	-38.5	-38.5	0.04 (3)	10.00			
F- H	0 / 131	-38.5	-38.5	0.07 (2)	10.00			
H- D	0 / 131	-38.5	-38.5	0.04 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-G:1) , BC=0.07/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.08/1.00 (B-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.03 (D) (INPUT = 1.00)

DWG NO. TAM T1903424
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB2	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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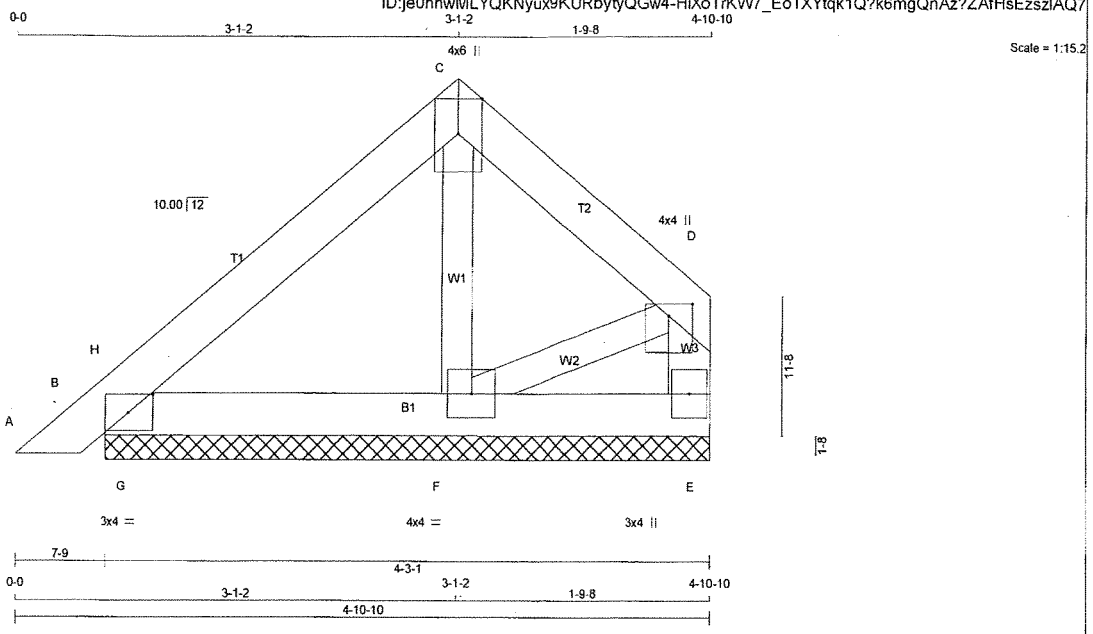

Scale = 1:15.2

TOTAL WEIGHT = 16 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	135	0	135	0	0	4-3-1	4-3-1
B	208	0	208	0	0	4-3-1	4-3-1
F	195	0	195	0	0	4-3-1	4-3-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	100	58 / 0	15 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	153	89 / 0	23 / 0	0 / 0	0 / 0	41 / 0	0 / 0
F	155	54 / 0	51 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	F- C	-83 / 0	0.01 (1)
B- H	-40 / 0	-78.0	-78.0	0.01 (1)	6.25	F- D	0 / 44	0.01 (1)
H- C	-57 / 0	-78.0	-78.0	0.06 (1)	6.25	G- H	-131 / 17	0.00 (1)
C- D	-54 / 0	-78.0	-78.0	0.04 (1)	6.25			
E- D	-119 / 0	0.0	0.0	0.01 (1)	7.81			
B- G	0 / 41	-38.5	-38.5	0.06 (1)	10.00			
G- F	0 / 41	-38.5	-38.5	0.06 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.05 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-H:1) , BC=0.06/1.00 (F-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.12/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

BWS NO. IAM 7903425
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB3	6	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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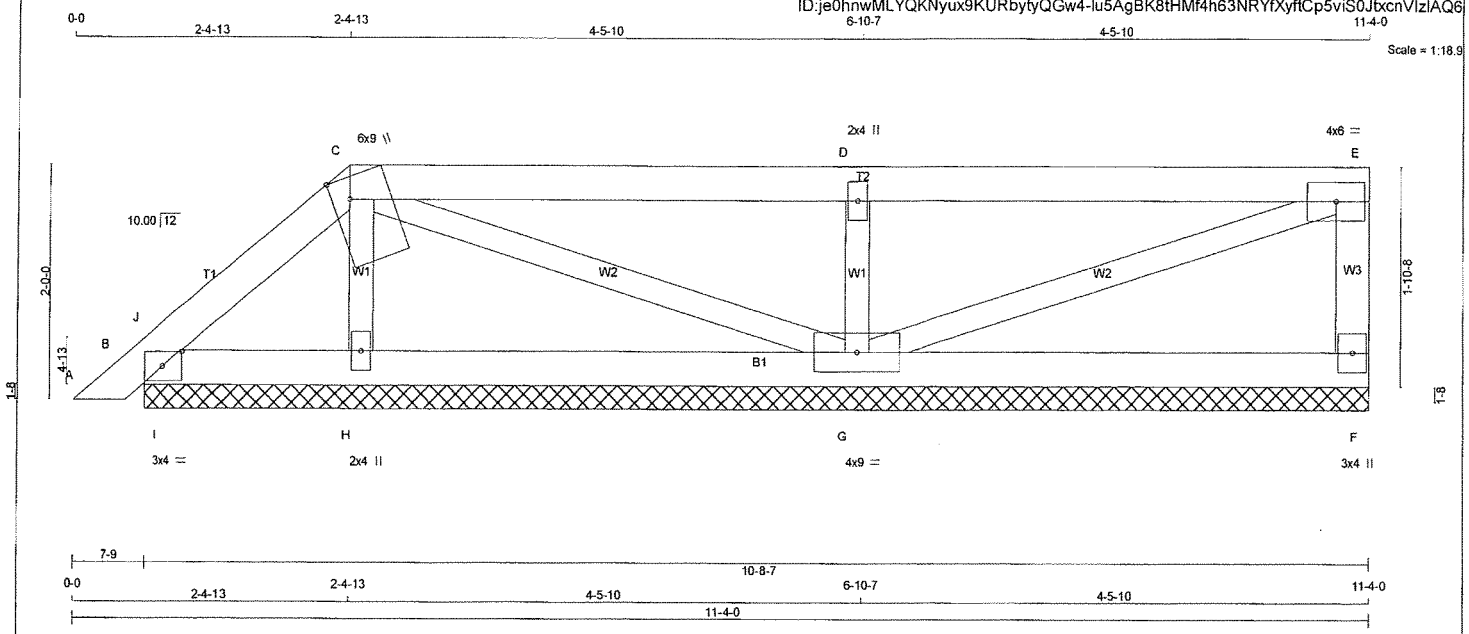

TOTAL WEIGHT = 6 X 37 = 219 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	201	0	201	0	0	10-8-7	10-8-7
B	154	0	154	0	0	10-8-7	10-8-7
H	293	0	293	0	0	10-8-7	10-8-7
G	642	0	642	0	0	10-8-7	10-8-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	154	72 / 0	37 / 0	0 / 0	0 / 0	45 / 0	0 / 0
B	110	75 / 0	8 / 0	0 / 0	0 / 0	27 / 0	0 / 0
H	231	88 / 0	71 / 0	0 / 0	0 / 0	72 / 0	0 / 0
G	488	238 / 0	109 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02	(1)	10.00	H- C	-148 / 0	0.02 (1)
B- J	-25 / 0	-78.0	-78.0	0.01	(1)	6.25	C- G	-24 / 0	0.01 (1)
J- C	-45 / 0	-78.0	-78.0	0.03	(1)	6.25	G- D	-435 / 0	0.06 (1)
C- D	0 / 0	-78.0	-78.0	0.26	(1)	10.00	G- E	0 / 0	0.00 (1)
D- E	0 / 0	-78.0	-78.0	0.26	(1)	10.00	I- J	-97 / 0	0.00 (1)
F- E	-133 / 0	0.0	0.0	0.01	(1)	7.81			
B- I	0 / 32	-38.5	-38.5	0.04	(1)	10.00			
I- H	0 / 32	-38.5	-38.5	0.07	(2)	10.00			
H- G	0 / 22	-38.5	-38.5	0.15	(3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.15	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (D-E:1) , BC=0.15/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 77903426
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB4	6	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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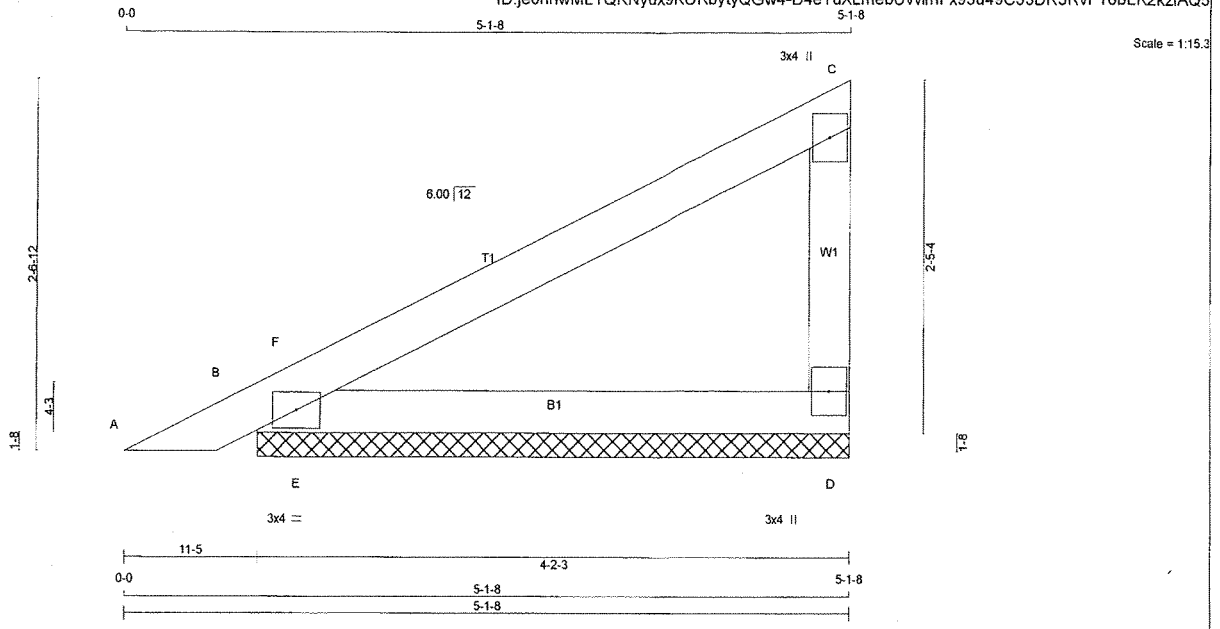


TOTAL WEIGHT = 6 X 13 = 80 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	244	0	244	0	0	4-2-3	4-2-3
B	312	0	312	0	0	4-2-3	4-2-3

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	186	88 / 0	44 / 0	0 / 0	0 / 0	54 / 0	0 / 0
B	234	125 / 0	44 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

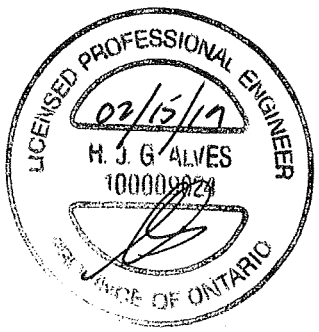
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 14	-78.0	-78.0	0.04 (1)	10.00	E- F	-149 / 98	0.00 (1)
B- F	-59 / 0	-78.0	-78.0	0.07 (3)	6.25			
F- C	0 / 6	-78.0	-78.0	0.19 (1)	10.00			
D- C	-149 / 0	0.0	0.0	0.02 (1)	7.81			
B- E	0 / 0	-38.5	-38.5	0.11 (1)	10.00			
E- D	0 / 0	-38.5	-38.5	0.16 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-F:1) , BC=0.16/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM T1903427
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB5	2	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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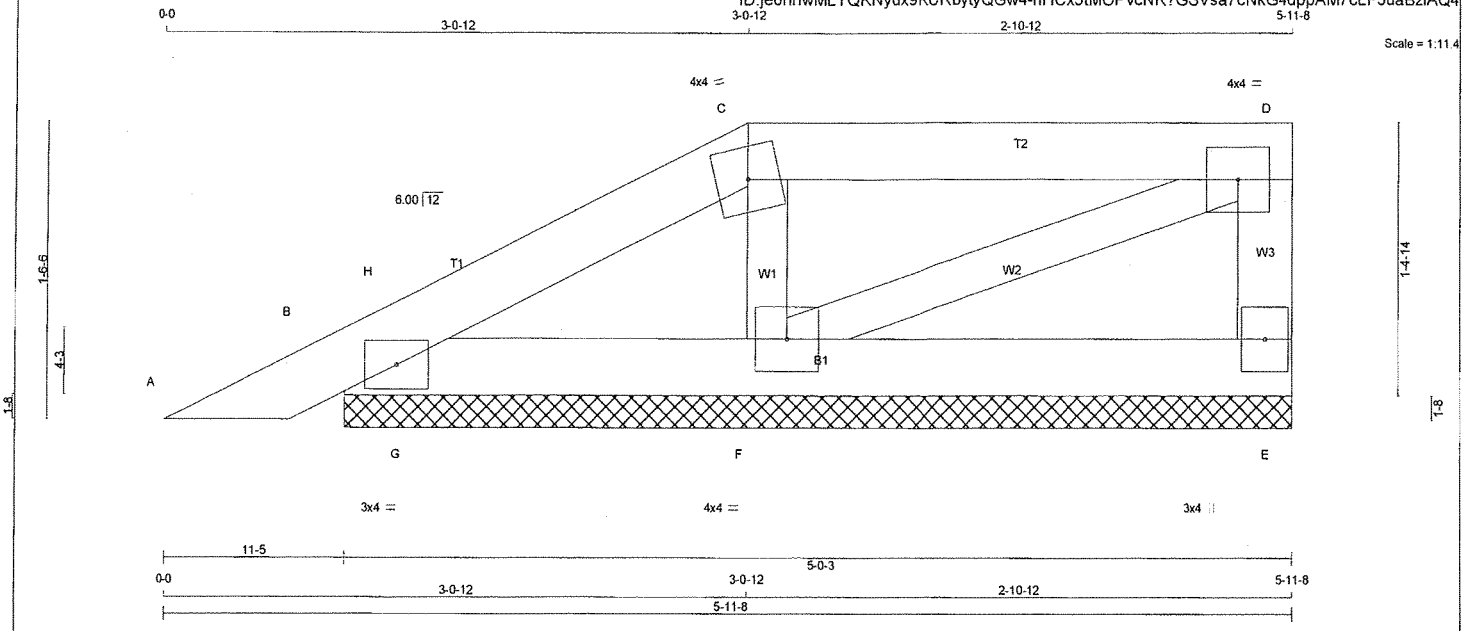


TOTAL WEIGHT = 2 X 17 = 33 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	156	0	156	0	0	5-0-3	5-0-3
B	189	0	189	0	0	5-0-3	5-0-3
F	308	0	308	0	0	5-0-3	5-0-3

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	117	61 / 0	24 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	138	86 / 0	16 / 0	0 / 0	0 / 0	35 / 0	0 / 0
F	239	101 / 0	65 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

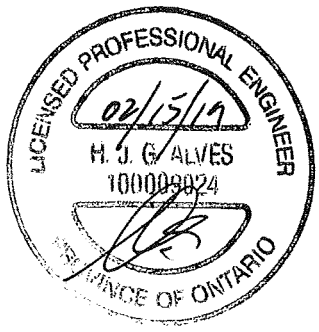
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-78.0	-78.0	0.04 (1)	10.00	F- C	-171 / 0	0.02 (1)
B- H	-30 / 0	-78.0	-78.0	0.01 (3)	6.25	F- D	0 / 7	0.00 (1)
H- C	-24 / 0	-78.0	-78.0	0.04 (1)	6.25	G- H	-72 / 1	0.00 (1)
C- D	-6 / 0	-78.0	-78.0	0.11 (1)	10.00			
E- D	-115 / 0	0.0	0.0	0.01 (1)	7.81			
B- G	0 / 22	-38.5	-38.5	0.05 (1)	10.00			
G- F	0 / 22	-38.5	-38.5	0.06 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.06 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-D:1) , BC=0.06/1.00 (F-G:2) , WB=0.02/1.00 (C-F:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.03 (C) (INPUT = 1.00)

DWG NO. TAM 1903428
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB6	3	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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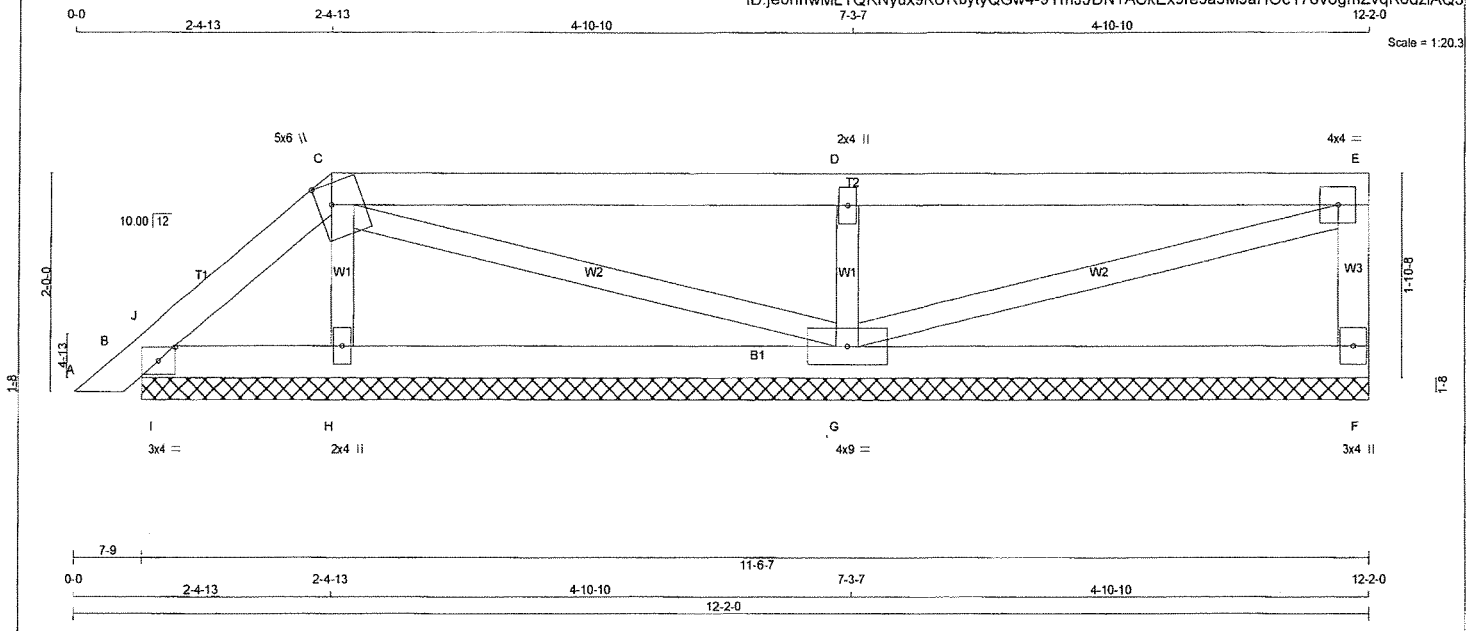


TOTAL WEIGHT = 3 X 39 = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	219	0	219	0	0	11-6-7	11-6-7
B	150	0	150	0	0	11-6-7	11-6-7
H	317	0	317	0	0	11-6-7	11-6-7
G	700	0	700	0	0	11-6-7	11-6-7

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	168	78 / 0	40 / 0	0 / 0	0 / 0	49 / 0	0 / 0
B	106	75 / 0	6 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	250	94 / 0	77 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	532	260 / 0	119 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

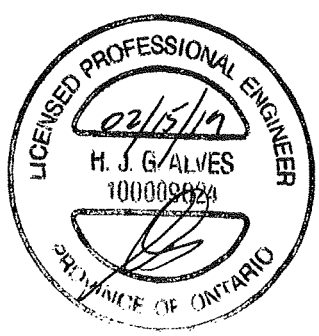
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	H- C	-160 / 0	0.02 (1)
B- J	-19 / 8	-78.0	-78.0	0.01 (1)	6.25	C- G	-23 / 0	0.01 (1)
J- C	-46 / 0	-78.0	-78.0	0.02 (1)	6.25	G- D	-476 / 0	0.07 (1)
C- D	0 / 0	-78.0	-78.0	0.32 (1)	10.00	G- E	0 / 0	0.00 (1)
D- E	0 / 0	-78.0	-78.0	0.32 (1)	10.00	I- J	-106 / 0	0.00 (1)
F- E	-145 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 32	-38.5	-38.5	0.04 (1)	10.00			
I- H	0 / 32	-38.5	-38.5	0.09 (2)	10.00			
H- G	0 / 22	-38.5	-38.5	0.18 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1) , BC=0.18/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM 7903429
STRUCTURAL
CONSIGNMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB7	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

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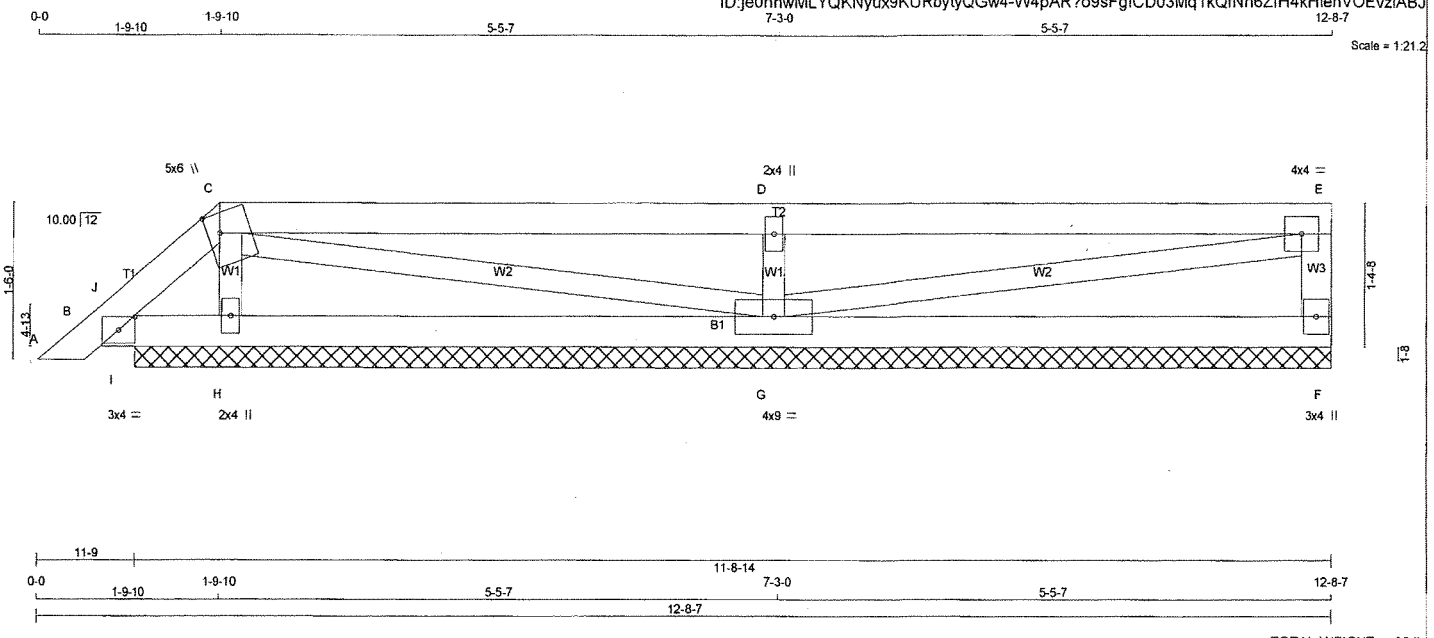

TOTAL WEIGHT = 39 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	239	0	239	0	0	11-8-14	11-8-14
H	443	0	443	0	0	11-8-14	11-8-14
G	767	0	767	0	0	11-8-14	11-8-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	184	83 / 0	46 / 0	0 / 0	0 / 0	54 / 0	0 / 0
H	336	166 / 0	74 / 0	0 / 0	0 / 0	96 / 0	0 / 0
G	584	281 / 0	134 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	H- C	-249 / 0	0.04 (1)
B- J	0 / 138	-78.0	-78.0	0.04 (1)	10.00	C- G	-9 / 60	0.01 (1)
J- C	-27 / 78	-78.0	-78.0	0.03 (1)	6.25	G- D	-532 / 0	0.07 (1)
C- D	-3 / 32	-78.0	-78.0	0.40 (1)	10.00	G- E	-32 / 3	0.02 (1)
D- E	-3 / 31	-78.0	-78.0	0.40 (1)	10.00	I- J	-129 / 0	0.00 (1)
F- E	-155 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	-66 / 13	-38.5	-38.5	0.02 (1)	6.25			
I- H	-66 / 13	-38.5	-38.5	0.11 (3)	6.25			
H- G	-91 / 12	-38.5	-38.5	0.23 (3)	6.25			
G- F	0 / 0	-38.5	-38.5	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (C-D:1) , BC=0.23/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

BWS NO. TAM 7903430
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400998	PB8	1	1	BLOCK 1 FREEHOLDUNITS 1 - 4 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Feb 14 17:06:51 2019 Page 1
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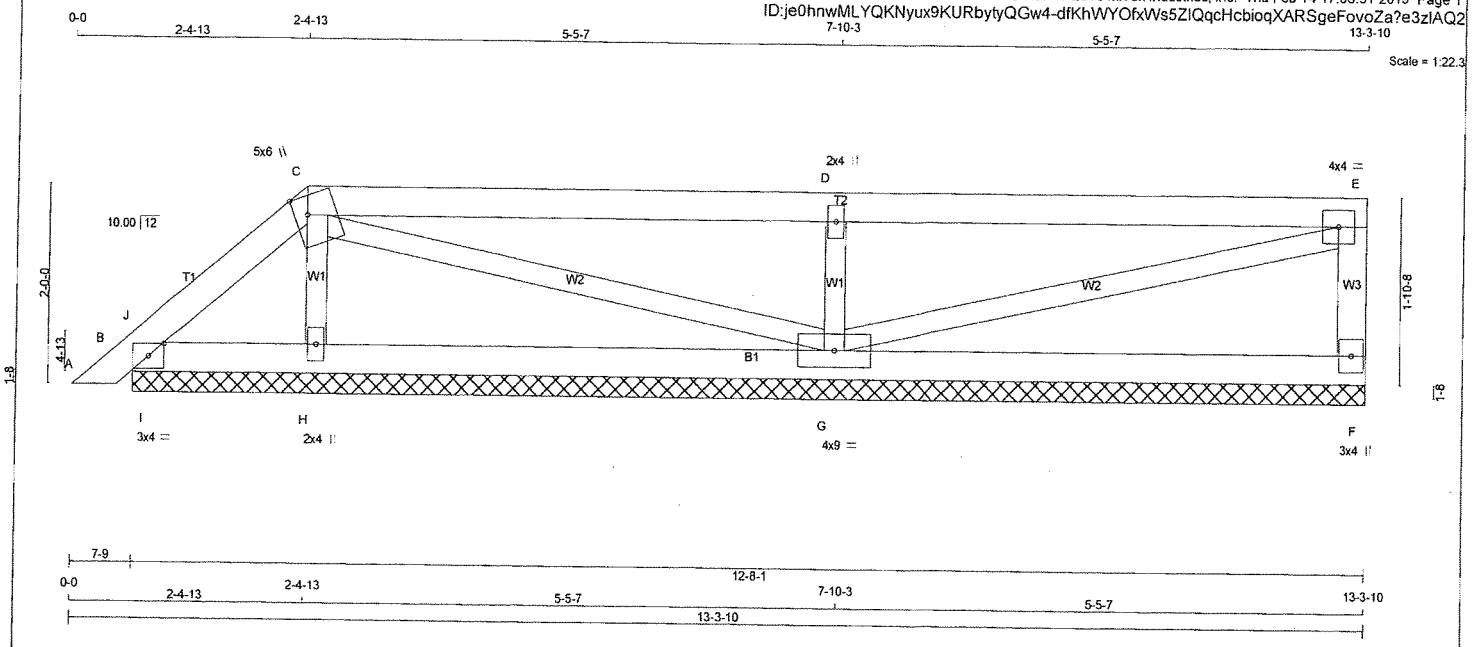


TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	245	0	245	0	0	12-8-1	12-8-1
B	143	0	143	0	0	12-8-1	12-8-1
H	351	0	351	0	0	12-8-1	12-8-1
G	780	0	780	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	187	87 / 0	45 / 0	0 / 0	0 / 0	55 / 0	0 / 0
B	100	76 / 0	2 / 0	0 / 0	0 / 0	23 / 0	0 / 0
H	277	103 / 0	87 / 0	0 / 0	0 / 0	87 / 0	0 / 0
G	593	289 / 0	133 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00
B- J	-10 / 21	-78.0	-78.0	0.02 (2)	6.25
J- C	-48 / 0	-78.0	-78.0	0.02 (1)	6.25
C- D	0 / 0	-78.0	-78.0	0.39 (1)	10.00
D- E	0 / 0	-78.0	-78.0	0.39 (1)	10.00
F- E	-162 / 0	0.0	0.0	0.02 (1)	7.81
B- I	0 / 32	-38.5	-38.5	0.04 (1)	10.00
I- H	0 / 32	-38.5	-38.5	0.11 (3)	10.00
H- G	0 / 21	-38.5	-38.5	0.23 (3)	10.00
G- F	0 / 0	-38.5	-38.5	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-176 / 0	0.03 (1)
C- G	-22 / 0	0.01 (1)
G- D	-531 / 0	0.08 (1)
G- E	0 / 0	0.00 (1)
I- J	-120 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
02/15/19
H. J. GALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (D-E:1) , BC=0.23/1.00 (G-H:3) , WB=0.08/1.00 (D-G:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

DWG NO. TAM 7903431
STRUCTURAL
CONSIDERATIONS ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

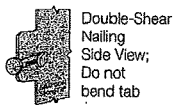
Installation:

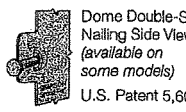
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

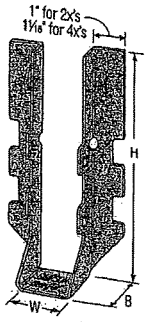
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.


Double-Shear Nailing Top View

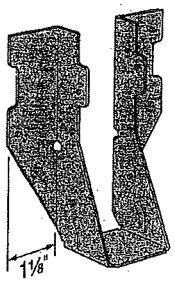

Double-Shear Nailing Side View; Do not bend tab


Dome Double-Shear Nailing Side View *(available on some models)*

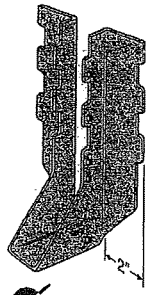
U.S. Patent 5,603,580



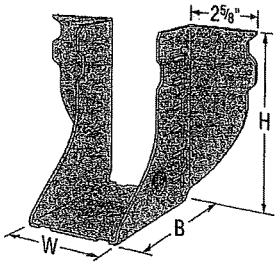
LUS28



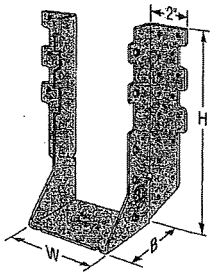
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)

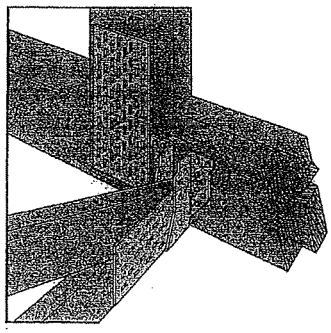


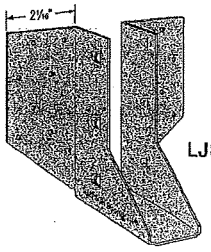
HGUS28-2



HHUS210-2

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)

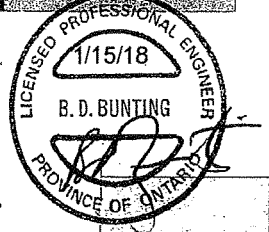




LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®



HHUS/HGUS

See Hanger Options information on pp. 125–127.

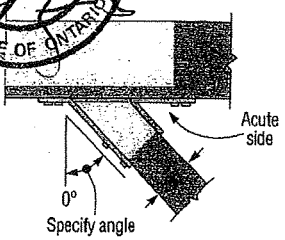
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2 1⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¼	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3 15⁄16	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7 1⁄16	3	6 1⁄16	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7 13⁄16	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁵⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⁵⁄₁₆	7⅝	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁵⁄₁₆	7⅝	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⁵⁄₁₆	9³⁄₁₆	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⁹⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⁹⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⁹⁄₁₆	8¾	2	5⅞	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

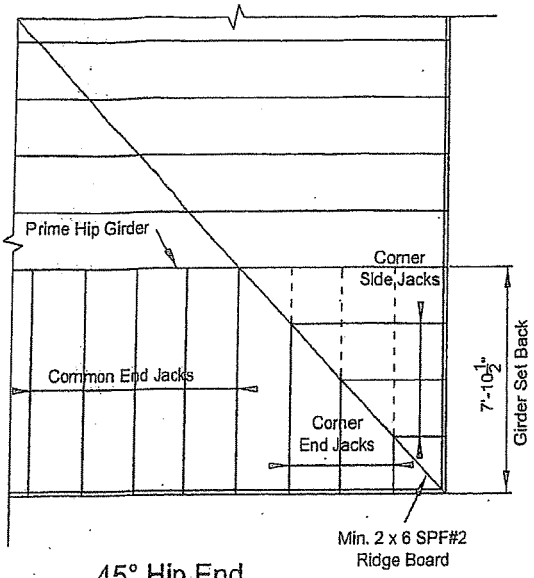
See footnotes on p. 258.

LUMBER SPECIFICATION

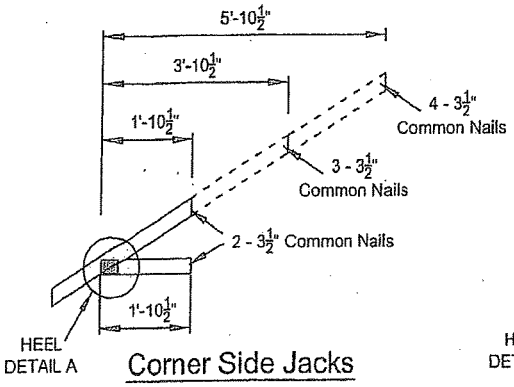
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

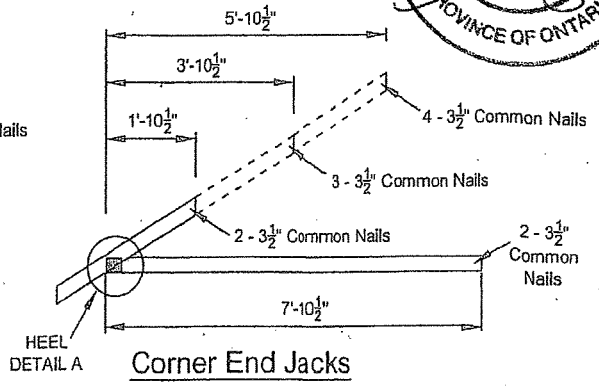
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F



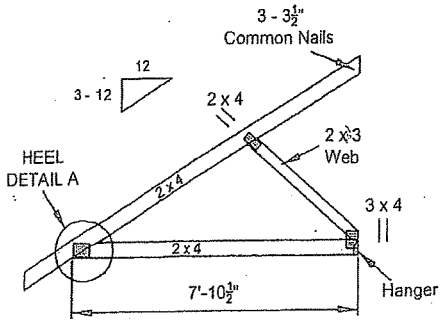
45° Hip End



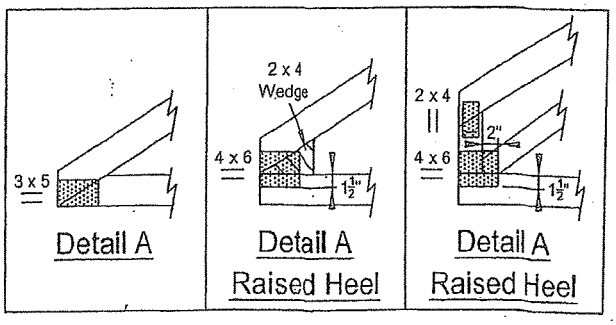
Corner Side Jacks



Corner End Jacks

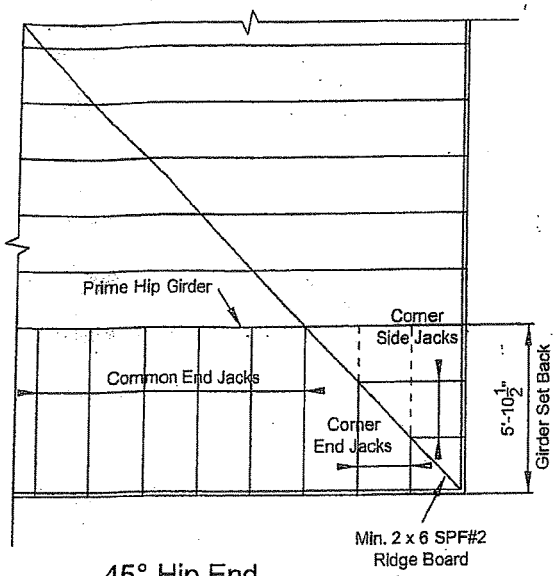


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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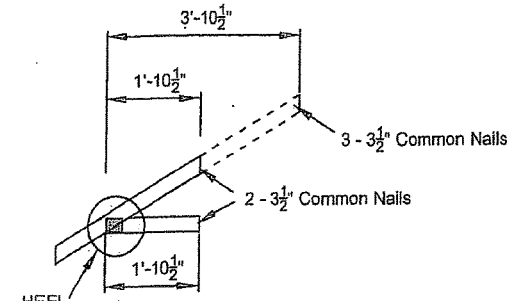
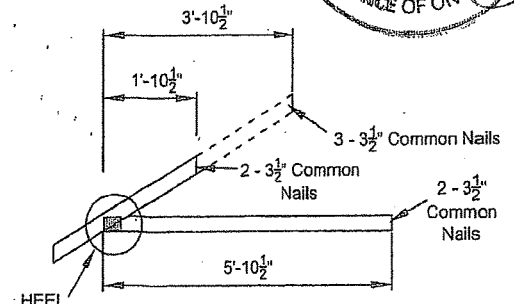
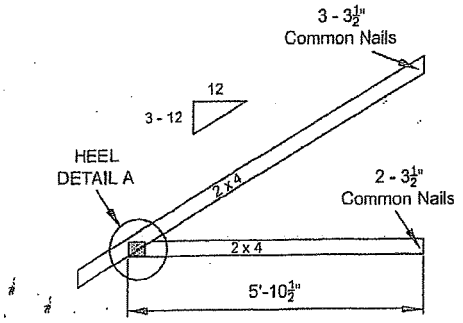


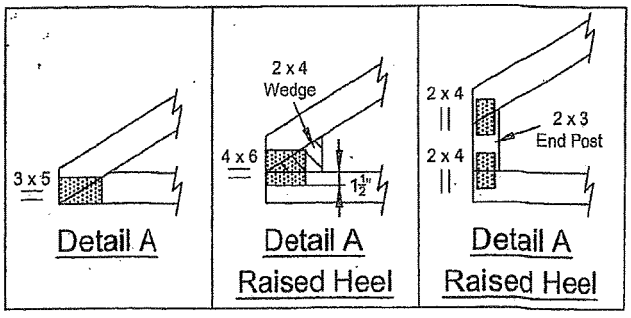
Corner Side Jacks



Corner End Jacks



Common End Jacks

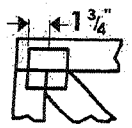


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

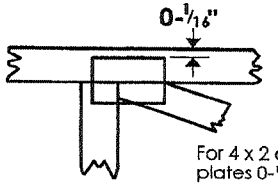
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

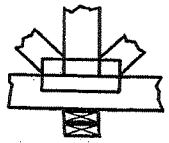
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

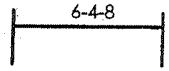


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

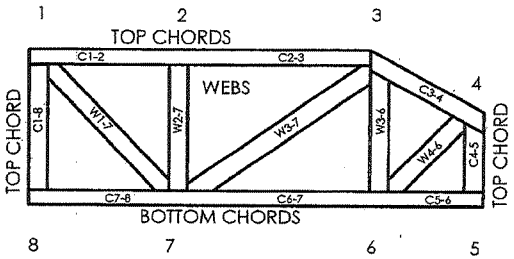
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

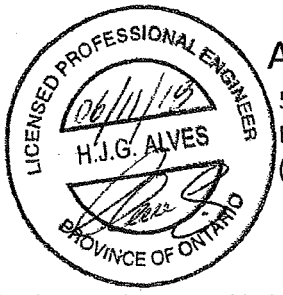
MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018